DATE 04/05/2007

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT
000025700

APPLICANT GREG TALLEY PHONE 352.214.1055

ADDRESS 1221 NW 238 TERRACE HIGH SPRINGS FL 32643

OWNER GREG TALLEY PHONE 352.214.1055

ADDRESS 161 SW ARROWBEND DRIVE LAKE CITY FL 32024

CONTRACTOR G&J BUILDERS, INC.(G. ROHNER) PHONE 352.214.1055

LOCATION OF PROPERTY 47-S TO C-242,TR TO ARROWHEAD ROAD,TR TO CANNON CREEK PLACE TO ARROWBEND DRIVE,TL AND THE LOT IS ON THE .

TYPE DEVELOPMENT SFD/UTILITY ESTIMATED COST OF CONSTRUCTION 83600.00

HEATED FLOOR AREA 1672.00 TOTAL AREA 2394.00 HEIGHT 18.00 STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 6'12 FLOOR CONC

LAND USE & ZONING RSF-2 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 0 FLOOD ZONE XPP DEVELOPMENT PERMIT NO.

PARCEL ID 24-4S-16-03114-117 SUBDIVISION CANNON CREEK PLACE

LOT 17 BLOCK PHASE UNIT TOTAL ACRES 0.50

000001362		CGC021619	Signature on file	
Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor	
18"X32'MITERED	07-235	BLK	JTH	N
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: PLAT REQUIRES MFE @ 97.5'. FINISH FLOOR CONFIRMATION LETTER REQUIRED. NOC ON FILE.

Check # or Cash CASH REC'D.

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power date/app. by Foundation date/app. by Monolithic date/app. by

Under slab rough-in plumbing date/app. by Slab date/app. by Sheathing/Nailing date/app. by

Framing date/app. by Rough-in plumbing above slab and below wood floor date/app. by

Electrical rough-in date/app. by Heat & Air Duct date/app. by Peri. beam (Lintel) date/app. by

Permanent power date/app. by C.O. Final date/app. by Culvert date/app. by

M/H tie downs, blocking, electricity and plumbing date/app. by Pool date/app. by

Reconnection date/app. by Pump pole date/app. by Utility Pole date/app. by

M/H Pole date/app. by Travel Trailer date/app. by Re-roof date/app. by

BUILDING PERMIT FEE $ 420.00 CERTIFICATION FEE $ 11.97 SURCHARGE FEE $ 11.97

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ 25.00 **TOTAL FEE** 543.94

INSPECTORS OFFICE CLERKS OFFICE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

ELK Roofing Products Specifications – Tuscaloosa, AL

PRESTIQUE® HIGH DEFINITION®

RAISED PROFILE®

Prestique Plus *High Definition* and Prestique Gallery Collection™

Product size	13¼"x 39⅜"
Exposure	5⅝"
Pieces/Bundle	16
Bundles/Square	4/98.5 sq.ft.
Squares/Pallet	11

50-year limited warranty period: 5-7**years non-prorated coverage for shingles and application labor with prorated coverage for remainder of limited warranty period, plus an option for transferability*. 5-year limited wind warranty*. Wind Coverage: standard 80 mph, extended 110 mph***

Prestique I *High Definition*

Product size	13¼"x 39⅜"
Exposure	5⅝"
Pieces/Bundle	16
Bundles/Square	4/98.5 sq.ft.
Squares/Pallet	14

40-year limited warranty period: 5-7**years non-prorated coverage for shingles and application labor with prorated coverage for remainder of limited warranty period, plus an option for transferability*. 5-year limited wind warranty*. Wind Coverage: standard 80 mph, extended 90 mph***

Prestique *High Definition*

Product size	13¼"x 38¾"
Exposure	5⅝"
Pieces/Bundle	22
Bundles/Square	3/100 sq.ft.
Squares/Pallet	16

30-year limited warranty period: 5-7**years non-prorated coverage for shingles and application labor with prorated coverage for remainder of limited warranty period, plus an option for transferability*. 5-year limited wind warranty*. Wind Coverage: standard 80 mph.

Raised Profile

Product size	13¼"x 38¾"
Exposure	5⅝"
Pieces/Bundle	22
Bundles/Square	3/100 sq.ft.
Squares/Pallet	16

30-year limited warranty period: 5-7**years non-prorated coverage for shingles and application labor with prorated coverage for remainder of limited warranty period, plus an option for transferability*. 5-year limited wind warranty*. Wind Coverage: standard 70 mph.

HIP AND RIDGE SHINGLES

Seal-A-Ridge® w/*FLX*™

Size: 12"x 12"
Exposure: 6½"
Pieces/Bundle: 45
Coverage: 4 Bundles = 100 linear feet

Vented RidgeCrest™ w/*FLX*™

Size: 13"x13¼"
Exposure: 9¼"
Pieces/Box: 26
Coverage: 5 boxes = 100 linear feet

Elk Starter Strip

52 Bundles/Pallet
18 Pallets/Truck
936 Bundles/Truck
19 Pieces/Bundle
1 Bundle = 120.33 linear feet

Available Colors (Check Availability): Antique Slate, Weatheredwood, Shakewood, Sablewood, Hickory, Barkwood, Forest Green, Wedgewood, Birchwood, Sandalwood.
Gallery Collection: Balsam Forest™, Weathered Sage™, Sienna Sunset™.

All Prestique, Raised Profile and Seal-A-Ridge, and Prestique Starter Strip roofing products contain sealant which activates with the sun's heat, bonding shingles into a wind and weather resistant cover that resists blow-offs and leaks.

Check for availability with built-in StainGuard® treatment to inhibit the discoloration of roofing granules caused by the growth of certain types of algae.

All Prestique and Raised Profile shingles meet UL® Wind Resistant (UL 997) and Class "A" Fire Ratings (UL 790); and ASTM Specifications D 3018, Type-I; D 3161, Type-I; E 108 and the requirements of ASTM D 3462.

All Prestique and Raised Profile shingles have approval from the Florida Building Code Commission, Metro-Dade County, ICBO, and Texas Department of Insurance.

***See actual limited warranty for conditions and limitations.**
**** Effective January 1, 2004, the seven year non-prorated Umbrella Coverage Period applies only when a full Elk Roof System is installed with the original installation of the Elk shingles, all in accordance with Elk's application instructions for such products. A full Elk roof system includes Elk Hip and Ridge shingles on all hips and ridges, Elk Starter Strip along all rake and eave edges, an Elk ventilation system, and Elk All-Climate Self-Adhering Underlayment in all valleys. Additionally, Elk All-Climate Self-Adhering Underlayment is required along the rake and eave edges of the roof in and north of the states of VA, KY, MO, KS, CO, UT, NV, & OR.**
*****For a limited Wind Warranty up to 110 mph for Prestique Gallery Collection, Prestique Plus, or 90 mph for Prestique I or Grandé, at least six (6) properly placed NAILS and Elk Starter Strip shingles are required. See application instructions printed on the shingle wrapper for additional requirements.**

Specifications

Scope: Work includes furnishing all labor, materials and equipment necessary to complete installation of (name) shingles specified herein. Color shall be (name of color). Hip and ridge type to be Elk Seal-A-Ridge with formula *FLX*.

All exposed metal surfaces (flashing, vents, etc.) to be painted with matching Elk roof accessory paint.

Preparation of Roof Deck: Roof deck to be dry, well-seasoned 1" x 6" (25.4mm x 152.4mm) boards; exterior-grade plywood (exposure 1 rated sheathing) at least 3/8" (9.525mm) thick conforming to the specifications of the American Plywood Association; 7/16" (11.074mm) oriented strandboard; or chipboard. Most fire retardant plywood decks are NOT approved substrates for Elk shingles. Consult Elk Field Service for application specifications over other decks and other slopes.

Materials: Underlayment for standard roof slopes, 4" per foot (101.6/304.8mm) or greater: apply non-perforated No. 15 or 30 asphalt-saturated felt underlayment. For Low slopes[4" per foot (101.6/304.8mm) to a minimum of 2" per foot (50.8/304.8mm)], use two plies of underlayment overlapped a minimum of 19". Fasteners shall be of sufficient length and holding power for securing material as required by the application instructions printed on shingle wrapper.

For areas where algae is a problem, shingles shall be (name) with StainGuard treatment, as manufactured by the Elk Tuscaloosa plant. Hip and ridge type to be Seal-A-Ridge with formula *FLX* with StainGuard treatment.

Complete application instructions are published by Elk and printed on the back of every shingle bundle. All warranties are contingent upon the correct installation as shown on the instructions. These instructions are the minimum required to meet Elk application requirements. In some areas, building codes may require additional application techniques or methods beyond our instructions. In these cases, the local code must be followed. Under no circumstances will Elk accept application requirements less than those contained in its application instructions.

For specifications in CSI format, call 800.354.SPEC (7732) or e-mail specinfo@elkcorp.com.

Southeast & Atlantic Office: 800.945.5551

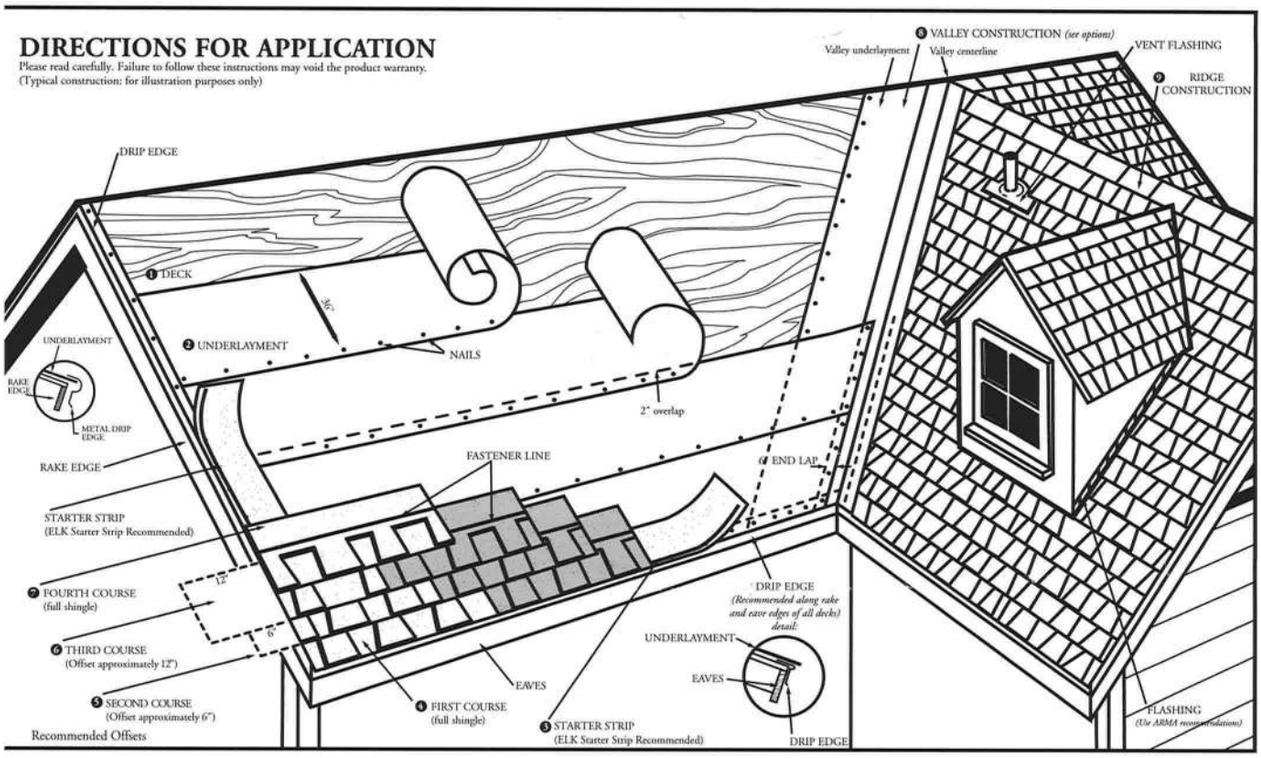
Corporate Headquarters: 800.354.7732

Plant Location: 800.945.5545

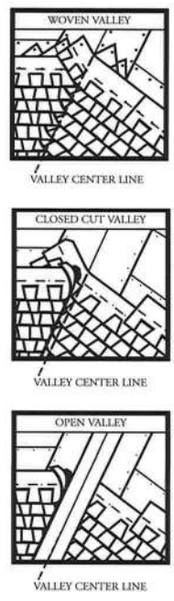
ELK The Premium Choice®
www.elkcorp.com

DIRECTIONS FOR APPLICATION

Please read carefully. Failure to follow these instructions may void the product warranty.
(Typical construction: for illustration purposes only)



❽ VALLEY CONSTRUCTION OPTION
(California Open and California Closed are also acceptable valleys.)



NOTE: For complete ARMA valley installation details, see ARMA roofing installation guide.

DIRECTIONS FOR APPLICATION

These application instructions are the minimum required to meet Elk's application requirements. Your failure to follow these instructions may void the product warranty. In some areas, the building codes may require additional application techniques or methods beyond our instructions. In these cases, the local code must be followed. Under no circumstances will Elk accept application requirements that are less than those printed here. Shingles should not be jammed tightly together. All attics should be properly ventilated. Note: It is not necessary to remove tape on back of shingle.

❶ DECK PREPARATION

Roof decks should be dry, well-seasoned 1" x 6" boards or exterior grade plywood minimum 3/8" thick and conform to the specifications of the American Plywood Association or 7/16" oriented strandboard, or 7/16" chipboard.

❷ UNDERLAYMENT

Apply underlayment (Non-Perforated No. 15 or 30 asphalt saturated felt). Elk Versashield® or self adhering underlayment is also acceptable. Cover drip edge at eaves only.

For low slope(2/12 up to 4/12), completely cover the deck with two plies of underlayment overlapping a minimum of 19". Begin by fastening a 19" wide strip of underlayment placed along the eaves. Place a full 36" wide sheet over the starter, horizontally placed along the eaves and completely overlapping the starter strip.

EAVE FLASHING FOR ICE DAMS (ASK A ROOFING CONTRACTOR, REFER TO ARMA MANUAL OR CHECK LOCAL CODES)

For standard slope (4/12 to less than 21/12), use coated roll roofing of no less than 50 pounds over the felt underlayment extending from the eave edge to a point at least 24" beyond the inside wall of the living space below or one layer of a self-adhered eave and flashing membrane.

For low slope (2/12 up to 4/12), use a continuous layer of asphalt plastic cement between the two plies of underlayment from the eave edge up roof to a point at least 24" beyond the inside wall of the living space below or one layer of a self-adhered eave and flashing membrane.

Consult the Elk Technical Services Department for application specifications over other decks and other slopes.

❸ STARTER SHINGLE COURSE

USE AN ELK STARTER STRIP OR THE HEADLAP OF A STRIP SHINGLE WITH THE ADHESIVE STRIP POSITIONED AT THE EAVE EDGE. With at least 3" trimmed from the end of the first shingle, start at the rake edge overhanging the eave and rake edges 1/2" to 3/4". Fasten 2" from the lower edge and 1" from each side.

❹ FIRST COURSE

Start at rake and continue course with full shingles laid flush with the starter course. Shingles may be applied with a course alignment of 45° on the roof

❺ SECOND COURSE

Offset the second course of shingles with respect to the first by approximately 6". Other offsets are approved if greater than 4".

❻ THIRD COURSE

Offset the next course by 6" with respect to the second course, or consistent with the original offset.

❼ FOURTH COURSE

Start at the rake and continue with full shingles across roof.

FIFTH AND SUCCEEDING COURSES.

Repeat application as shown for second, third, and fourth courses. Do not rack shingles straight up the roof. Offsets may be adjusted around valleys and penetrations.

❽ VALLEY CONSTRUCTION

Open, woven and closed cut valleys are acceptable when applied by Asphalt Roofing Manufacturing Association (ARMA) recommended procedures. For metal valleys, use 36" wide vertical underlayment prior to applying metal flashing (secure edge with nails). No nails are to be within 6" of valley center.

❾ RIDGE CONSTRUCTION

For ridge construction Elk recommends Class "A" Z®Ridge or Seal-A-Ridge® with formula *FLX*™ or RidgeCrest™ with *FLX* (See ridge package for installation instructions). Vented RidgeCrest or 3-tab shingles are also approved.

FASTENERS

While nailing is the preferred method for Elk shingles, Elk will accept fastening methods according to the following instructions.

Using the fastener line as a reference, nail or staple the shingle in the double thickness common bond area. For shingles without a fastener line, nails or staples must be placed between and/or in the sealant dots.

NAILS: Corrosive resistant, 3/8" head, minimum 12-gauge roofing nails. Elk recommends 1-1/4" for new roofs and 1-1/2" for roof-overs. In cases where you are applying shingles to a roof that has an exposed overhang, for new roofs only, 3/4" ring shank nails are allowed to be used from the eave's edge to a point up the roof that is past the outside wall line. 1" ring shank nails allowed for re-roof.

STAPLES: Corrosive resistant, 16-gauge minimum, crown width minimum of 15/16". Note: An improperly adjusted staple gun can result in raised staples that can cause a fish-mouthed appearance and can prevent sealing.

Fasteners should be long enough to obtain 3/4" deck penetration or penetration through deck, whichever is less. This product meets the requirements of the IRC 2003 code when fastened with 4 nails.

MANSARD APPLICATIONS

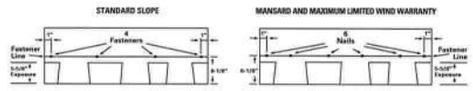
Correct fastening is critical to the performance of the roof. For slopes exceeding 60° (or 21/12) use six fasteners per shingle. Locate fasteners in the fastener area 1" from each side edge with the remaining four fasteners equally spaced along the length of the double thickness (laminated) area. Only fastening methods according to the above instructions are acceptable.

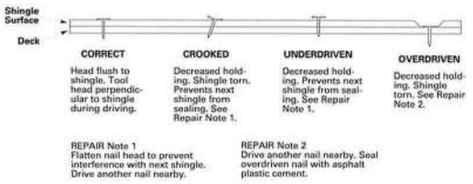
LIMITED WIND WARRANTY

- For a Limited Wind Warranty, all Prestique and Raised Profile™ shingles must be applied with 4 properly placed fasteners, or in the case of mansard applications, 6 properly placed fasteners per shingle.
- * For a Limited Wind Warranty up to 110 MPH for Prestique Gallery Collection or Prestique Plus or 90 MPH for Prestique I, shingles must be applied with 6 properly placed NAILS per shingle. SHINGLES APPLIED WITH STAPLES WILL NOT QUALIFY FOR THIS ENHANCED LIMITED WIND WARRANTY. Also, Elk Starter Strip shingles must be applied at the eaves and rake edges to qualify Prestique Plus, Prestique Gallery Collection and Prestique I shingles for this enhanced Limited Wind Warranty. Under no circumstances should the Elk Shingles or the Elk Starter Strip overhang the eaves or rake edge more than 3/4 of an inch.

HELP STOP BLOW-OFFS AND CALL-BACKS

A minimum of four fasteners must be driven into the DOUBLE THICKNESS (laminated) area of the shingle. Nails or staples must be placed along – and through – the "fastener line" or on products without fastener lines, nail or staple between and in line with sealant dots. CAUTION: Do not use fastener line for shingle alignment.





Refer to local codes which in some areas may require specific application techniques beyond those Elk has specified.
All Prestique and Raised Profile shingles have a U.L.® Wind Resistance Rating when applied in accordance with these instructions using nails or staples on re-roofs as well as new construction.

CAUTION TO WHOLESALER: Careless and improper storage or handling can harm fiberglass shingles. Keep these shingles completely covered, dry, reasonably cool, and protected from the weather. Do not store near various sources of heat. Do not store in direct sunlight until applied. DO NOT DOUBLE STACK. Systematically rotate all stock so that the material that has been stored the longest will be the first to be moved out.

ELK
The Premium Choice®
www.elkcorp.com

L01 # 11

Columbia County Building Permit Application

For Office Use Only Application # 0702-39 Date Received 2/15 By JW Permit # 362/25700

Application Approved by - Zoning Official BLK Date 16.02.07 Plans Examiner OK JTH Date 2-22-07

Flood Zone X per plat Development Permit N/A Zoning RSF-2 Land Use Plan Map Category RES. Low Den.

Comments LETTER of Auth. on file. Plat Requires MFE at 97.5' Finish Floor

(NOC) & EN. HEALTH — Confirmation Letter Required

Applicants Name GREG TALLEY Phone 352-214-1055

Address 21221 NW 238 AVENUE HIGH SPRINGS, FL 32643

Owners Name GREG TALLEY Phone 352-214-1055

911 Address 161 SW ARROWBEND DR. LAKE CITY FL 32024

Contractors Name G&J BLDRS INC Phone

Address 21221 N.W 238 AVE High Spring

Fee Simple Owner Name & Address —

Bonding Co. Name & Address —

Architect/Engineer Name & Address NICHOLAS GEISLER, ARCHITECT

Mortgage Lenders Name & Address N/A

Circle the correct power company – FL Power & Light – (Clay Elec.) – Suwannee Valley Elec. – Progressive Energy

Property ID Number 24-4S-16-03114-117 Estimated Cost of Construction 110,000

Subdivision Name CANNON CREEK PLACE Lot 17 Block ____ Unit ____ Phase ____

Driving Directions 47-S TO C-242-TR TO ARROWHEAD Rd. TR TO CANNON CREEK PLACE. TO ARROWBEND DR, TL Lot is on the R.

Type of Construction SFD - FRAMED Number of Existing Dwellings on Property 0

Total Acreage .50 Lot Size ____ Do you need a (Culvert Permit) or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 35' Side 80' Side 40' Rear 126'

Total Building Height 18' Number of Stories 1 Heated Floor Area 1672 TOTAL 2394 Roof Pitch 6/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner Builder or Agent (Including Contractor)

Contractor Signature [signature]

Contractors License Number CGC021619

Competency Card Number

NOTARY STAMP/SEAL

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me

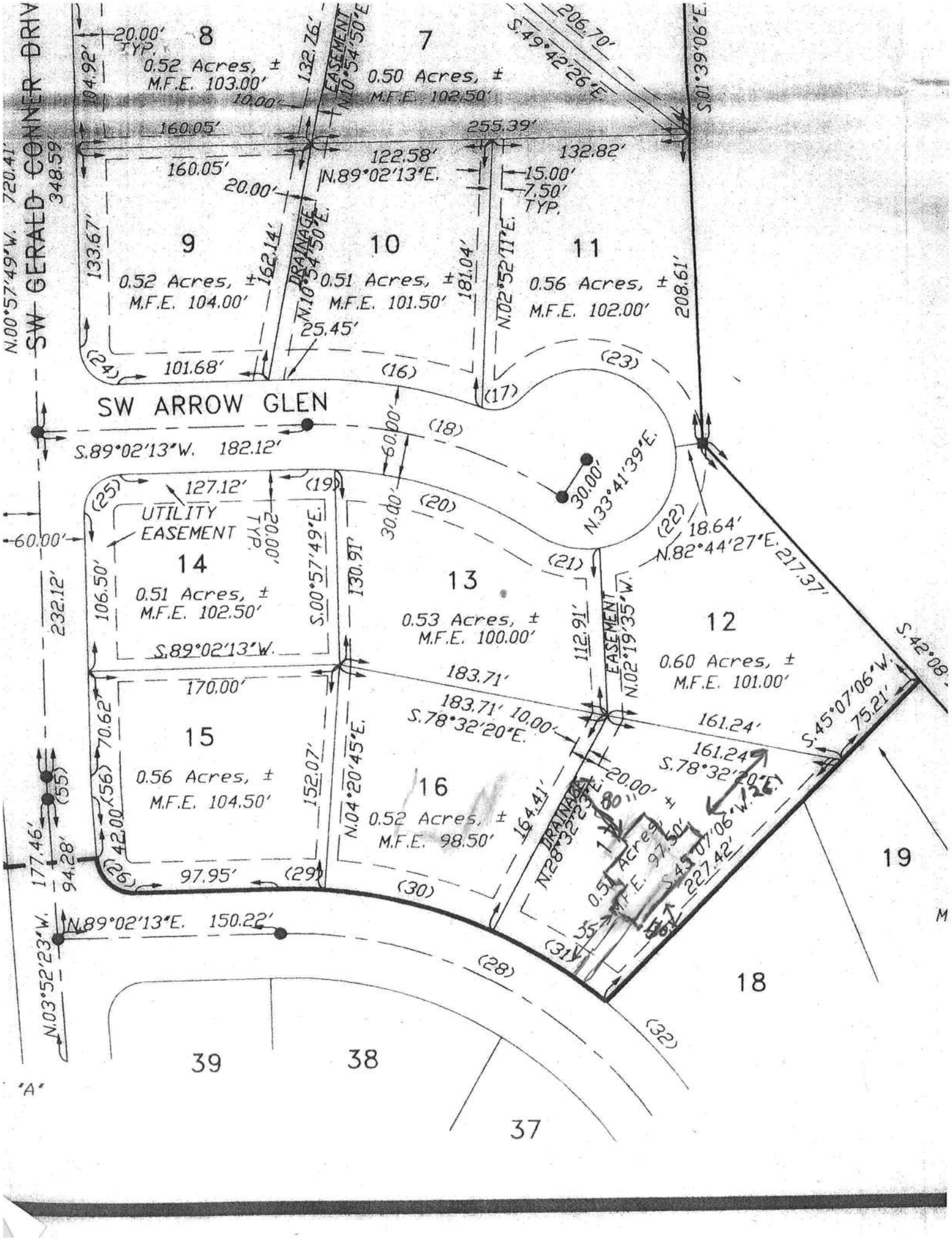
this 9 day of OCTOBER 2006.

Personally known ✓ or Produced Identification ____

Notary Signature [signature]

ROBERTA SABELLA
MY COMMISSION # DD267351
EXPIRES: January 13, 2008
1-800-3-NOTARY FL Notary Discount Assoc. Co.

Talked with Nelson for Greg 2-22-07


SW GERALD CONNER DRIVE
SW ARROW GLEN
8
0.52 Acres, ±
M.F.E. 103.00'
7
0.50 Acres, ±
M.F.E. 102.50'
9
0.52 Acres, ±
M.F.E. 104.00'
10
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M.F.E. 101.50'
11
0.56 Acres, ±
M.F.E. 102.00'
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M.F.E. 101.00'
15
0.56 Acres, ±
M.F.E. 104.50'
16
0.52 Acres, ±
M.F.E. 98.50'
0.51 Acres, ±
M.F.E. 97.50'
UTILITY EASEMENT
DRAINAGE EASEMENT
DRAINAGE EASEMENT
19
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39
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37
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Public Services

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File a Complaint
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Renew a License
Change License Status
Maintain Account
Change My Address
View Messages
Change My PIN
View Continuing Ed

Term Glossary

Online Help

03:20:22 PI

Search Results

Please see our glossary of terms for an explanation of the license status shown in these search results.

License Type	Name	Name Type	License Number/ Rank	Status/ Expires
Certified General Contractor	ROHNER, GEORGE JOSEPH	Primary	CGC021619 Cert General	Current, Active 08/31/2008

New S

| Terms of Use | | Privacy Statement |

@ CAM112M01 CamaUSA Appraisal System Columbia County
2/15/2007 13:58 **Legal Description Maintenance** **36000** **Land** **001**
Year T Property Sel **AG** **000**
2007 R 24-4S-16-03114-116 ... .. **Bldg** **000**
143 ARROWBEND DR SW **Xfea** **000**
TALLEY GREGORY S & **36000** **TOTAL** **B**

1 LOT 16 CANNON CREEK PLACE S/D. WD 1055-268. 2
3 4
5 6
7 8
9 10
11 12
13 14
15 16
17 18
19 20
21 22
23 24
25 26
27 28

Mnt **10/06/2005 CHUCK**

F1=Task F3=Exit F4=Prompt F10=GoTo PgUp/PgDn F24=More

NOTICE OF COMMENCEMENT FORM
COLUMBIA COUNTY, FLORIDA

THIS DOCUMENT MUST BE RECORDED AT THE COUNTY CLERKS OFFICE BEFORE YOUR FIRST INSPECTION

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and inaccordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.
IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Tax Parcel ID Number 24-4S-16-03114-117 **Permit Number** 25700

1. Description of property: (legal description of the property and street address or 911 address)
161 SW ARROWBEND DRIVE, LAKE CITY FL 32024

LOT 17 CANNON CREEK PLACE

2. General description of improvement: SINGLE FAMILY DWELLING

3. Owner Name & Address GREG TALLEY
21221 NW 238 AVENUE, High Springs, FL 32643 **Interest in Property** 100 %

4. Name & Address of Fee Simple Owner (if other than owner):

5. Contractor Name GEJ Buildings, Inc. **Phone Number** 386.
Address 21221 NW 238th Ave - High Springs FL 32643

6. Surety Holders Name **Phone Number**
Address
Amount of Bond

Inst:2007007815 Date:04/05/2007 Time:14:01
DC,P.DeWitt Cason,Columbia County B:1115 P:2152

7. Lender Name
Address

8. Persons within the State of Florida designated by the Owner upon whom notices or other documents may be served as provided by section 718.13 (1)(a) 7; Florida Statutes:
Name **Phone Number**
Address

9. In addition to himself/herself the owner designates _____ **of** _____ **to receive a copy of the Lien Notice as provided in Section 713.13 (1) –**
(a) 7. Phone Number of the designee

10. Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording, (Unless a different date is specified)

THE OWNER MUST SIGN THE NOTICE OF COMMENCEMENT AND NO ONE ELSE MAY BE PERMITTED TO SIGN IN HIS/HER STEAD.

Signature of Owner

Sworn to (or affirmed) and subscribed before day of 5th April, **20** 07.

Connie F. Scott **NOTARY STAMP/SEAL**
Signature of Notary

CONNIE F. SCOTT
MY COMMISSION # DD 559203
EXPIRES: June 1, 2010
Bonded Thru Notary Public Underwriters

Columbia County Building Department
Culvert Permit

Culvert Permit No.
000001362

DATE 04/05/2007 PARCEL ID # 24-4S-16-03114-117

APPLICANT GREG TALLEY PHONE 352.214.1055

ADDRESS 21221 NW 238 TERRACE HIGH SPRINGS FL 32643

OWNER GREG TALLEY PHONE 352.214.1055

ADDRESS 161 SW ARROWBEND DRIVE LAKE CITY FL 32024

CONTRACTOR G&J BUILDERS, INC. PHONE 352.

LOCATION OF PROPERTY 47-S TO C-242,TR TO ARROWHEAD ROAD,TR TO CANNON CREEK PLACE S.D. TO ARROWBEND DRIVE,TL AND THE LOT IS ON THE R.

SUBDIVISION/LOT/BLOCK/PHASE/UNIT CANNON CREEK PLACE 17

SIGNATURE Greg Talley

INSTALLATION REQUIREMENTS

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

[ ] Department of Transportation Permit installation approved standards.

[ ] Other _____

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00

COLUMBIA COUNTY GATEWAY TO FLORIDA

FORM 600A-2004 Tested sealed ducts must be certified in this house. EnergyGauge® 4.1

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**Greg Talley**	Builder:	**Greg Talley**
Address:	**Lot: 16, Sub: Cannon Creek, Plat:**	Permitting Office:	Columbia
City, State:	**Lake City, FL 32025-**	Permit Number:	25700
Owner:	**Rosewood Model**	Jurisdiction Number:	221000
Climate Zone:	**North**		

1. New construction or existing	New	
2. Single family or multi-family	Single family	
3. Number of units, if multi-family	1	
4. Number of Bedrooms	3	
5. Is this a worst case?	No	
6. Conditioned floor area (ft²)	1672 ft²	
7. Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)		
a. U-factor: (or Single or Double DEFAULT)	Description Area 7a (Sngle Default) 253.7 ft²	
b. SHGC: (or Clear or Tint DEFAULT)	7b. (Clear) 253.7 ft²	
8. Floor types		
a. Slab-On-Grade Edge Insulation	R=0.0, 180.0(p) ft	
b. N/A		
c. N/A		
9. Wall types		
a. Frame, Wood, Exterior	R=13.0, 974.3 ft²	
b. Frame, Wood, Adjacent	R=13.0, 172.0 ft²	
c. N/A		
d. N/A		
e. N/A		
10. Ceiling types		
a. Under Attic	R=30.0, 1750.0 ft²	
b. N/A		
c. N/A		
11. Ducts(Leak Free)		
a. Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 50.0 ft	
b. N/A		
12. Cooling systems		
a. Central Unit	Cap: 40.0 kBtu/hr SEER: 12.00	1300
b. N/A		
c. N/A		
13. Heating systems		
a. Electric Heat Pump	Cap: 40.0 kBtu/hr HSPF: 7.20	
b. N/A		
c. N/A		
14. Hot water systems		
a. Electric Resistance	Cap: 50.0 gallons EF: 0.90	
b. N/A		
c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	PT,	

Glass/Floor Area: 0.15

Total as-built points: 23668
Total base points: 24758

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: _____

DATE: 1-4-07

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 16, Sub: Cannon Creek, Plat: , Lake City, FL, 32025-** PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points
.18	1672.0	20.04	6031.2	Single, Clear	W	1.5	8.0	90.0	43.84	0.96	3780.1
				Single, Clear	W	13.5	8.0	30.0	43.84	0.43	561.4
				Single, Clear	SW	13.5	8.0	20.0	45.75	0.41	373.1
				Single, Clear	S	13.5	8.0	15.0	40.81	0.46	282.1
				Single, Clear	N	1.5	8.0	30.0	21.73	0.97	630.5
				Single, Clear	N	1.5	8.0	2.7	21.73	0.97	56.7
				Single, Clear	E	7.5	8.0	30.0	47.92	0.53	766.9
				Single, Clear	E	1.5	8.0	16.0	47.92	0.96	734.2
				Single, Clear	S	1.5	8.0	20.0	40.81	0.92	753.6
				As-Built Total:				**253.7**			**7938.6**
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points
Adjacent	172.0	0.70	120.4	Frame, Wood, Exterior			13.0	974.3	1.50		1461.4
Exterior	974.3	1.70	1656.3	Frame, Wood, Adjacent			13.0	172.0	0.60		103.2
Base Total:	**1146.3**		**1776.7**	**As-Built Total:**				**1146.3**			**1564.6**
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points
Adjacent	20.0	1.60	32.0	Exterior Insulated				20.0	4.10		82.0
Exterior	20.0	4.10	82.0	Adjacent Insulated				20.0	1.60		32.0
Base Total:	**40.0**		**114.0**	**As-Built Total:**				**40.0**			**114.0**
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points
Under Attic	1672.0	1.73	2892.6	Under Attic			30.0	1750.0	1.73 X 1.00		3027.5
Base Total:	**1672.0**		**2892.6**	**As-Built Total:**				**1750.0**			**3027.5**
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points
Slab	180.0(p)	-37.0	-6660.0	Slab-On-Grade Edge Insulation			0.0	180.0(p	-41.20		-7416.0
Raised	0.0	0.00	0.0								
Base Total:			**-6660.0**	**As-Built Total:**				**180.0**			**-7416.0**
INFILTRATION	Area X	BSPM =	Points					Area X	SPM =		Points
	1672.0	10.21	17071.1					1672.0	10.21		17071.1

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 16, Sub: Cannon Creek, Plat: , Lake City, FL, 32025-** PERMIT #:

BASE			AS-BUILT					
Summer Base Points: 21225.6			**Summer As-Built Points:**					**22299.9**
Total Summer Points	X System Multiplier	= Cooling Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points
			(sys 1: Central Unit 40000 btuh ,SEER/EFF(12.0) Ducts:Unc(S),Unc(R),Gar(AH),R6.0(INS)					
			22300	1.00	(1.09 x 1.000 x 1.00)	0.284	0.950	6567.6
21225.6	**0.4266**	**9054.9**	**22299.9**	**1.00**	**1.090**	**0.284**	**0.950**	**6567.6**

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 16, Sub: Cannon Creek, Plat: , Lake City, FL, 32025-** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	1672.0	12.74	3834.2	Single, Clear	W	1.5	8.0	90.0	28.84	1.01	2624.5	
				Single, Clear	W	13.5	8.0	30.0	28.84	1.21	1050.6	
				Single, Clear	SW	13.5	8.0	20.0	24.09	1.87	902.7	
				Single, Clear	S	13.5	8.0	15.0	20.24	3.42	1037.3	
				Single, Clear	N	1.5	8.0	30.0	33.22	1.00	997.5	
				Single, Clear	N	1.5	8.0	2.7	33.22	1.00	89.8	
				Single, Clear	E	7.5	8.0	30.0	26.41	1.27	1005.1	
				Single, Clear	E	1.5	8.0	16.0	26.41	1.02	430.9	
				Single, Clear	S	1.5	8.0	20.0	20.24	1.04	421.4	
				As-Built Total:				**253.7**			**8559.9**	
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Adjacent	172.0	3.60	619.2	Frame, Wood, Exterior			13.0	974.3	3.40		3312.6	
Exterior	974.3	3.70	3604.9	Frame, Wood, Adjacent			13.0	172.0	3.30		567.6	
Base Total:	**1146.3**		**4224.1**	**As-Built Total:**				**1146.3**			**3880.2**	
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =		Points	
Adjacent	20.0	8.00	160.0	Exterior Insulated				20.0	8.40		168.0	
Exterior	20.0	8.40	168.0	Adjacent Insulated				20.0	8.00		160.0	
Base Total:	**40.0**		**328.0**	**As-Built Total:**				**40.0**			**328.0**	
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X	WCM =	Points	
Under Attic	1672.0	2.05	3427.6	Under Attic			30.0	1750.0	2.05 X	1.00	3587.5	
Base Total:	**1672.0**		**3427.6**	**As-Built Total:**				**1750.0**			**3587.5**	
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Slab	180.0(p)	8.9	1602.0	Slab-On-Grade Edge Insulation			0.0	180.0(p	18.80		3384.0	
Raised	0.0	0.00	0.0									
Base Total:			**1602.0**	**As-Built Total:**				**180.0**			**3384.0**	
INFILTRATION	Area X	BWPM =	Points					Area X	WPM =		Points	
	1672.0	-0.59	-986.5					1672.0	-0.59		-986.5	

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 16, Sub: Cannon Creek, Plat: , Lake City, FL, 32025-** PERMIT #:

BASE			AS-BUILT						
Winter Base Points:		**12429.5**	**Winter As-Built Points:**						**18753.1**
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points	
			(sys 1: Electric Heat Pump 40000 btuh ,EFF(7.2) Ducts:Unc(S),Unc(R),Gar(AH),R6.0						
			18753.1	1.000	(1.069 x 1.000 x 1.00)	0.474	0.950	9019.8	
12429.5	**0.6274**	**7798.2**	**18753.1**	**1.00**	**1.069**	**0.474**	**0.950**	**9019.8**	

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 16, Sub: Cannon Creek, Plat: , Lake City, FL, 32025-** PERMIT #:

BASE			AS-BUILT						
WATER HEATING									
Number of Bedrooms	X Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X Tank Ratio	X Multiplier	X Credit Multiplier	= Total
3	2635.00	7905.0	50.0	0.90	3	1.00	2693.56	1.00	8080.7
			As-Built Total:						**8080.7**

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points +	Heating Points +	Hot Water Points =	Total Points	Cooling Points +	Heating Points +	Hot Water Points =	Total Points
9055	**7798**	**7905**	**24758**	**6568**	**9020**	**8081**	**23668**

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 16, Sub: Cannon Creek, Plat: , Lake City, FL, 32025-** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

Tested sealed ducts must be certified in this house.

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 84.2

The higher the score, the more efficient the home.

Rosewood Model, Lot: 16, Sub: Cannon Creek, Plat: , Lake City, FL, 32025-

1.	New construction or existing	New	
2.	Single family or multi-family	Single family	
3.	Number of units, if multi-family	1	
4.	Number of Bedrooms	3	
5.	Is this a worst case?	No	
6.	Conditioned floor area (ft²)	1672 ft²	
7.	Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)		
	a. U-factor: (or Single or Double DEFAULT)	7a. Description (Sngle Default)	Area 253.7 ft²
	b. SHGC: (or Clear or Tint DEFAULT)	7b. (Clear)	253.7 ft²
8.	Floor types		
	a. Slab-On-Grade Edge Insulation	R=0.0, 180.0(p) ft	
	b. N/A		
	c. N/A		
9.	Wall types		
	a. Frame, Wood, Exterior	R=13.0, 974.3 ft²	
	b. Frame, Wood, Adjacent	R=13.0, 172.0 ft²	
	c. N/A		
	d. N/A		
	e. N/A		
10.	Ceiling types		
	a. Under Attic	R=30.0, 1750.0 ft²	
	b. N/A		
	c. N/A		
11.	Ducts(Leak Free)		
	a. Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 50.0 ft	
	b. N/A		
12.	Cooling systems		
	a. Central Unit	Cap: 40.0 kBtu/hr	
		SEER: 12.00	
	b. N/A		
	c. N/A		
13.	Heating systems		
	a. Electric Heat Pump	Cap: 40.0 kBtu/hr	
		HSPF: 7.20	
	b. N/A		
	c. N/A		
14.	Hot water systems		
	a. Electric Resistance	Cap: 50.0 gallons	
		EF: 0.90	
	b. N/A		
	c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15.	HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	PT,	

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

Energy Code Compliance

Duct System Performance Report

Project Name:	Greg Talley	Builder:	Greg Talley
Address:		Permitting Office:	
City, State:	Lake City, FL 32025-	Permit Number:	
Owner:	Rosewood Model	Jurisdiction Number:	
Climate Zone:	North		

Total Duct System Leakage Test Results

CFM25 Total Duct Leakage Test Values			
Line	System	Duct Leakage Total	Duct Leakage to Outdoors
1	System1	_____ cfm25(tot)	_____ cfm25(out)
2	System2	_____ cfm25(tot)	_____ cfm25(out)
3	System3	_____ cfm25(tot)	_____ cfm25(out)
4	System4	_____ cfm25(tot)	_____ cfm25(out)
5	**Total House Duct System Leakage**	Sum lines 1-4 _____ Divide by _____ (Total Conditioned Floor Area) = _____ **(Q_n,tot)** ☐ **Receive credit if Q_n,tot≤ 0.03**	Sum lines 1-4 _____ Divide by _____ (Total Conditioned Floor Area) = _____ **(Q_n,out)** ☐ **Receive credit if Q_n,out≤ 0.03 AND Q_n,tot≤ 0.09**

I hereby certify that the above duct testing performance results demonstrate compliance with the Florida Energy Code requirements in accordance with Section 610.1.A.1, Florida Building Code, Building Volume, Chapter 13 for leak free duct system credit.

Signature: _____

Printed Name: _____

Florida Rater Certification #: _____

DATE: _____

Florida Building Code requires that testing to confirm leak free duct systems be performed by a Class 1 Florida Energy Gauge Certified Energy Rater. Certified Florida Class 1 raters can be found at: http://energygauge.com/search.htp

BUILDING OFFICIAL: _____

DATE: _____

STATE OF FLORIDA
DEPARTMENT OF HEALTH

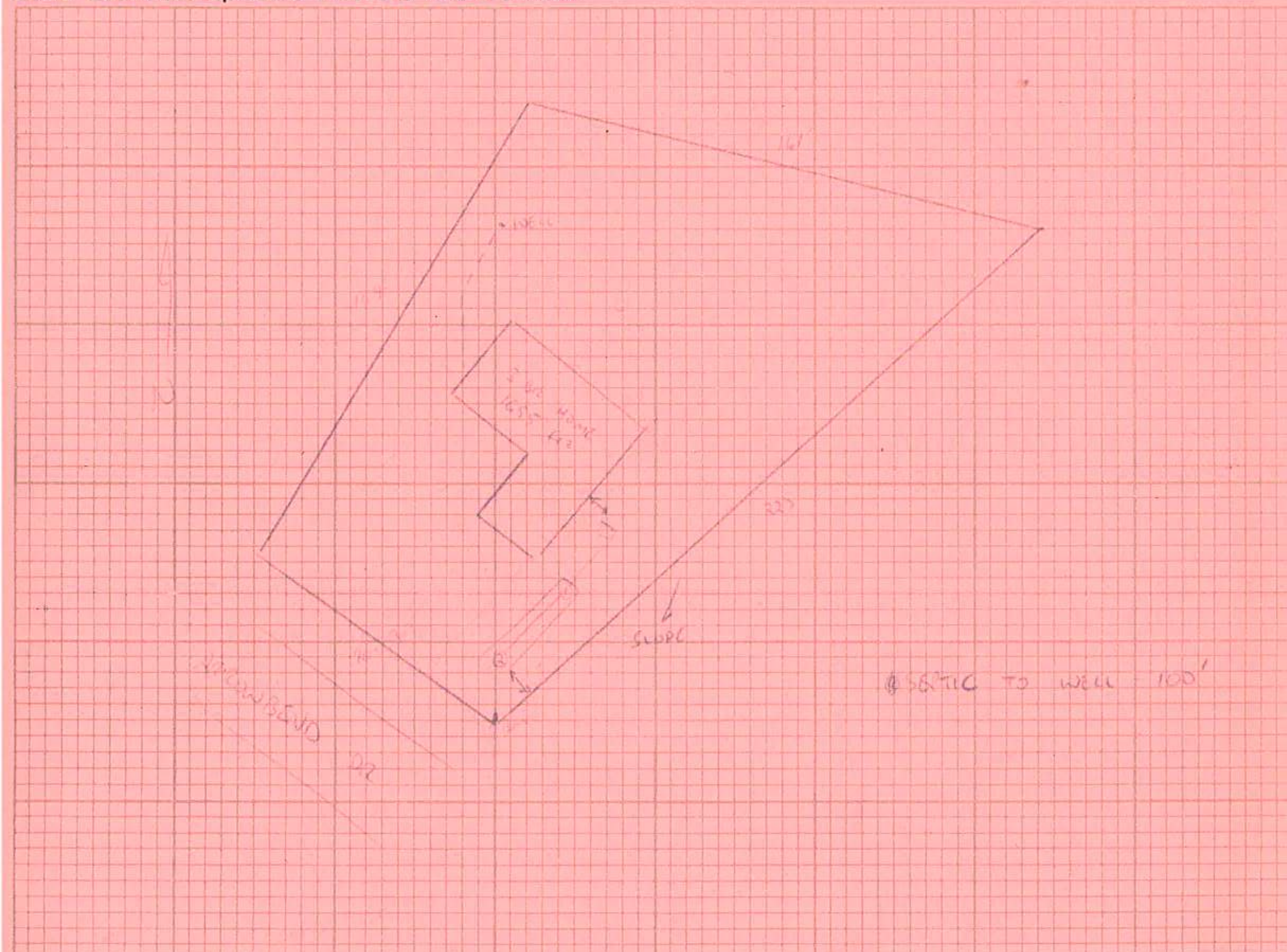
07-0235-N

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 12-SG-113153

PART II - SITE PLAN

Scale: Each block represents 5 feet and 1 inch = 50 feet.



Notes: ______________________________

Site Plan submitted by: ______________________ (Signature) OWNER (Title)

Plan Approved ______ Not Approved ______ Date 1/1/07

By ______________________ County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

25700

BRITT SURVEYING
Land Surveyors and Mappers
830 West Duval Street • Lake City, FL 32055
Phone (386) 752-7163 • Fax (386) 752-5573

04/27/07

L-18353

To Whom It May Concern:

C/o: G & J Builders

Re: Lot 17 Cannon Creek Place

The elevation of the foundation wall is found to be 98.33 feet. The minimum finished floor elevation is 97.50 feet according to the plat of record. The highest adjacent grade is 97.39 feet and the lowest adjacent grade is 97.17 feet. The elevations shown hereon are based on NGVD 29 datum.

L. Scott Britt
PLS #5757

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 24-4S-16-03114-117 **Building permit No.** 000025700

Use Classification SFD/UTILITY **Fire:** 57.78

Permit Holder G&J BUILDERS, INC.(G. ROHNER) **Waste:** 150.75

Owner of Building GREG TALLEY **Total:** 208.53

Location: 161 SW ARROWBEND DR., LAKE CITY, FL

Date: 01/08/2008

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

BRITT SURVEYING
830 West Duval Street • Lake City, FL 32055
Phone (386) 752-7163 • Fax (386) 752-5573

Land Surveyors and Mappers

02/11/08

L-19092

To Whom It May Concern:

C/o: G & J Builders

Re: Lot 17 Cannon Creek Place

The elevation of the foundation is found to be 98.39 feet. The recommended finished floor elevation is 97.50 feet as per the plat of record. The highest adjacent grade is 152.57 feet on the building pad. The highest adjacent grade is 98.0 feet and the lowest adjacent grade is 96.84 feet. The centerline of the adjacent road SW Arrowbend Drive is 95.83 feet. The elevations shown hereon are based on NGVD 29 Datum.

L. Scott Britt
PLS #5757

5700

Dwg.#0131071216

Project Information for:	L223413		
Builder:	GREG TALLEY	Date:	1/31/2007
Lot:	LOT 17 CANNON CREEK PL/	Start Number:	**1182**
Subdivision:	N/A	SEI Ref:	L223413
County or City:	COLUMBIA COUNTY		
Truss Page Count:	33		

Truss Design Load Information (UNO) Design Program: MiTek

Gravity		**Wind**		**Building Code:**	**FBC2004**
Roof (psf):	**42**	Wind Standard:	**ASCE 7-02**		
Floor (psf):	55	Wind Speed (mph):	**110**		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

ROHNER, GEORGE JOSEPH CGC021619
Address: 3031 SW 108 WAY
DAVIE,FLORIDA 33329 Designer: **156**

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549 Phone: 813-849-5769

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.
5. Where hangers are shown, Carried Member hanger capacity per Simpson C-2006 (SYP/Full Nailing Value) as an individual component. Building Designer shall verify the suitablity and use of Carrying Member hanger capacity.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	0131071182	1/31/2007				
2	CJ3	0131071183	1/31/2007				
3	CJ5	0131071184	1/31/2007				
4	EJ5	0131071185	1/31/2007				
5	EJ7	0131071186	1/31/2007				
6	EJ7A	0131071187	1/31/2007				
7	HJ7	0131071188	1/31/2007				
8	HJ9	0131071189	1/31/2007				
9	T01	0131071190	1/31/2007				
10	T01G	0131071191	1/31/2007				
11	T02	0131071192	1/31/2007				
12	T03	0131071193	1/31/2007				
13	T04	0131071194	1/31/2007				
14	T05	0131071195	1/31/2007				
15	T06	0131071196	1/31/2007				
16	T07	0131071197	1/31/2007				
17	T08	0131071198	1/31/2007				
18	T09	0131071199	1/31/2007				
19	T10	0131071200	1/31/2007				
20	T11	0131071201	1/31/2007				
21	T12	0131071202	1/31/2007				
22	T13	0131071203	1/31/2007				
23	T14	0131071204	1/31/2007				
24	T15	0131071205	1/31/2007				
25	T16	0131071206	1/31/2007				
26	T17	0131071207	1/31/2007				
27	T18	0131071208	1/31/2007				
28	T19	0131071209	1/31/2007				
29	T20	0131071210	1/31/2007				
30	T21	0131071211	1/31/2007				
31	T22	0131071212	1/31/2007				
32	T23	0131071213	1/31/2007				
33	T24	0131071214	1/31/2007				

JAN 31 2007

JANUARY 31,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Structural Engineering and Inspections, Inc. 16105 N. Florida Ave. Ste B Lutz, FL 33549 Thomas Miller. PE 56877, EB 9196

LATERAL TOE-NAIL DETAIL

ST-TOENAIL

NOTES:
1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 30 DEGREES WITH THE MEMBER AND STARTED 1/3 THE LENGTH OF THE NAIL FROM THE MEMBER END AS SHOWN.
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE BOTTOM CHORD SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

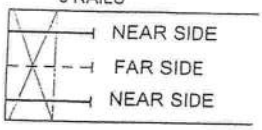
TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)

	DIAM.	SYP
3.5" LONG	.131	83.3
	.135	89.6
	.162	118.3
3.25" LONG	.128	80.5
	.131	83.3
	.148	102.1
3.0" LONG	.120	70.5
	.128	80.5
	.131	83.3
	.148	102.1

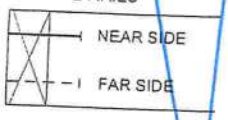
VALUES SHOWN ARE CAPACITY PER TOE-NAIL.
APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

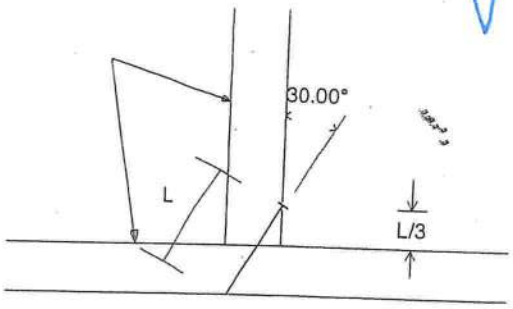
SQUARE CUT

SIDE VIEW (2x4, 2x6) 3 NAILS



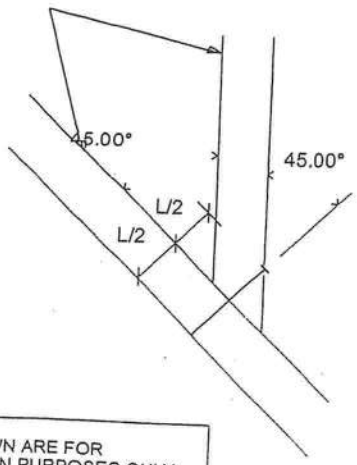
SIDE VIEW (2x3) 2 NAILS



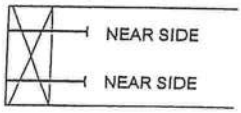


45 DEGREE ANGLE BEVEL CUT

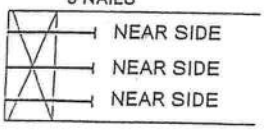
This detail may only be applied to Pre-engineered truss drawings signed and sealed by Structural Engineering and Inspections Inc.



