Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
					J1910495
L260933	T04	ROOF TRUSS	1	1	
					Job Reference (optional)

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NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 683 lb uplift at joint 2 and 683 lb uplift at joint 7.
- 7) Girder carries hip end with 7-0-0 end setback.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

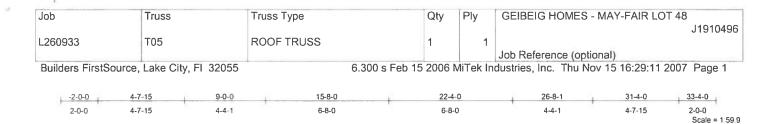
Vert: 1-3=-54, 3-6=-118(F=-64), 6-8=-54, 2-11=-10, 9-11=-22(F=-12), 7-9=-10

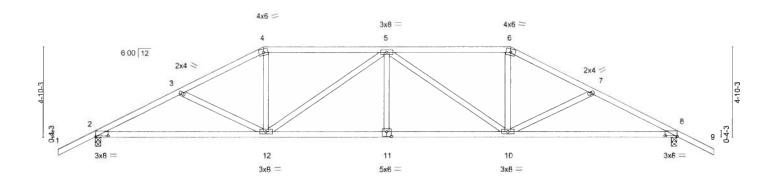
Concentrated Loads (lb)

Vert: 11=-411(F) 9=-411(F)

Justinum Lamin Francis Indiana (Indiana) or Palaria India Indiana Palaria (Indiana) Indiana Francis Indiana (Indiana)







	9-0-0			6-8-	U	t	9-8-0			9-0-0		
Plate Of	fsets (X,Y	'): [2:0-8-0,0-0-10], [8	3:0-8-0,0-0	0-10], [1	1:0-3-0,0	0-3-0]					,	
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defi	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	-0.15	8-10	>999	3 60	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	ВС	0.54	Vert(TL)	-0.29	8-10	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.51	Horz(TL)	0.09	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 160 lb	

15-8-0

LUMBER	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2

LUMBED

WEBS

2 X 4 SYP No.3

BRACING

22-4-0

TOP CHORD Structural wood sheathing directly applied or

4-7-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 7-6-3 oc bracing.

31-4-0

REACTIONS (lb/size) 2=1109/0-3-8, 8=1109/0-3-8

9-0-0

Max Horz 2=89(load case 6)

Max Uplift 2=-271(load case 6), 8=-271(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1832/959, 3-4=-1607/860, 4-5=-1410/832, 5-6=-1410/832,

6-7=-1607/860, 7-8=-1832/959, 8-9=0/47

BOT CHORD 2-12=-690/1571, 11-12=-690/1715, 10-11=-690/1715, 8-10=-690/1571

3-12=-198/193, 4-12=-142/418, 5-12=-465/206, 5-11=0/158, 5-10=-465/206, **WEBS**

6-10=-142/418, 7-10=-198/193

JOINT STRESS INDEX

2 = 0.65, 3 = 0.33, 4 = 0.68, 5 = 0.56, 6 = 0.68, 7 = 0.33, 8 = 0.65, 10 = 0.56, 11 = 0.40 and 12 = 0.56

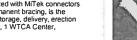
NOTES

1) Unbalanced roof live loads have been considered for this design.

B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48	
					J1	1910496
L260933	T05	ROOF TRUSS	1	1	25 2	
L					Job Reference (optional)	

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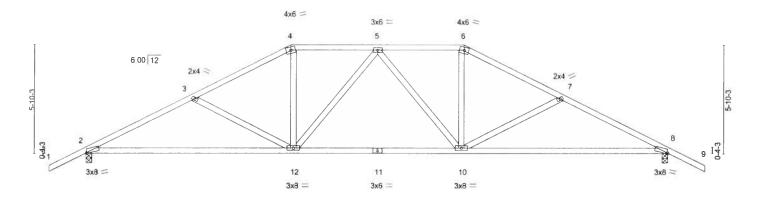
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 271 lb uplift at joint 2 and 271 lb uplift at joint 8.

LOAD CASE(S) Standard









	11-0-0					9-4-0						
Plate Of	ffsets (X,Y): [2:0-0-10,Edge],	[8:0-0-10,E	dge]								
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	-0.31	8-10	>999	360	MT20	244/190

20-4-0

L Т 0.66 **TCDL** 7.0 Lumber Increase 1.25 BC Vert(TL) -0.57 8-10 >654 240 Rep Stress Incr 0.24 **BCLL** 10.0 YES WB Horz(TL) 0.08 8 n/a n/a **BCDL** 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 158 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

4-6-1 oc purlins.

BOT CHORD Rigid ceiling directly applied or 7-6-2 oc

bracing.

REACTIONS (lb/size) 2=1109/0-3-8, 8=1109/0-3-8

Max Horz 2=-101(load case 7)

11-0-0

Max Uplift 2=-285(load case 6), 8=-285(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1806/974, 3-4=-1490/827, 4-5=-1282/803, 5-6=-1282/803,

6-7=-1490/827, 7-8=-1806/974, 8-9=0/47

BOT CHORD 2-12=-694/1546, 11-12=-502/1376, 10-11=-502/1376, 8-10=-694/1546

WEBS 3-12=-314/289, 4-12=-144/397, 5-12=-263/116, 5-10=-263/116, 6-10=-144/397,

7-10=-314/289

JOINT STRESS INDEX

2 = 0.89, 3 = 0.33, 4 = 0.61, 5 = 0.38, 6 = 0.61, 7 = 0.33, 8 = 0.89, 10 = 0.56, 11 = 0.48 and 12 = 0.56

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp Trible Company Countries B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

Continued on page 2



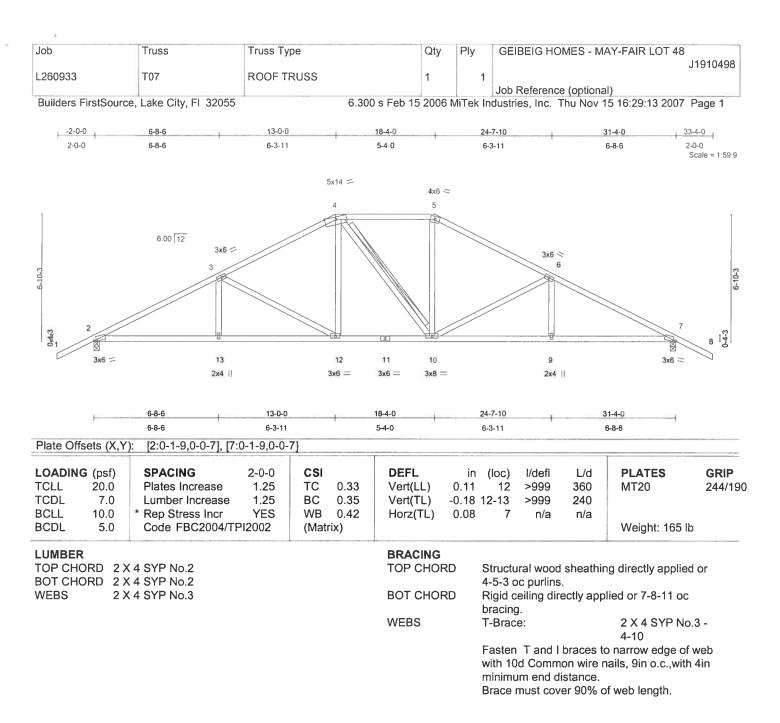


Job	Truss	Truss Type		Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
						J1910497
L260933	T06	ROOF TRUSS		1	1	
						Job Reference (optional)
Builders FirstSource	Lake City, FI 32055		6.300 s Feb 15	2006 N	liTek Ind	dustries, Inc. Thu Nov 15 16:29:12 2007 Page 2

- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 285 lb uplift at joint 2 and 285 lb uplift at joint 8.

LOAD CASE(S) Standard





REACTIONS (lb/size) 2=1109/0-3-8, 7=1109/0-3-8

Max Horz 2=-113(load case 7)

Max Uplift 2=-297(load case 6), 7=-297(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1817/950, 3-4=-1360/802, 4-5=-1151/786, 5-6=-1360/802,

6-7=-1817/950, 7-8=0/47

BOT CHORD 2-13=-668/1545, 12-13=-668/1545, 11-12=-374/1150, 10-11=-374/1150,

9-10=-668/1545, 7-9=-668/1545

WEBS 3-13=0/212, 3-12=-453/335, 4-12=-124/318, 4-10=-152/153, 5-10=-124/318,

6-10=-453/335, 6-9=0/212

JOINT STRESS INDEX

2 = 0.78, 3 = 0.39, 4 = 0.79, 5 = 0.68, 6 = 0.39, 7 = 0.78, 9 = 0.33, 10 = 0.56, 11 = 0.38, 12 = 0.34 and 13 = 0.33

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
					J1910498
L260933	T07	ROOF TRUSS	1	1	
					Job Reference (optional)

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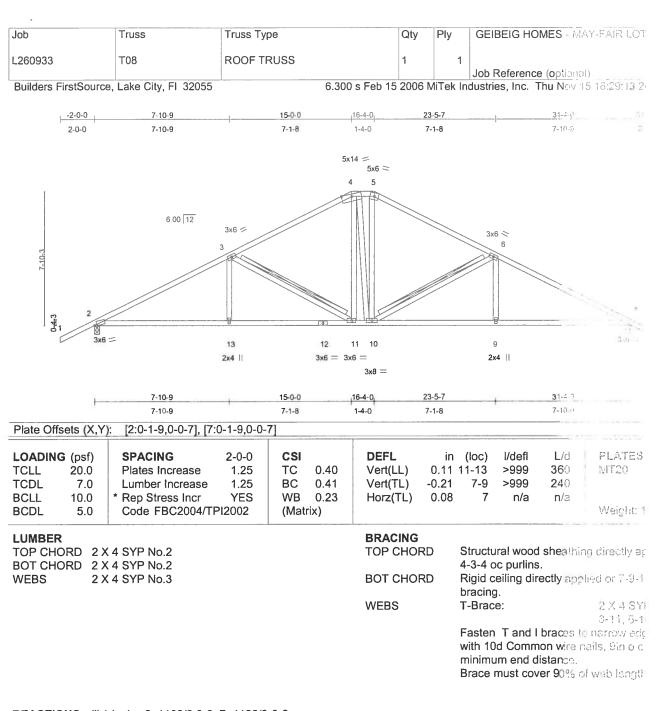
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 297 lb uplift at joint 2 and 297 lb uplift at joint 7.

LOAD CASE(S) Standard

Julium Leen Trium (Design (Inginesi Florida Ph. No. O-1889) 1406 (Inmadul Pay Plyd Boynton Weedh, 14. 55495





REACTIONS (lb/size) 2=1109/0-3-8, 7=1109/0-3-8

Max Horz 2=-125(load case 7)

Max Uplift 2=-308(load case 6), 7=-316(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1781/949, 3-4=-1235/765, 4-5=-1030/763, 5-6=-1239/767,

6-7=-1780/948, 7-8=0/47

BOT CHORD 2-13=-652/1504, 12-13=-652/1504, 11-12=-652/1504, 10-11=-298/1026,

9-10=-651/1503, 7-9=-651/1503

WEBS 3-13=0/250, 3-11=-558/407, 4-11=-164/290, 5-10=-162/364, 6-10=-552/403,

6-9=0/248, 4-10=-202/240

JOINT STRESS INDEX

2 = 0.77, 3 = 0.39, 4 = 0.79, 5 = 0.59, 6 = 0.39, 7 = 0.77, 9 = 0.33, 10 = 0.57, 11 = 0.34, 12 = 0.52 and 13 = 0.33.

