GENERAL NOTES:

CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

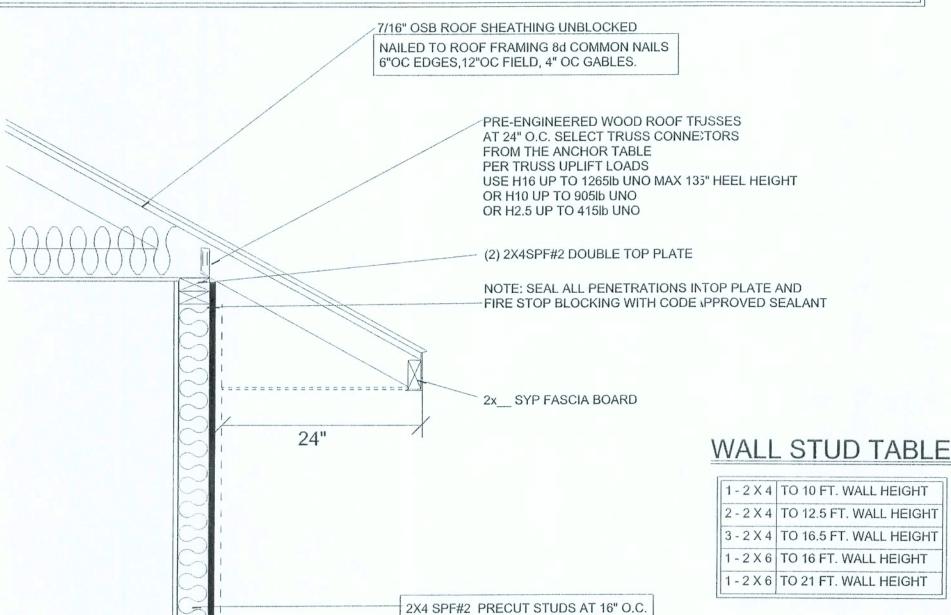
MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOREXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED II THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOVED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS - A-307. MINIMUM EMBEDMENT: 7" IN CONCRETE OR REINFORCED BOND BEAM; 15" IN GROUTED CMU.

CONCRETE - MINIMUM COMPRESSIVE STRENGTH, Fc = 2500 psi.

REBAR - GRADE 40 DEFORMED BARS, Fy = 36 ksi. ALL LAPS 40*Db (25"FOR #5) UNLESS OTHERWISE SPECIFIED.

NAILS - ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY SBC TEST REPORTS AS IAVING EQUAL STRUCTURAL VALUES.



WITH SP4 TOP, BOTTOM & 48"OC

8d COMMON NAILS

6" OC EDGE, 12" OC FIELD

STEEL WASHER 48" O.C

7/16" O.S.B. WALL SHEATHING FULLY BLOCKED

4" CONCRETE FLOOR SLAB REINFORCED WITH

2 x 4 P.T. PINE SOLE PLATE ANCHORED WITH

WITH 1/2"X10" ANCHOR BOLTS WITH 2X2X.140"