SIDE VIEW (2x3, 2x4) 2 NAILS



SIDE VIEW (2x6) 3 NAILS



VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

JAN 3 1 2007

Dwg.#0131071181

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Licensee Information

Name:	**ROHNER, GEORGE JOSEPH** (Primary Name) **R B K BUILDERS INC** (DBA Name)
Main Address:	**3031 SW 108 WAY** **PO BOX 290023** **DAVIE Florida 33329-0023**
County:	**BROWARD**
License Mailing:	
LicenseLocation:	**3031 SW 108 WAY** **PO BOX 290023** **DAVIE FL 33329-0023**
County:	**BROWARD**

License Information

License Type:	**Certified General Contractor**
Rank:	**Cert General**
License Number:	**CGC021619**
Status:	**Current,Active**
Licensure Date:	**03/14/1982**
Expires:	**08/31/2008**

Special Qualifications	**Qualification Effective**
Bldg Code Core Course Credit	
Qualified Business License Required	**02/20/2004**

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JANUARY 31,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0131071182

Job	Truss	Truss Type	Qty	Ply	G & J BLDRS. - LOT 17 CCP
L223413	CJ1	JACK	16	1	Job Reference (optional)

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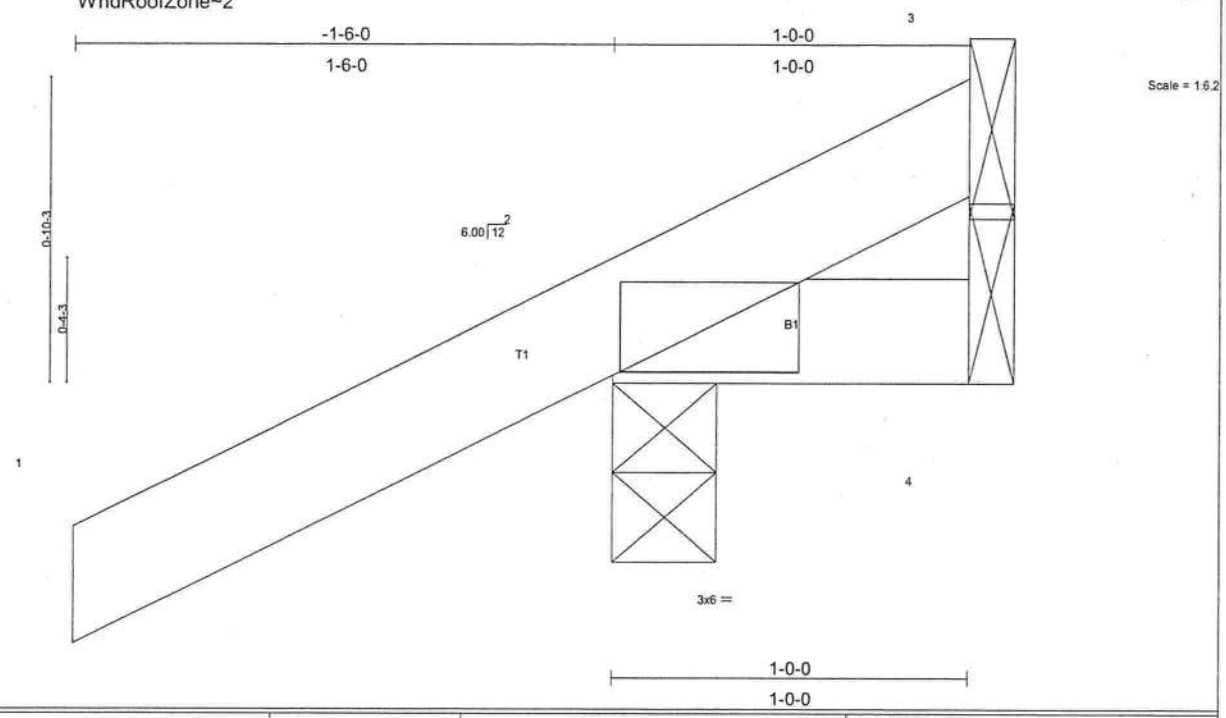
WndRoofZone~2



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.19	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 6 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=189/0-3-8, 4=14/Mechanical, 3=-40/Mechanical
Max Horz 2=85(load case 5)
Max Uplift2=-239(load case 5), 4=-12(load case 3), 3=-40(load case 1)
Max Grav 2=189(load case 1), 4=14(load case 1), 3=75(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-45/42
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.12

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 239 lb uplift at joint 2, 12 lb uplift at joint 4 and 40 lb uplift at joint 3.

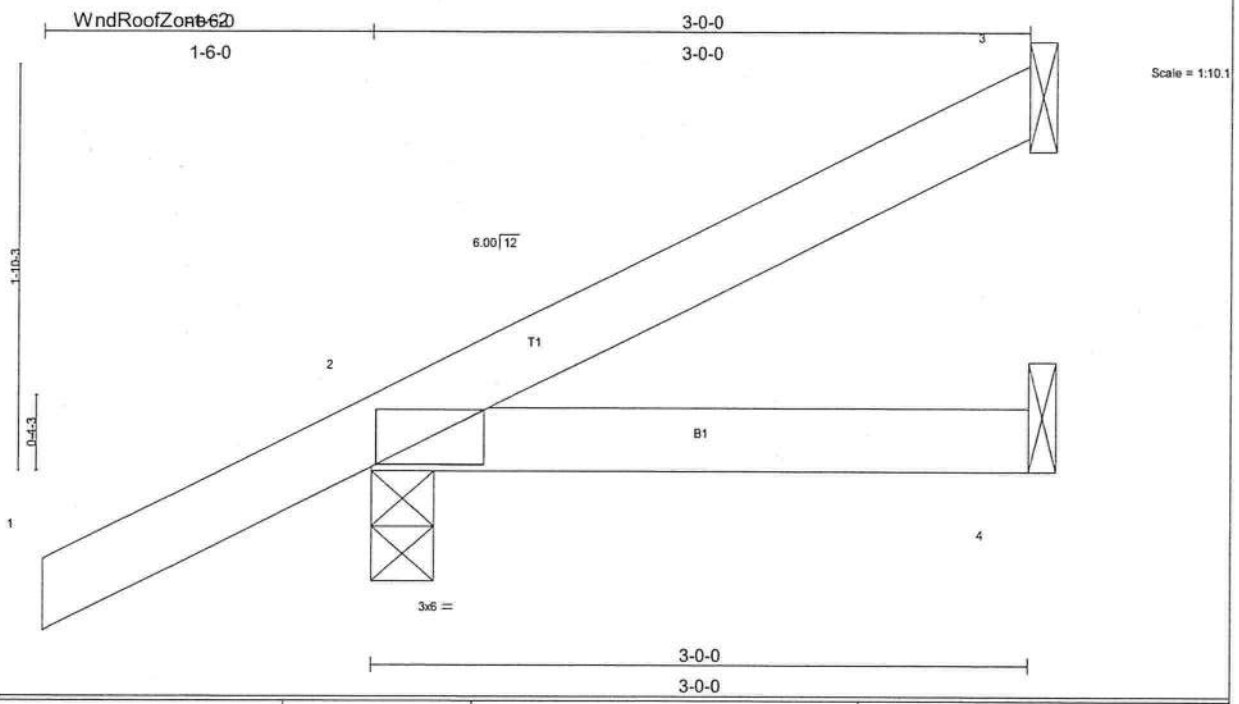
LOAD CASE(S) Standard

JANUARY 31,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0131071183

Job	Truss	Truss Type	Qty	Ply	G & J BLDRS. - LOT 17 CCP
L223413	CJ3	JACK	16	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 18 10:51:49 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=49/Mechanical, 2=232/0-3-8, 4=42/Mechanical
Max Horz 2=140(load case 5)
Max Uplift3=-48(load case 5), 2=-234(load case 5), 4=-34(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-53/16
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.13

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 48 lb uplift at joint 3, 234 lb uplift at joint 2 and 34 lb uplift at joint 4.

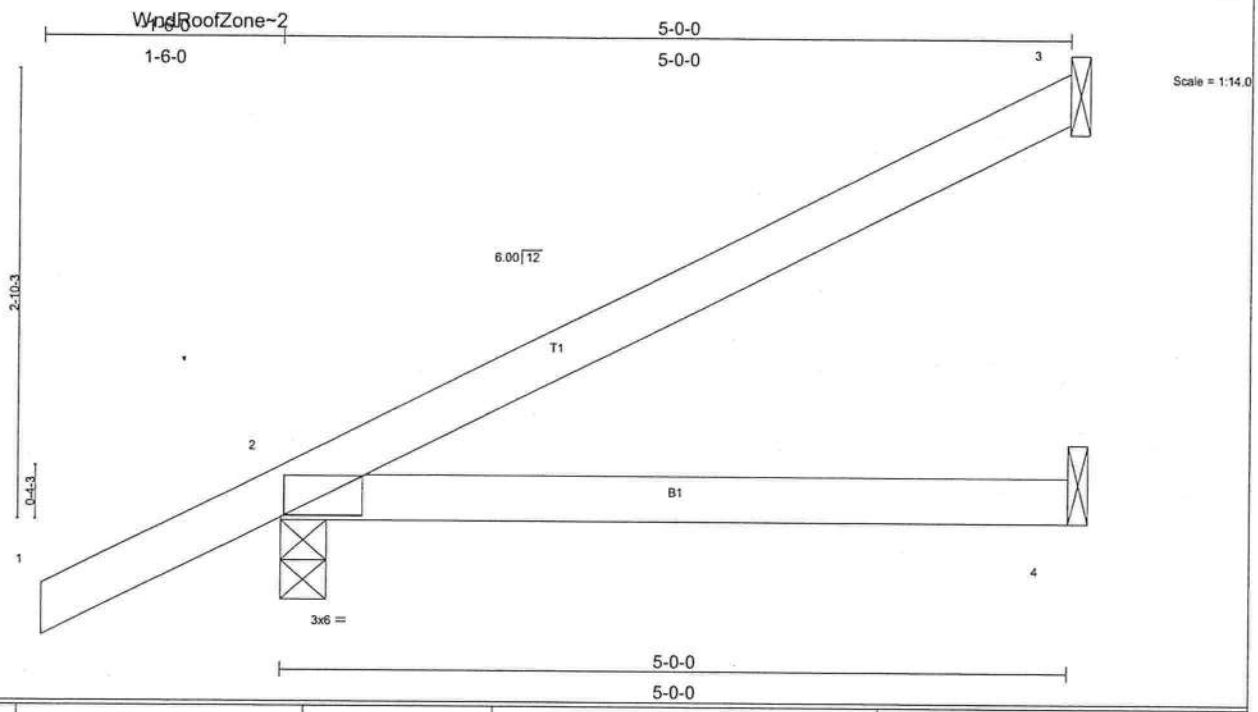
LOAD CASE(S) Standard

JANUARY 31,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0131071184

Job	Truss	Truss Type	Qty	Ply	G & J BLDRS. - LOT 17 CCP
L223413	CJ5	JACK	14	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 18 10:51:56 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	0.11	2-4	>547	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.30	Vert(TL)	0.09	2-4	>621	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 18 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=114/Mechanical, 2=305/0-3-8, 4=72/Mechanical
Max Horz 2=196(load case 5)
Max Uplift3=-127(load case 5), 2=-274(load case 5), 4=-58(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-114/41
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.16

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 127 lb uplift at joint 3, 274 lb uplift at joint 2 and 58 lb uplift at joint 4.

LOAD CASE(S) Standard

JANUARY 31,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0131071185

Job	Truss	Truss Type	Qty	Ply	G & J BLDRS. - LOT 17 CCP
L223413	EJ5	JACK	5	1	Job Reference (optional)

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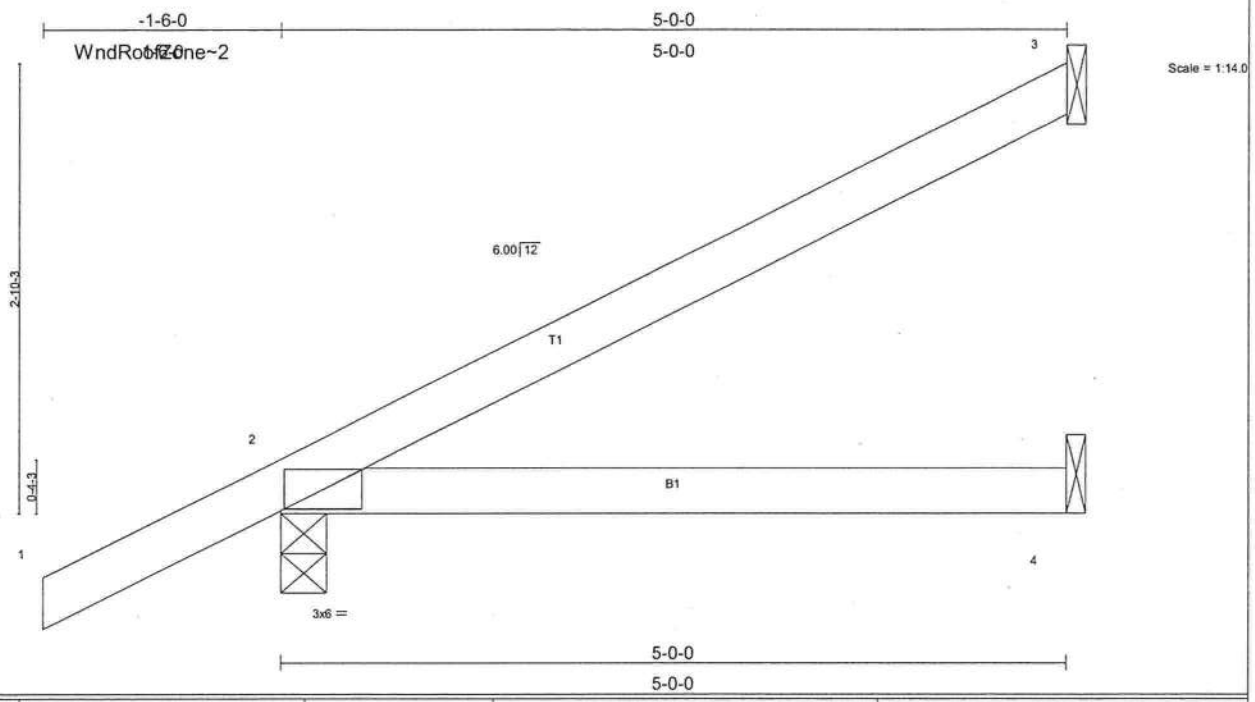

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	0.11	2-4	>547	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.30	Vert(TL)	0.09	2-4	>621	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 18 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=114/Mechanical, 2=305/0-3-8, 4=72/Mechanical
Max Horz 2=196(load case 5)
Max Uplift3=-127(load case 5), 2=-274(load case 5), 4=-58(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-114/41
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.16

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 127 lb uplift at joint 3, 274 lb uplift at joint 2 and 58 lb uplift at joint 4.

LOAD CASE(S) Standard

JANUARY 31,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0131071186

Job	Truss	Truss Type	Qty	Ply	G & J BLDRS. - LOT 17 CCP
L223413	EJ7	MONO TRUSS	24	1	Job Reference (optional)

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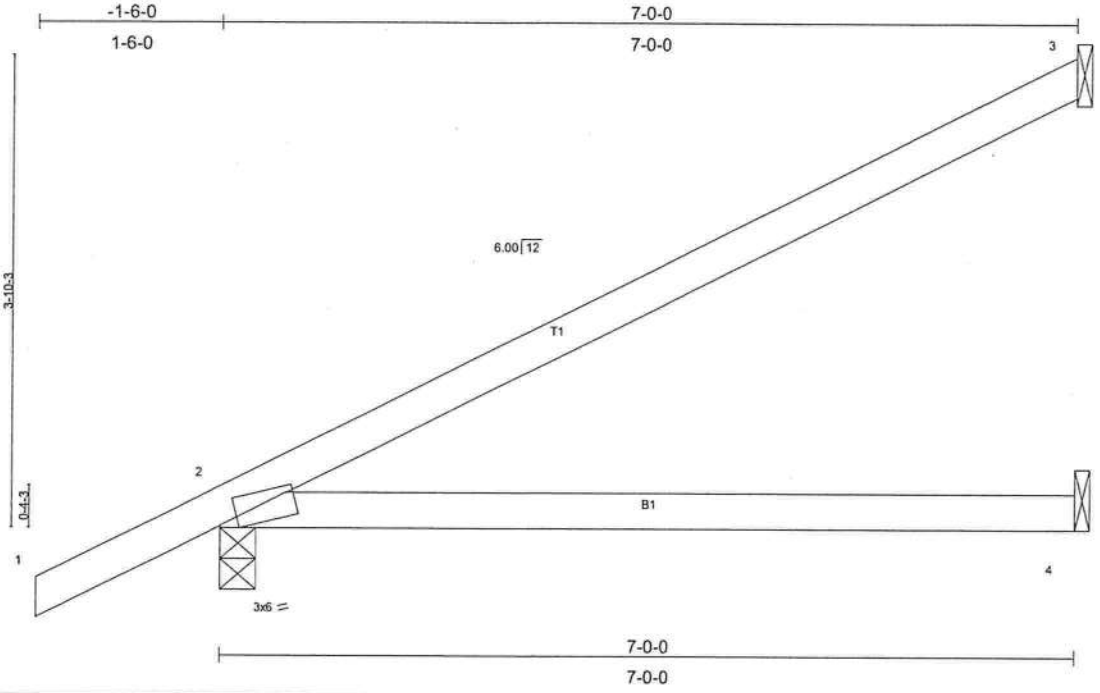

Plate Offsets (X,Y): [2:0-1-12,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.13	2-4	>606	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.22	2-4	>365	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 25 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=166/Mechanical, 2=385/0-3-8, 4=108/Mechanical
Max Horz 2=252(load case 5)
Max Uplift3=-173(load case 5), 2=-222(load case 5), 4=-1(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-106/59
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.89

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 173 lb uplift at joint 3, 222 lb uplift at joint 2 and 1 lb uplift at joint 4.

LOAD CASE(S) Standard

JANUARY 31,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0131071187

Job	Truss	Truss Type	Qty	Ply	G & J BLDRS. - LOT 17 CCP
L223413	EJ7A	MONO TRUSS	1	1	Job Reference (optional)

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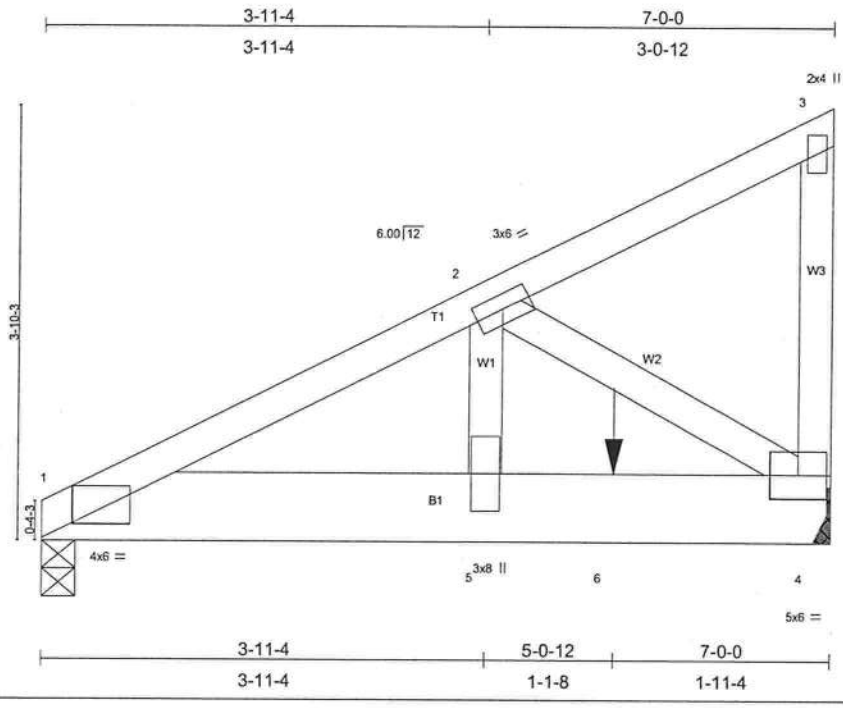

Plate Offsets (X,Y): [1:0-2-4,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.02	4-5	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	-0.03	4-5	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.40	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 44 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-1-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=866/0-3-8, 4=1482/Mechanical
Max Horz 1=185(load case 4)
Max Uplift1=-382(load case 4), 4=-778(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1484/627, 2-3=-79/0, 3-4=-66/72
BOT CHORD 1-5=-688/1290, 5-6=-688/1290, 4-6=-688/1290
WEBS 2-5=-540/1236, 2-4=-1477/785

JOINT STRESS INDEX
1 = 0.63, 2 = 0.92, 3 = 0.52, 4 = 0.38 and 5 = 0.40

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 382 lb uplift at joint 1 and 778 lb uplift at joint 4.
4) Girder carries tie-in span(s): 5-0-0 from 0-0-0 to 4-0-0
5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1546 lb down and 763 lb up at 5-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-93(F=-63), 4-5=-30, 1-3=-54
Concentrated Loads (lb)
Vert: 6=-1546(F)

JANUARY 31,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0131071188

Job	Truss	Truss Type	Qty	Ply	G & J BLDRS. - LOT 17 CCP
L223413	HJ7	MONO TRUSS	1	1	Job Reference (optional)

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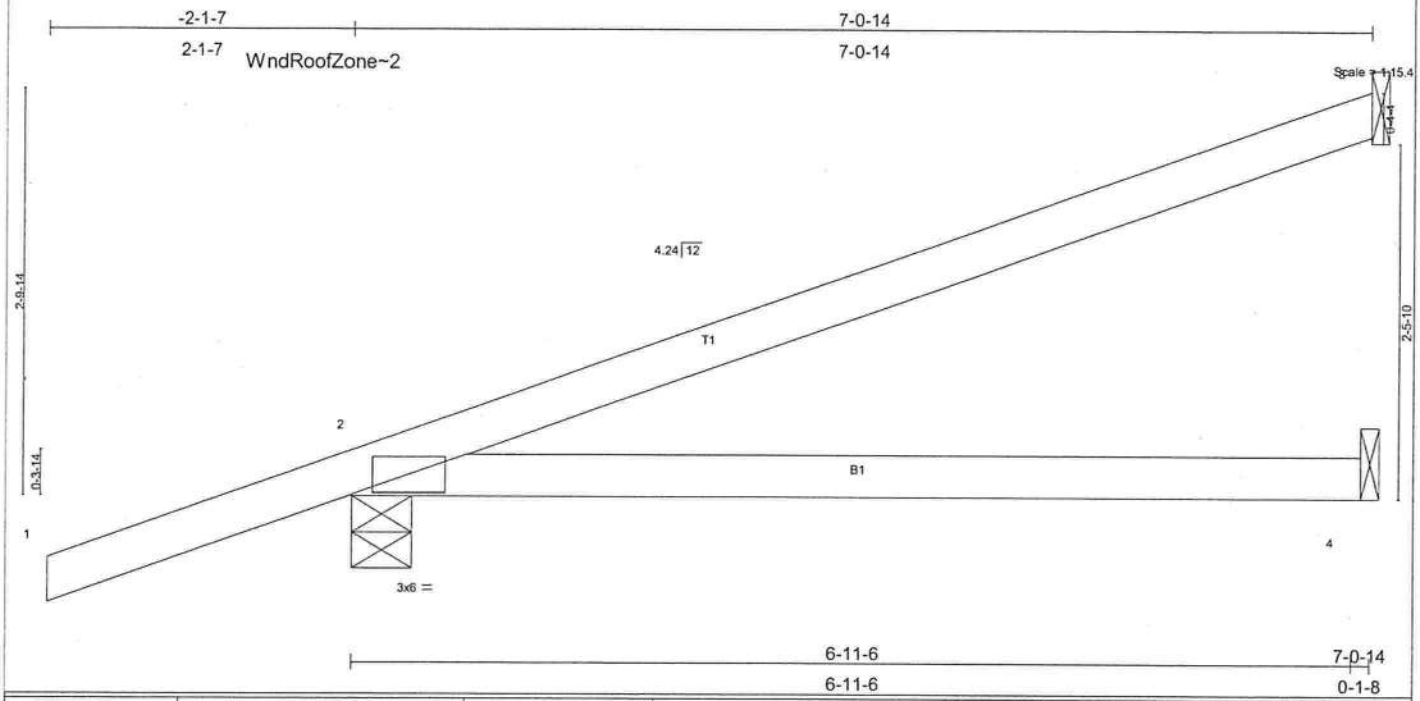

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	0.15	2-4	>548	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.32	Vert(TL)	-0.16	2-4	>496	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 25 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 7-0-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=199/Mechanical, 2=321/0-4-15, 4=118/Mechanical
Max Horz 2=182(load case 2)
Max Uplift3=-204(load case 2), 2=-344(load case 2), 4=-75(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/37, 2-3=-88/47
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.40

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 204 lb uplift at joint 3, 344 lb uplift at joint 2 and 75 lb uplift at joint 4.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=26, B=26)-to-3=-95(F=-21, B=-21), 2=-0(F=15, B=15)-to-4=-53(F=-12, B=-12)

JANUARY 31,2007 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0131071189

Job	Truss	Truss Type	Qty	Ply	G & J BLDRS. - LOT 17 CCP
L223413	HJ9	MONO TRUSS	7	1	Job Reference (optional)

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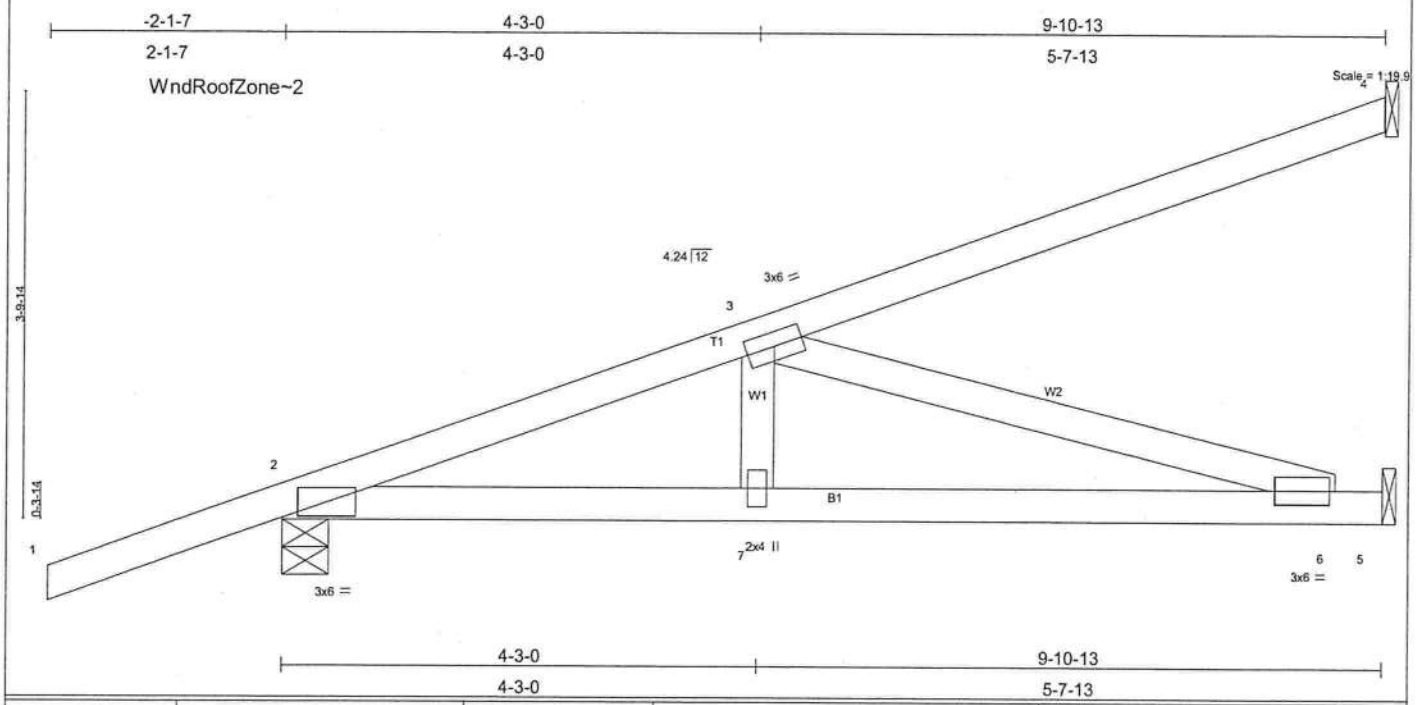

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	0.12	6-7	>959	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.18	6-7	>626	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.50	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 43 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-11-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-10-7 oc bracing.

REACTIONS (lb/size) 4=268/Mechanical, 2=483/0-4-15, 5=388/Mechanical
Max Horz 2=306(load case 2)
Max Uplift4=-289(load case 2), 2=-437(load case 2), 5=-253(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/37, 2-3=-941/551, 3-4=-124/65
BOT CHORD 2-7=-761/877, 6-7=-761/877, 5-6=0/0
WEBS 3-7=-126/209, 3-6=-914/792

JOINT STRESS INDEX
2 = 0.75, 3 = 0.33, 6 = 0.26 and 7 = 0.15

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 289 lb uplift at joint 4, 437 lb uplift at joint 2 and 253 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=26, B=26)-to-4=-134(F=-40, B=-40), 2=-0(F=15, B=15)-to-5=-74(F=-22, B=-22)

~~JANUARY 31,2007 TRUSS DESIGN ENGINEER:~~
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0131071190

Job	Truss	Truss Type	Qty	Ply	G & J BLDRS. - LOT 17 CCP
L223413	T01	COMMON	11	1	Job Reference (optional)

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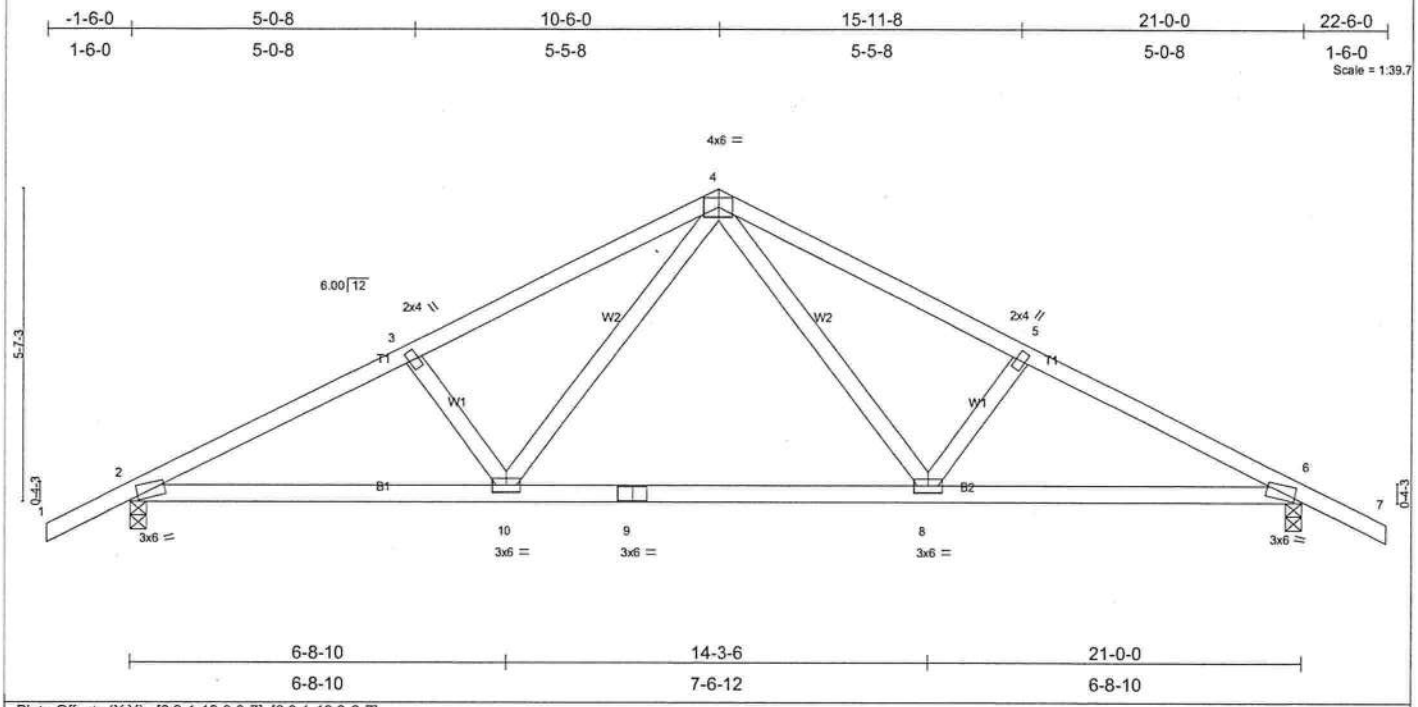

Plate Offsets (X,Y): [2:0-1-13,0-0-7], [6:0-1-13,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.27	8-10	>916	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.91	Vert(TL)	-0.44	8-10	>565	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.26	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 99 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-5-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-3-8 oc bracing.

REACTIONS (lb/size) 2=1148/0-3-8, 6=1148/0-3-8
Max Horz 2=-124(load case 6)
Max Uplift2=-586(load case 5), 6=-586(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1955/839, 3-4=-1805/823, 4-5=-1805/823, 5-6=-1955/840, 6-7=0/35
BOT CHORD 2-10=-731/1680, 9-10=-378/1117, 8-9=-378/1117, 6-8=-610/1680
WEBS 3-10=-231/266, 4-10=-343/779, 4-8=-343/779, 5-8=-231/266

JOINT STRESS INDEX
2 = 0.82, 3 = 0.34, 4 = 0.62, 5 = 0.34, 6 = 0.82, 8 = 0.56, 9 = 0.79 and 10 = 0.56

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 586 lb uplift at joint 2 and 586 lb uplift at joint 6.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-10=-30, 8-10=-80(F=-50), 6-8=-30

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0131071191

Job	Truss	Truss Type	Qty	Ply	G & J BLDRS. - LOT 17 CCP
L223413	T01G	GABLE	1	1	Job Reference (optional)

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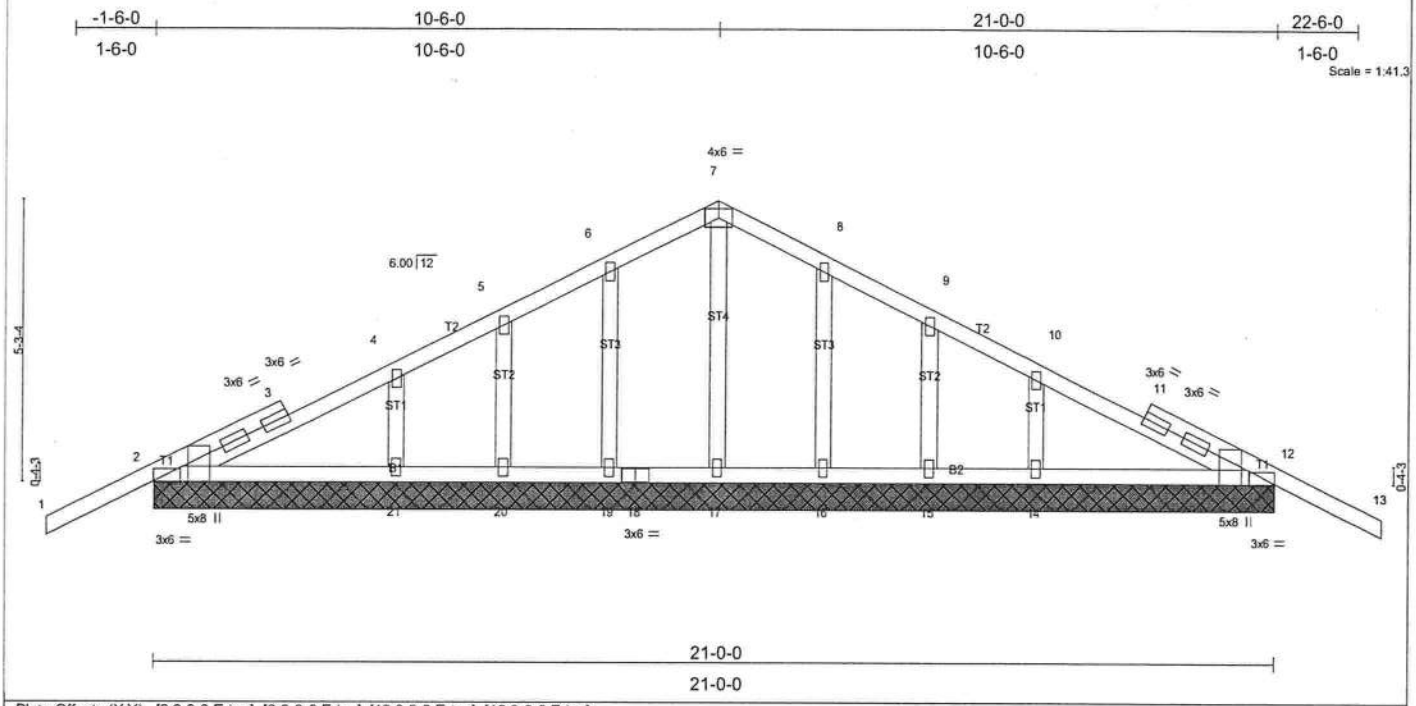

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-8,Edge], [12:0-3-8,Edge], [12:0-0-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.03	13	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.04	13	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.12	Horz(TL)	0.01	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 110 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=543/21-0-0, 12=543/21-0-0, 17=323/21-0-0, 19=315/21-0-0, 20=193/21-0-0, 21=527/21-0-0, 16=315/21-0-0, 15=193/21-0-0, 14=527/21-0-0
Max Horz 2=130(load case 5)
Max Uplift2=-354(load case 5), 12=-375(load case 6), 17=-30(load case 5), 19=-174(load case 5), 20=-145(load case 5), 21=-250(load case 5), 16=-172(load case 6), 15=-143(load case 6), 14=-255(load case 6)
Max Grav 2=546(load case 9), 12=546(load case 10), 17=323(load case 1), 19=319(load case 9), 20=193(load case 1), 21=528(load case 9), 16=319(load case 10), 15=193(load case 1), 14=528(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-18/99, 2-3=-87/54, 3-4=-120/162, 4-5=-26/108, 5-6=0/137, 6-7=-7/180, 7-8=-7/176, 8-9=0/129, 9-10=-26/85, 10-11=-65/162, 11-12=-52/54, 12-13=-18/99
BOT CHORD 2-21=-50/179, 20-21=-50/179, 19-20=-50/179, 18-19=-50/179, 17-18=-50/179, 16-17=-50/179, 15-16=-50/179, 14-15=-50/179, 12-14=-50/179
WEBS 7-17=-268/41, 6-19=-250/187, 5-20=-164/151, 4-21=-401/273, 8-16=-250/185, 9-15=-164/151, 10-14=-401/278

JOINT STRESS INDEX
2 = 0.66, 2 = 0.18, 3 = 0.00, 3 = 0.42, 3 = 0.43, 4 = 0.34, 5 = 0.34, 6 = 0.34, 7 = 0.25, 8 = 0.34, 9 = 0.34, 10 = 0.34, 11 = 0.00, 11 = 0.43, 11 = 0.42, 12 = 0.66, 12 = 0.18, 14 = 0.34, 15 = 0.34, 16 = 0.34, 17 = 0.34, 18 = 0.15, 19 = 0.34, 20 = 0.34 and 21 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 354 lb uplift at joint 2, 375 lb uplift at joint 12, 30 lb uplift at joint 17, 174 lb uplift at joint 19, 145 lb uplift at joint 20, 250 lb uplift at joint 21, 172 lb uplift at joint 16, 143 lb uplift at joint 15 and 255 lb uplift at joint 14.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-7=-114(F=-60), 7-13=-114(F=-60), 2-12=-30

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0131071192

Job	Truss	Truss Type	Qty	Ply	G & J BLDRS. - LOT 17 CCP
L223413	T02	HIP	1	1	Job Reference (optional)

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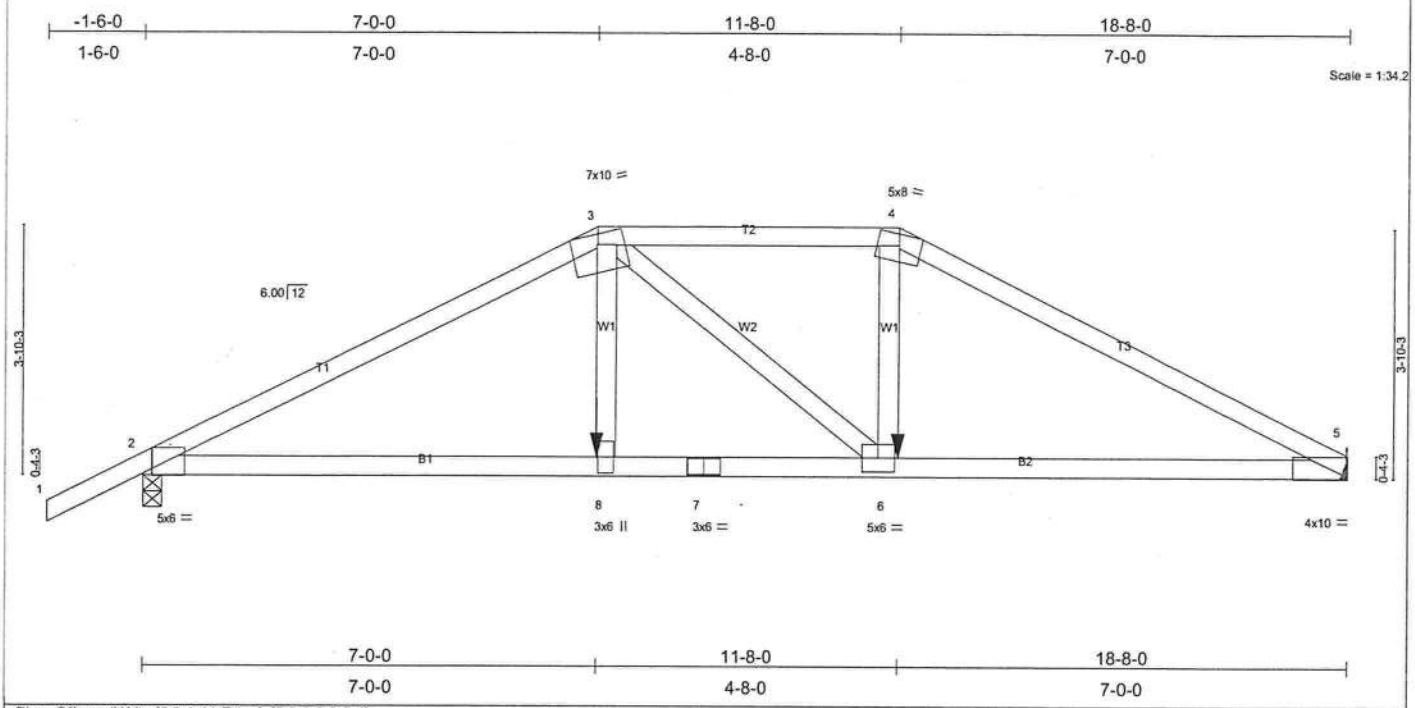

Plate Offsets (X,Y): [2:0-1-11,Edge], [5:0-0-0,0-0-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.15	5-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.79	Vert(TL)	-0.25	5-6	>884	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.28	Horz(TL)	0.08	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 78 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-2-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-2-14 oc bracing.

REACTIONS (lb/size) 5=1546/Mechanical, 2=1650/0-3-8
Max Horz 2=116(load case 4)
Max Uplift5=-830(load case 5), 2=-957(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2927/1564, 3-4=-2592/1524, 4-5=-2929/1594
BOT CHORD 2-8=-1341/2533, 7-8=-1354/2566, 6-7=-1354/2566, 5-6=-1314/2559
WEBS 3-8=-313/813, 3-6=-114/165, 4-6=-345/870

JOINT STRESS INDEX
2 = 0.73, 3 = 0.79, 4 = 0.75, 5 = 0.72, 6 = 0.31, 7 = 0.88 and 8 = 0.26

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 830 lb uplift at joint 5 and 957 lb uplift at joint 2.
6) Girder carries hip end with 7-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 356 lb up at 11-8-0, and 539 lb down and 356 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-121(F=-68), 4-5=-54, 2-8=-30, 6-8=-68(F=-38), 5-6=-30
Concentrated Loads (lb)
Vert: 8=-539(F) 6=-539(F)

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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0131071193

Job	Truss	Truss Type	Qty	Ply	G & J BLDRS. - LOT 17 CCP
L223413	T03	SPECIAL	1	1	Job Reference (optional)

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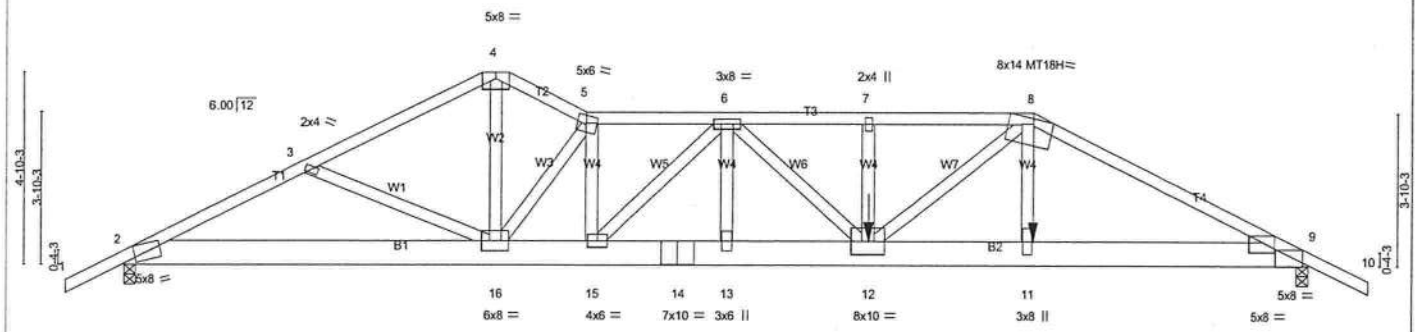
-1-6-0 | 4-8-15 | 9-0-0 | 9-8-0 | 11-8-0 | 15-2-14 | 18-9-12 | 23-0-0 | 30-0-0 | 31-6-0

1-6-0 | 4-8-15 | 4-3-1 | 0-8-0 | 2-0-0 | 3-6-14 | 3-6-14 | 4-2-4 | 7-0-0 | 1-6-0

Scale = 1:56.0



9-0-0 | 11-8-0 | 15-2-14 | 18-9-12 | 23-0-0 | 30-0-0

9-0-0 | 2-8-0 | 3-6-14 | 3-6-14 | 4-2-4 | 7-0-0

Plate Offsets (X,Y): [8:0-6-3,Edge], [9:0-1-10,Edge], [9:0-9-15,0-3-14]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.84	Vert(LL)	-0.38	12-13	>950	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.73	Vert(TL)	-0.60	12-13	>594	180	MT18H	244/190
BCLL	10.0	Rep Stress Incr	NO	WB	0.99	Horz(TL)	0.10	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 202 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W3 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-11-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-5-9 oc bracing.

REACTIONS (lb/size) 2=2138/0-3-8, 9=2965/0-3-8
Max Horz 2=-119(load case 5)
Max Uplift2=-1046(load case 4), 9=-1627(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/41, 2-3=-4125/1984, 3-4=-3833/1869, 4-5=-3773/1865, 5-6=-5215/2650, 6-7=-6800/3571, 7-8=-6800/3570, 8-9=-5931/3093, 9-10=0/41
BOT CHORD 2-16=-1681/3653, 15-16=-2370/5178, 14-15=-2986/6279, 13-14=-2986/6279, 12-13=-2986/6279, 11-12=-2612/5281, 9-11=-2598/5243
WEBS 3-16=-300/325, 5-16=-3166/1727, 5-15=-496/905, 6-15=-1566/838, 6-13=-110/349, 6-12=-472/803, 7-12=-291/349, 8-12=-981/1954, 8-11=-343/879, 4-16=-1492/3088

JOINT STRESS INDEX
2 = 0.86, 3 = 0.34, 4 = 0.95, 5 = 0.82, 6 = 0.79, 7 = 0.34, 8 = 0.83, 9 = 0.79, 9 = 0.54, 11 = 0.29, 12 = 0.44, 13 = 0.16, 14 = 0.95, 15 = 0.42 and 16 = 0.77

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1046 lb uplift at joint 2 and 1627 lb uplift at joint 9.
7) Girder carries hip end with 7-0-0 right side setback, 18-8-0 left side setback, and 7-0-0 end setback.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 356 lb up at 23-0-0, and 1482 lb down and 732 lb up at 18-9-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-5=-54, 5-7=-54, 7-8=-118(F=-64), 8-10=-54, 2-12=-30, 11-12=-65(F=-35), 9-11=-30
Concentrated Loads (lb)
Vert: 12=-1482(F) 11=-539(F)

JANUARY 31,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0131071194

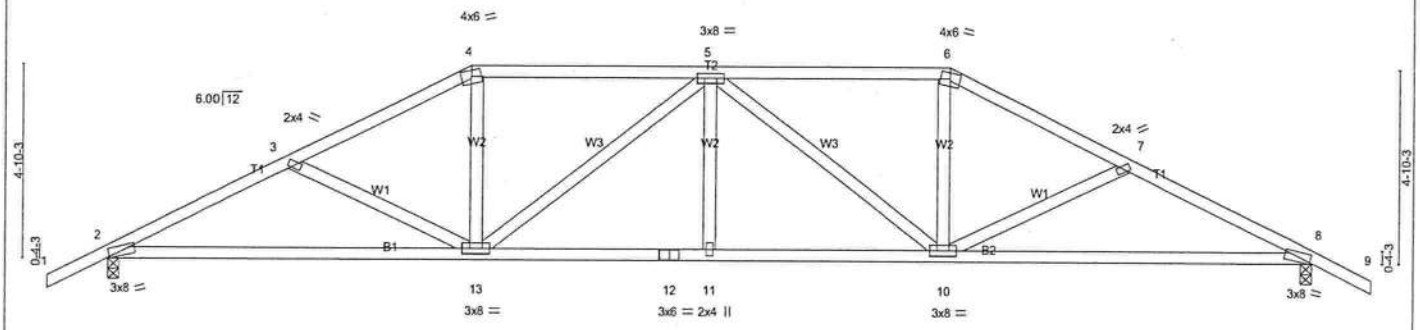
Job	Truss	Truss Type	Qty	Ply	G & J BLDRS. - LOT 17 CCP
L223413	T04	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 18 10:53:03 2007 Page 1

-1-6-0 | 4-7-15 | 9-0-0 | 15-0-0 | 21-0-0 | 25-4-1 | 30-0-0 | 31-6-0

1-6-0 | 4-7-15 | 4-4-1 | 6-0-0 | 6-0-0 | 4-4-1 | 4-7-15 | 1-6-0

Scale = 1:55.3



9-0-0 | 15-0-0 | 21-0-0 | 30-0-0

9-0-0 | 6-0-0 | 6-0-0 | 9-0-0

Plate Offsets (X,Y): [2:0-0-10,Edge], [8:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.21	2-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.68	Vert(TL)	-0.35	2-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.44	Horz(TL)	0.10	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 152 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-3-15 oc bracing.