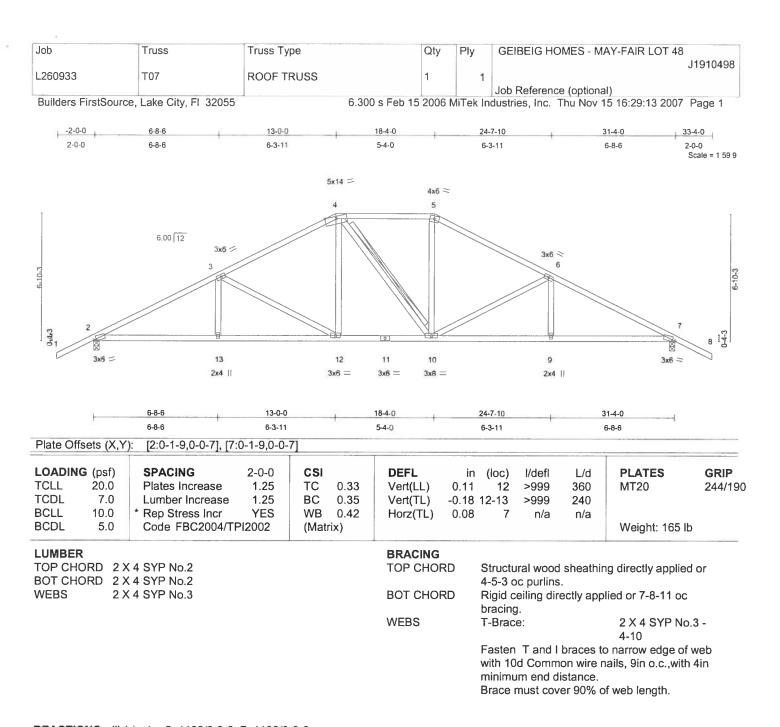
Continued on page 2



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



N



REACTIONS (lb/size) 2=1109/0-3-8, 7=1109/0-3-8

Max Horz 2=-113(load case 7)

Max Uplift 2=-297(load case 6), 7=-297(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1817/950, 3-4=-1360/802, 4-5=-1151/786, 5-6=-1360/802,

6-7=-1817/950, 7-8=0/47

BOT CHORD 2-13=-668/1545, 12-13=-668/1545, 11-12=-374/1150, 10-11=-374/1150,

9-10=-668/1545, 7-9=-668/1545

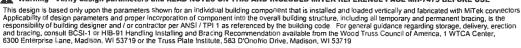
WEBS 3-13=0/212, 3-12=-453/335, 4-12=-124/318, 4-10=-152/153, 5-10=-124/318,

6-10=-453/335, 6-9=0/212

JOINT STRESS INDEX

2 = 0.78, 3 = 0.39, 4 = 0.79, 5 = 0.68, 6 = 0.39, 7 = 0.78, 9 = 0.33, 10 = 0.56, 11 = 0.38, 12 = 0.34 and 13 = 0.33 Continued on page 2







Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
					J1910498
L260933	T07	ROOF TRUSS	1	1	W 1931
					Job Reference (optional)
Builders FirstSour	rce Lake City EL 3205	5 6 30	10 c Eab 15 2006 N	ATOK In	dustries Inc. Thu New 15 16:20:12 2007 Page 2

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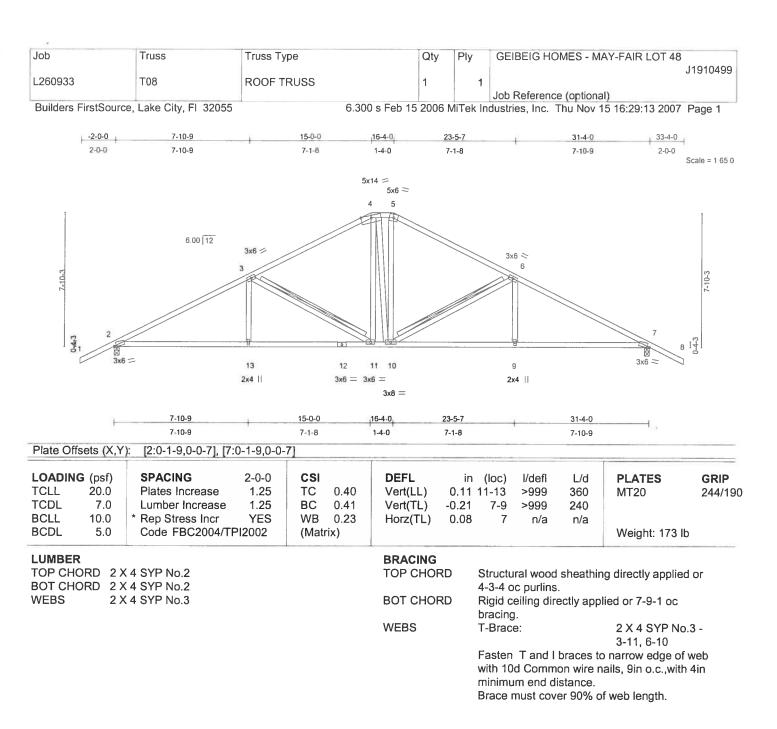
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 297 lb uplift at joint 2 and 297 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Les Trius (Design Coginser Flinish File District Fline Comment File Flori Howards Design 21





REACTIONS (lb/size) 2=1109/0-3-8, 7=1109/0-3-8

Max Horz 2=-125(load case 7)

Max Uplift 2=-308(load case 6), 7=-316(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1781/949, 3-4=-1235/765, 4-5=-1030/763, 5-6=-1239/767,

6-7=-1780/948, 7-8=0/47

BOT CHORD 2-13=-652/1504, 12-13=-652/1504, 11-12=-652/1504, 10-11=-298/1026,

9-10=-651/1503, 7-9=-651/1503

WEBS 3-13=0/250, 3-11=-558/407, 4-11=-164/290, 5-10=-162/364, 6-10=-552/403,

6-9=0/248, 4-10=-202/240

JOINT STRESS INDEX

2 = 0.77, 3 = 0.39, 4 = 0.79, 5 = 0.59, 6 = 0.39, 7 = 0.77, 9 = 0.33, 10 = 0.57, 11 = 0.34, 12 = 0.52 and 13 = 0.33

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
					J1910499
L260933	T08	ROOF TRUSS	1	1	
					Job Reference (optional)

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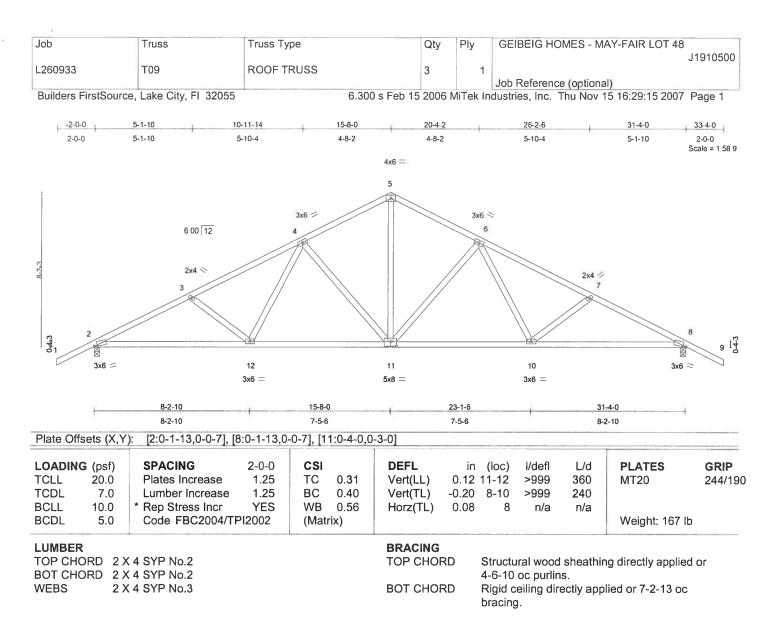
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 308 lb uplift at joint 2 and 316 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Lew Trues Copien Engineer Horida Pis No. 3-1869 Hind Chastal May Alvi Devitor beach, to 50405





REACTIONS (lb/size) 2=1109/0-3-8, 8=1109/0-3-8

Max Horz 2=-128(load case 7)

Max Uplift 2=-311(load case 6), 8=-311(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1850/1019, 3-4=-1619/933, 4-5=-1157/778, 5-6=-1157/778,

6-7=-1619/933, 7-8=-1850/1019, 8-9=0/47

BOT CHORD 2-12=-746/1590, 11-12=-493/1265, 10-11=-493/1265, 8-10=-746/1590

WEBS 3-12=-268/260, 4-12=-111/328, 4-11=-452/357, 5-11=-512/750, 6-11=-452/357,

6-10=-111/328, 7-10=-268/260

JOINT STRESS INDEX

Colinate of page 2

2 = 0.76, 3 = 0.33, 4 = 0.41, 5 = 0.36, 6 = 0.41, 7 = 0.33, 8 = 0.76, 10 = 0.44, 11 = 0.37 and 12 = 0.44

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp = 1: B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other

November 16,2007

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handfling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
					J1910500
L260933	T09	ROOF TRUSS	3	1	
					Job Reference (optional)

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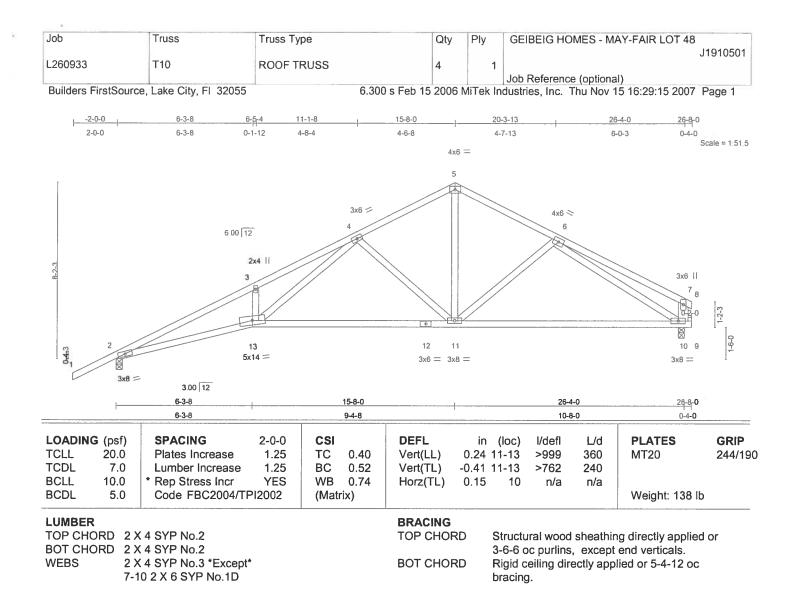
NOTES

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 311 lb uplift at joint 2 and 311 lb uplift at joint 8.

LOAD CASE(S) Standard





REACTIONS (lb/size) 2=956/0-3-8, 10=857/0-3-8

Max Horz 2=190(load case 6)

Max Uplift 2=-284(load case 6), 10=-180(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-2708/1450, 3-4=-2678/1612, 4-5=-1003/653, 5-6=-1007/653,

6-7=-371/235, 7-8=0/10, 7-10=-324/260

BOT CHORD 2-13=-1342/2405, 12-13=-686/1289, 11-12=-686/1289, 10-11=-510/959, 9-10=0/0

WEBS 3-13=-249/274, 4-13=-835/1385, 4-11=-595/455, 5-11=-382/599, 6-11=-205/210,

6-10=-842/488

JOINT STRESS INDEX

Continued on page 2

2 = 0.63, 3 = 0.33, 4 = 0.87, 5 = 0.41, 6 = 0.28, 7 = 0.56, 10 = 0.87, 11 = 0.56, 12 = 0.47 and 13 = 0.69

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

 *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Jidum Lam Truse Ceedan Engineer Floride PE No. 34888 1400 Chastel May Alva Woynton Weech, 4 C 55450

November 16,2007

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MfTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCS-1 or HIB-91 Handling installing and Bracing Recommendation authority. Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
					J191050 ²
L260933	T10	ROOF TRUSS	4	1	
					Job Reference (optional)

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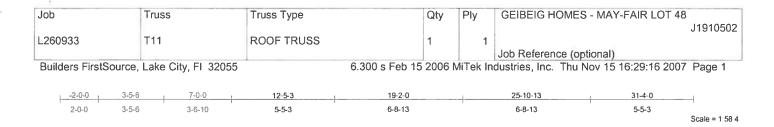
NOTES

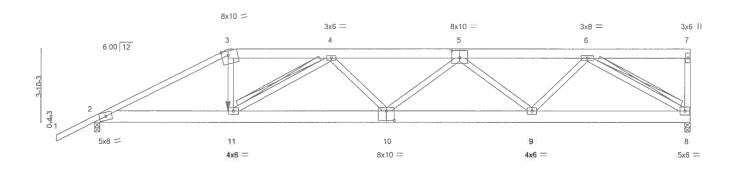
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 284 lb uplift at joint 2 and 180 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Less Truss Essan Engineer Florida Fill 11-11005 Florida Essantia May fillon Security Security







