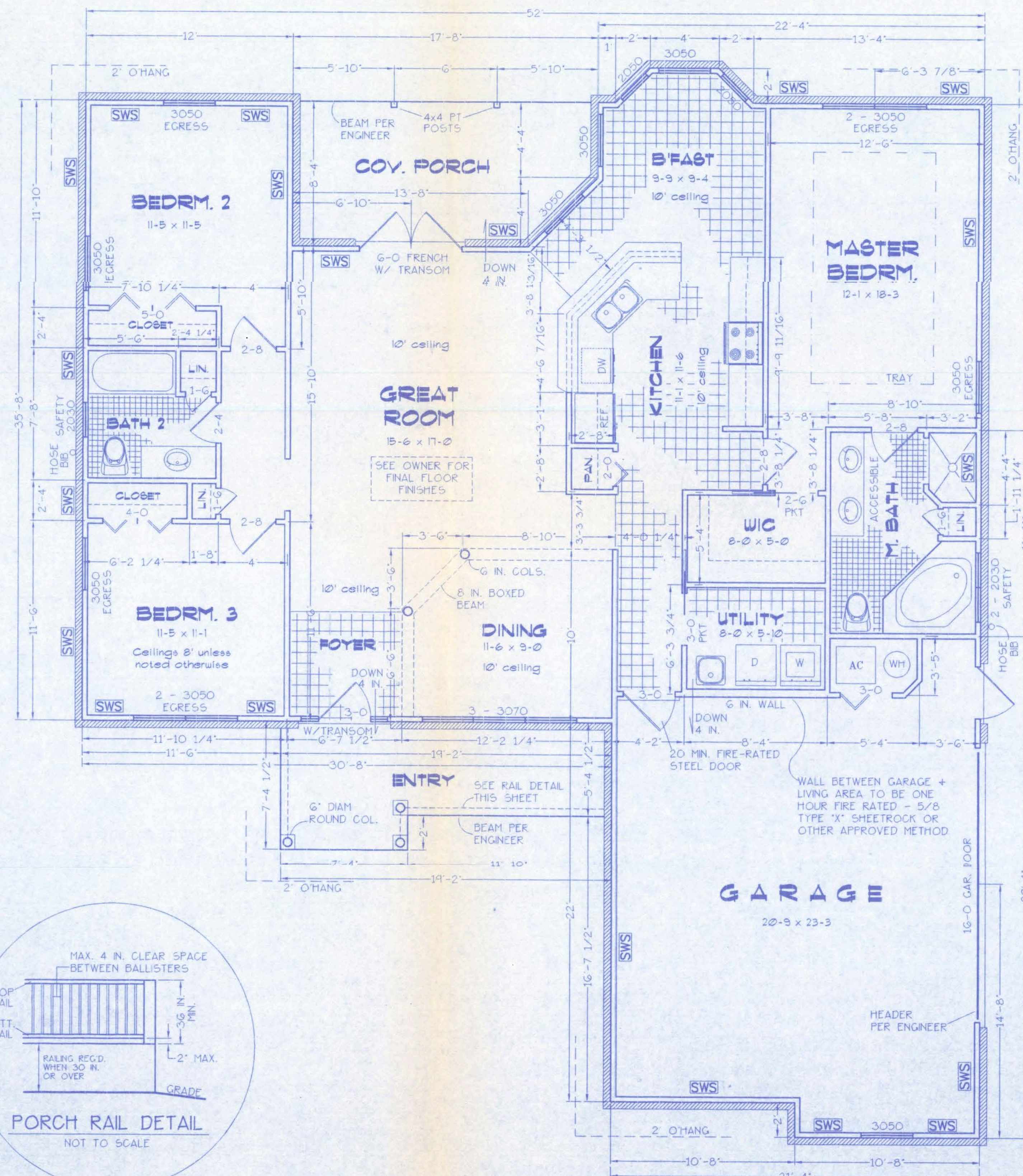


# Podlaszewski Residence

## AREA SUMMARY

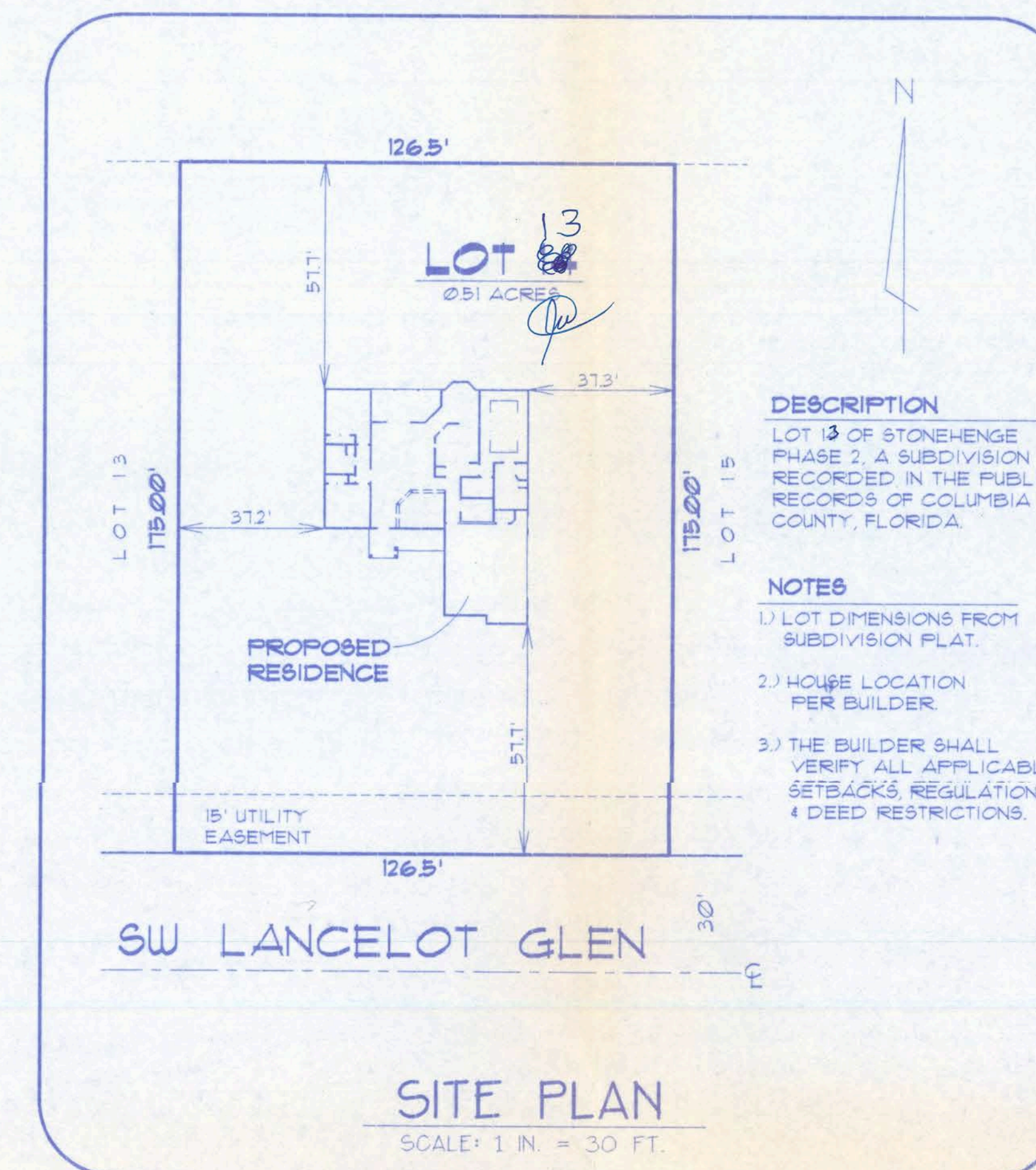
CONDITIONED	1685 SF
GARAGE	535 SF
FRONT ENTRY	118 SF
REAR COV. PORCH	139 SF
<b>TOTAL ROOFED</b>	<b>2477 SF</b>

**SWS** = Indicates a shearwall segment location referring to the labeled section of wall lying between the adjacent window / door openings in either direction. The shearwall areas have a height/width aspect ratio of 3-1/2 : 1 or wider.



## FLOOR PLAN

SCALE: 1/4 IN. = 1 FT.



## GENERAL NOTES

- See 'Wind Load Detail Sheet S-1' and Wind Engineer's Notes for data pertaining to Wind Design and compliance w/ Florida Building Code.
- All concrete used to be 2500 PSI strength or greater.
- HVAC duct and unit size/design is by engineered shop drawings from the AC contractor.
- Windows to be alum. framed and double glazed. Sizes shown are nominal and may vary with manufacturer.
- Roof Truss design is the responsibility of the supplier.
- The Truss Manufacturer shall prepare Shop Drawings indicating Truss placement, Girder locations, Truss-to-Truss Connections and any point loads. The Contractor shall notify the Designer of any point loads in excess of 2.0k for Fnd. Modification.
- Site analysis or preparation information is not a part of this plan and is the responsibility of the owner.
- Cabinet and millwork detail is not a part of this plan. The plan is a general design and details shall be the responsibility of the owner and/or contractor.

## Index to Sheets

SHEET A-1	SITE PLAN + FLOOR PLAN + GEN. NOTES
SHEET A-2	ELEVATIONS
SHEET A-3	FOUNDATION + SECTIONS
SHEET A-4	ELECTRICAL
SHEET S-1	WIND ENGINEERING

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code 2001, Section 1606 wind loads, to the best of my knowledge.

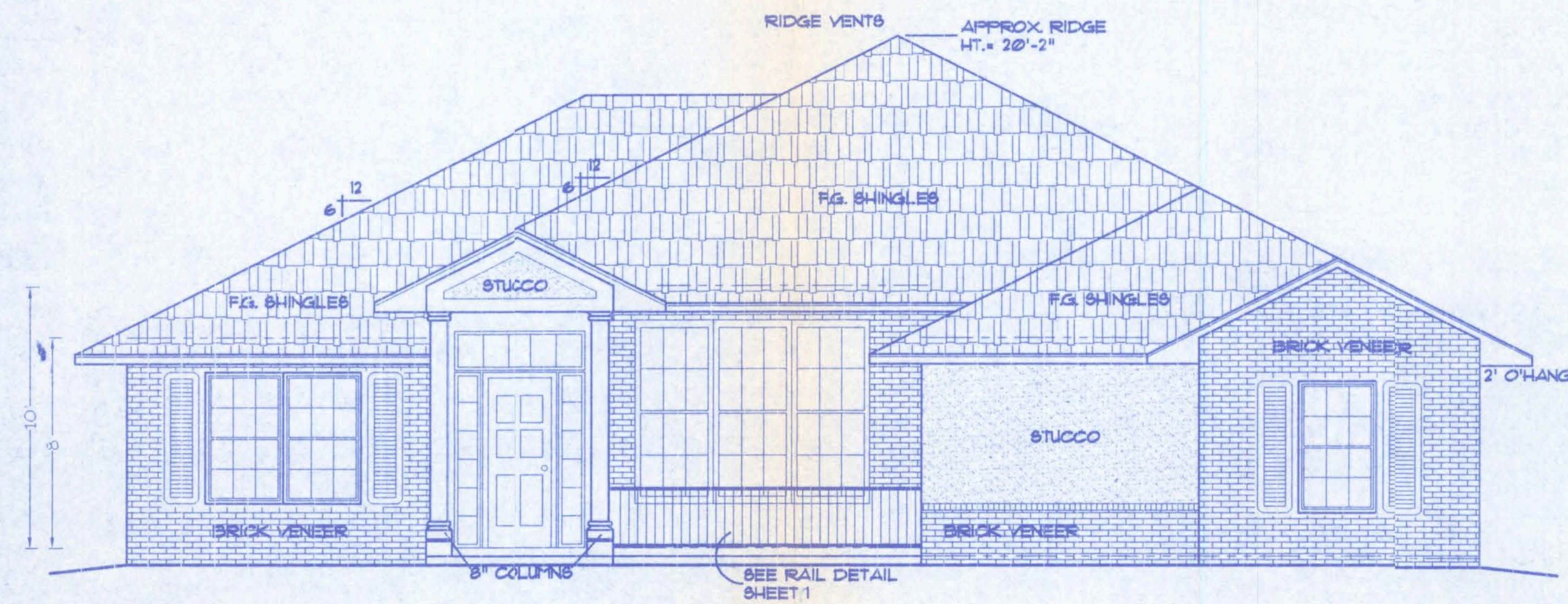
LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