REACTIONS (lb/size) 2=1337/0-3-8, 8=1337/0-3-8
Max Horz 2=111(load case 5)
Max Uplift2=-603(load case 5), 8=-603(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2248/867, 3-4=-2007/761, 4-5=-1768/733, 5-6=-1768/733, 6-7=-2007/761, 7-8=-2248/867, 8-9=0/35
BOT CHORD 2-13=-745/1964, 12-13=-720/2060, 11-12=-720/2060, 10-11=-720/2060, 8-10=-635/1964
WEBS 3-13=-245/265, 4-13=-129/588, 5-13=-466/278, 5-11=0/125, 5-10=-466/278, 6-10=-128/588, 7-10=-245/266

JOINT STRESS INDEX
2 = 0.78, 3 = 0.34, 4 = 0.63, 5 = 0.57, 6 = 0.63, 7 = 0.34, 8 = 0.78, 10 = 0.57, 11 = 0.34, 12 = 0.72 and 13 = 0.57

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 603 lb uplift at joint 2 and 603 lb uplift at joint 8.

LOAD CASE(S) Standard

JANUARY 31,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0131071195

Job	Truss	Truss Type	Qty	Ply	G & J BLDRS. - LOT 17 CCP
L223413	T05	HIP	1	1	Job Reference (optional)

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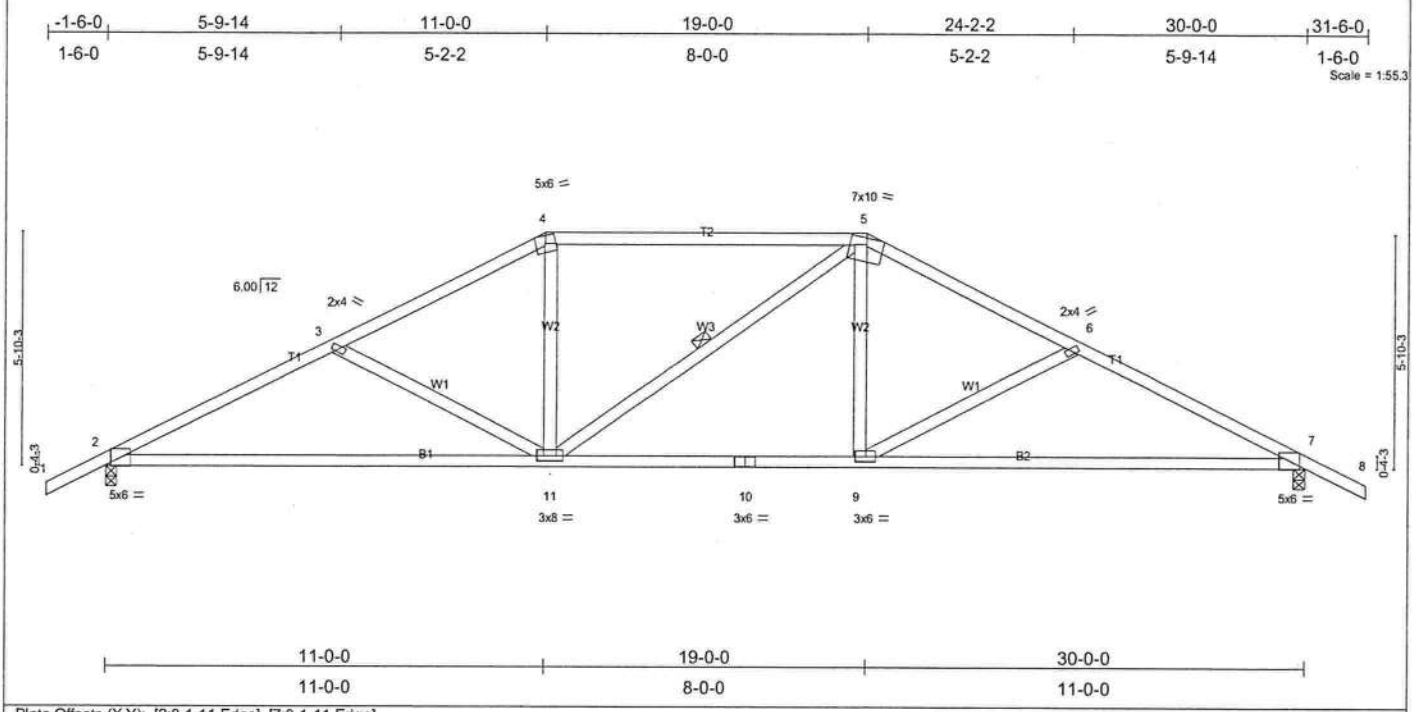

Plate Offsets (X,Y): [2:0-1-11,Edge], [7:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.40	7-9	>888	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.73	Vert(TL)	-0.68	7-9	>521	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.21	Horz(TL)	0.09	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 145 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-2-6 oc bracing.
WEBS 1 Row at midpt 5-11

REACTIONS (lb/size) 2=1337/0-3-8, 7=1337/0-3-8
Max Horz 2=128(load case 5)
Max Uplift2=-623(load case 5), 7=-623(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2170/888, 3-4=-1863/714, 4-5=-1625/708, 5-6=-1862/714, 6-7=-2170/889, 7-8=0/35
BOT CHORD 2-11=-767/1896, 10-11=-409/1624, 9-10=-409/1624, 7-9=-640/1896
WEBS 3-11=-319/337, 4-11=-88/434, 5-11=-146/147, 5-9=-101/435, 6-9=-320/337

JOINT STRESS INDEX
2 = 0.71, 3 = 0.34, 4 = 0.72, 5 = 0.75, 6 = 0.34, 7 = 0.71, 9 = 0.35, 10 = 0.60 and 11 = 0.57

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 623 lb uplift at joint 2 and 623 lb uplift at joint 7.

LOAD CASE(S) Standard

JANUARY 31,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0131071196

Job	Truss	Truss Type	Qty	Ply	G & J BLDRS. - LOT 17 CCP
L223413	T06	SPECIAL	1	1	Job Reference (optional)

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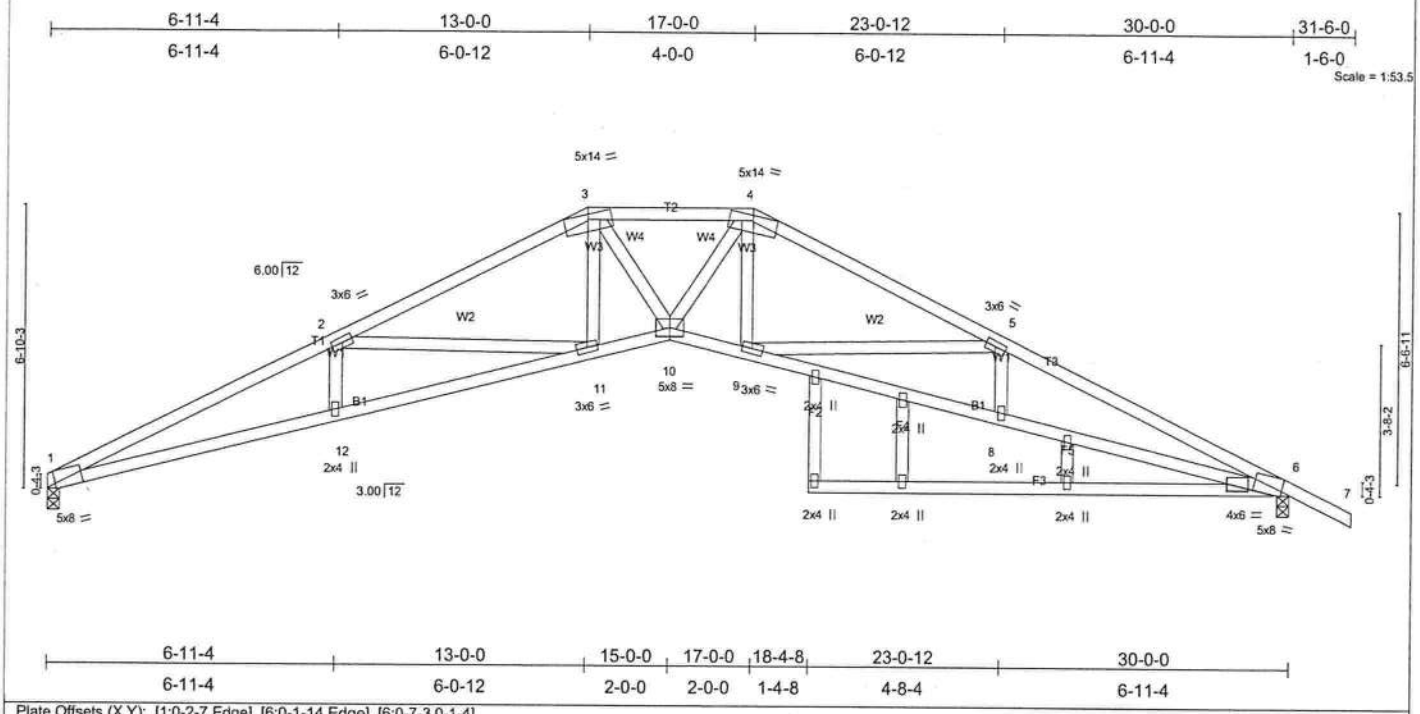

Plate Offsets (X,Y): [1:0-2-7,Edge], [6:0-1-14,Edge], [6:0-7-3,0-1-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.65	Vert(LL)	-0.46	10	>778	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.76	Vert(TL)	-0.73	8-9	>485	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.66	Horz(TL)	0.53	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 164 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D *Except*
F3 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-5-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-2-6 oc bracing. Except:
1 Row at midpt 8-9, 6-8
JOINTS 1 Brace at Jt(s): 8

REACTIONS (lb/size) 1=1245/0-3-8, 6=1339/0-3-8
Max Horz 1=-165(load case 6)
Max Uplift1=-521(load case 5), 6=-641(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4218/1705, 2-3=-3136/1142, 3-4=-3196/1186, 4-5=-3135/1078, 5-6=-4184/1499, 6-7=0/34
BOT CHORD 1-12=-1552/3814, 11-12=-1548/3808, 10-11=-846/2826, 9-10=-759/2825, 8-9=-1208/3776, 6-8=-1208/3780
WEBS 2-12=0/216, 2-11=-957/687, 3-11=-177/479, 3-10=-217/771, 4-10=-305/772, 4-9=-169/474, 5-9=-926/649, 5-8=0/206

JOINT STRESS INDEX
1 = 0.81, 2 = 0.41, 3 = 0.69, 4 = 0.69, 5 = 0.41, 6 = 0.81, 6 = 0.48, 8 = 0.34, 9 = 0.38, 10 = 0.88, 11 = 0.38, 12 = 0.34, 13 = 0.34, 14 = 0.34, 15 = 0.34, 16 = 0.34, 17 = 0.34 and 18 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 521 lb uplift at joint 1 and 641 lb uplift at joint 6.

LOAD CASE(S) Standard

JANUARY 31,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0131071197

Job	Truss	Truss Type	Qty	Ply	G & J BLDRS. - LOT 17 CCP
L223413	T07	SCISSOR	2	1	Job Reference (optional)

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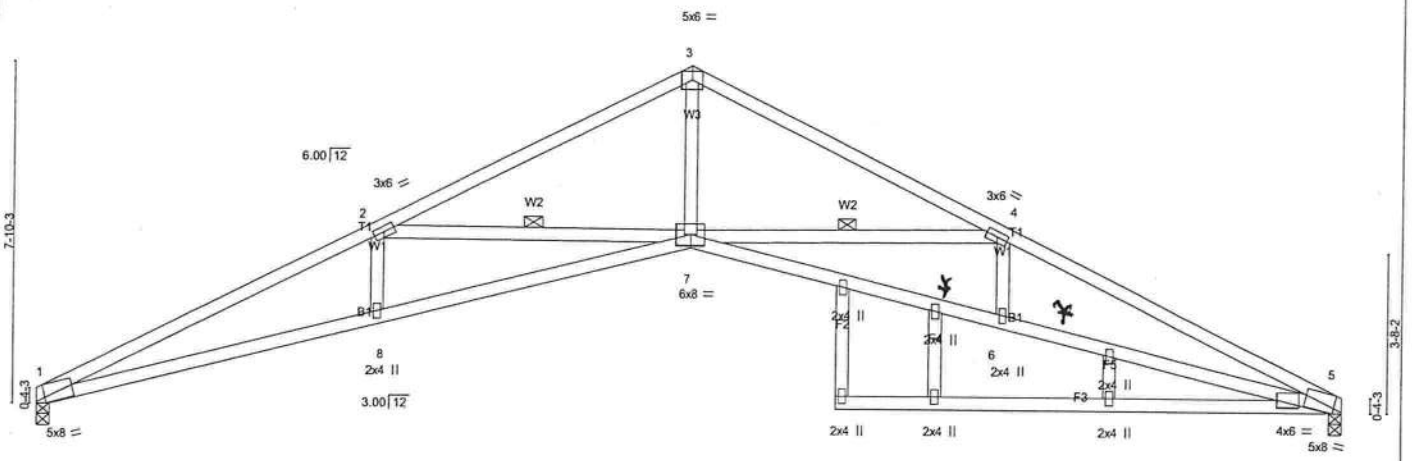
7-9-8 | 15-0-0 | 22-2-8 | 30-0-0
7-9-8 | 7-2-8 | 7-2-8 | 7-9-8

Scale = 1:50.9



FILLER CAN BE CUT BACK FOR ANGLE WAL

7-9-8 | 15-0-0 | 18-4-8 | 22-2-8 | 30-0-0
7-9-8 | 7-2-8 | 3-4-8 | 3-10-0 | 7-9-8

Plate Offsets (X,Y): [1:0-2-7,Edge], [5:0-1-14,Edge], [5:0-7-3,0-1-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.87	Vert(LL)	-0.48	7-8	>738	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.84	Vert(TL)	-0.78	7-8	>459	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.68	Horz(TL)	0.54	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 153 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D *Except*
F3 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 4-11-13 oc bracing. Except:
1 Row at midpt 6-7, 5-6
WEBS 1 Row at midpt 2-7, 4-7
JOINTS 1 Brace at Jt(s): 6

REACTIONS (lb/size) 1=1248/0-3-8, 5=1248/0-3-8
Max Horz 1=133(load case 4)
Max Uplift1=-537(load case 5), 5=-537(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4167/1765, 2-3=-2873/1090, 3-4=-2873/1116, 4-5=-4167/1634
BOT CHORD 1-8=-1636/3768, 7-8=-1631/3760, 6-7=-1379/3760, 5-6=-1384/3768
WEBS 2-8=0/251, 2-7=-1191/815, 3-7=-700/2104, 4-7=-1191/823, 4-6=0/251

JOINT STRESS INDEX
1 = 0.80, 2 = 0.41, 3 = 0.74, 4 = 0.41, 5 = 0.80, 5 = 0.47, 6 = 0.34, 7 = 0.89, 8 = 0.34, 9 = 0.34, 10 = 0.34, 11 = 0.34, 12 = 0.34, 13 = 0.34 and 14 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 537 lb uplift at joint 1 and 537 lb uplift at joint 5.

LOAD CASE(S) Standard

JANUARY 31,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0131071198

Job	Truss	Truss Type	Qty	Ply	G & J BLDRS. - LOT 17 CCP
L223413	T08	SCISSOR	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 18 10:53:32 2007 Page 1

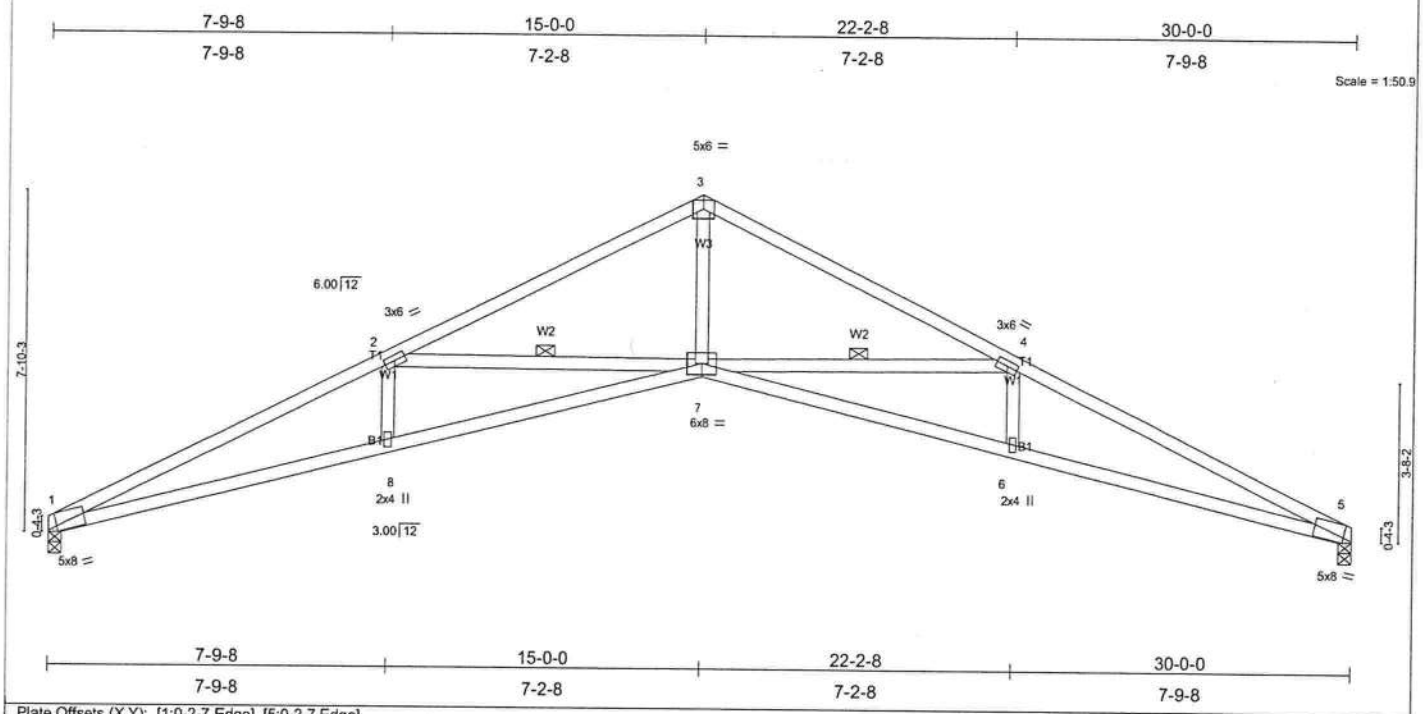


Plate Offsets (X,Y): [1:0-2-7,Edge], [5:0-2-7,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.87	Vert(LL)	-0.48	7-8	>738	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.77	Vert(TL)	-0.78	7-8	>459	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.68	Horz(TL)	0.54	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 128 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 4-11-13 oc bracing.
WEBS 1 Row at midpt 2-7, 4-7

REACTIONS (lb/size) 1=1248/0-3-8, 5=1248/0-3-8
Max Horz 1=133(load case 4)
Max Uplift1=-537(load case 5), 5=-537(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4167/1765, 2-3=-2873/1090, 3-4=-2873/1116, 4-5=-4167/1634
BOT CHORD 1-8=-1636/3768, 7-8=-1631/3760, 6-7=-1379/3760, 5-6=-1384/3768
WEBS 2-8=0/251, 2-7=-1191/815, 3-7=-700/2104, 4-7=-1191/823, 4-6=0/251

JOINT STRESS INDEX
1 = 0.80, 2 = 0.41, 3 = 0.74, 4 = 0.41, 5 = 0.80, 6 = 0.34, 7 = 0.89 and 8 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 537 lb uplift at joint 1 and 537 lb uplift at joint 5.

LOAD CASE(S) Standard

JANUARY 31,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0131071199

Job	Truss	Truss Type	Qty	Ply	G & J BLDRS. - LOT 17 CCP
L223413	T09	MONO HIP	1	1	Job Reference (optional)

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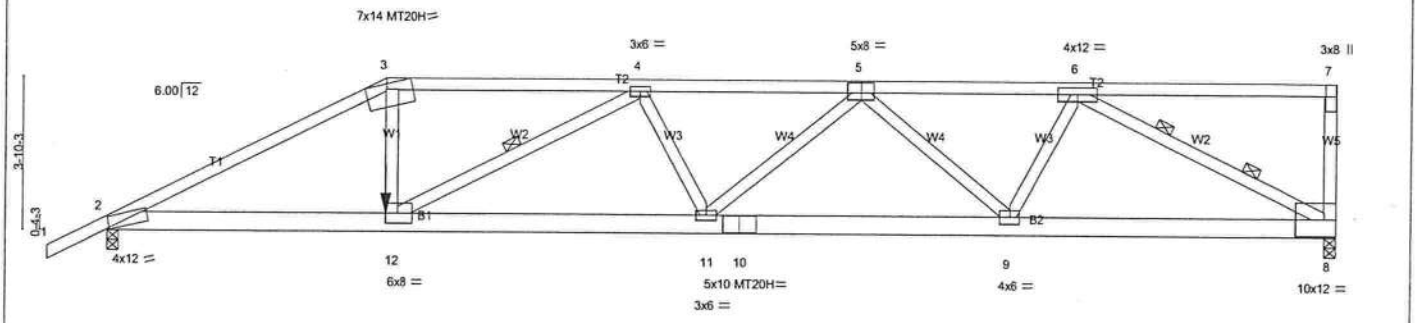
-1-6-0	7-0-0	13-5-11	19-0-0	24-6-4	31-0-0
1-6-0	7-0-0	6-5-11	5-6-5	5-6-5	6-5-12

Scale = 1:55.8



7-0-0	15-2-2	22-9-13	31-0-0
7-0-0	8-2-2	7-7-11	8-2-3

Plate Offsets (X,Y): [2:0-0-13,Edge], [3:0-6-3,Edge], [5:0-4-0,0-3-0], [12:0-3-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	1.00	Vert(LL)	0.41	11	>890	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.82	Vert(TL)	-0.66	11-12	>558	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.80	Horz(TL)	0.15	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 177 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W5 2 X 4 SYP No.2, W2 2 X 4 SYP No.2, W2 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-3-14 oc bracing.
WEBS 1 Row at midpt 4-12
2 Rows at 1/3 pts 6-8

REACTIONS (lb/size) 8=2852/0-3-8, 2=2709/0-3-8
Max Horz 2=257(load case 4)
Max Uplift8=-1660(load case 3), 2=-1467(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-5361/2945, 3-4=-4798/2725, 4-5=-6459/3671, 5-6=-4945/2781, 6-7=-182/118, 7-8=-372/367
BOT CHORD 2-12=-2691/4722, 11-12=-3764/6388, 10-11=-3562/6037, 9-10=-3562/6037, 8-9=-2506/4213
WEBS 3-12=-844/1796, 4-12=-1815/1249, 4-11=0/225, 5-11=-160/570, 5-9=-1475/1055, 6-9=-626/1664, 6-8=-4597/2723

JOINT STRESS INDEX
2 = 0.80, 3 = 0.92, 4 = 0.56, 5 = 0.80, 6 = 0.94, 7 = 0.80, 8 = 0.78, 9 = 0.94, 10 = 0.78, 11 = 0.45 and 12 = 0.49

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) All plates are MT20 plates unless otherwise indicated.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1660 lb uplift at joint 8 and 1467 lb uplift at joint 2.
6) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 356 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-7=-118(F=-64), 2-12=-56, 8-12=-65(F=-35)
Concentrated Loads (lb)
Vert: 12=-539(F)

JANUARY 31,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0131071200

Job	Truss	Truss Type	Qty	Ply	G & J BLDRS. - LOT 17 CCP
L223413	T10	MONO HIP	1	1	Job Reference (optional)

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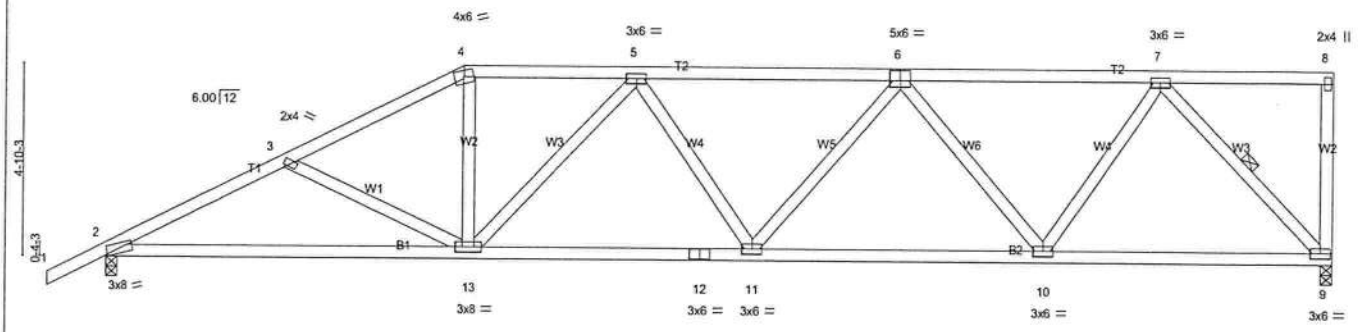
-1-6-0	4-7-15	9-0-0	13-4-11	20-0-2	26-7-5	31-0-0
1-6-0	4-7-15	4-4-1	4-4-11	6-7-7	6-7-3	4-4-11

Scale = 1:55.8



9-0-0	16-3-12	23-8-5	31-0-0
9-0-0	7-3-12	7-4-9	7-3-11

Plate Offsets (X,Y): [2:0-0-10,Edge], [6:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	-0.21	2-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.60	Vert(TL)	-0.35	2-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.45	Horz(TL)	0.09	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 166 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-3-4 oc bracing.
WEBS 1 Row at midpt 7-9

REACTIONS (lb/size) 9=1287/0-3-8, 2=1381/0-3-8
Max Horz 2=310(load case 5)
Max Uplift9=-620(load case 4), 2=-605(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2349/926, 3-4=-2097/862, 4-5=-1846/811, 5-6=-2157/987, 6-7=-1556/712, 7-8=-31/3, 8-9=-94/74
BOT CHORD 2-13=-948/2054, 12-13=-1017/2143, 11-12=-1017/2143, 10-11=-983/2009, 9-10=-528/1050
WEBS 3-13=-257/274, 4-13=-230/690, 5-13=-435/350, 5-11=0/95, 6-11=-52/234, 6-10=-721/432, 7-10=-341/939, 7-9=-1495/769

JOINT STRESS INDEX
2 = 0.80, 3 = 0.34, 4 = 0.46, 5 = 0.42, 6 = 0.64, 7 = 0.71, 8 = 0.54, 9 = 0.53, 10 = 0.71, 11 = 0.42, 12 = 0.71 and 13 = 0.57

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 620 lb uplift at joint 9 and 605 lb uplift at joint 2.

LOAD CASE(S) Standard

JANUARY 31,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0131071201

Job	Truss	Truss Type	Qty	Ply	G & J BLDRS. - LOT 17 CCP
L223413	T11	HIP	1	1	Job Reference (optional)

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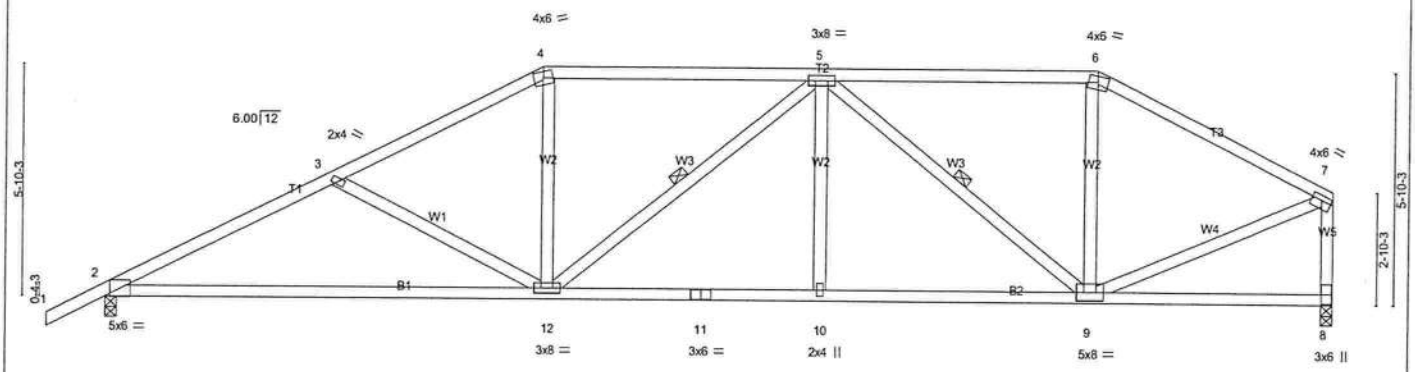
-1-6-0 | 5-9-14 | 11-0-0 | 18-0-0 | 25-0-0 | 31-0-0

1-6-0 | 5-9-14 | 5-2-2 | 7-0-0 | 7-0-0 | 6-0-0

Scale = 1:55.8



11-0-0 | 18-0-0 | 25-0-0 | 31-0-0

11-0-0 | 7-0-0 | 7-0-0 | 6-0-0

Plate Offsets (X,Y): [2:0-1-11,Edge], [7:0-3-0,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.38	2-12	>971	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.80	Vert(TL)	-0.65	2-12	>569	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.38	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 168 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-7-4 oc bracing.
WEBS 1 Row at midpt 5-12, 5-9

REACTIONS (lb/size) 2=1381/0-3-8, 8=1287/0-3-8
Max Horz 2=251(load case 5)
Max Uplift2=-634(load case 5), 8=-462(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2274/912, 3-4=-1958/757, 4-5=-1706/733, 5-6=-1171/546, 6-7=-1367/545, 7-8=-1203/499
BOT CHORD 2-12=-913/1989, 11-12=-736/1784, 10-11=-736/1784, 9-10=-736/1784, 8-9=-56/74
WEBS 3-12=-337/345, 4-12=-93/522, 5-12=-233/208, 5-10=0/172, 5-9=-842/404, 6-9=-27/293, 7-9=-463/1190

JOINT STRESS INDEX
2 = 0.71, 3 = 0.34, 4 = 0.75, 5 = 0.57, 6 = 0.80, 7 = 0.80, 8 = 0.29, 9 = 0.54, 10 = 0.34, 11 = 0.62 and 12 = 0.57

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 634 lb uplift at joint 2 and 462 lb uplift at joint 8.

LOAD CASE(S) Standard

JANUARY 31,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0131071202

Job	Truss	Truss Type	Qty	Ply	G & J BLDRS. - LOT 17 CCP
L223413	T12	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 18 10:54:02 2007 Page 1

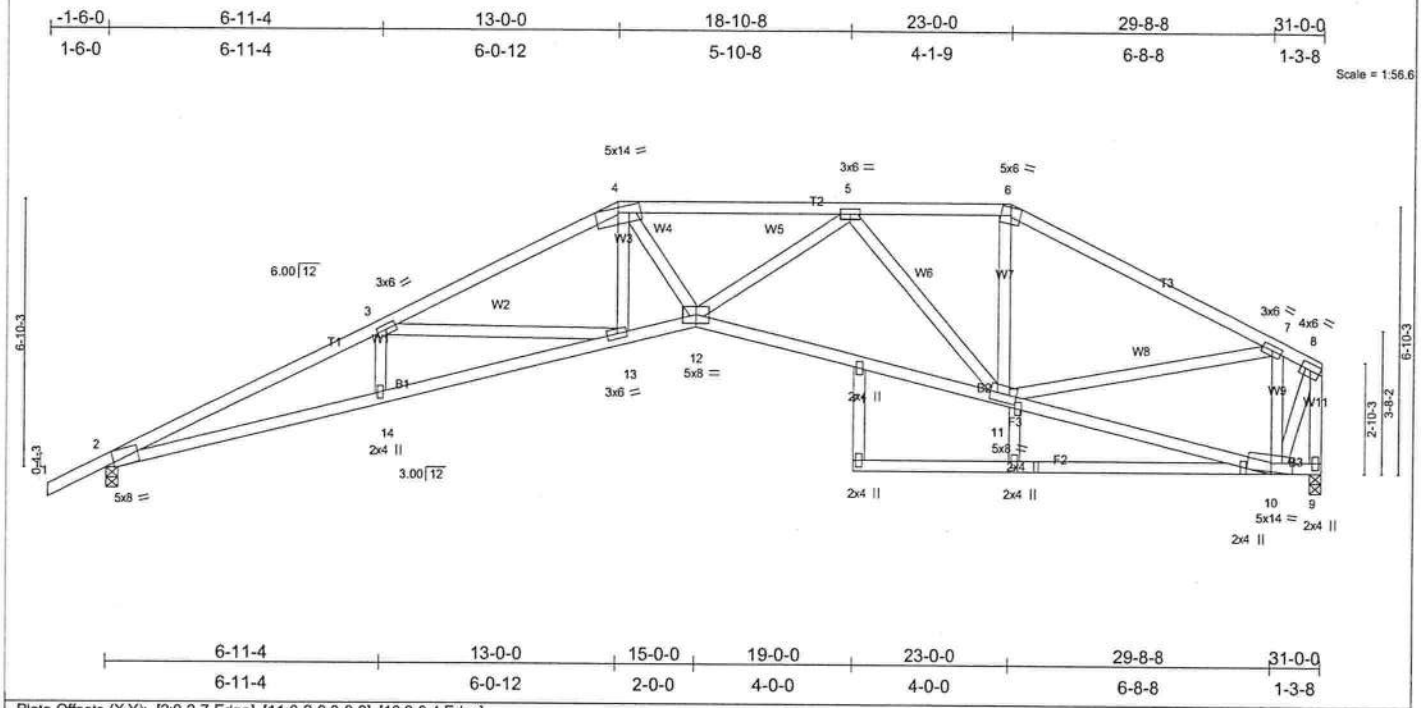


Plate Offsets (X,Y): [2:0-2-7,Edge], [11:0-2-0,0-0-0], [16:0-0-4,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.66	Vert(LL)	-0.42	13-14	>872	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.94	Vert(TL)	-0.68	13-14	>542	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.89	Horz(TL)	0.43	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 186 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-5-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

REACTIONS (lb/size) 2=1381/0-3-8, 9=1287/0-3-8
Max Horz 2=268(load case 5)
Max Uplift2=-651(load case 5), 9=-486(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-4352/1803, 3-4=-3321/1310, 4-5=-3440/1359, 5-6=-1640/719, 6-7=-1901/731, 7-8=-541/185, 8-9=-1298/452
BOT CHORD 2-14=-1762/3931, 13-14=-1763/3926, 12-13=-1128/3020, 11-12=-908/2538, 10-11=-221/525, 9-10=-10/29
WEBS 3-14=0/208, 3-13=-895/616, 4-13=-180/368, 4-12=-265/907, 5-12=-469/1249, 5-11=-1287/598, 6-11=-148/550, 7-11=-392/1164, 7-10=-1142/646, 8-10=-582/1262

JOINT STRESS INDEX
2 = 0.84, 3 = 0.41, 4 = 0.80, 5 = 0.73, 6 = 0.60, 7 = 0.59, 8 = 0.77, 9 = 0.58, 10 = 0.85, 11 = 0.50, 11 = 0.48, 12 = 0.94, 13 = 0.38, 14 = 0.34, 15 = 0.34, 16 = 0.36, 17 = 0.34 and 18 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 651 lb uplift at joint 2 and 486 lb uplift at joint 9.

LOAD CASE(S) Standard

JANUARY 31,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

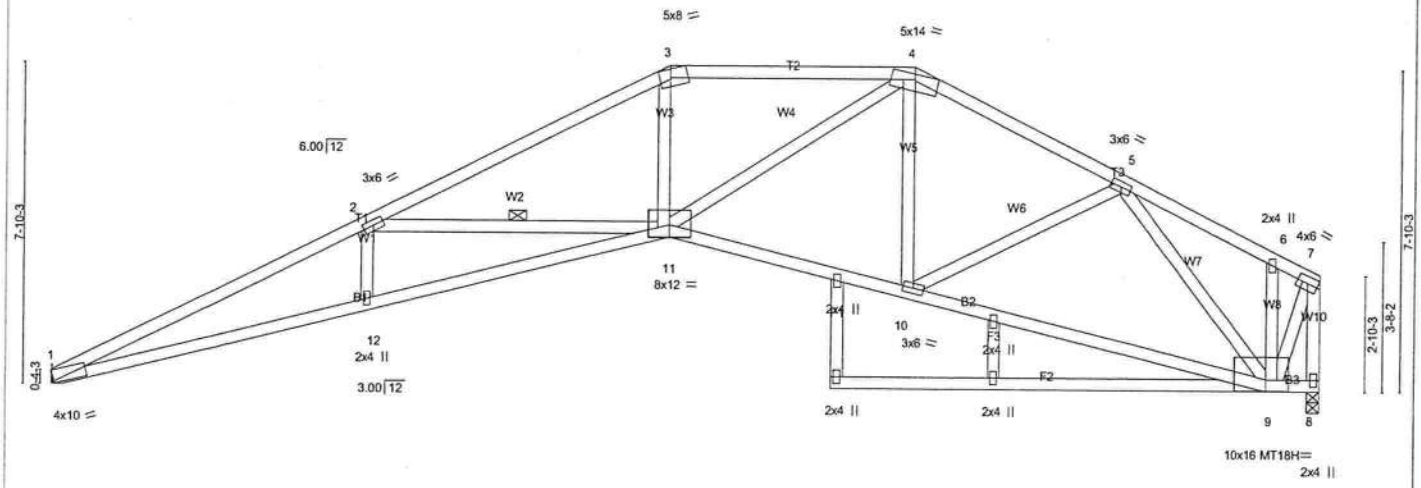
Dwg.#0131071203

Job	Truss	Truss Type	Qty	Ply	G & J BLDRS. - LOT 17 CCP
L223413	T13	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 18 10:54:09 2007 Page 1

7-7-12	15-0-0	21-0-0	26-1-0	29-8-8	31-0-0
7-7-12	7-4-4	6-0-0	5-1-0	3-7-8	1-3-8

Scale = 1:54.1



7-7-12	15-0-0	19-0-0	21-0-0	29-8-8	31-0-0
7-7-12	7-4-4	4-0-0	2-0-0	8-8-8	1-3-8

Plate Offsets (X,Y): [1:0-0-14,Edge], [3:0-3-3,Edge], [9:0-6-12,0-3-4], [11:0-6-0,0-3-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.89	Vert(LL)	-0.41	11-12	>894	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.79	Vert(TL)	-0.67	11-12	>554	180	MT18H	244/190
BCLL	10.0	Rep Stress Incr	YES	WB	0.91	Horz(TL)	0.39	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 185 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-7-8 oc bracing. Except:
1 Row at midpt 9-10
WEBS 1 Row at midpt 2-11

REACTIONS (lb/size) 1=1293/Mechanical, 8=1293/0-3-8
Max Horz 1=233(load case 5)
Max Uplift1=-549(load case 5), 8=-508(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4411/1947, 2-3=-3050/1246, 3-4=-2645/1198, 4-5=-1930/776, 5-6=-573/258, 6-7=-540/176, 7-8=-1431/439
BOT CHORD 1-12=-1914/3998, 11-12=-1910/3989, 10-11=-560/1741, 9-10=-495/1341, 8-9=-15/26
WEBS 2-12=0/259, 2-11=-1207/842, 3-11=-278/959, 4-11=-555/1173, 4-10=-118/152, 5-10=-156/504, 5-9=-1321/548, 6-9=-136/151, 7-9=-382/1259

JOINT STRESS INDEX
1 = 0.67, 2 = 0.41, 3 = 0.74, 4 = 0.77, 5 = 0.44, 6 = 0.34, 7 = 0.76, 8 = 0.56, 9 = 0.18, 10 = 0.38, 11 = 0.74, 12 = 0.34, 13 = 0.34, 14 = 0.34, 15 = 0.34 and 16 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 549 lb uplift at joint 1 and 508 lb uplift at joint 8.

LOAD CASE(S) Standard

JANUARY 31,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0131071204

Job	Truss	Truss Type	Qty	Ply	G & J BLDRS. - LOT 17 CCP
L223413	T14	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 18 10:54:16 2007 Page 1

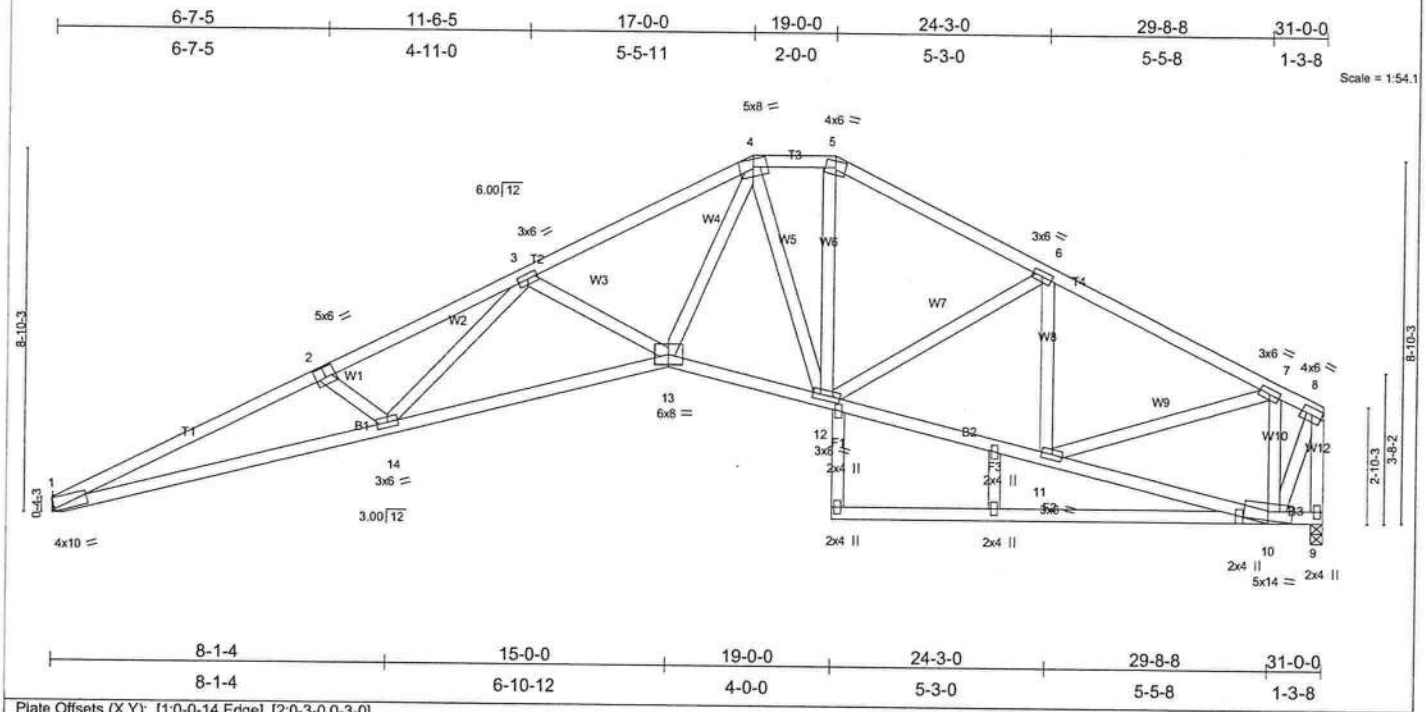


Plate Offsets (X,Y): [1:0-0-14,Edge], [2:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.64	Vert(LL)	-0.40	13-14	>929	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.78	Vert(TL)	-0.64	13-14	>577	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.60	Horz(TL)	0.37	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 196 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-4-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-6-7 oc bracing.
JOINTS 1 Brace at Jt(s): 11

REACTIONS (lb/size) 1=1293/Mechanical, 9=1293/0-3-8
Max Horz 1=250(load case 5)
Max Uplift1=-561(load case 5), 9=-526(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4383/2048, 2-3=-4147/1949, 3-4=-2936/1317, 4-5=-1649/810, 5-6=-1894/855, 6-7=-1768/719, 7-8=-542/211, 8-9=-1278/496
BOT CHORD 1-14=-2032/3992, 13-14=-1503/3239, 12-13=-719/1924, 11-12=-576/1589, 10-11=-202/484, 9-10=-13/34
WEBS 2-14=-255/317, 3-14=-408/817, 3-13=-655/516, 4-13=-831/1870, 4-12=-732/387, 5-12=-304/632, 6-12=-70/226, 6-11=-431/253, 7-11=-405/1124, 7-10=-1078/570, 8-10=-513/1158

JOINT STRESS INDEX
1 = 0.67, 2 = 0.67, 3 = 0.53, 4 = 0.77, 5 = 0.51, 6 = 0.41, 7 = 0.60, 8 = 0.70, 9 = 0.61, 10 = 0.78, 10 = 0.36, 11 = 0.61, 12 = 0.65, 12 = 0.34, 13 = 0.77, 14 = 0.51, 15 = 0.34, 16 = 0.34 and 17 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 561 lb uplift at joint 1 and 526 lb uplift at joint 9.

LOAD CASE(S) Standard

JANUARY 31,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0131071206

Job	Truss	Truss Type	Qty	Ply	G & J BLDRS. - LOT 17 CCP
L223413	T16	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 18 10:54:30 2007 Page 1

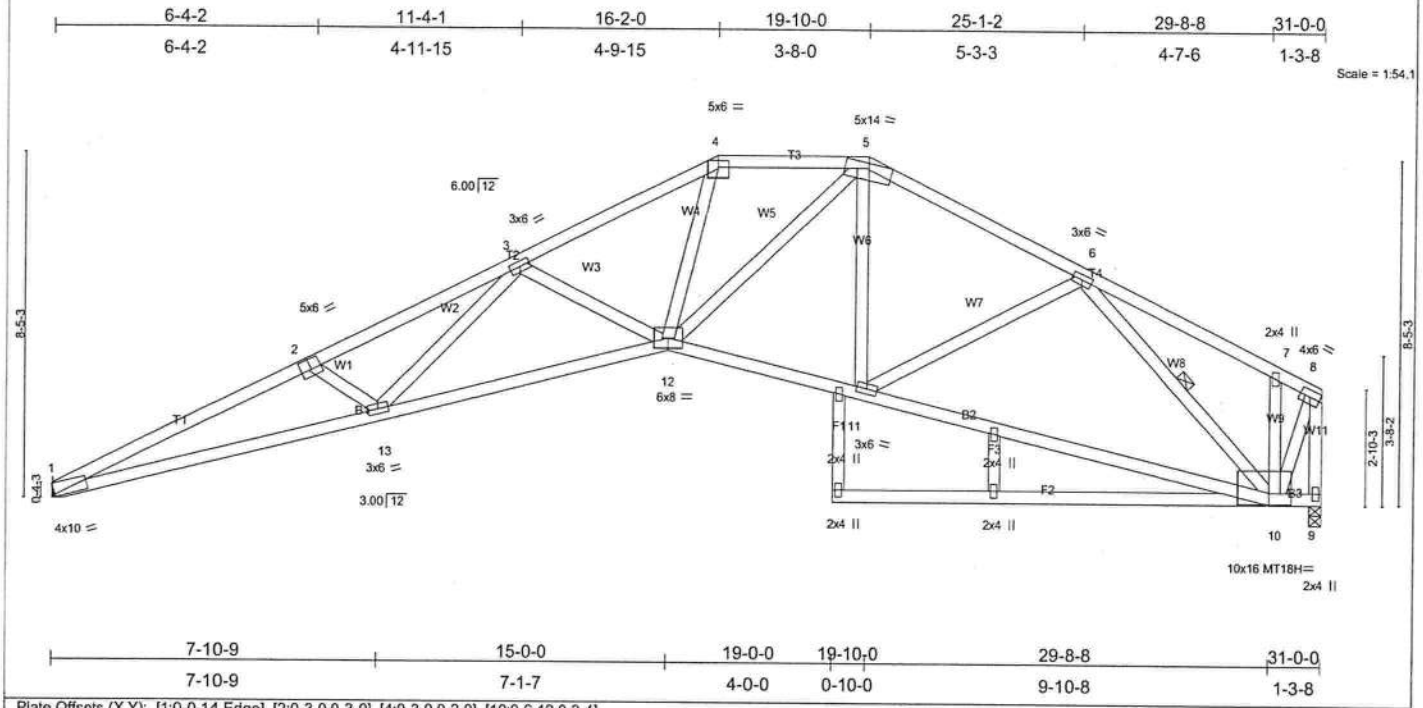


Plate Offsets (X,Y): [1:0-0-14,Edge], [2:0-3-0,0-3-0], [4:0-3-0,0-2-0], [10:0-6-12,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	-0.42	12-13	>883	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	1.00	Vert(TL)	-0.68	12-13	>547	180	MT18H	244/190
BCLL	10.0	Rep Stress Incr	YES	WB	0.44	Horz(TL)	0.39	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 190 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-5-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied. Except:
1 Row at midpt 10-11
WEBS 1 Row at midpt 6-10

REACTIONS (lb/size) 1=1293/Mechanical, 9=1293/0-3-8
Max Horz 1=243(load case 5)
Max Uplift1=-556(load case 5), 9=-519(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4415/2045, 2-3=-4167/1919, 3-4=-2945/1291, 4-5=-2296/1102, 5-6=-1934/818, 6-7=-622/294, 7-8=-556/171, 8-9=-1502/437
BOT CHORD 1-13=-2028/4021, 12-13=-1490/3261, 11-12=-576/1732, 10-11=-549/1468, 9-10=-15/22
WEBS 2-13=-256/325, 3-13=-384/800, 3-12=-645/508, 4-12=-424/1112, 5-12=-452/888, 5-11=-33/110, 6-11=-88/395, 6-10=-1349/580, 7-10=-199/235, 8-10=-422/1360

JOINT STRESS INDEX
1 = 0.68, 2 = 0.70, 3 = 0.52, 4 = 0.62, 5 = 0.50, 6 = 0.44, 7 = 0.34, 8 = 0.83, 9 = 0.55, 10 = 0.24, 11 = 0.38, 12 = 0.77, 13 = 0.50, 14 = 0.34, 15 = 0.34, 16 = 0.34 and 17 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 556 lb uplift at joint 1 and 519 lb uplift at joint 9.

LOAD CASE(S) Standard

JANUARY 31,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0131071207

Job	Truss	Truss Type	Qty	Ply	G & J BLDRS. - LOT 17 CCP
L223413	T17	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 18 10:54:37 2007 Page 1

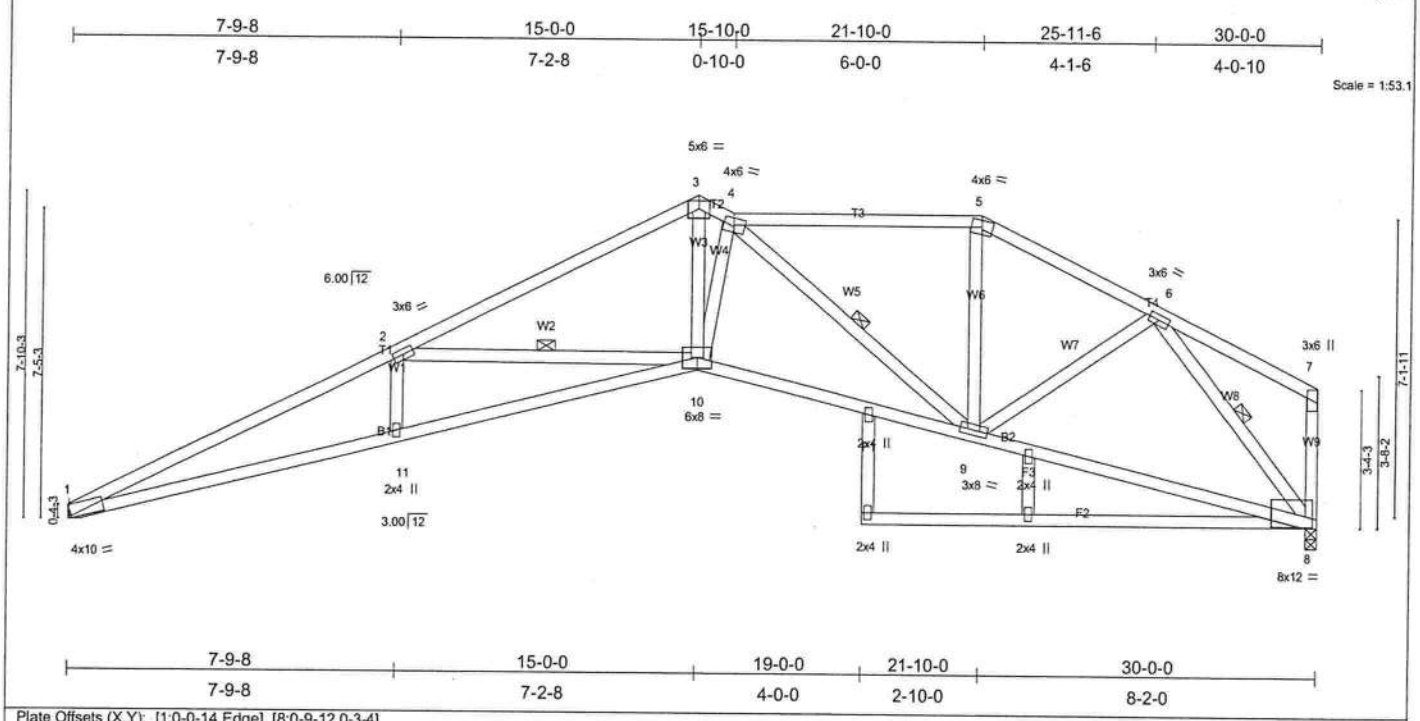


Plate Offsets (X,Y): [1:0-0-14,Edge], [8:0-9-12,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.89	Vert(LL)	-0.39	10-11	>912	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.78	Vert(TL)	-0.63	10-11	>566	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.65	Horz(TL)	0.39	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 179 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D *Except*
F2 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-7-12 oc bracing.
WEBS 1 Row at midpt 2-10, 4-9, 6-8

REACTIONS (lb/size) 1=1251/Mechanical, 8=1251/0-3-8
Max Horz 1=252(load case 5)
Max Uplift1=-532(load case 5), 8=-547(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4212/1880, 2-3=-2880/1205, 3-4=-2687/1182, 4-5=-1506/755, 5-6=-1708/776, 6-7=-135/74, 7-8=-155/114
BOT CHORD 1-11=-1870/3814, 10-11=-1865/3804, 9-10=-1024/2654, 8-9=-451/1050
WEBS 2-11=0/260, 2-10=-1198/814, 3-10=-789/2023, 4-10=-449/276, 4-9=-1394/639, 5-9=-131/419, 6-9=-187/609, 6-8=-1528/704

JOINT STRESS INDEX
1 = 0.64, 2 = 0.41, 3 = 0.71, 4 = 0.75, 5 = 0.74, 6 = 0.48, 7 = 0.28, 8 = 0.35, 9 = 0.73, 10 = 0.90, 11 = 0.34, 12 = 0.34, 13 = 0.34, 14 = 0.34 and 15 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 532 lb uplift at joint 1 and 547 lb uplift at joint 8.