1-#5, CONTINUOUS, IN CONCRETE BOND BEAM

AT SLAB EDGE INTERSECTION WITH STEMWALL

6X6-1.4/1.4 WELDED WIRE MESH PLACED ON CHARS

AT 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-NIL

POLY VAPOR BARRIER WITH 6" LAPS SEALED WITH

POLY TAPE OVER TERMITE-TREATED AND COMFACTED FILL

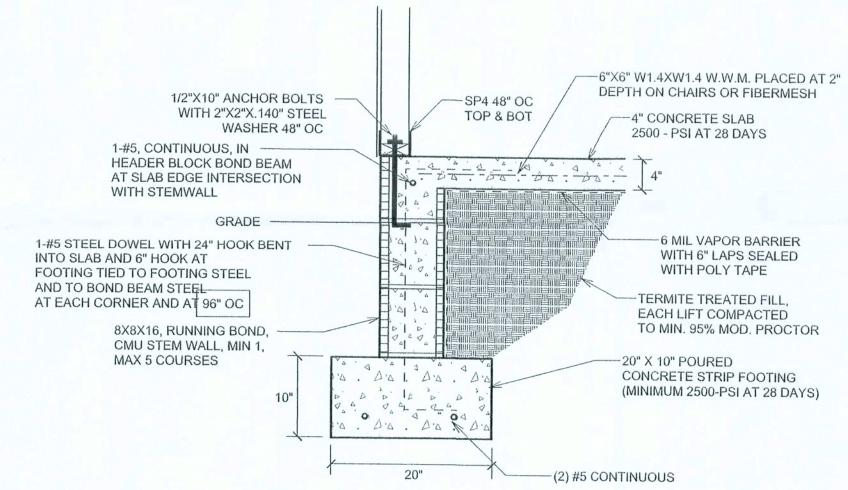
UNLESS OTHERWISE NOTED ON SHEARWALL LAYOUT

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS

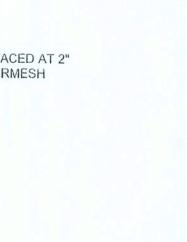
uplift lbs.	TRUSS CONNECTOR*	TO PLATES	TO RAFTER	TO STUJDS
< 415	H2.5	5-8d, 1 1/2"	5-8d, 1 1/2"	
< 750	H16	4-10d, 1 1/2"	2-10d, 1 1/2"	, , , , , , , , , , , , , , , , , , ,
< 905	H10	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 1120	H15 - 1	4-10d, 1 1/2"	12-10d, 1 1/2"	12-10d, 1 1/2"
< 1120	H15 - 2	4-10d, 1 1/2"	4-10d, 1 1/2"	12-10d, 1 1/2"
< 1265	H16 ~ 1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1265	H16 - 2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 860	MTS20	7-10d, 1 1/2"	7-10d, 1 1/2"	
< 1245	HTS20	12-10d, 1 1/2"	12-10d, 1 1/2"	
< 2490	2 - HTS20			
< 1785	LGT2	14 -16d, 1 1/2"	14 -16d, 1 1/2"	
	STUD STRAP CONNECTOR*			Population
< 735	SP4	6-10d, 1 1/2"		
< 905	LSTA21	16-10d, 1 1/2"		
< 1240	SPH4	10-10d, 1 1/2"		
< 1005	CS20	18-8d, 1 1/2"		
< 1650	CS16	28-8d, 1 1/2"		
	STUD ANCHORS*	TO STUDS	TO FLOOR	
< 1205	LTT19	8-16d	1/2" AB	
< 2185	LTTI31	18 - 10d	1/2" AB	
< 2565	HD2A	2 - 5/8" BOLTS	5/8" AB	
< 3480	HTT16	18 - 16d 1 1/2"	5/8" AB	
< 2200	ABU44	12-16d	1/2" AB	
< 2300	ABU66	12-16d	1/2" AB	

OPENINGS TO 5' WIDTH USE (1) OPENINGS 5' TO 7' WIDTH USE (2) SP4 TYPICAL STRAPPING (UNO) ANCHOR BOLTS MAY BE LOCATED AT EITHER SIDE OF KING STUDS. PLATE MUST BE CONT. BETWEEN BOLT AND KING STUDS ANCHOR BOLTS MUST BE WITHIN 12" OF KING STUDS 1/2" X 10" ANCHOR BOLTS AT 48" OC TYPICAL TYPICAL HEADER STRAPING DETAIL



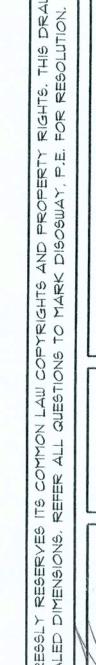
F1 - STEM WALL FOUNDATION SCALE: 1" = 1'-0"

INSPECTION REPORT SEESHT#Z



-6"X6" W1.4XW1.4 W.W.M. PLACED AT 2" DEPTH ON CHAIRS OR FIBERMESH GARAGE DOOR -POCKET -4" CONCRETE SLAB 2500 - PSI AT 28 DAYS 6 MIL VAPOR BARRIER (1) #5 CONTINUOUS WITH -WITH 6" LAPS SEALED STEM WALL BOND BEAM 18" WITH POLY TAPE TERMITE TREATED FILL,

I HEREBY CERTIFY THAT I HAVE EXAMINED THIS PLAN, AND THAT THE APPLICABLE PORTIONS OF THE PLAN, RELATING TO WIND ENGINEERING COMPLY WITH SECTION 1606, FLORIDA BUILDING CODE 2001, TO THE BEST OFMY KNOWLEDGE.



