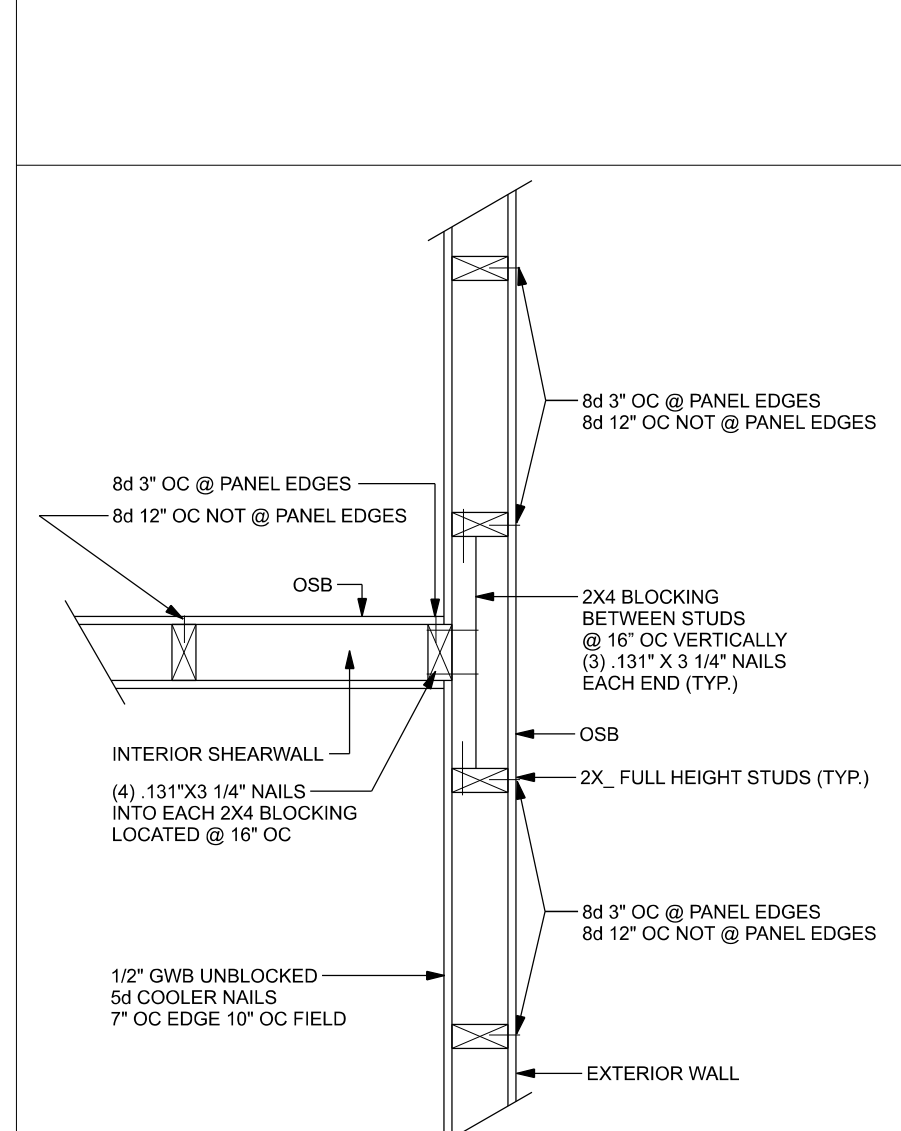
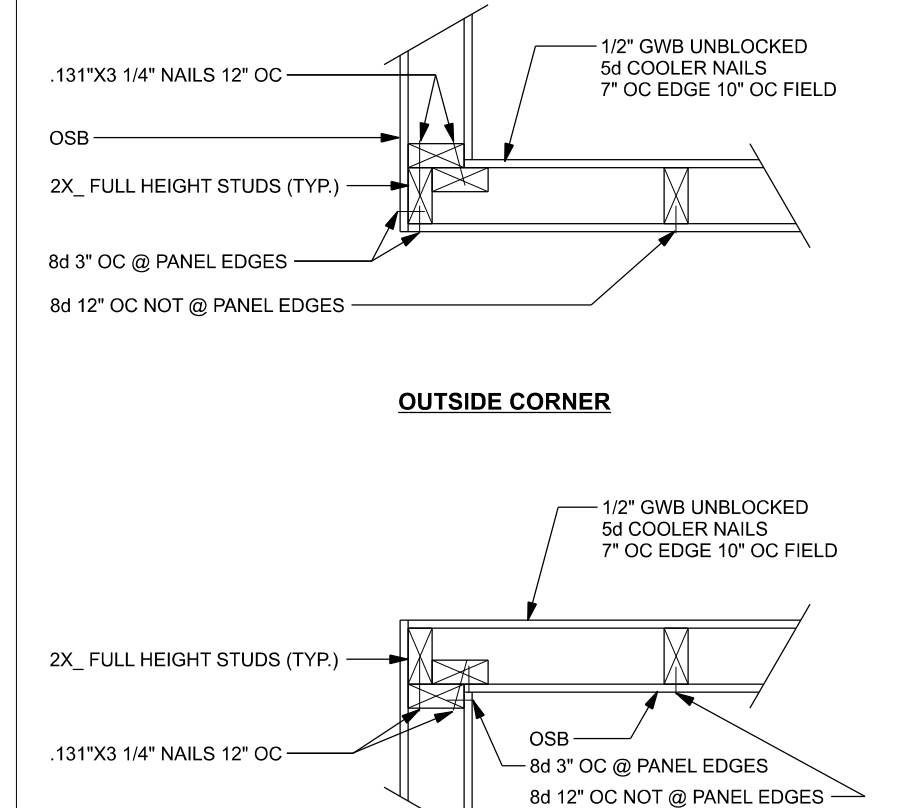


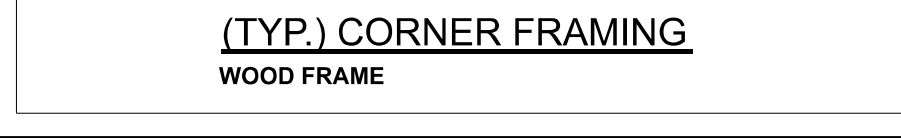
ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"



(TYP.) GABLE BRACING DETAIL
WOOD FRAME



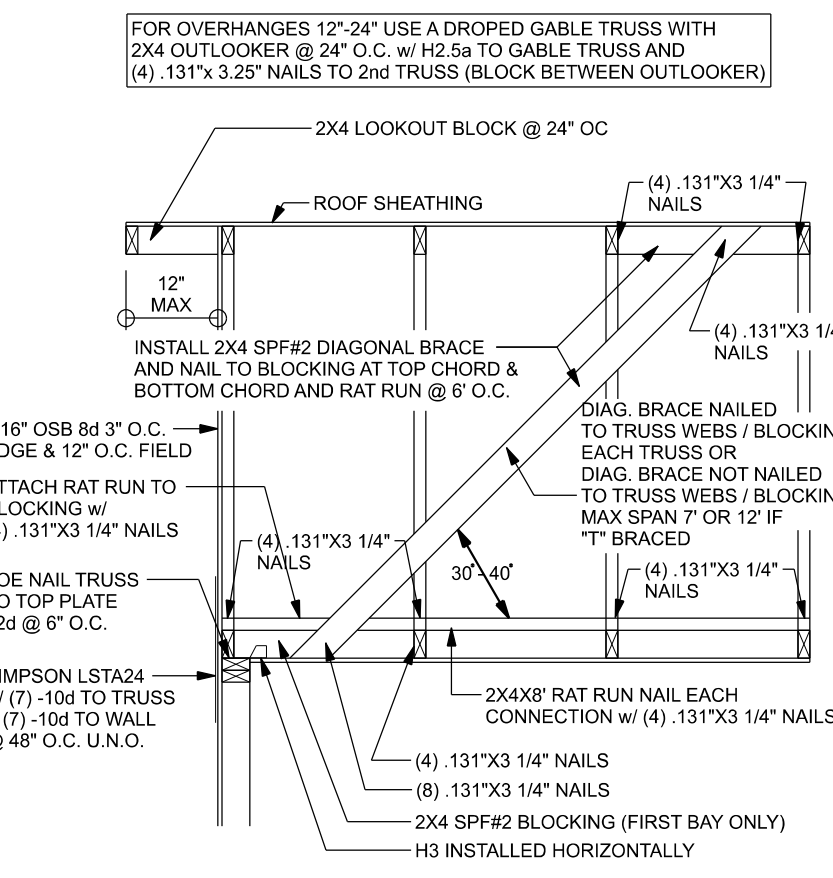
(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME



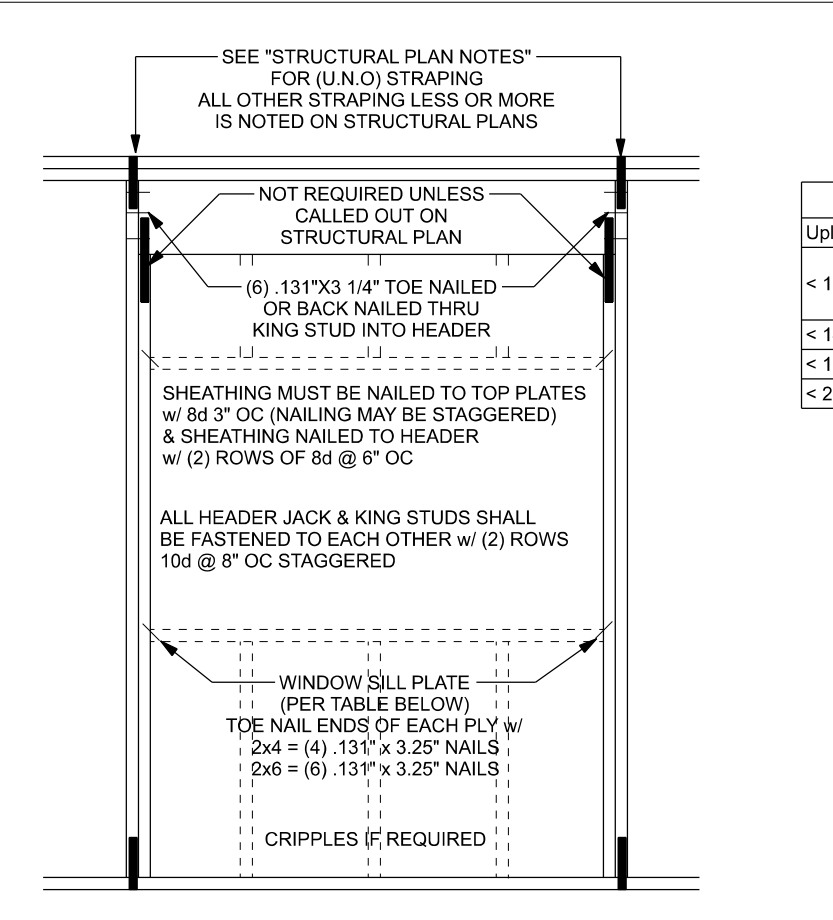
(TYP.) CORNER FRAMING
WOOD FRAME

Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field
120 mph Exp. B	7/16"	ASTM F1667 RSRs-01 (2 3/8" x 0.131")	6" oc	12" oc
120 mph Exp. C	13/32"	ASTM F1667 RSRs-03 (2 1/2" x 0.131") or ASTM F1667 RSRs-04 (3" x 0.120")	6" oc	6" oc
130 mph Exp. B	7/16"	ASTM F1667 RSRs-01 (2 3/8" x 0.131")	6" oc	6" oc
130 mph Exp. C	13/32"	ASTM F1667 RSRs-03 (2 1/2" x 0.131") or ASTM F1667 RSRs-04 (3" x 0.120")	6" oc	6" oc
130 mph Exp. D	13/32"	ASTM F1667 RSRs-03 (2 1/2" x 0.131") or ASTM F1667 RSRs-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. B	7/16"	ASTM F1667 RSRs-01 (2 3/8" x 0.131")	6" oc	6" oc
140 mph Exp. C	13/32"	ASTM F1667 RSRs-03 (2 1/2" x 0.131") or ASTM F1667 RSRs-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. D	13/32"	ASTM F1667 RSRs-03 (2 1/2" x 0.131") or ASTM F1667 RSRs-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. C	13/32"	ASTM F1667 RSRs-03 (2 1/2" x 0.131") or ASTM F1667 RSRs-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. D	13/32"	ASTM F1667 RSRs-03 (2 1/2" x 0.131") or ASTM F1667 RSRs-04 (3" x 0.120")	4" oc	4" oc

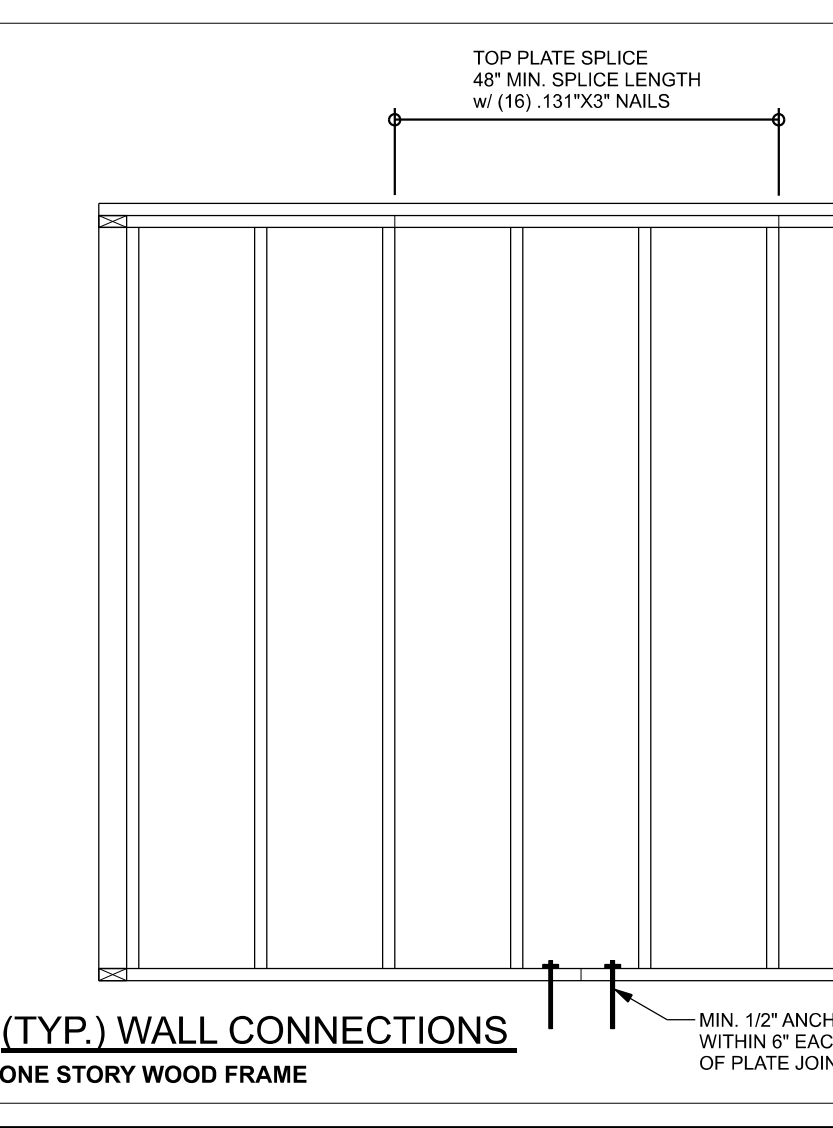
Note: For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 8 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field. Note: This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.



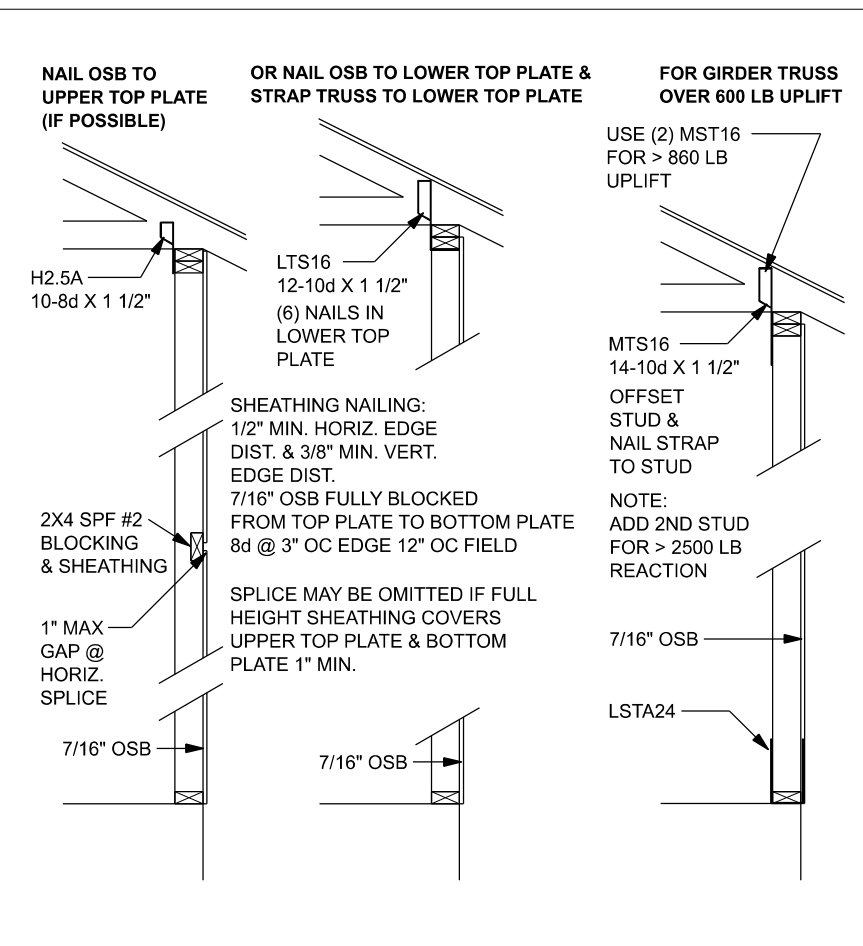
(TYP.) GABLE WALL w/ VAULTED CEILING
WOOD FRAME