7-0-0		15-4-2	8-4-2 7-7-11					3	1-4-0		
7-0-0								8-4-2	8	3-4-2	
sets (X,Y): [3:0-4-2,Edge], [5:	0-5-0,0-4-	8], [10:0)-5-0,0-6	-0]						
G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.21	10	>999	360	MT20	244/190
7.0	Lumber Increase	1.25	BC	0.27	Vert(TL)	-0.40	10-11	>933	240		
10.0	* Rep Stress Incr	NO	WB	0.88	Horz(TL)	0.08	8	n/a	n/a		
5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 220 lb	
	(psf) 20.0 7.0 10.0	7-0-0 sets (X,Y): [3:0-4-2,Edge], [5: G (psf)	7-0-0 sets (X,Y): [3:0-4-2,Edge], [5:0-5-0,0-4- G (psf)	7-0-0 8-4-2 sets (X,Y): [3:0-4-2,Edge], [5:0-5-0,0-4-8], [10:0 G (psf) SPACING 2-0-0 CSI 20.0 Plates Increase 1.25 TC 10.0 * Rep Stress Incr NO WB	7-0-0 8-4-2 sets (X,Y): [3:0-4-2,Edge], [5:0-5-0,0-4-8], [10:0-5-0,0-6-6-6-6-6-6-6-6-6-6-6-6-6-6-6-6-6-6	7-0-0 8-4-2 Sets (X,Y): [3:0-4-2,Edge], [5:0-5-0,0-4-8], [10:0-5-0,0-6-0] G (psf) 20.0 Plates Increase 1.25 TC 0.53 Vert(LL) 7.0 Lumber Increase 1.25 BC 0.27 Vert(TL) 10.0 * Rep Stress Incr NO WB 0.88 Horz(TL)	7-0-0 8-4-2 7-7-11 sets (X,Y): [3:0-4-2,Edge], [5:0-5-0,0-4-8], [10:0-5-0,0-6-0] G (psf) SPACING 2-0-0 CSI DEFL in 20.0 Plates Increase 1.25 TC 0.53 Vert(LL) -0.21 Lumber Increase 1.25 BC 0.27 Vert(TL) -0.40 10.0 * Rep Stress Incr NO WB 0.88 Horz(TL) 0.08	7-0-0 8-4-2 7-7-11 sets (X,Y): [3:0-4-2,Edge], [5:0-5-0,0-4-8], [10:0-5-0,0-6-0] G (psf) SPACING 2-0-0 CSI DEFL in (loc) 20.0 Plates Increase 1.25 TC 0.53 Vert(LL) -0.21 10 7.0 Lumber Increase 1.25 BC 0.27 Vert(TL) -0.40 10-11 10.0 * Rep Stress Incr NO WB 0.88 Horz(TL) 0.08 8	7-0-0 8-4-2 7-7-11 sets (X,Y): [3:0-4-2,Edge], [5:0-5-0,0-4-8], [10:0-5-0,0-6-0] G (psf) SPACING 2-0-0 CSI DEFL in (loc) I/defl 20.0 Plates Increase 1.25 TC 0.53 Vert(LL) -0.21 10 >999 7.0 Lumber Increase 1.25 BC 0.27 Vert(TL) -0.40 10-11 >933 10.0 * Rep Stress Incr NO WB 0.88 Horz(TL) 0.08 8 n/a	7-0-0 8-4-2 7-7-11 8 sets (X,Y): [3:0-4-2,Edge], [5:0-5-0,0-4-8], [10:0-5-0,0-6-0] G (psf) SPACING 2-0-0 CSI DEFL in (loc) 1/defl L/d 20.0 Plates Increase 1.25 TC 0.53 Vert(LL) -0.21 10 >999 360 7.0 Lumber Increase 1.25 BC 0.27 Vert(TL) -0.40 10-11 >933 240 10.0 * Rep Stress Incr NO WB 0.88 Horz(TL) 0.08 8 n/a n/a	7-0-0 8-4-2 7-7-11 8-4-2 sets (X,Y): [3:0-4-2,Edge], [5:0-5-0,0-4-8], [10:0-5-0,0-6-0] G (psf) SPACING 2-0-0 CSI DEFL in (loc) i/defl L/d PLATES 20.0 Plates Increase 1.25 TC 0.53 Vert(LL) -0.21 10 >999 360 MT20 7.0 Lumber Increase 1.25 BC 0.27 Vert(TL) -0.40 10-11 >933 240 10.0 * Rep Stress Incr NO WB 0.88 Horz(TL) 0.08 8 n/a n/a

LUMBER BRACING TOP CHORD 2 X 6 SYP No.1D *Except* TOP CHORD Structural wood sheathing directly applied or 1-3 2 X 4 SYP No.2 2-9-8 oc purlins, except end verticals. BOT CHORD 2 X 8 SYP 2400F 2.0E **BOT CHORD** Rigid ceiling directly applied or 8-11-13 oc **WEBS** 2 X 4 SYP No.3 bracing. **WEBS** T-Brace:

2 X 4 SYP No.3 -

4-11, 6-8

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 8=2194/0-3-8, 2=2140/0-3-8

Max Horz 2=162(load case 5)

Max Uplift 8=-755(load case 4), 2=-666(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/54, 2-3=-4181/1361, 3-4=-3745/1268, 4-5=-5112/1715, 5-6=-3909/1295,

6-7=-79/23, 7-8=-277/136

BOT CHORD 2-11=-1249/3685, 10-11=-1775/5042, 9-10=-1750/4987, 8-9=-1061/2959

3-11=-403/1381, 4-11=-1638/611, 4-10=0/239, 5-10=0/232, 5-9=-1427/603, **WEBS**

6-9=-354/1440, 6-8=-3400/1226

JOINT STRESS INDEX

2 = 0.85, 3 = 0.71, 4 = 0.46, 5 = 0.33, 6 = 0.87, 7 = 0.41, 8 = 0.84, 9 = 0.61, 10 = 0.64 and 11 = 0.62

Continued on page 2



Job Tr	russ	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
	1				J1910502
L260933 T1	11	ROOF TRUSS	1	1	
					Job Reference (optional)

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NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 755 lb uplift at joint 8 and 666 lb uplift at joint 2.
- 7) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

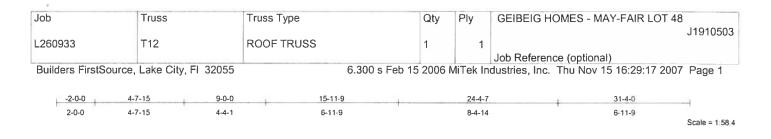
Uniform Loads (plf)

Vert: 1-3=-54, 3-7=-118(F=-64), 2-11=-10, 8-11=-22(F=-12)

Concentrated Loads (lb) Vert: 11=-411(F)

> Juine Lee Truss Cesan Endinger Planta Purito delles 1476 Chastel Pay Rive Devices Leader L. 1984





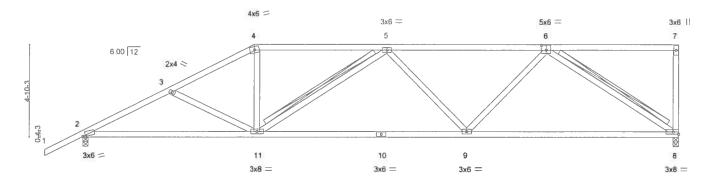




Plate Of	fisets (X,Y	'): [2:0-1-13,0-0-7], [6	5:0-3-0,0-	3-0]		T					,	
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.79	Vert(LL)	-0.25	`8-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	ВС	0.64	Vert(TL)	-0.45	8-9	>822	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.57	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	2002	(Mat	rix)	, ,					Weight: 160 lb	
	3.0			,,,,,	,						g 100 lb	

LUMBEK	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

BRACING TOP CHORD

OP CHORD Structural wood sheathing directly applied or 4-6-13 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 6-4-5 oc

bracing.

WEBS T-Brace:

2 X 4 SYP No.3 -

5-11, 6-8

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 8=989/0-3-8, 2=1113/0-3-8

Max Horz 2=195(load case 6)

Max Uplift 8=-270(load case 5), 2=-265(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1846/924, 3-4=-1622/824, 4-5=-1423/798, 5-6=-1520/785, 6-7=-63/9

, 7-8=-160/110

BOT CHORD 2-11=-969/1582, 10-11=-965/1730, 9-10=-965/1730, 8-9=-671/1207

WEBS 3-11=-191/195, 4-11=-130/428, 5-11=-369/201, 5-9=-309/265, 6-9=-168/510,

6-8=-1396/797

JOINT STRESS INDEX

2 = 0.80, 3 = 0.33, 4 = 0.66, 5 = 0.36, 6 = 0.68, 7 = 0.34, 8 = 0.65, 9 = 0.36, 10 = 0.67 and 11 = 0.56

2 = 0.00, 3 = 0.00, 4 = 0.00, 0 = 0.00, 7 = 0.04, 0 = 0.00, 10 = 0.07 and 11 = 0.00

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
					J1910503
L260933	T12	ROOF TRUSS	1	1	
					Job Reference (optional)
Duildoro EirotCoura	a Laka Cike EL 20	DOEE	C 200 - F-b 45 200C	ACT - L. L.	dustries Inc. Thu New 15 16:20:17 2007 Dec. 2

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NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 270 lb uplift at joint 8 and 265 lb uplift at joint 2.

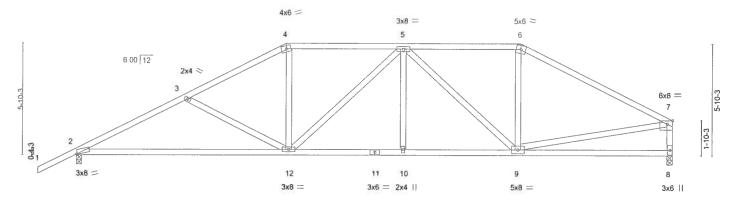
LOAD CASE(S) Standard

Johns Lee Trues Costan Indinos Migris Mis No 3-1905 1106 Costan Mis Mist Downley Nosett Le Di-155









11-0-0					6-2-0	0 6-2-0						
Plate Of	ffsets (X,Y): [2:0-0-10,Edge], [7:0-3-8,Ed	lge]								
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	-0.31	2-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	ВС	0.66	Vert(TL)	-0.57	2-12	>656	240		

BCLL 10.0 Rep Stress Incr YES WB 0.57 Horz(TL) 0.06 n/a n/a Code FBC2004/TPI2002 **BCDL** 5.0 (Matrix) Weight: 169 lb

17-2-0

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3 *Except*

7-8 2 X 4 SYP No.2

BRACING

BOT CHORD

TOP CHORD

23-4-0

Structural wood sheathing directly applied or

31-4-0

4-5-14 oc purlins, except end verticals. Rigid ceiling directly applied or 6-9-7 oc

bracing.

(lb/size) 2=1113/0-3-8, 8=989/0-3-8 REACTIONS

Max Horz 2=147(load case 6)

11-0-0

Max Uplift 2=-285(load case 6), 8=-167(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/47, 2-3=-1811/973, 3-4=-1502/831, 4-5=-1296/809, 5-6=-1102/723, TOP CHORD

6-7=-1324/714, 7-8=-943/556

BOT CHORD 2-12=-853/1550, 11-12=-667/1393, 10-11=-667/1393, 9-10=-667/1393,

8-9=-167/218

WEBS 3-12=-302/280, 4-12=-119/387, 5-12=-254/119, 5-10=0/124, 5-9=-492/196,

6-9=-18/281, 7-9=-362/897

JOINT STRESS INDEX

2 = 0.89, 3 = 0.33, 4 = 0.73, 5 = 0.56, 6 = 0.74, 7 = 0.64, 8 = 0.47, 9 = 0.41, 10 = 0.33, 11 = 0.46 and 12 = 0.56

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This Coffiliase's designed for C-C for members and forces, and for MWFRS for reactions specified.

November 16,2007

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MITek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
L260933	T12	ROOF TRUSS	1	1	J1910504
L200933	110	ROOF 1R033	1	<u> </u>	Job Reference (optional)

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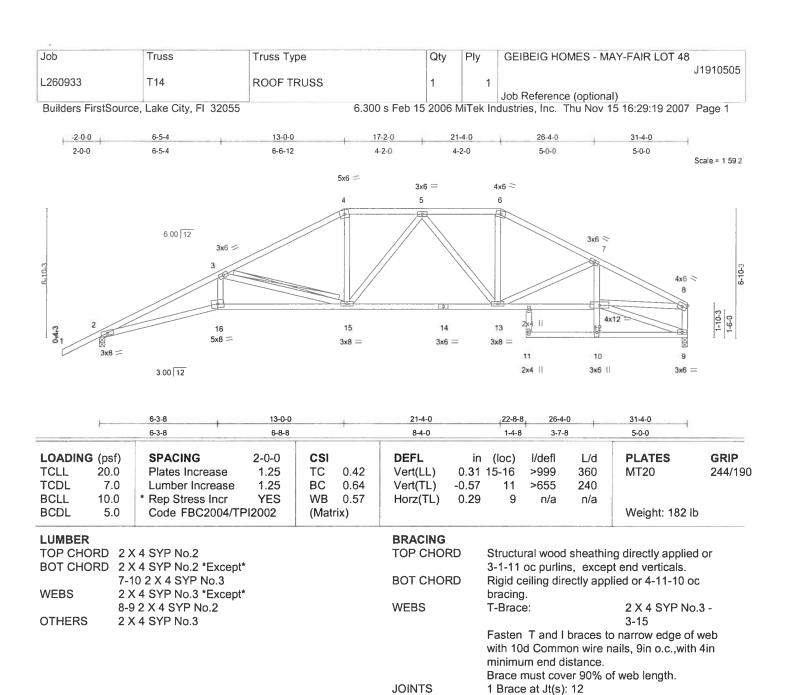
NOTES

- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 285 lb uplift at joint 2 and 167 lb uplift at joint 8.

