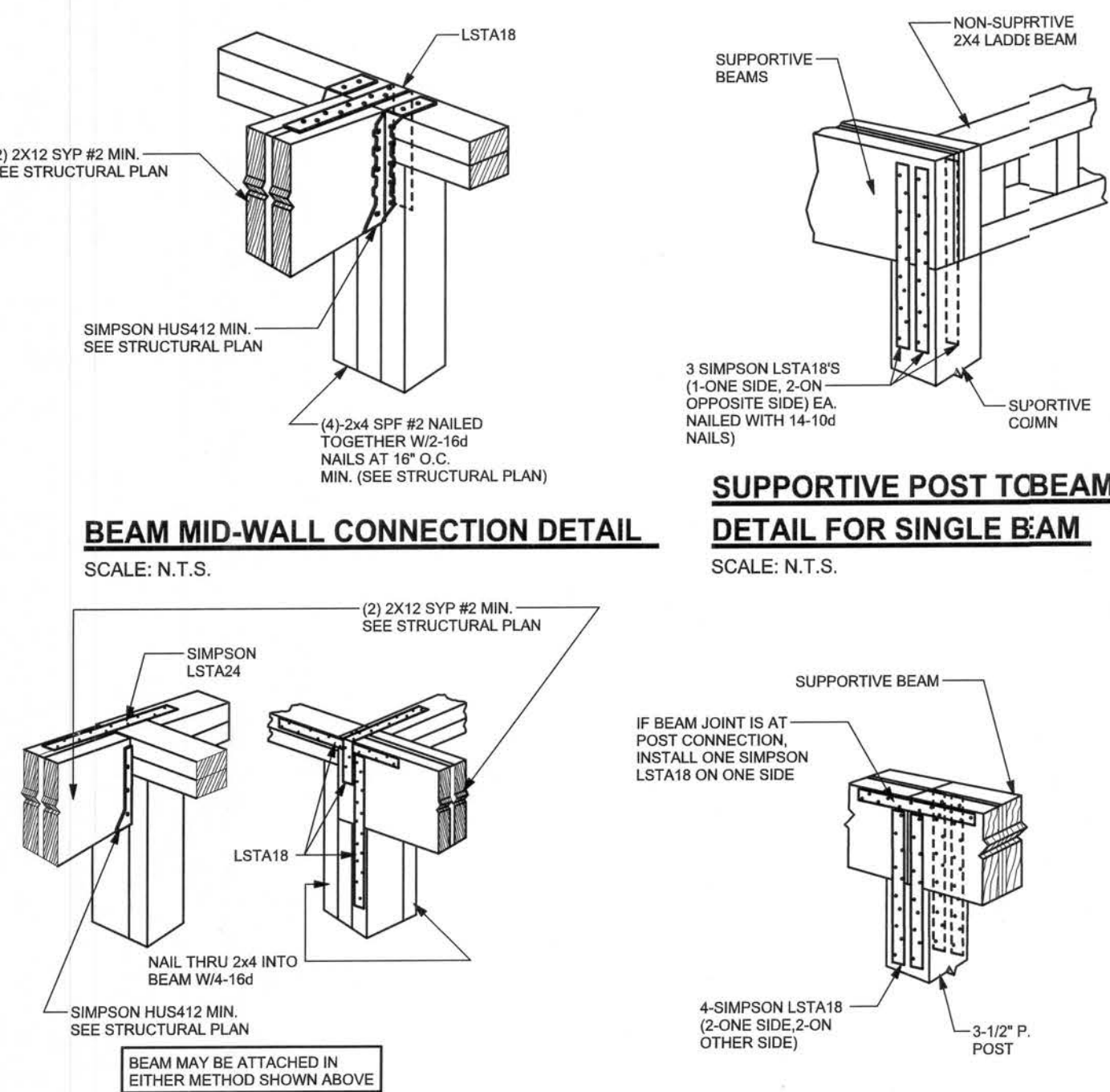


**ONE STORY WALL SECTION**  
SCALE: 3/4" = 1'-0"

**EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS**

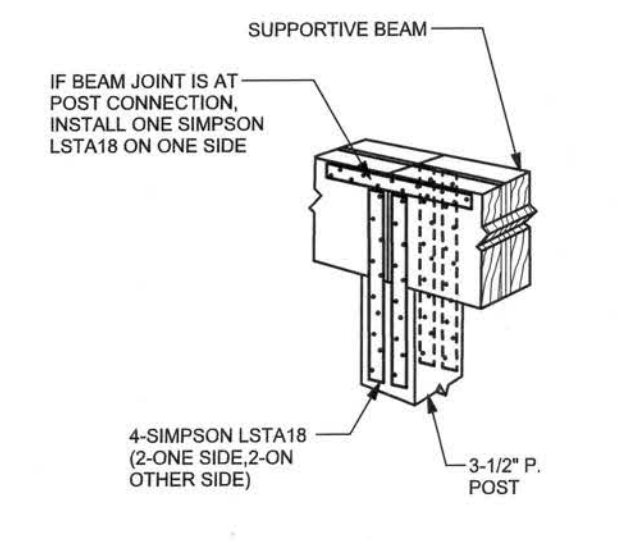
(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.208, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.



**BEAM CORNER CONNECTION DETAIL**  
SCALE: N.T.S.

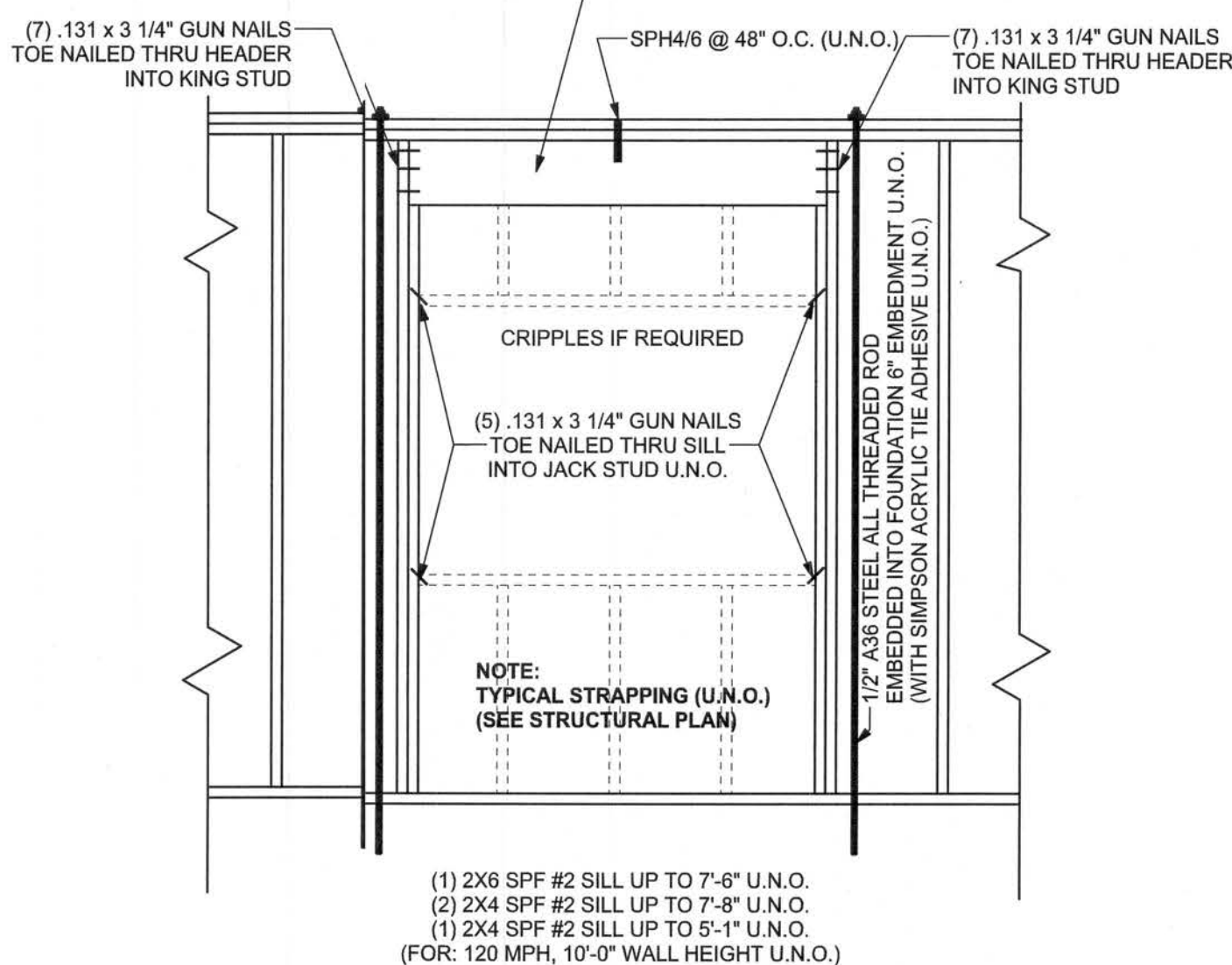
**SUPPORTIVE POST TO BEAM DETAIL FOR SINGLE BEAM**  
SCALE: N.T.S.



**SUPPORTIVE CENTER POST TO BEAM DETAIL**  
SCALE: N.T.S.

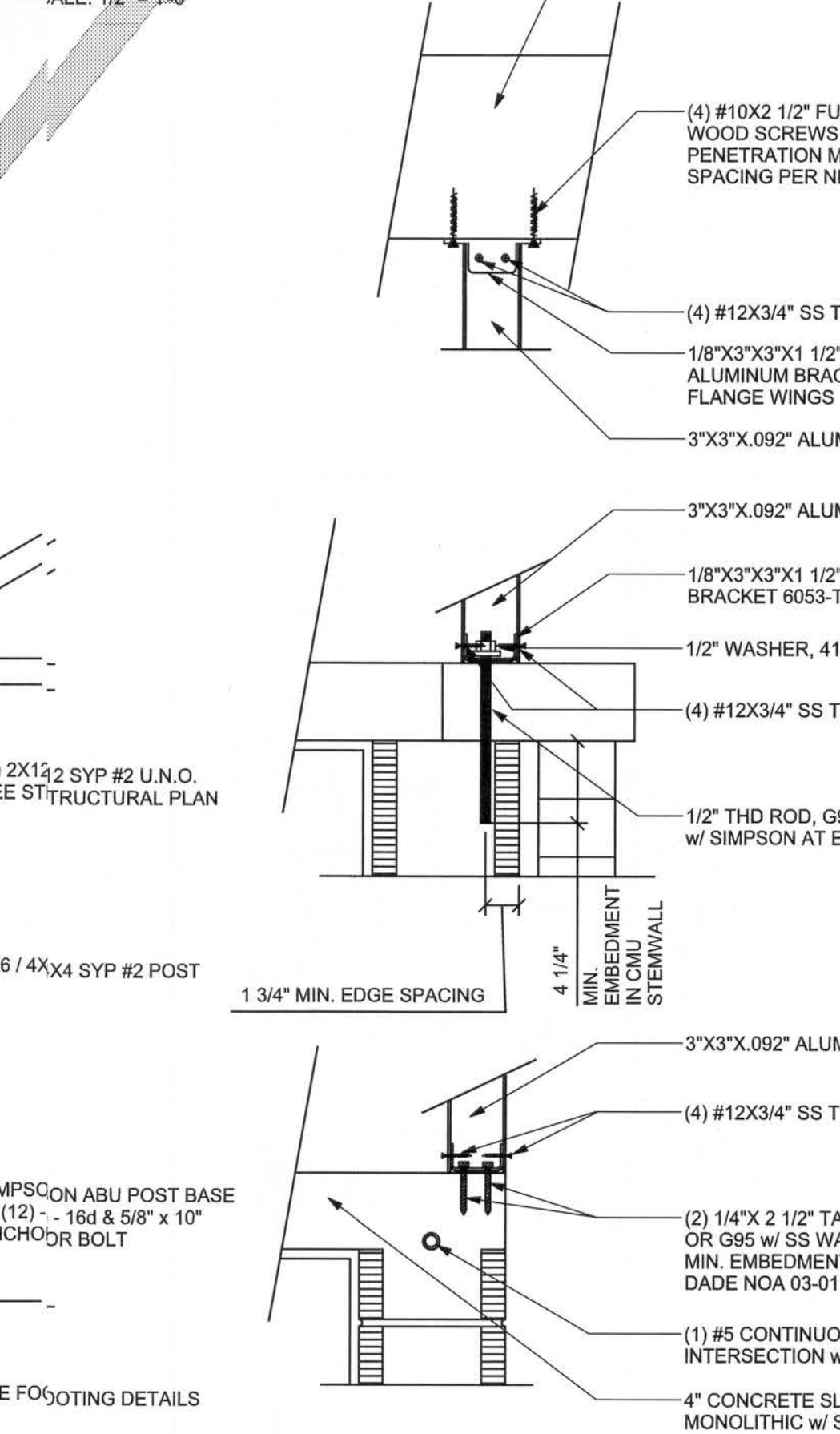
NOTE:  
IF TRUSS TO WALL STRAPS ARE NAILED TO THE HEADER THE SPH4/6 @ 48" O.C. ARE NOT REQUIRED