LOAD CASE(S) Standard

JANUARY 31,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0131071208

Job	Truss	Truss Type	Qty	Ply	G & J BLDRS. - LOT 17 CCP
L223413	T18	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 18 10:54:44 2007 Page 1

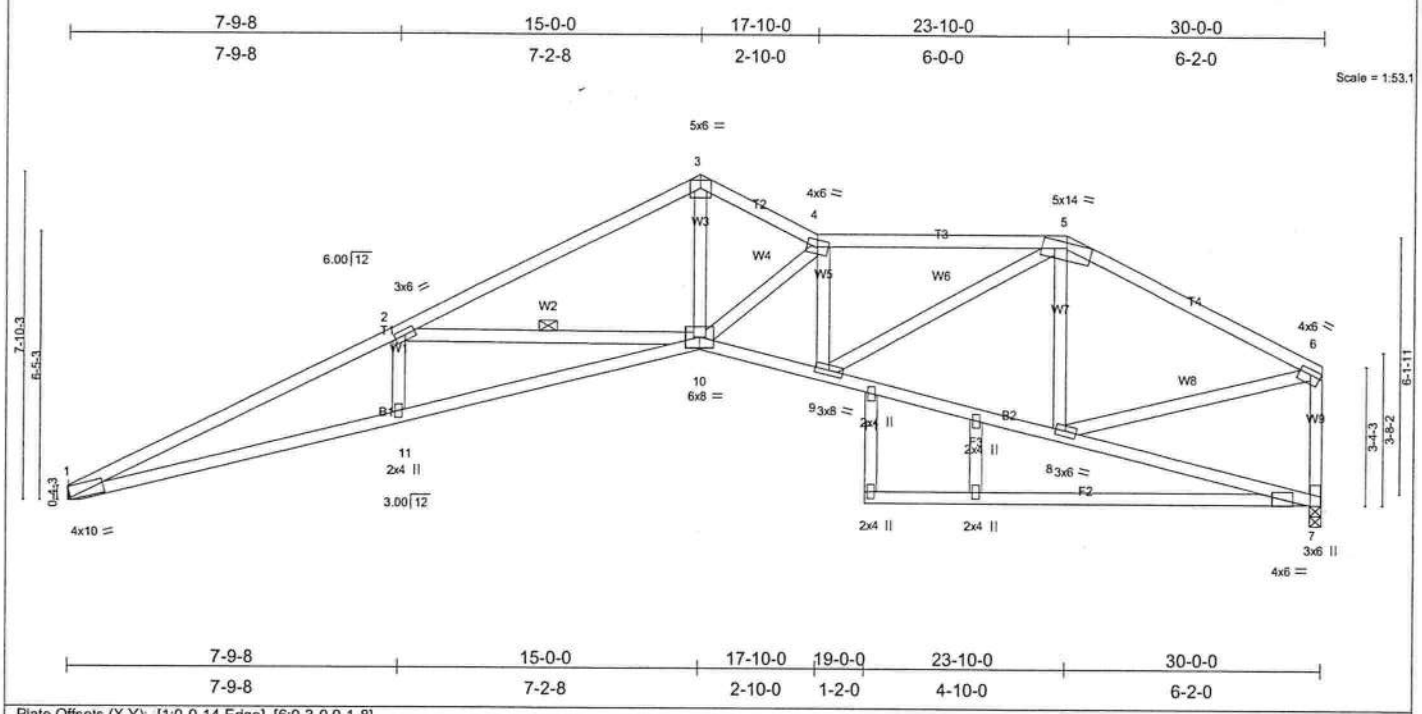


Plate Offsets (X,Y): [1:0-0-14,Edge], [6:0-3-0,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.89	Vert(LL)	-0.42	10-11	>858	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.77	Vert(TL)	-0.67	10-11	>532	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.69	Horz(TL)	0.40	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 174 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D *Except*
F2 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-7-12 oc bracing.
WEBS 1 Row at midpt 2-10
JOINTS 1 Brace at Jt(s): 8

REACTIONS (lb/size) 1=1251/Mechanical, 7=1251/0-3-8
Max Horz 1=252(load case 5)
Max Uplift1=-532(load case 5), 7=-547(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4220/1884, 2-3=-2864/1197, 3-4=-2782/1234, 4-5=-2809/1215, 5-6=-1603/700, 6-7=-1177/569
BOT CHORD 1-11=-1874/3822, 10-11=-1870/3812, 9-10=-1135/2961, 8-9=-548/1414, 7-8=-47/89
WEBS 2-11=0/262, 2-10=-1222/829, 3-10=-825/2166, 4-10=-563/345, 4-9=-1072/508, 5-9=-686/1652, 5-8=-468/294, 6-8=-509/1353

JOINT STRESS INDEX
1 = 0.64, 2 = 0.41, 3 = 0.76, 4 = 0.66, 5 = 0.97, 6 = 0.73, 7 = 0.33, 8 = 0.73, 9 = 0.93, 10 = 0.90, 11 = 0.34, 12 = 0.34, 13 = 0.12, 14 = 0.34, 15 = 0.34 and 16 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 532 lb uplift at joint 1 and 547 lb uplift at joint 7.

LOAD CASE(S) Standard

JANUARY 31,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

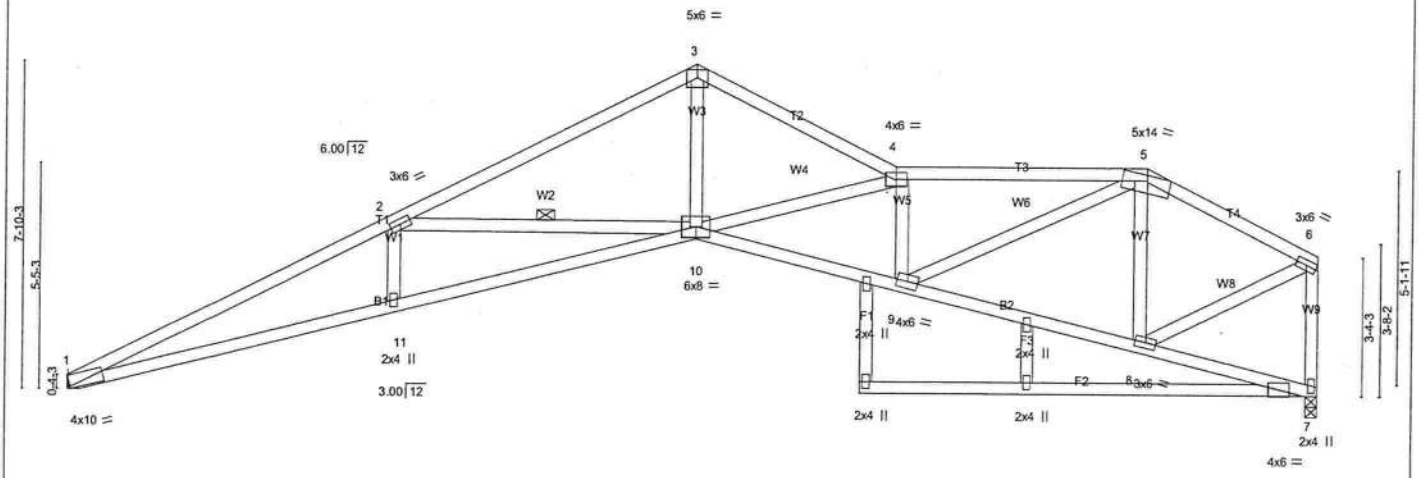
Dwg.#0131071209

Job	Truss	Truss Type	Qty	Ply	G & J BLDRS. - LOT 17 CCP
L223413	T19	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 18 10:54:51 2007 Page 1

7-9-8	15-0-0	19-10-0	25-10-0	30-0-0
7-9-8	7-2-8	4-10-0	6-0-0	4-2-0

Scale = 1:53.1



7-9-8	15-0-0	19-0-0	19-10-0	25-10-0	30-0-0
7-9-8	7-2-8	4-0-0	0-10-0	6-0-0	4-2-0

Plate Offsets (X,Y): [1:0-0-14,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.89	Vert(LL)	-0.43	10-11	>826	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.78	Vert(TL)	-0.70	10-11	>513	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.70	Horz(TL)	0.43	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 172 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D *Except*
F2 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-7-12 oc bracing.
WEBS 1 Row at midpt 2-10
JOINTS 1 Brace at Jt(s): 8

REACTIONS (lb/size) 1=1251/Mechanical, 7=1251/0-3-8
Max Horz 1=252(load case 5)
Max Uplift1=-532(load case 5), 7=-547(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4217/1884, 2-3=-2870/1197, 3-4=-2825/1231, 4-5=-3094/1346, 5-6=-1301/578, 6-7=-1207/558
BOT CHORD 1-11=-1874/3819, 10-11=-1870/3810, 9-10=-1309/3283, 8-9=-468/1161, 7-8=-16/37
WEBS 2-11=0/259, 2-10=-1224/829, 3-10=-820/2144, 4-10=-763/475, 4-9=-1293/607, 5-9=-871/2185, 5-8=-662/367, 6-8=-503/1248

JOINT STRESS INDEX
1 = 0.64, 2 = 0.41, 3 = 0.75, 4 = 0.91, 5 = 0.76, 6 = 0.76, 7 = 0.50, 8 = 0.70, 9 = 0.84, 10 = 0.90, 11 = 0.34, 12 = 0.34, 13 = 0.12, 14 = 0.34, 15 = 0.34 and 16 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 532 lb uplift at joint 1 and 547 lb uplift at joint 7.

LOAD CASE(S) Standard

JANUARY 31,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0131071210

Job	Truss	Truss Type	Qty	Ply	G & J BLDRS. - LOT 17 CCP
L223413	T20	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 18 10:54:57 2007 Page 1

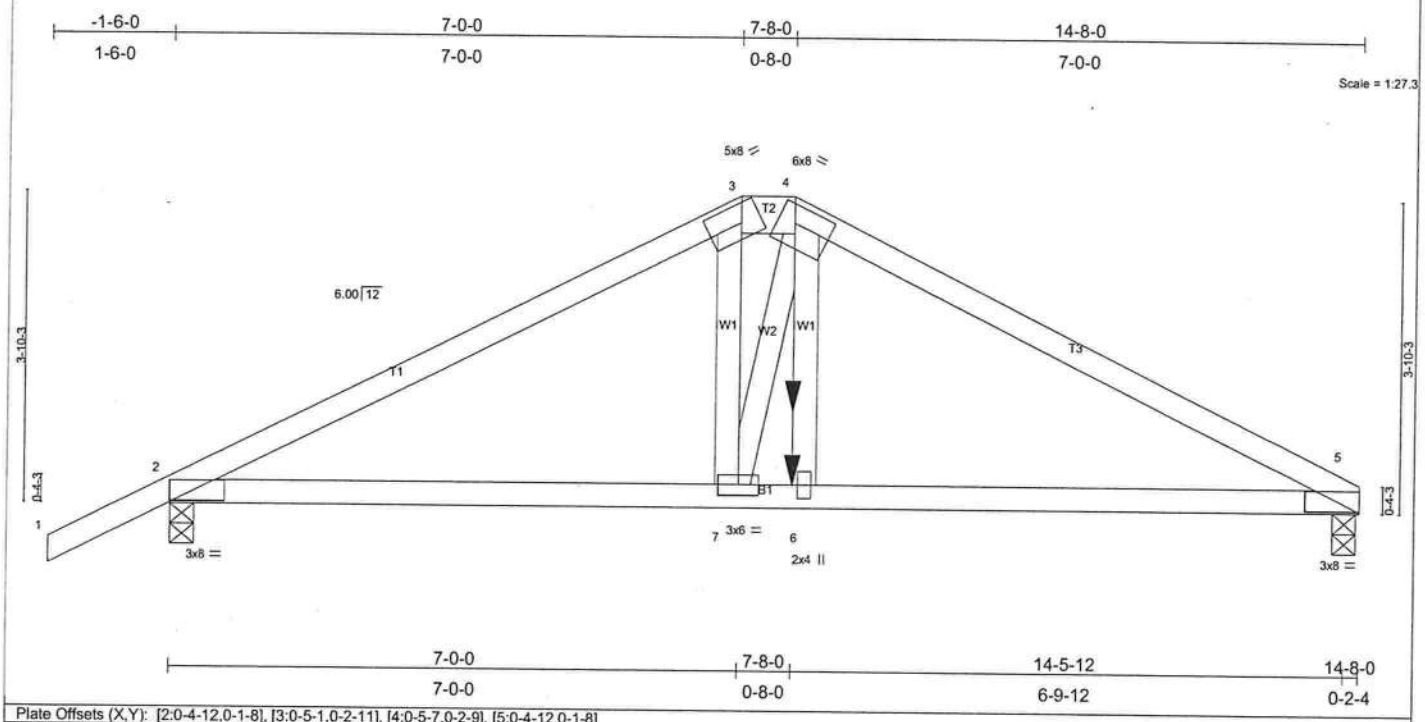


Plate Offsets (X,Y): [2:0-4-12,0-1-8], [3:0-5-1,0-2-11], [4:0-5-7,0-2-9], [5:0-4-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	0.17	5-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.70	Vert(TL)	-0.19	5-6	>921	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.31	Horz(TL)	0.04	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 64 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except* T2 2 X 6 SYP No.1D
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-5-1 oc bracing.

REACTIONS (lb/size) 5=1176/0-3-8, 2=1275/0-3-8
Max Horz 2=115(load case 4)
Max Uplift5=-850(load case 5), 2=-975(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2151/1459, 3-4=-1822/1367, 4-5=-2149/1462
BOT CHORD 2-7=-1244/1850, 6-7=-1196/1821, 5-6=-1216/1850
WEBS 3-7=-625/969, 4-7=-318/358, 4-6=-493/720

JOINT STRESS INDEX
2 = 0.80, 3 = 0.48, 4 = 0.58, 5 = 0.76, 6 = 0.53 and 7 = 0.63

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 850 lb uplift at joint 5 and 975 lb uplift at joint 2.
6) Girder carries hip end with 7-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 356 lb up at 7-8-0, and 539 lb down and 356 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

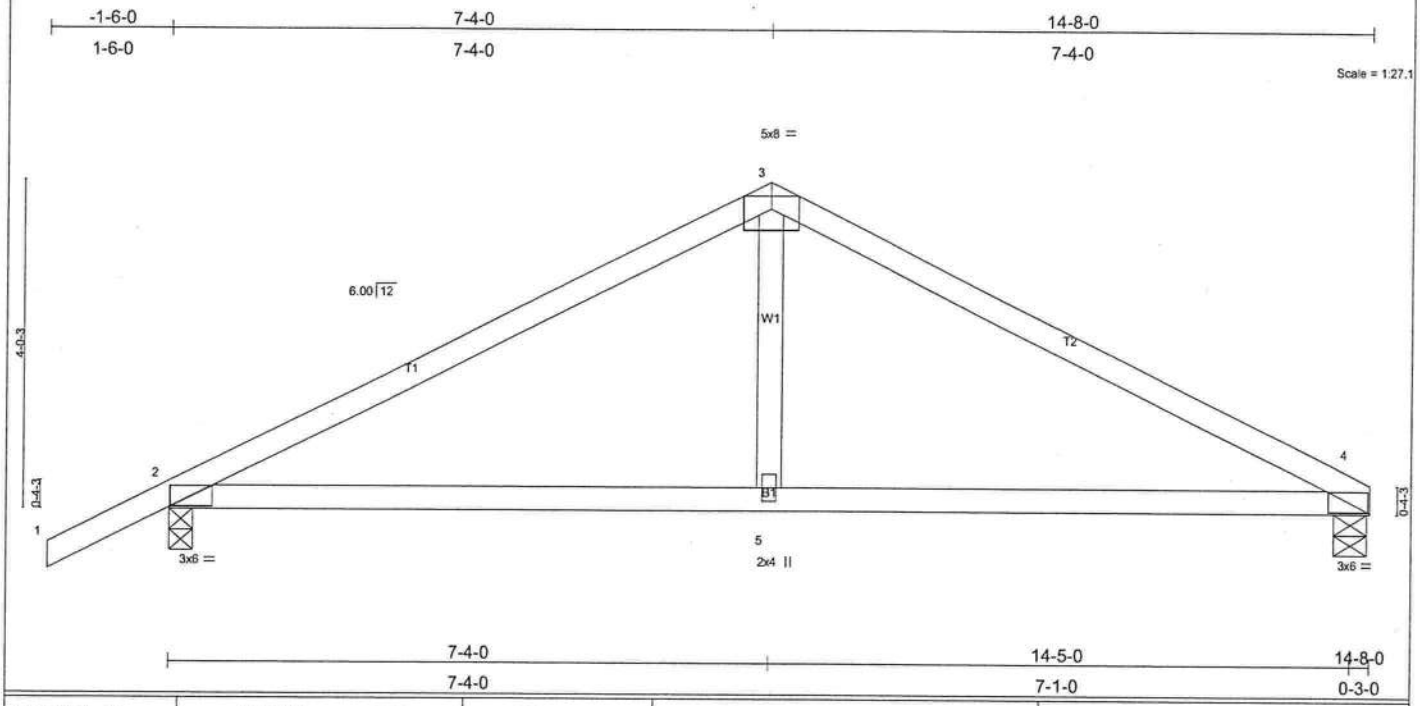
LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-118(F=-64), 4-5=-54, 2-7=-30, 6-7=-65(F=-35), 5-6=-30
Concentrated Loads (lb)
Vert: 7=-539(F) 6=-539(F)

JANUARY 31,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0131071211

Job	Truss	Truss Type	Qty	Ply	G & J BLDRS. - LOT 17 CCP
L223413	T21	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 18 10:55:04 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	0.24	4-5	>726	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.59	Vert(TL)	0.20	4-5	>854	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.14	Horz(TL)	-0.02	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 54 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-6-6 oc bracing.

REACTIONS (lb/size) 4=596/0-5-0, 2=695/0-3-8
Max Horz 2=119(load case 5)
Max Uplift4=-473(load case 6), 2=-599(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-870/975, 3-4=-869/972
BOT CHORD 2-5=-786/706, 4-5=-786/706
WEBS 3-5=-561/283

JOINT STRESS INDEX
2 = 0.69, 3 = 0.87, 4 = 0.69 and 5 = 0.21

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 473 lb uplift at joint 4 and 599 lb uplift at joint 2.

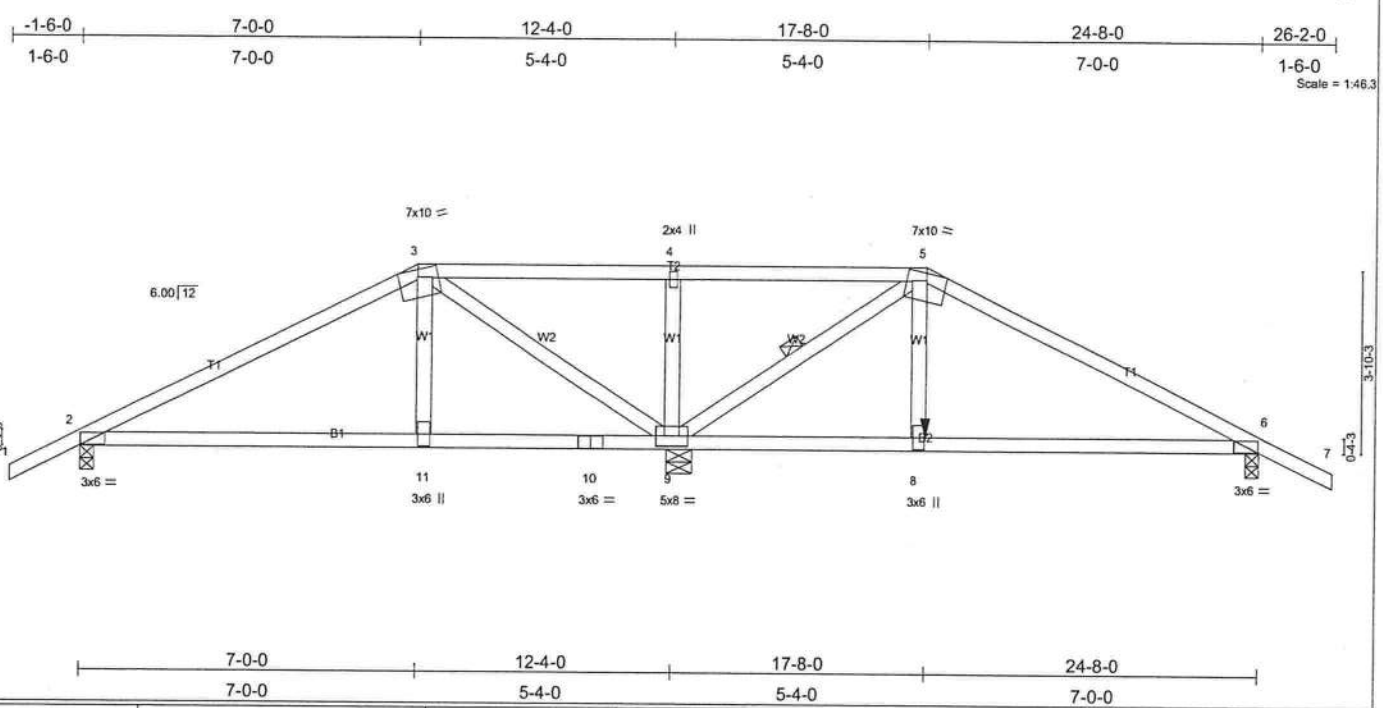
LOAD CASE(S) Standard

JANUARY 31,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0131071212

Job	Truss	Truss Type	Qty	Ply	G & J BLDRS. - LOT 17 CCP
L223413	T22	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 18 10:55:11 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	0.13	2-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.15	2-11	>952	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.54	Horz(TL)	0.02	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 113 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-7-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-8-12 oc bracing.
WEBS 1 Row at midpt 5-9

REACTIONS (lb/size) 2=455/0-3-8, 9=2033/0-6-7, 6=798/0-3-8
Max Horz 2=-95(load case 5)
Max Uplift2=-452(load case 4), 9=-1215(load case 3), 6=-507(load case 5)
Max Grav 2=465(load case 8), 9=2033(load case 1), 6=808(load case 9)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-342/265, 3-4=-179/433, 4-5=-180/433, 5-6=-1118/594, 6-7=0/35
BOT CHORD 2-11=-167/233, 10-11=-175/242, 9-10=-175/242, 8-9=-412/964, 6-8=-400/929
WEBS 3-11=-197/207, 3-9=-789/605, 4-9=-476/481, 5-9=-1656/913, 5-8=-300/847

JOINT STRESS INDEX
2 = 0.59, 3 = 0.88, 4 = 0.34, 5 = 0.88, 6 = 0.59, 8 = 0.28, 9 = 0.38, 10 = 0.46 and 11 = 0.28

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 452 lb uplift at joint 2, 1215 lb uplift at joint 9 and 507 lb uplift at joint 6.
6) Girder carries hip end with 7-0-0 right side setback, 12-4-0 left side setback, and 7-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 356 lb up at 17-8-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-54, 4-5=-118(F=-64), 5-7=-54, 2-9=-30, 8-9=-65(F=-35), 6-8=-30
Concentrated Loads (lb)
Vert: 8=-539(F)

JANUARY 31,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0131071213

Job	Truss	Truss Type	Qty	Ply	G & J BLDRS. - LOT 17 CCP
L223413	T23	MONO HIP	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 18 10:55:18 2007 Page 1

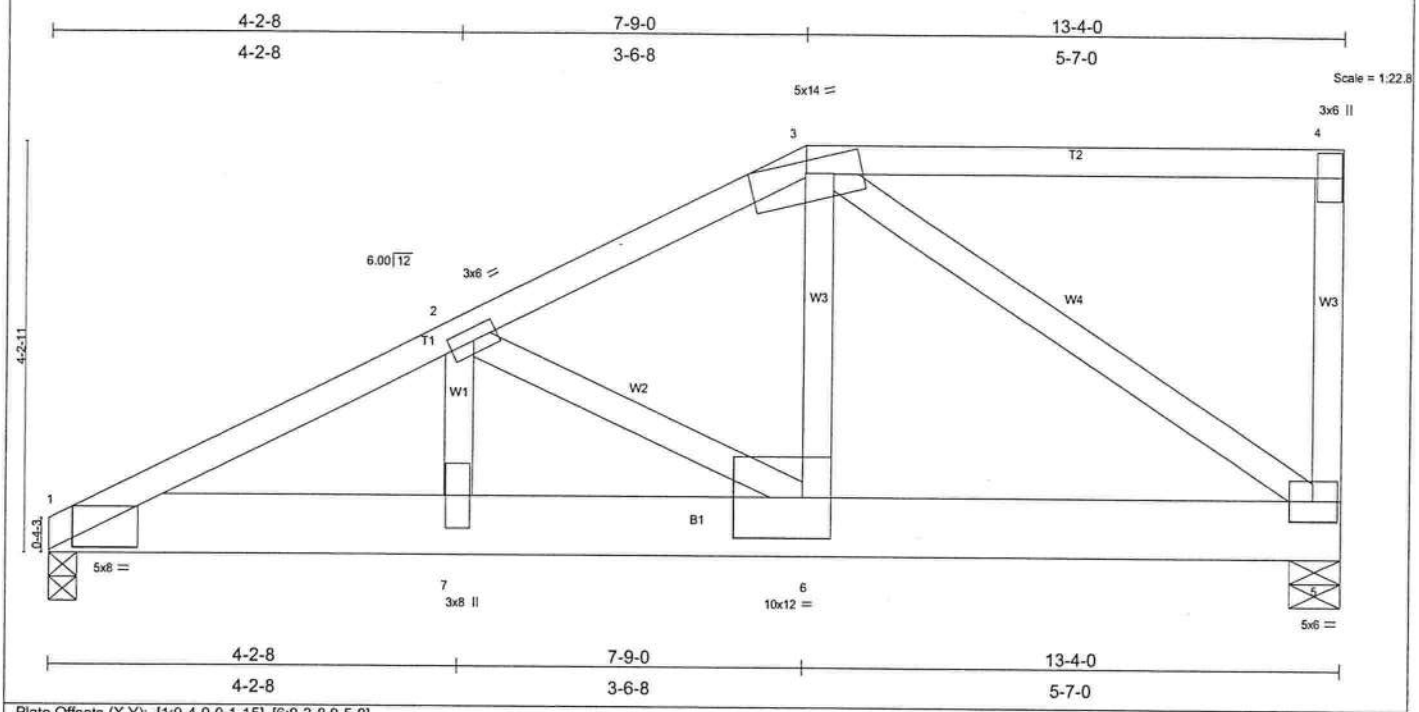


Plate Offsets (X,Y): [1:0-4-0,0-1-15], [6:0-3-8,0-5-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.07	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.33	Vert(TL)	-0.12	6-7	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.81	Horz(TL)	0.02	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 178 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3 *Except*
W3 2 X 4 SYP No.2, W3 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-11-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=4519/0-3-8, 5=4519/0-6-7
Max Horz 1=211(load case 4)
Max Uplift1=-2182(load case 4), 5=-2202(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-7009/3353, 2-3=-4528/2171, 3-4=-145/83, 4-5=-170/141
BOT CHORD 1-7=-3160/6262, 6-7=-3160/6262, 5-6=-2071/4202
WEBS 2-7=-958/2062, 2-6=-2542/1334, 3-6=-2197/4595, 3-5=-4936/2434

JOINT STRESS INDEX
1 = 0.78, 2 = 0.76, 3 = 0.87, 4 = 0.29, 5 = 0.61, 6 = 0.46 and 7 = 0.34

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
4) Provide adequate drainage to prevent water ponding.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2182 lb uplift at joint 1 and 2202 lb uplift at joint 5.
7) Girder carries tie-in span(s): 31-0-0 from 0-0-0 to 13-4-0

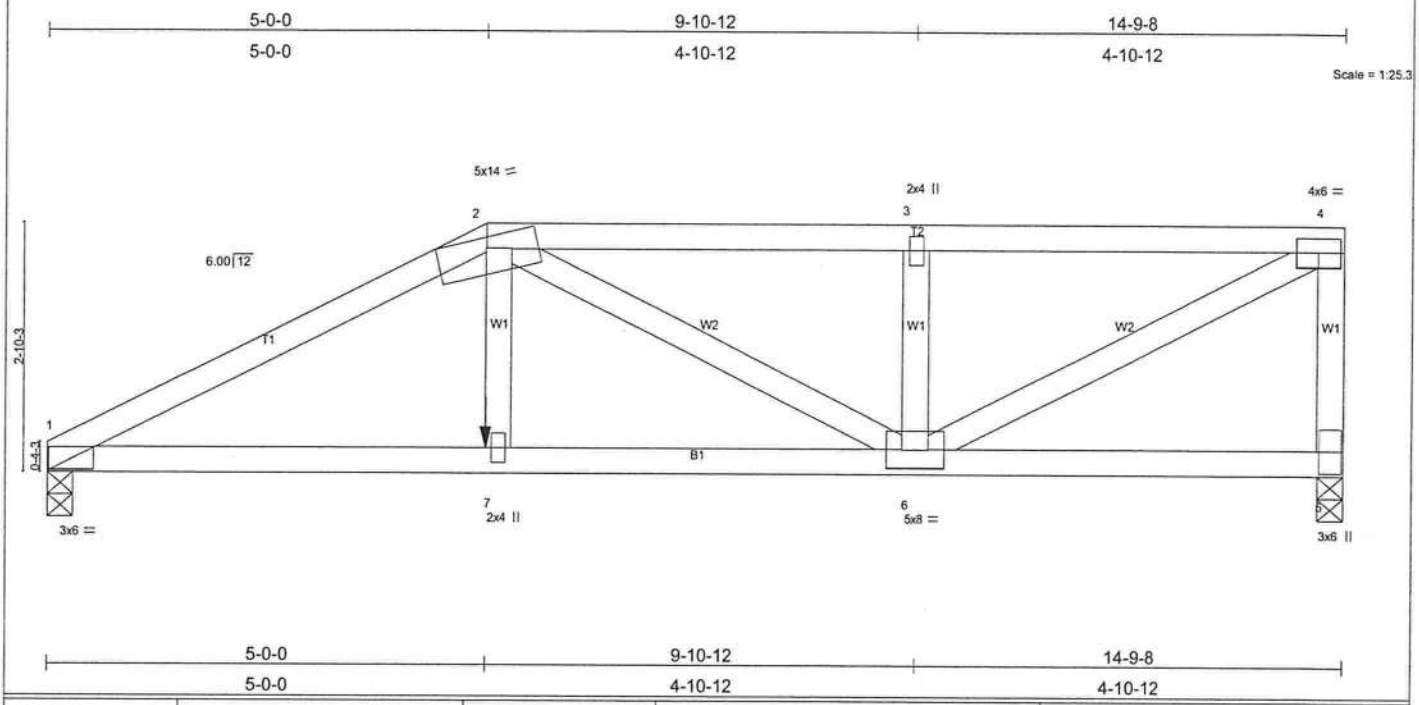
LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-54, 1-5=-639(F=-609)

JANUARY 31,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0131071214

Job	Truss	Truss Type	Qty	Ply	G & J BLDRS. - LOT 17 CCP
L223413	T24	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 18 10:55:24 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	0.08	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	-0.11	6-7	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.58	Horz(TL)	0.02	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 70 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-10-5 oc bracing.

REACTIONS (lb/size) 1=950/0-3-8, 5=1059/0-3-8
Max Horz 1=143(load case 4)
Max Uplift1=-681(load case 4), 5=-809(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1738/1211, 2-3=-1442/1096, 3-4=-1442/1096, 4-5=-933/750
BOT CHORD 1-7=-1120/1507, 6-7=-1136/1533, 5-6=-73/94
WEBS 2-7=-298/461, 2-6=-103/154, 3-6=-446/461, 4-6=-1163/1530

JOINT STRESS INDEX
1 = 0.73, 2 = 0.72, 3 = 0.26, 4 = 0.74, 5 = 0.39, 6 = 0.70 and 7 = 0.34

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp C; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 681 lb uplift at joint 1 and 809 lb uplift at joint 5.
5) Girder carries hip end with 0-0-0 right side setback, 5-0-0 left side setback, and 5-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 245 lb down and 162 lb up at 5-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-4=-91(F=-37), 1-7=-30, 5-7=-50(F=-20)
Concentrated Loads (lb)
Vert: 7=-245(F)

JANUARY 31,2007 TRUSS DESIGN ENGINEER:
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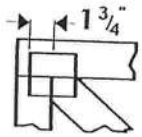
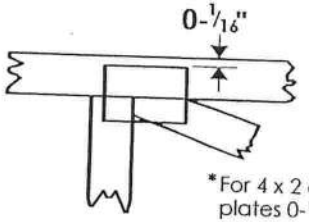
Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

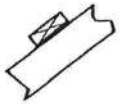
*This symbol indicates the required direction of slots in connector plates.

***Plate location details available in MiTek 20/20 software or upon request.**

PLATE SIZE

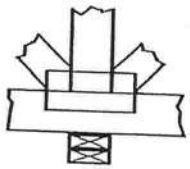
4 x 4 — The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

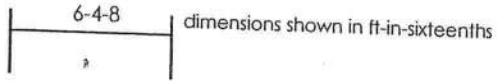


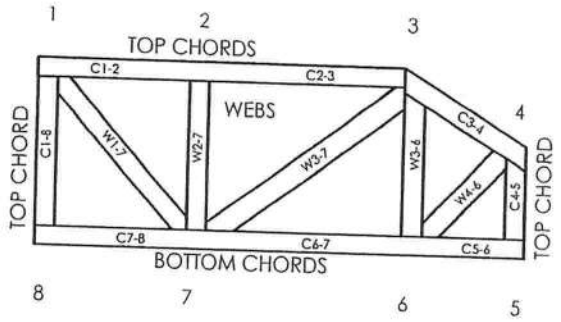
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI1: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System





JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 95-43, 96-20-1, 96-67, 84-32
ICBO	4922, 5243, 5363, 3907
SBCCI	9667, 9730, 9604B, 9511, 9432A

MiTek®

TEE-LOK®

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI1.
2. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
3. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
4. Cut members to bear tightly against each other.
5. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI1.
6. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI1.
7. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
8. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
9. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
10. Plate type, size, orientation and location dimensions shown indicate minimum plating requirements.
11. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
12. Top chords must be sheathed or purlins provided at spacing shown on design.
13. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
14. Connections not shown are the responsibility of others.
15. Do not cut or alter truss member or plate without prior approval of a professional engineer.
16. Install and load vertically unless indicated otherwise.

Patio

PRODUCT PERFORMANCE DATA

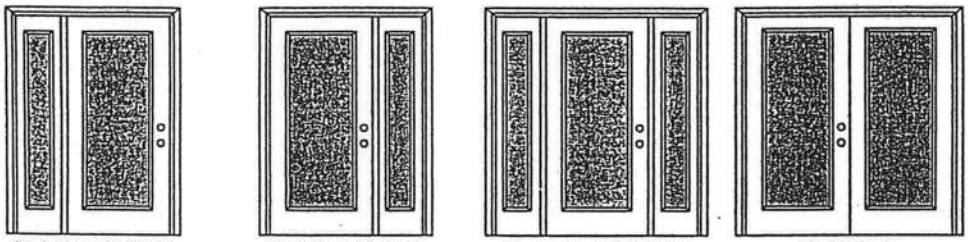
HURRICANE CODE

WOOD-EDGE STEEL DOOR IN WOOD FRAME

PRODUCT CERTIFICATION SHEET 01-0314.29 (DADE-OUTSWING)

Valid for the following side-hinged door arrangements (Sheet 1 of 2):

GLAZED DOORS - 6'8" NOMINAL HEIGHT:

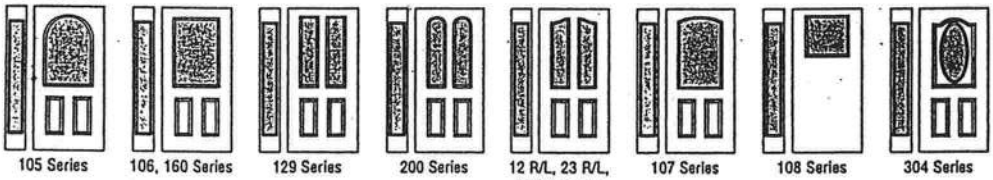


Single Door with Sidelite on Hinge side | Single Door with Sidelite on Strike side | Single Door with 2 Sidelites | Double Doors

DOOR STYLES - 1/4 GLASS:

100 Series | 133, 135 Series | 136 Series | 680 Series | 822 Series

1/2 GLASS:



105 Series | 106, 160 Series | 129 Series | 200 Series | 12 R/L, 23 R/L, 24 R/L Series | 107 Series | 108 Series | 304 Series

MULTI_WE_STL_WD_GLZ-1

Entergy Entry Systems

Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREM makes it!

FILE COPY

PRODUCT PERFORMANCE DATA

HURRICANE CODE

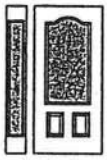
WOOD-EDGE STEEL DOOR IN WOOD FRAME

PRODUCT CERTIFICATION SHEET 01-0314.29 (DADE-OUTSWING)

Valid for the following side-hinged door arrangements (Sheet 2 of 2):

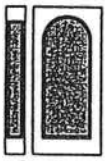
3/4 GLASS:

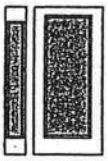

404 Series


410 Series


450 Series

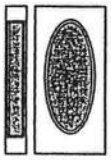
FULL GLASS:


109 Series

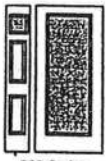

114, 120, 122 Series


152 Series

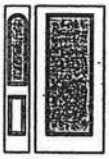

149 Series

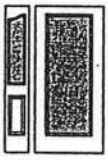

300 Series

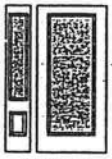
SIDELITE STYLES:

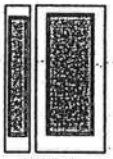

680 Series

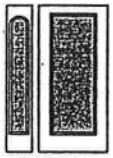

129 Series


200 Series


12R, 12L, 23R, 23L, 24R, 24L Series


450 Series


152 Series


149 Series

DP +50.5-50.5 IMPACT NO

MULTI_WE_STL_WD_GLZ-2

Entergy
Entry Systems

PREM makes it.

MIAMI-DADE

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

CONTRACTOR LICENSING SECTION
(305) 375-2527 FAX (305) 375-2558

CONTRACTOR ENFORCEMENT DIVISION
(305) 375-2966 FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 372-6339

PRODUCT CONTROL NOTICE OF ACCEPTANCE

Premdor Entry Systems
911 E. Jeferson, P.O. Box 76
Pittsburgh ,KS 66762

Your application for Notice of Acceptance (NOA) of:
Entergy 6-8 S-W/E Outswing Glazed Double w/sidelites Residential Insulated Steel Doors
under Chapter 8 of the Code of Miami-Dade County governing the use of Alternate Materials and Types of Construction, and completely described herein, has been recommended for acceptance by the Miami-Dade County Building Code Compliance Office (BCCO) under the conditions specified herein.

This NOA shall not be valid after the expiration date stated below. BCCO reserves the right to secure this product or material at any time from a jobsite or manufacturer's plant for quality control testing. If this product or material fails to perform in the approved manner, BCCO may revoke, modify, or suspend the use of such product or material immediately. BCCO reserves the right to revoke this approval, if it is determined by BCCO that this product or material fails to meet the requirements of the South Florida Building Code.

The expense of such testing will be incurred by the manufacturer.

ACCEPTANCE NO.: 01-0314.29
EXPIRES: 04/02/2006

Raul Rodriguez
Chief Product Control Division

THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL CONDITIONS
BUILDING CODE & PRODUCT REVIEW COMMITTEE

This application for Product Approval has been reviewed by the BCCO and approved by the Building Code and Product Review Committee to be used in Miami-Dade County, Florida under the conditions set forth above.

Francisco J. Quintana, R.A.
Director
Miami-Dade County
Building Code Compliance Office

APPROVED: 06/05/2001

Premdor Entry Systems

ACCEPTANCE No.: 01-0314.29

APPROVED : JUN 05 2001

EXPIRES : April 02, 2006

NOTICE OF ACCEPTANCE: SPECIFIC CONDITIONS

1. SCOPE

1.1 This renews the Notice of Acceptance No. 00-0321.31 which was issued on April 28, 2000. It approves a residential insulated door, as described in Section 2 of this Notice of Acceptance, designed to comply with the South Florida Building Code (SFBC), 1994 Edition for Miami-Dade County, for the locations where the pressure requirements, as determined by SFBC Chapter 23, do not exceed the Design Pressure Rating values indicated in the approved drawings.

2. PRODUCT DESCRIPTION

2.1 The **Series Entergy 6-8 S-W/E Outswing Glazed Double Residential Insulated Steel Door with Sidelites** and its components shall be constructed in strict compliance with the following documents: Drawing No **31-1028-EW-O**, Sheets 1 through 6 of 6, titled "Premdor (Entergy Brand) Double Door with Sidelites in Wood Frames with Bumper Threshold (Outswing)," prepared by manufacturer, dated 7/29/97 with revision C dated 01/11/00, bearing the Miami-Dade County Product Control approval stamp with the Notice of Acceptance number and approval date by the Miami-Dade County Product Control Division. These documents shall hereinafter be referred to as the approved drawings.

3. LIMITATIONS

3.1 This approval applies to single unit applications of pair of doors and single door only, as shown in approved drawings. Single door units shall include all components described in the active leaf of this approval.

4. INSTALLATION

4.1 The residential insulated steel door and its components shall be installed in strict compliance with the approved drawings.

4.2 Hurricane protection system (shutters): the installation of this unit <u>will require</u> a hurricane protection system.

5. LABELING

5.1 Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved".

6. BUILDING PERMIT REQUIREMENTS

6.1 Application for building permit shall be accompanied by copies of the following:

6.1.1 This Notice of Acceptance

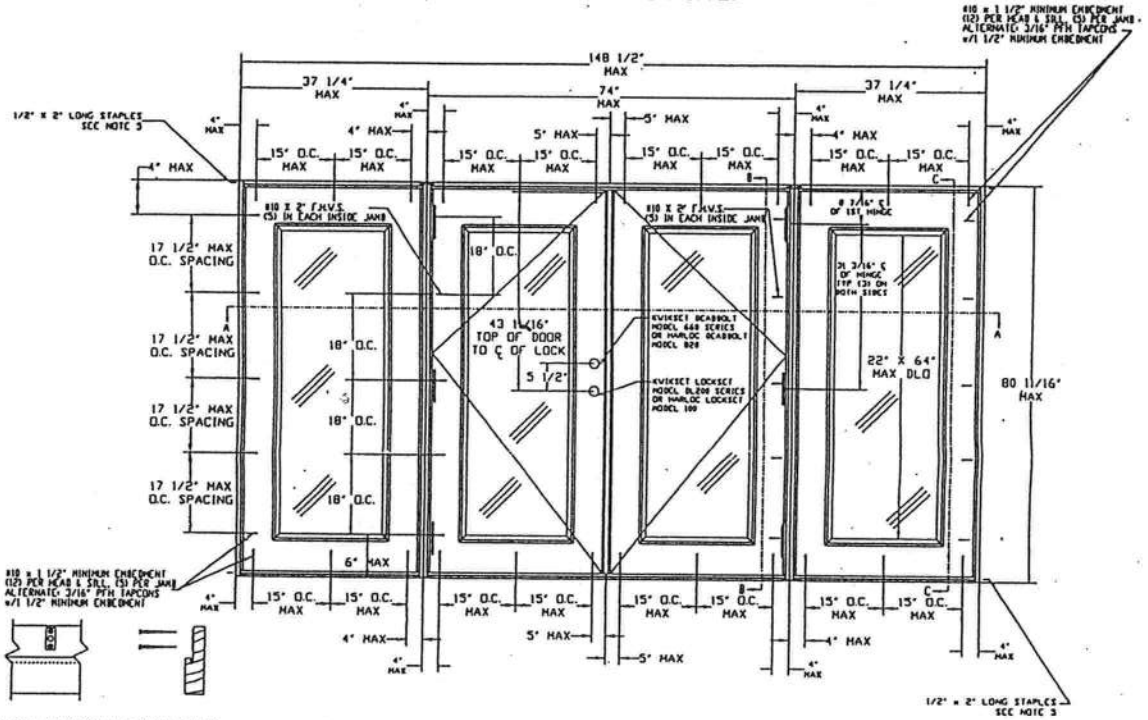
6.1.2 Duplicate copies of the approved drawings, as identified in Section 2 of this Notice of Acceptance, clearly marked to show the components selected for the proposed installation.

6.1.3 Any other documents required by the Building Official or the South Florida Building Code (SFBC) in order to properly evaluate the installation of this system.

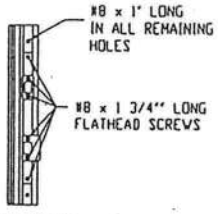
Manuel Perez

Manuel Perez, P.E. Product Control Examiner
Product Control Division

PREMDOR (ENTERGY BRAND) DOUBLE DOOR WITH SIDELITES IN WOOD FRAMES WITH A BUMPER THRESHOLD (OUTSWING)



ATTACH ASTRAGAL THROW BOLT STRIKE PLATE TO THE HEADER AND THRESHOLD WITH #10 x 1 3/4" FLATHEAD SCREWS



ASTRAGAL

NOTES:

1.) WOOD BUCKS BY OTHERS. MUST BE ANCHORED PROPERLY TO TRANSFER LOADS TO THE STRUCTURE.

2.) THE PRECEDING DRAWINGS ARE INTENDED TO QUALIFY THE FOLLOWING INSTALLATIONS.

A. WOOD FRAME CONSTRUCTION WHERE DOOR SYSTEM IS ANCHORED TO A MINIMUM TWO BY WOOD OPENING.

B. MASONRY OR CONCRETE CONSTRUCTION WHERE DOOR SYSTEM IS ANCHORED TO A MINIMUM TWO BY STRUCTURAL WOOD BUCK.

C. MASONRY OR CONCRETE CONSTRUCTION WHERE DOOR SYSTEM IS ANCHORED DIRECTLY TO CONCRETE OR MASONRY WITH OR WITHOUT A NON-STRUCTURAL ONE BY WOOD BUCK.

3. ALL ANCHORING SCREWS TO BE #10 WITH MINIMUM 1 1/2" EMBEDMENT INTO WOOD SUBSTRATE OR 3/16" PFH TAPCONS WITH 1 1/2" MINIMUM EMBEDMENT MASONRY.

4. UNIT MUST BE INSTALLED WITH "MIAMI-DADE COUNTY APPROVED" SHUTTERS

5. THREE STAPLES PER SIDE JAMB INTO HEADER ON SIDELITES AND DOOR, THREE STAPLES PER JAMB INTO THRESHOLD ON SIDELITES AND DOOR.

6. LATEX SEALANT TO BE APPLIED AT SIDE BY SIDE JAMBS AND SIDELITES.

7. DOOR/SIDELITE HEADER, DOOR/SIDELITE JAMBS, AND SIDELITE BASE CORNERS ARE COPED AND BUTT JOINED.

8. DOORS SHALL BE PRE-PAINTED WITH A WATER-BASED EPOXY RUST INHIBITIVE PRIMER PAINT WITH A DRY FILM THICKNESS OF 0.8 TO 1.2 MIL.

9. FRAMES SHALL BE PRE-PAINTED WITH AN ACRYLIC LATEX WATER-BASED/ WATER-REDUCIBLE WHITE PRIMER WITH A DRY FILM THICKNESS OF 0.8 TO 1.2 MIL.

DESIGN PRESSURE RATINGS		
	WHERE WATER INFILTRATION REQUIREMENT IS NEEDED *	WHERE WATER INFILTRATION REQUIREMENT IS NOT NEEDED
Positive	+ 50.5 psf	+50.5 psf
Negative	NOT APPROVED*	-50.5 psf

* UNITS SHALL BE INSTALLED ONLY AT LOCATIONS PROTECTED BY A CANOPY OR OVERHANG SUCH THAT THE ANGLE BETWEEN THE EDGE OF CANOPY OR OVERHANG TO SILL IS LESS THAN 45 DEGREES. UNLESS UNIT IS INSTALLED IN NON-HABITABLE AREAS WHERE THE UNIT AND THE AREA ARE DESIGNED TO ACCEPT WATER INFILTRATION.

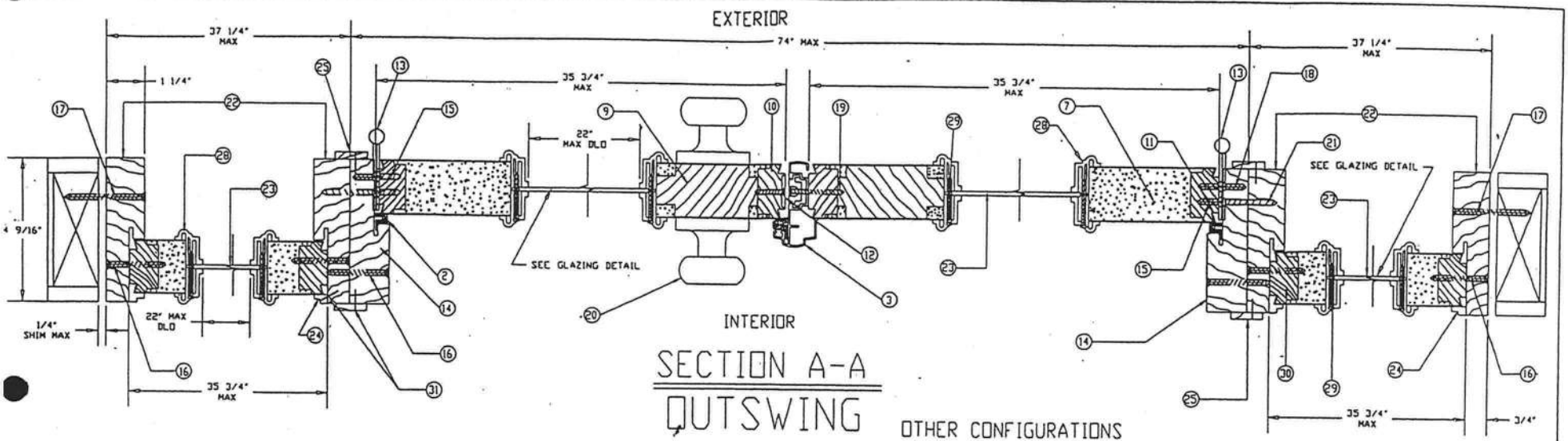
APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE JUN 05 2001
BY
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 01-0314.2

LTR	REVISIONS	DATE	BY
C	DADE COUNTY MODIFICATIONS	1/11/00	JD
B	ADDED PAGE 5 (DOOR OPTIONS)	12-1-98	RS
A	ADD OTHER DOOR CONFIGURATIONS	12/16/97	RS

UNITS: UNLESS NOTED, FRAC. ± DEC. ± ANG. ±
EXTRUSIONS: UNLESS NOTED, STD. COMM. TOL.
ENGINEER:
DR. BY R.S. | DATE 7-29-97
PART NAME: ENTRANCE DOOR ENERGY BRAND DOUBLE DOOR W/SIDELITES
MATL:
SCALE: N.T.S.