07 / Oct. / 04

MARK DISOSWAY

SEAL

SHEET NAME

STRUCTURAL

JOB NIMBER

31111a -

DETALS

P.E. 53915



TRUSSES SHALL BE DESIGNED BY A LICENSED ENGINEER IN ACCORDANCE WITH THE REQUIREMENTS OF THE "NATIONAL FOREST PRODUCTS ASSOCIATION" MANUAL FOR "STRESS RATED LUMBER AND IT'S CONNECTIONS" AND "TRUSS PLATE INSTITUTE" SUGGESTED GUIDELINES FOR TEMPORARY AND PERMANENT BRACING AND HANDLING OF TRUSSES. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING, ANY STICK FRAMING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, MAXIMUM UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE DESIGNING ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS, BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

APPROXIMATE FINISH GRADE 8" CONCRETE BLOCK STEMWALL, GROUT REINFORCED CELLS HEIGHT VARIES (MINIMUM 2 COURSES MAX 5 COURSES)

1-#5 STEEL DOWEL WITH 24" HOOK 3ENT INTO SLAB AND 6" HOOK @ FOOTING TIED TO FOOTING-STEEL, AND TO BOND BEAM STEEL AT EACH CORNER AND AT 96" O.C

20" X 10" POURED CONCRETE STRIFFOOTING (MINIMUM 2500-PSI AT 28 DAYS) REINFORCED WITH 2- #5 REBARS, CONTINUOUS

SINGLE STORY EXTERIOR WALL SECTION

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBC 2001, SECTION 1606 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WINDLOAD ENGINEER.

IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING

THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WINDLOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

WIR	OAD
200	IANT TO SECTION 1606.1.7 OF THE FLORIDA BUILDING CODE THE FOLLOWING DATA RELATING TO WIND LOADS WAS USED RATION OF THIS PLAN:
11	ASIC WIND SPEED = 110 MPH

1.) BASIC WIND SPEED = 110 MPH 2.) WIND INPORTANCE FACTOR = 1

3.) BUILDING CATAGORY = II 4.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

5.) DESIGNWIND PRESSURE (DOORS & WINDOWS) + 21.8 / -29.1 PSF GARAGEDOOR 9X7 = + 19.3 / - 24.1 PSF 16X7 AN) LARGER = + 18.5 / -20 PSF

DESIGN LOADS ROOF LIVE L)AD 20psf

FLOOR LIVE IOAD 40psf

FLOOR LIVE IOAD BEDROOMS 30psf

SCALE: 1" = 1'-0"