LOT 14 STONEHENGE  
Location: PHASE 2 Job No.: 512122

**A-1**

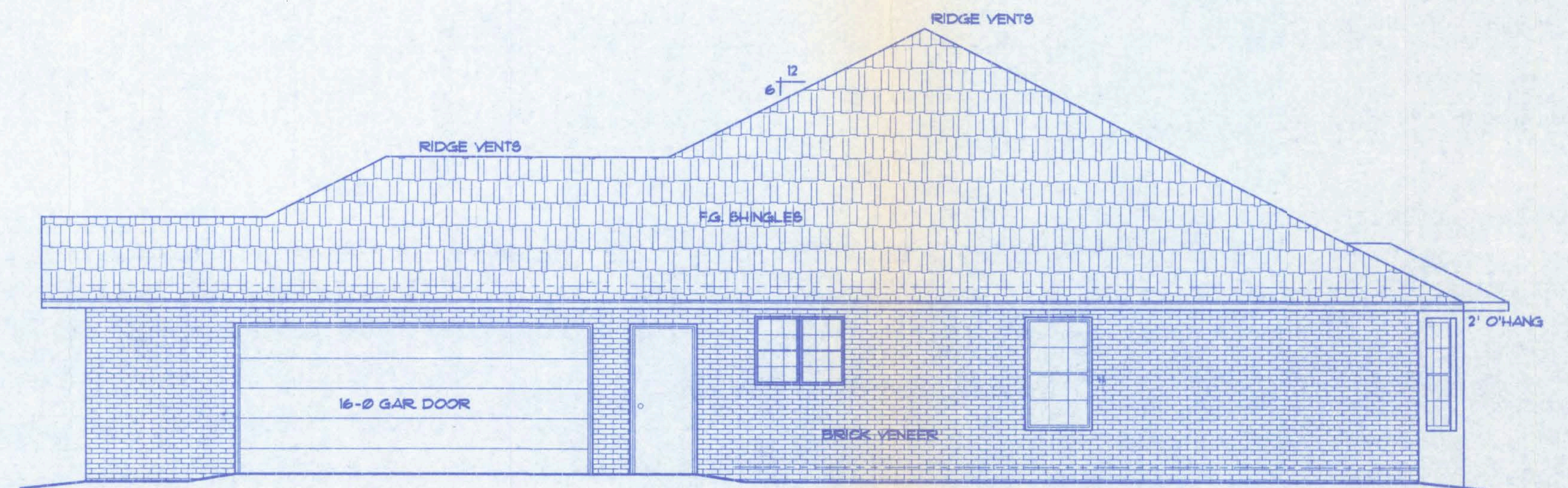
FILE: 05-048	<b>PODLASZEWSKI RESIDENCE</b>	SHEET: 1 OF 4
DATE: 12-10-05		CAD FILE: 05048
DRAWN: T A D	PREPARED BY: <b>TIM DELBENE</b> Drafting + Technical Services	REV:
CHECK: T A D	192 SW Sagewood Gln., Lake City, FL 32024 Phone (386) 755-5891	REV:





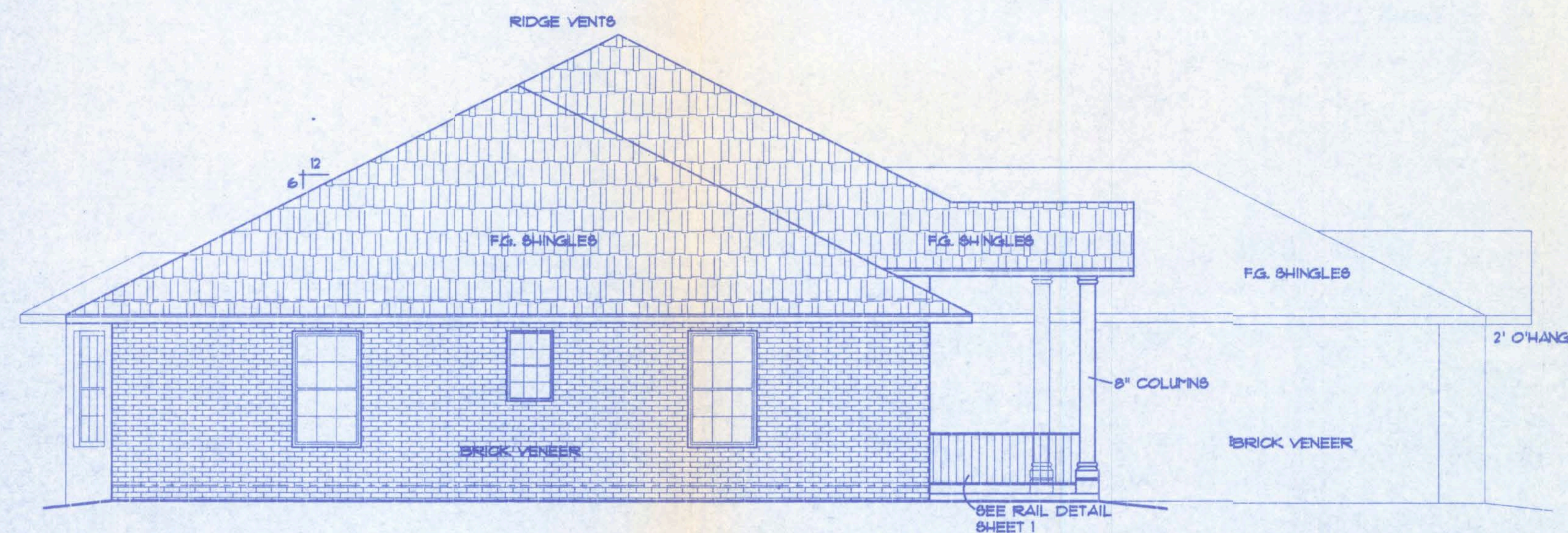
FRONT ELEVATION

SCALE: 1/4 IN. = 1 FT.



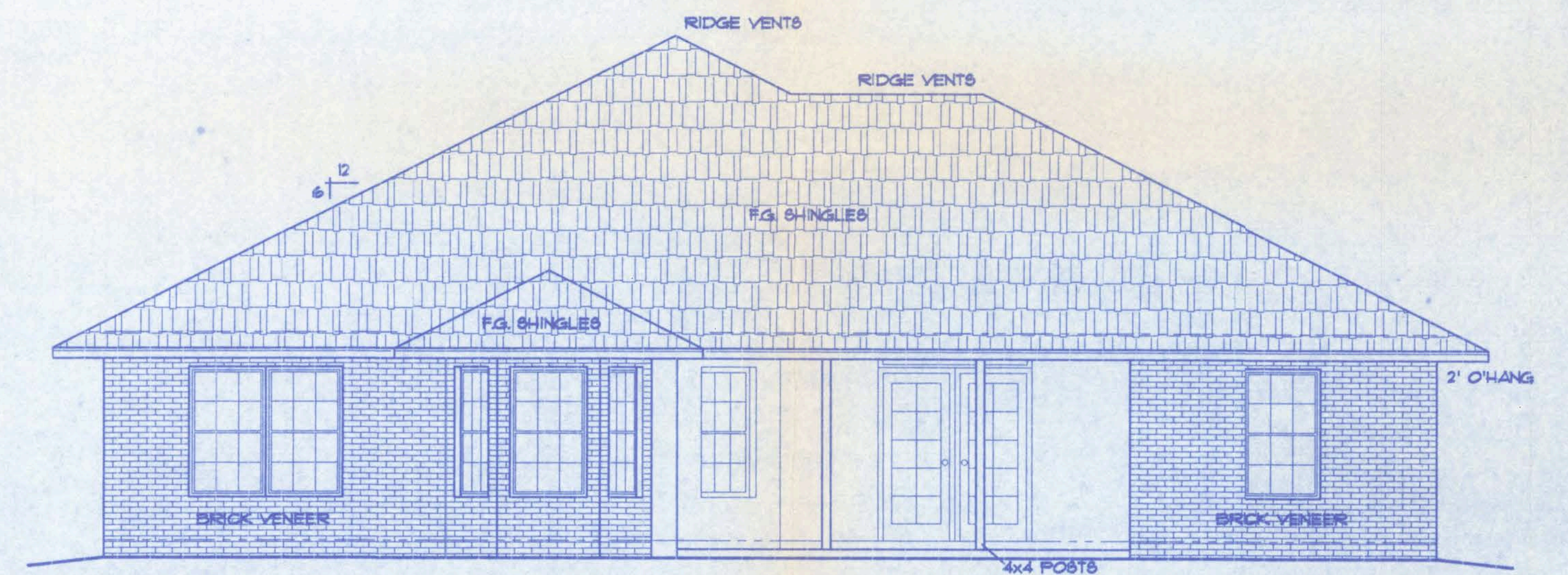
RIGHT ELEVATION

SCALE: 1/4 IN. = 1 FT.



LEFT ELEVATION

SCALE: 1/4 IN. = 1 FT.



REAR ELEVATION

SCALE: 1/4 IN. = 1 FT.

WINDLOAD ENGINEER: Mark Disoway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code 2001, Section 1606 wind loads, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

LOT 14 STONEHEDGE  
PHASE 2

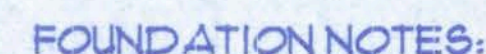
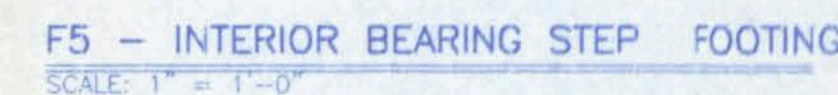
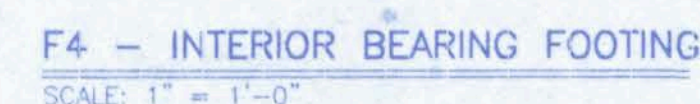
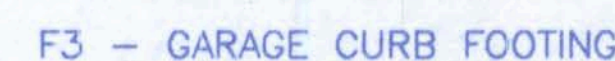
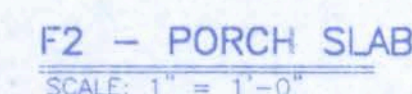
Location: Job No.: 512122

*M. A. Disoway*  
26 DEC 07

A-2

FILE: 05-048	PODLASZEWSKI RESIDENCE	SHEET: 2 OF 4
DATE: 12-10-05		CAD FILE: 05048
DRAWN: T A D	PREPARED BY: TIM DELBENE Drafting + Technical Services	REV:
CHECK: T A D	192 SW Sagewood Gln. Lake City, FL 32024 Phone: (386) 755-5891	REV:

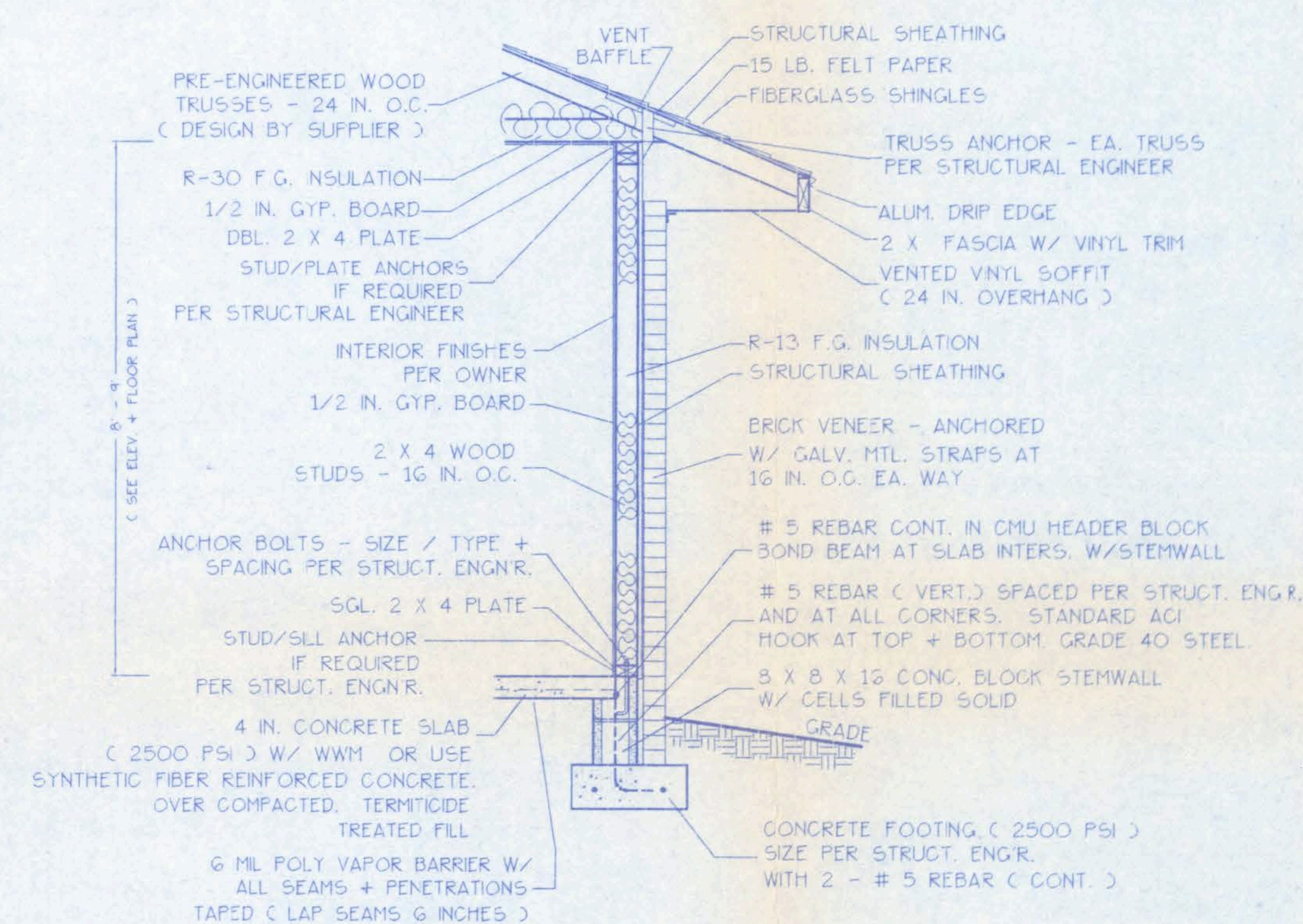




- CONTRACTOR SHALL EXAMINE ROOF TRUSS PLAN ( BY SUPPLIER ) TO DETERMINE ANY ADDITIONAL BEARING REQUIREMENTS BEFORE FINALIZING THE FOUNDATION PLAN.
- ALL CONCRETE @ 2500 PSI STRENGTH ( MIN. )
- SEE FLOOR PLAN FOR DIMENSIONS.
- SITE ANALYSIS AND PREPARATION DATA IS NOT A PART OF THIS PLAN AND IS THE RESPONSIBILITY OF THE CONTRACTOR / OWNER.

## FOUNDATION PLAN

SCALE: 1/4 IN. = 1 FT.



## WALL SECTION NOTES:

- This Typical Wall Section is for Estimating purposes only.
- All data shown in this Wall Section shall be subject to review and final input by the Structural Engineer.

### DESIGN WALL SECTION

## NON-STRUCTURAL DATA

SCALE: 1/2 IN. = 1 FT.

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056,  
386-754-5419

CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code 2001, Section 1606 wind loads, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

LOT 14 STONEHENGE

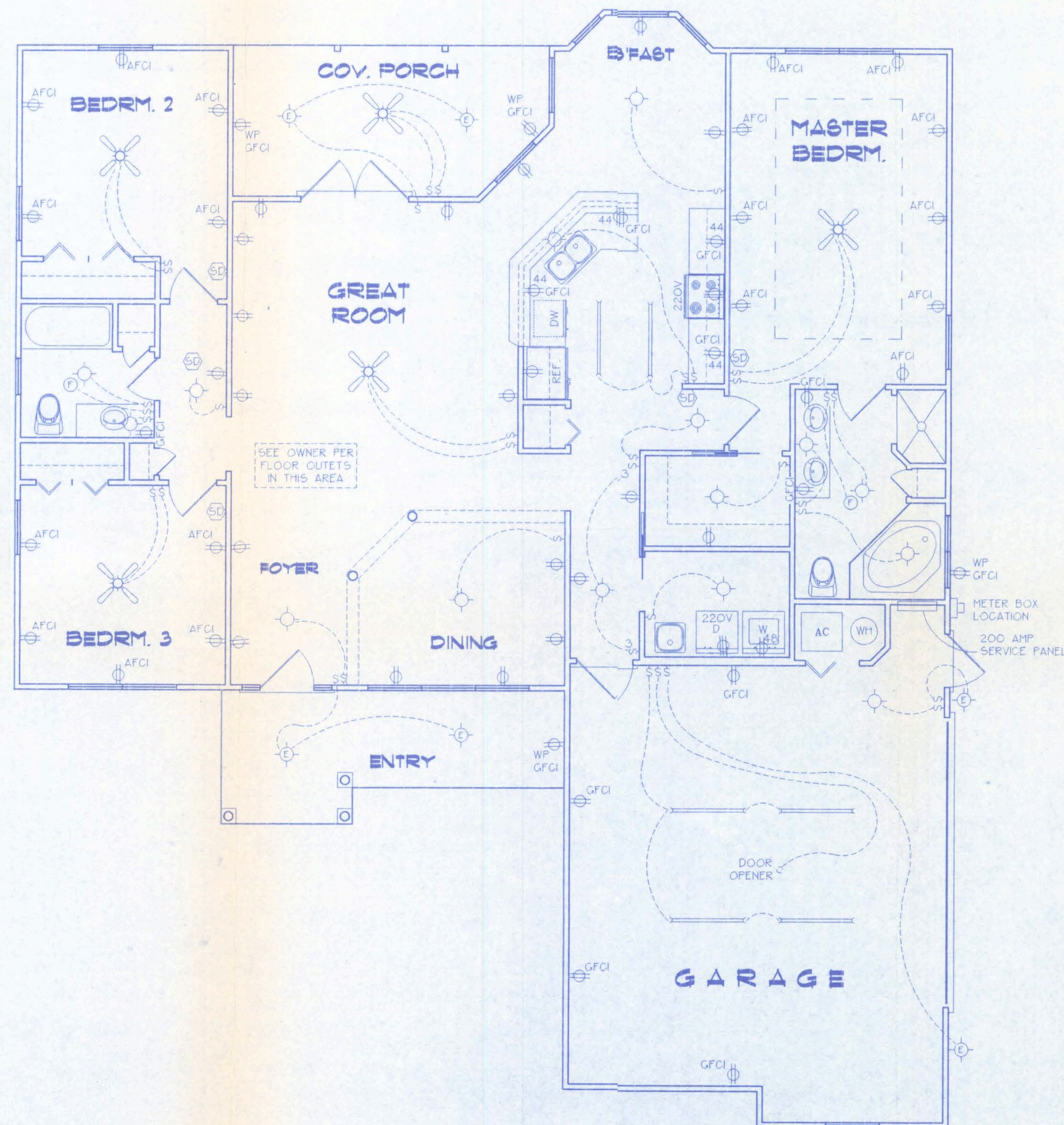
Location: PHASE 2 Job No.:                     

Job No.: 516122

FILE: O5-O48	<b>PODLASZEWSKI RESIDENCE</b>	SHEET: 3 OF 4
DATE: 12-10-05		CAD FILE: O5O48
DRAWN: T A D	PREPARED BY: <b>TIM DELBENE</b> Drafting & Technical Services	REV:
CHECK: T A D	192 SW 5596wood Gh. Lee City, FL 32024 Phone C 386 : 735-5891	REV:

A-3





ELECTRICAL SYMBOL LEGEND	
	= FLOURESCENT LIGHTING FIXTURE
	= CEILING LIGHT FIXTURE
	= EXTERIOR LIGHTING FIXTURE
	= LIGHT SWITCH
	= THREE-WAY SWITCH
	= 110 V. DUPLEX OUTLET
	= SPECIAL HEIGHT 110 V. DUPLEX OUTLET
	= GROUND FAULT CIRC. OUTLET
	= AFCI OUTLET
	= 110 V. SINGLE RECEPTACLE OUTLET
	= 220 VOLT OUTLET ( 4 WIRE )
	= FAN LOCATION ( CEILING )
	= FAN LOCATION ( EXHAUST )
	= SMOKE DETECTOR

#### ELECTRICAL PLAN NOTES

- WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE 1999.
- ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS. + IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION 1999.
- ELECTRICAL CONTR. SHALL BE RESPONSIBLE FOR THE DESIGN + SIZING OF ELECTRICAL SERVICE AND CIRCUITS.

**ELECTRICAL PLAN**  
NOT TO SCALE

LOT 14 STONEHENGE  
PHASE 2

**A-4**

FILE: 05-048	<b>PODLASZEWSKI RESIDENCE</b>	SHEET: 4 OF 4
DATE: 12-10-05		CAD FILE: 05048
DRAWN: T A D	PREPARED BY: <b>TIM DELBENE</b> Drafting + Technical Services	REV:
CHECK: T A D		REV:

192 SW Sagewood Dr. Lake City, FL 32024  
Phone: ( 386 ) 755-5891



# REVISIONS

SOFTPLAN  
ARCHITECTURAL DESIGN SOFTWARE

# ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 - 16d	14 - 16d	
		<b>HEAVY GIRDER TIEDOWNS*</b>			<b>TO FOUNDATION</b>
< 3965	< 3330	MSGT		22 - 10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10680	< 6485	HGT-2		16 - 10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 - 10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 - 10d	2-5/8" THREADED ROD 12" EMBEDMENT
		<b>STUD STRAP CONNECTOR*</b>			<b>TO STUDS</b>
< 435	< 435	SSP DOUBLE TOP PLATE	3 - 10d		4 - 10d
< 455	< 420	SSP SINGLE SILL PLATE	1 - 10d		4 - 10d
< 825	< 825	DSP DOUBLE TOP PLATE			8 - 10d
< 825	< 600	DSP SINGLE SILL PLATE	2 - 10d		8 - 10d
< 885	< 760	SP4			6 - 10d, 1 1/2"
< 1240	< 1065	SPH4			10 - 10d, 1 1/2"
< 885	< 760	SP6			6 - 10d, 1 1/2"
< 1240	< 1065	SPH6			10 - 10d, 1 1/2"
< 1235	< 1165	LSTA18	14 - 10d		
< 1235	< 1235	LSTA21	16 - 10d		
< 1030	< 1030	CS20	18 - 8d		
< 1705	< 1705	CS16	28 - 8d		
		<b>STUD ANCHORS*</b>		<b>TO STUDS</b>	<b>TO FOUNDATION</b>
< 1350	< 1305	LTT19	8 - 16d		1 1/2" AB
< 2310	< 2310	LTT131	18 - 10d, 1 1/2"		1 1/2" AB
< 2775	< 2570	HTY16			5/8" AB
< 4175	< 3695	HTY16	16 - 16d		5/8" AB
< 1400	< 1400	PHW42	16 - 16d		
< 3335	< 3335	HPWH22	16 - 16d		
< 2200	< 2200	ABU44	12 - 16d		1 1/2" AB
< 2300	< 2300	ABU66	12 - 16d		1 1/2" AB
< 2320	< 2320	ABU88	16 - 16d		2-5/8" AB

# GENERAL NOTES:

TRUSSER: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS ASSUMING 100 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, Fc = 3000 PSI

WELDED WIRE REINFORCED SLAB: 8" x 6" W14 x W14, FB = 80KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 10 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD. PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIO OF SLAB BEAMS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W/M OR REINFORCING STEEL (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE).