FOR LESS THAN 1500 lb UPLIFT USE 2 X 2 X 1/8" WASHER  
FOR LESS THAN 3750 lb UPLIFT USE 3 X 3 X 1/8" WASHER



**TYPICAL 1 STORY HEADER STRAPPING DETAIL**  
SCALE: 1/2" = 1'-0"

**OPTIONAL THREADED ROD TO FLOOR BEAM OR FLOOR JOIST**  
SCALE: 1/2" = 1'-0"



**4X4 / 6X6 PORCH POST DETAIL**  
SCALE: 1/2" = 1'-0"

**OPTIONAL ALUMINUM PORCH POST & HEADER ANCHORS**  
SCALE: N.T.S.

**ANCHOR TABLE**

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	HSA	3-8d	3-8d	
< 455	< 265	HS	4-8d	4-8d	
< 360	< 235	HM	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H8	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	10-10d, 1 1/2"	10-10d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 890	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2-HTS24			
< 2050	< 1785	LG72	14-16d	14-16d	
<b>HEAVY RIGID TIE-DOWNS*</b>					
< 3965	< 3330	MG7		22-10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
<b>STUD STRAP CONNECTOR*</b>					
< 435	< 435	SSP DOUBLE TOP PLATE	3-10d		4-10d
< 455	< 420	SSP SINGLE SILL PLATE	1-10d		4-10d
< 825	< 825	DSP DOUBLE TOP PLATE	6-10d		8-10d
< 825	< 600	DSP SINGLE SILL PLATE	2-10d		8-10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
<b>STUD ANCHORS*</b>					
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTT131	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT18	18-16d		5/8" AB
< 1400	< 1400	RAH42	16-16d		
< 3335	< 3335	HPAH22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU68	12-16d		1/2" AB
< 2320	< 2320	ABU88	18-16d		2-5/8" AB

**GENERAL NOTES:**

**TRUSSES:** TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END, 2X6 RAFTERS 700 LB EACH END.

**SITE PREPARATION:** SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

**FOUNDATION:** CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

**CONCRETE:** MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS,  $F_c = 3000$  PSI.

**WELDED WIRE REINFORCED SLAB:** 6" x 8" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A188, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 9".

**FIBER CONCRETE SLAB:** CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

**CONTROL JOINTS:** WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W/M OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

**REBAR:** ASTM A615, GRADE 60, DEFORMED BARS,  $F_y = 60$  KSI. ALL LAP SPLICES 40" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

**GLULAM BEAMS:** GLULAM BEAM, GLB, 24F-V3SP,  $F_b = 2.4$  ksi,  $E = 1800$  ksi. UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS. 7/16" OSB SHEATHING, UNBLOKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4"OC UNO.

**STRUCTURAL CONNECTORS:** MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT, AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

**ANCHOR BOLTS:** A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED CONCRETE OR 10" IN GROUTED CMU.

**WASHERS:** WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64" WITH 5/8" BOLTS TO BE 3" x 3" x 9/64" WITH 3/4" BOLTS TO BE 3" x 3" x 9/64". UNO.

**NAILS:** ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

**BUILDER'S RESPONSIBILITY**

**THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.**

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS; TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

**ROOF SYSTEM DESIGN**

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

**GRADE & SPECIES TABLE**

		Fb (psi)	E (10 <sup>6</sup> psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

**DESIGN DATA**

**WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1**

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60 FT IN EXP. B, 30 FT IN EXP. C AND > 10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50X HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE  
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

1. BASIC WIND SPEED = 110 MPH
2. WIND EXPOSURE = B
3. WIND IMPORTANCE FACTOR = 1.0
4. BUILDING CATEGORY = II
5. ROOF ANGLE = 10-45 DEGREES
6. MEAN ROOF HEIGHT = < 30 FT
7. INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
8. COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone	Effective Wind Area (ft <sup>2</sup> )			
	10	100		
1	19.9	-21.8	18.1	-18.1
2	19.9	-25.5	18.1	-21.8
2 On		-40.6		-40.6
3	19.9	-25.5	18.1	-21.8
3 On		-40.3		-42.4
4	21.8	-23.6	18.5	-20.4
5	21.8	-29.1	18.5	-22.6
Doors & Windows				
Worst Case (Zone 5, 10 ft <sup>2</sup> )		21.8	-29.1	
8x7 Garage Door		19.5	-22.9	
16x7 Garage Door		18.5	-21.0	

DESIGN LOADS	
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, < 3:12)
ROOF	20 PSF (FLAT OR < 4:12)
	16 PSF (4:12 TO < 12:12)
	12 PSF (12:12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
SOIL BEARING CAPACITY	1000PSF
NOT IN FLOOD ZONE (BUILDER TO VERIFY)	

REVISIONS	



WINDLOAD ENGINEER: Mark Disoway, P.E. No. 33515, FCB 698, Lake City, FL 32055, 386-754-5419

**DIMENSIONS:** Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.

MARK DISOWAY  
P.E. 030816  
25 Jul 07  
SEAL

**Isaac Construction**

Nancy Ferguson  
Residence

Mark Disoway P.E.  
P.O. Box 868  
Lake City, Florida 32056  
Phone: (386) 754 - 5419  
Fax: (386) 269 - 4871

PRINTED DATE:  
July 25, 2007

DRAWN BY:  
David Disoway

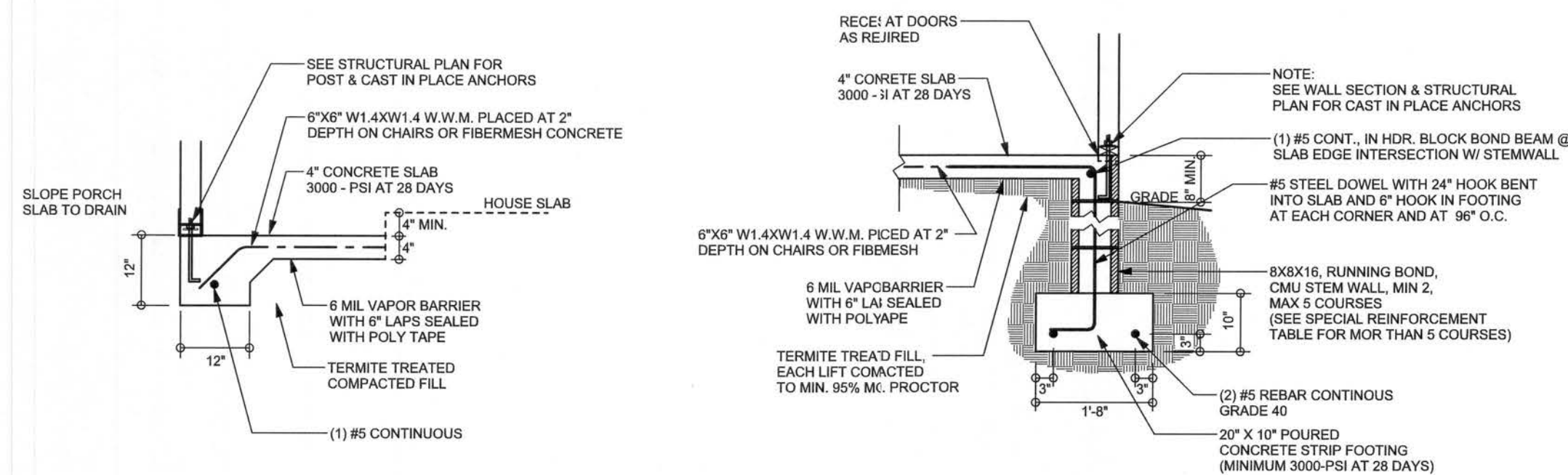
CHECKED BY:

FINALS DATE:  
18 / Jul / 07

JOB NUMBER:  
707172

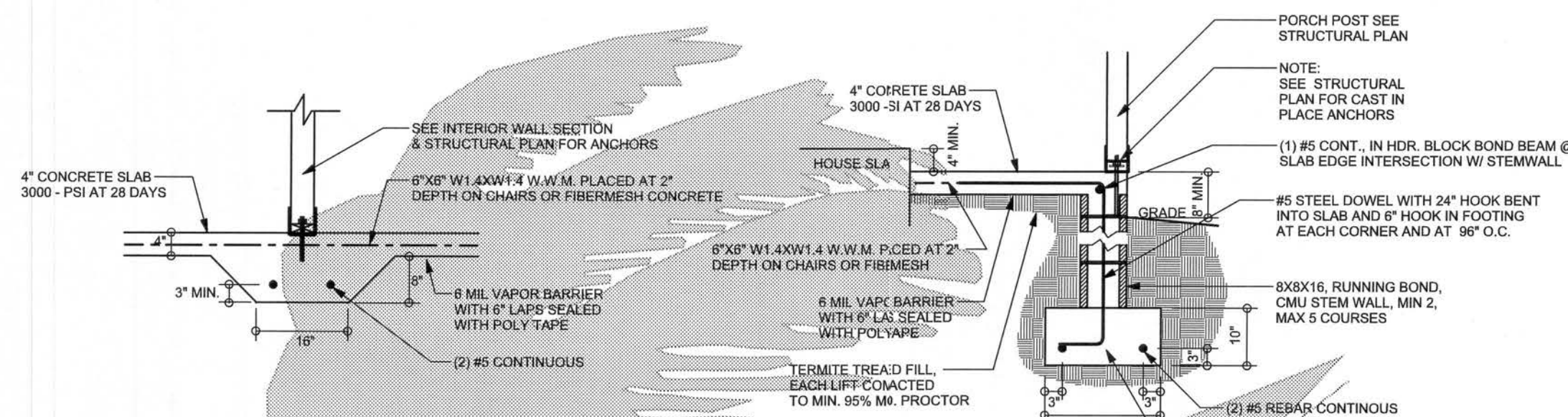
DRAWING NUMBER  
**S-1**  
OF 3 SHEETS





F5 PORCH FOOTING  
S-2 SCALE: 1/2" = 1'-0"

F9 STEM WALL FOOTING  
S-2 SCALE: 1/2" = 1'-0"



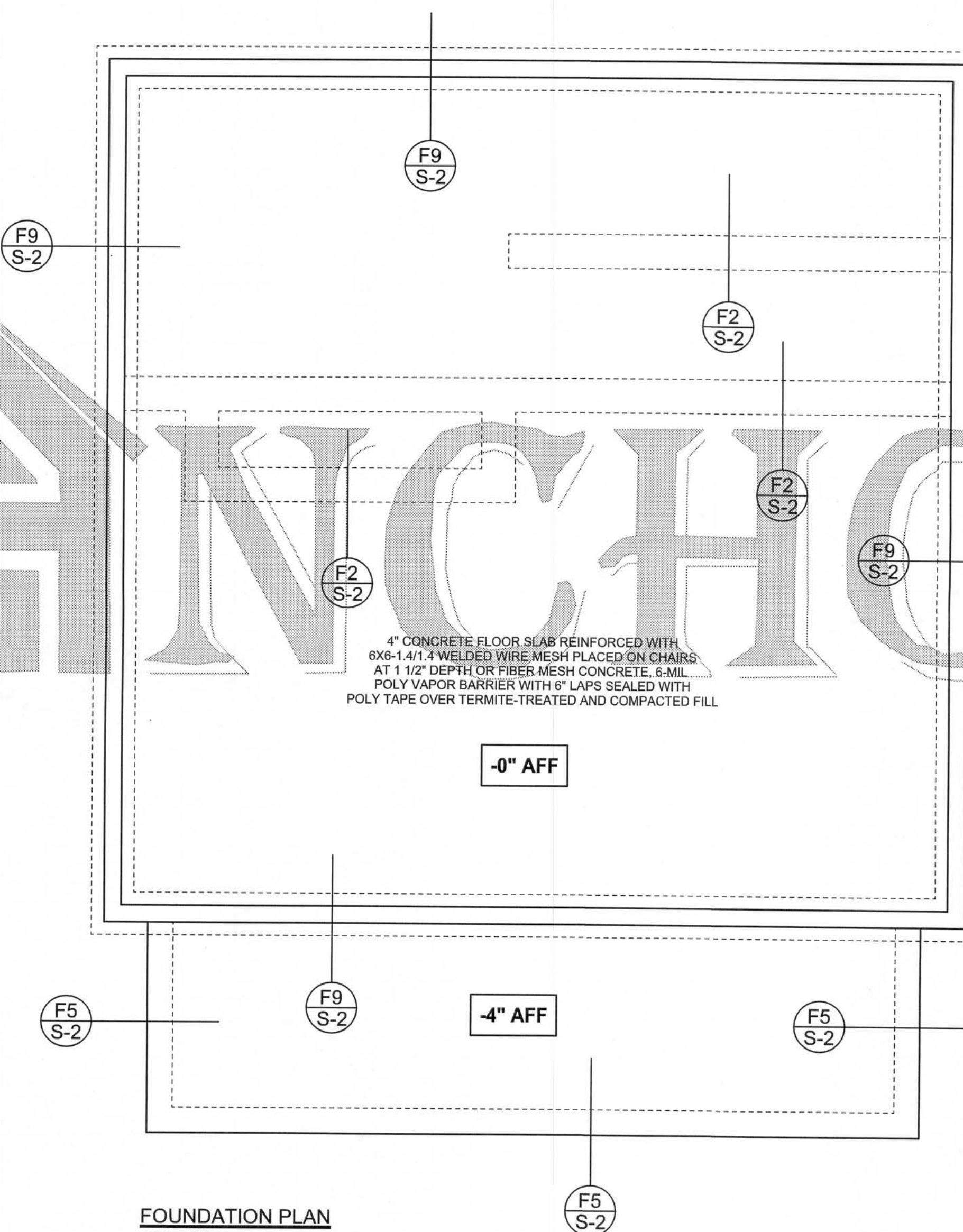
F2 INTERIOR BEARING FOOTING  
S-2 SCALE: 1/2" = 1'-0"

F12 ALT. STEM WALL PORCH FOOTING  
S-2 SCALE: 1/2" = 1'-0"

#### ALL STEM WALL TABLE

This table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the rebar extension at the top. The vertical side is to be placed toward the tension side of the CU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 9' high, add Diagonal ladder reinforcement at 16" O.C. vertically or a horizontal bond bar with 1/6" continuous at mid height. For higher parts of the wall 12" CMU may be used in reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



FOUNDATION PLAN  
SCALE: 1/4" = 1'-0"

DIMENSIONS ON STRUCTURAL SHEETS  
ARE NOT EXACT. REFER TO ARCHITECTURAL  
FLOOR PLAN FOR ACTUAL DIMENSIONS

WINDLOAD ENGINEER: Mark Disoway,  
PE, NCS015, P.O. Box 868, Lake City, FL  
32056-754-5419

DIMENSIONS:  
Standard dimensions supercede scaled  
dimensions. Refer all questions to  
Mark Disoway, P.E. for resolution.  
Do not proceed without clarification.