LOAD CASE(S) Standard

Judice Less Truss Consider Coderage Phristal Plant of Made Fish Code Consider Play (Nivel





REACTIONS (lb/size) 2=1121/0-3-8, 9=1019/0-3-8

Max Horz 2=158(load case 6)

Max Uplift 2=-292(load case 6), 9=-164(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-3387/1749, 3-4=-1782/963, 4-5=-1537/939, 5-6=-1389/840,

6-7=-1600/871, 7-8=-2147/1051, 8-9=-1022/543

BOT CHORD 2-16=-1584/3027, 15-16=-1510/2868, 14-15=-677/1527, 13-14=-677/1527,

12-13=-891/1896, 10-12=0/199, 7-12=-11/397, 10-11=0/0, 9-10=-47/17

WEBS 3-16=-327/781, 3-15=-1389/855, 4-15=-166/459, 5-15=-159/94, 5-13=-325/142,

6-13=-173/436, 7-13=-571/338, 8-12=-808/1775, 9-12=-28/110

JOINT STRESS INDEX

2 = 0.77, 3 = 0.56, 4 = 0.58, 5 = 0.39, 6 = 0.57, 7 = 0.43, 8 = 0.72, 9 = 0.39, 10 = 0.38, 11 = 0.33, 12 = 0.77, 13 = 0.56, 14 = 0.580.54, 15 = 0.61, 16 = 0.90 and 17 = 0.33November 16,2007 Continued on page 2

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erect and bracing, consult BCS-I or HIB-91 Handling Installing and Bracing Recommendation aublate from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
					J191050
L260933	T14	ROOF TRUSS	1	1	
					Job Reference (optional)

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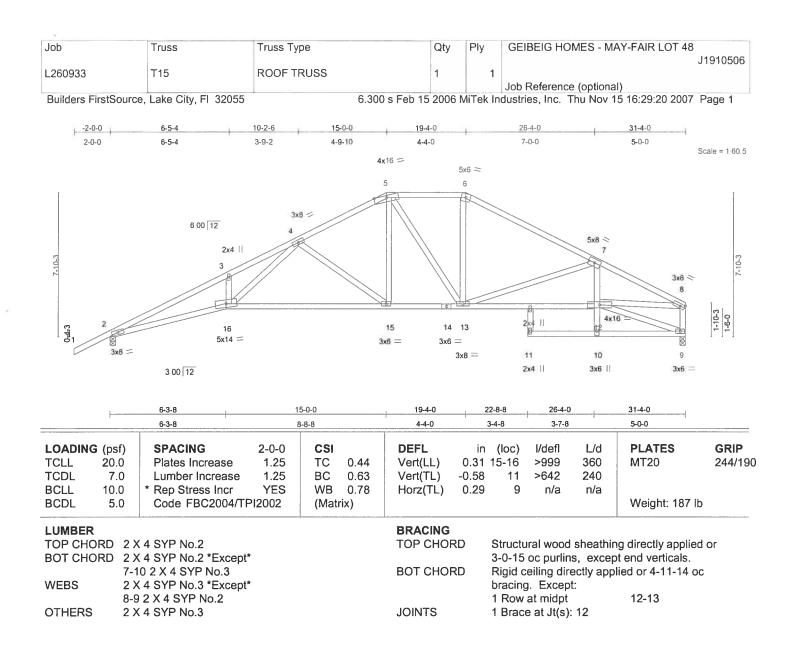
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 292 lb uplift at joint 2 and 164 lb uplift at joint 9.

LOAD CASE(S) Standard

Dalois Lass Trips (Ession Engineer Histor Parasis Pay Plos Histor Enasta Pay Plos Loviton beauty





REACTIONS (lb/size) 2=1121/0-3-8, 9=1019/0-3-8

Max Horz 2=170(load case 6)

Max Uplift 2=-302(load case 6), 9=-177(load case 7)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-3367/1728, 3-4=-3309/1864, 4-5=-1506/882, 5-6=-1260/827,

6-7=-1489/837, 7-8=-2160/1069, 8-9=-1019/541

BOT CHORD 2-16=-1560/3004, 15-16=-942/1862, 14-15=-543/1304, 13-14=-543/1304,

12-13=-951/1964, 10-12=0/196, 7-12=0/450, 10-11=0/0, 9-10=-81/17

WEBS 3-16=-202/231, 4-16=-804/1449, 4-15=-708/503, 5-15=-263/497, 5-13=-228/107,

6-13=-113/368, 7-13=-748/452, 8-12=-840/1800, 9-12=0/147

Trues Coston Champer Flatins Ph. Islands Physics 1100 Censul Hay Flyr Goynton Weson, Ft. 55456

JOINT STRESS INDEX

2 = 0.77, 3 = 0.33, 4 = 0.72, 5 = 0.63, 6 = 0.59, 7 = 0.51, 8 = 0.94, 9 = 0.38, 10 = 0.41, 11 = 0.33, 12 = 0.69, 13 = 0.57, 14 = 0.47, 15 = 0.34, 16 = 0.82 and 17 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
	1				J1910506
L260933	T15	ROOF TRUSS	1	1	
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:20 2007 Page 2

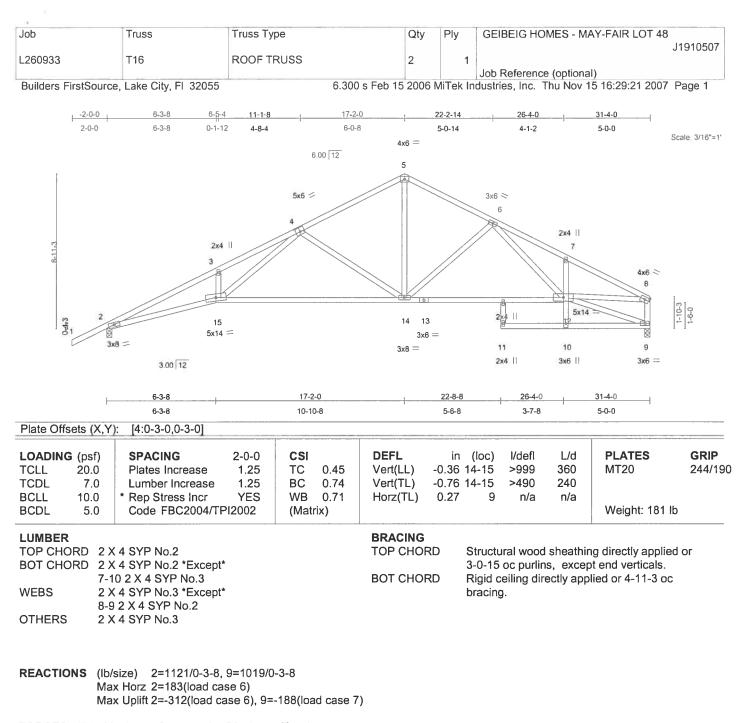
NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 302 lb uplift at joint 2 and 177 lb uplift at joint 9.

LOAD CASE(S) Standard

Johns Lee Trues Caston Chomsor Horida Mi Na. 3-1865 1966 Chastal May Flori Bovinso Leasts, A. 20015





FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-3381/1752, 3-4=-3330/1901, 4-5=-1327/806, 5-6=-1309/814,

6-7=-1970/1104, 7-8=-1964/988, 8-9=-1027/557

BOT CHORD 2-15=-1582/3019, 14-15=-894/1745, 13-14=-667/1420, 12-13=-667/1420,

10-12=0/194, 7-12=-232/238, 10-11=0/0, 9-10=-122/0

WEBS 3-15=-213/248, 4-15=-868/1565, 4-14=-748/554, 5-14=-462/803, 6-14=-428/315,

6-12=-208/552, 8-12=-748/1609, 9-12=-21/191

Trues Cesion Chomast Platide PE 140 Histor 1 100 Chastel Rey Nich Coynton Cason, 10 60456

JOINT STRESS INDEX

2 = 0.78, 3 = 0.33, 4 = 0.69, 5 = 0.66, 6 = 0.39, 7 = 0.33, 8 = 0.75, 9 = 0.40, 10 = 0.45, 11 = 0.33, 12 = 0.47, 13 = 0.65, 14 = 0.56, 15 = 0.93 and 16 = 0.33

Continued on page 2



		J191050
2	1	
		Job Reference (optional)
	2	2 1

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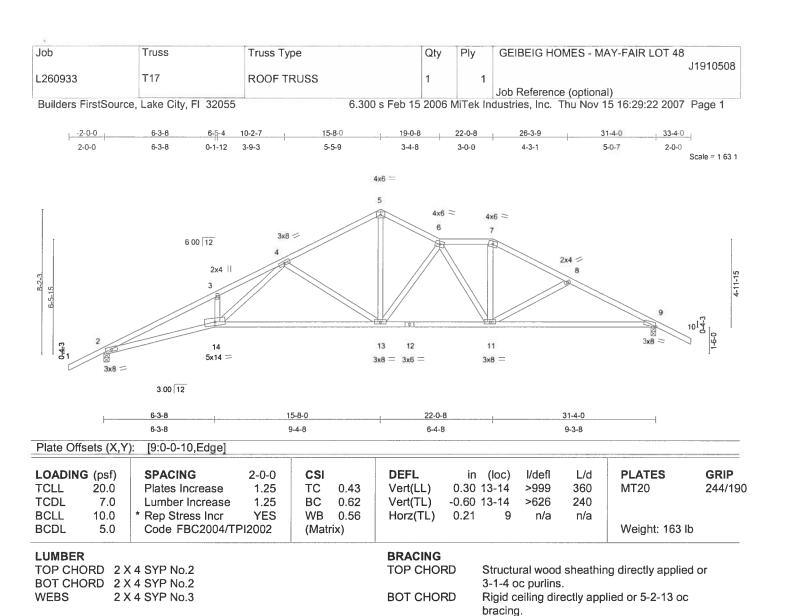
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 312 lb uplift at joint 2 and 188 lb uplift at joint 9.

LOAD CASE(S) Standard

Judius Les Truss Coston Chaineer Horids Fit No. 3-1889 1400 Chastal May Mive Boviton Lesson 1-1 Judio





REACTIONS (lb/size) 2=1109/0-3-8, 9=1109/0-3-8

Max Horz 2=158(load case 6)

Max Uplift 2=-311(load case 6), 9=-310(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-3322/1665, 3-4=-3262/1801, 4-5=-1429/851, 5-6=-1378/861,

6-7=-1375/848, 7-8=-1576/884, 8-9=-1826/991, 9-10=0/47

BOT CHORD 2-14=-1416/2963, 13-14=-843/1831, 12-13=-628/1538, 11-12=-628/1538,

9-11=-717/1566

WEBS 3-14=-201/230, 4-14=-749/1437, 4-13=-732/506, 5-13=-532/933, 6-13=-542/355,

6-11=-314/188, 7-11=-208/452, 8-11=-246/219

JOINT STRESS INDEX

2 = 0.76, 3 = 0.33, 4 = 0.72, 5 = 0.54, 6 = 0.34, 7 = 0.47, 8 = 0.33, 9 = 0.70, 11 = 0.57, 12 = 0.54, 13 = 0.56 and 14 = 0.84

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

8) Arrayide adeguate drainage to prevent water ponding.