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, Fy = 60 KSI. ALL LAP SPLICES 48" DB (30" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM GLB, 24F-V3SP, Fb = 2.4Msi, E = 1800Ksi. UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH #4 COMMON NAILS (13d), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4"OC UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

# BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS SIZE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

# ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND REACTION LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL MOVEMENT. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES OR THEIR SEALED TRUSS SHEETS.

# GRADE & SPECIES TABLE

	Fb (psi)	E (10 <sup>6</sup> psi)
2x8 SYP #2	1200	1.6
2x10 SYP #2	1050	1.6
2x12 SYP #2	975	1.6
GLB 24F-V3 SP	2400	1.8
LSL TIMBERSTRAND	1700	1.7
LVL MICROLAM	2900	2.0
PSL PARALAM	2900	2.0

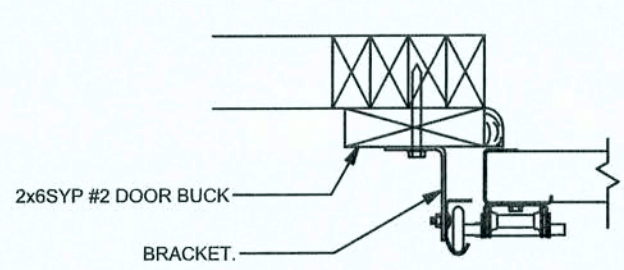
# MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI530.1-02 Section	Specific Requirements
1.4A Compressive strength	8" block bearing walls F'm = 1500 psi
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 6.5"x2 1/2"x11.5"
2.4 Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F Coating for corrosion protection	Anchors, sheet metal lies completely embedded in mortar or grout, ASTM A525, Class 90, 0.60 oz/lb2 or 304SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal lies not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb2 or 304SS
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

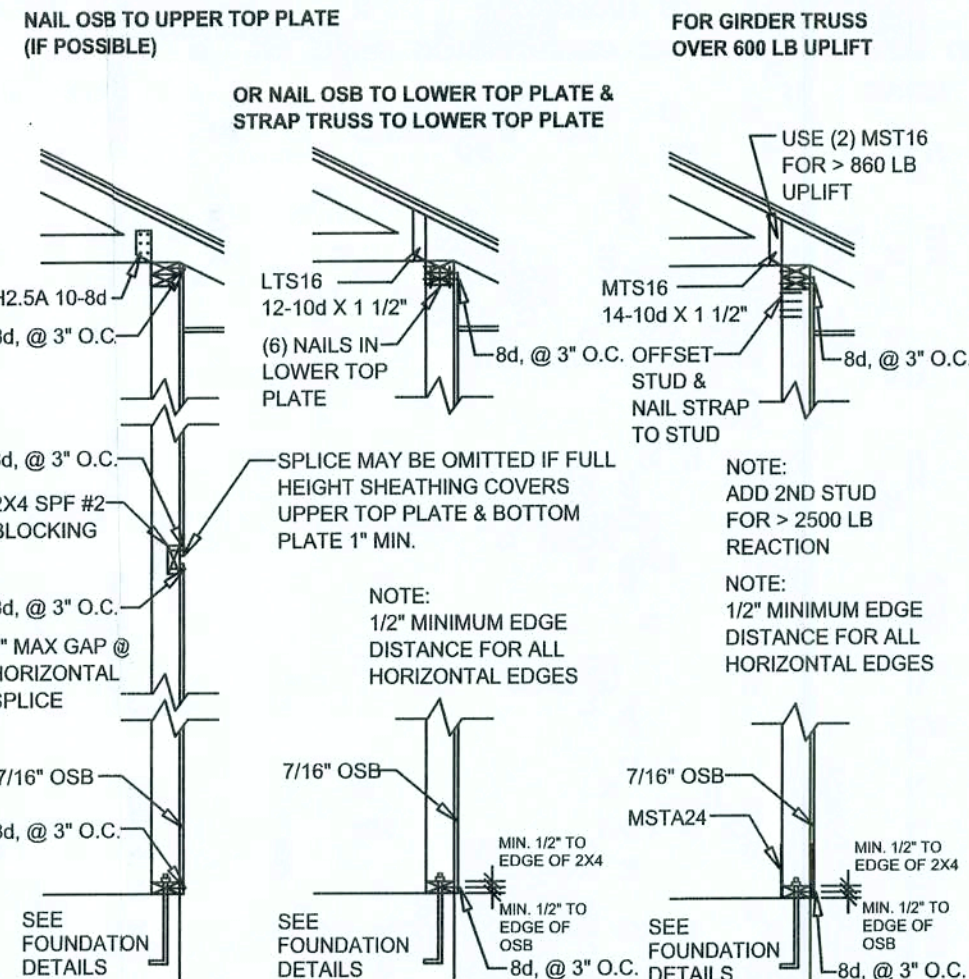
2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT  
ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSINK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4" GN PER TABLE BELOW.

DOOR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF .131 x 3 1/4" GN
8' - 10'	24" O.C.	5" O.C.	5" O.C.
11' - 15'	18" O.C.	4" O.C.	4" O.C.
16' - 18'	16" O.C.	3" O.C.	3" O.C.



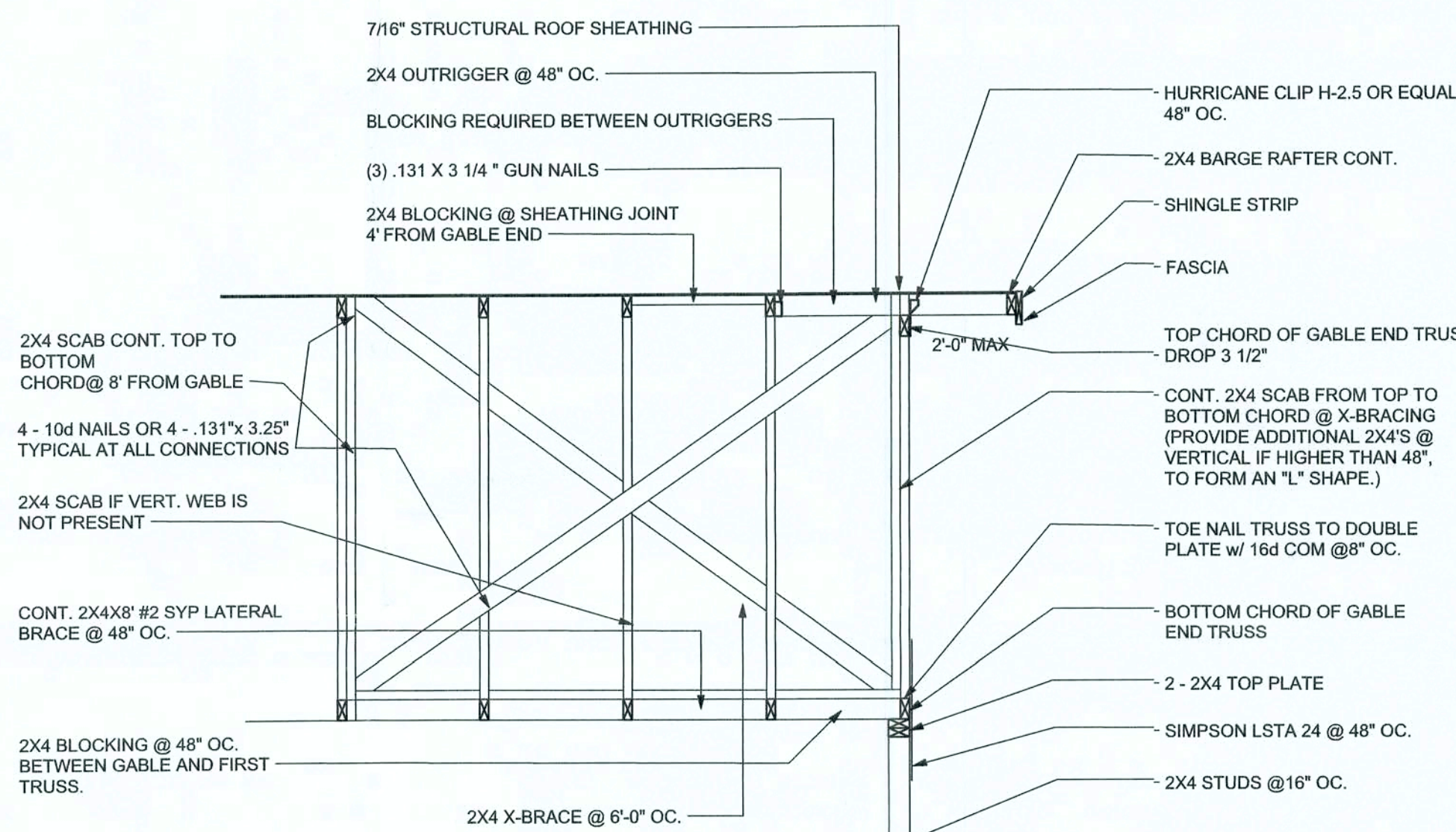
# GARAGE DOOR BUCK INSTALLATION DETAIL

SCALE: N.T.S.



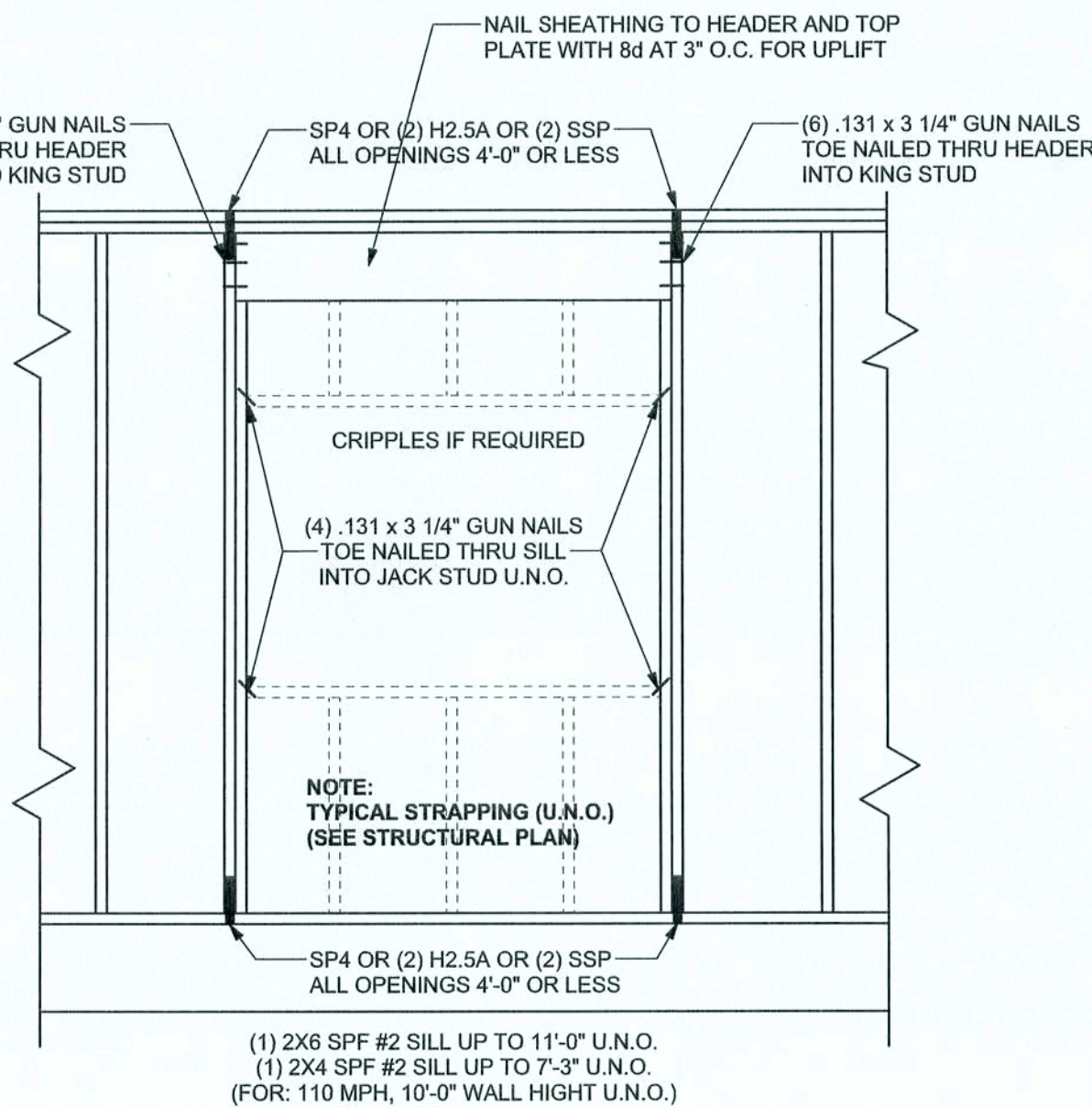
# W61 - SHEATHING NAILING FOR TRUSS UPLIFT