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form or manner without first this express written  
permission and consent of Mark Disoway.

CERTIFICATION: I hereby certify that I have  
examined this plan, and that the applicable  
portion of the plan, relating to wind engineering,  
complies with section R301.2.1, Florida building  
code residential 2004, to the best of my  
knowledge.

LIMITATION: This design is valid for one  
building, at specified location.

MARK DISOWAY  
P.E. 3/3/15  
25 JUL 07  
SEAL

Isaac Construction

Nancy Ferguson  
Residence

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PRINTED DATE:  
July 25, 2007

DRAWN BY:  
Days Disoway

CHECKED BY:

FINAL DATE:  
18 JUL 07

JOB NUMBER:  
707172

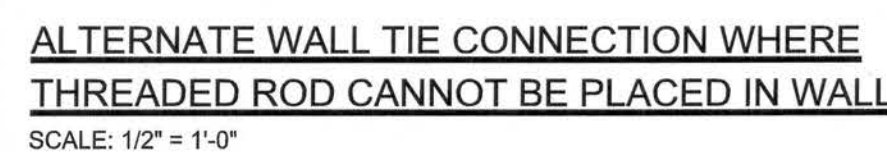
DRAWING NUMBER

S-2  
OF 3 SHEETS





**SOFTPLAN**  
ARCHITECTURAL DESIGN SOFTWARE



- |      |  |
|------|--|
| SN-1 | ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)  |
| SN-2 | ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)  |
| SN-3 | DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS   |
| SN-4 | PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BC511-03, BC511-B1, BC511-B2, & BC511-B3. BC511-B1, BC511-B2, & BC511-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE |

- INDICATES LOCATION OF:  
1ST FLOOR 1/2" A307 ALL THREADED ROD
- ⊗ INDICATES LOCATION OF:  
2ND FLOOR 1/2" A307 ALL THREADED ROD

- 
- (2) 2X12X0', 1J 1K
- HEADER/BEAM CALL-OUT (U.N.O.)
  - NUMBER OF KING STUDS (FULL LENGTH)
  - NUMBER OF JACK STUDS (UNDER HEADER)
  - SPAN OF HEADER
  - SIZE OF HEADER MATERIAL
  - NUMBER OF PLIES IN HEADER

- |              |          |        |
|--------------|----------|--------|
|              | REQUIRED | ACTUAL |
| TRANSVERSE   | 38.0'    | 45.0'  |
| LONGITUDINAL | 28.6'    | 46.0'  |

- |                          |  |
|--------------------------|--|
| <p><b>SWS = 0.0'</b></p> | 1ST FLOOR EXTERIOR WALL                                      |
| <p><b>SWS = 0.0'</b></p> | 2ND FLOOR EXTERIOR   |
| <p><b>IBW</b></p>        | 1ST FLOOR INTERIOR BEARING WALLS<br>SEE DETAILS ON SHEET S-1 |
| <p><b>IBW</b></p>        | 2ND FLOOR INTERIOR BEARING WALLS<br>SEE DETAILS ON SHEET S-1 |

CONNECTIONS, WALL, & HEADER DESIGN IS BASED  
ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING  
FURNISHED BY BUILDER. ANDERSON TRUSS  
JOB #7079

**LOAD ENGINEER:** Mark Discovsky,  
No.53915, P.O.B. 663, Lake City, FL  
56, 386-754-5419

**DIMENSIONS:**  
Detailed dimensions supercode scaled  
versions. Refer all questions to  
Mark Discovsky, P.E. for resolution.  
Do not proceed without clarification.

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permission and consent of Mark Discovsky.

**ATTESTATION:** I hereby certify that I have  
examined this plan, and that the applicable  
provisions of the plan, relating to wind engineering  
comply with section R301.21, Florida building  
code-residential 2004, to the best of my  
knowledge.

**ATTESTATION:** This design is valid for one  
year.

MARK DISOSWAY  
P.E. 53915

Isaac Construction

Nancy Ferguson  
Residence

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PRINTED DATE:  
July 25, 2007

DRAWN BY: David Disosway	CHECKED BY:
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INALS DATE:	
8 / Jul / 07	

JOB NUMBER:  
707172

DRAWING NUMBER

**S-3**

OF 3 SHEETS





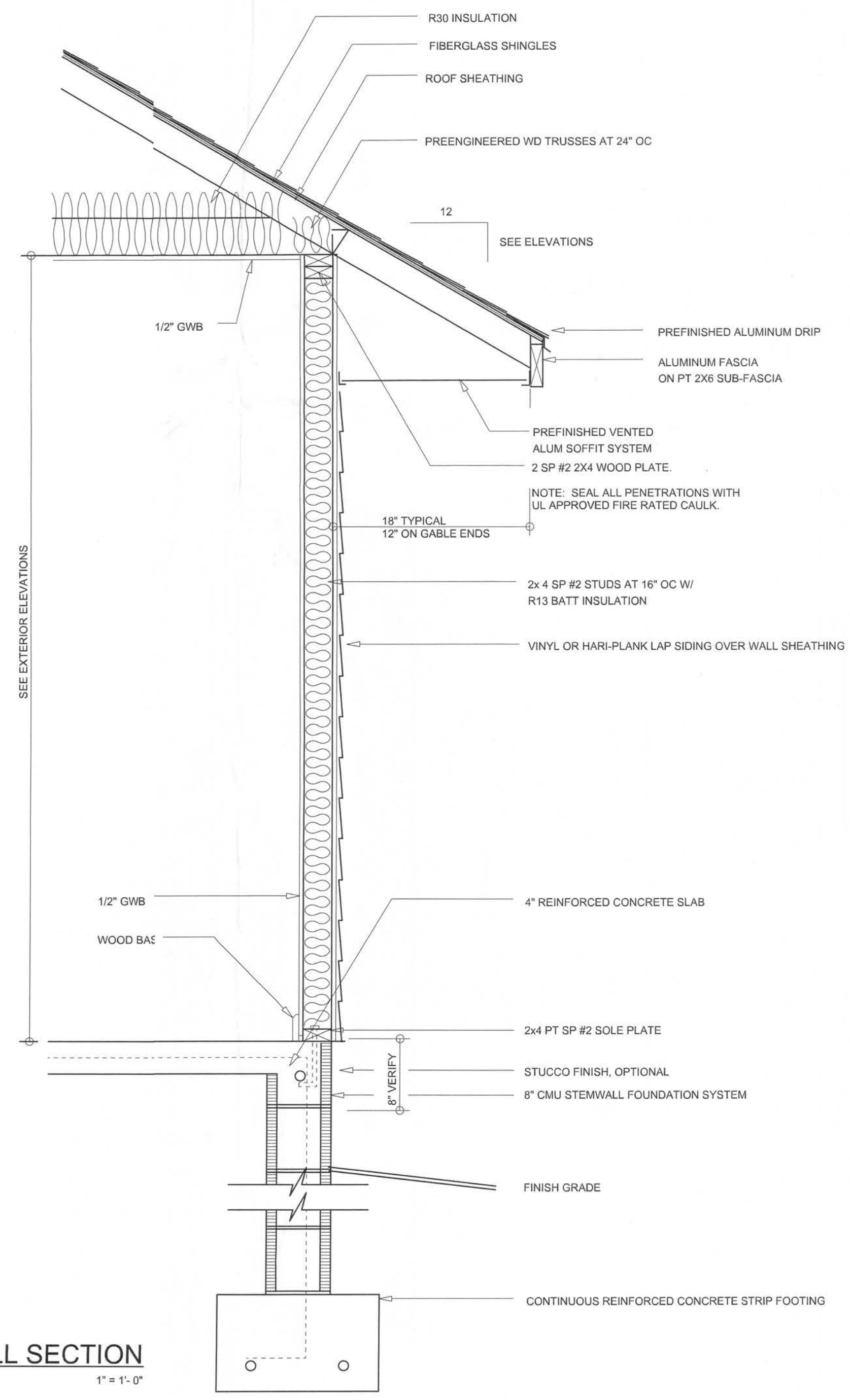




**REAR ELEVATION**  
SCALE: 1/4" = 1'-0"



**FRONT ELEVATION**  
SCALE: 1/4" = 1'-0"



**TYPICAL WALL SECTION**  
SCALE: 1" = 1'-0"



**RIGHT ELEVATION**  
SCALE: 1/4" = 1'-0"

REVISIONS
June 21, 2007

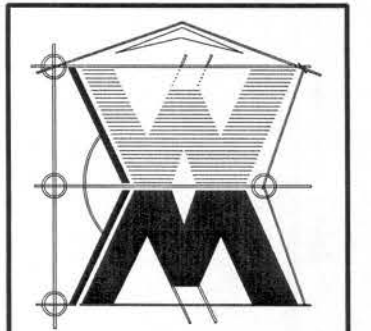
**SOFTPLAN**  
ARCHITECTURAL DESIGN SOFTWARE

**TYPICAL WALL SECTION**  
SCALE: 1/4" = 1'-0"

**EXTERIOR ELEVATIONS**  
SCALE: 1/4" = 1'-0"

A NEW CUSTOM HOME FOR:  
**NANCY FERGUSON**  
PROJECT ADDRESS:  
**ISAAC CONSTRUCTION**  
125 SW MIDTOWN PLACE, SUITE 101, LAKE CITY, FL 32055

©WILLIAM MYERS  
**DESIGN**  
P.O. BOX 1513  
LAKE CITY, FL 32056  
(386) 758-8406  
will@willmyers.net