Julius Lee Pruse (Design (Induneer Florida ME No. Unising 1 106 (Indusia) May (Nort Noymon Weson, FL 1545)

November 16,2007

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling installing and Bracing Recommendation authority Mood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
1.000000	T47	DOOF TOUGO			J1910508
L260933	117	ROOF TRUSS	1	1	
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:22 2007 Page 2

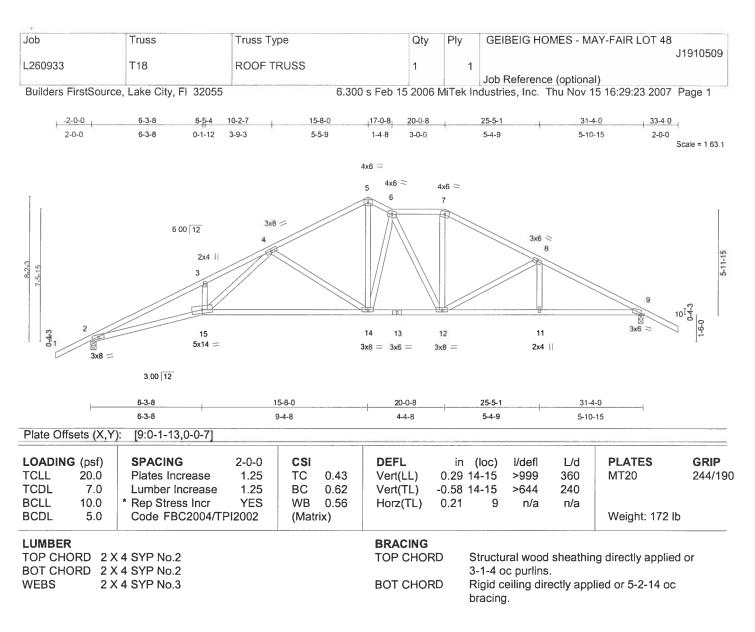
NOTES

- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 311 lb uplift at joint 2 and 310 lb uplift at joint 9.

LOAD CASE(S) Standard

Julium Law Truss Cassan Endinger Planda Pt. No. Jakat 1466 Chasim Pay Plon Bayrian treasin, i.e. buris





REACTIONS (lb/size) 2=1109/0-3-8, 9=1109/0-3-8

Max Horz 2=158(load case 6)

Max Uplift 2=-311(load case 6), 9=-310(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-3322/1663, 3-4=-3263/1800, 4-5=-1432/849, 5-6=-1314/838,

6-7=-1250/823, 7-8=-1455/852, 8-9=-1835/957, 9-10=0/47

BOT CHORD 2-15=-1415/2963, 14-15=-839/1829, 13-14=-479/1316, 12-13=-479/1316,

11-12=-681/1565, 9-11=-681/1565

WEBS 3-15=-204/232, 4-15=-752/1438, 4-14=-725/503, 5-14=-478/867, 6-14=-439/248,

6-12=-222/58, 7-12=-153/343, 8-12=-379/261, 8-11=0/191

JOINT STRESS INDEX

2 = 0.76, 3 = 0.33, 4 = 0.72, 5 = 0.59, 6 = 0.44, 7 = 0.60, 8 = 0.39, 9 = 0.76, 11 = 0.33, 12 = 0.60, 13 = 0.47, 14 = 0.67 and 15 = 0.83

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This Coffilissia designate for C-C for members and forces, and for MWFRS for reactions specified.

Tribe Caston Chamber Plands PE no altem 1106 Chastal Bay Blod Boynton Besch, tt busic

November 16,2007

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Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
<i>S</i> .					J1910509
L260933	T18	ROOF TRUSS	1	1	
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:23 2007 Page 2

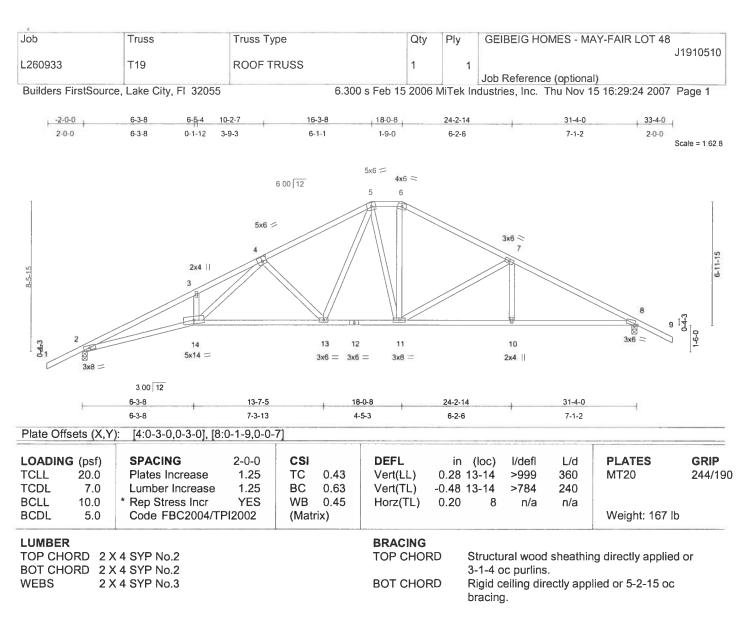
NOTES

- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 311 lb uplift at joint 2 and 310 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Les Truss Cesson Ceores Horida Mil Fic. 3-1866 Horida Mil Fic. 3-1866 Horida Ceasini May Micel





REACTIONS (lb/size) 2=1109/0-3-8, 8=1109/0-3-8

Max Horz 2=162(load case 6)

Max Uplift 2=-313(load case 6), 8=-293(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-3302/1671, 3-4=-3237/1800, 4-5=-1618/972, 5-6=-1129/789,

6-7=-1336/806, 7-8=-1804/952, 8-9=0/47

BOT CHORD 2-14=-1422/2944, 13-14=-852/1846, 12-13=-387/1152, 11-12=-387/1152,

10-11=-664/1529, 8-10=-664/1529

WEBS 3-14=-189/217, 4-14=-742/1390, 4-13=-682/488, 5-13=-353/613, 5-11=-266/137,

6-11=-211/389, 7-11=-475/343, 7-10=0/223

JOINT STRESS INDEX

2 = 0.76, 3 = 0.33, 4 = 0.56, 5 = 0.48, 6 = 0.62, 7 = 0.39, 8 = 0.78, 10 = 0.33, 11 = 0.67, 12 = 0.39, 13 = 0.53 and 14 = 0.82

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

৪১নিনেপ্ৰাৰ্থক এব প্ৰয়ান্ত্ৰাক drainage to prevent water ponding.

November 16,2007

warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE



Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
					J1910510
L260933	T19	ROOF TRUSS	1	1	
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:24 2007 Page 2

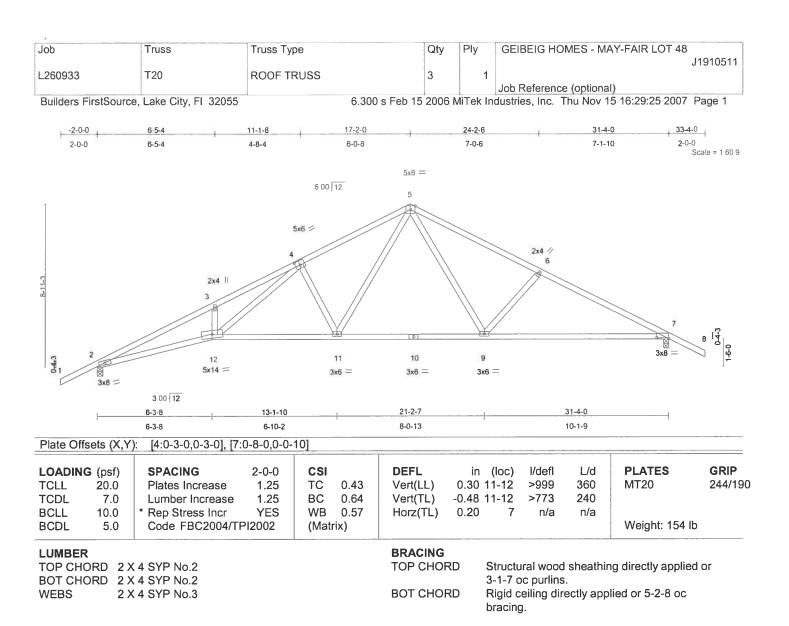
NOTES

- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 313 lb uplift at joint 2 and 293 lb uplift at joint 8.

LOAD CASE(S) Standard

Julium Lorer Trungs (Jessich Cheinspr Heinige bu Flot Strom 1466 Chessis May Mive Jewiton Lessis 14, 2004);





REACTIONS (lb/size) 2=1109/0-3-8, 7=1109/0-3-8

Max Horz 2=167(load case 6)

Max Uplift 2=-317(load case 6), 7=-298(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-3301/1691, 3-4=-3256/1843, 4-5=-1680/1043, 5-6=-1527/924,

6-7=-1783/981, 7-8=0/47

BOT CHORD 2-12=-1441/2944, 11-12=-775/1724, 10-11=-353/1082, 9-10=-353/1082,

7-9=-690/1516

WEBS 3-12=-219/254, 4-12=-843/1504, 4-11=-621/460, 5-11=-433/730, 5-9=-227/434,

6-9=-367/343

JOINT STRESS INDEX

2 = 0.76, 3 = 0.33, 4 = 0.67, 5 = 0.63, 6 = 0.33, 7 = 0.65, 9 = 0.43, 10 = 0.38, 11 = 0.57 and 12 = 0.86

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp \$\frac{1}{2}\$. B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Trues fossion incursor Figures PM No. 3-1888 I no Chasial Pay Rivel Boynton Lesson, PL 0040

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Continued on page 2

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE



Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
L260933	T20	ROOF TRUSS	2	1	J191051
L200933	120	1001 11033	3	,	Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Nov 15 16:29:25 2007 Page 2

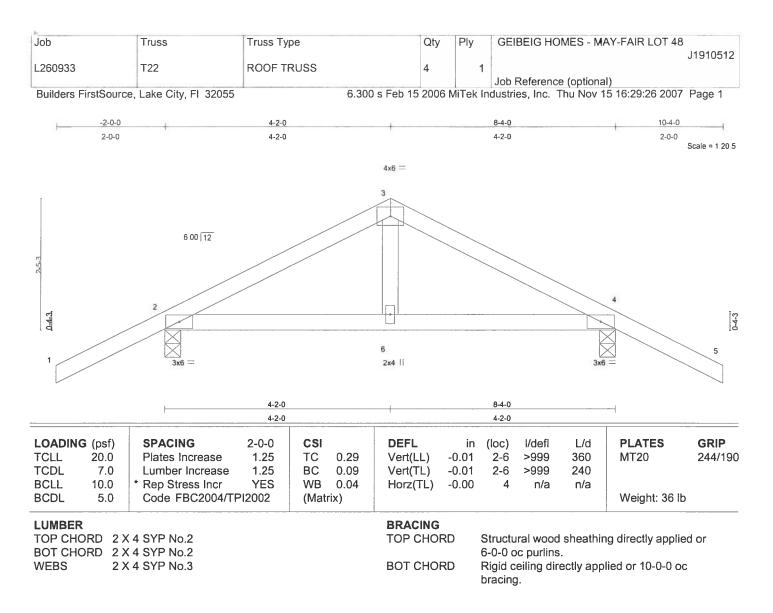
NOTES

- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 317 lb uplift at joint 2 and 298 lb uplift at joint 7.

LOAD CASE(S) Standard

Juine Les Tros Design (Indeser House Mil No. 3-1888 1406 Crasia May Mod Boynon Beach, Fl. Susse





REACTIONS (lb/size) 2=373/0-3-8, 4=373/0-3-8

Max Horz 2=-60(load case 7)

Max Uplift 2=-260(load case 6), 4=-260(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-294/448, 3-4=-294/448, 4-5=0/47

BOT CHORD 2-6=-242/212, 4-6=-242/212

WEBS 3-6=-204/128

JOINT STRESS INDEX

2 = 0.57, 3 = 0.35, 4 = 0.57 and 6 = 0.09

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

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Job	Truss	Truss Type		Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
L260933	T22	ROOF TRUSS		4	1	J1910512
		11000		T		Job Reference (optional)
Builders FirstSourc	e, Lake City, Fl 32055		6.300 s Feb 15	2006 N	1iTek In	dustries, Inc. Thu Nov 15 16:29:26 2007 Page 2

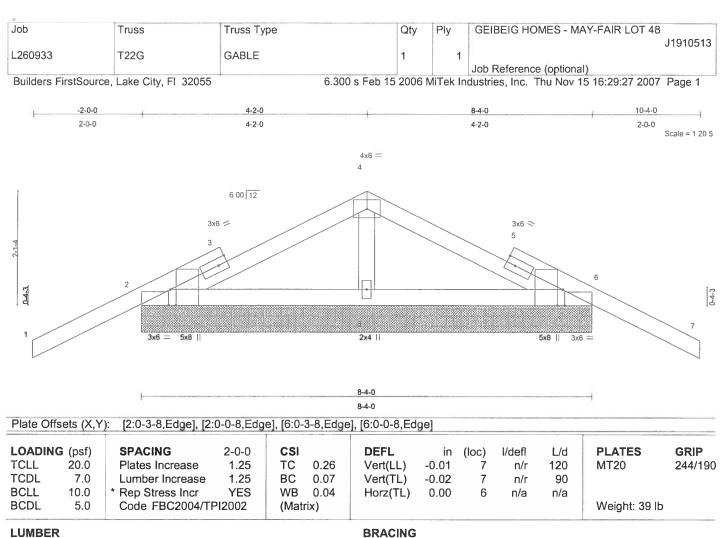
NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 260 lb uplift at joint 2 and 260 lb uplift at joint 4.