SCALE: 1/2"=1'-0"



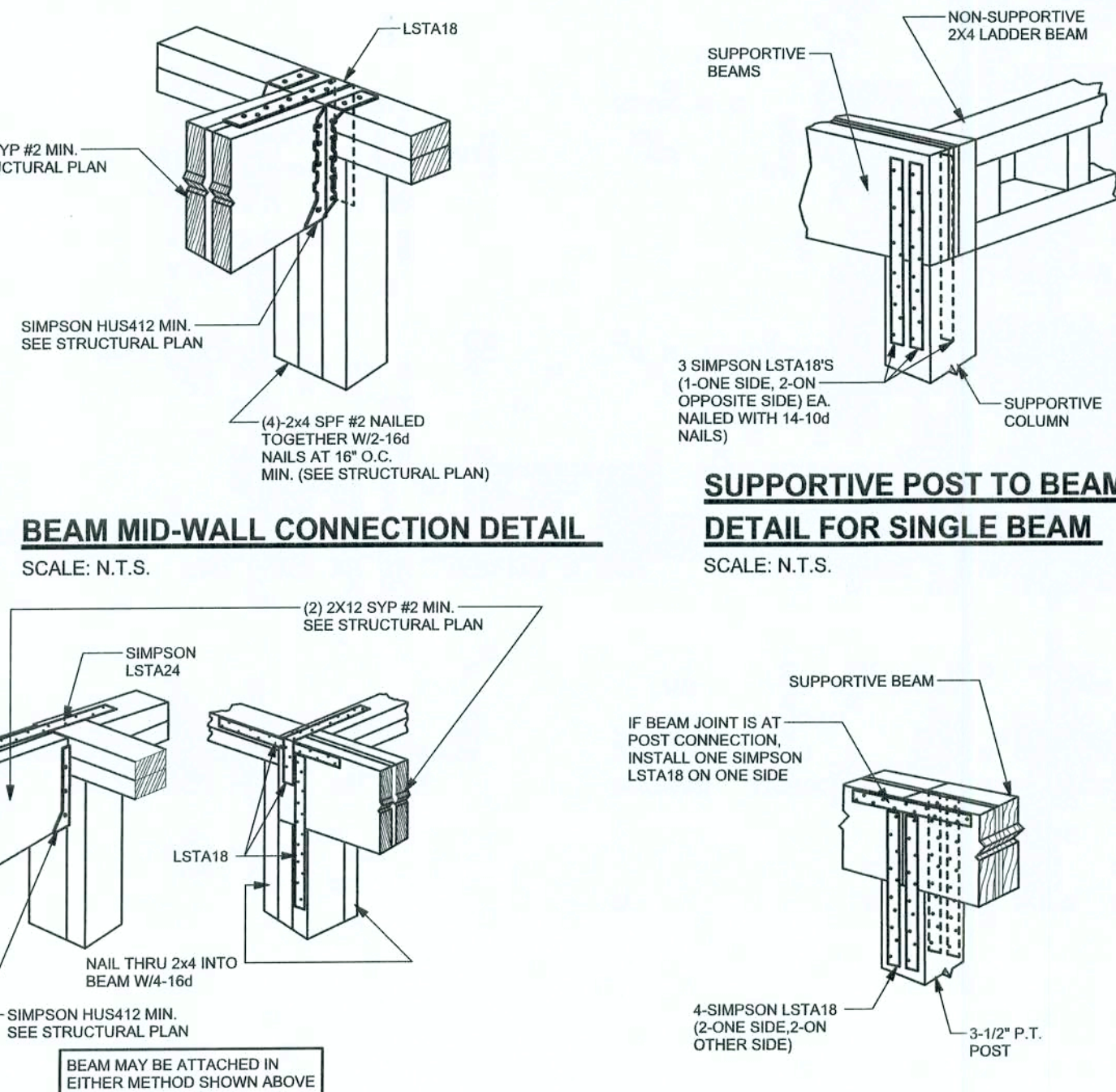
# TYPICAL GABLE END ( X-BRACING )

ALL MEMBERS SHALL BE SYP



# TYPICAL HEADER STRAPPING DETAIL (SP4/6)

SCALE: 1/2" = 1'-0"

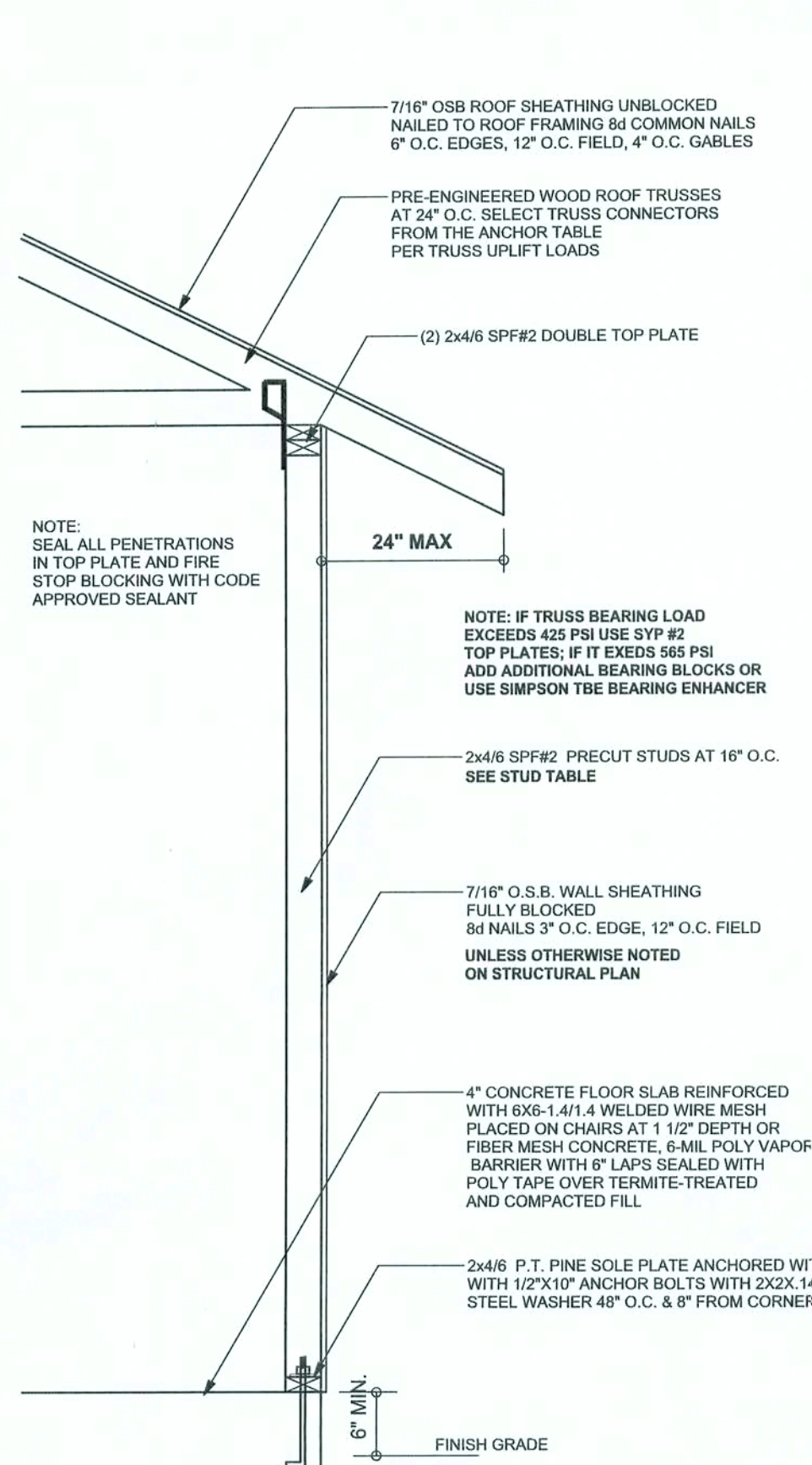


# BEAM CORNER CONNECTION. DETAIL

SCALE: N.T.S.

# SUPPORTIVE CENTER POST TO BEAM DETAIL

SCALE: N.T.S.