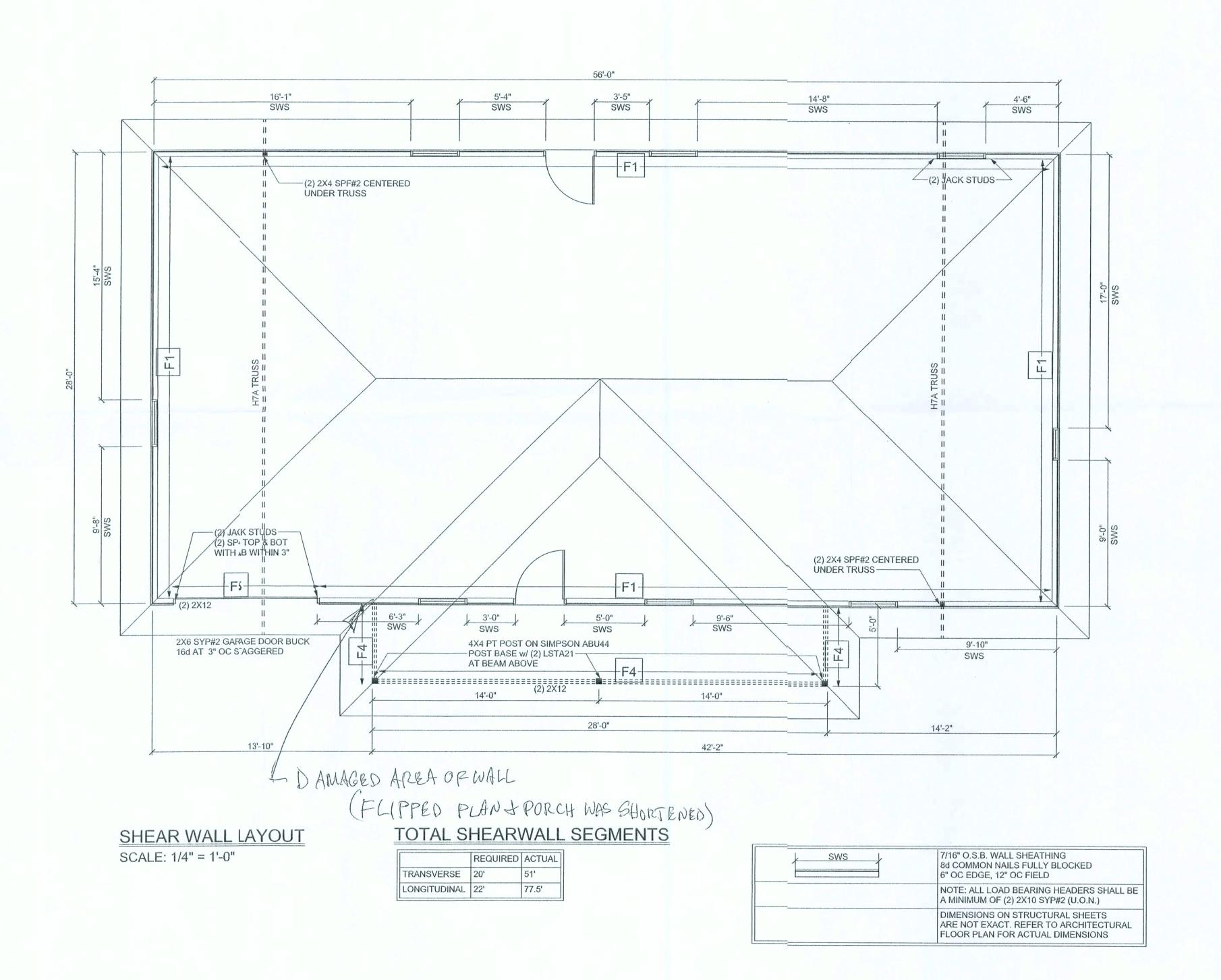
DEPTH ON CHAIR(S OR FIBERMESH -4" CONCRETE SLAB 2500 - PSI AT 28 DAAYS 4" MIN HOLUSE SLAB SLOPE PORCH SLAB TO DRAIN -6 MIL VAP(OR BARRIER WITH 6" LAAPS SEALED WITH POLYY TAPE -(1) #5 CONTINUOUS -TERMITE TIREATED FILL, EACH LIFT COMPACTED TO MIN. 955% MOD. PROCTOR F4 - PORCH SLAB

-6"X6" W1.4XW1.4 W.W.M. PLACED AT 2"

STEEL

EACH LIFT COMPACTED TO MIN. 95% MOD. PROCTOR (2) #5 CONTINUOUS WITH STEM WALL FOUNDATION STEEL

F5 - GARAGE DOOR POCKET



IN SPECTION REPORT I INSPECTED DAMAGE TO

THE FRONT WALL OF THIS
HOUSE DONE BY VEHICLE COLLISION.

THE DAMAGE APPEARED CONFINED
TO THE WOOD FRAMED STUDWALL

WITH NO DAMAGE TO ROOF,

CEILING, OR FLOOR SLAB, REPAIR

SHOULD BE DONE TO MATCH

DETAILS ON SHERT I, REPLACE

STUDS + SHEATHING IN DAMAGED

AREA

AREA

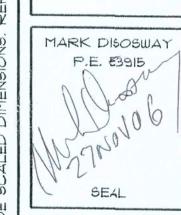
CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. (ANDERSON TRUSS CO. INC. JOB# 01 - 142 DATE 04/17/02 DESCRIPTION JEFFERY HILL)

I HEREBY CERTIFY THAT I HAVE EXAMINED THIS PLAN, AND THAT THE APPLICABLE PORTIONS OF THE PLAN, RELATING TO WIND ENGINEERING COMPLY WITH SECTION 1606, FLORIDA BUILDING CODE 2001, TO THE BEST OFMY KNOWLEDGE.

07 / Oct./ 04

RESIDENCE

JEFFERY HILL



SHEET NAME

SHEAR WALL LAYOUT

JOB NUMBER 311111a-2