LOAD CASE(S) Standard

Jakus Les Truss Cossion Cropper Vibrios Pit No. 1-1660 1406 Crossis May (Nort Leouter Lesson III (North





TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

OTHERS 2 X 4 SYP No.3

TOP CHORD

Structural wood sheathing directly applied or

8-4-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 6-0-0 oc

bracing.

REACTIONS (lb/size) 2=227/8-4-0, 6=227/8-4-0, 8=294/8-4-0

Max Horz 2=-63(load case 7)

Max Uplift 2=-204(load case 6), 6=-214(load case 7), 8=-69(load case 6) Max Grav 2=239(load case 10), 6=239(load case 11), 8=294(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-12/35, 3-4=0/98, 4-5=0/98, 5-6=-4/35, 6-7=0/47

BOT CHORD 2-8=-48/104, 6-8=-48/104

4-8=-247/143 **WEBS**

JOINT STRESS INDEX

2 = 0.39, 2 = 0.00, 3 = 0.00, 3 = 0.22, 4 = 0.30, 5 = 0.00, 5 = 0.22, 6 = 0.39, 6 = 0.00 and 8 = 0.09

NOTES

1) Unbalanced roof live loads have been considered for this design.

Continue (ess) page MiTek "Standard Gable End Detail"

2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal

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Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
L260933	T22G	GABLE	1	1	J191051;
			·		Job Reference (optional)

Builders FirstSource, Lake City, FI 32055

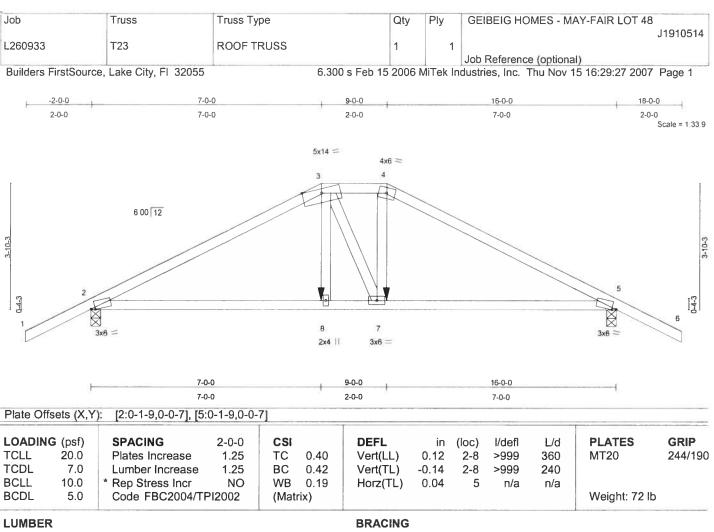
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- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 204 lb uplift at joint 2, 214 lb uplift at joint 6 and 69 lb uplift at joint 8.

LOAD CASE(S) Standard

Julium Lee Priva (Seegn Chornear Plande Pe Na. biseb 1900 Cessal Sey Myd 604(157 1686), t. bod 90





TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 **WEBS** 2 X 4 SYP No.3

TOP CHORD

Structural wood sheathing directly applied or

4-4-10 oc purlins.

BOT CHORD Rigid ceiling directly applied or 7-5-5 oc

bracing.

REACTIONS (lb/size) 2=1103/0-3-8, 5=1103/0-3-8

Max Horz 2=77(load case 5)

Max Uplift 2=-595(load case 5), 5=-595(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/47, 2-3=-1778/804, 3-4=-1526/770, 4-5=-1781/806, 5-6=0/47 TOP CHORD

BOT CHORD 2-8=-675/1504, 7-8=-684/1523, 5-7=-658/1507 **WEBS** 3-8=-262/480, 3-7=-146/159, 4-7=-303/592

JOINT STRESS INDEX

2 = 0.77, 3 = 0.87, 4 = 0.76, 5 = 0.77, 7 = 0.38 and 8 = 0.34

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

November 16,2007

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 BEFORE USE

Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
					J1910514
L260933	T23	ROOF TRUSS	1	1	
					Job Reference (optional)

Builders FirstSource, Lake City, FI 32055

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NOTES

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 595 lb uplift at joint 2 and 595 lb uplift at joint 5.
- 7) Girder carries hip end with 7-0-0 end setback.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

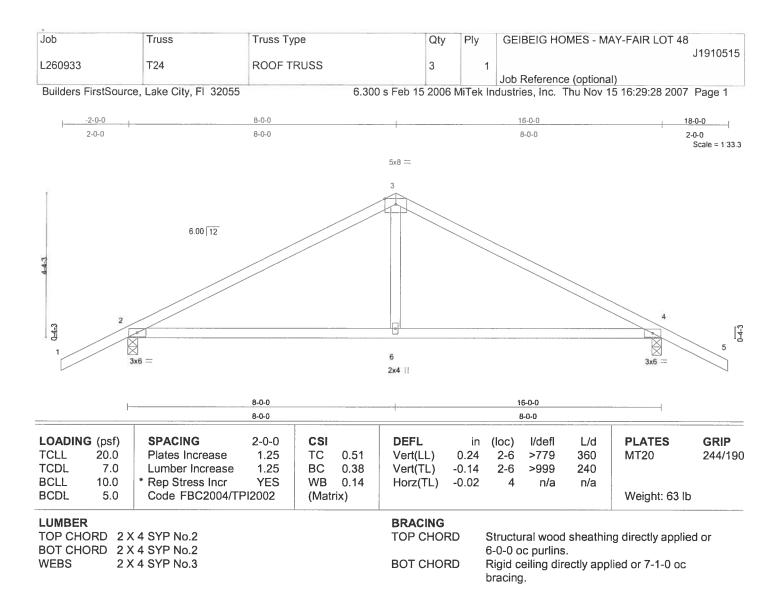
Vert: 1-3=-54, 3-4=-118(F=-64), 4-6=-54, 2-8=-10, 7-8=-22(F=-12), 5-7=-10

Concentrated Loads (lb)

Vert: 8=-411(F) 7=-411(F)

Julius Lane Prints Deposit Indiaper Florida Parantai Pay Mich





REACTIONS (lb/size) 2=619/0-3-8, 4=619/0-3-8

Max Horz 2=-83(load case 7)

Max Uplift 2=-404(load case 6), 4=-404(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-720/981, 3-4=-720/981, 4-5=0/47

BOT CHORD 2-6=-689/562, 4-6=-689/562

WEBS 3-6=-489/273

JOINT STRESS INDEX

2 = 0.69, 3 = 0.93, 4 = 0.69 and 6 = 0.19

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=13ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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Job	Truss	Truss Type	Qty	Ply	GEIBEIG HOMES - MAY-FAIR LOT 48
					J1910515
L260933	T24	ROOF TRUSS	3	1	
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055

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NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 404 lb uplift at joint 2 and 404 lb uplift at joint 4.

LOAD CASE(S) Standard

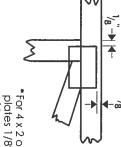


Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



*This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

4 × 4

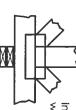
The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING



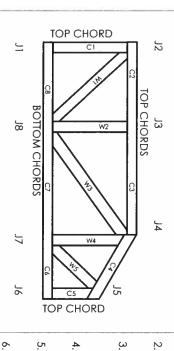
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

ICBO

96-31, 96-67

ВОСА

)

3907, 4922

SBCCI

WISC/DILHR

960022-W, 970036-N

9667, 9432A

NER

561





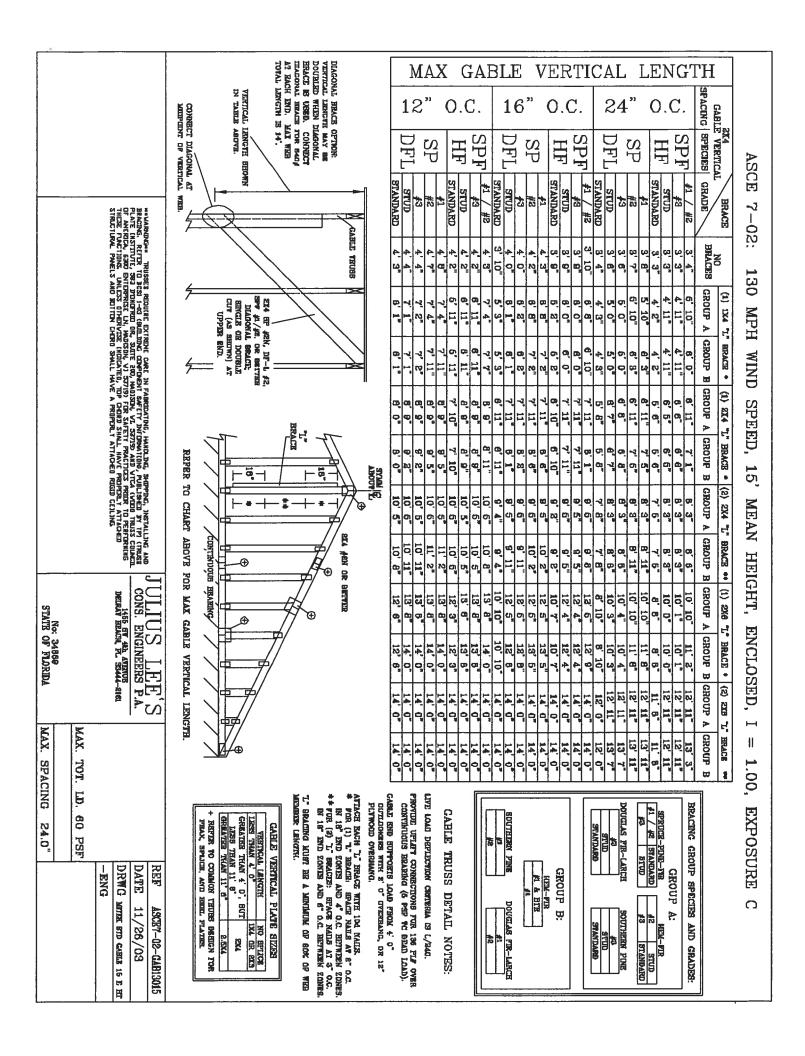
MiTek Engineering Reference Sheet: MII-7473