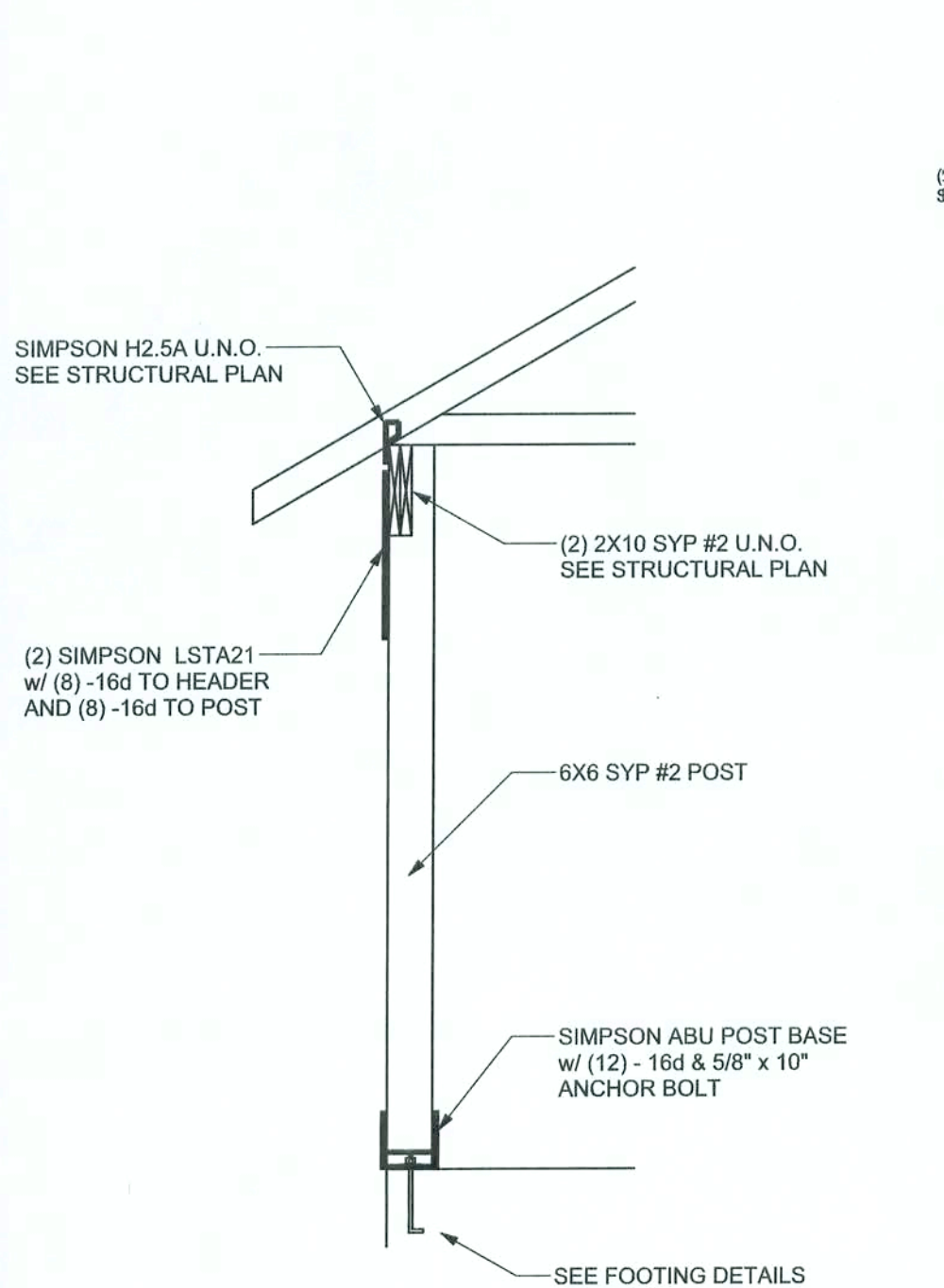
# ONE STORY WALL SECTION

SCALE: 3/4" = 1'-0"

# EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.2(8). EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13'-6" O.C.



# TYPICAL PORCH POST DETAIL

SCALE: 1/2" = 1'-0"

WINDLOAD ENGINEER: Mark Disoway, PE No. 53815, POB 886, Lake City, FL 33506, 386-754-5419

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code 2004 residential, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY  
P.E. 53815

SEAL

Podlaszewski  
Residence

13 ADDRESS:  
Lot 14 Phase 2 Stonechase S/D  
Columbia County, Florida

Mark Disoway P.E.  
P.O. Box 868  
Lake City, Florida 32056  
Phone: (386) 754 - 5419  
Fax: (386) 269 - 4871  
windloadengineer@bellsouth.net

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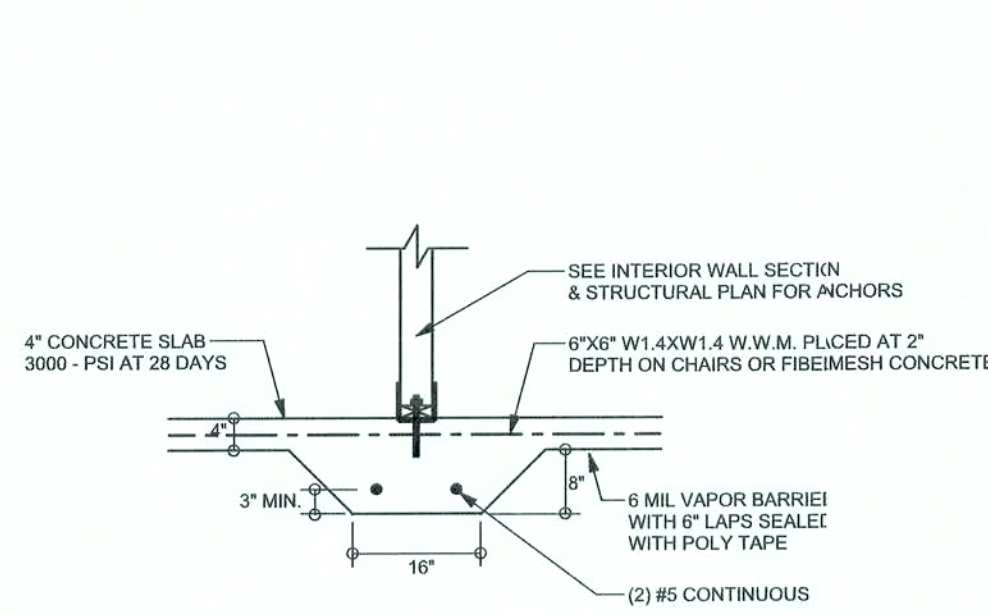
S-1

OF 3 SHEETS

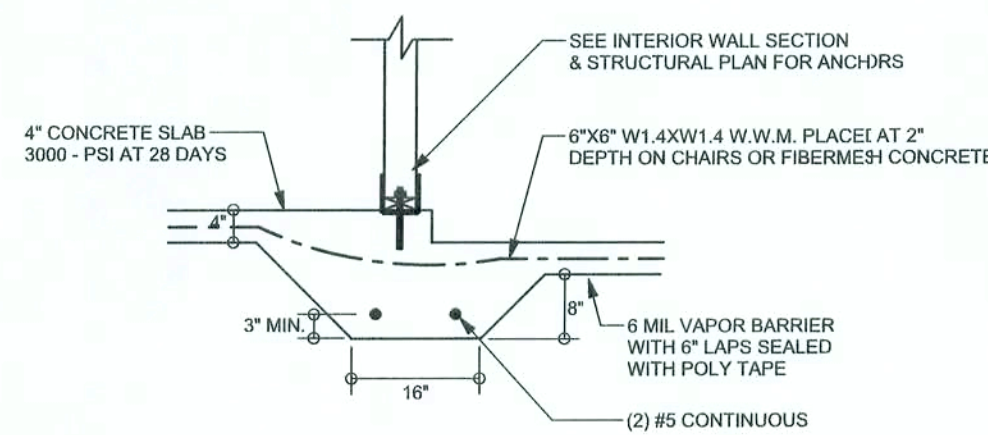


REVISIONS	

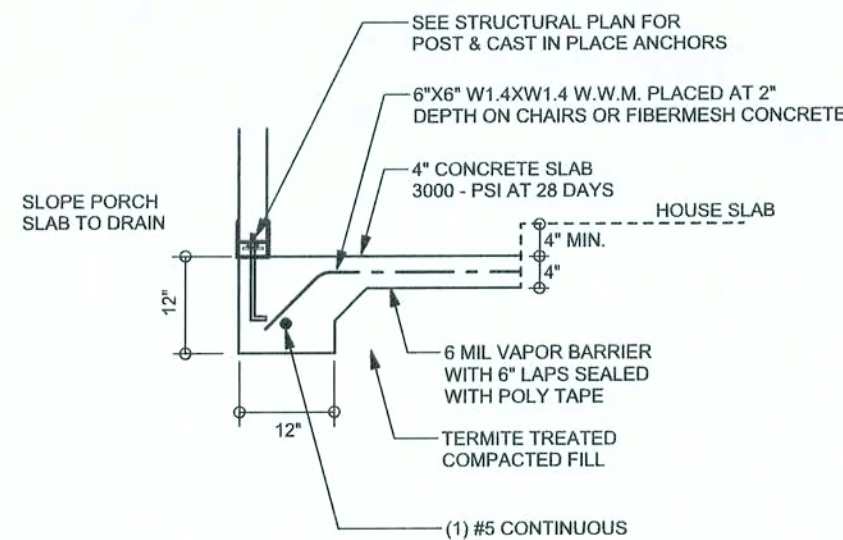
SOFTPLAN  
ARCHITECTURAL DESIGN SOFTWARE



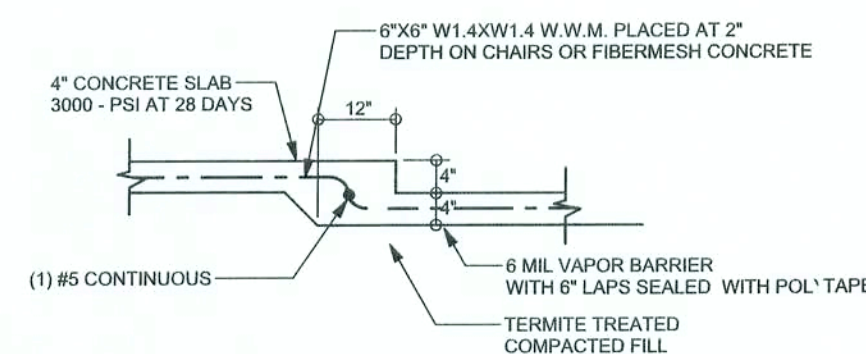
**F2**  
**S-2** INTERIOR BEARING FOOTING  
SCALE: 1/2" = 1'-0"



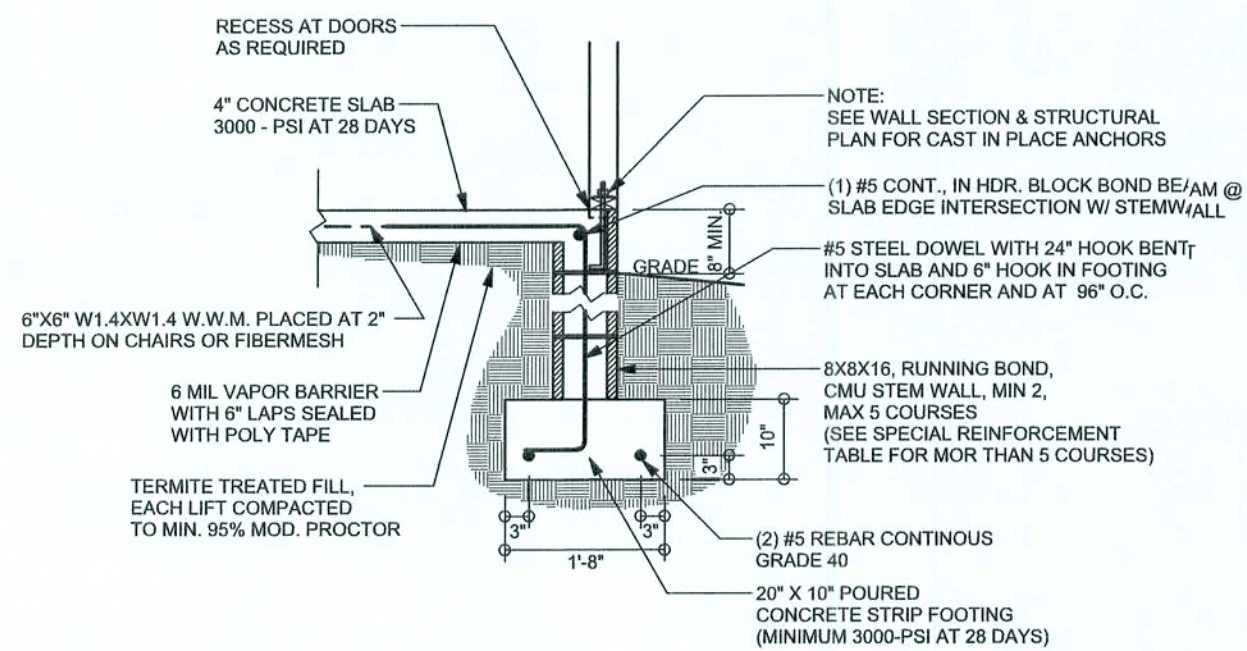
**F3**  
**S-2** INTERIOR BEARING STEP FOOTING  
SCALE: 1/2" = 1'-0"