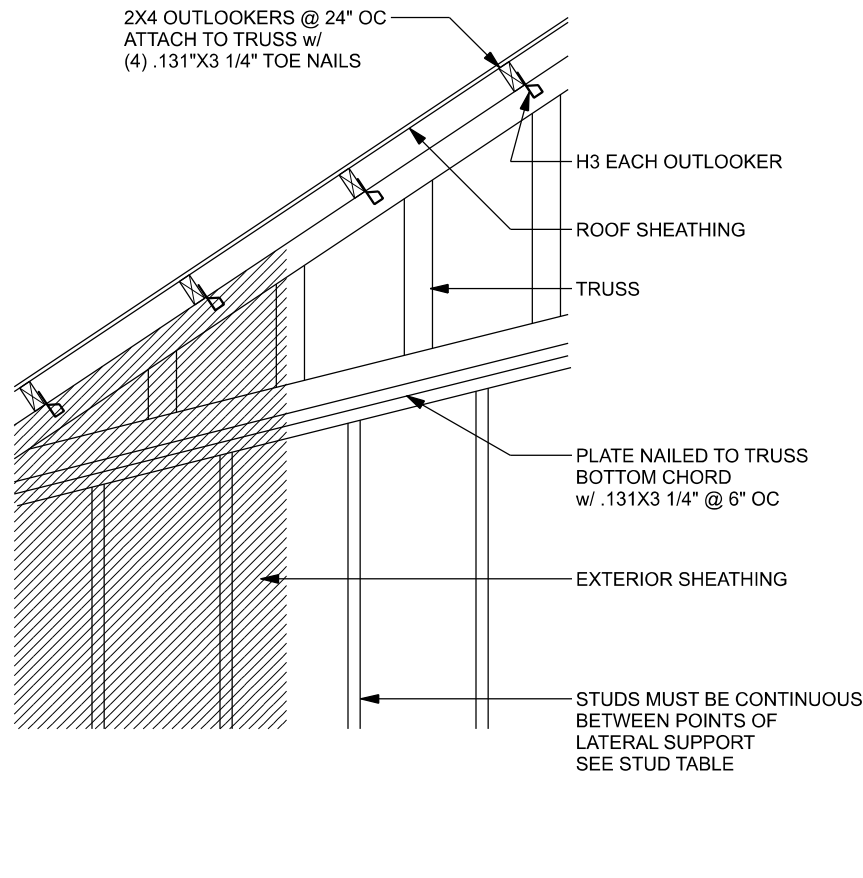
TYPICAL HEADER STRAPPING DETAIL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



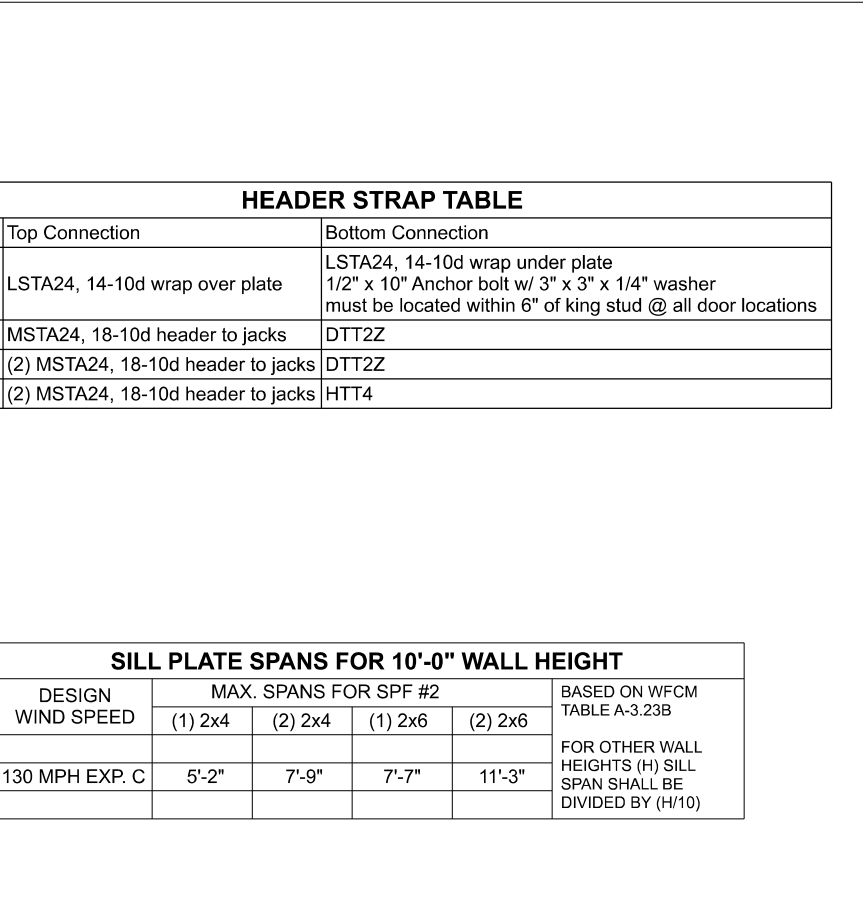
(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME



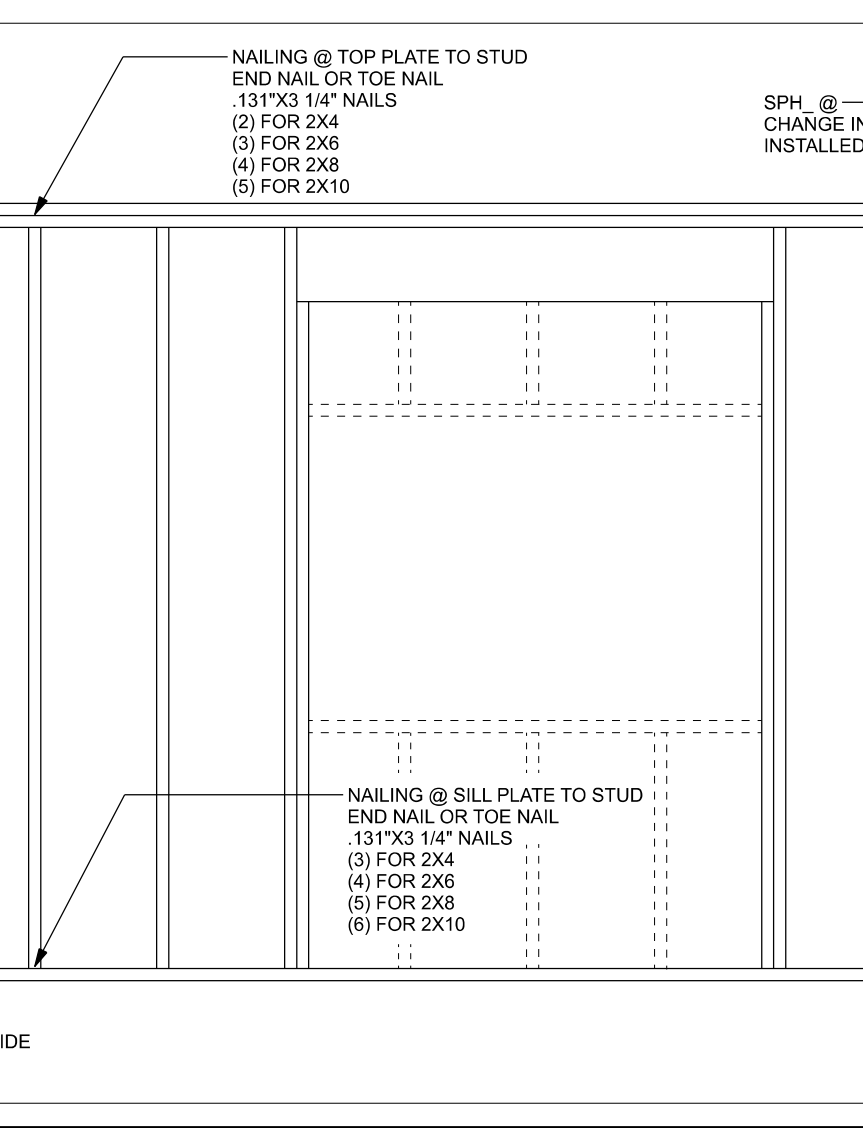
SHEATHING FOR UPLIFT ATTACHMENT DETAILS
ONE STORY WOOD FRAME



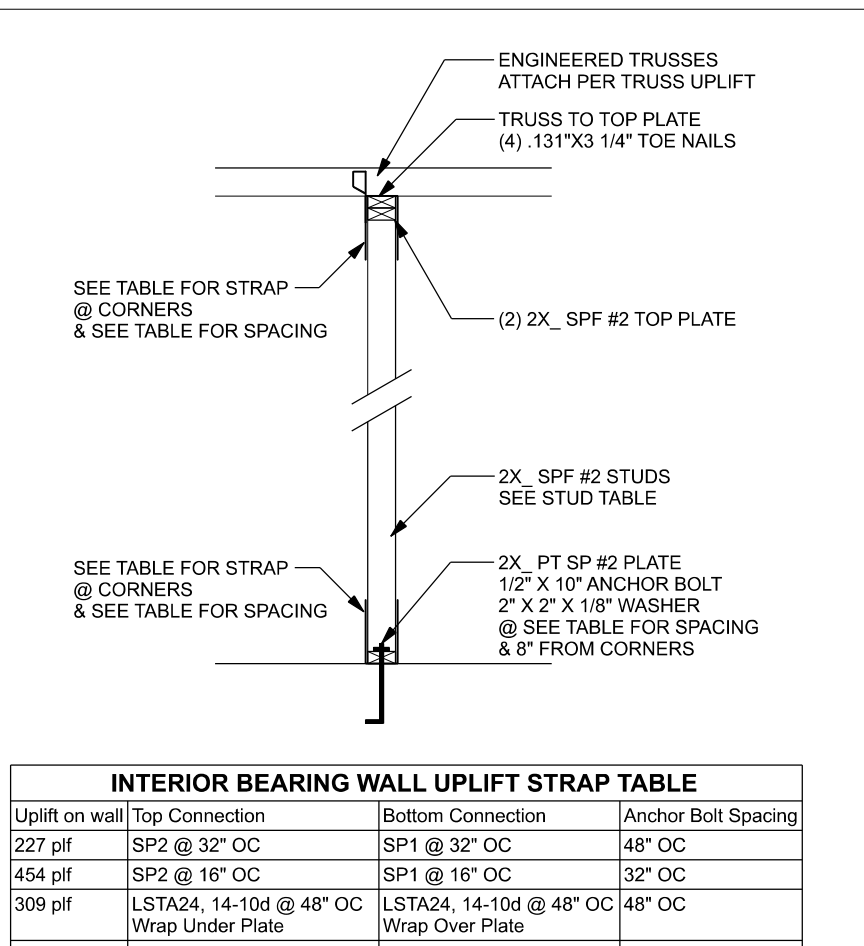
(TYP.) PORCH POST
ONE STORY WOOD



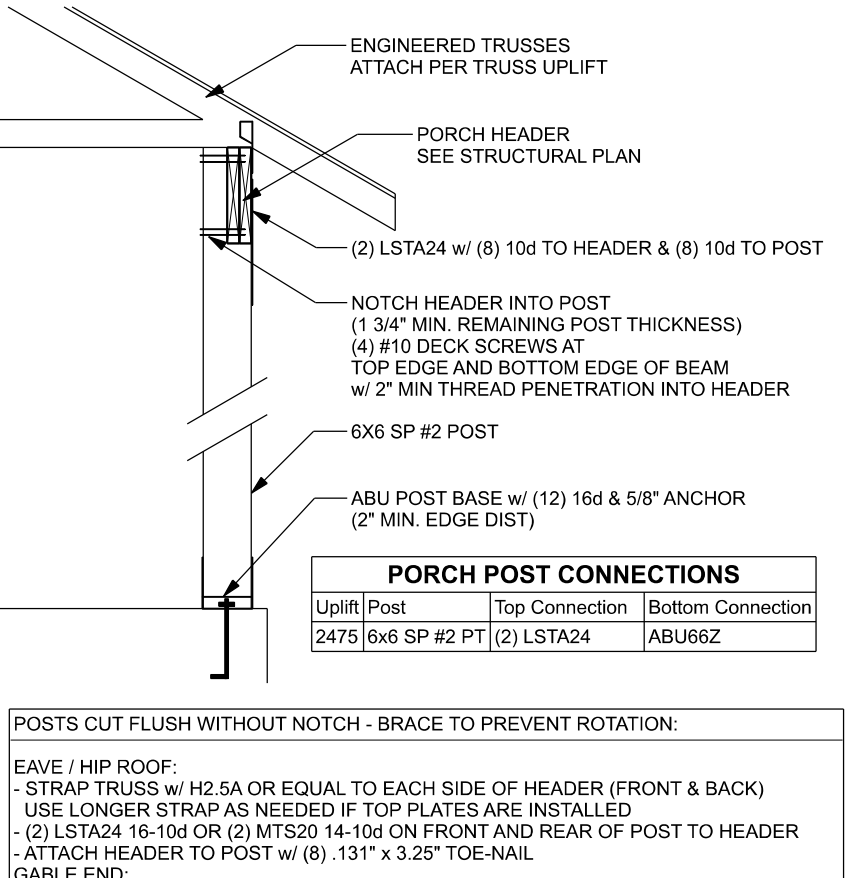
(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS



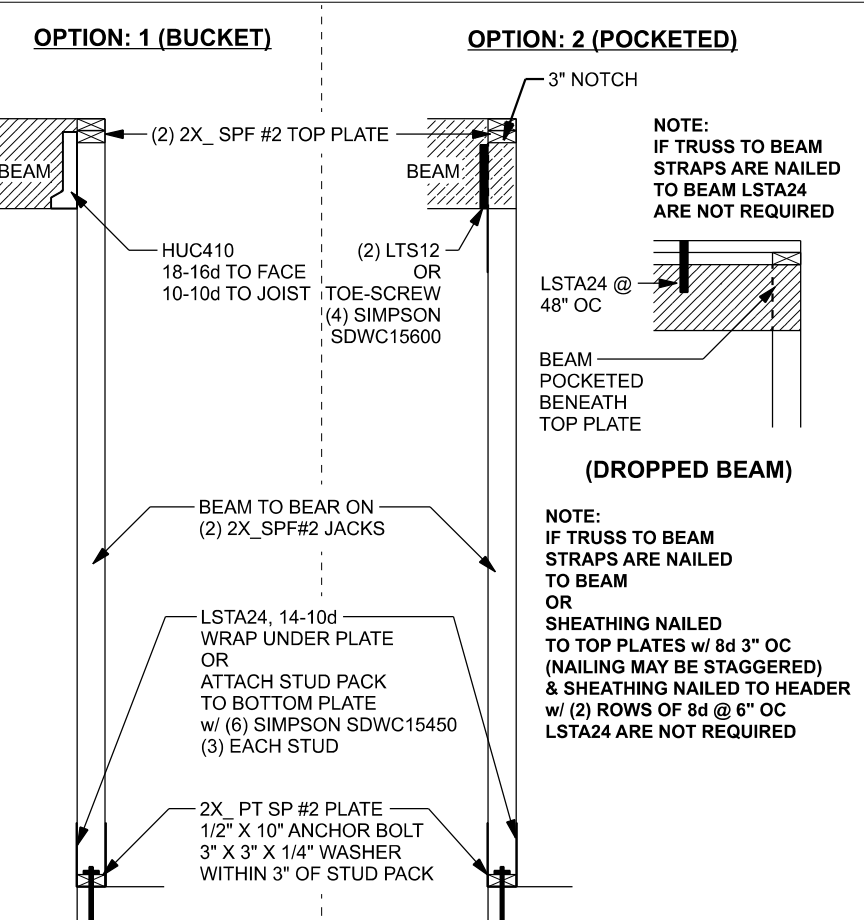
(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME



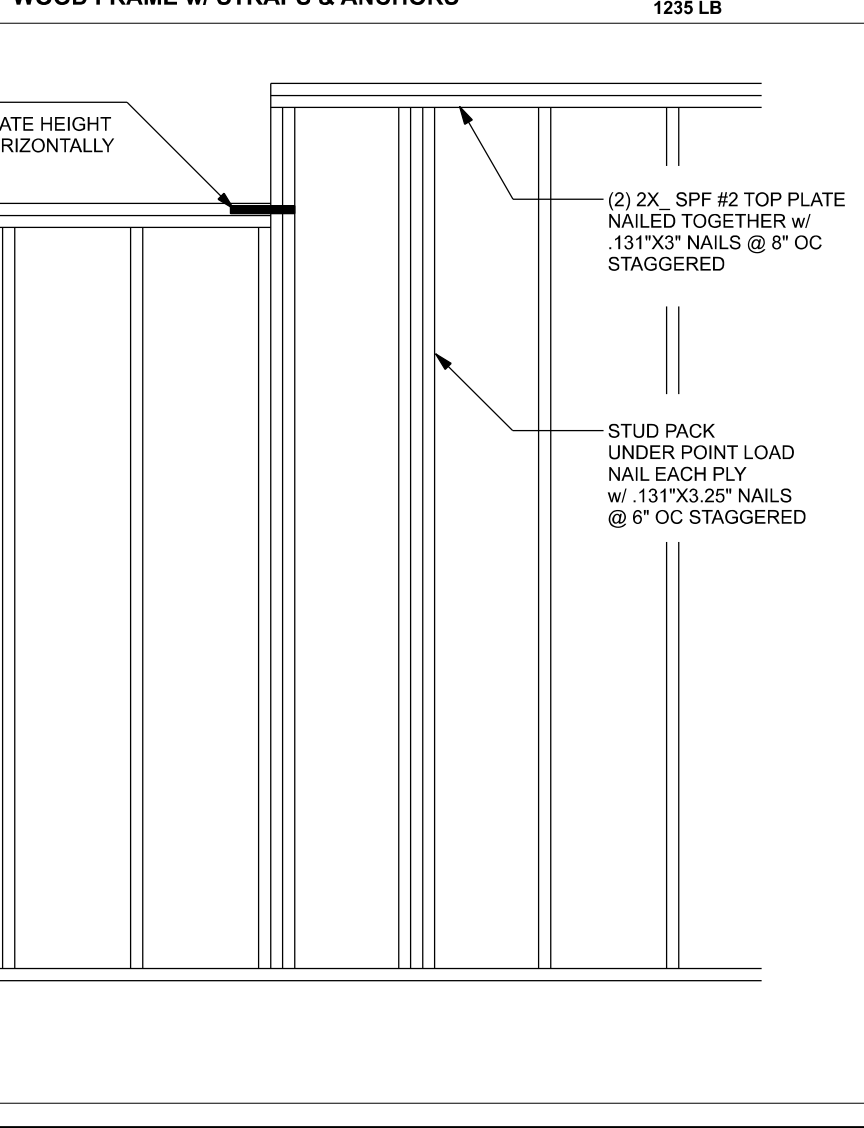
(TYP.) GIRDER TRUSS HOLD DOWN DETAIL
WOOD FRAME w/ STRAPS & ANCHORS



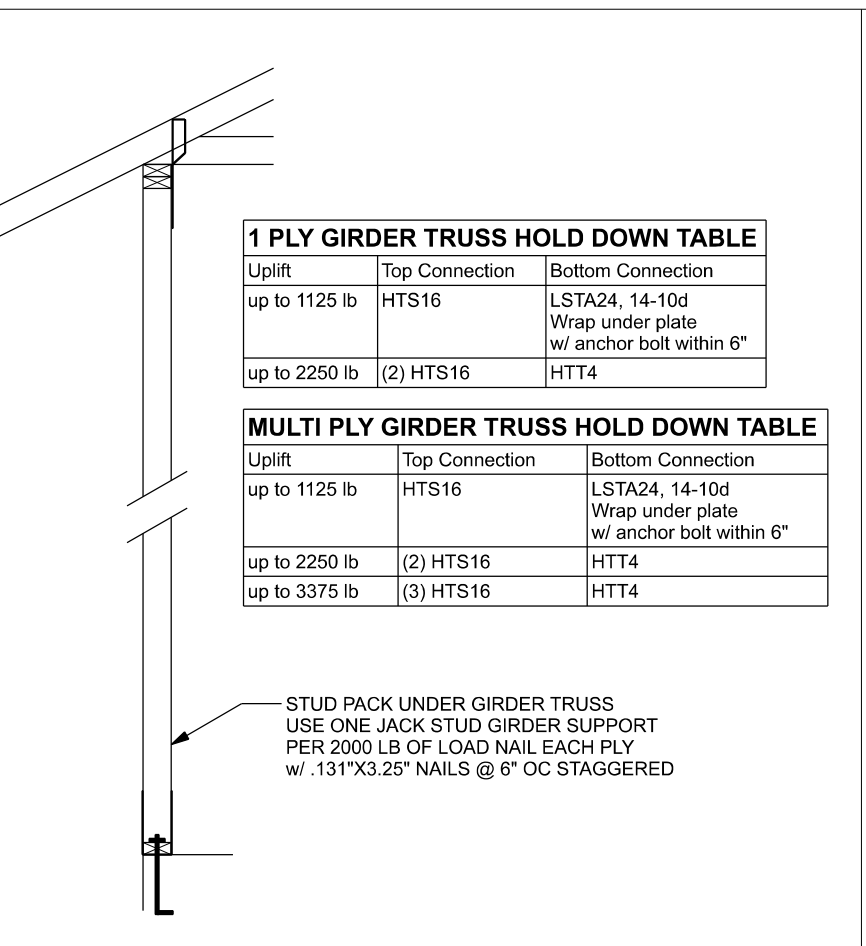
(TYP.) GARAGE DOOR BUCK ATTACHMENT
WOOD FRAME



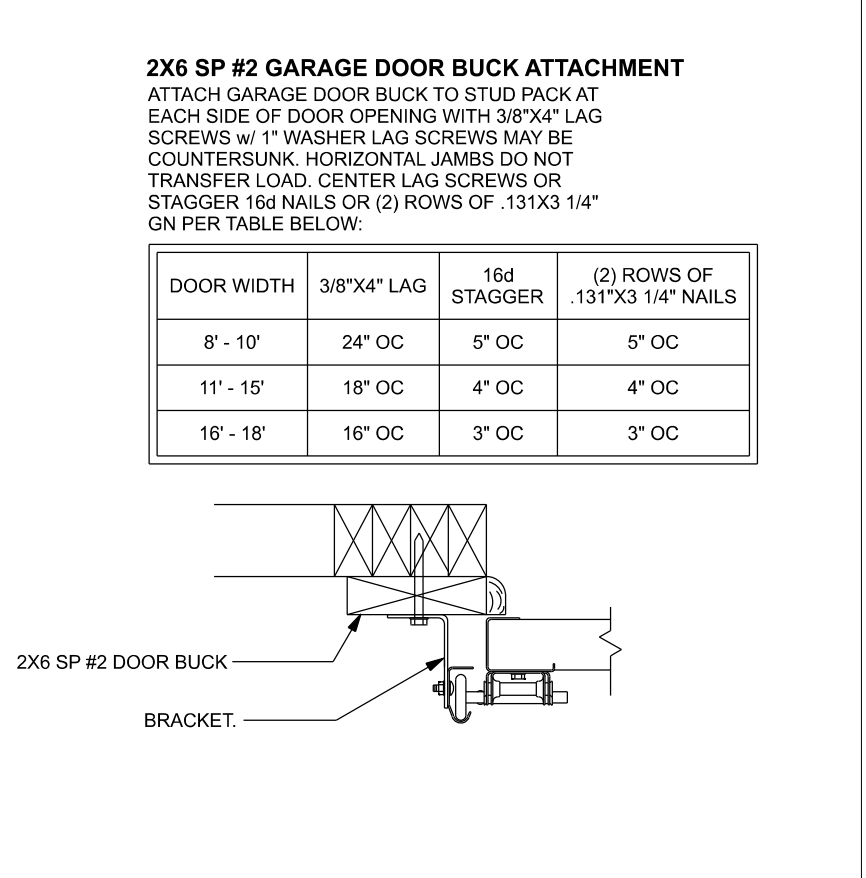
TYPICAL HEADER STRAPPING DETAIL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



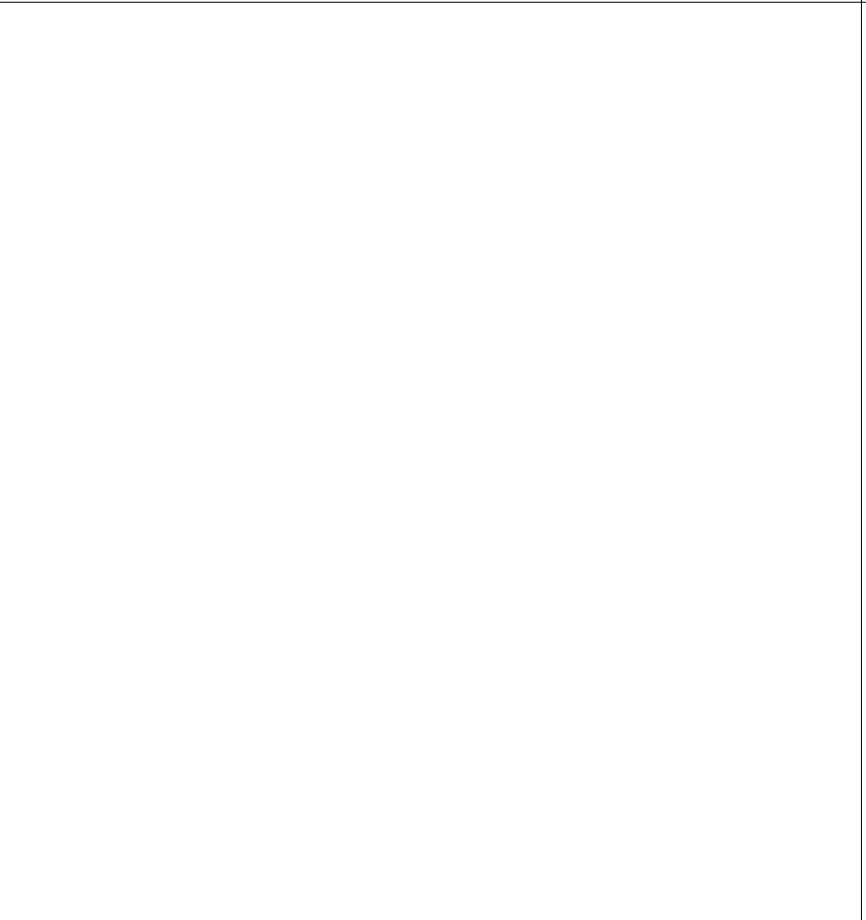
(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME



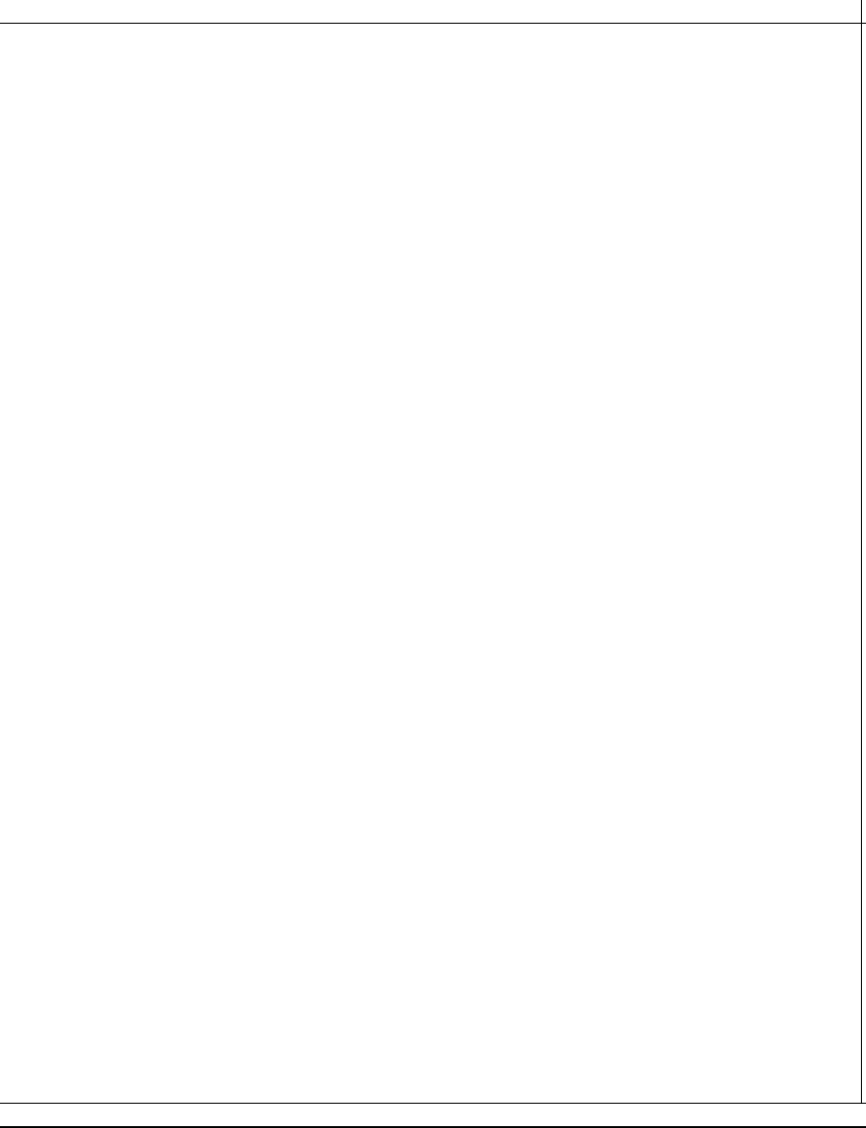
(TYP.) GIRDER TRUSS HOLD DOWN DETAIL
WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) GARAGE DOOR BUCK ATTACHMENT
WOOD FRAME



TYPICAL HEADER STRAPPING DETAIL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS

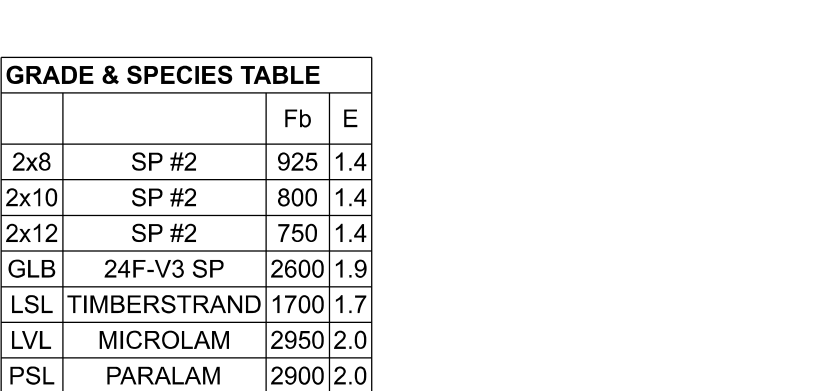


(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME

Uplift SP/Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
615	485	SDWC15600	-
415	290	H3	4-8d x 1 1/2"
575	495	H2.5A	5-8d x 1 1/2"
1340	1015	H10A	9-10d 1 1/2"
720	620	LTS12-20	6-10d 1 1/2"
1000	860	MTS12-30	7-10d 1 1/2"
1450	1245	HTS20-30	12-10d 1 1/2"
Uplift SP	Uplift SPF	Strap Ties	To One Member To Other Member
1235	1235	LSTA21	8-10d
1640	1455	MSTA24	9-10d
1030	1030	CS20	7-10d
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud To Plate
585	535	SP1	6-10d
1065	605	SP2	6-10d
771	771	LSTA24	10-10d
1235	1235	LSTA24	14-10d
Uplift SP	Uplift SPF	Holdowns @ Stewall	To Stud / Post Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"
4235	3640	HTT4	18-16d x 1 1/2"
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post Anchor
1925	1800	DTT22	8-SDS 1/4"x1 1/2"
4235	3640	HTT4	18-16d x 1 1/2"
Uplift SP	Uplift SPF	Post Bases @ Stewall	To Post Anchor
2475	ABU4Z	12-16d	5/8"x12" Drill & Epoxy
1905	ABU6Z	12-16d	5/8"x12" Drill & Epoxy
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post Anchor
2475	ABU4Z	12-16d	5/8"x12" Drill & Epoxy
1905	ABU6Z	12-16d	5/8"x12" Drill & Epoxy

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:
THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 10' O.C. x 0.8 = 12.8' O.C.)

DOOR WIDTH	3/8"x4" LAG	16d STAGGER	(2) ROWS OF 131"x3 1/4" NAILS
8' - 10'	24" OC	5" OC	5" OC
11' - 15'	18" OC	4" OC	4" OC
16' - 18'	16" OC	3" OC	3" OC



(TYP.) GARAGE DOOR BUCK INSTALLATION
WOOD FRAME

		Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	MICROSTRAND	1700	1.7
LVL	TIMBERALAM	2950	2.0
PSL	PARALAM	2900	2.0

DESIGN CRITERIA & LOADS:

BUILDING CODE	8TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2023)
CODE FOR DESIGN LOADS	ASCE 7-22
BASIC WIND SPEED (ASCE 7-22, 3S Gust)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	I
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
C&C DESIGN PRESSURES	SEE TABLE
FLOOR LOADING	40 PSF LIVE LOAD
ROOFS OTHER THAN SLEEPING ROOM	30 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
ROOF LOADING	20 PSF LIVE LOAD
FLAT OR < 4:12	12:12 & GREATER
SOIL BEARING CAPACITY	1500 PSF
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE

COMPONENT & CLADING DESIGN PRESSURES 130 MPH (EXP C)	ZONE 4 INTERIOR	ZONE 5 END 4' FROM ALL OUTSIDE CORNER
EFFECTIVE WIND AREA (FT2)		
0 - 20	+25.6(Vasd) -27.8(Vasd)	+25.6(Vasd) -34.2(Vasd)
0 - 20	+42.6(Vult) -46.2(Vult)	+42.6(Vult) -57(Vult)
GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C)		
9x7 GARAGE DOOR	+22.6(Vasd) -25.5(Vasd)	
16x7 GARAGE DOOR	+21.7(Vasd) -24.1(Vasd)	

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT AND, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER HAS SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN. FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 2500 PSI. WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, F_y = 80ksi, WELDED WIRE REINFORCEMENT FABRIC (W.W.F.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH: 12 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1118. SUPPLIER TO PROVIDE ASTM C 1118 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1:5 AND TYPICAL SPACING OF CUTS TO BE LEFT. DO NOT CUT W/WW OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDING TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 40, DEFORMED BARS, F_y = 40 KSI, ALL LAP SPLICES 40" DB (25" FOR #5 BARS), UNO, ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH 318-6, I.G.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL. DIAPHRAGMS, SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 10" IN GROUTED CMU.

BUILDER'S RESPONSIBILITY:
THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN ONITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN:
THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FIBER REQUIRED REACTIONS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

FL PE 53915
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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

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JOB NUMBER:
241011
S-1
OF 3 SHEETS

Corey Amira Custom Homes

Kline Res.