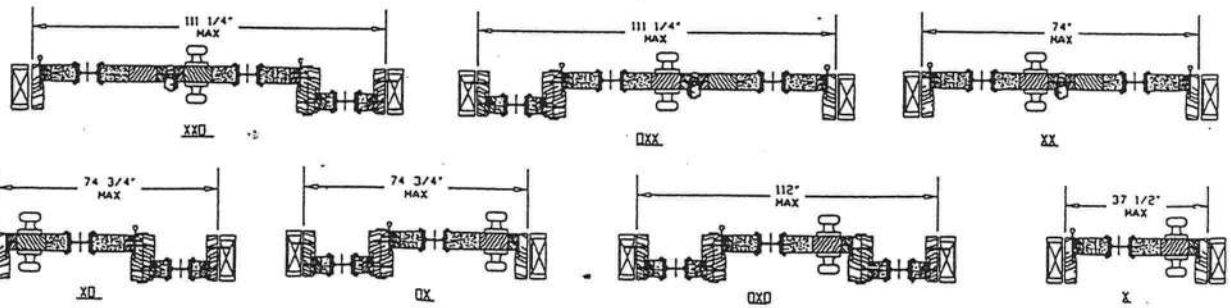
PREMDOR ENTRY SYSTEMS
911 E. JEFFERSON
PITTSBURG, KS 66762

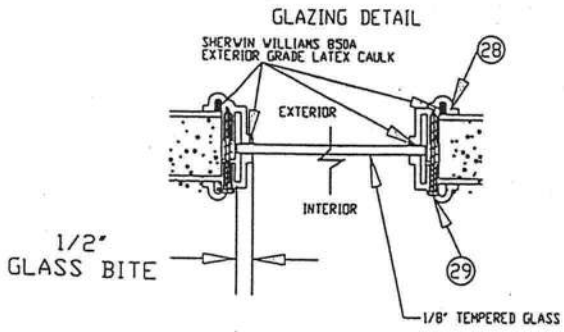
31-1028-EW-0
SHEET 1 OF 6
REVISION LETTER C



SECTION A-A
OUTSWING

OTHER CONFIGURATIONS





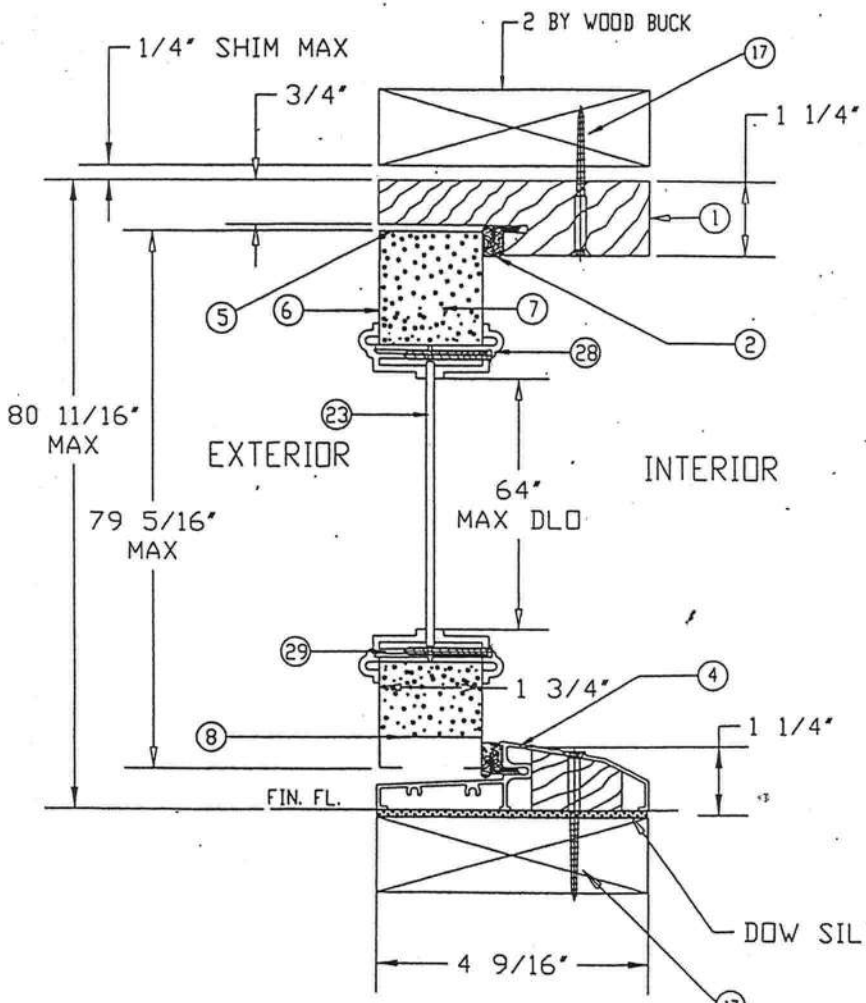
APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE JUN 05 2001
BY
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 01-0314.29

C	DADE COUNTY MODIFICATIONS	6/11/01	JD
B	ADDED PAGE 5 (DOOR OPTIONS)	10-1-98	RS
A	ADD SCREWS TO LITE FRAMES & ADD OTHER DOOR CONFIGURATIONS	7-10-97	RS
LTR	REVISIONS	DATE	BY

LINEAR UNLESS NOTED, FRAC. ± ; DEC. ± ; ANG. ±
EXTRUSIONS UNLESS NOTED, STD. CONT. TOL.'S
ENGINEER:
DR. BY: R.S. DATE: 7-29-97
PART NAME: ENTRANCE DOOR W/STRUCTURE (A-A)
MATL:
SCALE: NTS

PREMDOR ENTRY SYSTEMS
90 E. JEFFERSON
PITTSBURG, KS 66762

31-1028-EW-0
SHEET 2 OF 6
REVISION LETTER C



SECTION B-B

MATERIALS LIST

ITEM NO.	DESCRIPTION	PART NUMBER	COMMENTS
1	WOOD HEAD JAMB	EW-14	1 1/4" X 4 9/16" MTL. TO BE PINE OR EQUIVALENT
2	COMPRESSION WEATHERSTRIP	EW-25	LOCKSCREEN BRAND LOXSEAL 9650 (BRONZE)
3	ALUMINUM ASTRAGAL	EW-12	PREMDOR BRAND OR EQUIVALENT- 5/8" ALUMINUM ASTRAGAL
4	ALUMINUM-BUMPER THRESHOLD	EW-15	PREMDOR BRAND OR EQUIVALENT - 1 1/4" x 4 9/16"
5	TOP CHANNEL	EW-08	PREMDOR BRAND - 1 11/16" - 20 GA STEEL
6	STEEL SKIN	26 ga. (.017 +.004 -.000)	MIN. YIELD STRENGTH 45,000 PSI; MIN. THICKNESS PER TENSILE TEST REPORT IS .021"
7	POLYURETHANE FOAM CORE	BASF FOAM - DENSITY 2.0 TO 2.5 lbs./Ft³	
8	BOTTOM CHANNEL	EW-07	PREMDOR BRAND - 1 11/16" - 20 GA STEEL
9	WOOD LOCK BLOCK	EW-09	4" X 9 1/2" MTL. TO BE PINE OR EQUIVALENT
10	STRIKE STILE	EW-06	15/16" X 1 11/16" MTL. TO BE PINE OR EQUIVALENT
11	HINGE STILE	EW-05	15/16" X 1 11/16" MTL. TO BE PINE OR EQUIVALENT
12	LOCK PREP FILLER PLATE	EW-10	PREMDOR BRAND - .050" THICK- MTL. TO BE POLYETHYLENE
13	4"x4" HINGE	EW-16	HAGER BRAND HINGE OR EQUIVALENT - .097 THICK (STEEL)
14	WOOD HINGE JAMB	EW-13	1 1/4" X 4 9/16" MTL. TO BE PINE OR EQUIVALENT
15	#10 x 3/4" F.H.W.S.		(4) SCREWS PER HINGE INTO DOOR
16	#10 X 2" F.H.W.S.		(5) SCREWS THROUGH HINGE JAMB INTO SIDELITE JAMB, 8" DOWN FROM TOP, MAX 18" O.C. THEREAFTER (10) SCREWS THROUGH STRIKE JAMB INTO SIDELITE JAMB, 4" DOWN FROM TOP MAX 8" O.C. THEREAFTER (6) SCREWS THROUGH EACH SIDELITE JAMB INTO SIDELITE, 4" DOWN FROM TOP, MAX 15" O.C. THEREAFTER
17	#10 F.H.W.S. W/MINIMUM 1 1/2 EMBEDMENT OR 3/16" PITH TAPPING W/MINIMUM 1 1/2" EMBEDMENT		REFER TO ELEVATION VIEW, FOR # OF SCREWS USED AND LOCATIONS
18	#10 X 3/4" F.H.W.S.		(2) SCREWS PER HINGE INTO JAMB
19	#8 x 2" F.H.W.S.		(2) SCREWS AT EACH STRIKE PLATE
20	LOCKSET		KWIKSET BRAND 200 LOCK OR HARLOC BRAND 100 LOCK
21	#10 X 1 3/4" F.H.W.S.		(2) SCREWS PER HINGE INTO JAMB
22	WOOD SIDELITE JAMB	EW-19	1 1/4" X 4 9/16" MTL. TO BE PINE OR EQUIVALENT
23	22" X 64" SINGLE PANEL GLASS	EW-20	TEMPERED GLASS IN POLYPROPYLENE FRAME- DC-1643 - (ODL-23 1/8" CLEAR TEMPERED GLASS
24	SIDELITE TRIM (WOOD)	EW-21	5/16 x 1/2" MTL. TO BE PINE OR EQUIVALENT
25	WOOD CASING	EW-22	1/8" x 1" MTL. TO BE PINE OR EQUIVALENT - ITEMS ARE MOLDINGS USED FOR "SIDE BY SIDE JAMBS" AS MULLIONS
26	WOOD SIDELITE HEAD JAMB	EW-23	1 1/4" X 4 9/16" MTL. TO BE PINE OR EQUIVALENT
27	WOOD SIDELITE BASE	EW-24	1 1/4" X 4 9/16" MTL. TO BE PINE OR EQUIVALENT
28	POLYPROPYLENE LITE FRAME	DC-1643, ODL-2	HP Polypropylene by ODL
29	#6 X 1 1/2" PAN HEAD SCREWS		18 PER FRAME SCREW SPACING TO BE 3" IN FROM EACH CORNER AND NOT TO EXCEED 14" OC THERE AFTER
30	SIDELITE STILES	EW-26	15/16" X 1 11/16" MTL. TO BE PINE OR EQUIVALENT
31	PIN NAIL		3/4" LONG NAIL, 4" IN FROM END, MAX 8" O.C. THEREAFTER, USED ON MULLIONS AND TRIM

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE JUN 05 2001
BY
PRODUCT CONTROL DIVISION
BUILDING CODE CO' 'NCE OFFICE
ACCEPTANCE NO. 314.29

LIMITS: UNLESS NOTED, FRAC. : DEC. : ANG. :
EXTRUSIONS: UNLESS NOTED, STD. COM'L. TOL'S.
ENGINEER:
DR. BY R.S. DATE 7-29-97

LTR.	REVISIONS	DATE	BY
B	DADE COUNTY MODIFICATIONS	1/11/01	JD
A	ADDED PAGE 5 (DOOR OPTIONS)	10-1-98	RS

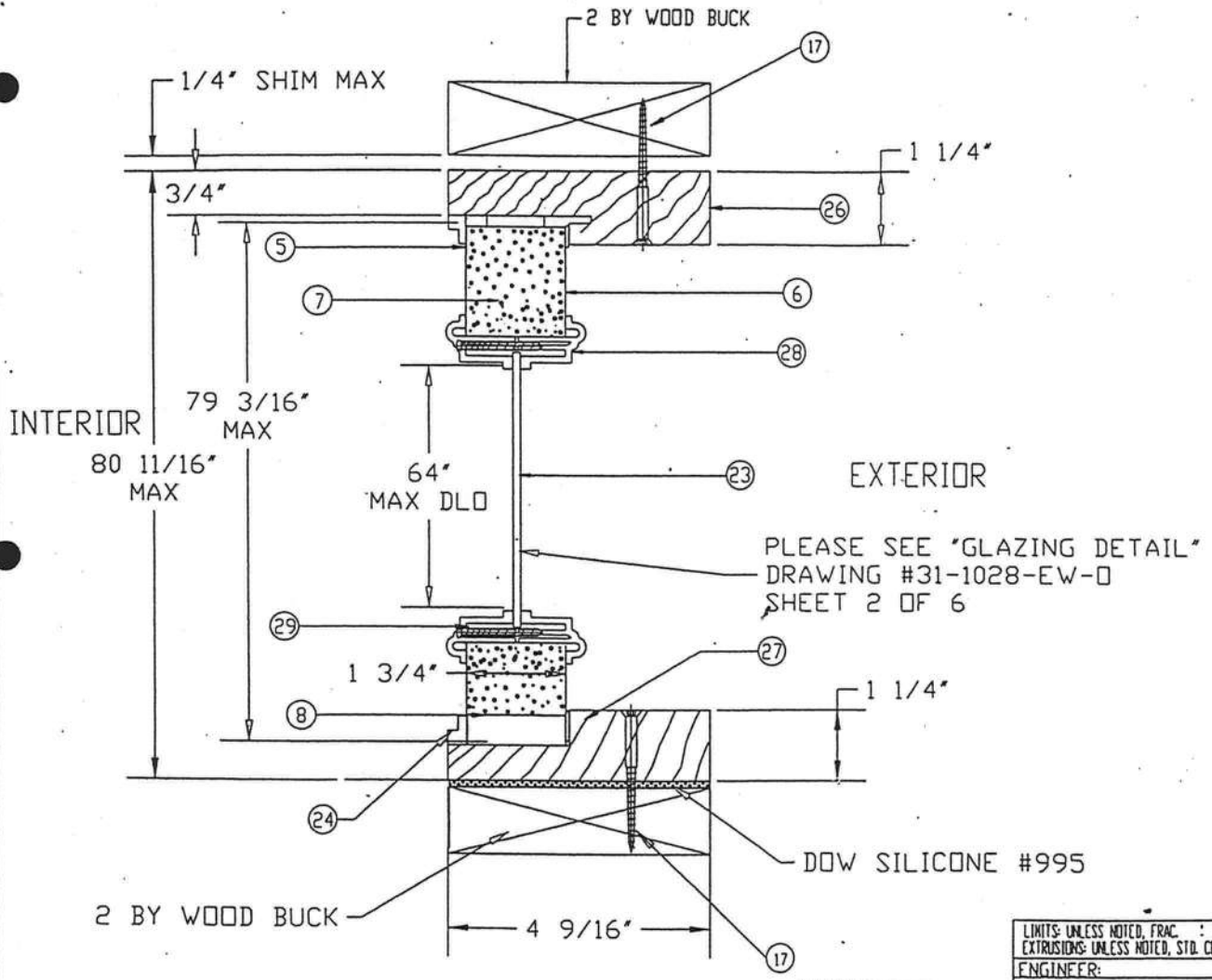
PART NAME: ENTERGY WOOD EDGE DOOR (B-B)
MAT'L: SCALE:

PREMDOR ENTRY SYSTEMS
911 E. JEFFERSON
PITTSBURG, KS. 66762

A

31-1028-EW-0
SHEET 3 OF 6

REVISION LETTER B



SECTION C-C

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE JUN 05 2001
BY
PRODUCT CONTROL DIVISION
BUILDING CODE COMP[illegible]E OFFICE
ACCEPTANCE NO. 01 [illegible]4.29

LTR.	REVISIONS	DATE	BY
D	DADE COUNTY MODIFICATIONS	1/11/01	JD
C	MATERIAL WAS POLYSTYRENE	6-2-99	RS
B	ADDED PAGE 5 (DOOR OPTIONS)	10-1-98	RS
A	ADD SCREWS TO LITE FRAME & MATERIAL LIST	12-18-97	R.S.

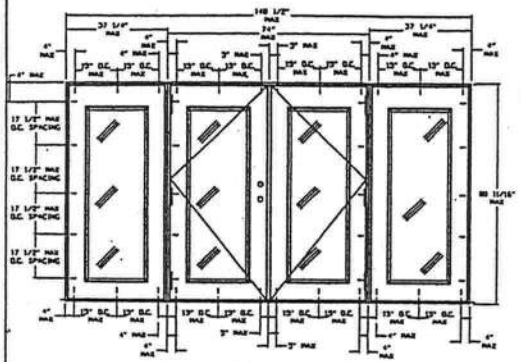
LIMITS: UNLESS NOTED, FRAC. : DEC. : ANG. :
EXTRUSIONS: UNLESS NOTED, STD. COM'L. TOL'S.
ENGINEER:
DR. BY R.S. | DATE 7-29-97
PART NAME: ENTERGY WOOD EDGE SIDELITE (C-C)
MAT'L: | SCALE:

PREMDOR ENTRY SYSTEMS
911 E. JEFFERSON
PITTSBURG, KS. 66762

31-1028-EW-0
SHEET 4 OF 6

REVISION LETTER D

OTHER DOOR CONFIGURATIONS



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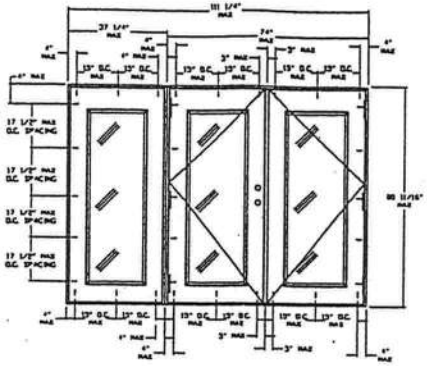

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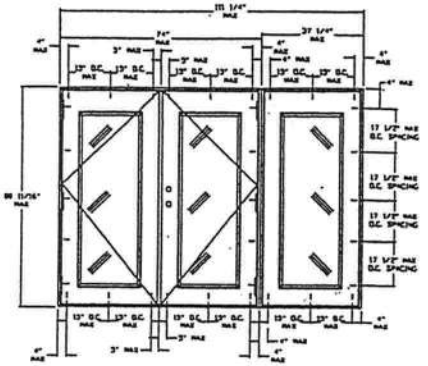

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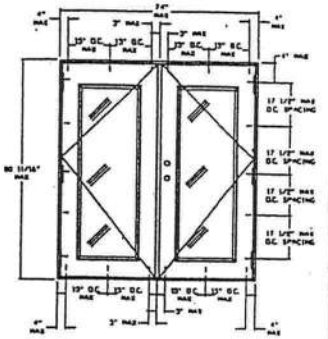

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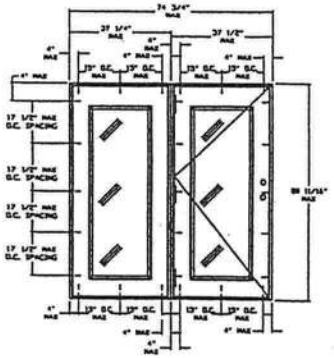

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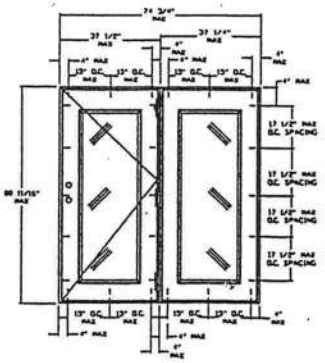

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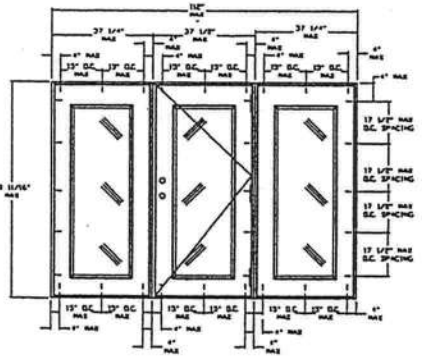

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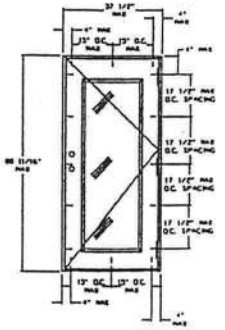


X

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE JUN 05 2001
BY
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 01-0314.29

LIMITS: UNLESS NOTED, FRAC. : DEC : ANG. :
EXTRUSIONS: UNLESS NOTED, STD COMM. TOL'S.

ENGINEER:

DR. BY J.D. | DATE 1-11-01

LTR	REVISIONS	DATE	BY

PART NAME:

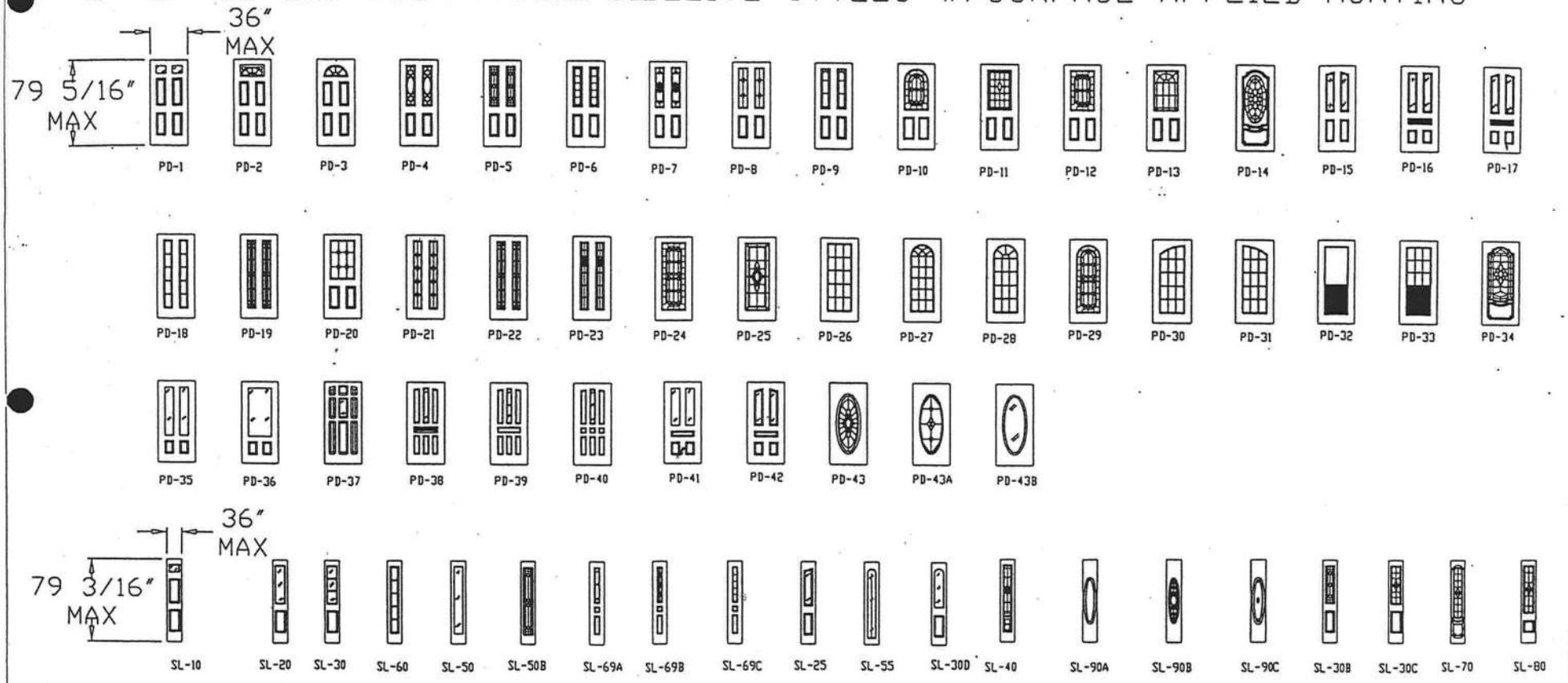
MAT'L: | SCALE:

PREMDOR ENTRY SYSTEMS
911 E. JEFFERSON
PITTSBURG, KS. 66762

31-1028-EW-□
SHEET 5 OF 6

REVISION LETTER

OTHER GLAZED DOOR PANEL/SIDELITE STYLES W/SURFACE APPLIED MUNTINS



APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE JUN 05 2001
BY
PRODUCT CONTROL DIVISION
'NG CODE COMPLIANCE OFFICE
'TANCE NO. 01-0314.29

LIMITS: UNLESS NOTED, FRAC. : DEC. : ANG. :
EXTRUSIONS: UNLESS NOTED, STD. COM'L. TOL'S.
ENGINEER:
DR. BY J.D. | DATE 1/15/01

LTR	REVISIONS	DATE	BY

PART NAME: PREMDOR DOOR OPTIONS
MAT'L: SCALE:

PREMDOR ENTRY SYSTEMS
911 E. JEFFERSON
PITTSBURG, KS. 66762

31-1028-EW-0
SHEET 6 OF 6
REVISION LETTER

Residential System Sizing Calculation

Summary

Rosewood Model

Lake City, FL 32025-

Project Title:
Greg Talley

Code Only
Professional Version
Climate: North

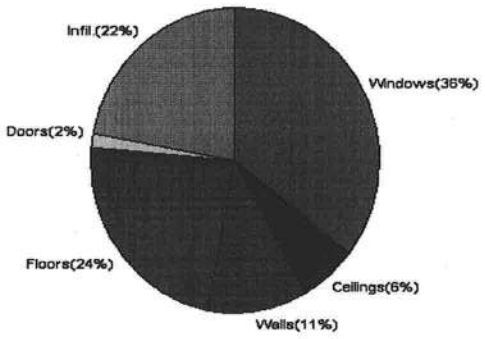
1/10/2007

Location for weather data: Gainesville - Defaults: Latitude(29) Altitude(152 ft.) Temp Range(M) Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(54gr.)					
Winter design temperature	33	F	Summer design temperature	92	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	37	F	Summer temperature difference	17	F
Total heating load calculation	**33349**	**Btuh**	**Total cooling load calculation**	**35273**	**Btuh**
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh
Total (Electric Heat Pump)	119.9	40000	Sensible (SHR = 0.75)	105.7	30000
Heat Pump + Auxiliary(0.0kW)	119.9	40000	Latent	144.9	10000
			Total (Electric Heat Pump)	113.4	40000

WINTER CALCULATIONS

Winter Heating Load (for 1672 sqft)

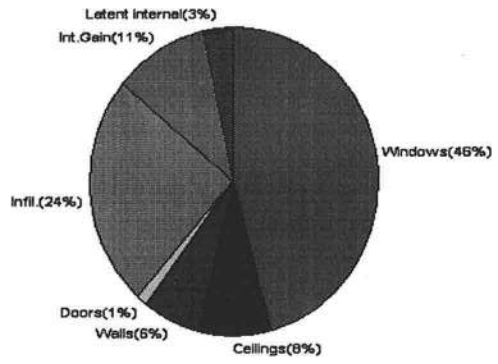
Load component			Load	
Window total	254	sqft	11921	Btuh
Wall total	1146	sqft	3765	Btuh
Door total	40	sqft	518	Btuh
Ceiling total	1750	sqft	2062	Btuh
Floor total	180	sqft	7859	Btuh
Infiltration	178	cfm	7224	Btuh
Duct loss			0	Btuh
Subtotal			**33349**	**Btuh**
Ventilation	0	cfm	0	Btuh
TOTAL HEAT LOSS			**33349**	**Btuh**



SUMMER CALCULATIONS

Summer Cooling Load (for 1672 sqft)

Load component			Load	
Window total	254	sqft	16103	Btuh
Wall total	1146	sqft	2292	Btuh
Door total	40	sqft	392	Btuh
Ceiling total	1750	sqft	2898	Btuh
Floor total			0	Btuh
Infiltration	156	cfm	2904	Btuh
Internal gain			3780	Btuh
Duct gain			0	Btuh
Sens. Ventilation	0	cfm	0	Btuh
Total sensible gain			**28370**	**Btuh**
Latent gain(ducts)			0	Btuh
Latent gain(infiltration)			5703	Btuh
Latent gain(ventilation)			0	Btuh
Latent gain(internal/occupants/other)			1200	Btuh
Total latent gain			**6903**	**Btuh**
TOTAL HEAT GAIN			**35273**	**Btuh**



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For Florida residences only

EnergyGauge® System Sizing
PREPARED BY: _____
DATE: 1-10-07

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

Rosewood Model

Project Title:
Greg Talley

Code Only
Professional Version
Climate: North

Lake City, FL 32025-

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

1/10/2007

Component Loads for Whole House

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	1, Clear, Metal, 1.27	W	90.0	47.0	4229 Btuh
2	1, Clear, Metal, 1.27	W	30.0	47.0	1410 Btuh
3	1, Clear, Metal, 1.27	SW	20.0	47.0	940 Btuh
4	1, Clear, Metal, 1.27	S	15.0	47.0	705 Btuh
5	1, Clear, Metal, 1.27	N	30.0	47.0	1410 Btuh
6	1, Clear, Metal, 1.27	N	2.7	47.0	127 Btuh
7	1, Clear, Metal, 1.27	E	30.0	47.0	1410 Btuh
8	1, Clear, Metal, 1.27	E	16.0	47.0	752 Btuh
9	1, Clear, Metal, 1.27	S	20.0	47.0	940 Btuh
	Window Total		254(sqft)		11921 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	974	3.3	3200 Btuh
2	Frame - Wood - Adj(0.09)	13.0	172	3.3	565 Btuh
	Wall Total		1146		3765 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Adjacent		20	12.9	259 Btuh
2	Insulated - Exterior		20	12.9	259 Btuh
	Door Total		40		518Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin)	30.0	1750	1.2	2062 Btuh
	Ceiling Total		1750		2062Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab On Grade	0	180.0 ft(p)	43.7	7859 Btuh
	Floor Total		180		7859 Btuh
			Zone Envelope Subtotal:		26125 Btuh
Infiltration	Type	ACH X	Zone Volume	CFM=	
	Natural	0.80	13376	178.3	7224 Btuh
Ductload	Proposed leak free, R6.0, Supply(Attic), Return(Attic)			(DLM of 0.00)	0 Btuh
Zone #1			**Sensible Zone Subtotal**		**33349 Btuh**

Manual J Winter Calculations

Residential Load - Component Details (continued)

Rosewood Model

Lake City, FL 32025-

Project Title:
Greg Talley

Code Only
Professional Version
Climate: North

1/10/2007

WHOLE HOUSE TOTALS

	Subtotal Sensible	33349 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	33349 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

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For Florida residences only

System Sizing Calculations - Winter

Residential Load - Room by Room Component Details

Rosewood Model

Project Title:
Greg Talley

Code Only
Professional Version
Climate: North

Lake City, FL 32025-

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

1/10/2007

Component Loads for Zone #1: Main

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	1, Clear, Metal, 1.27	W	90.0	47.0	4229 Btuh
2	1, Clear, Metal, 1.27	W	30.0	47.0	1410 Btuh
3	1, Clear, Metal, 1.27	SW	20.0	47.0	940 Btuh
4	1, Clear, Metal, 1.27	S	15.0	47.0	705 Btuh
5	1, Clear, Metal, 1.27	N	30.0	47.0	1410 Btuh
6	1, Clear, Metal, 1.27	N	2.7	47.0	127 Btuh
7	1, Clear, Metal, 1.27	E	30.0	47.0	1410 Btuh
8	1, Clear, Metal, 1.27	E	16.0	47.0	752 Btuh
9	1, Clear, Metal, 1.27	S	20.0	47.0	940 Btuh
	Window Total		254(sqft)		11921 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	974	3.3	3200 Btuh
2	Frame - Wood - Adj(0.09)	13.0	172	3.3	565 Btuh
	Wall Total		1146		3765 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Adjacent		20	12.9	259 Btuh
2	Insulated - Exterior		20	12.9	259 Btuh
	Door Total		40		518Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin)	30.0	1750	1.2	2062 Btuh
	Ceiling Total		1750		2062Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab On Grade	0	180.0 ft(p)	43.7	7859 Btuh
	Floor Total		180		7859 Btuh
			Zone Envelope Subtotal:		26125 Btuh
Infiltration	Type	ACH X	Zone Volume	CFM=	
	Natural	0.80	13376	178.3	7224 Btuh
Ductload	Proposed leak free, R6.0, Supply(Attic), Return(Attic)			(DLM of 0.00)	0 Btuh
Zone #1			**Sensible Zone Subtotal**		**33349 Btuh**

Manual J Winter Calculations

Residential Load - Component Details (continued)

Rosewood Model

Lake City, FL 32025-

Project Title:
Greg Talley

Code Only
Professional Version
Climate: North

1/10/2007

WHOLE HOUSE TOTALS

	Subtotal Sensible	33349 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	33349 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

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For Florida residences only

System Sizing Calculations - Summer

Residential Load - Whole House Component Details

Rosewood Model

Lake City, FL 32025-

Project Title:
Greg Talley

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F 1/10/2007

Component Loads for Whole House

	Type*		Overhang		Window Area(sqft)			HTM		Load
Window	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded	
1	1, Clear, 1.27, None,N,N	W	1.5ft	8ft.	90.0	0.0	90.0	37	94	8464 Btuh
2	1, Clear, 1.27, None,N,N	W	13.5f	8ft.	30.0	30.0	0.0	37	94	1124 Btuh
3	1, Clear, 1.27, None,N,N	SW	13.5f	8ft.	20.0	20.0	0.0	37	75	749 Btuh
4	1, Clear, 1.27, None,N,N	S	13.5f	8ft.	15.0	15.0	0.0	37	43	562 Btuh
5	1, Clear, 1.27, None,N,N	N	1.5ft	8ft.	30.0	0.0	30.0	37	37	1124 Btuh
6	1, Clear, 1.27, None,N,N	N	1.5ft	8ft.	2.7	0.0	2.7	37	37	101 Btuh
7	1, Clear, 1.27, None,N,N	E	7.5ft	8ft.	30.0	19.3	10.7	37	94	1726 Btuh
8	1, Clear, 1.27, None,N,N	E	1.5ft	8ft.	16.0	0.0	16.0	37	94	1505 Btuh
9	1, Clear, 1.27, None,N,N	S	1.5ft	8ft.	20.0	20.0	0.0	37	43	749 Btuh
	Window Total				254 (sqft)					16103 Btuh

Walls	Type	R-Value/U-Value	Area(sqft)	HTM	Load
1	Frame - Wood - Ext	13.0/0.09	974.3	2.1	2032 Btuh
2	Frame - Wood - Adj	13.0/0.09	172.0	1.5	260 Btuh
	Wall Total		1146 (sqft)		2292 Btuh
Doors	Type		Area (sqft)	HTM	Load
1	Insulated - Adjacent		20.0	9.8	196 Btuh
2	Insulated - Exterior		20.0	9.8	196 Btuh
	Door Total		40 (sqft)		392 Btuh
Ceilings	Type/Color/Surface	R-Value	Area(sqft)	HTM	Load
1	Vented Attic/DarkShingle	30.0	1750.0	1.7	2898 Btuh
	Ceiling Total		1750 (sqft)		2898 Btuh
Floors	Type	R-Value	Size	HTM	Load
1	Slab On Grade	0.0	180 (ft(p))	0.0	0 Btuh
	Floor Total		180.0 (sqft)		0 Btuh
				Zone Envelope Subtotal:	21685 Btuh

Infiltration	Type	ACH	Volume(cuft)	CFM=	Load
	SensibleNatural	0.70	13376	156.1	2904 Btuh
Internal gain		Occupants	Btuh/occupant	Appliance	Load
		6	X 230 +	2400	3780 Btuh
Duct load	Proposed leak free, R6.0, Supply(Attic), Return(Attic)			DGM = 0.00	0.0 Btuh
				Sensible Zone Load	**28370 Btuh**

Manual J Summer Calculations

Residential Load - Component Details (continued)

Rosewood Model

Lake City, FL 32025-

Project Title:
Greg Talley

Code Only
Professional Version
Climate: North

1/10/2007

WHOLE HOUSE TOTALS

	Sensible Envelope Load All Zones	**28370 Btuh**
	Sensible Duct Load	0 Btuh
	Total Sensible Zone Loads	**28370 Btuh**
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
Whole House	**Total sensible gain**	**28370 Btuh**
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	5703 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	0 Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200 Btuh
	Latent other gain	0 Btuh
	Latent total gain	**6903 Btuh**
	TOTAL GAIN	**35273 Btuh**

*Key: Window types (Pn - Number of panes of glass)
(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))
(Ornt - compass orientation)

For Florida residences only

System Sizing Calculations - Summer

Residential Load - Room by Room Component Details

Rosewood Model

Project Title:
Greg Talley

Code Only
Professional Version
Climate: North

Lake City, FL 32025-

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F 1/10/2007

Component Loads for Zone #1: Main

Window	Type* Pn/SHGC/U/InSh/ExSh/IS	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load
1	1, Clear, 1.27, None,N,N	W	1.5ft	8ft.	90.0	0.0	90.0	37	94	8464 Btuh
2	1, Clear, 1.27, None,N,N	W	13.5f	8ft.	30.0	30.0	0.0	37	94	1124 Btuh
3	1, Clear, 1.27, None,N,N	SW	13.5f	8ft.	20.0	20.0	0.0	37	75	749 Btuh
4	1, Clear, 1.27, None,N,N	S	13.5f	8ft.	15.0	15.0	0.0	37	43	562 Btuh
5	1, Clear, 1.27, None,N,N	N	1.5ft	8ft.	30.0	0.0	30.0	37	37	1124 Btuh
6	1, Clear, 1.27, None,N,N	N	1.5ft	8ft.	2.7	0.0	2.7	37	37	101 Btuh
7	1, Clear, 1.27, None,N,N	E	7.5ft	8ft.	30.0	19.3	10.7	37	94	1726 Btuh
8	1, Clear, 1.27, None,N,N	E	1.5ft	8ft.	16.0	0.0	16.0	37	94	1505 Btuh
9	1, Clear, 1.27, None,N,N	S	1.5ft	8ft.	20.0	20.0	0.0	37	43	749 Btuh
	Window Total				254 (sqft)					16103 Btuh

Walls	Type	R-Value/U-Value	Area(sqft)	HTM	Load
1	Frame - Wood - Ext	13.0/0.09	974.3	2.1	2032 Btuh
2	Frame - Wood - Adj	13.0/0.09	172.0	1.5	260 Btuh
	Wall Total		1146 (sqft)		2292 Btuh

Doors	Type	Area (sqft)	HTM	Load
1	Insulated - Adjacent	20.0	9.8	196 Btuh
2	Insulated - Exterior	20.0	9.8	196 Btuh
	Door Total	40 (sqft)		392 Btuh

Ceilings	Type/Color/Surface	R-Value	Area(sqft)	HTM	Load
1	Vented Attic/DarkShingle	30.0	1750.0	1.7	2898 Btuh
	Ceiling Total		1750 (sqft)		2898 Btuh

Floors	Type	R-Value	Size	HTM	Load
1	Slab On Grade	0.0	180 (ft(p))	0.0	0 Btuh
	Floor Total		180.0 (sqft)		0 Btuh

			Zone Envelope Subtotal:		21685 Btuh
Infiltration	Type	ACH	Volume(cuft)	CFM=	Load
	SensibleNatural	0.70	13376	156.1	2904 Btuh
Internal gain		Occupants	Btuh/occupant	Appliance	Load
		6	X 230 +	2400	3780 Btuh
Duct load	Proposed leak free, R6.0, Supply(Attic), Return(Attic)			DGM = 0.00	0.0 Btuh
			Sensible Zone Load		**28370 Btuh**

Manual J Summer Calculations

Residential Load - Component Details (continued)

Rosewood Model

Lake City, FL 32025-

Project Title:
Greg Talley

Code Only
Professional Version
Climate: North

1/10/2007

WHOLE HOUSE TOTALS

	Sensible Envelope Load All Zones	**28370 Btuh**
	Sensible Duct Load	0 Btuh
	Total Sensible Zone Loads	**28370 Btuh**
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
Whole House	**Total sensible gain**	**28370 Btuh**
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	5703 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	0 Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200 Btuh
	Latent other gain	0 Btuh
	Latent total gain	**6903 Btuh**
	TOTAL GAIN	**35273 Btuh**

*Key: Window types (Pn - Number of panes of glass)
(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))
(Ornt - compass orientation)

For Florida residences only

Residential Window Diversity

MidSummer

Rosewood Model

Lake City, FL 32025-

Project Title:
Greg Talley

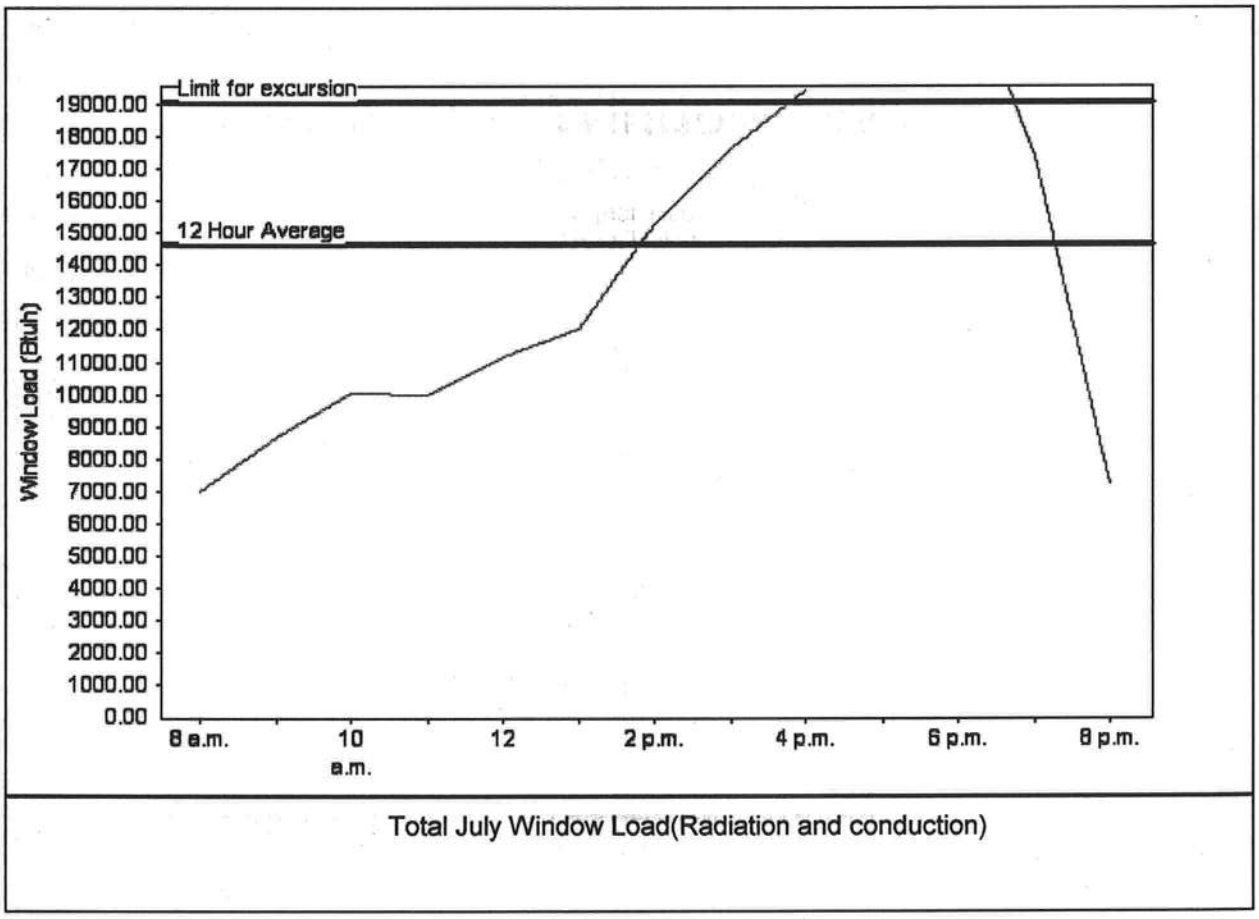
Code Only
Professional Version
Climate: North

1/10/2007

Weather data for: Gainesville - Defaults

Summer design temperature	92 F	Average window load for July	14659 Btu
Summer setpoint	75 F	Peak window load for July	24192 Btu
Summer temperature difference	17 F	Excusion limit(130% of Ave.)	19056 Btu
Latitude	29 North	Window excursion (July)	5136 Btuh

WINDOW Average and Peak Loads



Total July Window Load(Radiation and conduction)

This application has glass areas that produce large heat gains for part of the day. Variable air volume devices are required to overcome spikes in solar gain for one or more rooms. Install a zoned system or provide zone control for problem rooms. Single speed equipment may not be suitable for the application.

EnergyGauge® System Sizing for Florida residences only

PREPARED BY: _____

DATE: _____

National Evaluation Service, Inc.
5203 Leesburg Pike, Suite 708, Falls Church, Virginia 22041-3401
Phone: 703/931-2187 Fax: 703/931-6506
website: www.nateval.org

INTERNATIONAL CODE COUNCIL

NATIONAL EVALUATION REPORT

Report No. NER-432
Reissued June 1, 2000

SIMPSON STRONG-TIE® COMPANY, INC., CONNECTORS

SIMPSON STRONG-TIE® COMPANY, INC.
4637 CHABOT DRIVE, SUITE 200
PLEASANTON, CALIFORNIA 94588

1.0 SUBJECT

Simpson Strong-Tie® Connectors:

1.1 ABE Adjustable Post Bases
1.2 CBA Adjustable Column Bases
1.3 EPB44T Elevated Post Base
1.4 H2.5, H10-2, H15, and H15-2, Hurricane Ties
1.5 HGT-2, HGT-3, and HGT-4 Heavy Girder Tiedowns
1.6 LSSU Field Slope and Skewable Hangers
1.7 LTHMA Light Multiple Truss Hanger
1.8 LTHJ Light Truss Hip/Jack Hangers
1.9 LTP4 Lateral Tie Plate
1.10 LTTI31 Tension Tie
1.11 MSC Multiple Seat Connectors
1.12 RSP4 Reversible Stud Plate Tie
1.13 SP Stud Plate Connectors
1.14 SS Stud Shoes
1.15 THG2A Skewed Truss Girder Hangers
1.16 TWB T-Type Wall Bracing

2.0 PROPERTY FOR WHICH EVALUATION IS SOUGHT

Structural

3.0 DESCRIPTION

3.1 ABE ADJUSTABLE POST BASES

The two-part bases are die-formed from No. 16 gage galvanized steel complying with ASTM A653 CS, with a minimum yield strength of 28,000 psi (193 MPa) and a minimum tensile strength of 38,000 psi (262 MPa). The base plate includes an adjustment slot for a 1/2 inch (12.7 mm) diameter anchor bolt, and a 1 inch (25.4 mm) pedestal is built into the four-sided standoff plate. The post bases are manufactured to fit nominal 3 1/2 inch by 3 1/2 inch (89 mm by 89 mm) wood posts. See Table 1 of this report for details and fastener schedules.

3.2 CBA ADJUSTABLE COLUMN BASES

The two piece bases attach a wood post to existing concrete footings and slabs and are adjustable to accommodate minimum 5 1/2 inch by 5 1/2 inch (140 mm by 140 mm) posts. The two pieces have two 3/4 inch (19 mm) diameter bolt holes in the base for attachment to the concrete elements. To connect to the posts, the projecting side flanges have 5/8 inch (15.9 mm) diameter bolt holes. Both pieces are formed from No. 10 gage galvanized steel that conforms to ASTM A653 SS Grade 33, with a minimum yield strength of 33,000 psi (227 MPa) and a minimum tensile strength of 45,000 psi (310 MPa). See Table 2 of this report for details, allowable loads, and fastener schedules.

3.3 EPB44T ELEVATED POST BASE

The two part base consists of a No. 12 gage galvanized steel channel. The channel includes a die drawn 5/8 inch (15.9 mm) diameter threaded hole trunnion at it's center, which accommodates a 5 inch long (127 mm), 5/8 inch (15.9 mm) diameter zinc plated threaded rod. The threaded rod and channel assembly adjusts to a maximum post elevation of 2 1/2 inches (63.4 mm) above the concrete and a minimum concrete embedment of 2 1/2 inches (63.4 mm). The post base is manufactured to accommodate nominal 4 inch by 4 inch (101.6 mm by 101.6 mm) wood posts. The steel meets ASTM A653 SS Grade 33 specifications with a minimum yield strength of 33,000 psi (227 MPa) and a minimum tensile strength of 45,000 psi (310 MPa). See Table 3 of this report for details, allowable loads, and fastener schedules.

3.4 H2.5, H10-2, H15, H15-2 HURRICANE TIES

Hurricane Ties are anchors designed to connect rafters or joists to wall plates or studs. The H2.5 and H10-2 are formed from No. 18 gage galvanized steel. The H15 and H15-2 are formed from No. 16 gage galvanized steel. The steel meets ASTM A653 FS with a minimum yield strength of 28,000 psi (193 MPa) and a minimum tensile strength of 38,000 psi (262 MPa). See Table 4 of this report for details and fastener schedules.

The H2.5 is a twisted strap tie used to attach a rafter or stud member to the side of a top plate or bottom sole plate. The lower end is fastened to the wall plate and is long enough to locate the nails into each of the two top plates.

The H10-2 is formed from a 5 1/2 inch (140 mm) plate, and has one 3 1/8 inch (79.4 mm) wide, 3 1/32 inch (80 mm) deep slot in the upper half of the plate, diagonal to the edge. The slot material is bent to form a 1 9/16 inch (39.7 mm) flange on each side. The H10-2 attaches double nominal 2 inch (50.8 mm) wide solid sawn rafters or joists to the top of a wall at the double plate.

The H15 and H15-2 are formed into an inverted U-shaped element with a 2 1/2 inch (63.5 mm) wide flange bent at right angles near the U-bend. The flanges are twisted again at 90 degree angles at the bottom of the connector. The H15 has a formed seat 1 5/8 inches (41.3 mm) wide and attaches over the heel of a single ply truss or rafter to the wall stud member below. The H15-2 has a formed seat 3 1/4 inches (82.6 mm) wide and attaches the heel of a double ply truss or rafter to a double nominal 2 inch (50.8 mm) thick wall stud member below. Both ties are capable of being field formed to accommodate rafter or truss pitches from 0:12 minimum to 7:12 maximum.

3.5 HGT-2, HGT-3, AND HGT-4 HEAVY GIRDER TIEDOWNS

The HGT-2, HGT-3, and HGT-4 are formed from No. 7 gage steel conforming to ASTM A570 Grade 33 with a minimum yield strength of 33,000 psi (227 MPa) and a minimum tensile strength of 52,000 psi (358 MPa), and a welded insert plate made from 1/2 inch (12.7 mm) thick A36 hot-rolled steel with a minimum yield strength of 33,000 psi (227 MPa) and a minimum ultimate strength of 45,000 psi (310 MPa). The HGT-2 attaches to the heel of a two-ply truss. The HGT-3 attaches to the heel of a three-ply truss. The HGT-4 attaches to the heel of a four-ply truss. The HGT tiedowns are anchored to concrete or wood construction with bolts, can accommodate top chord slopes from 3:12 minimum to 8:12 maximum and are provided with crescent washers for sloped top chord installations. See Table 5 of this report for details, allowable loads and fastener schedules.

3.6 LSSU FIELD SLOPE AND SKEWABLE HANGERS

The LSSU is fabricated from No.18 gage galvanized steel. It is designed to be sloped up or down 45 degrees and skewed right or left to 45 degrees. The steel complies with ASTM A653 FS having a minimum yield strength of 33,000 psi (227 MPa) and a minimum tensile strength of 45,000 psi (310 MPa). Use of the hangers to laterally support members is beyond the scope of this report. See Table 6 of this report for details, allowable loads, and fastener schedules.