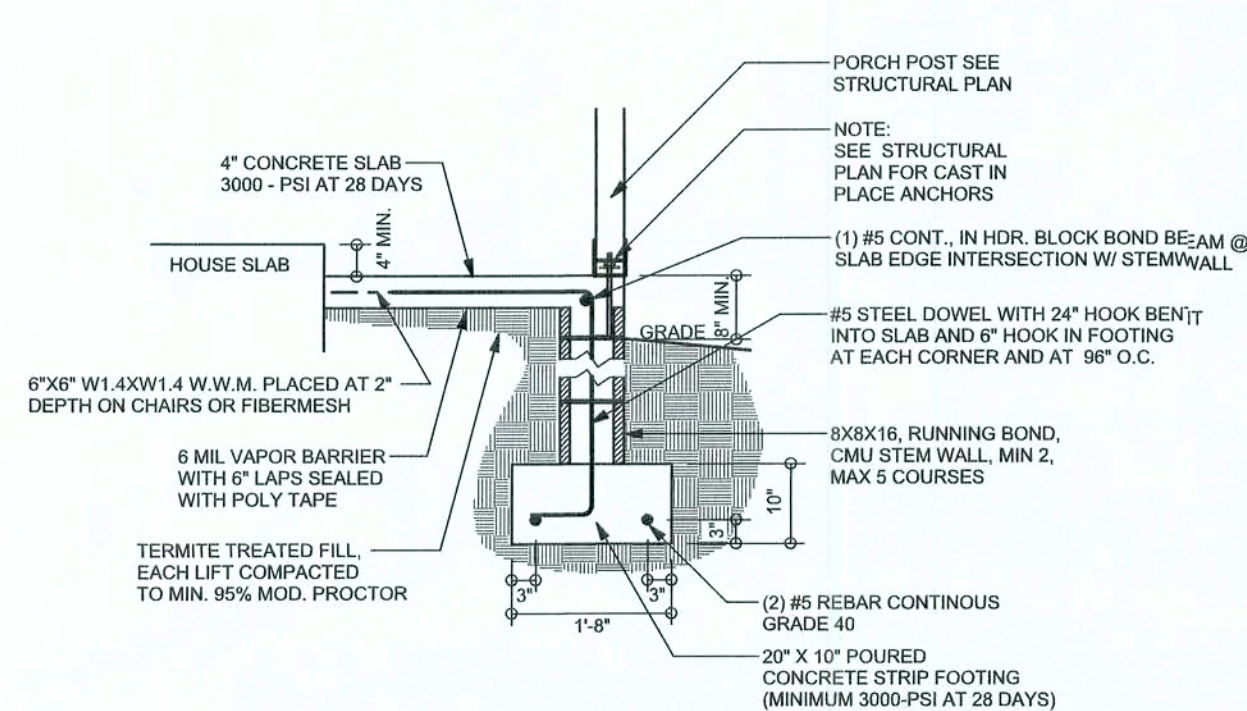
**F5**  
**S-2** PORCH FOOTING  
SCALE: 1/2" = 1'-0"



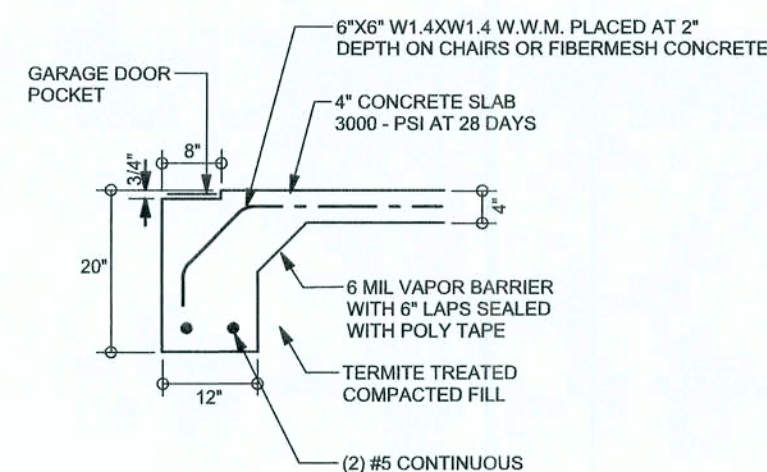
**F6**  
**S-2** TYPICAL NON-BEARING STEP FOOTING  
SCALE: 1/2" = 1'-0"



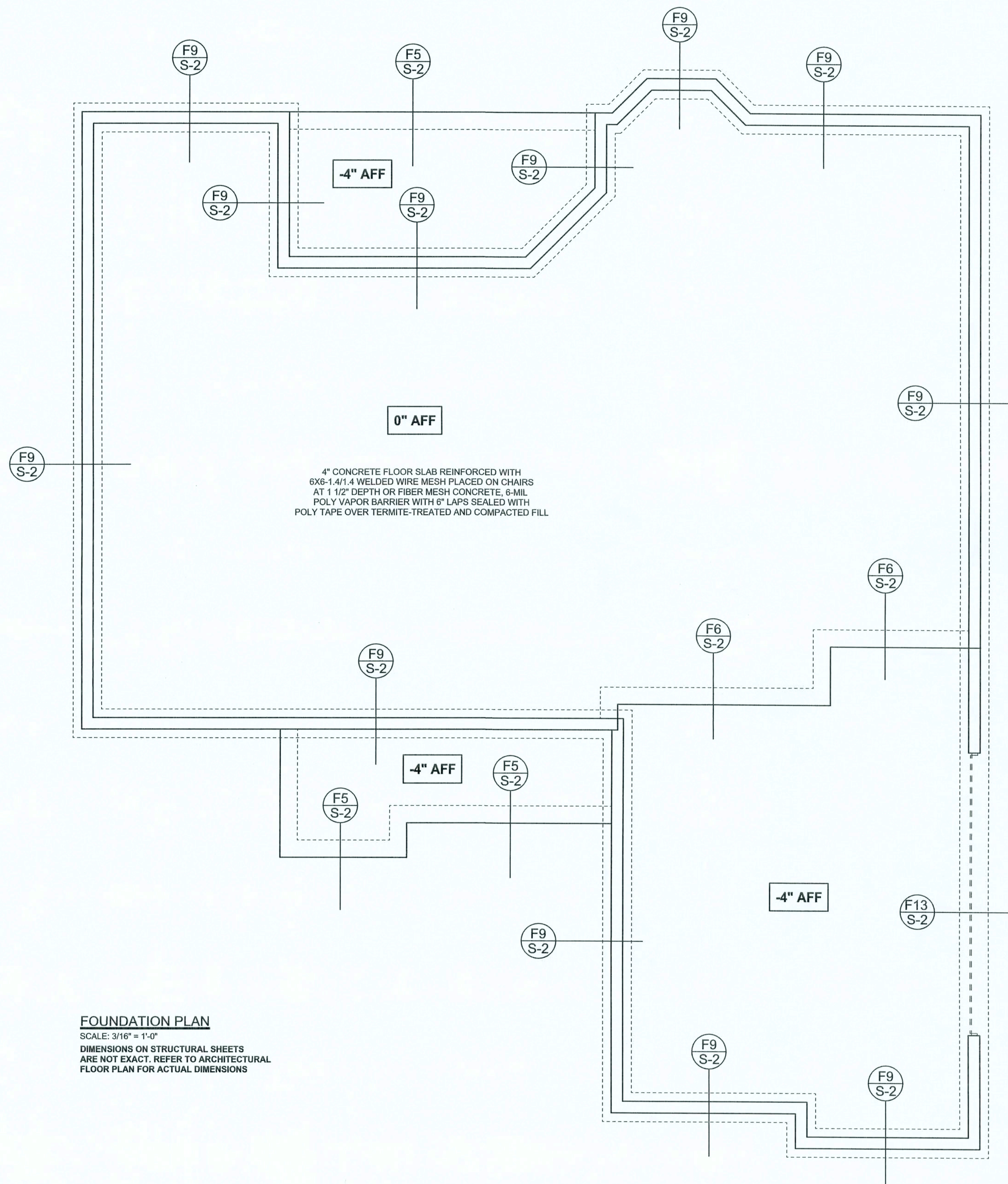
**F9**  
**S-2** STEM WALL FOOTING  
SCALE: 1/2" = 1'-0"



**F12**  
**S-2** ALT. STEM WALL PORCH FOOTING  
SCALE: 1/2" = 1'-0"



**F13**  
**S-2** ALT. STEM WALL GARAGE DOOR FOOTING  
SCALE: 1/2" = 1'-0"



**FOUNDATION PLAN**  
SCALE: 3/16" = 1'-0"  
DIMENSIONS ON STRUCTURAL SHEETS  
ARE NOT EXACT. REFER TO ARCHITECTURAL  
FLOOR PLAN FOR ACTUAL DIMENSIONS

WINDLOAD ENGINEER: Mark Disosway,  
P.E. No. 53915, P.O. Box 868, Lake City, FL  
32056, 386-754-5419

**DIMENSIONS:**  
Stated dimensions supersede scaled  
dimensions. Refer all questions to  
Mark Disosway, P.E. for resolution.  
Do not proceed without clarification.

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form or manner without first the express written  
permission and consent of Mark Disosway.

**CERTIFICATION:** I hereby certify that I have  
examined this plan, and that the applicable  
portions of the plan, relating to wind engineering,  
comply with section R301.2.1, Florida building  
code 2004 residential, to the best of my  
knowledge.

**LIMITATION:** This design is valid for one  
building, at specified location.

MARK DISOSWAY  
P.E. 53915

26 DEC 05

SEAL

Podlaszewski  
Residence

ADDRESS:  
Lot 14 Phase 2 Stonehenge S/D  
Columbia County, Florida

Mark Disosway P.E.  
P.O. Box 868  
Lake City, Florida 32056  
Phone: (386) 754 - 5419  
Fax: (386) 269 - 4871  
windloadengineer@bellsouth.net

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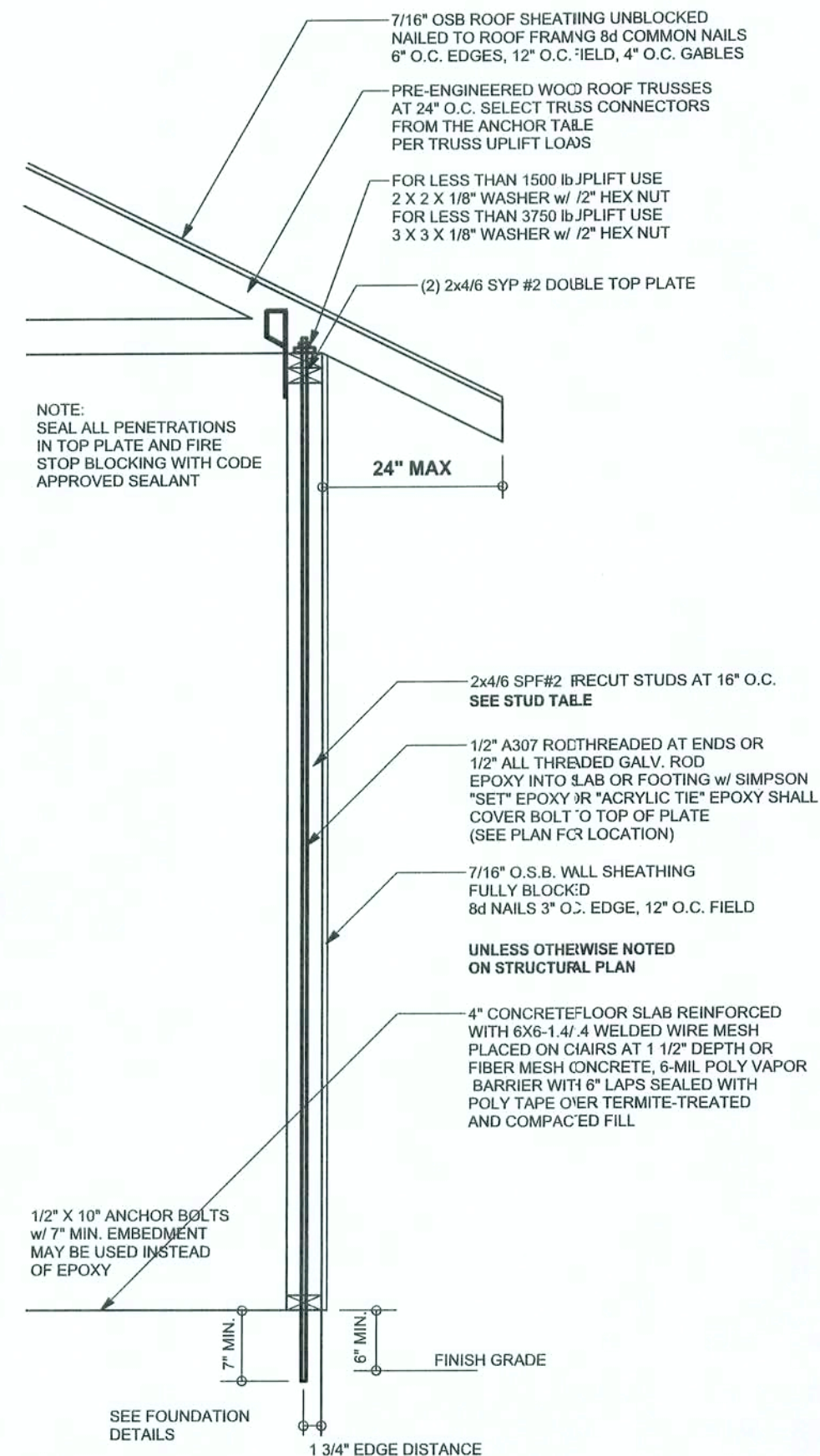
**S-2**