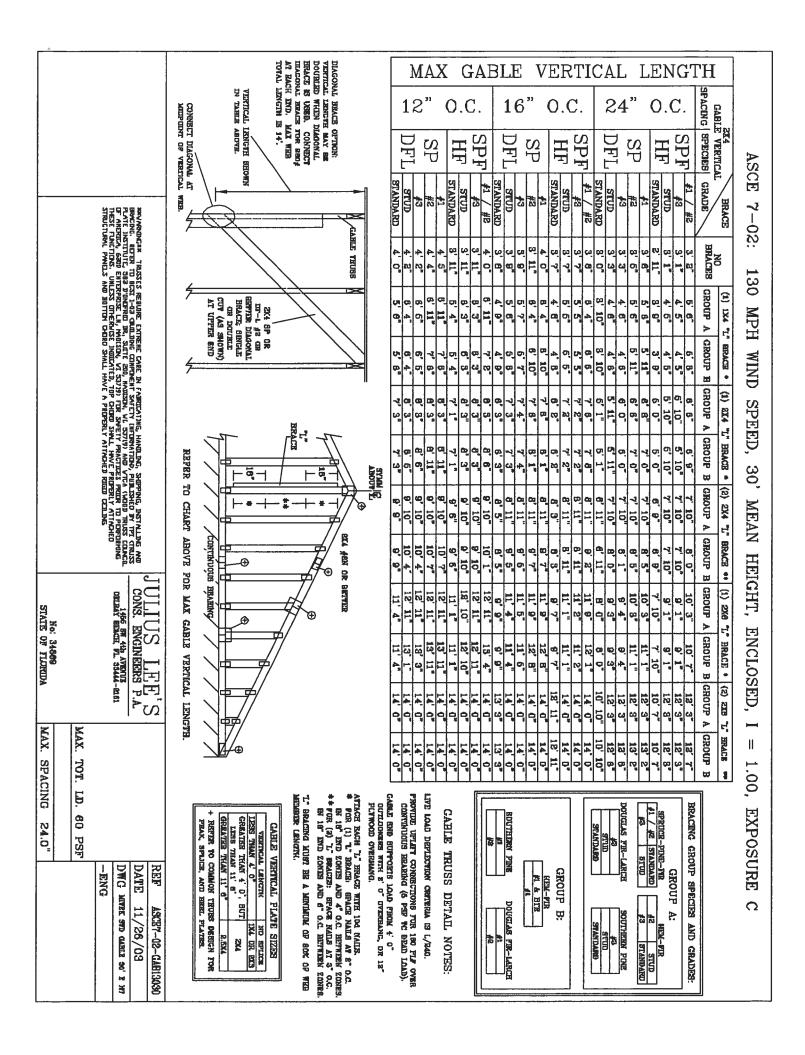
System A General Safety Notes

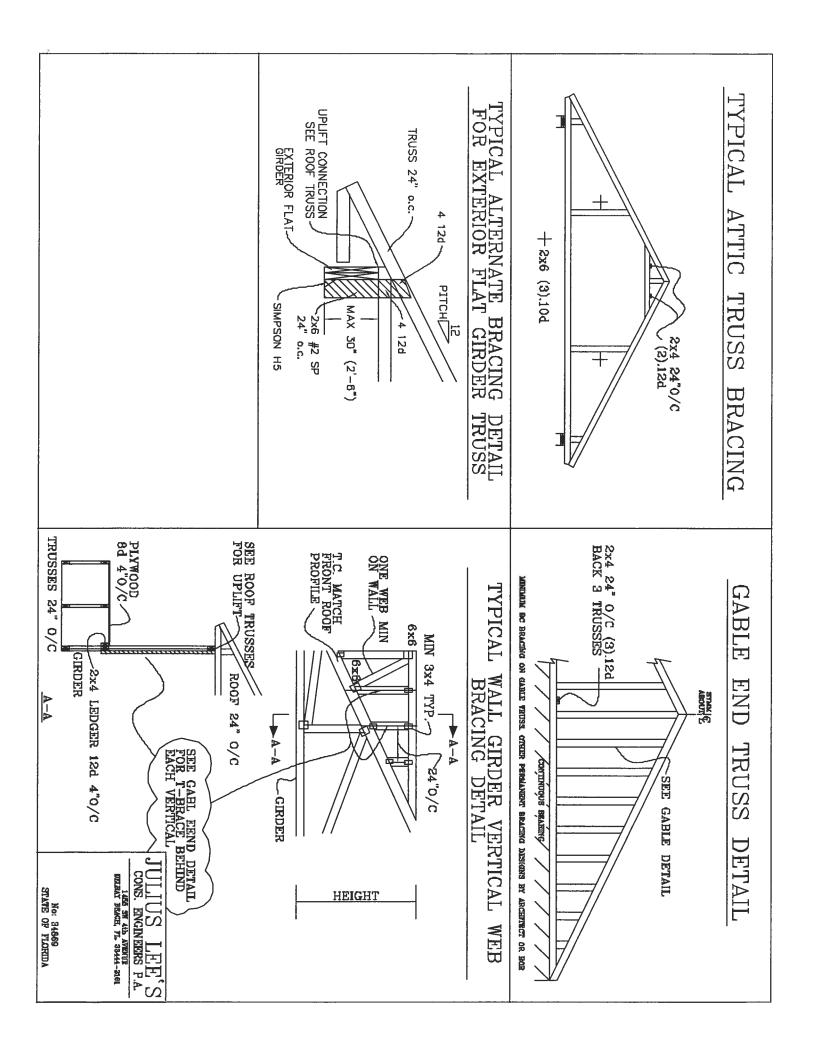
Failure to Follow Could Cause Property Damage or Personal Injury

- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
- Unless otherwise noted, locate chord splices at 1/2 panel length (± 6" from adjacent joint.)
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
 Unless expressly noted, this design is not
- Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection
- Plate type, size and location dimensions shown indicate minimum plating requirements.
- Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
- Top chords must be sheathed or purlins provided at spacing shown on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
- Do not overload roof or floor trusses with stacks of construction materials.
- 14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
- Care should be exercised in handling, erection and installation of trusses.

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TOP CHORD CHORD \$**₹**₹ 金金金 BETTER BETTER

PIGGYBACK DETAIL

TYPE

SPANS

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REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. TRUSS TOP CHORD WITH 1.5X3 PLATE. ATTACH VERTICAL WEBS TO

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUBS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED FURLIN SPACING.

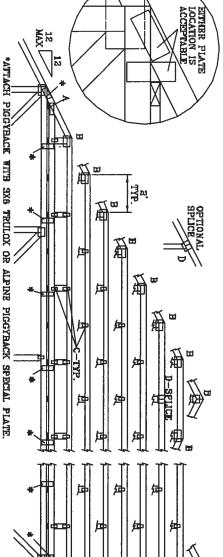
THIS DETAIL IS APPLICABLE FOR THE POLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MBAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I, EXP C, WIND TO DI=5 PSF, WIND BC DI=5 PSF

130 MFH WIND, 30' MEAN HCT, ASCE 7-02, BLDG, LOCATED ANYWHERE IN ROOF, CAT II, WIND TO DL=6 PSF WIND BC DL=6 PSF

110 MPH WIND, 30' MEAN HGT, FEC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TO DL-5 PEF WIND BC DL-5 PEF

FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX. 20' FLAT TOP CHORD MAX SPAN F MAX SIZE OF EXIZ #2 OR HETTER



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5**X**4

6X6

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BXd

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EX3

.6X4

1.5X4

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4X8 234

9XB

9 9

> БХВ 236

2,5X4

2.6X4

2x4 "T" BRACE. SAI 10' TO 14' MEMBER. OR BETTER MEMBER. ATTACH W	7'9" TO 10' MEMBER, OR BETTER, AND MEMBER, ATTACH WITH 8	o' To 7'9" NO BRACING	WEB LENGTH REQU	WEB BRACING	
SAME GRADE, SPECIES AS WEB TER, AND 80% LENGTH OF WEB I WITH 16d NAILS AT 4° OC.	SAME GRADE, SPECIES AS WEB BR, AND 80% LENGTH OF WEB WITH 8d NAMES AT 4" OC.		REQUIRED BRACING	CHART	

ATTACH TEETH TO THE PIGGYBACK AT THE TIME
(4) 0.120" X 1.375" NALS PER FACE PER PLY. PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE SPACE 4' OC OR LESS.

THIS DRAWING REPLACES DRAWINGS 8 1/4" 634,016 634,017 & 647,045 ಬ್ಳ

CONS. ENGINEERS P.A. 1456 SW 4th Avenue No: 94868 STATE OF FLORIDA Ω 1.15 DUR. 1.33 DUR. 50 PSF AT .25 DUR. FAC MAX LOADING 47 PSF 55 PSF AT 33 DUR. FAC 24.0 AT FAC DATE REF DRWG MITEK STD ENG Ħ 09/12/07 PIGGYBACK

PIGGY

SPACING

WAVANING TRESSES REQUIRE CYTROME EME IN FABRICATING, HAMILING, SHIPPING, INTRALLING AND BACHNG REFER TO JEST I-DO GUILLING COMPRISHED FARET INFORMATIN, PUBLICATION PIPE CRIESS PARTE INSTITUTE, 350 GOVERNORD SE, SUITE 20M, MARSON V. 132759 AND WITH CHOOS RESECUTED IN FREE CHARGES IN FARET, 6300 INTERPRESE HE, MARSON, MI 132759 FIRE SAFETY PROFITER TO PERFORMS INFORMATION OF HER PORT OF HER TO PERFORM THE CHOOS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANCES AND BOTTOM CHOOS SHALL HAVE A PROPERLY ATTACHED BIGGO CELLING.

VALLEYTRUSS DETAIL

TOP BOT CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER. 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER. 2X4 SP #3 OR BETTER.

- 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).
- * ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: BUILDING, EXP. C, RESIDENTIAL, WIND TO DI=5 PSF. FEC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 15d ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED (2) 18d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR FOR

EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9". UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEH, VALLEY WEH, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING,

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION

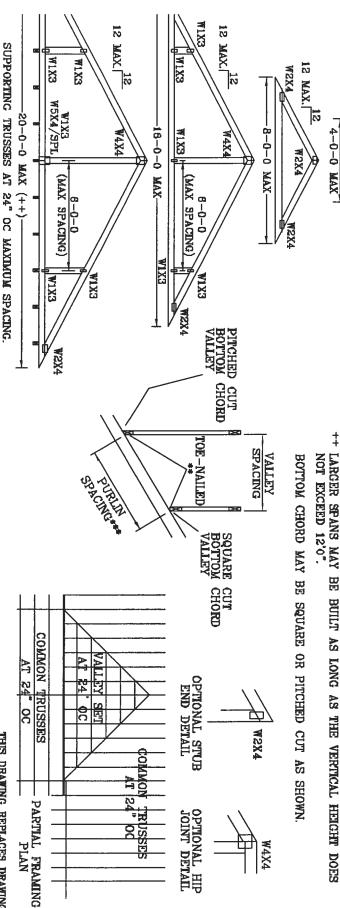
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN

ENGINEERS' SEALED BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON DESIGN.

-

++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

CUT FROM 2X6 OR LARGER AS REQ'D



JOINT DETAIL W4X4

MEMORANHISM TRUSSEX REQUIRE EXTREME EMPRIENT SAFETY AMOUNTS, SHIPPING, DISTALLING AND MANURES REFER TO INCIL I-LID GUILLING CEMPRENT SAFETY AMOUNTS, MAILEND BY THE CRIMINS PLANTED BY THE CRIMINS PLANTED BY THE CRIMINS PLANTED BY THE COLUMNITY OF AMERICA, GOID OFFENDER OF MANUREN WE STOP AND ATTA VOOD THESE COLUMNITY OF AMERICA, GOID GET AMERICA, GOID OFFENDE OFFENDE BY MANUREN WE STOP STRUCTULA TO CHORD SHALL HAVE FOR THE ANTIGHT OF THE CRIMINS THE CRIMINS AND MOTITOR CHORD SHALL HAVE A PROPERLY ATTACHED BIGD CELLING. CONS. ENGINEERS P.A. DELIRAY BEACH, IL SEALAH-EHEI No: 34869 STATE OF FLORIDA $\hat{\Omega}$ BC DI TC DL IC SPACING BC DURFAC 1.25 TOT. LD. F E 80 32 THIS DRAWING REPLACES DRAWING A105 15 엉 1.25 PSF PSF PSF PSF DATE PSF REF DRWG -ENG Ħ VALTRUSS1103 11/26/09 VALLEY DETAIL

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 — EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

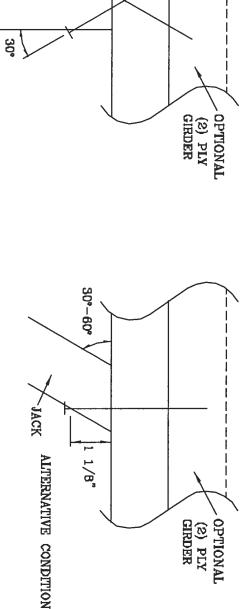
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 18d (0.162"X3.5") COMMON TOE-NAILS

NUMBER OF		SOUTHERN PINE	DOUGLAS	DOUGLAS FIR-LARCH	HEM-FIR	FIR	SPRUCE PINE FIR	PINE FIR
TOE-NAILS	1 PLY	2 PLIES 1 PLY		2 PLIES	1 PLY	1 PLY 2 PLIES	1 PLY	2 PLIES
8	197#	256#	181#	234#	156#	203#	154#	199#
အ	296#	383#	271#	351#	234#	304#	230#	#86Z
.4	394#	611#	361#	468#	312#	406#	307#	397#
თ	493#	639#	452#	585#	390#	507#	384#	496#
ALL VALLE	EG MAY PE		DAY AB UK	I HALVINGOUS	NUMBAGILIA	ALL VALUES WAY BE MILLIMDIED BY ADDRODRIATE DITRATION OF LOAD FACTOR	Calcula.	

ALL VALUES AT WITH MODITION OF AFFINOR NAME DONALION S LOAD PACTOR.