3.7 LTHMA LIGHT MULTIPLE TRUSS HANGER

The LTHMA is fabricated from No. 16 gage galvanized steel conforming to ASTM A653 FS with a minimum yield strength of 33,000 psi (227 MPa) and a minimum tensile strength of 45,000 psi (310 MPa). The LTHMA is designed to carry up to three single-ply truss members intersecting at one point. The connector has three formed stirrups that are capable of being field sloped down from 0 to 45 degrees from the horizontal. See Table 7 of this report for details and fastener schedules.

3.8 LTHJ LIGHT TRUSS HIP/JACK HANGERS

The LTHJ hangers are formed from No.18 gage galvanized steel conforming to ASTM A653 FS having a minimum yield strength of 28,000 psi (193 MPa) and a minimum tensile strength of 38,000 psi (262 MPa). The hangers allow a 45 degree member and a 90 degree member to attach to the supporting member at the same location. Use of the hangers to laterally support members is beyond the scope of this report. See Table 8 of this report for hanger details, allowable loads, and fastener schedules.

3.9 LTP4 LATERAL TIE PLATE

The LTP4 is a 3 inch by 4 1/4 inch (76.2 mm by 108 mm), No. 20 gage, galvanized flat steel plate. The steel complies with ASTM A653 FS with a minimum yield strength of 33,000 psi (227 MPa) psi and a minimum tensile strength of 45,000 psi (310 MPa). See Table 9 of this report for details, allowable loads, and fastener schedules.

3.10 LTTI31 TENSION TIE

The LTTI31 is formed from No.18 gage steel strap that is 3 3/4 inches (95.2 mm) wide with a 90 degree bend at one end. The bend is 2 3/4 inches (69.8 mm) long, with an 11/16 inch (17.5 mm) diameter hole punched in the center to provide anchorage with a 5/8 inch (15.9 mm) diameter bolt. A No. 3 gage load transfer plate is installed in the bend in lieu of a washer. The No.18 gage steel complies with ASTM A653 SS and has a minimum yield strength of 33,000 psi (227 MPa) and a minimum tensile strength of 45,000 psi (310 MPa). The No. 3 gage steel complies with ASTM A570 with a minimum yield strength of 33,000 psi (227 MPa) and a minimum tensile strength of 52,000 psi (358 MPa). See Table 10 of this report for allowable loads and fastener schedules.

3.11 MSC MULTIPLE SEAT CONNECTORS

The MSC1.81 and MSC2 connectors are three No. 11 gage U-shaped hangers that are factory-welded to a single No. 3 gage steel angle with 1/8 inch (3.2 mm) fillet welds. The MSC4 connector consists of three No. 7 gage U-shaped stirrups that are factory-welded to a single No. 3 gage steel angle with 3/16 inch (4.8 mm) fillet welds. The MSC series connector is designed to carry up to three members intersecting at one point, with the center member perpendicular to the carried member. The two U-shaped stirrups for the side members can accommodate horizontal skews up to 45 degrees from the center member and down slopes from 0 to 45 degrees. The Nos. 11, 7 and 3 gage steel conform to ASTM A570 Grade 33 with a minimum yield strength of 33,000 psi (227 MPa) and a minimum tensile strength of 52,000 psi (358 MPa). See Table 11 of this report for details, allowable loads, and fastener schedules.

3.12 RSP4 REVERSIBLE STUD PLATE TIE

The RSP4 Reversible Stud Plate Tie is a No. 20 gage galvanized sheet metal plate cut in the shape of a "1/4". The RSP4 has two triangular and two rectangular tabs, bent at 90 degrees from the face of the "1/4", which act as placement guides. The RSP4 is a dual-purpose, reversible tie-plate used to secure vertical wood-stud framing members (typically 2 x 4, 2 x 6, and 2 x 8) to the bottom sole plate of a stud wall, or secure the top of the same stud member to the double top plate members.

In a stud-to-double-top-plate condition, the RSP4 rectangular tabs on the vertical leg of the "1/4" act as guides to position the nailing pattern to allow for an equal number of nails to be distributed to each top plate. In the stud-to-bottom sole-plate application, the RSP4 is inverted (the horizontal leg is on the bottom), and is installed such that the triangular tabs on the horizontal leg rest on top of the sole plate to position the tie plate for consistent nail placement. The steel complies with ASTM A653 FS, with a minimum yield strength of 28,000 psi (193 MPa) and a minimum tensile strength of 38,000 psi (262 MPa). See Table 12 of this report for details, allowable loads, and fastener schedules.

3.13 SP STUD PLATE CONNECTORS

SP connectors are die-formed from No. 20 gage galvanized steel conforming to ASTM A653 FS, with a minimum yield strength of 28,000 psi (193 MPa) and a minimum tensile strength of 38,000 psi (262 MPa). SP1 and SP2 connectors fasten to a single plate, or a double plate, respectively. See Table 13 of this report for allowable loads and fastener schedules.

3.14 SS STUD SHOES

SS stud shoes are formed from No.18 gage galvanized steel complying with ASTM A653 FS with a minimum yield strength of 28,000 psi (193 MPa) and a minimum tensile strength of 38,000 psi (262 MPa). The stud shoes reinforce notched wood studs. See Table 14 of this report for allowable loads and fastener schedules.

3.15 THG2A SKEWED TRUSS GIRDER HANGERS

THG2A hangers are formed from No. 10 gage galvanized steel, complying with ASTM A653 SS Grade 33, with a minimum yield strength of 33,000 psi (227 MPa) and a minimum tensile strength of 45,000 psi (310 MPa). The hangers allow the 45 degree attachment of a multiple girder truss to a carrying member that is attached to a vertical component with two 3/4 inch (19 mm) diameter machine bolts. Use of the hangers to laterally support members is beyond the scope of this report. See Table 15 of this report for details, allowable loads, and fastener schedules.

3.16 TWB T-Type Wall Bracing

The TWB10, TWB12 and TWB14 bracing are formed from No. 22 gage (0.030 inch) galvanized steel complying with ASTM A653 FS, having a minimum yield strength of 28,000 psi (193 MPa) and a minimum tensile strength of 38,000 psi (262 MPa). The flanges are 9/16 inch (143 mm) wide. The kerf leg is 9/16 inch (14.3 mm) deep with an overall width of 1/8 inch (3.2 mm). The wall bracings provide racking resistance for wood-framed walls during construction. See Table 16 of this report for details and fastener schedules.

4.0 DESIGN AND INSTALLATION

4.1 DESIGN CRITERIA

Load capacities shall be limited to lumber members with a minimum specific gravity of 0.50. Adjustments to allowable loads are permitted in accordance with the applicable code as referenced in this report. The maximum adjusted load shall not exceed the maximum design load shown in the tables.

The allowable loads for Simpson Strong-Tie Connectors are based on the lowest load obtained from comparing the following:

1. Test load, at which 1/8 inch (3.2 mm) deflection occurs at either end.
2. Lowest ultimate test load divided by 3.0.
3. Allowable loads on fasteners and wood, calculated in accordance with the applicable code as referenced in this report.

4.2 INSTALLATION

Connectors shall be installed in accordance with this report and the manufacturers installation instructions. Current copies of the installation instructions shall be available at all times on the job site during installation.

4.3 NAILS

Nails used with the Simpson Strong-Tie products described in this report shall comply with Federal Specification FF-N-105B and shall have the following minimum bending yield strengths, F_{yb}:

PENNYWEIGHT, COMMON TYPE	NAIL DIAMETER (in.)	F_{yb} (psi)
8d	0.131	100,000
10d	0.148	90,000
12d	0.148	90,000
16d	0.162	90,000

For SI: 1 in. = 25.4 mm, 1 psi = 6.89 kPa.

4.4 SHEET METAL COATING

Galvanized connectors conform to ASTM A653, G 60.

4.5 BOLTS

References to anchors, bolts or MB's (machine bolts) are for structural quality studs or through bolts equal to or better than ASTM Standard A307, Grade A.

5.0 IDENTIFICATION

Connectors described in this report shall be stamped with the words "Simpson Strong-Tie", the model number, and the National Evaluation Service report number, for field identification.

6.0 EVIDENCE SUBMITTED

6.1 Manufacturer's descriptive literature and published installation instructions dated January 1996.

6.2 Load tests performed in accordance with applicable provisions of ASTM D1761 witnessed by TEI Consulting Engineers, signed by Rostam Esfandiari, P.E.

6.3 Structural calculations prepared by Simpson Strong-Tie Co., Inc., signed and sealed by Karen W. Colonias, P.E., Daphne N. Schonert, P.E., and Evon M. C. Ballash, P.E.

6.4 LSU Series--torsional capacity tests performed in accordance with ASTM D1761, prepared by Testing Engineers Incorporated, File 01325, Lab MN087, dated November 30, 1981, signed by Dushyant Manmohan.

6.5 Simpson Strong-Tie Welding Operations and Procedures Manual, dated October 20, 1998, Revision 8. Signed by representatives of Simpson Strong-Tie and Professional Services Industries.

7.0 CONDITIONS OF USE

The National Evaluation Service Committee finds that the Simpson Strong-Tie® Connectors described in this report comply with the requirements of the 2000 *International Building Code*, the *BOCA National Building Code/1999*, the 1999 *Standard Building Code*, the 2000 *International Residential Code*, and the 1997 *Uniform Building Code*, subject to the following conditions:

7.1 Loads shall not exceed values shown in the tables of this report. These loads are based on the use of the tabulated fasteners, wood species with a specific gravity of 0.50 or higher, lumber moisture content less than 19 percent, and a maximum in-service temperature of 100 °F (37.8 °C).

7.2 Framing members are designed in accordance with the requirements of the applicable code, as referenced in this report.

7.3 Load capacities of connectors used under conditions different from those indicated in Section 7.1 of this report, are beyond the scope of this report and shall be verified by tests or calculations by a registered engineer.

7.4 Beams or headers shall have the following minimum widths, based on nail sizes attaching the hanger to the beams or headers:

NAIL SIZE	BEAM OR HEADER WIDTH (in.)
8d common	1.57
10d common	1.78
16d common	1.94

For SI: 1 in. = 25.4 mm.

7.5 Design calculations and details for specific applications shall be furnished to the code official verifying compliance with this report and the 2000 *International Building Code*, the *BOCA National Building Code/1999*, the 1999 *Standard Building Code*, the 2000 *International Residential Code*, and the 1997 *Uniform Building Code*, as applicable. The individual preparing such documents shall posses the necessary credentials regarding competency and qualifications as required by the applicable code and the professional registration laws of the state where the construction is undertaken.

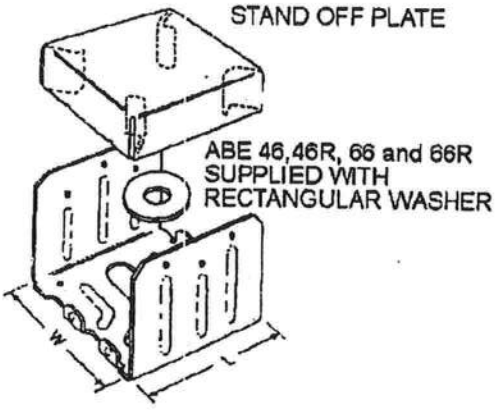
7.6 This report is subject to periodic re-examination. For information on the current status, consult the evaluation report listing or contact the NES.

Table 1 — ABE ADJUSTABLE POST BASES [1,2,3]

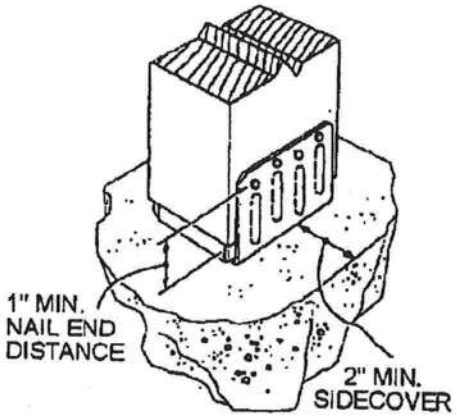
MODEL NO.	DIMENSIONS		FASTENERS [5]		ALLOWABLE LOADS [6]	
	W (in.)	L (in.)	Post (qty - size)	Anchor (in.)	Uplift [7,8] (133/160) (lbf)	Down (100) (lbf)
ABE44	3 9/16	3-1/2	6 - 10d	1/2 MB	520	6665

For SI: 1 in. = 25.4 mm; 1 psi = 6.89 kPa; 1 lbf = 4.45 N.

1. Use a 1/2 inch diameter anchor bolt embedded a minimum of 4 inches into the concrete in accordance with the applicable code.
2. Anchor bolt type, length and embedment to be specified by the designer, shall be designed to resist the uplift force, and shall be specified on the approved construction documents.
3. Minimum side cover shall be 2 inches.
4. Minimum concrete strength shall be 2,000 psi.
5. Nails shall be 0.148 inch in diameter by 3 inches long (10d common).
6. Loads shall not be increased for short-term load duration.
7. Both 133 % and 160 % load durations shall be limited to the loads listed.
8. A maximum value of 133 % load duration shall be used for wind and seismic loading in regions adopting the Uniform Building Code.



ABE 44



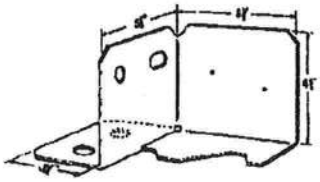
Typical ABE Installation

Table 2 — CBA ADJUSTABLE COLUMN BASES [1,2]

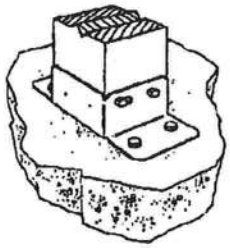
MODEL NO.	FASTENERS		ALLOWABLE UPLIFT LOAD [4,5,6,8,9] (133/160) (lbf)
	Anchor [3] qty-dia (in.)	Post qty-dia (in.)	
CBA66	4 - 3/4	2 - 5/8 MB	3,000

For SI: 1 in. = 25.4 mm; 1 psi = 6.89 kPa; 1 lbf = 4.45 N.

1. Minimum concrete strength shall be 2,000 psi.
2. Anchor bolt side cover shall be as specified on the approved construction documents.
3. Anchor bolt type, length and embedment to be specified by the designer, shall be designed to resist the uplift force, and shall be specified on the approved construction documents.
4. Loads shall not be increased for short-term load duration.
5. Loads apply only when the products are installed in pairs.
6. Loads are in pounds.
7. Allowable load has been increased 33 % and 60 % for wind or earthquake loading with no further increase allowed.
8. Both 133 % and 160 % load durations shall be limited to the loads listed.
9. A maximum value of 133 % load duration shall be used for wind and seismic loading in regions adopting the Uniform Building Code.



CBA66



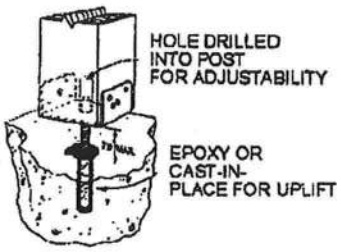
Typical CBA66 Installation

Table 3 — EPB44T ELEVATED POST BASE

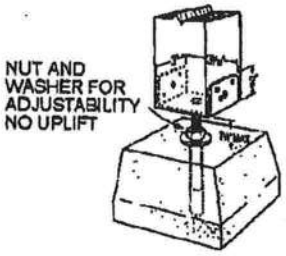
MODEL NO.	NAILS (qty - size)	ALLOWABLE LOADS (lbf)			
		Uplift		Lateral [4,5,6] (133/160)	Down (100)
		(133)	(160)		
EPB44T	6 - 10d	1130	1165	410	3275

For SI: 1 lbf = 4.45 N.

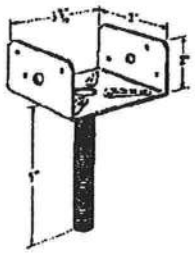
1. Load shall not be increased for short-term loading.
2. Uplift & lateral loads require the threaded rod to be set in wet concrete or attached to cured concrete with epoxy.
3. Lateral load is for both perpendicular and parallel to the connector directions.
4. Allowable load has been increased 33 % and 60 % for wind or earthquake loading with no further increases allowed.
5. Both 133 % and 160 % load durations shall be limited to the loads listed.
6. A maximum value of 133 % load duration shall be used for wind and seismic loading in regions adopting the Uniform Building Code.



EPB44T Installed with Epoxy



EPB44T Installed with Nut and Washer (not supplied)



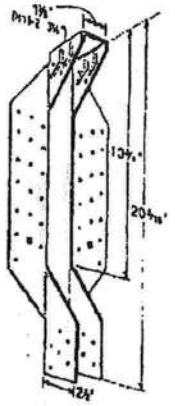
EPB44T

Table 4 — HURRICANE TIES

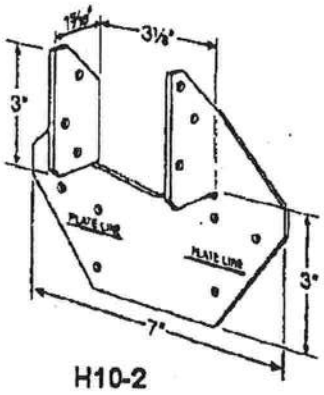
MODEL NO.	FASTENERS (qty - size)			ALLOWABLE LOADS [1,2,4,5] (lbf)			
	To Rafters	To Plates	To Studs	Uplift		Lateral (133/160)[4]	
				(133)	(160)	F1	F2
H2.5	---	4-8d×1-1/2	5-8d×1-1/2	400	400	---	---
H10-2	6-10d	6-10d	---	760	760	455	395
H15	4-10d×1-1/2	4-10d×1-1/2	12-10d×1-1/2	1300	1300	575	---
H15-2	4-10d×1-1/2	4-10d×1-1/2	12-10d×1-1/2	1300	1300	575	---

For SI: 1 in. = 25.4 mm; 1 lbf = 4.45 N.

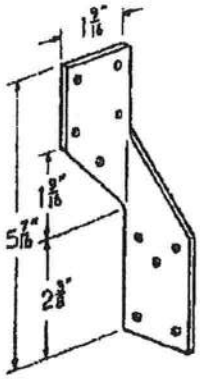
1. Loads have been increased 33 % and 60 % for wind or earthquake loading with no further increases allowed.
2. Allowable loads are for one anchor. A minimum rafter thickness of 2-1/2 inches shall be used when framing anchors are installed on each side of the joist and on the same side of the plate.
3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to all such forces shall be provided where required.
4. Both 133 % and 160 % load durations shall be limited to the loads listed.
5. A maximum value of 133 % load duration shall be used for wind and seismic loading in regions adopting the Uniform Building Code.



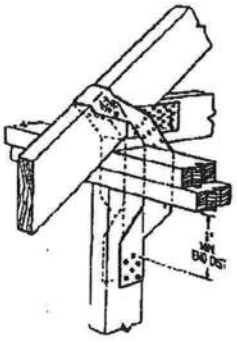
H15
(15-2 similar)



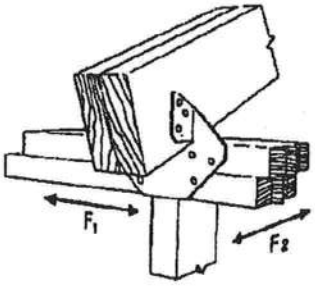
H10-2



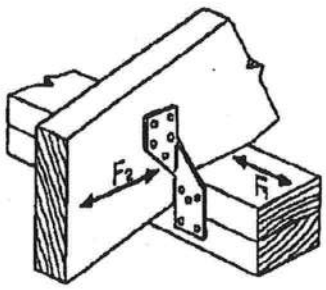
H2.5



Typical H15
Installation



H10-2 Installed



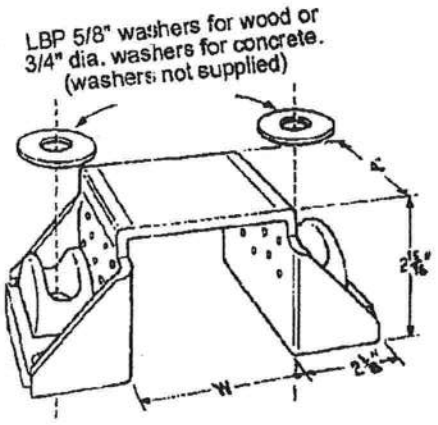
Typical H2.5
Installation

Table 5 — HGT HEAVY GIRDER TIEDOWNS

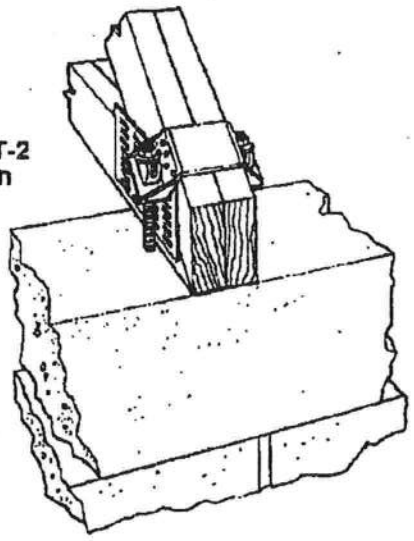
MODEL NO.	W (In.)	O.C. DIM BETWEEN ANCHORS (In.)	FASTENERS			ALLOWABLE UPLIFT (133/160)[5,6] (lbf)
			Anchor Dia. (In.)		Girder (qty - size)	
			Concrete	Wood		
HGT-2	3-7/16	5-3/4	3/4	LBP 5/8	16-10d	10980
HGT-3	5-1/8	7-1-2	3/4	LBP 5/8	16-10d	10530
HGT-4	6-3/4	9-3/6	3/4	LBP 5/8	16-10d	10530

For SI: 1 in. = 25.4 mm; 1 lbf = 4.45 N.

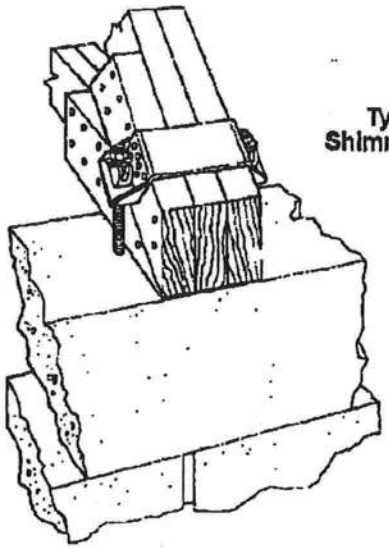
1. Attached members shall be designed to resist applied loads.
2. Allowable loads have been increased 33 % and 60 % for wind or earthquake loading with no further increase allowed.
3. When the HGT-3 is used with a 2-ply girder or beam, shimming shall be required. Shimming shall be a similar size and grade of lumber as the girder, and the entire assembly shall be fastened to act as one unit.
4. Anchor bolt type, length and embedment to be specified by the designer, shall be designed to resist the uplift force, and shall be specified on the approved construction documents.
5. Both 133 % and 160 % load durations shall be limited to the loads listed.
6. A maximum value of 133 % load duration shall be used for wind and seismic loading in regions adopting the Uniform Building Code.



HGT-2
(HGT-3 & 4 similiar)



Typical HGT-2 Installation



Typical HGT-3 Shimmed Installation

Table 6 — LSSU FIELD SLOPE AND SKEWABLE HANGERS [1,2]

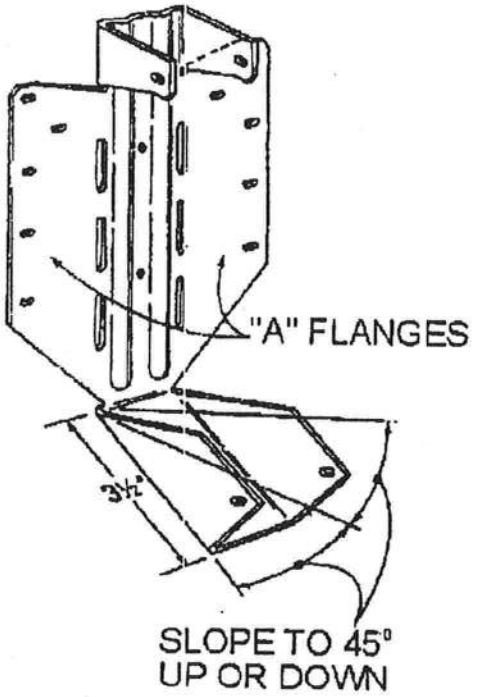
MODEL NO.	DIMENSIONS		FASTENERS [3]		ALLOWABLE LOADS					
					Uplift [4,6]		Slope Only		Skewed [3,5]	
	W (in.)	H (in.)	Header (qty-size)	Joist (qty-size)	(133)	(160)	Norm (100)	Max (125)	Norm (100)	Max (125)
LSSU28	1-9/16	7-1/8	10-10d	5-10d×1-1/2	450	450	1110	1390	990	990
LSSU210	1-9/16	8-1/2	10-10d	7-10d×1-1/2	730	785	1110	1390	1000	1205
LSSU125	1-13/16	8-1/2	10-10d	7-10d×1-1/2	730	785	1110	1390	1000	1205
LSSU135	5-5/16	8-1/2	10-10d	7-10d×1-1/2	730	785	1110	1390	1000	1205

For SI: 1 in. = 25.4 mm; 1 lbf = 4.45 N.

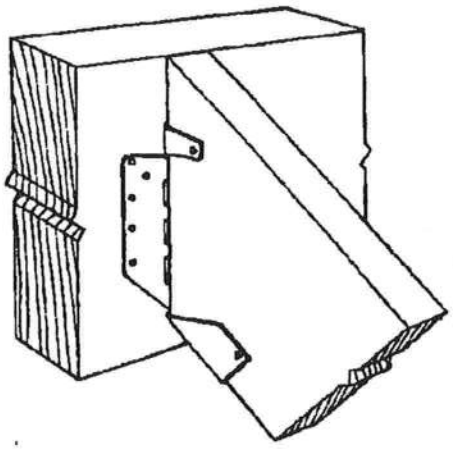
1. Loads are in pounds force.
2. Torsional capacity is 75 pounds times joist depth, summarized as follows:

JOIST DEPTH (in.)	TORSIONAL CAPACITY (lbf)
8	600
10	750

3. Nails shall be 0.148 inch in diameter by 3 inches long (10d common) and 0.148 inch in diameter by 1½ inches long for N10.
4. Uplift loads have been increased 33 % and 60 % for wind or earthquake loading with no further increase allowed.
5. Use nine (9) header nails for skewed LSSU28, LSSU210, LSSU125, LSSU135.
6. A maximum value of 133 % load duration shall be used for wind and seismic loading in regions adopting the Uniform Building Code.



LSSU28



Typical LSSU Sloped Installation

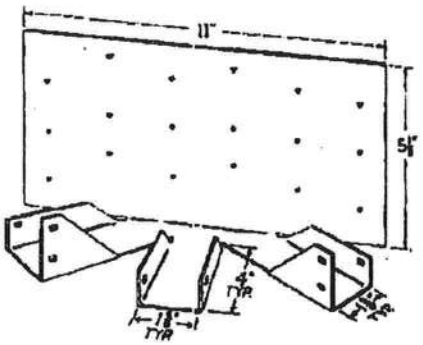
Table 7 — LTHMA LIGHT MULTIPLE TRUSS HANGER

MODEL NO.	HEADER NOMINAL SIZE (In.)	FASTENERS (qty - size)		
		Header	Hips	Jack
LTHMA	1 ply 2×4	12-10d×1-1/2	6-10d×1-1/2	2-10d×1-1/2
	2 ply 2×4	12-10d	6-10d×1-1/2	2-10d×1-1/2
	1 ply 2×6	18-10d×1-1/2	6-10d×1-1/2	2-10d×1-1/2
	2 ply 2×6	18-10d	6-10d×1-1/2	2-10d×1-1/2

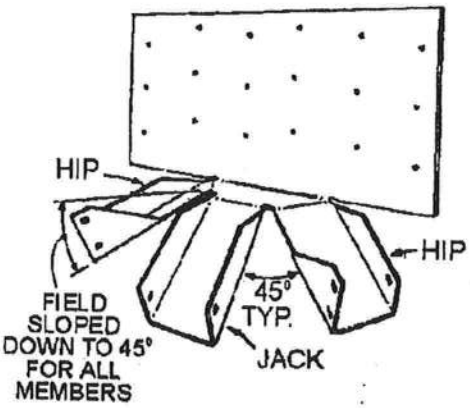
MODEL NO.	HEADER NOMINAL SIZE (in.)	WOOD SPECIES	ALLOWABLE LOADS (lbf)											
			Uplift (133/160)[6]			Floor (100)			Snow (115)			Roof (125/133/160)[7,8]		
			Hip	Jack	Total	Hip	Jack	Total	Hip	Jack	Total	Hip	Jack	Total
LTHMA	1 ply 2×4	Douglas Fir	55	20	130	485	110	1080	540	125	1205	540	125	1205
	2 ply 2×4		55	20	130	600	130	1330	675	150	1500	675	150	1500
	1 ply 2×6		55	20	130	635	140	1410	635	140	1410	635	140	1410
	2 ply 2×6		85	25	195	900	200	2000	1035	230	2300	1050	240	2340

For SI: 1 in. = 25.4 mm; 1 lbf = 4.45 N.

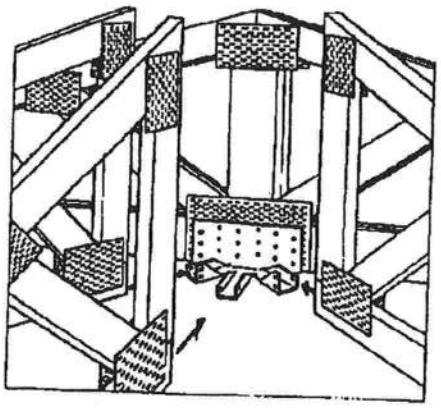
1. The total load is the sum of all three carried members.
2. Uplift loads include 133 % and 160 % increase for wind and earthquake loading with no further increases allowed.
3. Snow and roof loads are 115 % and 125 % of floor, respectively, unless limited by other criteria.
4. Combine two hips and one jack load for total capacity.
5. Total load shall be evenly distributed about the centerline to avoid eccentric loading.
6. Both 133 % and 160 % load durations shall be limited to the loads listed.
7. 125%, 133 % and 160 % load durations shall be limited to the loads listed.
8. A maximum value of 133 % load duration shall be used for wind and seismic loading in regions adopting the Uniform Building Code



LTHMA with jacks and hips non-sloped



LTHMA with jacks and hips sloped down 45°



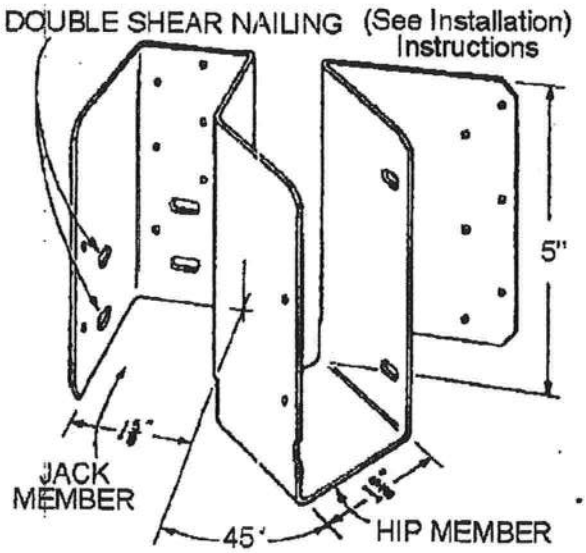
Typical LTHMA Installation

Table 8 — LTHJ HIP/JACK HANGERS [1]

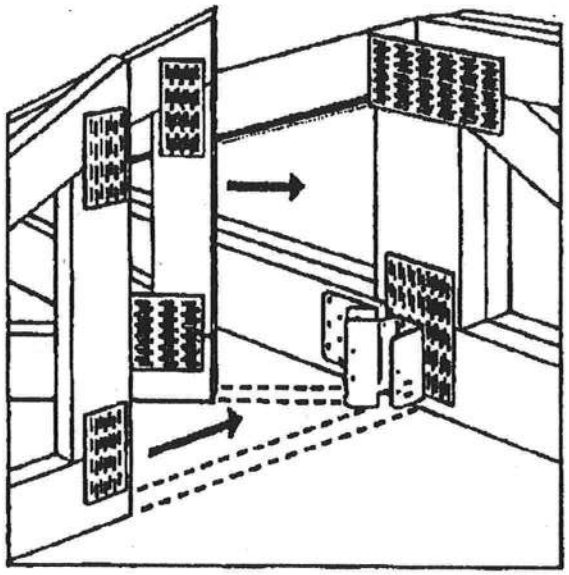
MODEL NO.	FASTENERS [2]			AVG ULT (lbf)	ALLOWABLE LOADS [3,4] (lbf)				
					Uplift			Roof	
	Header (qty-size)	Hip (qty-size)	Jack (qty-size)		(133)	(160)	Floor (100)	Snow (115)	Const (125)
LTHJR/L	12-10d	4-10d×1-1/2	2-10d×1-1/2 and 2-10d	7,317	HIP				
					490	590	1150	1320	1435
					JACK				
					250	250	385	440	480

For SI: 1 in. = 25.4 mm; 1 lbf = 4.45 N.

1. Use of the hanger to laterally support members is beyond the scope of this report.
2. Nails shall be 0.148 inch in diameter by 3 inches long (10d common) and 0.148 inch in diameter by 1½ inches long for N10. The 16d nails shall be 0.162 inch in diameter by 3½ inches long.
3. Distribute 75 % maximum of the total load to the hip member and 25 % maximum to the jack member.
4. Uplift loads have been increased 33 % and 60 % for wind or earthquake loading with no further increase allowed.



LTHJR
Hip Skewed 45° Right
(LTHJL similar)



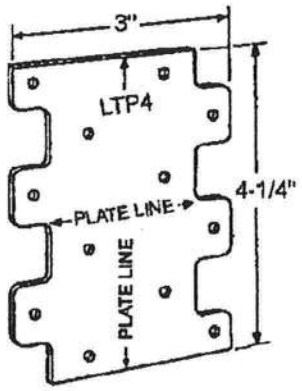
Typical LTHJL Installation

Table 9 — LTP LATERAL TIE PLATE

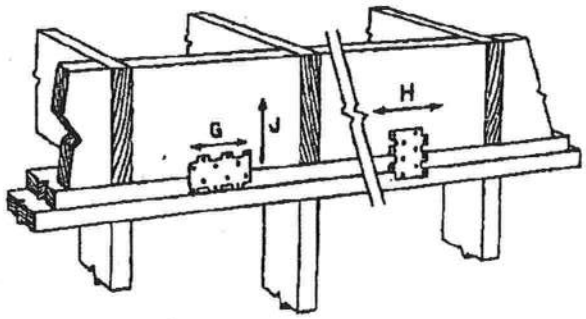
MODEL NO.	FASTENERS (qty-size)		DIRECTION OF LOAD	ALLOWABLE LOADS [1] (lbf)		
	Plates	Joist		(100)	(125)	(133)
LTP4	6-8d×1-1/2	6-8d×1-1/2	G	515	645	685
	6-8d×1-1/2	6-8d×1-1/2	J	515	645	685
	6-8d×1-1/2	6-8d×1-1/2	H	515	645	685

For SI: 1 in. = 25.4 mm; 1 lbf = 4.45 N.

1. 125 % and 133 % values are permitted to be used for roof and wind or earthquake loading, respectively.



LTP4
Lateral Tie Plate



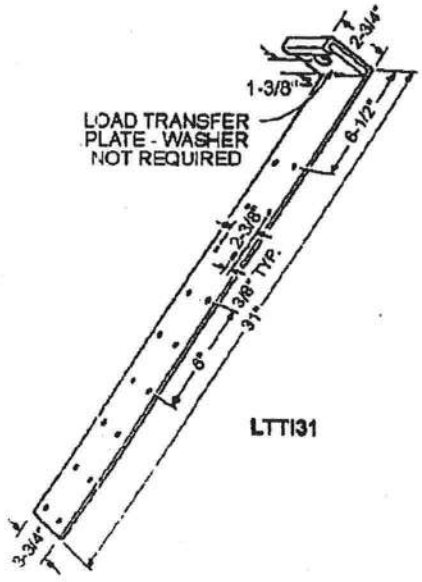
Typical LTP4 Installations to Transfer Shear Forces

Table 10 — LTTI TENSION TIE

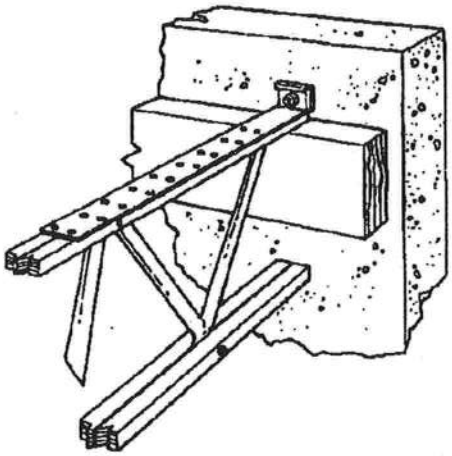
MODEL NO.	DIMENSIONS (In.)		FASTENERS		ALLOWABLE LOADS [2] (lbf)	
					Douglas Fir	
	W	L	Anchor Diameter (In.)	Nails (qty-size)	Tension (133)	Compression (133)
LTT131	3-3/4	31	5/8	18-10d×1-1/2	1805	305

For SI: 1 in. = 25.4 mm; 1 lbf = 4.45 N.

1. Anchor bolt type, length and embedment to be specified by the designer, shall be designed to resist the tension force, and shall be specified on the approved construction documents.
2. Allowable loads have been increased 33 % for wind or earthquake loads with no further increases allowed.



LTTI31



Typical LTTI31 Installation

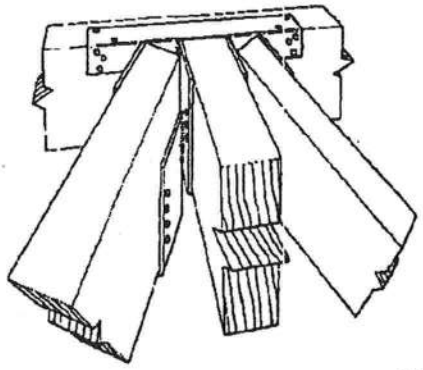
Table 11 — MSC MULTIPLE SEAT CONNECTORS

MODEL NO.	DIMENSIONS (In.)				HIPS		FASTENERS (qty-size)	
	W	H	TF	L	Max. Skew	Max. Slope	Header	Joists
MSC2	1-9/16	5-1/2 min.	2-7/8	12	45°	0°	10-16d	18-10d×1-1/2
						45°	10-16d	26-10d×1-1/2
MSC1.81	1-13/16	5-1/2 min.	2-7/8	12	45°	0°	10-16d	18-10d×1-1/2
						45°	10-16d	26-10d×1-1/2
MSC4	3-9/16	7-1/2 min.	2-7/8	18	45°	0°	10-16d	18-10d
						45°	10-16d	26-10d

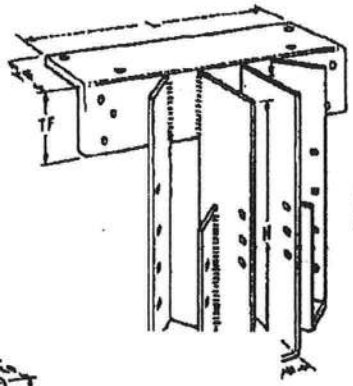
MODEL NO.	HIPS		ALLOWABLE LOADS (lbf)					
	Max. Skew	Max. Slope	Floor (100)			Snow (115) / Roof (125)[7]		
			Hip	Jack	Total	Hip	Jack	Total
MSC2	45°	0°	2535	1265	6335	2535	1265	6335
		45°	2010	1005	5025	2010	1005	5025
MSC1.81	45°	0°	2535	1265	6335	2535	1265	6335
		45°	2010	1005	5025	2010	1005	5025
MSC4	45°	0°	3335	1665	8335	3335	1665	8335
		45°	3335	1665	8335	3335	1665	8335

For SI: 1 in. = 25.4 mm, 1 lbf = 4.45 N.

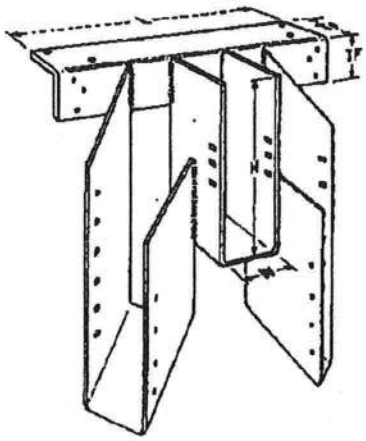
1. Allowable loads are per member.
2. Snow and roof loads are 115 % and 125 % of floor, respectively, unless limited by other criteria.
3. For hips with combined skew and slope angles > 0° and ≤ 45°, use load values for maximum skew = 45°, and maximum slope= 45°.
4. Use total load for cases when there is no center member.
5. W1 = W2 = W3 unless specified otherwise.
6. Total load shall be evenly distributed about the centerline to avoid eccentric loading.
7. Both 115 % and 125 % load durations shall be limited to the loads listed.



Typical MSC410 Installation



MSC1.81 Hips are skewed 45° and sloped 0°



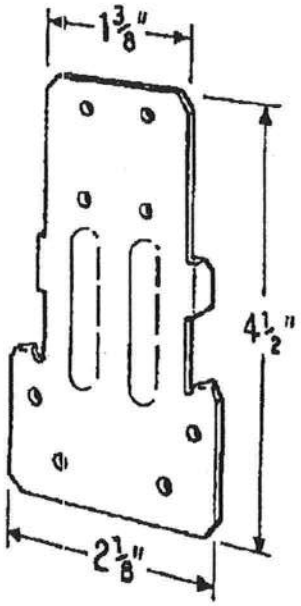
MSC410 Hips are sloped and skewed 45°

Table 12 — RSP4 REVERSIBLE STUD PLATE TIE

MODEL NO.	W (In.)	L (In.)	FASTENERS		ALLOWABLE LOADS (lbf)		
			Stud (qty-size)	Plate (qty-size)	Uplift[4,5] (133/160)	Lateral (133/160)[4,5]	
						F1[1]	F2[2]
RSP4(1)	2-1/8	4-1/2	4-8d×1-1/2	4-8d×1-1/2	315	210	280
RSP4(2)	2-1/8	4-1/2	4-8d×1-1/2	4-8d×1-1/2	450	210	305

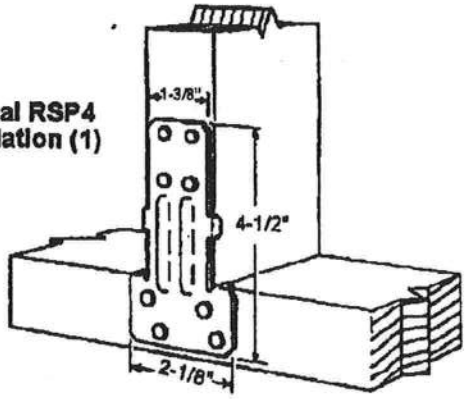
For SI: 1 in. = 25.4 mm; 1 lbf = 4.45 N

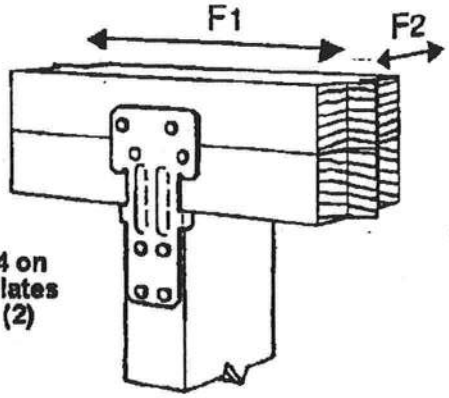
1. "F1" denotes direction parallel to plate load.
2. "F2" denotes direction perpendicular to plate load.
3. Allowable loads have been increased 33 % and 60 % for wind or earthquake load with no further increase allowed.
4. Both 133 % and 160 % load durations shall be limited to the loads listed.
5. A maximum value of 133 % load duration shall be used for wind and seismic loading in regions adopting the Uniform Building Code.



RSP4

Typical RSP4 Installation (1)





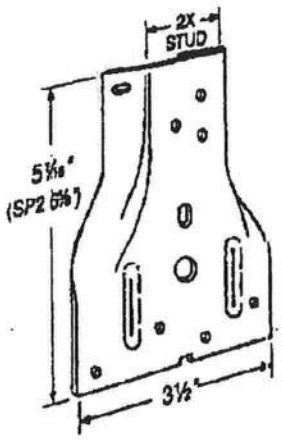
Typical RSP4 on Double Top Plates Installation (2)

TABLE 13 - SP STUD PLATE CONNECTORS

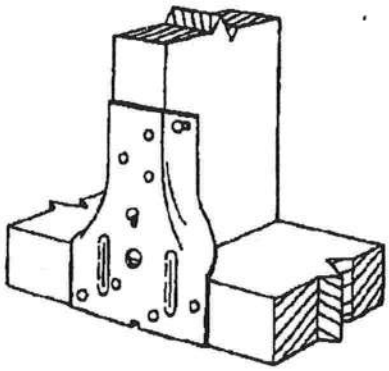
MODEL NO.	FASTENERS [2] (qty-size)		ALLOWABLE UPLIFT LOAD [3,4] (lbf)	
	Stud [1]	Plate	(133)	(160)
SP1	6-10d	4-10d	585	585
SP2	6-10d	6-10d	890	1065

For SI: 1 in. = 25.4 mm; 1 lbf = 4.45 N.

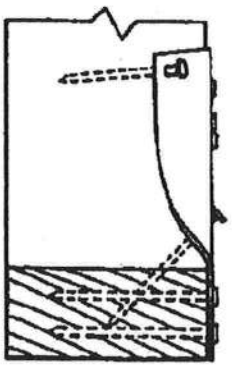
1. One stud nail shall be driven at an angle through the stud into the plate.
2. Nails shall be 0.148 inch in diameter by 3 inches long (10d common).
3. Allowable load includes a 33 % and 60 % increase for wind or earthquake loading with no further increase allowed.
4. A maximum value of 133 % load duration shall be used for wind and seismic loading in regions adopting the Uniform Building Code.



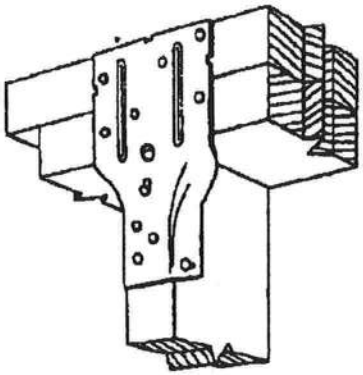
SP1



Typical SP1 Installation



SP1 Nailing Profile



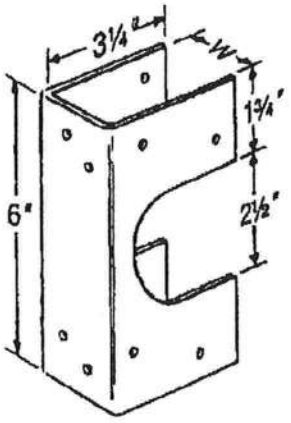
Typical SP2 Installation

Table 14 — SS STUD SHOES

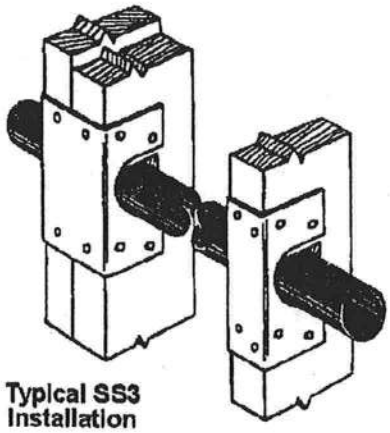
MODEL NO.	W (In.)	FASTENERS[1] (qty-size)	ALLOWABLE LOADS (lbf)		
			Floor (100)	Roof[2] (125)	Uplift (133)
SS1.5	1-9/16	12-10d×1-1/2	500	500	----
SS2.5	2-9/16	12-10d×1-1/2	500	500	----
SS3	3	12-10d	665	785	----
SS4.5	4-9/16	14-10d×1-1/2	665	785	----
HSS2	1-9/16	12-SDS 1/4×1-1/2	1215	1215	1025
HSS2-2	3	12-SDS 1/4×1-1/2	1215	1215	1025
HSS4	3-9/16	12-SDS 1/4×1-1/2	1215	1215	1025

For SI: 1 in. = 25.4 mm; 1 lb. = 4.45 N.

1. Nails shall be 0.148 inch in diameter by 1½ (N10) or 3 inches long (10d common).
2. Roof loads are 125 % of floor loads, unless limited by other criteria.

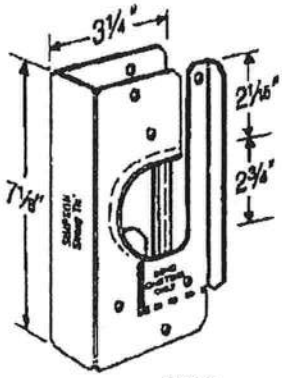


SS

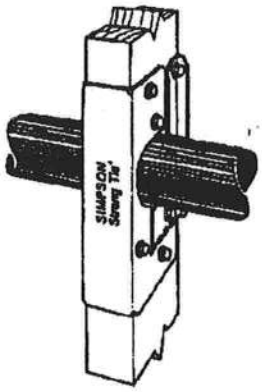


Typical SS3 Installation

Typical SS1.5 Installation



HSS



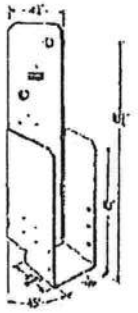
Typical HSS Installation

TABLE 15 — THG2A SKEWED TRUSS GIRDER HANGERS [1]

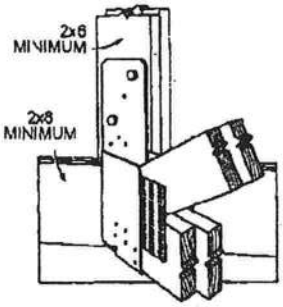
MODEL NO.	FASTENERS [2]			ALLOWABLE LOADS [4] (lbf)				
	Carrying Member		Carried Member (qty-size)	Uplift [3] (133)	Length of Bolt in Carrying Member	Floor (100)	Roof [5]	
	Bolts (in.)	Nails (qty-size)					Snow (115)	Const (125)
THG2AR/L	2-3/4 MB	4-10d	9-10d	1465	1-1/2	1385	1595	1735
					3	2750	3160	3435
					4-1/2	2985	3435	3730
					6	2985	3435	3730

For SI: 1 in. = 25.4 mm; 1 psi = 6.89 kPa; 1 lbf = 4.45 N.

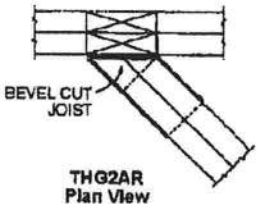
1. Use of the hanger to laterally support trusses is beyond the scope of this report.
2. Nails shall be 0.148 inch in diameter by 3 inches long (10d common).
3. Uplift loads have been increased by 33% for wind or earthquake, with no further increase allowed.
4. The allowable loads given are based on southern pine lumber and the lower of the following: test ultimate divided by three, the load producing 1/8 inch deflection, the bolt values and the seat bearing value at 565 psi plus the allowable joist nail rating.
5. Down loads given include a 25 % increase above the normal allowable load for a seven-day load duration. Load adjustments for other load durations in accordance with the applicable code, as referenced herein, are permitted but shall not exceed the table values.



THG2AR



Typical THG2AR Installation



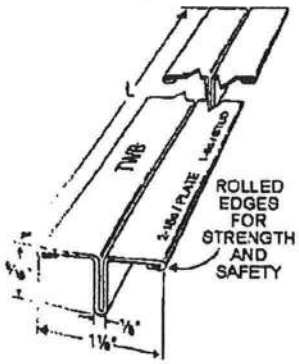
THG2AR
Plan View

TABLE 16 — TWB T-TYPE WALL BRACING [1,2]

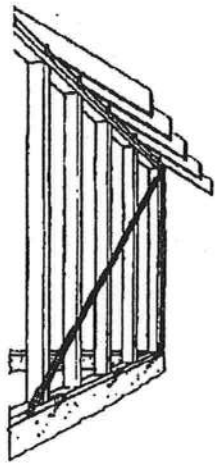
MODEL NO.	LENGTH	ANGLE FOR 8 ft WALL	FASTENERS [3] (qty-size)	
			Plates	Studs
TWB10	9'-3"	55°	2-16d	1-8d
TWB12	11'-4"	45°	2-16d	1-8d
TWB14	14'-2"	45°	2-16d	1-8d

For SI: 1 ft = 304.8 mm; 1 in. = 25.4 mm; 1 lbf = 4.45 N.