OF 3 SHEETS

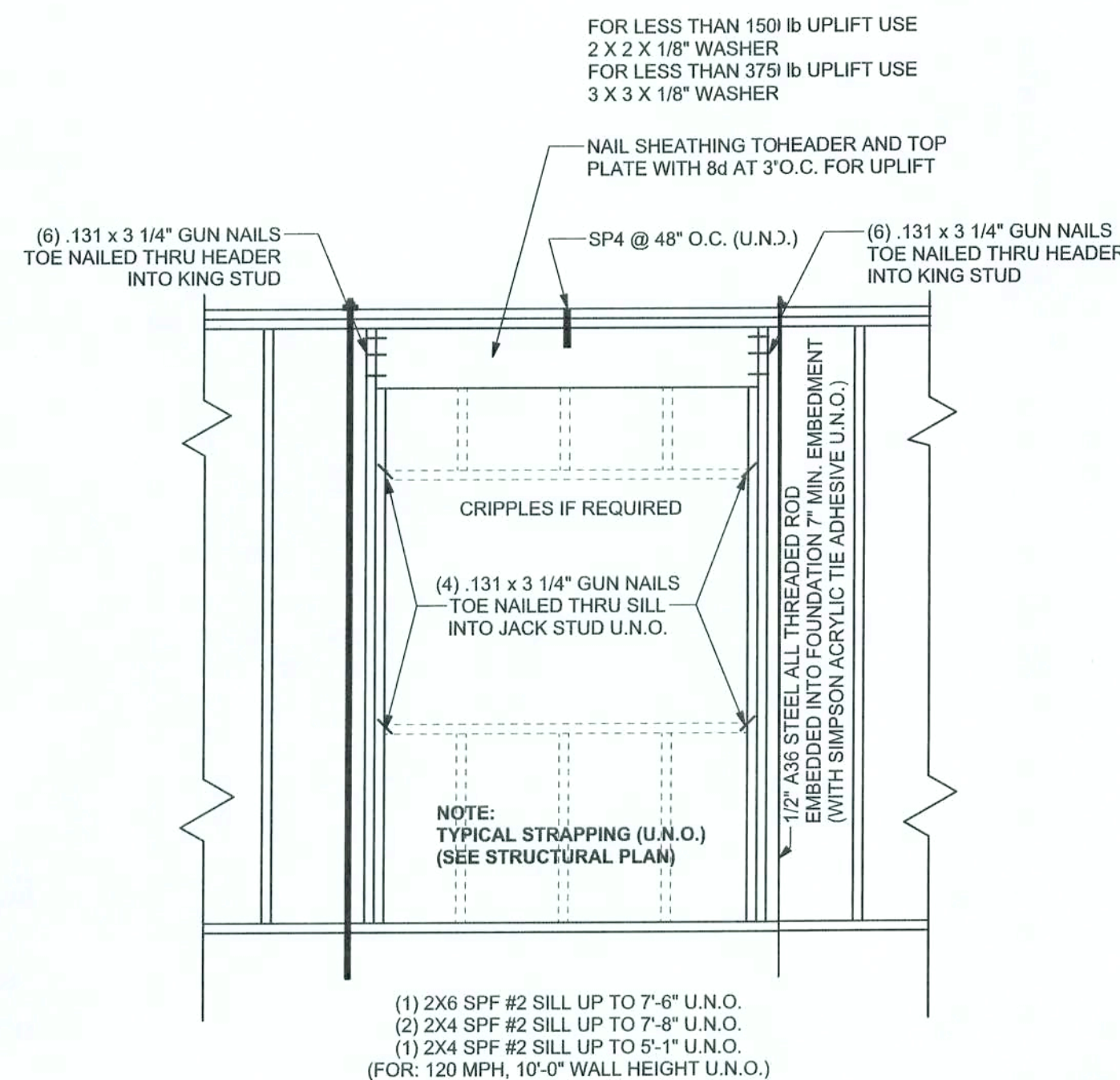


REVISIONS	

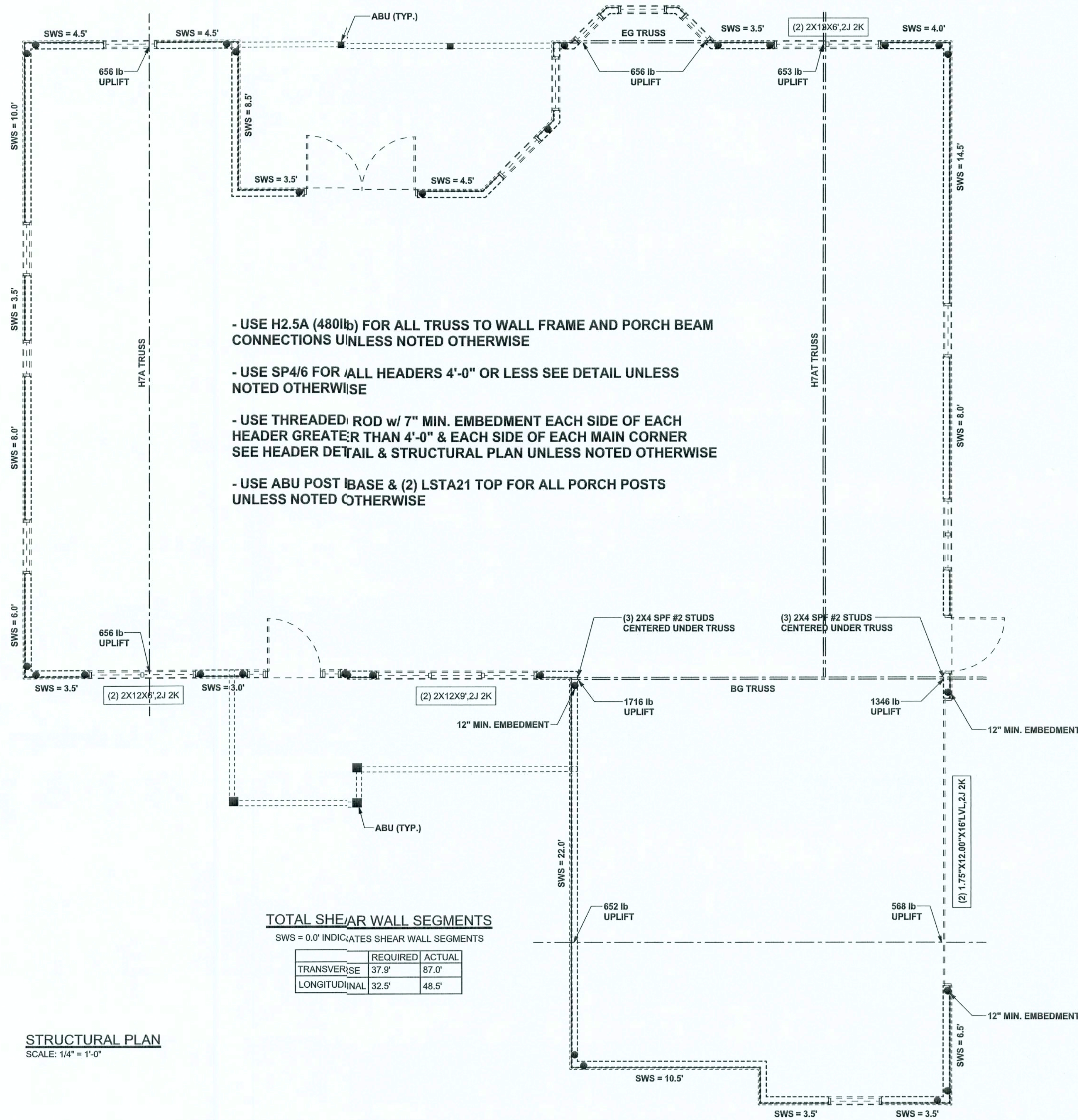
SOFTPLAN  
ARCHITECTURAL DESIGN SOFTWARE



ONE STORY WALL SECTION  
SCALE: 3/4\" = 1'-0"



TYPICAL HEADER STRAPING DETAIL RODS  
SCALE: 1/2\" = 1'-0"



#### TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	37.9'	87.0'
LONGITUDINAL	32.5'	48.5'

STRUCTURAL PLAN  
SCALE: 1/4\" = 1'-0"

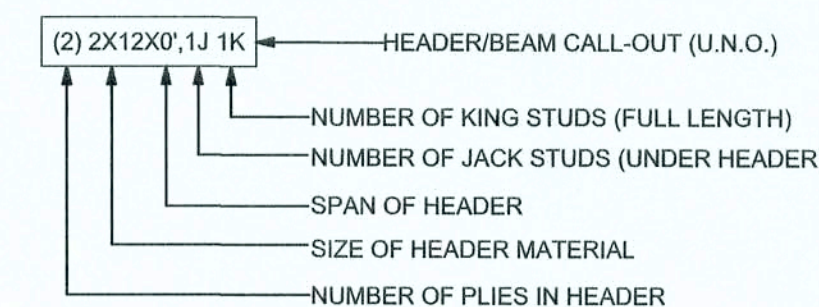
#### STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCS11-03, BCS11-01, BCS11-02, & BCS11-03. BCS11-01, BCS11-02, & BCS11-03 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

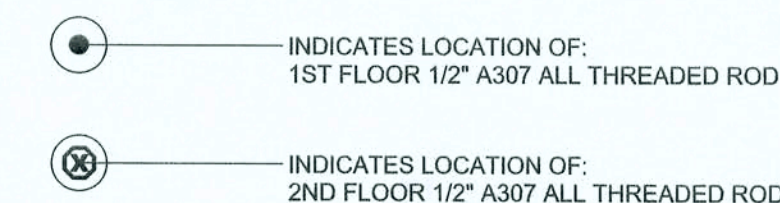
#### WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

#### HEADER LEGEND



#### THREADED ROD LEGEND



CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER, ANDERSON TRUSS CO. (JOB #5-552)

WINDLOAD ENGINEER: Mark Disoway, P.E. No.53915, POB 868, Lake City, FL 32056, 386-754-5419

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LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY  
P.E. 53915

*Mark Disoway*  
2/6/2005  
SEAL

Podlaszewski  
Residence

ADDRESS:  
Lot 14 Phase 2 Stonehenge S/D  
Columbia County, Florida

Mark Disoway P.E.  
P.O. Box 868  
Lake City, Florida 32056  
Phone: (386) 754 - 5419  
Fax: (386) 269 - 4871  
windloadengineer@bellsouth.net

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**S-3**  
OF 3 SHEETS