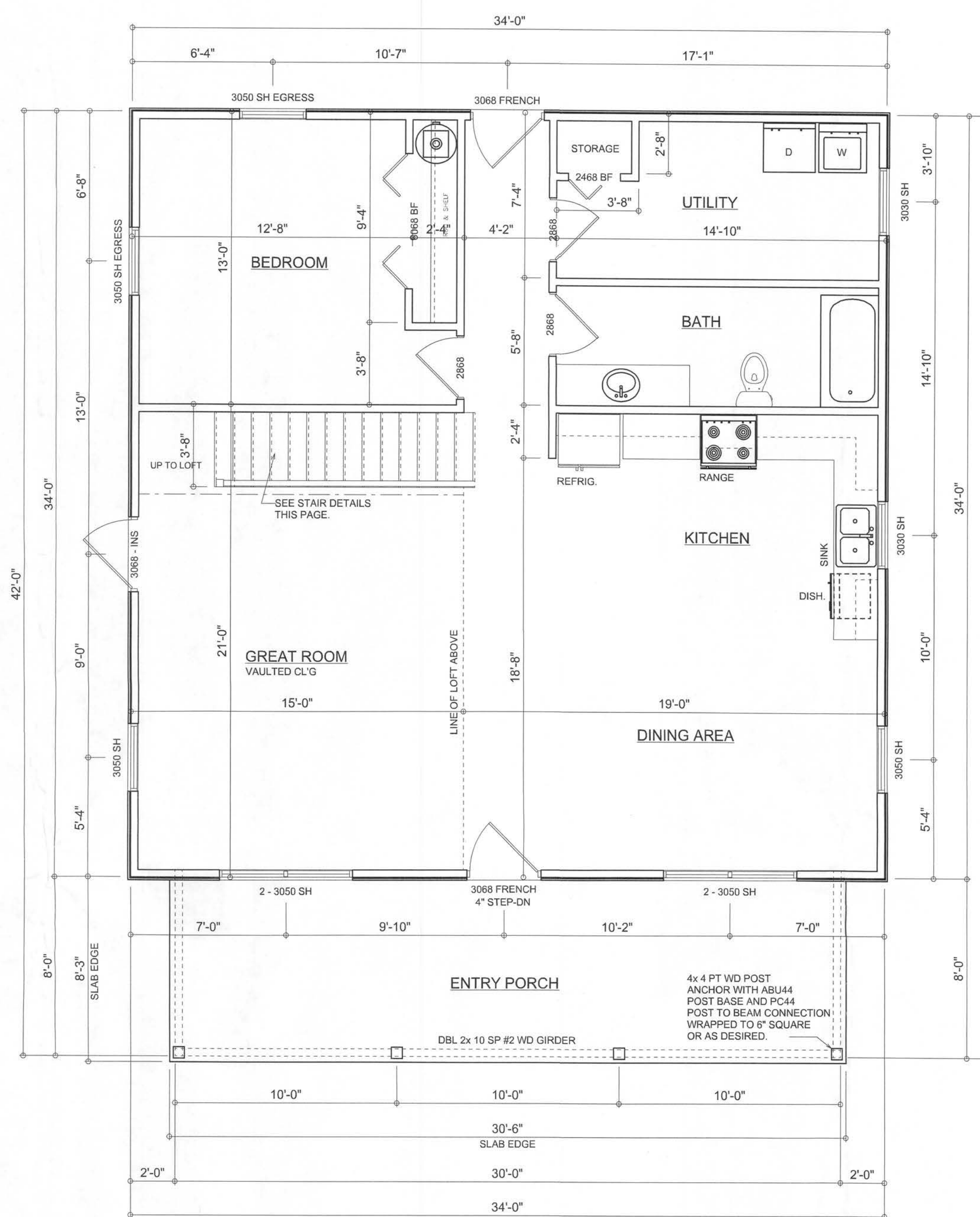


JOB NUMBER  
**070604**

SHEET NUMBER  
**A.1**  
OF 3 SHEETS

*Will C. Myers*





## MAIN FLOOR PLAN

SCALE:  $1/4" = 1'-0"$

AREA SUMMARY		
LIVING AREA	1156	S.F.
LOFT AREA	380	S.F.
ENTRY PORCH AREA	240	S.F.
TOTAL AREA	1776	S.F.

REVISIONS
June 21, 2007

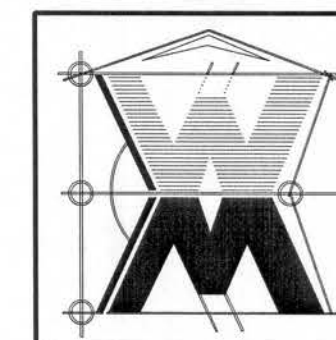
**SOFTPLAN**

MAIN FLOOR PLAN  
SCALE: 1/4" = 1'-0"

LOFT FLOOR PLAN  
SCALE: 1/8" = 1'-0"

A NEW CUSTOM HOME FOR:  
**NANCY FERGUSON**  
 PROJECT ADDRESS:  
**SAC CONSTRUCTION**  
 25 SW MIDTOWN PLACE, SUITE 101, LAKE CITY, FL 32055

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JOB NUMBER  
070604

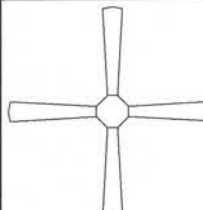













SHEET NUMBER

## A.2

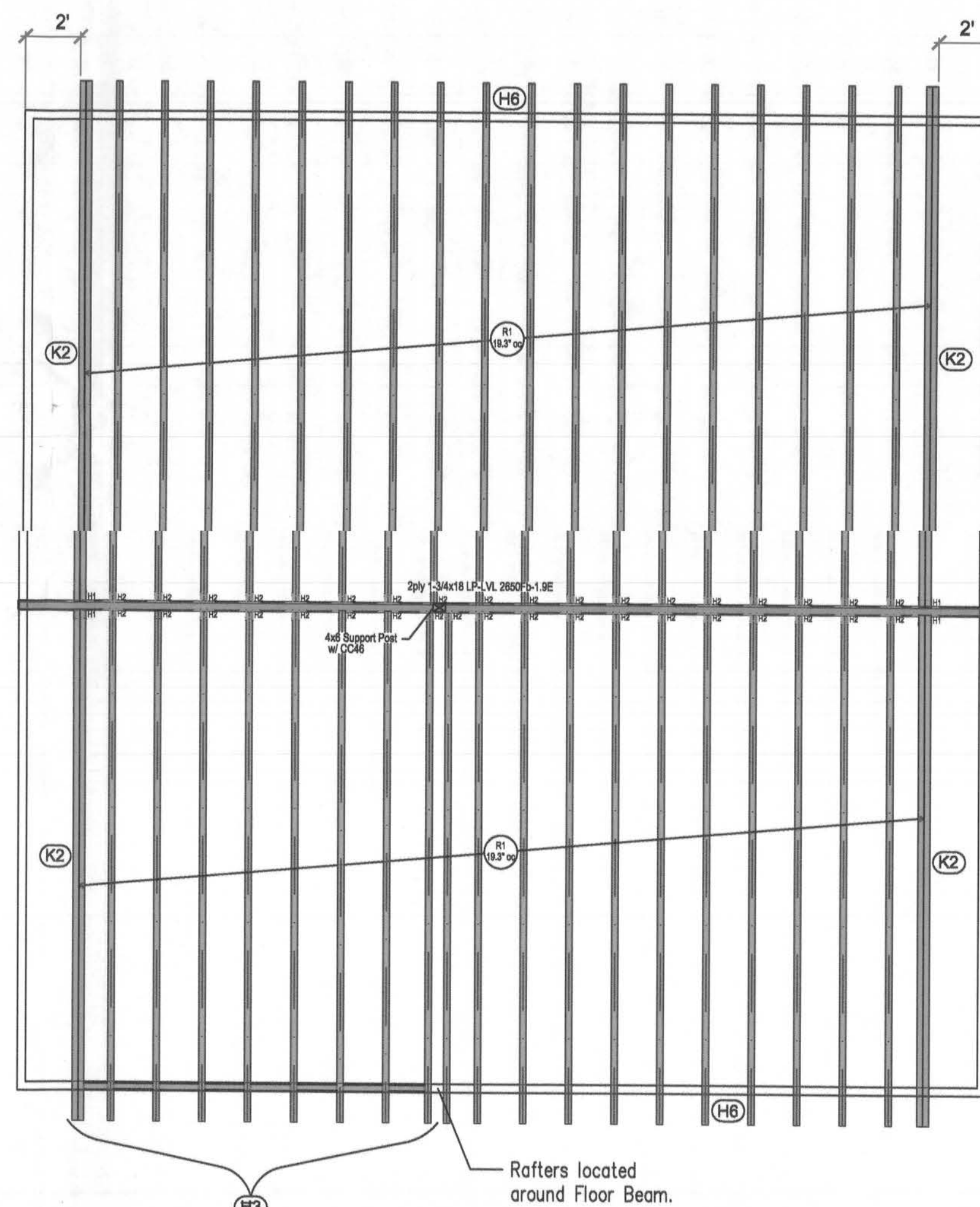
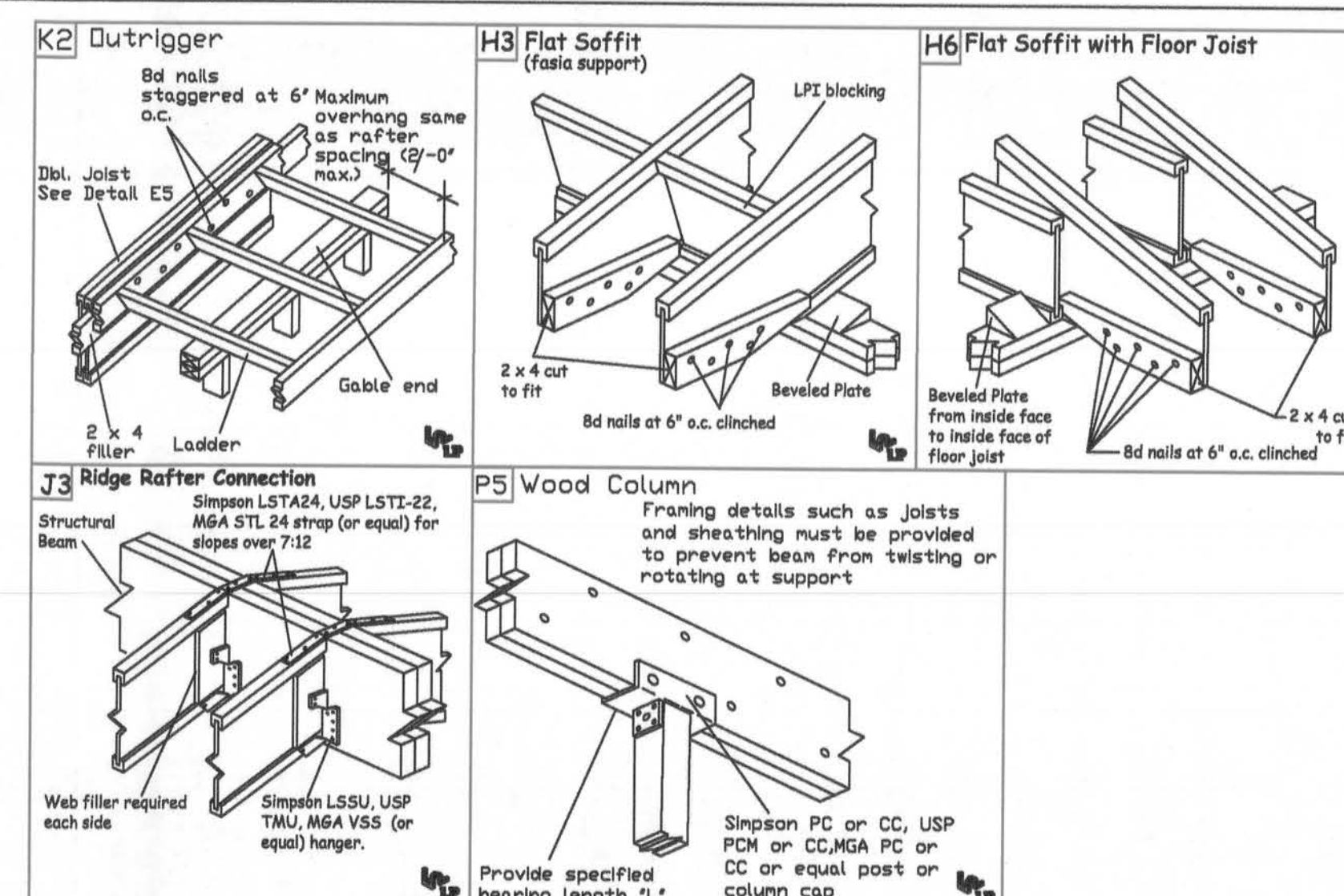
OF 3 SHEETS

Wahl C-777



ELECTRICAL LEGEND	
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	RECESSED CAN LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET
	220v OUTLET
	GFI DUPLEX OUTLET
	TELEVISION JACK
	TELEPHONE JACK
	SMOKE DETECTOR (see note below)
	WALL SWITCH
	3 WAY WALL SWITCH
	WATER PROOF GFI OUTLET
	2 OR 4 TUB FLOURESCENT FIXTURE





## ROOF PLACEMENT PLAN

Beam Schedule			
Label	Qty	Description	Length
M1	2	1-3/4x18 LP-LVL 2B5	36'-0"
Rafter Schedule			
Label	Qty	Description	Length
R1	45	14" LPI 20Plus	25'-0"
Connector Schedule			
Label	Qty	Description	
H2	37	LSSUH310	
H3	1	CC48	
Blocking Schedule			
		Description	Length
		14" LPI 20Plus	13' L F

**NOTICE:**





PLACEMENT PLAN IS OFFERED AS A SALES PRESENTATION  
HAS NOT BEEN REVIEWED OR APPROVED BY AN ENGINEER.

STRUCTURAL ANALYSIS HAS BEEN MADE OF SEISMIC,  
TENSILE, SHEAR, STACKING, OR OTHER FORCES WHICH MAY  
ACT ON THESE COMPONENTS, THEREFORE THE USE OF THESE  
PRODUCTS SHALL BE SPECIFIED BY THE DESIGNER OF THE  
COMPLETE STRUCTURE."