1. The TWB is intended to provide racking resistance to woodframe walls during construction. It is not designed to replace the shearwall load-carrying components.
2. The TWB10 shall be limited to a maximum load of 160 pounds. The TWB12 and TWB14 shall be limited to a maximum load of 190 pounds.
3. The 16d nails shall be 0.162 inch in diameter by 3½ inches long; the 8d nails shall be 0.131 inch in diameter by 2½ inches long.



TWB



Typical TWB
Exterior Wall Installation

National Evaluation Service, Inc.

Participating Members:

BOCA Evaluation Services, Inc.	ICBO Evaluation Service, Inc.	SBCCI Public Safety Testing and Evaluation Services, Inc.
4051 West Flossmoor Road Country Club Hills, Illinois 60478-5795 (708) 799-2305	5360 Workman Mill Road Whittier, California 90601-2299 (310) 699-0543	900 Montclair Road, Suite A Birmingham, Alabama 35213-1206 (205) 591-1853

NATIONAL EVALUATION REPORT

NER-443
Reissued December 1, 1996

SIMPSON STRONG-TIE® CONNECTORS

SIMPSON STRONG-TIE® COMPANY, INC.
4637 CHABOT DRIVE, SUITE 200
PLEASANTON, CALIFORNIA 94588

1.0 SUBJECT

Simpson Strong-Tie® Connectors:

1.1 CWB Compression Wall Brace

1.2 GH Girder Hangers

1.3 GLTV and HGLTV Beam Hangers

1.4 LPC4 Light Post Cap

1.5 LS Skewable Angle Series

1.6 LSTA/MSTA Light and Medium Strap Tie Series

1.7 LTB Light Tension Bridging

1.8 MA Series Mudsill Anchors

1.9 PB Series Post Bases

1.10 PC/EPC Series Post Caps

1.11 SP/SPA Stud Plate Ties

1.12 THM-2 Truss Multiple Hanger

1.13 WB/WBC Wall Bracing

1.14 DJT14 Deck Joist Tie

1.15 DBT1 Deck Board Tie

1.16 DPT Deck Post Ties

1.17 DRT 8 Deck Railing Tie

2.0 PROPERTY FOR WHICH EVALUATION IS SOUGHT

Structural Connections for Wood Construction.

3.0 DESCRIPTION

3.1 CWB COMPRESSION WALL BRACE

The CWB106 and CWB126 braces are cold-formed to a 90-degree angle from No. 18 gage galvanized steel complying with ASTM A 653-SQ Grade 33, with a minimum yield strength of 33,000 psi (230 MPa) and a minimum tensile strength of 45,000 psi (310 MPa). Angle legs are $^{15}/_{16}$ inch (23.8 mm) long. One leg provides a staggered pattern of 0.136-inch-diameter (3.45 mm) holes spaced at $^{1}/_{2}$ inch (12.7 mm) on center. The brace ends have angular cuts and a three-hole pattern of 0.171-inch-diameter (4.34 mm) holes. The CWB106 and the CWB126 braces are 9 feet $5^{3}/_{4}$ inches (2.89 m) and 11 feet $4^{3}/_{8}$ inches (3.46 m) long, respectively. The devices are used to brace wood-frame construction, with a single brace considered equivalent to one nominal 1-by-4 wood let-in brace. The braces have been evaluated for both tension and compression loads. The CWB106 and CWB126 braces shall be installed at angles of 60 and 45 degrees from the horizontal, respectively. The wall studs shall be spaced at 16 inches (406 mm) on center, maximum. A 1-inch-deep (25.4 mm) saw cut shall be provided in the studs and plates for installation of the brace. The brace shall be attached at both the top and bottom plates with two 16d common nails and at each intermediate stud with one 8d common nail. Dimension details and fastener schedules shall be in accordance with Table 1.

3.2 GH GIRDER HANGERS

The GH46 and GH48 girder hangers provide support for floor girders connected to concrete or grouted masonry foundation walls, complying with the applicable code. The devices are No. 12 gage painted steel. The U-shaped stirrups are welded to the face of a semi-U top which is 6 inches (153 mm) wide and not less than 6 inches (153 mm) in depth on one face with a 1-inch (25.4 mm) return leg. The device shall be mounted on top of the foundation wall and under a minimum nominal 2-inch-by-6-inch (51 mm by 153 mm) mudsill, which shall be installed in accordance with the applicable code. The steel complies with ASTM A 570 Grade 33, with a minimum yield of 33,000 psi (230 MPa) and a minimum tensile strength of 52,000 psi (360 MPa). Dimension details, allowable loads and fastener schedules shall be in accordance with Table 2.

3.3 GLTV AND HGLTV BEAM HANGERS

The GLTV stirrup is formed from a strip of sheet steel, No. 7 gage by 5 inches (127 mm) wide, bent into a "U" shape and welded to an angled top flange made of No. 3 gage steel. The hangers are designed for use with structural composite lumbers. They shall be installed on a wood header having a minimum allowable compres-

sion perpendicular-to-the-grain value of 500 psi (3447 kPa), or on a steel header. The HGLTV is similar, except that the stirrup is 6 inches (153 mm) wide and the top flange dimension, nailing schedule and welds are increased. The steel complies with ASTM A 570 Grade 33 specifications, with a minimum yield strength of 33,000 psi (230 MPa) and a minimum tensile strength of 52,000 psi (360 MPa). Dimension details, allowable loads and fastener schedules shall be in accordance with Table 3.

3.4 LPC4 LIGHT POST CAP

The light post cap is a two-piece connector formed from No. 18 gage galvanized steel. The connector is designed to join a nominal 4-by post to members $2^1/_2$ to $3^1/_2$ inches (63.5 mm to 88.9 mm) wide. The connectors shall be used in pairs. The steel complies with ASTM A 653-SQ Grade 33, with a minimum yield strength of 33,000 psi (230 MPa) and a minimum tensile strength of 45,000 psi (310 MPa). Allowable loads and the fastener schedule shall be in accordance with Table 4.

3.5 LS SKEWABLE ANGLE SERIES

The skewable angle is formed from No. 18 gage, galvanized, die-formed steel complying with ASTM A 653-LFQ, with a minimum yield strength of 33,000 psi (230 MPa) and a minimum tensile strength of 45,000 psi (310 MPa). The angle is designed with slots at the bend to allow field skewing from 0 to 135 degrees. Allowable loads and fastener schedules shall be in accordance with Table 5.

3.6 LSTA/MSTA LIGHT AND MEDIUM STRAP TIE SERIES

The light and medium straps are designed to act as tension ties between two butting wood members. The straps are formed from Nos. 16, 18 and 20 gage galvanized steel and punched to receive 10d or 16d common nails. The steel complies with ASTM A 653-SQ Grade 40 Special, with a minimum yield strength of 42,000 psi (290 MPa) and a minimum tensile strength of 56,000 psi (390 MPa). Allowable loads and fastener schedules shall be in accordance with Table 6.

3.7 LTB LIGHT TENSION BRIDGING

The LTB is die-formed from No. 22 gage galvanized steel complying with ASTM A 653-LFQ, with a minimum yield strength of 33,000 psi (230 MPa) and minimum tensile strength of 45,000 psi (310 MPa) for LTB21 and LTB42; and a 28,000 psi (195 MPa) yield strength and 38,000 psi (260 MPa) tensile strength for LTB20 and LTB40. The bridging is a tension-type utilizing either a single or an over/under application. The bridging shall be installed in pairs. Installation details and the fastener schedule shall be in accordance with Table 7.

3.8 MA SERIES MUDSILL ANCHORS

The mudsill anchors, formed from No. 16 gage galvanized steel, have a tapered, U-shaped body with the outboard flanges extending $4^1/_2$ inches (114.3 mm) upwards beyond the web section to bear against the sides and top of the mudsill. The web sections are $4^5/_8$ inches (117.5 mm) deep with a top width between outboard flanges of either $3^5/_8$ or $5^5/_8$ inches (92.1 mm or 142.9 mm) for nominal 4-by or 6-by sill plates, respectively. Adjacent to the flanges, at each side of the web, triangular tabs are bent horizontally to act as a gage and as a restraint to the flanges. The flanges are prepunched with $^5/_{32}$-inch-diameter (4 mm) holes at $^1/_4$ inch (6.4 mm) on center, staggered in two rows, for 10d common nails $1^1/_2$ inches (38 mm) long. The nails shall be installed at the edge and top of the mudsill. The steel complies with ASTM A 653-LFQ, with a minimum yield strength of 28,000 psi (195 MPa) and a minimum tensile strength of 38,000 psi (260 MPa). Allowable loads and fastener schedules shall be in accordance with Table 8.

3.9 PB SERIES POST BASES

The post bases consist of No. 12 gage, galvanized, die-formed, channel-shaped members, having deformed prongs protruding from the back of the web in line with each channel flange. The prongs shall be embedded into uncured concrete immediately after screeding to provide shear and uplift resistance for the supported post. The base-channel flanges shall be nailed to the post with 16d common nails. The post bases are manufactured to fit nominal 4-by-4, 4-by-6 and 6-by-6 surfaced and rough-sawn posts. The steel complies with ASTM A 653-CQ, with a minimum yield strength of 28,000 psi (195 MPa) and minimum tensile strength of 38,000 psi (260 MPa). Allowable loads and fastener schedules shall be in accordance with Table 9.

3.10 PC/EPC SERIES POST CAPS

The post caps are die-formed from No. 12 or No. 16 gage galvanized steel into a channel section to support beams on posts. Model numbers with a -16 suffix are formed from No. 16 gage material. The ends of the channel web are cut along each side of the beam seat and bent downward as tabs to engage opposite faces of the post. The post caps are manufactured to fit various combinations of post and beams and shall be attached to both the beam and post with 16d common nails. The EPC post caps are designed for end-post connections in lieu of continuous members. The steel complies with ASTM A 653-CQ and ASTM A 653-LFQ requirements for No. 12 gage and No. 16 gage, respectively, with a minimum yield strength of 28,000 psi (195 MPa) and a minimum tensile strength of 38,000 psi (260 MPa). Dimension details, allowable loads and fastener schedules shall be in accordance with Table 10.

3.11 SP/SPA STUD PLATE TIES

The SP/SPA 4, 6 and 8 stud plate ties are die-formed from No. 20 gage galvanized steel complying with ASTM A 653-LFQ, with a minimum yield strength of 28,000 psi (195 MPa) and a minimum tensile strength of 38,000 psi (260 MPa). The SP tie is attached to the edge of the wood member, while the SPA is twisted and attaches to the face of the member. They are designed to anchor double top plates to the stud. Dimension details, allowable loads and fastener schedules shall be in accordance with Table 11.

3.12 THM-2 TRUSS MULTIPLE HANGER

The THM-2 is designed to carry multiple truss members. The hanger is fabricated from No. 10 gage galvanized steel complying with ASTM A 653-SQ Grade 33, with a minimum yield strength of 33,000 psi (230 MPa) and a minimum tensile strength of 45,000 psi (310 MPa). The code-required minimum-7-bolt diameter distance from the end of the vertical member has been designed into the connector. The bottom cord of the carrying truss shall not exceed a nominal 2-by-8 member in order to maintain the required distance. Allowable loads and fastener schedules shall be in accordance with Table 12.

3.13 WB/WBC WALL BRACING

WB106 and WB126 wall braces are formed from $1^1/_4$-inch-wide (32 mm), No. 18 gage galvanized steel strips, complying with ASTM A 653-SQ Grade 33, with a minimum yield strength of 33,000 psi (230 MPa) and a minimum tensile strength of 45,000 psi (310 MPa). Wall bracing is also available in coiled form designated as WB106C, WB126C and WB143C straps. The braces are used in wood-frame construction, with a pair of braces equivalent to one nominal 1-by-4 let-in brace as prescribed by the applicable code. Braces shall be installed opposing each other, with each brace installed at an angle not more than 60 degrees nor less than 45 degrees from the horizontal. Four nail holes are provided at the ends of each brace for 16d nails, and shall be used to attach the top and bottom plates with two 16d common nails. Additionally, $^9/_{64}$-inch-diameter (3.6 mm) holes are punched at 1 inch (25.4 mm) on center for the entire length of the brace in two staggered rows spaced $^3/_4$ inch (19 mm) apart, to permit attachment to each intermediate stud with one 8d common nail. Dimension details and fastener schedules shall be in accordance with Table 13.

3.14 DJT14 DECK JOIST TIE

The tie is used to attach joists to posts and is die-formed from No. 14 gage galvanized steel. The connector shall be bolted or nailed to

a minimum nominal 4-by-4 post and a minimum nominal 2-by-4 joist. Steel complies with ASTM A 653-CQ, with a minimum yield strength of 28,000 psi (195 MPa) and a minimum tensile strength of 38,000 psi (260 MPa). Allowable loads and fastener schedules shall be in accordance with Table 14.

3.15 DBT1 DECK BOARD TIE

The tie is used to attach minimum $1^1/_4$-inch-thick (32 mm) board decking to joists in conjunction with toe-nailed 16d common nails and 10d by $1^1/_2$-inch (38 mm) common nails. Ties are die-formed from No. 18 gage galvanized steel complying with ASTM A 653-CQ, with a minimum yield strength of 28,000 psi (195 MPa) and a minimum tensile strength of 38,000 psi (260 MPa). Fastener installation shall be in accordance with Table 15.

3.16 DPT DECK POST TIES

The DPT 5, DPT 6 and DPT 7 are die-formed from No. 14 gage galvanized steel. The DPT 5 attaches 2-by-4 posts to the outside of a deck. The DPT 7 attaches 4-by-4 posts to the edges of a deck. Connectors shall be used in pairs spaced 5 inches (127 mm) on center. The DPT 6 attaches 4-by posts to the top surface of a deck. For the DPT 5 and DPT 7, the deck construction shall include a nominal 2-by-10 minimum fascia and a nominal 2-by-8 minimum rim joist. For the DPT 6, the deck construction shall include a nominal 2-by-8 minimum fascia and a nominal 2-by-6 minimum rim joist. Steel for the post ties complies with ASTM A 653-CQ, with a minimum yield strength of 28,000 psi (195 MPa) and a minimum tensile strength of 38,000 psi (260 MPa). Allowable loads and fastener schedules shall be in accordance with Table 16.

3.17 DRT 8 DECK RAILING TIE

The DRT 8 is die-formed from No. 18 gage galvanized steel, and connects the handrail to a post. The connector shall be attached to the post and handrail with wood screws. Steel complies with ASTM A 653-CQ, with a minimum yield strength of 28,000 psi (195 MPa) and a minimum tensile strength of 38,000 psi (260 MPa). Allowable loads and the fastener schedule shall be in accordance with Table 17.

3.18 MATERIALS

Galvanized connectors conform to ASTM A 653, G 60. Nongalvanized connectors have a painted coating.

Nails shall be common nails and have a diameter, length and bending yield strength complying with the values noted in the 1991 National Design Specification for Wood Construction, except for the length of special nails noted in Tables 8, 12, 15 and 16.

3.19 DESIGN

The design of the connected wood members shall be submitted to and approved by the building official. Tabulated design loads for the Simpson Strong-Tie connectors are based on the following criteria:

- Test load that causes $^1/_8$-inch (3 mm) deflection.
- Lowest ultimate test load with a safety factor of 3.
- Allowable fastener and compression perpendicular-to-grain values in accordance with the 1991 National Design Specification for Wood Construction, based on wood with a specific gravity of 0.50, such as Douglas fir-larch, except for allowable loads noted in Tables 16 and 17, which have also been evaluated for redwood.
- Torsional capacity is based on the ability of the joist hanger to resist 75 pounds (334 N) times the depth of the joist at 0.125 inch (3 mm) of movement.

4.0 INSTALLATION

Load capacities shown are based on wood with a minimum specific gravity of 0.50 and a moisture content of less than 19 percent. Tabulated allowable design loads are for normal duration of loading. Adjustments to these values are permitted for other durations of loading, i.e., plus 15 percent for two months duration (snow), or plus 33 percent for wind or earthquake. Tabulated allowable design loads shall be reduced by 10 percent for full design load applied longer than 10 years. The resulting allowable design load after duration-of-load adjustments shall not exceed the maximum design load indicated in the tables.

Connector installation shall comply with this report and the manufacturer's installation instructions. Connectors for Wood Construction, Product and Instruction Manual, dated January 1995. A copy of these instructions and this report shall be available at all times on the jobsite during installation.

For conditions of high temperature, a load reduction is required. See the National Design Specification for temperature effects.

5.0 IDENTIFICATION

Each of the connectors described in this report shall be stamped with the words "Simpson Strong-Tie," the model number and the evaluation report number (NER-443) for field identification.

6.0 EVIDENCE SUBMITTED

6.1 Manufacturer's descriptive literature and published installation instructions.

6.2 Load tests performed by TEI Consulting Engineers and signed by Rostam Esfandiari, P.E.:

Item	Work No.	Date
CWB	85328	April 22, 1986
CWB	85328-62	August 25, 1986
GH46	93085.120	November 29, 1993
GLTV	95001.100	July 26, 1995
HGLTV	95001.099	July 12, 1995
LPC4	89008.136	February 27, 1990
LPC4	89008.135	February 27, 1990
LS30	93085.118	November 24, 1993
LS50	93085.119	November 24, 1993
LS90	89008-46	August 2, 1989
LS90	89008-30	July 20, 1989
LS90	89008-47	August 2, 1989
MA4	OL16-290-424	February 22, 1974
PB66	86085	August 5, 1986
PB44	88018.190	March 31, 1989
PB44	88018.185	March 31, 1989
PB44	89008-25	June 5, 1989
PC-46	85328-139	February 23, 1987
EPC-46	85328-140	February 23, 1987
PC46	88018-20	April 29, 1988
EPC46	88-18-19	April 29, 1988
PC44-16	88018-116	October 19, 1988
EPC44-16	88018-115	October 19, 1988
PC48-16	88018-117	October 19, 1988
GLTI430	87005-16	May 18, 1987
EPC48-16	88018-118	October 19, 1988
PC44	88018-97	October 7, 1988
EPC44	88018-96	October 6, 1988
PC48	88018.103	October 12, 1988
EPC48	88018-102	October 12, 1988
LSTA	89008-48	August 8, 1989
THM-2	89008-95	November 15, 1989

Item	Work No.	Date
THM-2	88-18-57	July 29, 1988
DJT14	89008-101	November 15, 1989
DPT5 & DRT8	91006-38, 91006-44	July 31, 1991
DPT6 & DRT8	91006-41	July 31, 1991
DPT6 & DRT8	91006-98	October 23, 1991
DPT7 & DRT8	91006-39, 91006-45	July 31, 1991

6.3 Structural calculations prepared by Simpson Strong-Tie Company, Inc., and signed and sealed by Karen W. Colonias, P.E.:

Item	Date
CWB	April 12, 1990
GH	April 12, 1990, and February 28, 1996
GLTV/HGLTV	April 12, 1990, and February 22, 1996
LPC4	April 12, 1990
LS	April 12, 1990, and February 28, 1996
LSTA/MSTA	April 12, 1990, revised May 10, 1996
LTB	February 12, 1990
MA	June 23, 1987
PB	July 26, 1988, revised April 12, 1990
PC/EPC	April 12, 1990, revised May 10, 1996
SP4, 6, 8	August 10, 1989
LSTA	November 3, 1989
THM2	August 18, 1988, revised August 24, 1990
GTLV/HGLTV	August 24, 1990
WB/WBC	September 20, 1991
DJT14	October 25, 1991, revised May 10, 1995

6.4 Load tests performed by TEI Consultants and signed by Roger Tansley, P.E.:

Item	Work No.	Date
GLTV410	95001.100	July 26, 1995
HGLTV410	95001.099	July 12, 1995

6.5 Letter dated July 14, 1995, from TEI Consultants and signed by Roger Tansley, P.E., regarding verification of testing in accordance with ASTM D 1761.

6.6 Quality control manual, dated December 1994.

7.0 CONDITIONS OF USE

The National Evaluation Service Committee finds that the Simpson Strong-Tie® connectors described in this report comply with the BOCA National Building Code/1996, the 1994 Standard Building Code, and the 1994 Uniform Building Code, subject to the following conditions:

7.1 Connector loads are determined in accordance with the applicable code. Loads in the tables are predicated on the use of fasteners indicated in the tables, wood with a minimum specific gravity of 0.50 (except redwood) and a lumber moisture content less than 19 percent. Where redwood is referenced, specific gravity is assumed to be 0.37.

7.2 The scope of this evaluation report is limited to use of these connectors with lumber that has not been pressure-treated with chemicals such as those for fire-retardant treatment and preservative treatment.

7.3 Framing members shall be designed in accordance with the requirements referenced in the applicable code.

7.4 Loads for duration of load other than normal shall be adjusted in accordance with the 1991 National Design Specification for Wood Construction up to the "maximum" allowable tabulated load.

7.5 Beams or headers shall have the following minimum widths based on nail sizes attaching the hanger to the beams or headers, except that the GLTV and HGLTV connectors require a minimum 3-inch (76.2 mm) header:

Nail Size	Beam or Header Width	$F_{y\theta}$
6d	1.36 inches (34.5 mm)	100,000 psi (690 MPa)
8d	1.57 inches (39.9 mm)	100,000 psi (690 MPa)
10d	1.78 inches (45.2 mm)	90,000 psi (620 MPa)
16d	1.94 inches (49.3 mm)	90,000 psi (620 MPa)

7.6 Plans specifying connectors listed in this report shall be accompanied by calculations demonstrating that the allowable loads noted in this report are not exceeded. These calculations shall be signed and sealed by a registered design professional, as required by the applicable code.

7.7 The connectors shall be manufactured, identified and installed in accordance with this report and the manufacturer's installation instructions.

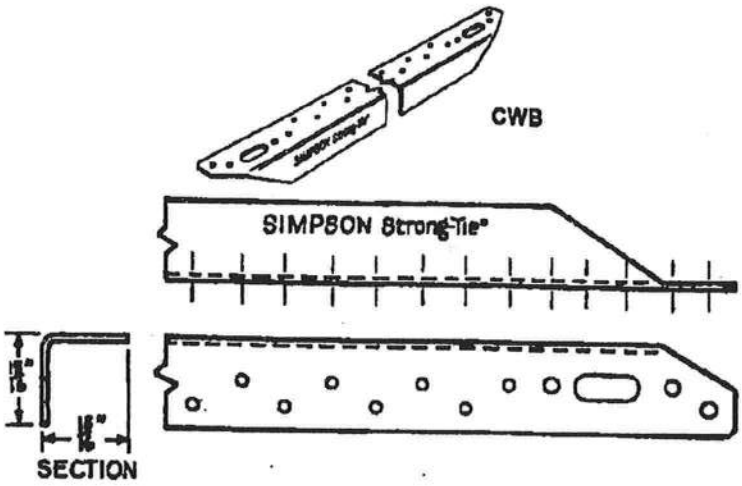
7.8 Connectors have not been evaluated for simultaneous loadings such as wind or seismic plus live and dead loads.

7.9 This report is subject to periodic re-examination. For information on the current status, consult the evaluation report listing or contact one of the participating members of the NES.

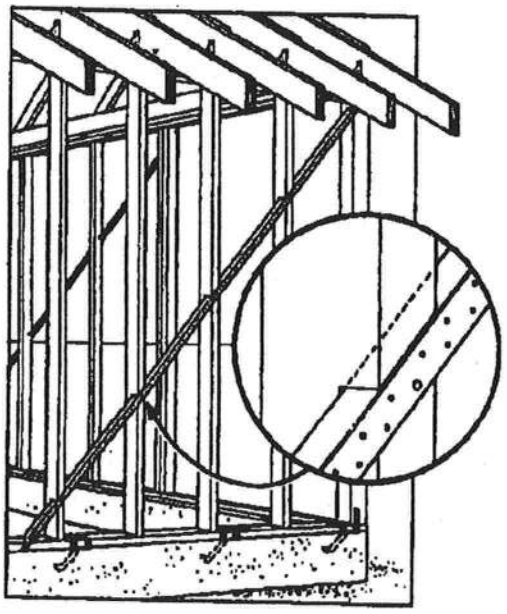
TABLE 1 - CWB[1,2]

MODEL NO.	DIMENSIONS (feet - x 304.8 for mm) (inches - x 25.4 for mm)		ANGLE FOR 8' WALL	FASTENERS[3]	
	LENGTH	SECTION		PLATES	STUDS
CWB106	9' - 5¾"	15/16 x 15/16	60°	2 - 16d	1 - 8d
CWB126	11' - 4⅜"	15/16 x 15/16	45°	2 - 16d	1 - 8d

1. The CWB is designed to provide racking resistance equivalent to a 1x4 let in during construction. It is not designed to replace the shearwall load carrying components.
2. The CWB is limited to a maximum load of 200 pounds (x 4.45 for N).
3. The 16d common nails are 0.162" x 3½" long. The 8d common nails are 0.131" x 2½" long (x 25.4 for mm).



CWB Cross-Section Detail

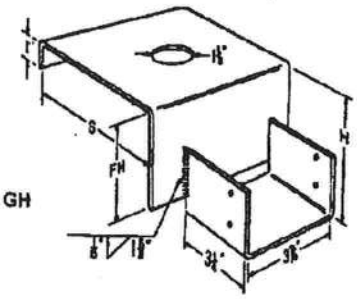


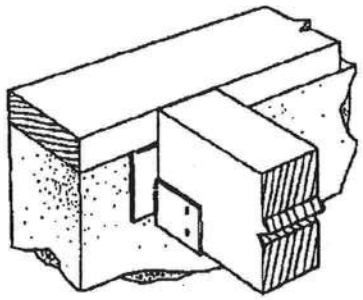
Typical CWB Installation

TABLE 2 - GH[1,2]

MODEL NO.	GIRDER	H (INCH)	S (INCH)	ALLOWABLE GRAVITY LOADS (LBS)[3,4] (x 4.45 for N)
		x 25.4 for mm		FLOOR/ROOF
GH46-6	4 x 6	4	6	2000
GH46-8	4 x 6	4	8	2000
GH48-6	4 x 8	6	6	2000
GH48-8	4 x 8	6	8	2000

1. Install 4 - 16d common nails into the girder.
2. Nails are 16d common 0.162" x 3½" long (x 25.4 for mm).
3. Loads are in pounds.
4. Allowable loads in the table are limited by test results. No load duration increase is allowed.



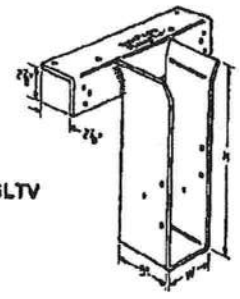


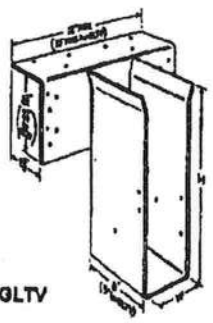
Typical GH Installation

TABLE 3 - GLTV & HGLTV

MODEL NO.	DIMENSIONS (INCH) (x 25.4 for mm)				FASTENERS[1]			ALLOWABLE GRAVITY LOADS[2,3,4]	
	W	MIN H	B	L	CARRING MEMBER TOP	CARRING MEMBER FACE	CARRIED MEMBER	FLOOR (100)	ROOF (125)
GLTV3.5	3⅜	9¼	5	10	4 - 16d	6 - 16d	6 - 16d	7000	7000
GLTV5.5	5½	9¼	5	10	4 - 16d	6 - 16d	6 - 16d	7000	7000
HGLTV3.5	3⅜	9¼	6	12	6 - 16d	12 - 16d	6 - 16d	10500	10500
HGLTV5.5	5½	9¼	6	12	6 - 16d	12 - 16d	6 - 16d	10500	10500

1. Nails are 16d common 0.162 x 3½" long (x 25.4 for mm).
2. Loads are in pounds (x 4.45 for N).
3. Allowable loads in the table are limited by test results. No load duration increase is allowed.
4. The connectors provide torsional resistance up to a maximum joist depth of 32 inches (x 25.4 for mm).
5. Wood headers supporting the hangers shall have a minimum width of 3 inches and a minimum allowable compression perpendicular to the grain value of 500 psi (x 6.895 for kPa).



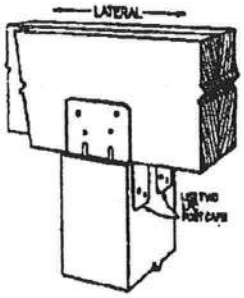


HGLTV

TABLE 4 - LPC4

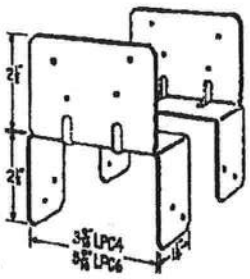
MODEL NO.	FASTENERS[1]		ALLOWABLE LOADS[2,3] (lbs) (x 4.45 for N)	
	BEAM	POST	UPLIFT	LATERAL[4]
LPC4	8 -10d	8 -10d	760	325

1. Nails are 10d common 0.148 x 3" long (x 25.4 for mm).
2. Loads are in pounds (x 4.45 for N).
3. Allowable loads in the table are limited by test results. No load duration increase is allowed.
4. Allowable loads are for hangers used in pairs.



Typical LPC4 Installation

Allows For Beam Widths 2½" to 3½"

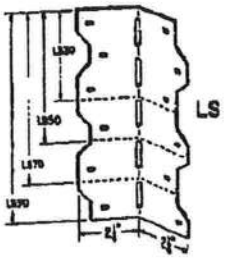


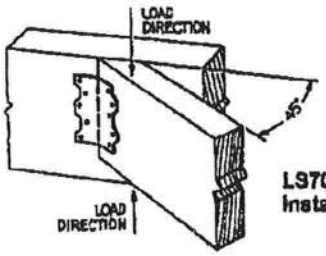
LPC4

TABLE 5 - LS

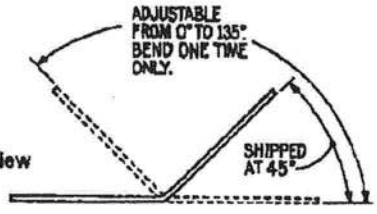
MODEL NO.	LENGTH	FASTENERS[1]	ALLOWABLE LOADS[2,3] (lbs.) (x 4.45 for N)	
			NORMAL	MAXIMUM
LS30	3⅜	6 - 10d	335	420
LS50	4⅞	8 - 10d	450	560
LS70	6⅜	10 - 10d	560	670
LS90	7⅞	12 - 10d	670	840

1. Nails are 10d common 0.148 x 3" long (x 25.4 for mm).
2. Loads are in pounds (x 4.45 for N).
3. Allowable loads are for vertical loads only, no lateral loads allowed.





LS70 Installed



LS Top View

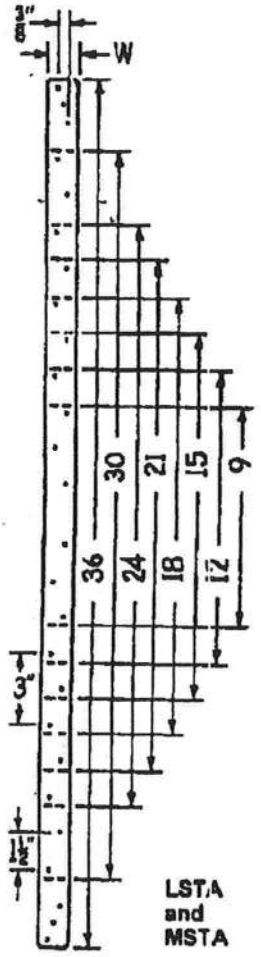


TABLE 6 - LSTA/MSTA

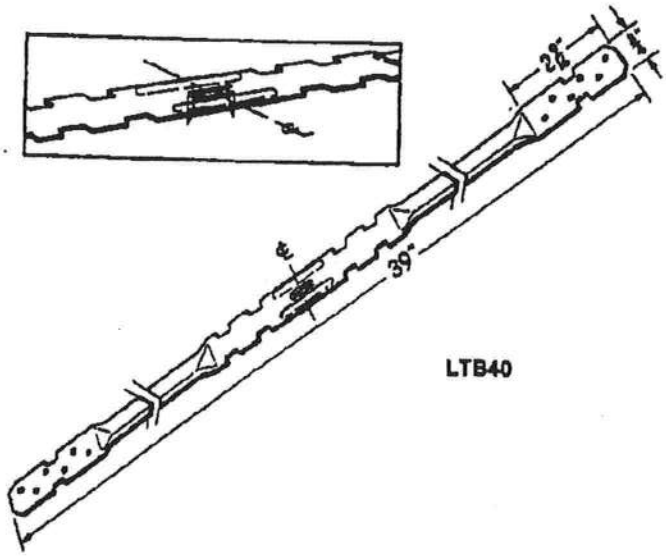
MODEL NO.	MATERIAL	DIMENSIONS(inch) (x 25.4 for mm)		FASTENERS (TOTAL)	MAXIMUM ALLOWABLE LOADS[2,3] (x 4.45 for N)	
		W	L		10d	16d
LSTA9	20 GA	1¼	9	8	605	720
LSTA12	20 GA	1¼	12	10	755	900
LSTA15	20 GA	1¼	15	12	905	1080
LSTA18	20 GA	1¼	18	14	1055	1260
LSTA21	20 GA	1¼	21	16	1205	1295
LSTA24	20 GA	1¼	24	18	1295	1295
LSTA30	18 GA	1¼	30	22	1670	1715
LSTA36	18 GA	1¼	36	26	1715	1715
MSTA9	18 GA	1¼	9	8	610	725
MSTA12	18 GA	1¼	12	10	760	905
MSTA15	18 GA	1¼	15	12	910	1090
MSTA18	18 GA	1¼	18	14	1065	1270
MSTA21	18 GA	1¼	21	16	1215	1450
MSTA24	18 GA	1¼	24	18	1370	1630
MSTA30	16 GA	1¼	30	22	1685	2010
MSTA36	16 GA	1¼	36	26	1995	2135

1. Nails are 16d common 0.162 x 3½" long or 10d common 0.148 x 3" long (x 25.4 for mm).
2. Loads are in pounds.
3. Maximum allowable loads have been increased 33% for wind or earthquake loading, no further increase allowed. Reduce the allowable loads by 33% for normal loading criteria.

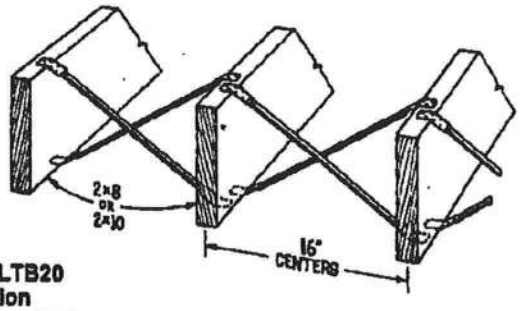
TABLE 7 - LTB

MODEL NO.	JOIST	SPACING(inch) (x 25.4 for mm)	LENGTH(inch) (x 25.4 for mm)
LTB20[1,2]	2x8, 2x10	16"	19.5"
LTB21[1,2]	2x8, 2x10, 2x12	16"	21"
LTB40[2,3]	2x8, 2x10	16"	39"
LTB42[2,3]	2x8, 2x10, 2x12	16"	42"

1. Install 2 - 6d nails each end for the LTB20 and LTB21.
2. Nails are 6d common 0.113 x 2" long (x 25.4 for mm).
3. Install prongs and 2 - 6d nails in the other end for the LTB40 and LTB42.



LTB40



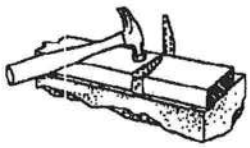
Typical LTB20 Installation (LTB40 Similar)

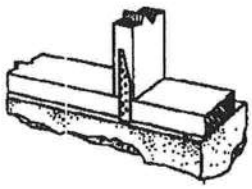
Space Bridging to Avoid Contact Noises

TABLE 8 - MA[1]

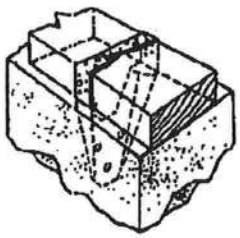
MODEL NO.	SILL SIZE	W	FASTENERS[2,3]		ALLOWABLE LOADS [4,5] (lbs.) (x 4.45 for N)		
			SIDES TOTAL	TOP	UPLIFT	PARALLEL TO SILL PLATE	PERPENDICULAR TO SILL PLATE
MA4	2 x 4	3⅝	2 - 10d x 1½	2 - 10d x 1½	830	480	1180
	3 x 4		4 - 10d x 1½	2 - 10d x 1½	1060	680	1180
MA6	2 x 6	5⅝	2 - 10d x 1½	4 - 10d x 1½	1060	680	1180
	3 x 6		4 - 10d x 1½	4 - 10d x 1½	1290	680	1180

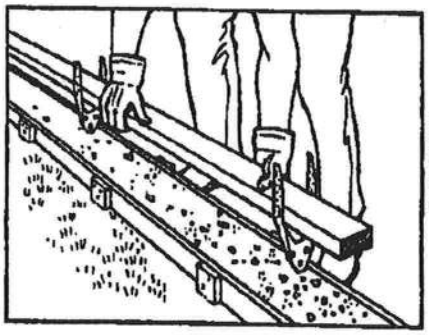
1. Minimum concrete strength shall be 2000 psi(x 6.895 for kPa) at 28 days and shall have a minimum thickness of 6 inches (x 25.4 for mm).
2. Nails are evenly divided between each side at the spacing and edge distance required by code.
3. Nails are 0.148 x 1½" long (x 25.4 for mm).
4. Loads shall not be increased for short-term load duration.
5. Loads are in pounds (x 4.45 for N).



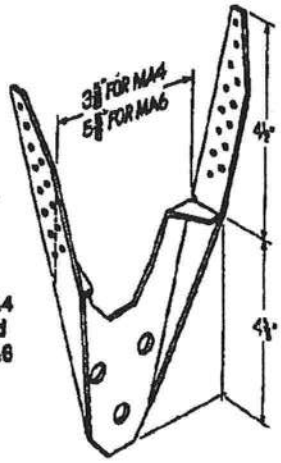


Typical MA4 and MA6 installed





Optional method with mudsill anchors in place for positioning into screeded concrete.

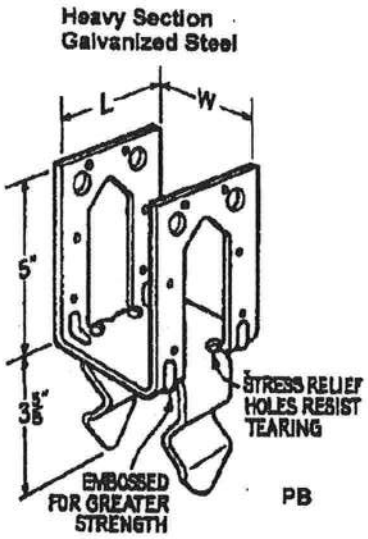


MA4 and MA6

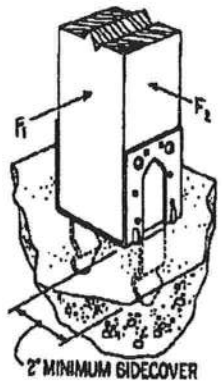
TABLE 9 - PB[1,2]

MODEL NO.	DIMENSIONS(Inch) (x 25.4 for mm)		ALLOWABLE LOADS[3,4] (lbs.) (x 4.45 for N)			
	W	L	12 - 16d NAILS[5]			2 -½ MB[6]
			UPLIFT	F_1	F_2	UPLIFT
PB44	3 9/16	3¼	1365	765	1325	—
PB44R	4	3¼	1365	765	1325	—
PB46	5½	3¼	1365	765	1325	—
PB66	5½	5⅜	1640	765	1325	1640
PB46R	6	3⅜	1365	765	1325	1640
PB66R	6	5⅜	1640	765	1325	1640

1. Minimum concrete strength is 2000 psi (x 6.895 for kPa).
2. Minimum side cover is 2" (x 25.4 for mm).
3. Loads are in pounds (x 4.45 for N).
4. Allowable loads in the table are limited by test results. No load duration increase is allowed.
5. Nails are 16d common 0.162 x 3½" long (x25.4 for mm).
6. The ½ - inch diameter (x 25.4 for mm) machine bolts shall be equal to ro better than ASTM A307 quality. The length of the bolt shall be sufficient to allow proper installation of a nut and washer on the threaded end.



PB

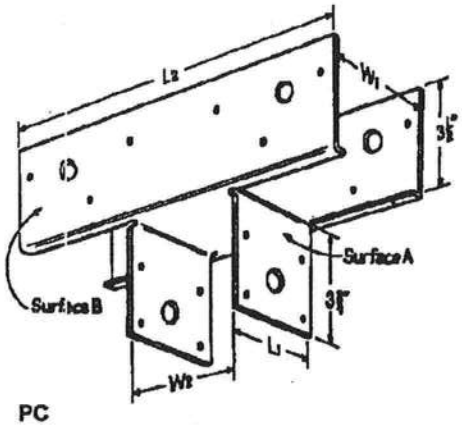


Typical PB Installed

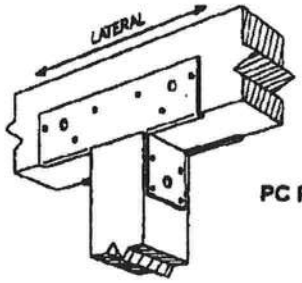
TABLE 10 - PC/EPC

MODEL NO.	POST SIZE	DIMENSIONS (Inch) (x 25.4 for mm)					FASTENERS[1]			ALLOWABLE LOADS[2,3] (lbs.) (x 4.45 for N)			
		W1	W2	L1	L2	L3	SURFACE A	SURFACE B	SURFACE C	UPLIFT		LATERAL[4]	
										NORM	MAX	PC[5]	EPC[6]
PC44-16	4 x 4	3 3/16	3 9/16	2 5/8	11	7 3/8	4 - 16d	6 - 16d	4 - 16d	1000	1000	925	1000
PC44	4 x 4	3 3/16	3 9/16	2 5/8	11	7 3/8	4 - 16d	6 - 16d	4 - 16d	1105	1470	925	1070
PC46-16	4 x 6	3 3/16	5 1/2	2 5/8	13	9 1/4	4 - 16d	6 - 16d	4 - 16d	1000	1000	925	1000
PC46	4 x 6	3 3/16	5 1/2	2 5/8	13	9 1/4	4 - 16d	6 - 16d	4 - 16d	1105	1470	925	1070
PC48-16	4 x 8	3 3/16	7 1/2	2 5/8	15	11 1/4	4 - 16d	8 - 16d	6 - 16d	1000	1000	1475	1285
PC48	4 x 8	3 3/16	7 1/2	2 5/8	15	11 1/4	4 - 16d	8 - 16d	6 - 16d	1105	1470	2075	1610
PC64-16	4 x 6	5 1/2	3 9/16	4 9/16	11	7 3/8	4 - 16d	6 - 16d	4 - 16d	1000	1000	925	1000
PC64	4 x 6	5 1/2	3 9/16	4 9/16	11	7 3/8	4 - 16d	6 - 16d	4 - 16d	1105	1470	925	1070
PC66-16	6 x 6	5 1/2	5 1/2	4 9/16	13	9 1/4	4 - 16d	6 - 16d	6 - 16d	1000	1000	925	1285
PC66	6 x 6	5 1/2	5 1/2	4 9/16	13	9 1/4	4 - 16d	6 - 16d	6 - 16d	1105	1470	925	1610
PC68	6 x 8	5 1/2	7 1/2	4 9/16	15	11 1/4	4 - 16d	8 - 16d	6 - 16d	1105	1470	2075	1610
PC84	4 x 8	7 1/2	3 9/16	6 9/16	11	7 3/8	4 - 16d	6 - 16d	6 - 16d	1105	1470	925	1610
PC86	6 x 8	7 1/2	5 1/2	6 9/16	13	9 1/4	4 - 16d	6 - 16d	6 - 16d	1105	1470	925	1610
PC88	8 x 8	7 1/2	7 1/2	6 9/16	15	11 1/4	4 - 16d	8 - 16d	6 - 16d	1105	1470	2075	1610

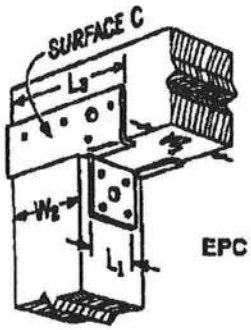
1. Nails are 16d common 0.162 x 3½" long (x 25.4 for mm).
2. Loads are in pounds.
3. Loads shall not be increased for short term load duration.
4. Allowable lateral loads are for loads applied parallel to the beam.
5. PC = post cap.
6. EPC = end post cap.



PC



PC Post Cap

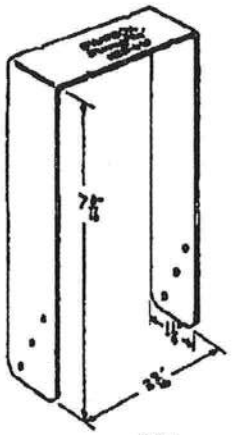


EPC End Post Cap

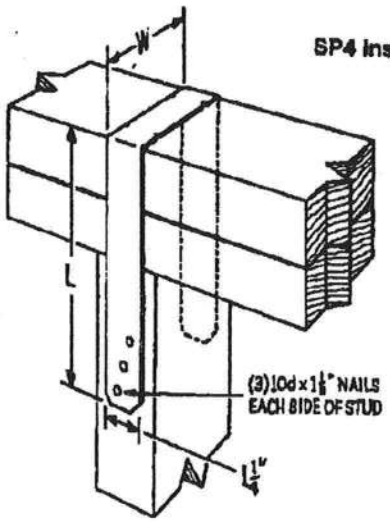
TABLE 11 - SP/SPA

MODEL NO.	MATERIAL	DIMENSIONS (Inch) (x 25.4 for mm)		NUMBER OF FASTENERS[1]	NORMAL ALLOWABLE LOADS[2,3] (lbs.) (x 4.45 for N)	
		W	L		10d x 1½	16d x 2½
SP4/SP4A	20 GA	3 9/16	7 3/16	6	550	600
SP6/SP6A	20 GA	5 9/16	7 3/4	6	550	600
SP8/SP8A	20 GA	7 5/16	8 5/16	8	550	600

1. Nails are 10d common, 0.148 x 1½" long or 16d common 0.162 x 2½" long (x 25.4 for mm).
2. Loads are in pounds.
3. Loads are permitted to be increased for short-term load duration.



SP4
(SP6 & SP7 Similar)

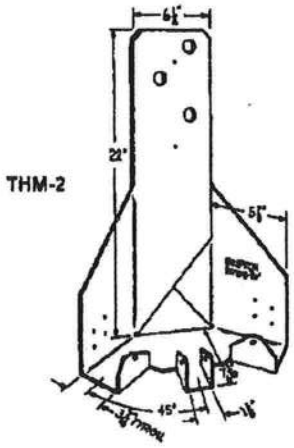


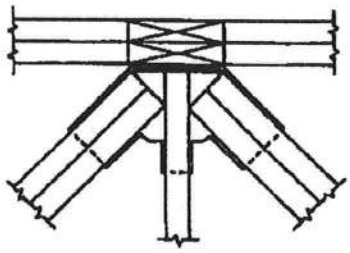
SP4 Installed

TABLE 12 - THM-2

MODEL NO.	FASTENERS[2]				ALLOWABLE LOADS[3,4,5] (x 4.45 for N)							
	CARRYING MEMBER		CARRIED MEMBER		UPLIFT				LENGTH OF BOLT IN WOOD MEMBER(inch) (x 25.4 for mm)	ROOF (100)		ROOF (125)	
					HIP		JACK						
	BOLTS[6]	NAILS	HIP	JACK	NORM	MAX	NORM	MAX		HIP	JACK	HIP	JACK
THM-2	3 - 1	2 - 16d	5 - 10d x 1½	2 - 10d x 1½	460	520	195	260	1½	1105	555	1385	690
									3	2215	1110	2770	1385
									4½	3250	1625	3640	1820
									6	3250	1625	3640	1820

1. Use of the hanger to laterally support trusses where required by design is beyond the scope of this report.
2. Nails are 10d common 0.148 x 1½" long and 16d common 0.162 x 3½" long (x 25.4 for mm).
3. Loads are in pounds.
4. Loads shall not be increased for short-term load duration.
5. The total load shall be evenly distributed about the center line to avoid eccentric loading.
6. The 1-inch diameter machine bolts must be equal to or better than ASTM A 307 quality. Length of the bolt shall be sufficient to allow proper installation of a nut and washer on the threaded end.

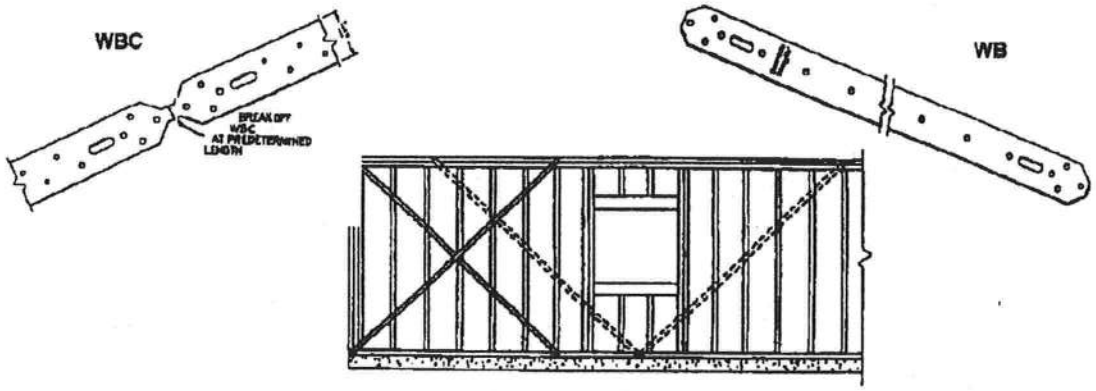




THM-2 Plan View

TABLE 13 - FASTENER SCHEDULE FOR WB AND WBC WALL BRACING

MODEL NO.	DIMENSIONS (feet/inches) (x 304.8 for mm)/(x 25.4 for mm)		ANGLE & WALL SIZE	FASTENERS	
	LENGTH	SECTION		PLATES	STUDS
WB106	9' - 5⅝"	1¼	8' at 60°	2 - 16d	1 - 8d
WB126	11' - 4⅜"	1¼	8' at 45°	2 - 16d	1 - 8d
WB106C	9' - 6"	1¼	8' at 60°	2 - 16d	1 - 8d
WB126C	11' - 4¾"	1¼	8' at 45°	2 - 16d	1 - 8d
WB143C	14' - 3"	1¼	10' at 45°	2 - 16d	1 - 8d

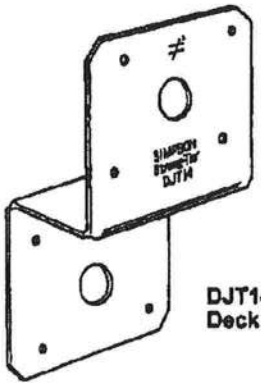


WB or WBC Wall Bracing "X" and "V" Applications

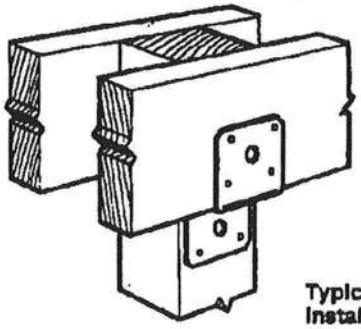
TABLE 14 - FASTENER SCHEDULE FOR THE DJT14

MODEL NO.	FASTENERS		ALLOWABLE GRAVITY LOADS[1,2] (lbs.) (x 4.45 for N)			
			NAILS		BOLTS	
	NAILS	BOLTS[3]	FLOOR (100)	ROOF (125)	FLOOR (100)	ROOF (125)
DJT14	8 - 16d	2 - ⅝ MB	1100	1375	1400	1400

1. Loads and fasteners noted in the table are for one DJT.
2. Roof loads are 125% of floor loads. When floor loads are adjusted for other load duration, in accordance with the applicable code, they shall not exceed those in the roof column.
3. The ⅝ - inch - diameter machine bolts shall be equal to or greater than ASTM A 307 quality, length shall be sufficient to allow proper installation of a nut and washer on the threaded end.



DJT14
Deck Joist Tie



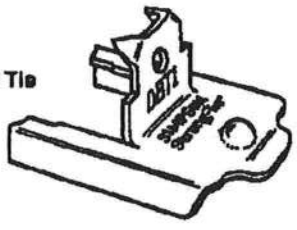
Typical DJT14
Installation

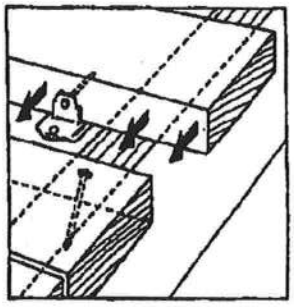
TABLE 15 - FASTENER SCHEDULE FOR THE DBT1

MODEL NO.	FASTENERS[1,2]
	DECK BOARD
DBT1	2 PRONGS & 1 - 10d X 1½"

1. 2-Pronge and 1-10d x 1½ nail are used to attach DBT1 to edge of deck board. Other edge of deck board shall be toe nailed using 1-16d common.
2. Minimum deck board thickness shall be 1¼" (⅝).