PROJECT ADDRESS:
20364 S US HWY 441, High Springs, FL

FL PE 53915

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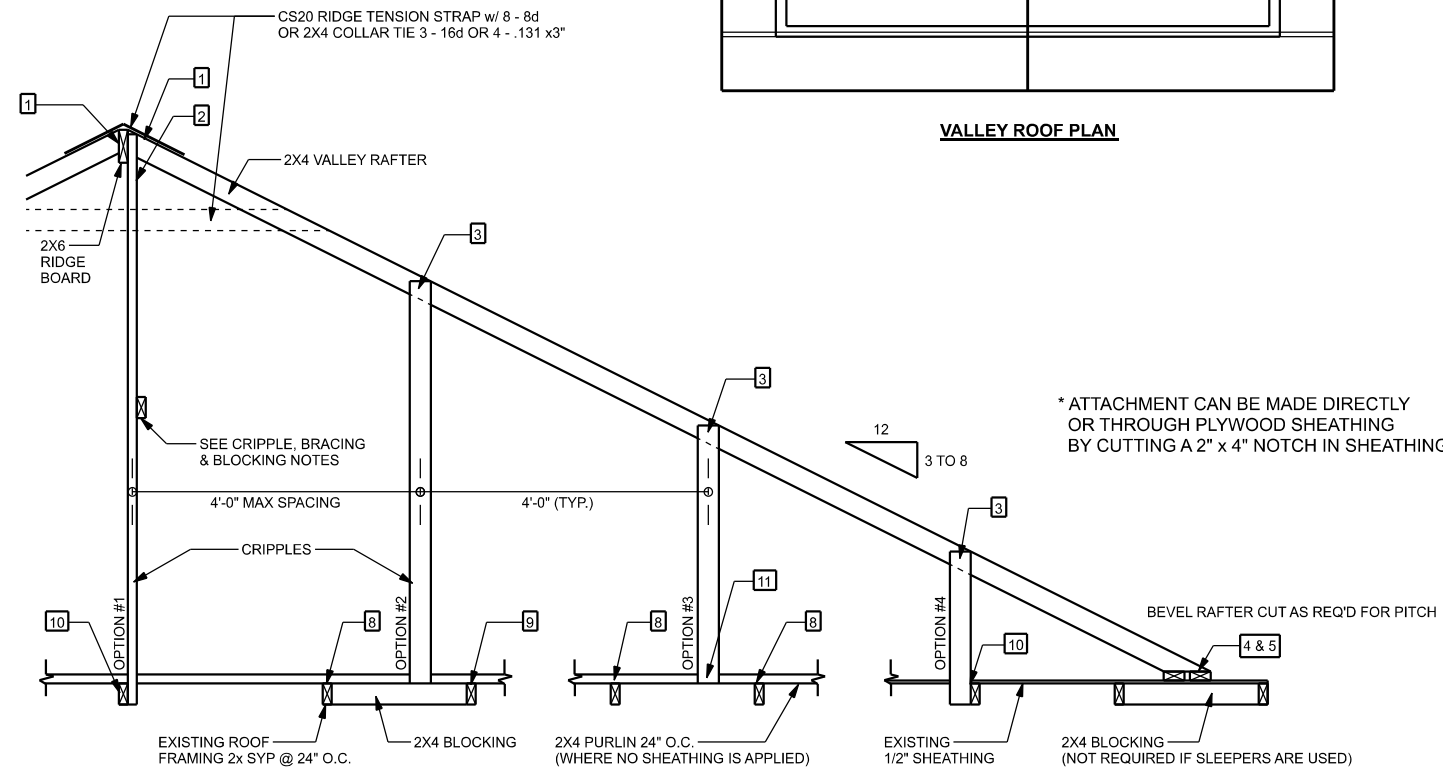
disowaydesign@gmail.com

JOB NUMBER:

241011

S-1

OF 3 SHEETS



SECTION CUT PARALLEL TO VALLEY RAFTER

VALLEY ROOF PLAN MEMBER LEGEND

===== TRUSS
 = = = = TRUSS UNDER VALLEY FRAMING
 | | | | | VALLEY RAFTER OR RIDGE
 ● CRIPPLE
 CRIPPLES 4'-0" O.C. FOR 20 psf (TL) AND 10 psf (TD) (TYP. SHINGLE ROOF) MAX.

CONNECTION REQUIREMENT NOTES

1	2X4 RAFTERS TO RIDGE	3-16@ 0 R - 131' x 3" TOE NAILS
2	CRIPPLE TO RIDGE	3-16@ 0 R - 131' x 3" FACE NAILS
3	CRIPPLE TO RAFTERS	3-16@ 0 R - 131' x 3" FACE NAILS
4	RAFTER TO SLEEPER OR BLOCKING	3-16@ 0 R - 131' x 3" TOE NAILS
5	SLEEPER TO TRUSS	4-16@ 0 R - 131' x 3" FACE NAILS EACH TRUSS
6	RIDGE BOARD TO ROOF BLOCK	3-16@ 0 R - 131' x 3" TOE NAILS
7	RIDGE BOARD TO TRUSS	3-16@ 0 R - 131' x 3" TOE NAILS
8	PURLIN TO TRUSS (TYP)	3-16@ 0 R - 131' x 3" NAILS
9	PURLIN TO TRUSS (IF CRIPPLE IS ATTACHED TO PURLIN)	4-16@ 0 R - 131' x 3" NAILS
10	TRUSS TO BLOCKING	3-16@ 0 R - 131' x 3" END NAILS
11	CRIPPLE TO TRUSS	3-16@ 0 R - 131' x 3" FACE NAILS
12	CRIPPLE TO PURLIN	3-16@ 0 R - 131' x 3" FACE NAILS

GENERAL NOTES

MAXIMUM RAFTER SPANS
8'-0" FOR 2X4, 9'-0" FOR 2X6 SPF #2 OR SYP #2.

MAXIMUM ROOF AREA PER SUPPORT
1612 IN ZONES 2 & 3, 2492 IN ZONE 1. (EXAMPLE: 4'-0" O.C. X 4'-0" SPAN
= 1612 OR 2'-0" X 8'-0" SPAN = 1612)
PURLINS REQUIRED 2'-0" O.C. IF EXISTING SHEATHING IS REMOVED.
PURLINS SHOULD OVERLAP SHEATHING ONE TRUSS SPACING MINIMUM
IN CASES THAT THIS IS IMPRACTICAL, OVERLAP SHEATHING A MINIMUM
OF 6", AND NAIL UPWARDS THROUGH SHEATHING INTO PURLIN WITH A
MINIMUM OF 6" OF CONCRETE THROUGH SHEATHING.

THIS DRAWING APPLIES TO VALLEYS WITH THE FOLLOWING CONDITIONS:
 SOME DISTANCE BETWEEN HILLS; 40' OR LESS

- * MAXIMUM VAIL HEIGHT: 14" OR LESS
- * MAXIMUM VAIL LENGTH: 12" OR LESS
- * MAXIMUM MEAN ROOF PITCH: 30 FEET
- * MAXIMUM TOTAL LOADING:
 - MEETS FSC / ASCE 7 WIND REQUIREMENTS
 - EXPOSURE CATEGORY "C" 1.0, 1.5, 2.0
 - ENCLOSURE CARLOADING

CRIPPLE BRACING & BLOCKING NOTES

2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 5'0" TO 10'0" LONG AND 12" OR MORE IN DEPTH. CLBS ARE TO BE INSTALLED AT 10'0" ON CENTER WITH NAILING AS 18" O.C. OR SHORTER MAY BE 90% OF CRIPPLE LENGTHS. CRIPPLES OVER 10'0" LONG REQUIRE TWO CLBS OR BOTH CRIPPLES W/ 1" OF SCAB USE STRESS REDUCED LUGS. CRIPPLES 5'0" OR SHORTER MAY BE 90% OF CRIPPLE LENGTHS. CRIPPLES WITH NARROW DEPTH OF CRIPPLE CAN FACE RIDGE OR RAFTER.

3X6 OR 4X6 CRIPPLES ARE TO BE USED FOR CRIPPLES 10'0" OR LONGER. CRIPPLES ARE TO BE INSTALLED INTO RIDGE BOARD.

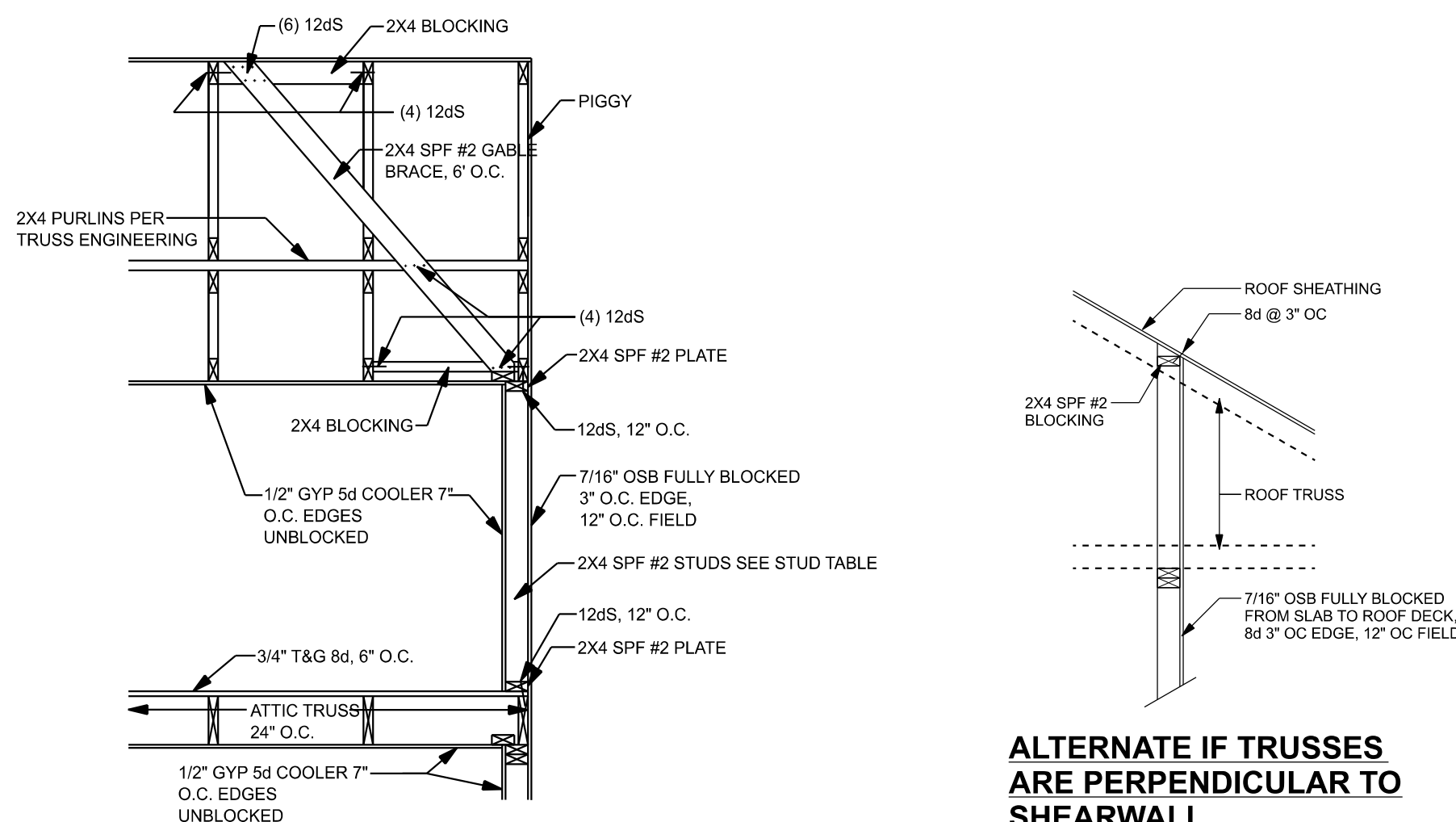
INSTALL & JOINT UNDER JOINTS OF SLEEPERS ARE NOT USED.

INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FALL BETWEEN 12" ON CENTER OR LONGER. CRIPPLES 12" OR LONGER ARE TO BE JOINTED UNDER CRIPPLES.

* APPLY ALL NAILING IN ACCORDANCE TO NDS-1979 SECTION 12. NAILS ARE COMMON LOWER

ROOF OVER FRAMING & BRACING DETAIL

SCALE: N.T.S.

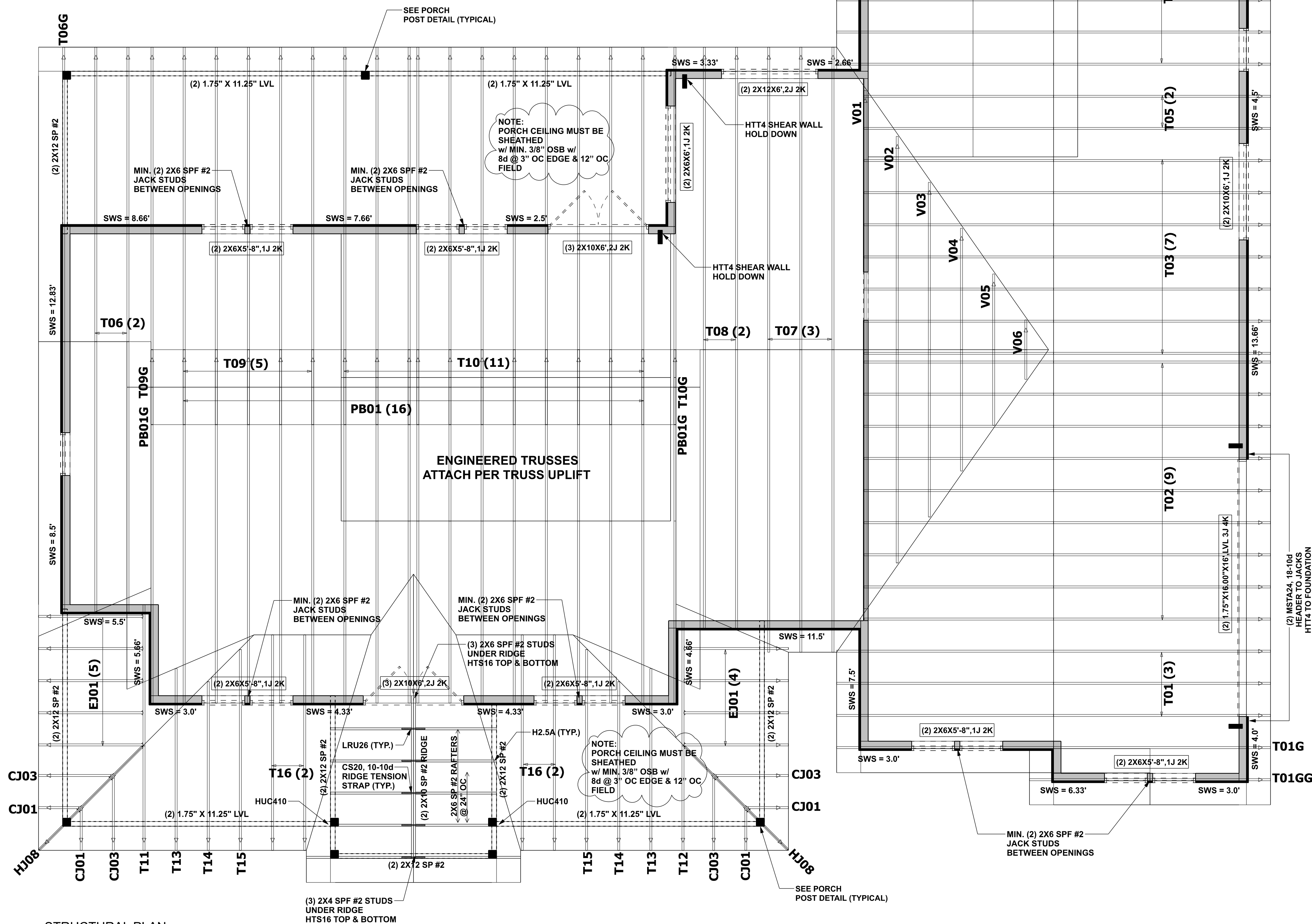


BONUS ROOM / GABLE END BRACING

SCALE: 1/2"=1'-0"

**ALTERNATE IF TRUSSES
ARE PERPENDICULAR TO
SHEARWALL**

NOTE:
IF THE ABOVE DETAIL IS USED
ON THE FRONT & REAR PORCH WALL
THE FRONT & REAR PORCH CEILING DOES NOT
NEED TO BE SHEATHED



STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

SN-1 DIMENSIONS ON STRUCTURAL SHEETS
ARE NOT EXACT. REFER TO ARCHITECTURAL
FLOOR PLAN FOR ACTUAL DIMENSIONS

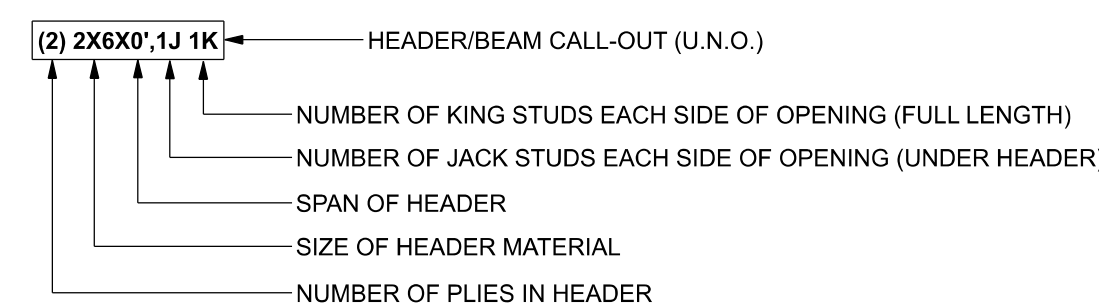
PERMANENT TRUSS BRACING IS TO BE INSTALLED AT
LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS.
SN-2 LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03,
BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3
ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED
TRUSS PACKAGE

UNLESS NOTED OTHERWISE (MINIMUM REQUIREMENTS) ***SEE STRUCTURAL PLAN FOR ANY SPECIFIC CALL OUTS***	
BEAM / HEADERS (SIZE)	ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SP #2 (UNO)
HEADERS (JACK & KING STUDS)	ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (UNO)
HEADERS (STRAPING)	ALL HEADERS w/ UPLIFT TO BE STRAPPED DOWN @ EACH SIDE WITH (1) LSTA24, 14-104 @ TOP & BOTTOM OF WALL WRAP UNDER BOTTOM PLATE & OVER TOP PLATE 1/2" X 10" ANCHOR BOLT w/ 3" X 3 x 7-1/4" WASHER MUST BE LOCATED WITHIN 6" OF KING STUD @ ALL DOOR LOCATIONS (U.N.O.)
JACK STUDS UNDER GIRDER TRUSS	USE ONE JACK STUD GIRDER SUPPORT PER 2000 LB LOAD

ACTUAL vs REQUIRED SHEARWALL

	TRANSVERSE	LONGITUDINAL
ACTUAL	20073 LBF	20671 LBF
REQUIRED	19875 LBF	16731 LBF

HEADER LEGEND



CONNECTIONS, WALL, & HEADER DESIGN IS BASED
ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING
FURNISHED BY BUILDER. BUILDERS FIRST SOURCE
JOB #4223140

Corey Amira Custom Homes

Kline Res.

PROJECT ADDRESS:
20364 S US HWY 441, High Springs, FL

FL PE 53915

This item has been digitally signed and sealed by Mark Disoway PE on digital signature date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.



C=US, O=Florida
dnQualifier=A01
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CN=Mark d
Disosway
2024-09-19 15:2
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DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering, comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

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JOB NUMBER:

241011

S-3

OF 3 SHEET