NOTES:

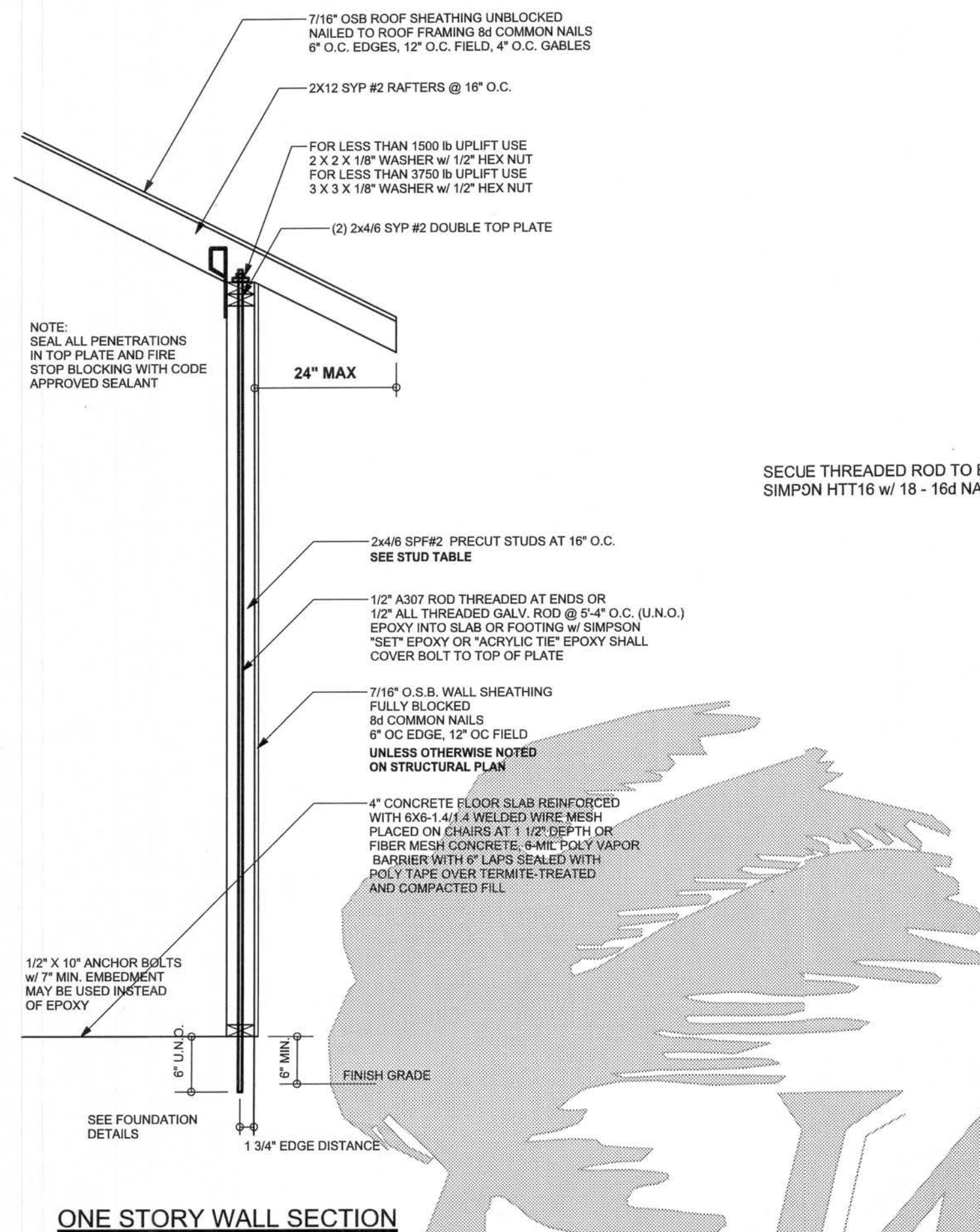
1. DEALER SUES REPRESENTATIVE SHALL CHECK AND CONFIRM ALL LENGTHS, TAILIES, SERIES, AND ALL INFORMATION WITH CONTRACTOR PRIOR TO PLACING ORDER FOR MATERIALS. THIS PLACEMENT PLAN IS PRELIMINARY AND NOT A CONSTRUCTION DOCUMENT.
2. THE LOAD BEARING WALLS, POSTS, AND/OR PIERS ON THIS PLACEMENT PLAN ARE TO BE CONCEALED BY THE INSTALLATION OF YOUR STRUCTURAL WOOD PRODUCTS. PLEASE NOTIFY WOODFORD THENCEMENT DIRECT, IN WRITING OF ANY CHANGES IN BEARING LOCATIONS. THE DESIGN OF BEARING LOCATIONS IS THE SOLE RESPONSIBILITY OF THE DESIGNER OF THE COMPLETE STRUCTURE.
3. ALL METAL CONNECTORS SPECIFIED ARE TO BE SIMPSON STRONG TIE CO., INC. OR EQUAL. ALL HANGERS AND CONNECTORS IN CONTACT WITH WOOD MUST BE PRESERVED TO PREVENT CORROSION. ALL PRESSURE TREATED WOOD AND HANGERS MUST BE RECEIVED BY THE CONTRACTOR, AND THE CONTRACTOR MUST BE RESPONSIBLE FOR INSURE PROPER COATINGS ON HANGERS, CONNECTORS AND ATTACHMENT MATERIALS.
4. ☒ INDICATES UNIFORM OR CONCENTRATED ROPR BRACING LOCATIONS OTHERWISE, ALL ROPR BRACE LOADS SHALL BE TRANSFERRED TO FIRST FLOOR WALLS AND NOT TO FLOOR JOISTS.
5. ☒ JOIST SPACING 24" o.c. SHALL HAVE ADEQUATE SUBFLOOR TO INSURE STIFFNESS BETWEEN JOIST.
6. ☒ LATERAL BRACING TO BE PROVIDED ALONG BOTTOM FLANGES OF ☒ JOIST. AT 8'-0" o.c. IF RIGID SHEATHING IS NOT APPLIED.

### LEGEND

==::==	=WALL ABOVE		=ROOF LOAD
=====	=WALL BELOW	----	=BEAM
	=JOIST MARKER	— —	=I-JOIST
	=BEAM MARKER	—┘—	=HANGFR
			=DETAIL MARK

CUSTOMER: Anderson Truss		
DRAWN BY: IJK	PROJECT: Furguson Res.	
DATE: 7-4-07	REV. 2	DESCRIPTION:
REV. 1	REV. 3	W/F# 7079



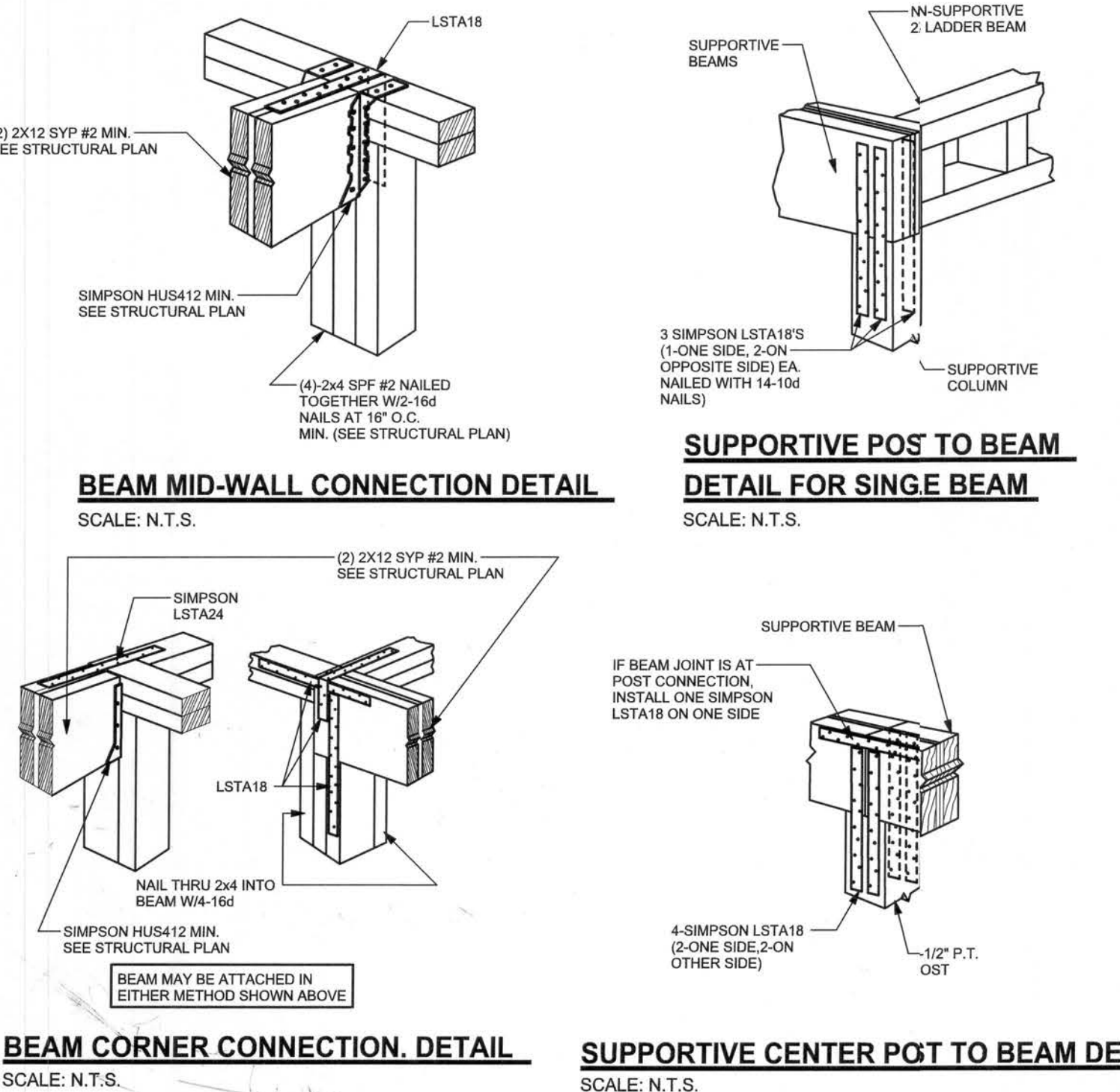


ONE STORY WALL SECTION  
SCALE: 3/4" = 1'-0"

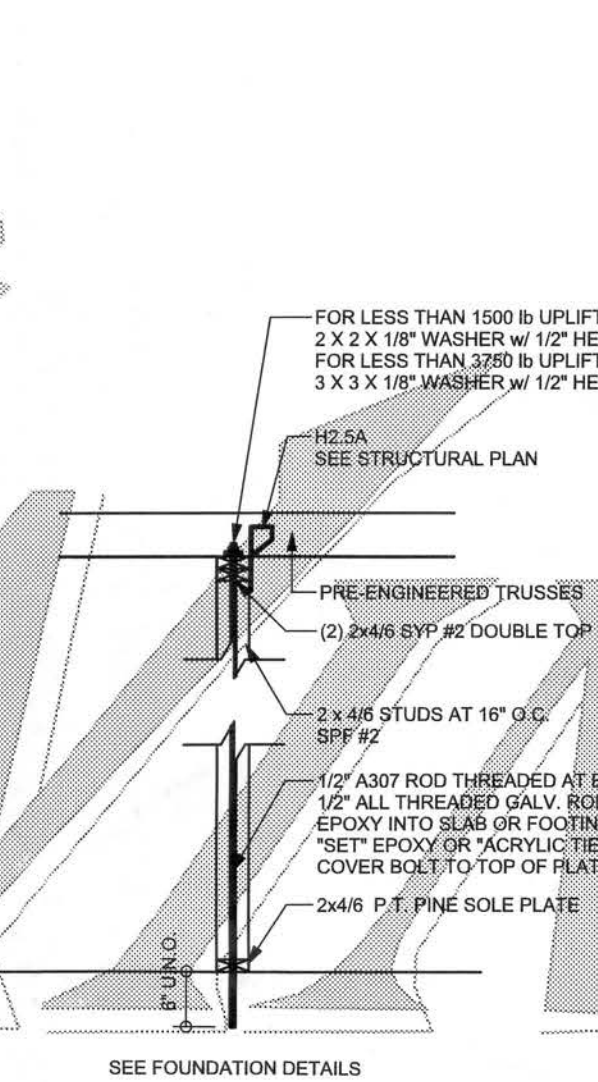
EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

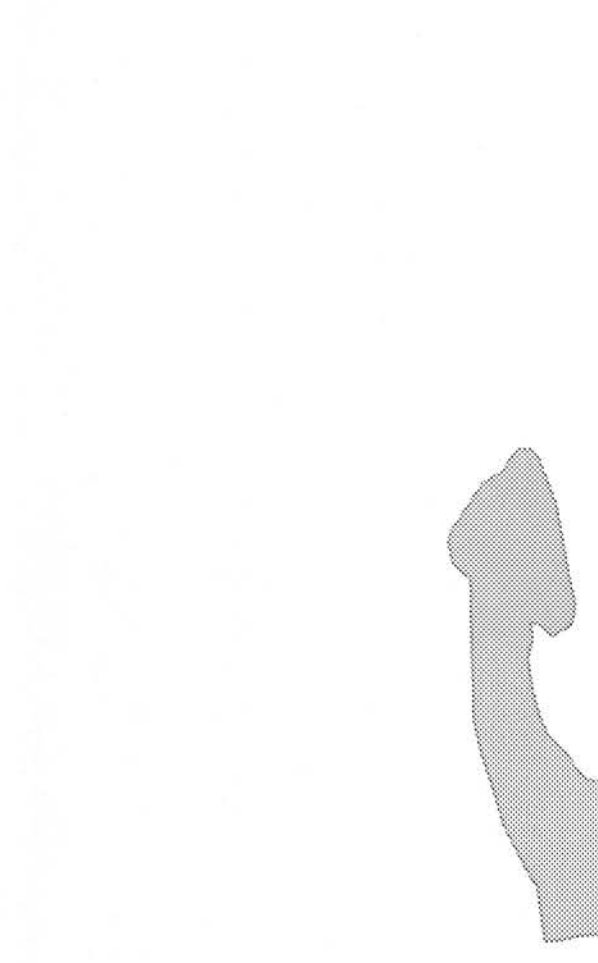
THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.208.  
EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS  
RESISTING INTERIOR ZONE WIND LOADS 110 MPH EXPOSURE B.  
STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING  
LOCATED WITHIN FEET OF CORNERS FOR END ZONE LOADING.  
EXAMPLE: 16" O.C. x 0.85 = 13.6" O.C.