1/B"

JACK

THIS DRAWING REPLACES DRAWING 784040

			STRUCTURAL PANELS AND BOTTON CHORD SHALL HAVE A PROPERLY ATTACHED ROOD CELENG	PLATE INSTITUTE, 389 YOUTRO DR., SUITE 200, NADISCH, VL 337(9) AND VTCA (WIDD TRUSS COLNCIL TAKENDA, 6000 ENTIDENSE, LM, MARISON, VI 337(9) FTR SAFETY PRACTICES PRODE TO PERFORMING THESE THE CONTROL OF THE PROPERTY PRACTICES PRODE TO PERFORMING THESE THE CONTROL OF THE PROPERTY PRACTICES OF THE PROPERTY PROPER	***VARUNCI ** TRUSSES REGURE EXTREME CARE IN FAMILIARING, HARDLING, SHIPPING, INSTALLING AND BRACING. BETTER TO BIXX 1-48 COUNTING COMPINENT SAFETY INFORMATIONS, PILINING IN THE CRUIPS	
STATE OF FLORIDA	No: 34889			DELPAY HEACH, TL SHIFT-ZIEL	CONS. ENGINEERS P.A.	S, HHI SOLINE
SPACING	DUR. FAC.	TOT. LD.	вс ш	BC DL	TC DL	TC LL
	FAC. 1.00	PSF	PSF	PSF	PSF	PSF
			-ENG JL	DR W G	DATE	REF
			JL.	DRWG CNTONAIL1103	DATE 09/12/07	TOE-NAIL

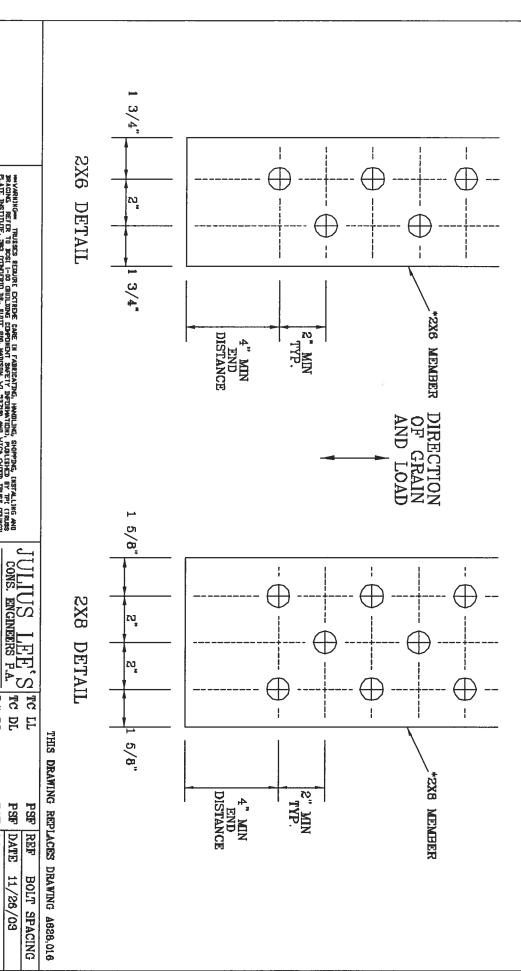
BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN

BOLT HOLES SHALL BE A MINIMUM OF 1/52" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. QUANTITIES AS NOTED ON SEALED DESIGN MUST BE IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



WYARINGO* TRUSSES SECURE EXTEDS CARE IN FABRIATING, JACANES. REFOR TO JESS 1-DO GIULLIONG COMPONITO SAFETY INFO PLATE INSTITUTE, 353 O'DOLFRID JN., SUITE EM, MAUSSIN, VI. 3749 FOR SAFE OF AMERICA, 6500 ENTERPOISE LM, NAIDSON, VI. 3749 FOR SAFE THESE FINCTIONS. UNLESS THERVISE JOHEN ED, TID ENDERS SAFE STRUCTURAL FAMELS AND JOTTOM CHICKO SHALL HAVE A PROPERLY.

DELRAY BEACH, FL 33444—ZIGI

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11/26/09

CNBOLTSP1103

No: 34869 STATE OF FLORIDA

SPACING DUR. FAC TOT. LD.

TRULOX CONNECTION DETA.

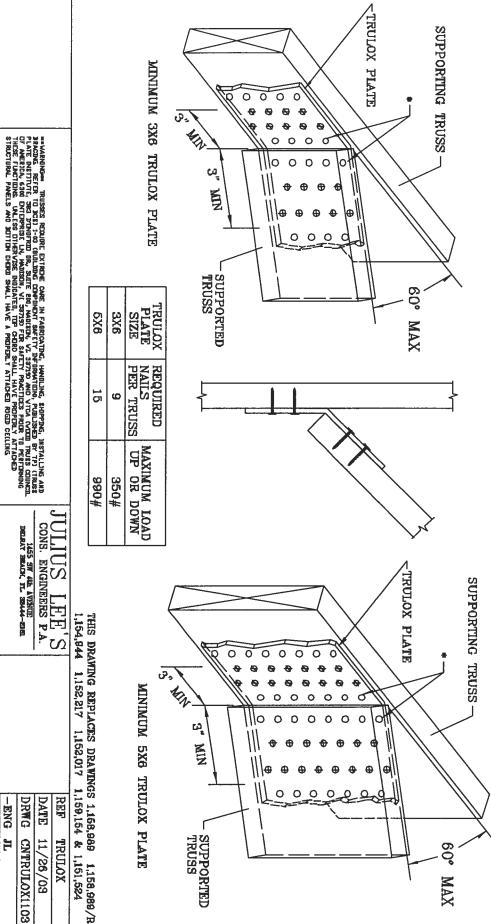
SHOWN (+). 11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE

NAILS MAY BE OMITTED FROM THESE ROWS

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

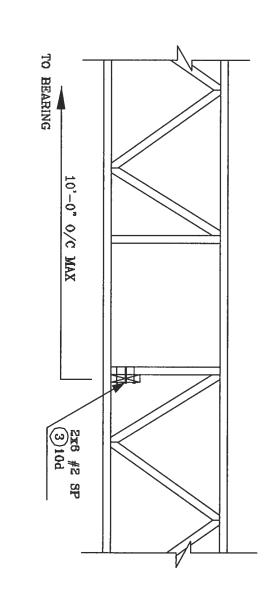
TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING INFORMATION NOT SHOWN. THIS DETAIL FOR LUMBER, PLATES, AND OTHER

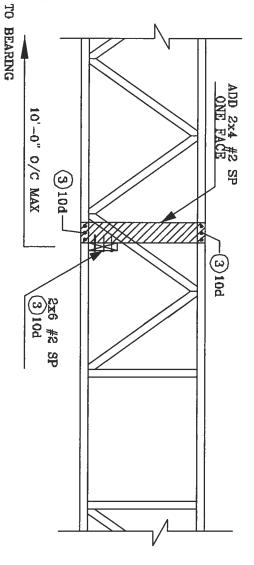


No: 34869 STATE OF FLORIDA

SYSTEM-42 OR FLAT TRUSS

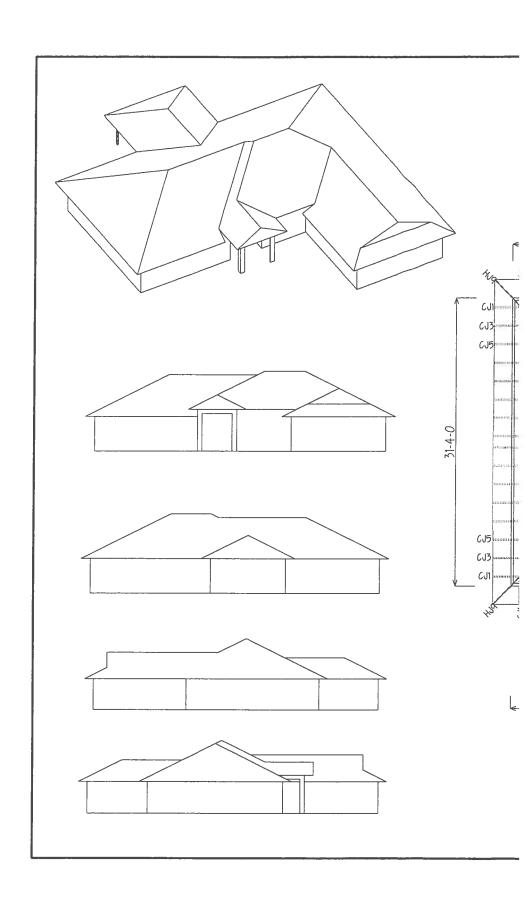


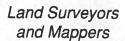
ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



JULIUS LEE'S cons. engineers p.a.

No: 84869 STATE OF FLORIDA







BRITT SURVEYING

830 West Duval Street • Lake City, FL 32055 Phone (386) 752-7163 • Fax (386) 752-5573

04/30/08

L-19273

To Whom It May Concern:

C/o: Trent Giebeig

Re: Lot 48 May-Fair Unit 3

Mulfact

The elevation of the top of the foundation is found to be 147.98 feet. The minimum floor elevation as per the construction plans is 144.50 feet. The centerline of the adjacent road SW Mayfair Lane is 147.18 feet. The highest adjacent grade is 146.41 feet. The lowest adjacent grade is 144.07 feet. The elevations shown hereon are based on NGVD 29 Datum.

L. Scott Britt PLS #5757 ATTN: WEEGIC

Columbia County Building Department Culvert Waiver

Culvert Waiver No. 000001492

DATE: 07/02/2008 BUILDING PERMIT NO	o. <u>C6494</u>
APPLICANT TRENT GIEBEIG	PHONE 397-0545
ADDRESS 697 SE HOLLY TERR	LAKE CITY FL 32025
OWNER PETE GIEBEIG	PHONE 752-7968
ADDRESS 402 SW MAYFAIR LN	LAKE CITY FL 32055
CONTRACTOR TRENT GIEBEIG	PHONE 397-0545
LOCATION OF PROPERTY 247 S, R INTO MAYFAIR S/D.	, 3RD LOT ON THE LEFT PAST
LUCILLE CT	
SUBDIVISION/LOT/BLOCK/PHASE/UNITMAYFAIR	48 3
PARCEL ID # 11-4S-16-02911-348	
I HEREBY CERTIFY THAT LUNDERSTAND AND WILL FULLY COUNTY PUBLIC WORKS DEPARTMENT IN CONNECTION V SIGNATURE:	
A SEPARATE CHECK IS REQUIRED MAKE CHECKS PAYABLE TO BCC	Amount Paid 50.00
PUBLIC WORKS DEPARTM	IENT USE ONLY
I HEREBY CERTIFY THAT I HAVE EXAMINED THIS APPLICA CULVERT WAIVER IS:	TION AND DETERMINED THAT THE
APPROVED	NOT APPROVED - NEEDS A CULVERT PERMI
COMMENTS:	

ANY QUESTIONS PLEASE CONTACT THE PUBLIC WORKS DEPARTMENT AT 386-752-5955.

135 NE Hernando Ave., Suite B-21

Lake City, FL 32055

Phone: 386-758-1008 Fax: 386-758-2160



New Construction Subterranean Termite Soil Treatment Record

OMB Approval No. 2502-0525

This form is completed by the licensed Pest Control Company.

Public reporting burden for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.

Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Operator and builder,	unless stated othe	rwise. # 26494	
Section 1: General Information (Treating Company Information)		4- 11	
Company Name: Aspen Post Centrol, Inc.			
Company Address: 301 RTV Cela Terrace			
Company Business License No.			383-755-3611
FHA/VA Case No. (if any)			
Section 2: Builder Information			
Company Name: Trent Giebeig Const.		_ Company Phone No	397 <i>-0545</i>
Section 3: Property Information			
Location of Structure(s) Treated (Street Address or Legal Description, Ci	1.00	407 SW 1	Maufair Ln.
Mayfair S/D		/	
Type of Construction (More than one box may be checked) Slab Approximate Depth of Footing: Outside	Basement	Crawl Typ	Other
Brand Name of Product(s) Used EPA Registration No. Approximate Final Mix Solution % Approximate Size of Treatment Area: Sq. ft.	near ft	Linear ft. of N	Masonry Voids 240
Name of Applicator(s) 5. Gregory	Certification No	(if required by State law)	JF104976
The applicator has used a product in accordance with the product label and state federal regulations.	e requirements. All		=1,1,0
Authorized Signature //www.mon. // Vagouy	· · · · · · · · · · · · · · · · · · ·	Date	3/0/00

Warning: HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010. 1012; 31 U.S.C. 3729, 3802)



O C C T A Z C Y

COLUMBIA COUNTY, FLORIDA

HANDADAMAKAN ANAMAHAN MARAMAKAN MARAMAKAN BANDAN MANDAN MANDAN MANDAN MANDAN MANDAN MANDAN MANDAN MANDAN MANDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building

and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 11-4S-16-02911-348

Fire: 57.78

Building permit No. 000026494

14/11/11/11/11

Waste: 150.75

Total: 208.53

Location: 402 SW MAYFAIR LANE, LAKE CITY, FL

Owner of Building PETE GIEBEIG

Permit Holder TRENT GIEBEIG

Use Classification SFD, UTILITY

Date: 01/21/2009

Building Inspector

POST IN A CONSPICUOUS PLACE (Business Places Only)