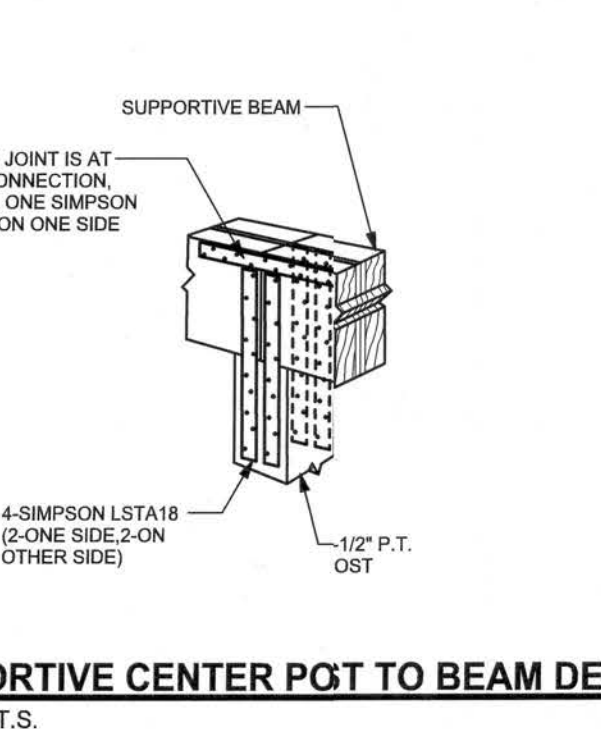
OPTIONAL THREADED ROD TO  
FLOOR BEAM OR FLOOR JOIST  
SCALE: 1/2" = 1'-0"



INTERIOR BEARING WALL  
SCALE: 1/2" = 1'-0"



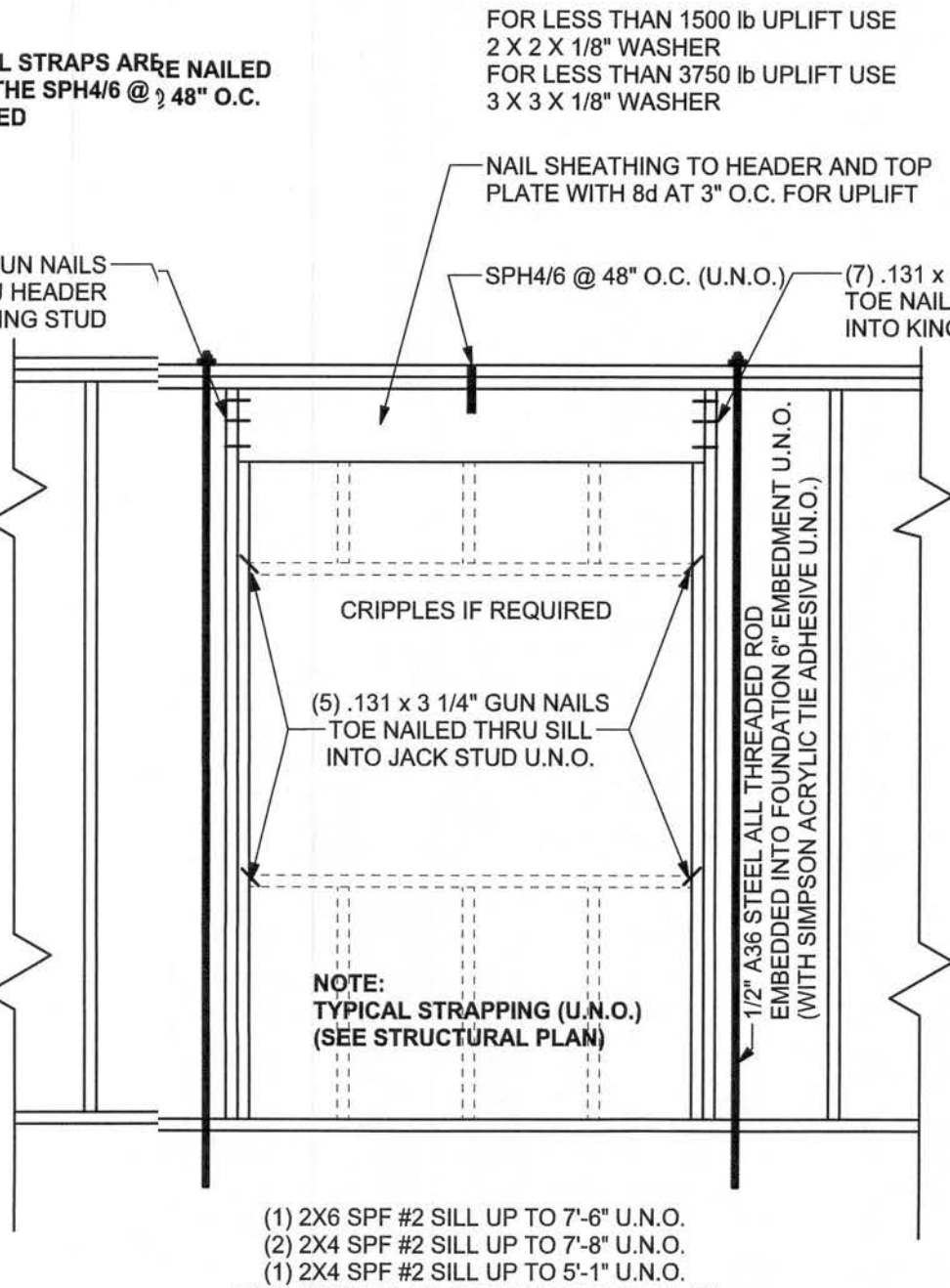
SUPPORTIVE POST TO BEAM  
DETAIL FOR SINGLE BEAM  
SCALE: N.T.S.



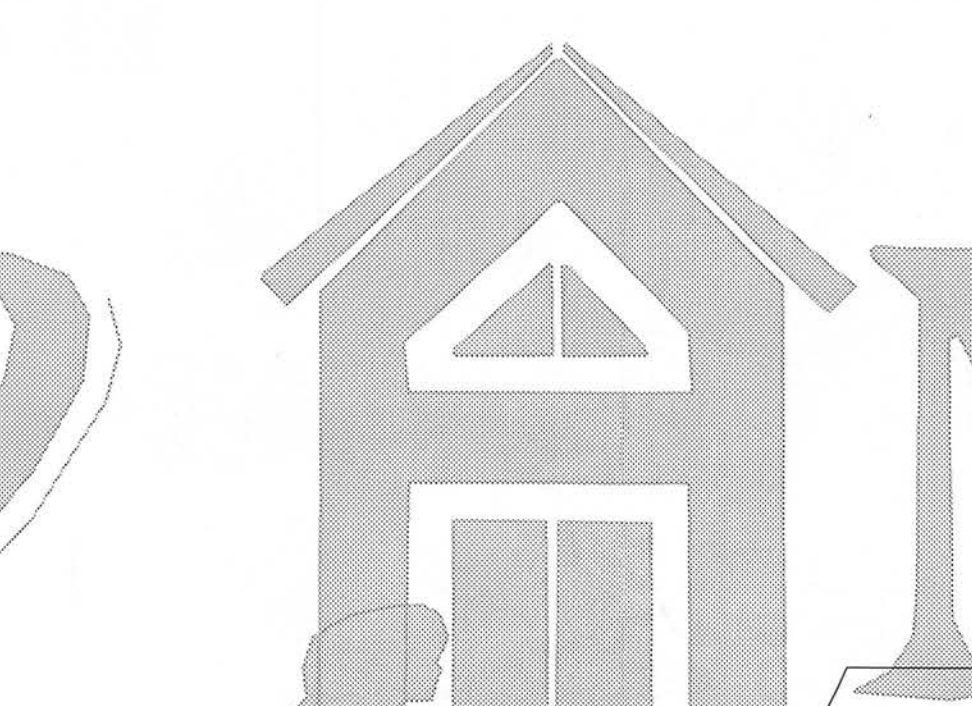
4X4 / 6X6 PORCH POST DETAIL  
SCALE: 1/2" = 1'-0"



OPTIONAL ALUMINUM PORCH POST & HEADER ANCHORS  
SCALE: N.T.S.



TYPICAL 1 STORY HEADER STRAPING DETAIL  
SCALE: 1/2" = 1'-0"



ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS  
MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	HSA	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 380	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5A	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H8	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-2	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-2	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 780	< 655	H10-2	6-10d	6-10d	
< 1470	< 1285	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1285	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2-HTS24			
< 2050	< 1785	LG72	14-16d	14-16d	
<b>HEAVY GIRDER TIEDOWNS*</b>					
< 3965	< 3330	MG7		22-10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10930	< 9035	HGT-3		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 8250	HGT-4		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
<b>STUD STRAP CONNECTOR*</b>					
< 435	< 435	SSP DOUBLE TOP PLATE	3-10d		
< 455	< 420	SSP SINGLE SILL PLATE	1-10d		
< 825	< 825	DSP DOUBLE TOP PLATE	6-10d		
< 825	< 600	DSP SINGLE SILL PLATE	2-10d		
< 885	< 760	SP4		6-10d, 1 1/2"	
< 1240	< 1065	SPH4		10-10d, 1 1/2"	
< 885	< 760	SP6		6-10d, 1 1/2"	
< 1240	< 1065	SPH6		10-10d, 1 1/2"	
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
<b>STUD ANCHORS*</b>					
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTT31	16-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18-15d		5/8" AB
< 1400	< 1400	PAH42	16-16d		
< 3335	< 3335	HPAH22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18-16d		2-5/8" AB

GRADE & SPECIES TABLE

		Fb (psi)	E (10 <sup>6</sup> psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

GENERAL NOTES:

**TRUSSES:** TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X8 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

**SITE PREPARATION:** SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

**FOUNDATION:** CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE.

**CONCRETE:** MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS,  $F_c = 3000$  PSI.

**WELDED WIRE REINFORCED SLAB:** 8" x 8" W1 x W1.4, FB = 80KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A188, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

**FIBER CONCRETE SLAB:** CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 12 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

**CONTROL JOINTS:** WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIO OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. ONCE THE FINISH FLOORING IS APPLIED, RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.

**REBAR:** ASTM A 615, GRADE 60, DEFORMED BARS,  $F_y = 60$  KSI. ALL LAP SPLICES @ 48" (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, UNO.

**GLULAM BEAMS:** GLULAM BEAM, GLB, 24F-V3SP,  $F_b = 2400$  PSI,  $E = 1800$  KSI. UNO. SUPPLIER MAY SUPPLY IN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SCHED. DIMS.

**ROOF SHEATHING:** ALL ROOFS ARE HORIZONTAL DIAPHRAGMS. 7/16" OSB SHEATHING, UNLOCKED, STAGGERED, FASTENED WITH 8d COMMON NAILS (13d), 6" O.C. PANEL EDGES, 12" O.C. INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4" O.C. UNO.

**STRUCTURAL CONNECTIONS:** MANUFACTURER'S AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND TRUSSES ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

**ANCHOR BOLTS:** A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

**WASHERS:** WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64", WITH 5/8" BOLTS TO BE 3" x 3" x 9/64", WITH 3/4" BOLTS TO BE 3" x 3" x 9/64". UNO.

**NAILS:** ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

**BUILDER'S RESPONSIBILITY**

**THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK:**

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED AND SEALED BY A DESIGN PROFESSIONAL. FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS, THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

**WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1**

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60 FT IN EXP. B, 30 FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY OCCURRY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

1. BASIC WIND SPEED = 110 MPH

2. WIND EXPOSURE = B

3. WIND IMPORTANCE FACTOR = 1.0

4. BUILDING CATEGORY = II

5. ROOF ANGLE = 10-45 DEGREES

6. MEAN ROOF HEIGHT = <30 FT

7. INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

8. COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone	Effective Wind Area (ft <sup>2</sup> )	10	100
1	19.9 - 21.8	18.1	-18.1
2	19.9 - 25.5	18.1	-21.8
2 Oth		-40.6	-40.6
3	19.9 - 25.5	18.1	-21.8
3 Oth		-68.3	-42.4
4	21.8 - 23.6	18.5	-20.4
5	21.8 - 29.1	18.5	-22.6
Doors & Windows Worst Case (Zone 5, 10 ft <sup>2</sup> )		21.8	-29.1
8x7 Garage Door		19.5	-22.9
16x7 Garage Door		18.5	-21.0

**DESIGN LOADS**

FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)

30 PSF (SLEEPING ROOMS)

30 PSF (ATTICS WITH STORAGE)

10 PSF (ATTICS WITHOUT STORAGE, <3-12)

ROOF 20 PSF (FLAT OR <4-12)

16 PSF (4-12 TO <12-12)

12 PSF (12-12 AND GREATER)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

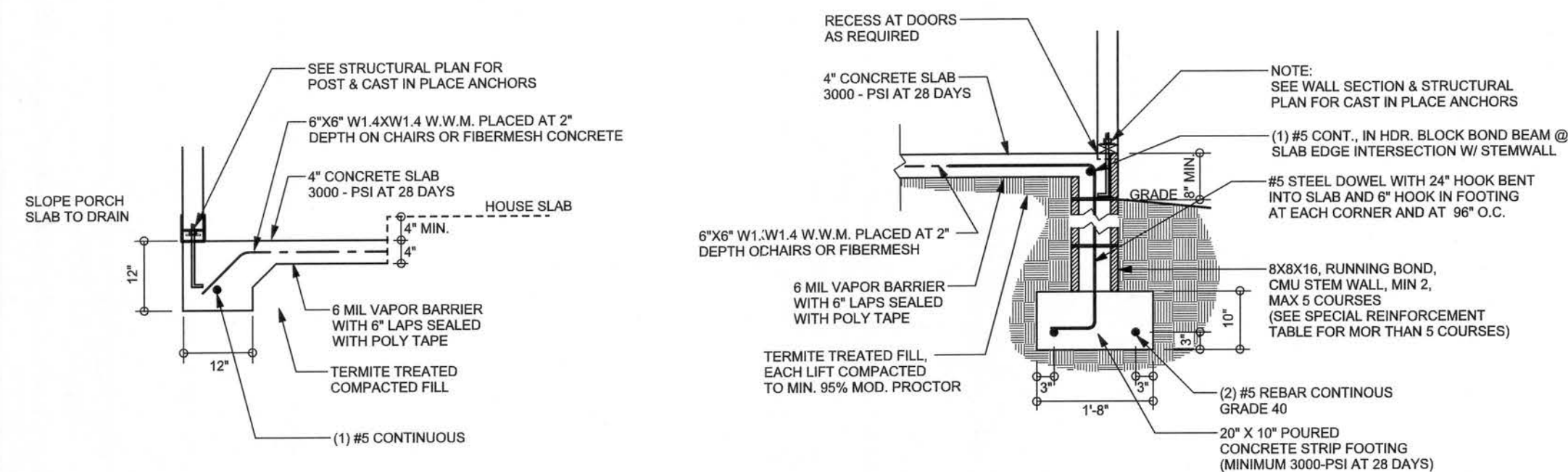
SOIL BEARING CAPACITY 1000PSF

NOT IN FLOOD ZONE



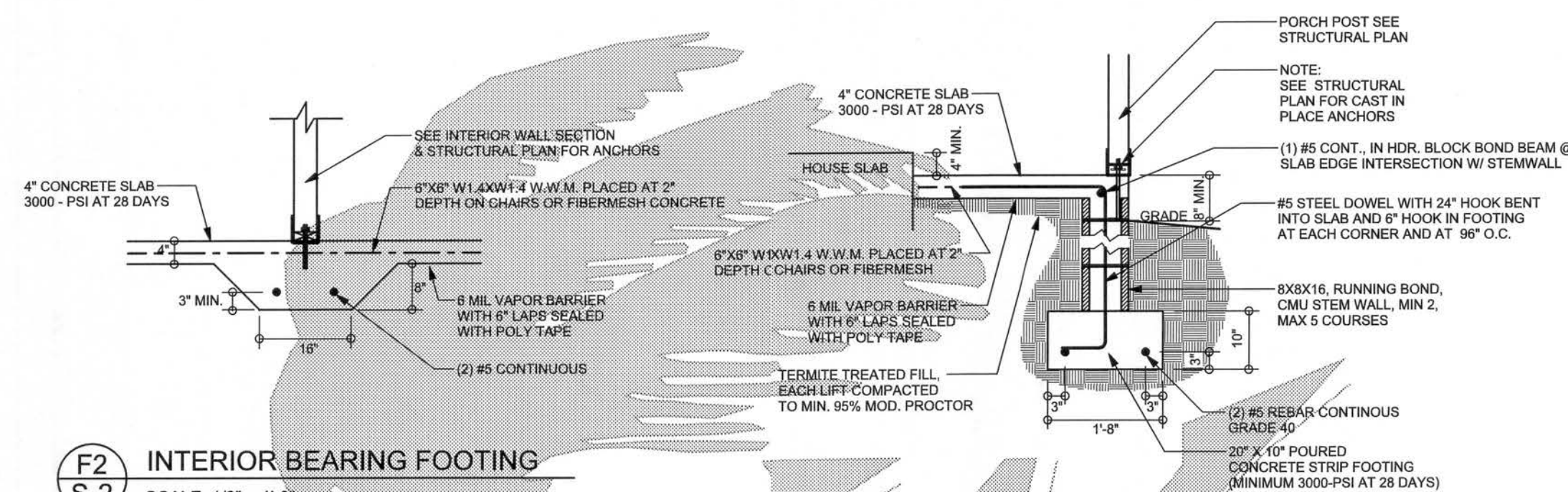
REVISIONS

SOFTPLAN  
ARCHITECTURAL DESIGN SOFTWARE



**F5**  
**S-2** PORCH FOOTING  
SCALE: 1/2" = 1'-0"

**F9**  
**S-2** STEM WALL FOOTING  
SCALE: 1/2" = 1'-0"



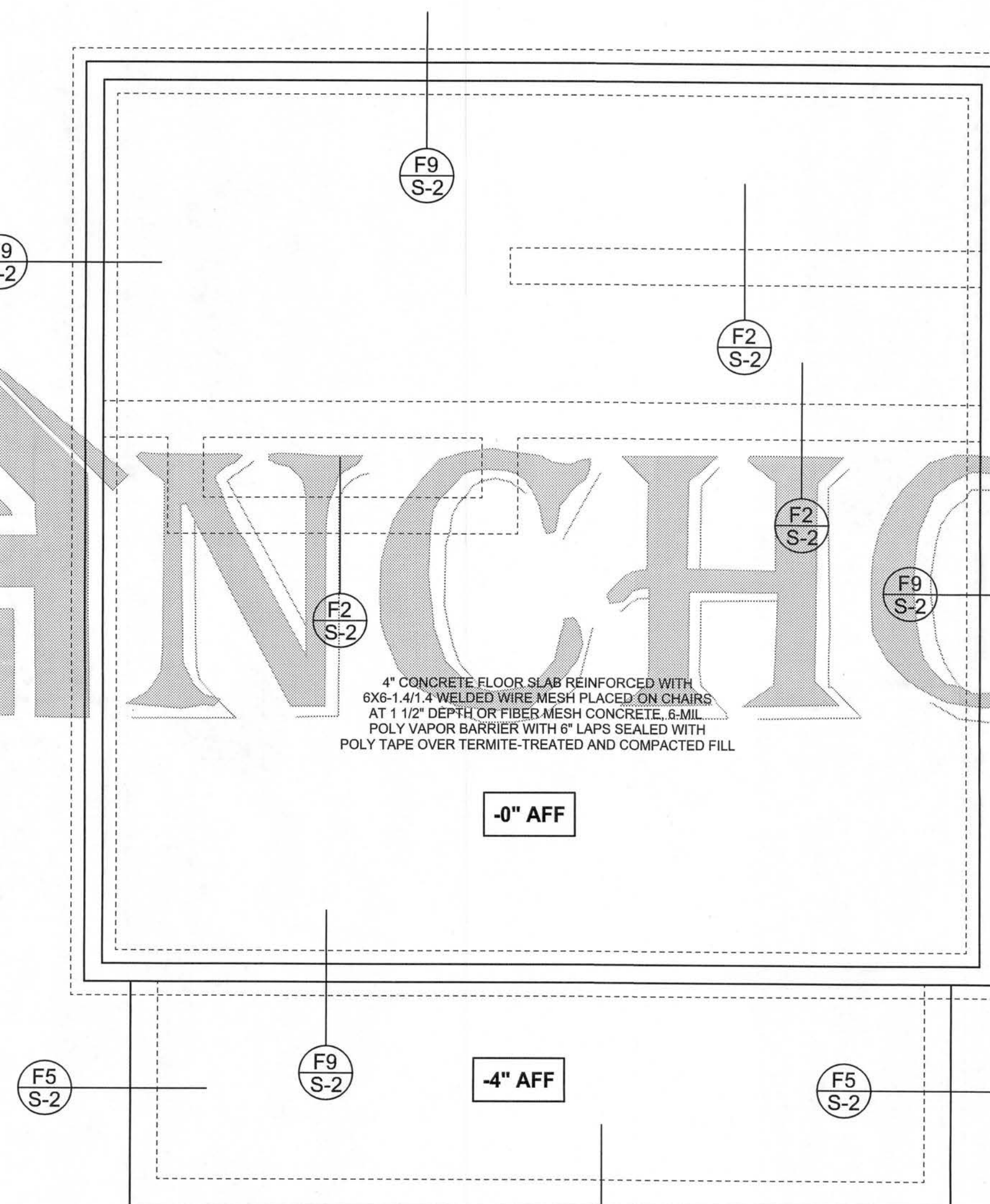
**F2**  
**S-2** INTERIOR BEARING FOOTING  
SCALE: 1/2" = 1'-0"

**F12**  
**S-2** ALT. STEM WALL PORCH FOOTING  
SCALE: 1/2" = 1'-0"

**TALL STEM WALL TABLE**

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced side of the wall. The vertical table is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Duowall ladder reinforcement at 16" O.C. vertically or a horizontal bond beam with 165 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL UNBALANCED HEIGHT (FEET)	BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



**FOUNDATION PLAN**

SCALE: 1/4" = 1'-0"

DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

WINDLOAD ENGINEER: Mark Disosway,  
P.E. No. 53915, P.O. Box 868, Lake City, FL  
32056, 386-754-5419

**DIMENSIONS:**  
Shaded dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section F301.2.1, Florida building code residential 2004, to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.

MARK DISOSWAY  
P.E. 53915  
15 Jul 07  
SEAL

**Isaac Construction**

Nancy Ferguson  
Residence

ADDRESS:  
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Mark Disosway P.E.  
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Lake City, Florida 32056  
Phone: (386) 754 - 5419  
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PRINTED DATE:  
July 18, 2007

DRAWN BY: David Disosway  
CHECKED BY:

FINALS DATE:  
18 / Jul / 07

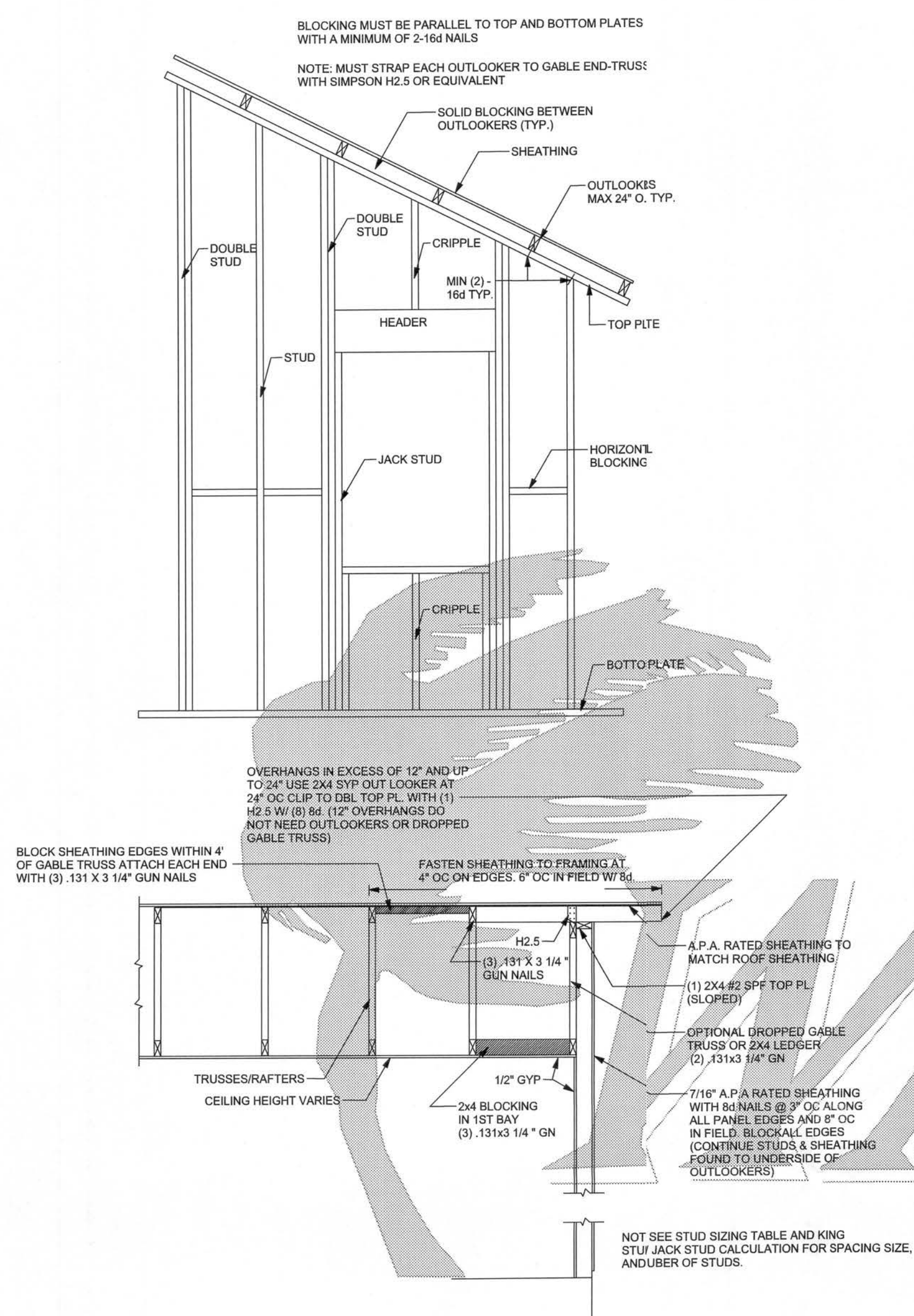
JOB NUMBER:  
707172

DRAWING NUMBER  
**S-2**

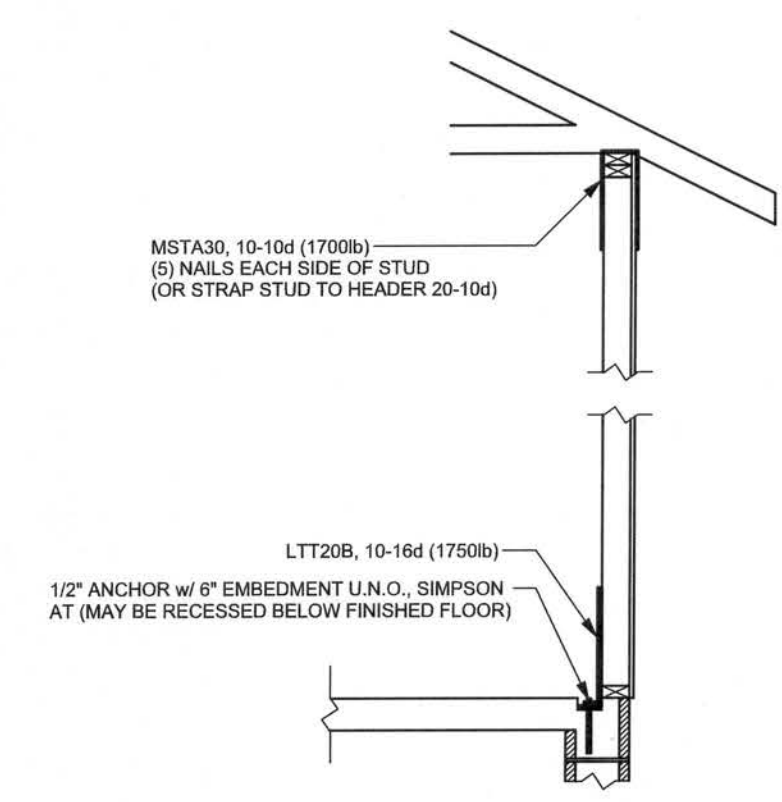
OF 3 SHEETS



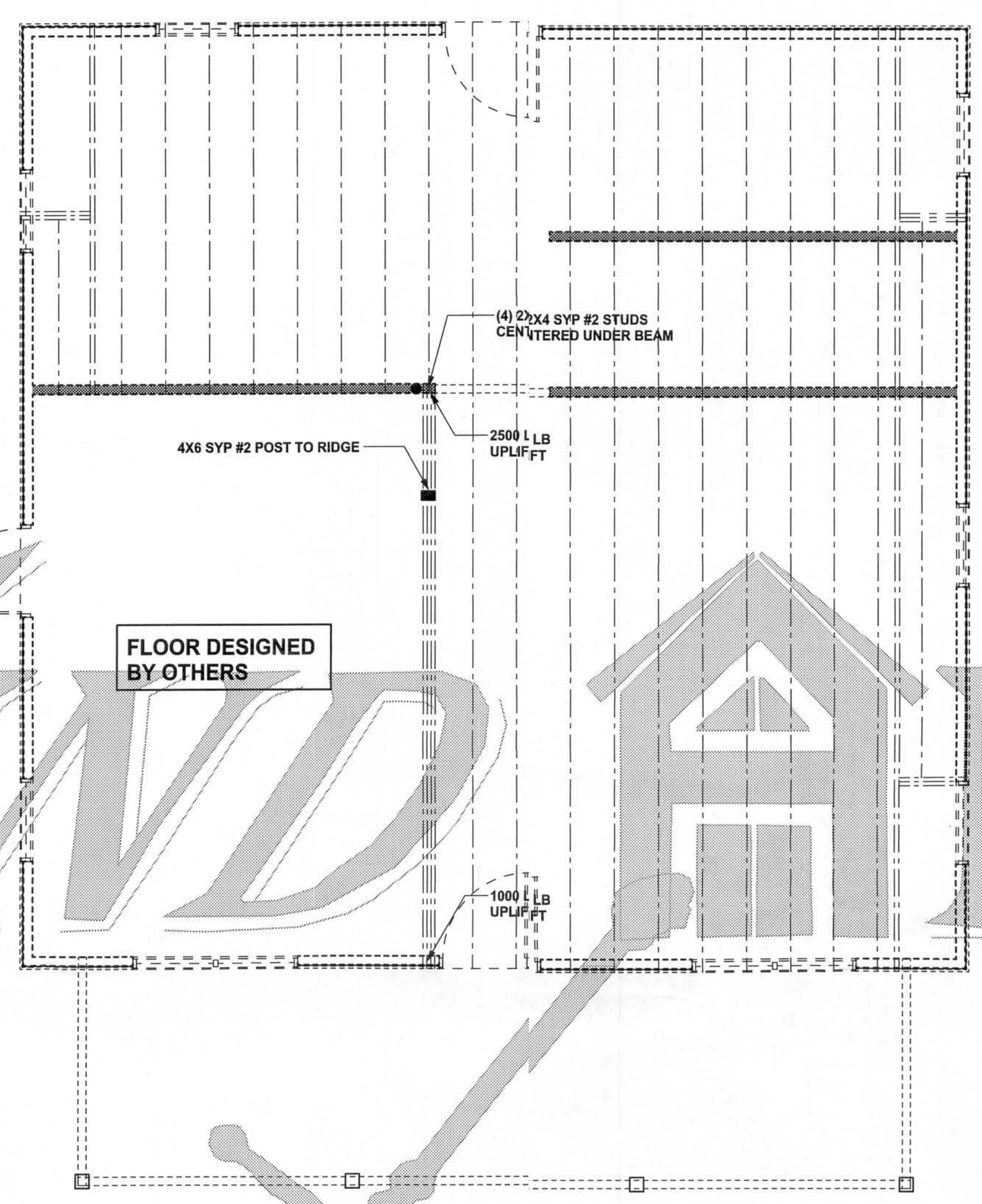
REVISIONS	



**GABLE END WALL BALLOON FRAMING DETAIL**  
SCALE: 1/2" = 1'-0"



**ALTERNATE WALL TIE CONNECTION WHEN THREADED ROD CANNOT BE PLACED IN W/L.**  
SCALE: 1/2" = 1'-0"



**STRUCTURAL FLOOR PLAN**  
SCALE: 1/4" = 1'-0"

**STRUCTURAL PLAN NOTES**

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

**THREADED ROD LEGEND**

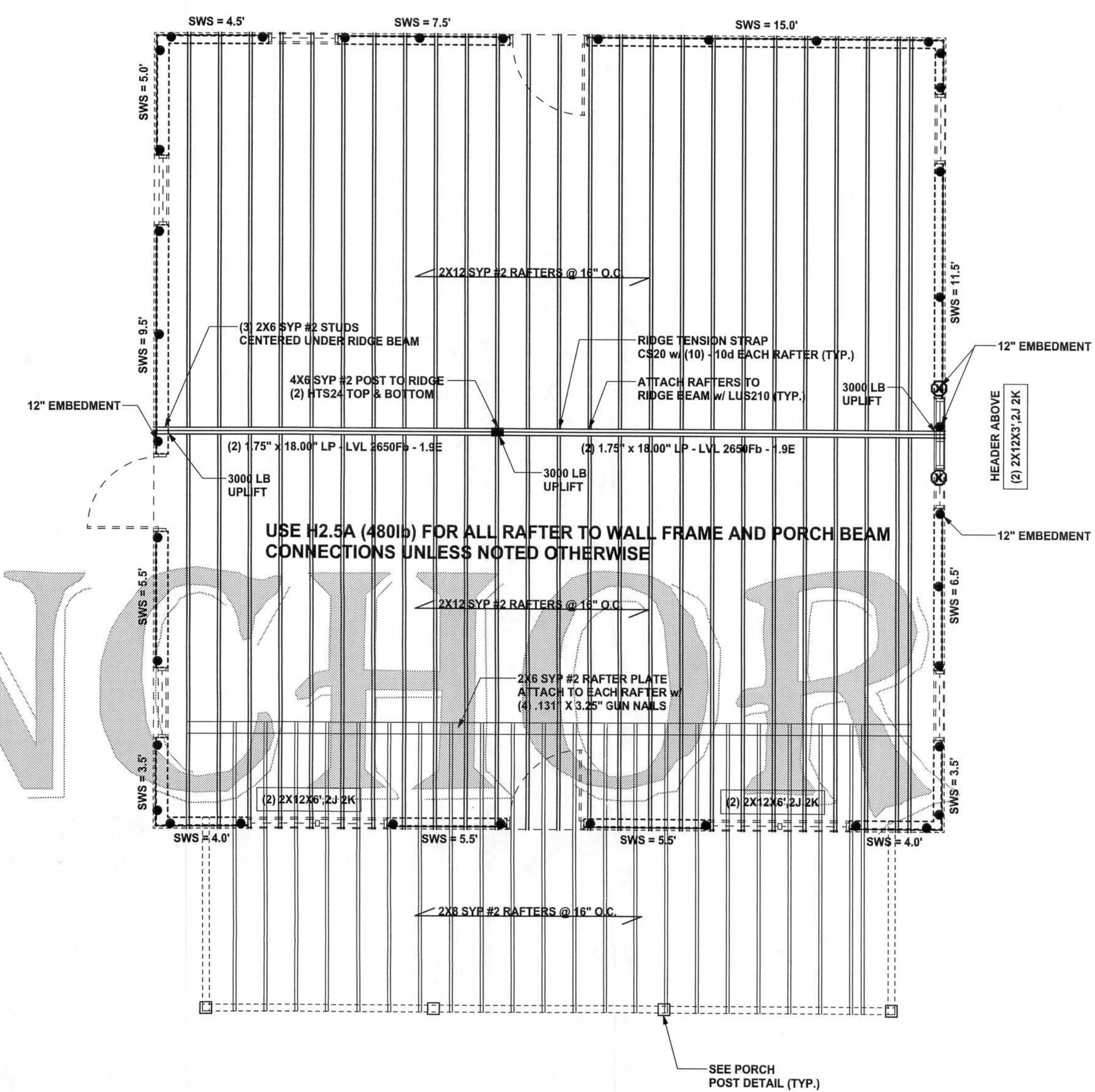
- INDICATES LOCATION OF: 1ST FLOOR 1/2" A307 ALL THREADED ROD
- ⊙ INDICATES LOCATION OF: 2ND FLOOR 1/2" A307 ALL THREADED ROD

**HEADER LEGEND**

- (2) 2X12X0', 1J 1K — HEADER/BEAM CALL-OUT (U.N.O.)
- NUMBER OF KING STUDS (FULL LENGTH)
- NUMBER OF JACK STUDS (UNDER HEADER)
- SPAN OF HEADER
- SIZE OF HEADER MATERIAL
- NUMBER OF PLIES IN HEADER

**TOTAL SHEAR WALL SEGMENTS**

	SWS = 0.0' INDICATES SHEAR WALL SEGMENTS
	REQUIRED ACTUAL
TRANSVERSE	38.0' 45.0'
LONGITUDINAL	28.6' 46.0'



**STRUCTURAL ROOF PLAN**  
SCALE: 1/4" = 1'-0"

**WALL LEGEND**

SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER, ANDERSON TRUSS JOB #7079

WINDLOAD ENGINEER: Mark Disoway, P.E. No. 53915, P.O. Box 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY  
P.E. 53915  
12/1/07  
SEAL

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**S-3**  
OF 3 SHEETS