DBT1
Deck Board Tie



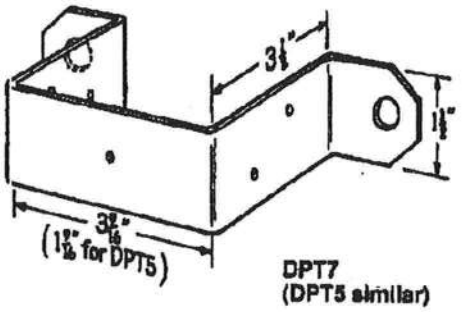


Typical DBT1 Installation.
Connectors slide under anchored deck board.

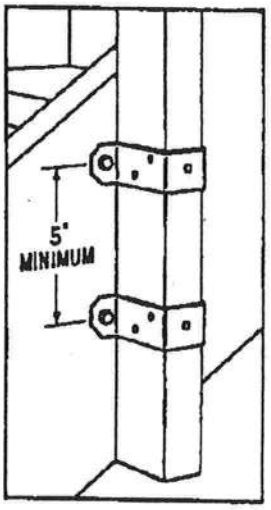
TABLE 16 - FASTENER SCHEDULE FOR THE DPT5, DPT6 AND DPT7[1,2]

MODEL NO.	FASTENERS		MINIMUM POST SPACING[3] (feet/inches) (x 304.8 for mm/ x 25.4 for mm)	
	FASCIA/RIM JOIST	POST	DOUGLAS FIR-LARCH	REDWOOD (Close Grain)
DPT5[4]	2 - ⅜ MB[5]	5 - 10d x 1½	2 - 1	1 - 2
DPT6	8 - 16d	8 - 16d	1 - 7	1 - 3
DPT7[4]	2 - ⅜ MB[5]	5 - 10d	2 - 5	1 - 9

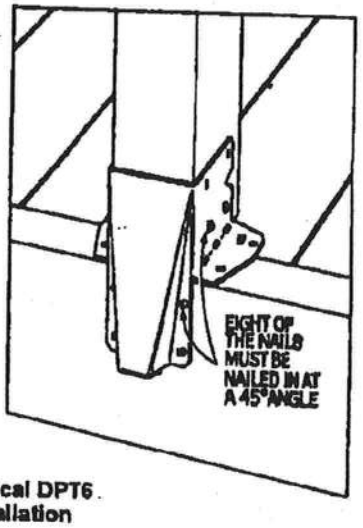
1. Limited to Group R residential dwellings and a maximum rail height of 36 inches.
2. For DPT5 and DPT7, minimum rim joist size is 2x8, nominal and minimum fascia size is 2x10, nominal. For DPT6, minimum rim joist size is 2x8, nominal, and minimum fascia size is 2x8, nominal.
3. Post spacing is based on an assumed handrail loading of 50 plf or a 300 pound point load.
4. Fastener quantities are for a single DPT5 or DPT7. Proper installation is in pairs 5 inches apart, center to center. A standard washer is required with each nut.
5. Bolts shall penetrate through a minimum nominal 2x rim joist (1½-inch minimum)(x 25.4 for mm) and a minimum nominal 2x fascia (1½-inch minimum)(x 25.4 for mm). Machine bolts shall be ASTM A 307 quality or better and have sufficient length to allow proper installation of a nut and washer on the threaded end.



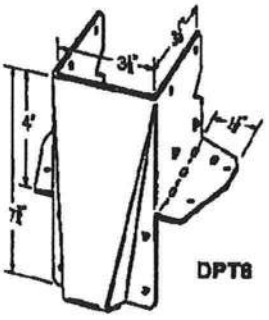
DPT7
(DPT5 similar)



Typical DPT5
Stairway Installation



Typical DPT6
Installation

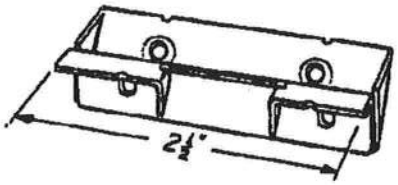


DPT8

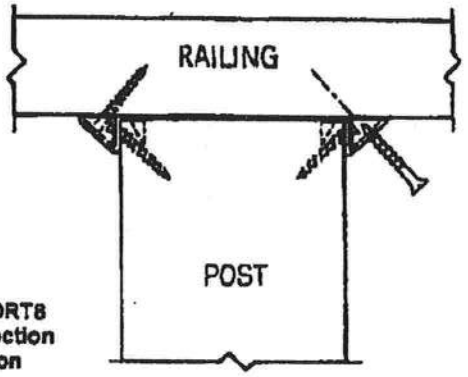
TABLE 17 - FASTENER SCHEDULE FOR THE DRT8

MODEL NO.	FASTENERS[1]		ALLOWABLE HORIZONTAL LOADS (lbs.) (x 4.45 for N)		MAXIMUM SPACING (feet/inches)[4] (x 304.8 for mm/x 25.4 for mm)	
	RAIL[2]	POST[3]	REDWOOD (Close Grain)	DOUG FIR-LARCH	REDWOOD (Close Grain)	DOUG FIR-LARCH
DRT8	2	2	80	113	4' 0"	5' 7"

1. Fastener quantities are for a single DRT8. Installation is in pairs with one DRT8 on either side of the post. Fasteners are No. 8 x 1¼ -inch wood screws.
2. Minimum deck rail thickness is 1½ inches (x 25.4 for mm).
3. Minimum post size is 2 x 4, nominal.
4. Post spacing is based on an assumed handrail loading of 50 plf (x .0146 for N/mm) or a 300 pound (x 4.45 for N) point load.



DRT8



Typical DRT8 Cross Section Installation

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

CONTRACTOR LICENSING SECTION
(305) 375-2527 FAX (305) 375-2558

CONTRACTOR ENFORCEMENT SECTION
(305) 375-2966 FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 372-6339

PRODUCT CONTROL NOTICE OF ACCEPTANCE

Simpson Strong Tie Company, Inc.
4637 Chabot Drive
Pledsanton CA 94588

Your application for Product Approval of:
Wood Connectors
under Chapter 8 of the Code of Miami-Dade County governing the use of Alternate Materials and Types of Construction, and completely described herein, has been recommended for acceptance by the Miami-Dade County Building Code Compliance Office (BCCO) under the conditions specified herein.

This approval shall not be valid after the expiration date stated below. BCCO reserves the right to secure this product or material at anytime from a jobsite or manufacturer's plant for quality control testing.
If this product or material fails to perform in the approved manner, BCCO may revoke, modify, or suspend the use of such product or material immediately. BCCO reserves the right to revoke this approval, if it is determined BCCO that this product or material fails to meet the requirements of the South Florida Building Code.

The expense of such testing will be incurred by the manufacturer.

Acceptance No.: 99-0713.05
Expires: 10/13/2002

Raul Rodriguez
Chief Product Control Division

THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL CONDITIONS

BUILDING CODE & PRODUCT REVIEW COMMITTEE

This application for Product Approval has been reviewed by the BCCO and approved by the Building Code and Product Review Committee to be used in Dade County, Florida under the conditions set forth above.

Francisco J. Quintana, R.A.
Director
Miami-Dade County
Building Code Compliance Office

Approved: 10/14/1999

Simpson Strong-Tie Co., Inc.

ACCEPTANCE NO: 99-0713.05

APPROVED : OCT 1 3 1999

EXPIRES : OCT 1 3 2002

NOTICE OF ACCEPTANCE: SPECIFIC CONDITIONS

1. SCOPE

1.1 This approves wood connectors; as described in Section 2 of this Notice of Acceptance, designed to comply with the South Florida Building Code (SFBC), 1994 Edition for Miami-Dade County. For the locations where the actual loads as determined by SFBC Chapter 23, do not exceed the design load indicated in the approved drawings.

2. PRODUCT DESCRIPTION

2.1 The Simpson Strong-Tie Wood Connectors shall be fabricated and used in strict compliance with the following documents: Drawing No. no number, titled "ABA Standoff Post Bases", "ABA Standoff Post Bases", "AC/ACE Post Caps", "BC Post Caps", "L/LS Reinforcing and Skeable Angles", "MTS Twist Straps", & "TA9 Staircase Angles", prepared by Simpson Strong-Tie Co., Inc., dated 03/04/99 sheet 1 through 7 of 7. The drawings shall bear the Miami-Dade County Product Control Approval stamp with the Notice of Acceptance number and approval date by the Miami-Dade Product Control Division. These documents shall hereinafter be referred to as the approved drawings.

3. LIMITATIONS

3.1 Allowable loads are for Douglas Fir-Larch or better with a specific gravity of 0.50 and moisture content of 19% or less.

3.2 Allowable loads are based on testing per ASTM D1761 and calculations per National Design Specifications for Wood Construction 1991 Edition & 1993 Errata.

4. INSTALLATION

4.1 The wood connectors shall be installed in strict compliance with the approved drawings.

5. LABELING

5.1 Each wood connector shall bear a permanent label with the manufacturer's name or logo, city, state and the following statement: "Miami-Dade County Product Control Approved".

6. BUILDING PERMIT

6.1 Application for Building Permit shall be accompanied by copies of the following:

6.1.1 This Notice of Acceptance

6.1.2 Duplicate copies of the approved drawings as identified in Section 2 of this Notice of Acceptance, clearly marked to show the hangers and angles selected for the proposed installation.

6.1.3 Any other document required by the Building Official or the SFBC in order to properly evaluate the installation of these products.

Candido Font, PE, Sr. Product Control Examiner
Product Control Division

Simpson Strong-Tie Co., Inc.

ACCEPTANCE NO.: 99-0713.05

APPROVED :OCT 13 1999

EXPIRES :OCT 13 2002

NOTICE OF ACCEPTANCE STANDARD CONDITIONS

1. Renewal of this Acceptance (approval) shall be considered after a renewal application has been filed and the original submitted documentation, including test supporting data, engineering documents, are no older than eight (8) years.

2. Any and all approved products shall be permanently labeled with the manufacturer's name, city, state, and the following statement: "Miami-Dade County Product Control Approved", or as specifically stated in the specific conditions of this Acceptance.

3. Renewals of Acceptance will not be considered if:
a) There has been a change in the South Florida Building Code affecting the evaluation of this product and the product is not in compliance with the code changes;
b) The product is no longer the same product (identical) as the one originally approved;
c) If the Acceptance holder has not complied with all the requirements of this acceptance, including the correct installation of the product;
d) The engineer who originally prepared, signed and sealed the required documentation initially submitted is no longer practicing the engineering profession.

4. Any revision or change in the materials, use, and/or manufacture of the product or process shall automatically be cause for termination of this Acceptance, unless prior written approval has been requested (through the filing of a revision application with appropriate fee) and granted by this office.

5. Any of the following shall also be grounds for removal of this Acceptance:
a) Unsatisfactory performance of this product or process.
b) Misuse of this Acceptance as an endorsement of any product, for sales, advertising or any other purpose.

6. The Notice of Acceptance number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the Notice of Acceptance is displayed, then it shall be done in its entirety.
7. A copy of this Acceptance as well as approved drawings and other documents, where it applies, shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at all time. The engineer need not reseal the copies.

8. Failure to comply with any section of this Acceptance shall be cause for termination and removal of Acceptance.

9. This Notice of Acceptance consists of pages 1, 2 and this last page 3.

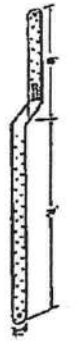
Candido Font, PE, Sr. Product Control Examiner
Product Control Division

END OF THIS ACCEPTANCE

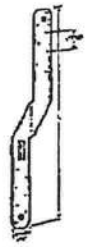
LTS/MTS TWIST STRAPS

Model No.	L	Fasteners[1]		Allowable Loads			
		10d	10dx1½"	Uplift 10d	Uplift 10dx1½"	Parallel to Plate (F_1)	Perpendicular to Plate (F_2)
MTS12	12	14-10d	14-10dx1½"	815		75	125
MTS16	16	14-10d	14-10dx1½"	815		75	125
MTS18	18	14-10d	14-10dx1½"	815		75	125
MTS20	20	14-10d	14-10dx1½"	815		75	125
MTS30	30	14-10d	14-10dx1½"	815		–	–

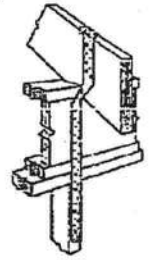
[1]Install half of the fasteners in each end of the strap to achieve full loads. When wrapped over a truss, install one nail on the side of the truss in the hole closet to the middle of the strap, three nails on top of the truss and three nails on the back of the truss. When attached to a double top plate, install one nail in the front of the upper plate closes to the middle of the strap, two nails in the front of the lower plate and three nails in the bottom of the lower plate.



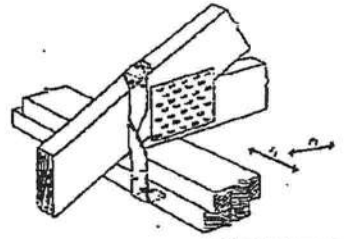
MTS30



MTS12
(Other Sizes Similar)



Typical MTS30 Installation



MTS Installation as a Truss-to-Top Plate Tie

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE 10/13 1999
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
Compliance No. 99-1013.05

GENERAL NOTES:

1) Steel shall conform to 16 gauge ASTM A-653 FS with $F_{y,min}$ = 28 ksi and $F_{u,min}$ = 38 ksi and have a minimum galvanized coating of G60.
2) Fasteners diameter is 0.148".
3) Allowable loads have increased 33% for wind loading with no other duration increases allowed.
4) Allowable loads are based on the National Design Specification for Wood Construction 1991 Edition & 1993 Errata for Douglas Fir-Larch (G=0.50 or better) and tests performed in accordance with ASTM D-761.

FOR OFFICE USE

SIMPSON STRONG-TIE CO.,INC.
4637 Chabot Drive, Suite 200
Pleasanton, CA 94588

TITLE: MTS Twist Straps

Drawing No.: Sheet No.: 6/7

Drawing Date: 3-4-99 Revision Date: --

Evon M.C. Ballosh, P.E. Civil #PE0051762

MIAMI-DADE

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

CONTRACTOR LICENSING SECTION
(305) 375-2527 FAX (305) 375-2555

CONTRACTOR ENFORCEMENT DIVISION
(305) 375-2966 FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 372-6339

PRODUCT CONTROL NOTICE OF ACCEPTANCE

Simpson Strong-Tie Company, Inc.
4637 Chabot Drive Suite 200
Pleasanton ,CA 94588

Your application for Notice of Acceptance (NOA) of:
Varous Connectors: H, HH, FC, A
under Chapter 8 of the Code of Miami-Dade County governing the use of Alternate Materials and Types of Construction, and completely described herein, has been recommended for acceptance by the Miami-Dade County Building Code Compliance Office (BCCO) under the conditions specified herein.

This NOA shall not be valid after the expiration date stated below. BCCO reserves the right to secure this product or material at any time from a jobsite or manufacturer's plant for quality control testing. If this product or material fails to perform in the approved manner, BCCO may revoke, modify, or suspend the use of such product or material immediately. BCCO reserves the right to revoke this approval, if it is determined by BCCO that this product or material fails to meet the requirements of the South Florida Building Code.

The expense of such testing will be incurred by the manufacturer.

ACCEPTANCE NO.: 00-0926.01
EXPIRES: 01/11/2004

Raul Rodriguez
Chief Product Control Division

THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL CONDITIONS
BUILDING CODE & PRODUCT REVIEW COMMITTEE

This application for Product Approval has been reviewed by the BCCO and approved by the Building Code and Product Review Committee to be used in Miami-Dade County, Florida under the conditions set forth above.

Francisco J. Quintana, R.A.
Director
Miami-Dade County
Building Code Compliance Office

APPROVED: 01/11/2001

Simpson Strong-Tie Co., Inc.

ACCEPTANCE NO: 00-0926.01

APPROVED: JAN 1 1 2001

EXPIRES: JAN 1 1 2004

NOTICE OF ACCEPTANCE: SPECIFIC CONDITIONS

1. **SCOPE**

1.1 This renews the Notice of Acceptance No. 97-0107.05, which was issued on 08/14/97. It approves wood connectors; as described in Section 2 of this Notice of Acceptance, designed to comply with the South Florida Building Code (SFBC), 1994 Edition for Miami-Dade County. For the locations where the actual loads as determined by SFBC Chapter 23, do not exceed the design load indicated in the approved drawings.

2. **PRODUCT DESCRIPTION**

2.1 The **Simpson Strong-Tie Wood Connectors** shall be fabricated and used in strict compliance with the following documents: Drawing with No. SSTMD-001 and sheets 1 through 6 of 6, titled "Hurricane Ties, HH Header Hangers, FC Framing Clips, A34 Framing Anchors, A35 Framing Anchors and A35F Framing Anchors", prepared by Simpson Stong-Tie Co., Inc., dated 09/14/00 with no revisions. The drawings shall bear the Miami-Dade County Product Control Approval stamp with the Notice of Acceptance number and approval date by the Miami-Dade Product Control Division. These documents shall hereinafter be referred to as the approved drawings.

3. **LIMITATIONS**

3.1 Allowable loads are for Douglas Fir-Larch or better with a specific gravity of 0.50 and moisture content of 19% or less.

3.2 Allowable loads are based on testing per ASTM D1761 and calculations per National Design Specifications for Wood Construction 1991 Edition & 1993 Errata.

4. **INSTALLATION**

4.1 The wood connectors shall be installed in strict compliance with the approved drawings.

5. **LABELING**

5.1 Each wood connector shall bear a permanent label with the manufacturer's name or logo, city, state and the following statement: "Miami-Dade County Product Control Approved".

6. **BUILDING PERMIT**

6.1 Application for Building Permit shall be accompanied by copies of the following:

6.1.1 This Notice of Acceptance

6.1.2 Duplicate copies of the approved drawings as identified in Section 2 of this Notice of Acceptance, clearly marked to show the hangers and angles selected for the proposed installation.

6.1.3 Any other document required by the Building Official or the SFBC in order to properly evaluate the installation of these products.

Candido Font, PE, Sr. Product Control Examiner
Product Control Division

Simpson Strong-Tie Co., Inc.

ACCEPTANCE NO.: 00-0926.01

APPROVED: JAN 1 1 2001

EXPIRES: JAN 1 1 2004

NOTICE OF ACCEPTANCE STANDARD CONDITIONS

1. Renewal of this Acceptance (approval) shall be considered after a renewal application has been filed and the original submitted documentation, including test supporting data, engineering documents, are no older than eight (8) years.

2. Any and all approved products shall be permanently labeled with the manufacturer's name, city, state, and the following statement: "Miami-Dade County Product Control Approved", or as specifically stated in the specific conditions of this Acceptance.

3. Renewals of Acceptance will not be considered if:
a) There has been a change in the South Florida Building Code affecting the evaluation of this product and the product is not in compliance with the code changes;
b) The product is no longer the same product (identical) as the one originally approved;
c) If the Acceptance holder has not complied with all the requirements of this acceptance, including the correct installation of the product;
d) The engineer who originally prepared, signed and sealed the required documentation initially submitted is no longer practicing the engineering profession.

4. Any revision or change in the materials, use, and/or manufacture of the product or process shall automatically be cause for termination of this Acceptance, unless prior written approval has been requested (through the filing of a revision application with appropriate fee) and granted by this office.

5. Any of the following shall also be grounds for removal of this Acceptance:
a) Unsatisfactory performance of this product or process.
b) Misuse of this Acceptance as an endorsement of any product, for sales, advertising or any other purpose.

6. The Notice of Acceptance number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the Notice of Acceptance is displayed, then it shall be done in its entirety.
7. A copy of this Acceptance as well as approved drawings and other documents, where it applies, shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at all time. The engineer need not reseal the copies.

8. Failure to comply with any section of this Acceptance shall be cause for termination and removal of Acceptance.

9. This Notice of Acceptance consists of pages 1, 2 and this last page 3.

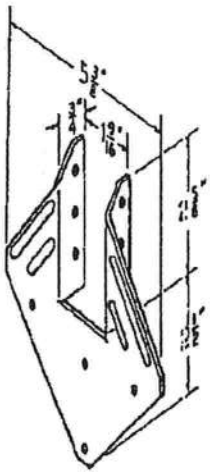
Candido Font, PE, Sr. Product Control Examiner
Product Control Division

END OF THIS ACCEPTANCE

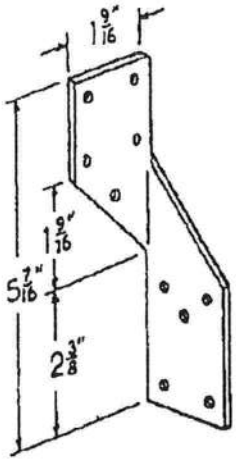
HURRICANE TIES

Model No.	GA.	Fasteners		Allowable Loads		
		Rafter	Plate or Stud	Uplift	F1	F2
H1	18	8 - 8d x 1½	4 - 8d	490	445	165
H2.5	18	5 - 8d x 1½	5 - 8d	410	150	150
H3	18	4 - 8d x 1½	4 - 8d	435	110	125
H4	20	4 - 8d x 1½	4 - 8d	—	150	150
H5	18	4 - 8d x 1½	4 - 8d	—	115	180

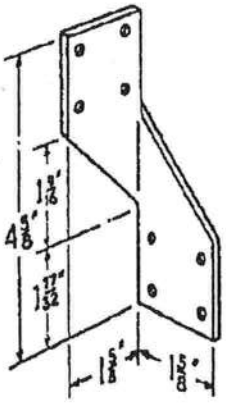
1. In order to comply with the South Florida Building Code Section 2908.05, two (2) H2, H2.5, or H3 connectors shall be installed for a minimum uplift of 700#.
2. Allowable loads are for one tie only.
3. A minimum rafter thickness of 2 ½ " must be used when framing anchors are installed on each side of the rafter and on the same side of the plate.



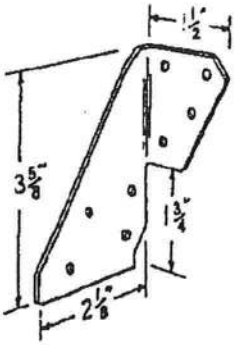
H1



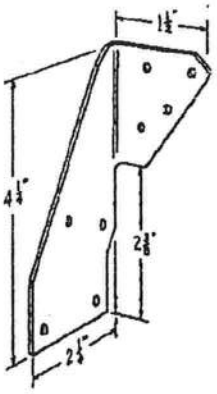
H2.5



H3



H4

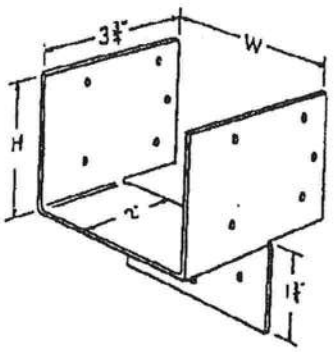


H5

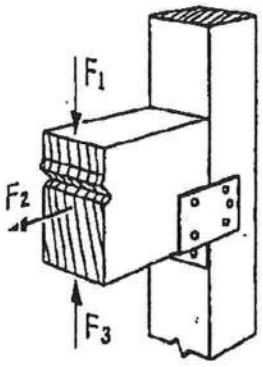
HEADER HANGERS

MODEL NO.	GA	Dimensions		Fasteners 16d		Allowable Loads		
		W	H	Stud	Header	F1	F2	F3
HH4	16	3½	2 - 13/16	8	4	1195	530	530
HH6	16	5½	5 - ⅛	12	6	1595	800	800

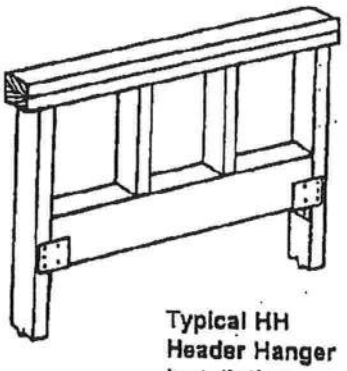
1. The above allowable loads require a minimum 2½" lumber thickness.



HH4



HH Load Directions



Typical HH Header Hanger Installation

GENERAL NOTES

1) Steel for the HH4 and HH6 shall conform to ASTM A-653 FS with $F_{y,min}$ = 28 ksi and $F_{u,min}$ = 38 ksi and have a minimum galvanized coating of G90.

2) Fasteners are common wire nails unless otherwise noted.

3) Allowable uplift loads have been increased 33% for wind loading with no other duration increases allowed.

4) Allowable down loads have not been increased by any duration factor.

5) Allowable loads are based on the National Design Specification for Wood Construction 1991 Edition &1993 Errata, for Douglas Fir-Larch (G=.50 or better) and tests performed in accordance with ASTM D1761.

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE January 11, 2001
BY
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0926.01

ICE USE

SIMPSON STRONG-TIE CO., INC.
4637 Chabot Drive, Suite 200
Pleasanton, CA 94588

TITLE: HH HEADER HANGERS

Drawing No.: SSTMD-001 | Sheet No.: 2/6

Drawing Date: 9-14-00 | Revision Date: — —

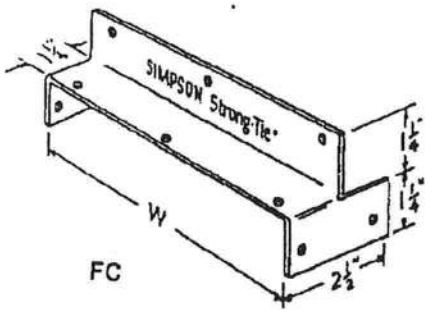
Evon M.C. Ballash, P.E. Civil #PE0051762

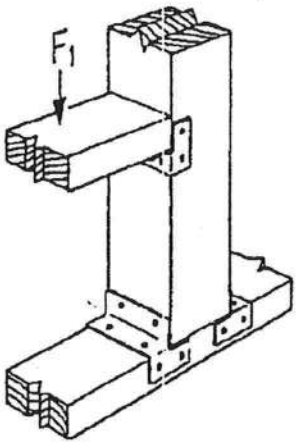
12/18/00
Evon M. C. Ballash

FC FRAMING CLIPS

MODEL NO.	GA	W	FASTENERS		ALLOWABLE LOAD
		In	Post	Header	
FC4	16	3⁹⁄₁₆	6 - 16d	2 - 16d	800
FC6	16	5½	7 - 16d	3 - 16d	920

1. The above allowable loads require a minimum 2½ " lumber thickness.
2. Loads may not be increased for short-term loading.





Typical FC Load Direction

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE January 11, 2001
BY
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0926.01

GENERAL NOTES

1) Steel for the FC4 and FC6 shall conform to ASTM A-653 FS with $F_{y,min}$ = 28 ksi and $F_{u,min}$ = 38 ksi and have a minimum galvanized coating of G90.

2) Fasteners are common wire nails unless otherwise noted.

3) Allowable uplift loads have been increased 33% for wind loading with no other duration increases allowed.

4) Allowable down loads have not been increased by any duration factor.

5) Allowable loads are based on the National Design Specification for Wood Construction 1991 Edition &1993 Errata, for Douglas Fir-Larch (G=.50 or better) and tests performed in accordance with ASTM D1761.

FFICE USE

SIMPSON STRONG-TIE CO., INC.
4637 Chabot Drive, Suite 200
Pleasanton, CA 94588

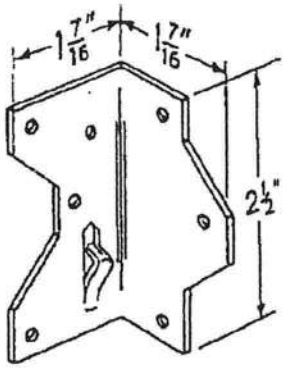
TITLE: FC FRAMING CLIPS	
Drawing No.: 9-14-00	Sheet No.: 3/6
Drawing Date: SSTMD-001	Revision Date: — —
Evon M C Ballesh P E	Civil #DE0051762

12/18/00
Evon M C Ballesh

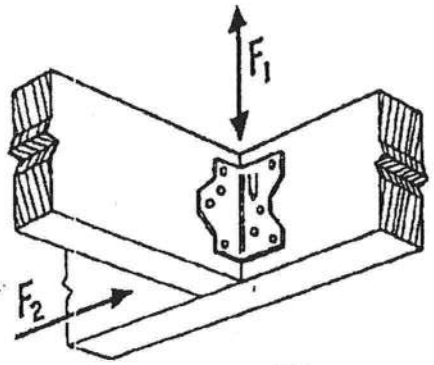
FRAMING ANCHORS

Model No.	GA	Load Direction	Fasteners 8d x 1½		Allowable Load
			Header	Joist	
A34	18	F1	4	4	345
A34	18	F2	4	4	280

1. Allowable loads are for one anchor. When anchors are installed on each side of the joist, the minimum joist thickness is 3".



A34



A34

GENERAL NOTES

1) Steel for the A34, A35and A35F shall conform to ASTM A-653 FS with $F_{y,min}$ = 33 ksi and $F_{u,min}$ = 45 ksi and have a minimum galvanized coating of G90.

2) Fasteners are common wire nails unless otherwise noted.

3) Allowable uplift loads have been increased 33% for wind loading with no other duration increases allowed.

4) Allowable down londs have not been increased by any duration factor.

5) Allowable loads are based on the National Design Specification for Wood Construction 1991 Edition &1993 Errata, for Douglas Fir-Larch (G=.50 or better) and tests performed in accordance with ASTM D1761.

APPROVED AS COMPLYING WITH THE
SOUTH FLORIDA BUILDING CODE
DATE January 11, 2001
BY
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0926.01

FICE USE

SIMPSON STRONG-TIE CO., INC.
4637 Chabot Drive, Suite 200
Pleasanton, CA 94588

TITLE: A34 FRAMING ANCHORS

Drawing No.: SSTMD-001 Sheet No.: 4/6

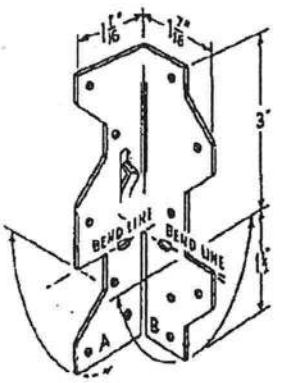
Drawing Date: 9-14-00 Revision Date: --

Evon M.C. Ballash, P.E. Civil #PE0051762

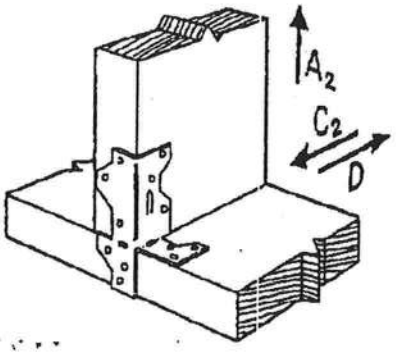
12/18/00
Evon C Ballash

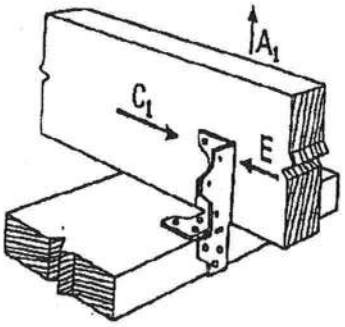
FRAMING ANCHORS

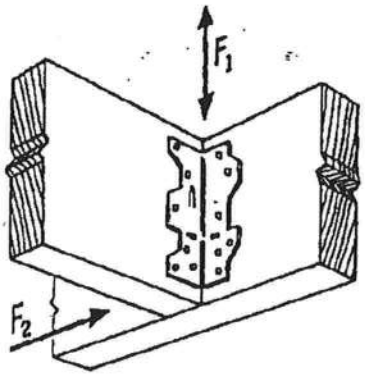
Model No.	GA	Load Direction	Fasteners 8d x 1½		Allowable Load
			Header	Joist	
A35	18	A1	6	3	260
A35	18	E	6	3	260
A35	18	C1	6	3	170
A35	18	A2	6	6	260
A35	18	C2	6	6	260
A35	18	D	6	6	150
A35	18	F1	6	6	450
A35	18	F2	6	6	430



A35







GENERAL NOTES

1) Steel for the A34, A35and A35F shall conform to ASTM A-653 FS with $F_{y,min}$ = 33 ksi and $F_{u,min}$ = 45 ksi and have a minimum galvanized coating of G90.

2) Fasteners are common wire nails unless otherwise noted.

3) Allowable uplift loads have been increased 33% for wind loading with no other duration increases allowed.

4) Allowable down loads have not been increased by any duration factor.

5) Allowable loads are based on National Design Specification for Wood Construction 1991 Edition &1993 Errata, for Douglas Fir-Larch (G=.50 or better) and tests performed in accordance with ASTM D1761.

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE January 11, 2001
BY
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0926.01

CE USE

SIMPSON STRONG-TIE CO., INC.
4637 Chabot Drive, Suite 200
Pleasanton, CA 94588

TITLE: A35 FRAMING ANCHORS

Drawing No.: SSTMD-001 Sheet No.: 5/6

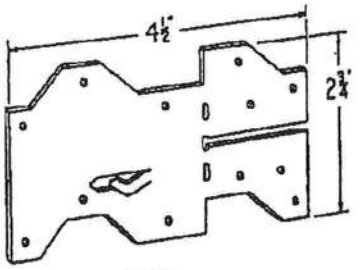
Drawing Date: 9-14-00 Revision Date: — —

Evon M.C. Ballash, P.E. Civil #PE0051762

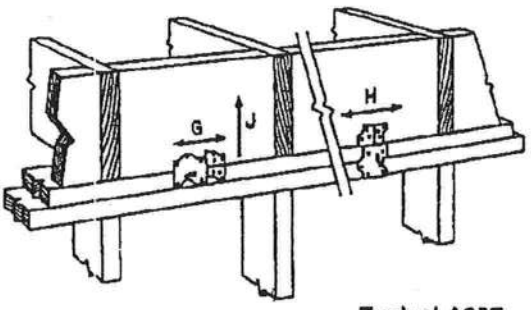
12/18/00
Evon M C Ballash

FRAMING ANCHORS

Model No.	GA	Load Direction	Fasteners 8 x 1½		Allowable Load
			Header	Joist	
A35F	18	G	6	6	500
A35F	18	J	6	6	200
A35F	18	H	6	6	440



A35F



Typical A35F Installation

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE January 11, 2001
BY
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0926.01

GENERAL NOTES

1) Steel for the A34, A35and A35F shall conform to ASTM A-653 FS with $F_{y,min}$ = 33 ksi and $F_{u,min}$ = 45 ksi and have a minimum galvanized coating of G90.

2) Fasteners are common wire nails unless otherwise noted.

3) Allowable uplift loads have been increased 33% for wind loading with no other duration increases allowed.

4) Allowable down loads have not been increased by any duration factor.

5) Allowable loads are based on the National Design Specification for Wood Construction 1991 Edition & 1993 Errata, for Douglas Fir-Larch (G=.50 or better) and tests performed in accordance with ASTM D1761.

OFFICE USE

SIMPSON STRONG-TIE CO., INC.
4637 Chabot Drive, Suite 200
Pleasanton, CA 94588

TITLE: A35F FRAMING ANCHORS

Drawing No.: SSTMD-001 | Sheet No.: 6/6

Drawing Date: 9-14-00 | Revision Date: - -

Evon M.C. Ballash, P.E. Civil #PE0051762

12/8/00
Evon M C Ballash

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901
FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2902
FAX (305) 372-6339

Jeff Dunagan
Simpson Strong-Tie Company, Inc.
4120 Dublin Boulevard Suite 400
Dublin, CA 94568

NOTICE OF PROPOSED ACTION

To: ***Members of the Board of Rules and Appeals*** and
Simpson Strong-Tie Company, Inc., Applicant

In accordance with Dade County Administrative Order 10-3, which governs the product review process, the Product Control Division of the Office of Code Compliance, intends to issue a Product Control Notice of Acceptance to **Simpson Strong-Tie Company, Inc.** for **Wood Connectors**, ***No.*** 02-0402.01 , to allow its use in Dade County and its municipalities.

To: ***Members of the Board of Rules and Appeals*** ;

The documentation being provided to you represents the recommendation of the Product Control Division of the Office of Code Compliance in regards to the submittal of **Simpson Strong-Tie Company, Inc.** for **Wood Connectors**, ***No. 02-0402.01***. Under the provisions of Dade County Administrative Order 10-3, which governs the product review process. You must review this documentation. If within 20 days from the date of mailing, we do not receive any written objection stating the reason(s) for your disapproval, this product will be automatically approved.

To: **Simpson Strong-Tie Company, Inc., Applicant**

The Product Control Division of the Office of Code Compliance, in accordance with Dade County Administrative Order 10-3, which governs the product review process, has issued this notice of proposed action and intends to issue a Product Control Notice of Acceptance for your **Wood Connectors**, ***No. 02-0402.01***, to be used in Dade County and its municipalities, unless a member of the Board of Rules and Appeals or yourself has any objections. Should you not be in accord with this notice of proposed action and wish to appeal our recommendation, you must make a written request, stating the reasons for your objection(s), to our office within 20 days of the date of mailing. Upon receipt of your written request a hearing date will be set so that you can present your objection(s) to the Board of Rules and Appeals .

Sincerely,

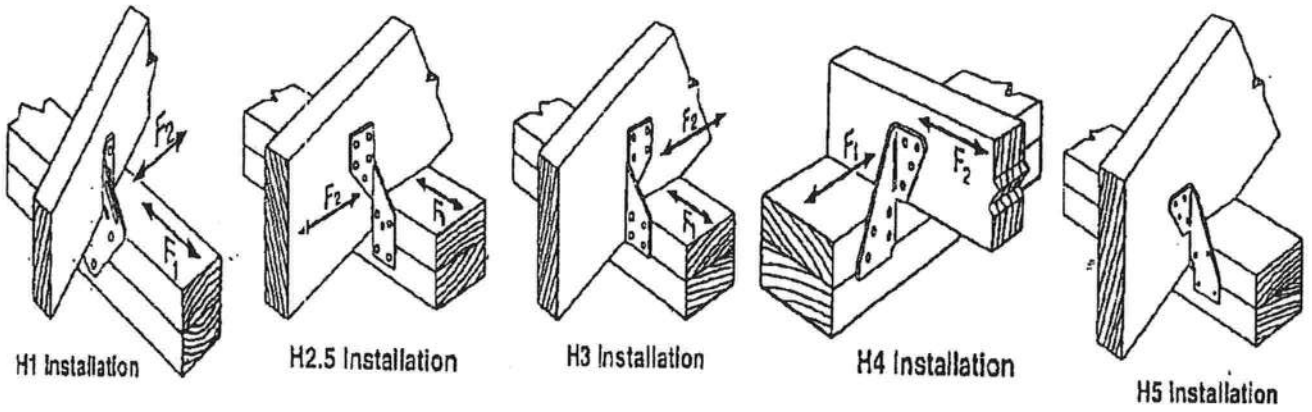
Raul Rodriguez
Chief Product Control Division

Francisco J. Quintana, R.A.
Director

DATE OF MAILING: 04/12/2002

Mailed by: P. Rojade

\\s0450001\pc2000\templates\notice proposed action.dot



GENERAL NOTES

1) Steel for the H1, H2.5, H3, and H5 shall conform to ASTM A-653 FS with $F_{y,min}$ = 28 ksi and $F_{u,min}$ = 38 ksi and have a minimum galvanized coating of G90. Steel for the H4 shall conform to ASTM A-653 SS GR 33 with Fy,min = 33 ksi and Fu,min = 45 ksi and have a minimum galvanized coating of G90.

2) Fasteners are common wire nails unless otherwise noted.

3) Allowable uplift loads have been increased 33% for wind loading with no other duration increases allowed.

4) Allowable loads are based on the National Design Specification for Wood Construction 1991 Edition &1993 Errata, for Douglas Fir-Larch (G=.50 or better) and tests performed in accordance with ASTM D1761.

APPROVED AS COMPLYING WITH THE:
SOUTH FLORIDA BUILDING CODE
DATE January 11, 2001
BY .....
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0926.01

OFFICE USE

SIMPSON STRONG-TIE CO., INC.
4637 Chabot Drive, Suite 200
Pleasanton, CA 94588

TITLE: HURRICANE TIES

Drawing No.: SSTMD-001 Sheet No.: 1/6

Drawing Date: 9-14-00 Revision Date: - -

Evon M.C. Ballash, P.E. Civil #PE0051762

12/18/2
Evon M. C Ballash

MIAMI-DADE

BUILDING CODE COMPLIANCE OFFICE (BCCO)
PRODUCT CONTROL DIVISION

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

Simpson Strong-Tie Co., Inc.
4120 Dublin Blvd., Suite 400
Dublin, CA 94568

SCOPE:
This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by Miami-Dade County Product Control Division and accepted by the Board of Rules and Appeals (BORA) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Division (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BORA reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Division that this product or material fails to meet the requirements of the applicable building code.

This product is approved as described herein, and has been designed to comply with the High Velocity Hurricane Zone of the Florida Building Code.

DESCRIPTION: Wood Connectors

APPROVAL DOCUMENT: Drawing No. no number, titled "A Angles", "CS Coiled Strap", LPC Post Caps", "LTHJ Truss Hip/Jack Hangers", & "SP Stud Plate Ties", sheets 1 through 5, prepared, signed and sealed by Evon Ballash, P.E., dated 7/9/99, bearing the Miami-Dade County Product Control Renewal stamp with the Notice of Acceptance number and expiration date by the Miami-Dade County Product Control Division.

MISSILE IMPACT RATING: None

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

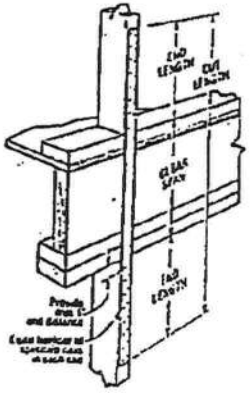
ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

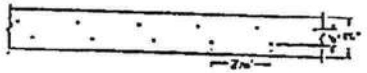
INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.
This NOA renews NOA # 99-0623.04 and, consists of this page 1 as well as approval document mentioned above.
The submitted documentation was reviewed by **Raul Rodriguez.**

CS Coiled Strap

Model No.	Total Length	End Length	Cut Length	Fasteners (Total) 1	Allowable Loads
CS16S	100'	12"	clear span + 24"	22-8d	925
CS18S	200'	10"	clear span + 20"	18-10d	
CS18-R	25'	10"	clear span + 20"	18-10d	

1) Minimum Nail Penetration Shall Be 1⅜" for 8d & 1⅞" for 10d





APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE December 29 1999
BY
PRODUCT CONTROL DIVISION
REGIONAL CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 99-0722.04

GENERAL NOTES:

1) Steel shall conform to 18 gauge ASTM A-653 FS with $F_{y,min}$ = 33 ksi and $F_{u,min}$ = 45 ksi and have a minimum galvanized coating of G60
2) Fasteners are common wire nails unless noted.
3) Allowable uplift loads have been increased 33% for wind loading with no other duration increases allowed.
4) Allowable loads are based on the National Design Specification for Wood Construction 1991 Edition & 1993 Errata, for Douglas Fir-Larch (G=0.50 or better) and tests performed in accordance with ASTM D1761.

FOR OFFICE USE

SIMPSON STRONG-TIE CO., INC.
4637 Chabot Drive, Suite 200
Pleasanton, CA 94588

TITLE: CS Coiled Strap

Drawing No.
Sheet No. 2/5
Drawing Date 2-2-98
Revision Date --

Evon M.C. Ballash, P.E. Civil #PE0051762

7/15/02
Evon M C Ballash

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

CONTRACTOR LICENSING SECTION
(305) 375-2527 FAX (305) 375-2558

CONTRACTOR ENFORCEMENT DIVISION
(305) 375-2966 FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 372-6339

PRODUCT CONTROL NOTICE OF ACCEPTANCE

Simpson Strong-Tie Company, Inc.
4637 Chabot Drive Suite 200
Pleasanton ,CA 94588

Your application for Notice of Acceptance (NOA) of:
Wood Connectors
under Chapter 8 of the Code of Miami-Dade County governing the use of Alternate Materials and Types of Construction, and completely described herein, has been recommended for acceptance by the Miami-Dade County Building Code Compliance Office (BCCO) under the conditions specified herein.

This NOA shall not be valid after the expiration date stated below. BCCO reserves the right to secure this product or material at any time from a jobsite or manufacturer's plant for quality control testing. If this product or material fails to perform in the approved manner, BCCO may revoke, modify, or suspend the use of such product or material immediately. BCCO reserves the right to revoke this approval, if it is determined by BCCO that this product or material fails to meet the requirements of the South Florida Building Code.

The expense of such testing will be incurred by the manufacturer.

ACCEPTANCE NO.: 00-0512.11
EXPIRES: 05/10/2004

Raul Rodriguez
Chief Product Control Division

THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL CONDITIONS
BUILDING CODE & PRODUCT REVIEW COMMITTEE

This application for Product Approval has been reviewed by the BCCO and approved by the Building Code and Product Review Committee to be used in Miami-Dade County, Florida under the conditions set forth above.

Francisco J. Quintana, R.A.
Director
Miami-Dade County
Building Code Compliance Office

APPROVED: 05/10/2001

Simpson Strong-Tie Co., Inc.

ACCEPTANCE NO: 00-0512.11

APPROVED: MAY 10 2001

EXPIRES: MAY 10 2004

NOTICE OF ACCEPTANCE: SPECIFIC CONDITIONS

1. **SCOPE**

1.1 This approves wood connectors; as described in Section 2 of this Notice of Acceptance, designed to comply with the South Florida Building Code (SFBC), 1994 Edition for Miami-Dade County. For the locations where the actual loads as determined by SFBC Chapter 23; do not exceed the design load indicated in the approved drawings.

2. **PRODUCT DESCRIPTION**

2.1 The **Simpson Strong-Tie Wood Connectors** shall be fabricated and used in strict compliance with the following documents: Drawing with No. S-2068 and sheets 1 through 5 of 5, titled "Post Bases, Seismic & Hurricane Ties and Embedded Truss Anchors", prepared by Simpson Stong-Tie Co., Inc., dated 10/23/00 with latest revision on 03/08/01. The drawings shall bear the Miami-Dade County Product Control Approval stamp with the Notice of Acceptance number and approval date by the Miami-Dade Product Control Division. These documents shall hereinafter be referred to as the approved drawings.

3. **LIMITATIONS**

3.1 Allowable loads are for Douglas Fir-Larch or better with a specific gravity of 0.50 and moisture content of 19% or less.

3.2 Allowable loads are based on testing per ASTM D1761 and calculations per National Design Specifications for Wood Construction 1991 Edition & 1993 Errata.

4. **INSTALLATION**

4.1 The wood connectors shall be installed in strict compliance with the approved drawings.

5. **LABELING**

5.1 Each wood connector shall bear a permanent label with the manufacturer's name or logo, city, state and the following statement: "Miami-Dade County Product Control Approved".

6. **BUILDING PERMIT**

6.1 Application for Building Permit shall be accompanied by copies of the following:

6.1.1 This Notice of Acceptance

6.1.2 Duplicate copies of the approved drawings as identified in Section 2 of this Notice of Acceptance, clearly marked to show the hangers and angles selected for the proposed installation.

6.1.3 Any other document required by the Building Official or the SFBC in order to properly evaluate the installation of these products.

Candido Font, PE, Sr. Product Control Examiner
Product Control Division

Simpson Strong-Tie Co., Inc.

ACCEPTANCE NO.: 00-0512.11

APPROVED: MAY 10 2001

EXPIRES: MAY 10 2004

NOTICE OF ACCEPTANCE STANDARD CONDITIONS

1. Renewal of this Acceptance (approval) shall be considered after a renewal application has been filed and the original submitted documentation, including test supporting data, engineering documents, are no older than eight (8) years.

2. Any and all approved products shall be permanently labeled with the manufacturer's name, city, state, and the following statement: "Miami-Dade County Product Control Approved", or as specifically stated in the specific conditions of this Acceptance.

3. Renewals of Acceptance will not be considered if:
a) There has been a change in the South Florida Building Code affecting the evaluation of this product and the product is not in compliance with the code changes;
b) The product is no longer the same product (identical) as the one originally approved;
c) If the Acceptance holder has not complied with all the requirements of this acceptance, including the correct installation of the product;
d) The engineer who originally prepared, signed and sealed the required documentation initially submitted is no longer practicing the engineering profession;

4. Any revision or change in the materials, use, and/or manufacture of the product or process shall automatically be cause for termination of this Acceptance, unless prior written approval has been requested (through the filing of a revision application with appropriate fee) and granted by this office.
5. Any of the following shall also be grounds for removal of this Acceptance:
a) Unsatisfactory performance of this product or process.
b) Misuse of this Acceptance as an endorsement of any product, for sales, advertising or any other purpose.

6. The Notice of Acceptance number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the Notice of Acceptance is displayed, then it shall be done in its entirety.

7. A copy of this Acceptance as well as approved drawings and other documents, where it applies, shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at all time. The engineer need not reseal the copies.

8. Failure to comply with any section of this Acceptance shall be cause for termination and removal of Acceptance.

9. This Notice of Acceptance consists of pages 1, 2 and this last page 3.

Candido Font, PE, Sr. Product Control Examiner
Product Control Division

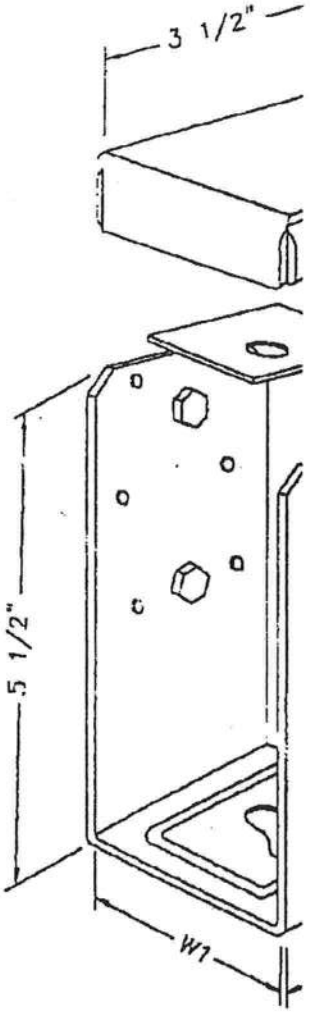
END OF THIS ACCEPTANCE

SIMPSON STRONG-TIE CO., INC.

ABU & EPB ELEVATED POST BASES, H10 SEISMIC & HURRICANE TIE
HETA/HETAL EMBEDDED TRUSS ANCHORS & PB44 POST BASE

ABU ELEVATED POST BASES GENERAL NOTES

1. THIS PRODUCT IS DESIGNED TO MEET THE SOUTH FLORIDA BUILDING CODE 1994 EDITION FOR MIAMI-DADE COUNTY.
2. THE STEEL SHALL CONFORM TO ASTM A-653 CS WITH Fy. MIN. = 28ksi & Fu. MIN. = 38ksi FOR BASE & ASTM A-653 SS GRADE 33 WITH Fy MIN. = 33 ksi & Fu MIN. = 45ksi FOR THE STRAP, & HAVE A MINIMUM GALVANIZED COATING OF G60.
3. FASTENERS ARE COMMON WIRE NAILS UNLESS OTHERWISE NOTED.
4. ALLOWABLE DOWNLIFT LOADS HAVE NOT BEEN INCREASED BY ANY DURATION FACTOR.
5. ALLOWABLE UPLIFT LOADS HAVE BEEN INCREASED 33% FOR WIND LOADING. LOADS MAY NOT BE INCREASED FOR SHORT-TERM LOADING.
6. ALLOWABLE LOADS ARE BASED ON THE NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION 1991 EDITION & 1993 ERRATA, FOR DOUGLAS FIR-LARCH (G = 0.50 OR BETTER) & TESTS PERFORMED IN ACCORDANCE WITH ASTM D1761.
7. NOT RECOMMENDED FOR NON-TOP SUPPORTED INSTALLATIONS SUCH AS FENCES.
8. FOR PRE-POUR INSTALLED ANCHORS, EMBED 4" INTO CONCRETE. FOR EPOXY OR WEDGE ANCHORS, SELECT DESIGN & INSTALL ACCORDING TO MANUFACTURER'S RECOMMENDATIONS & SOUND ENGINEERING PRACTICES.



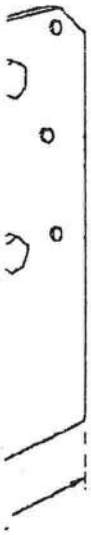
ABU44 ELEVATED

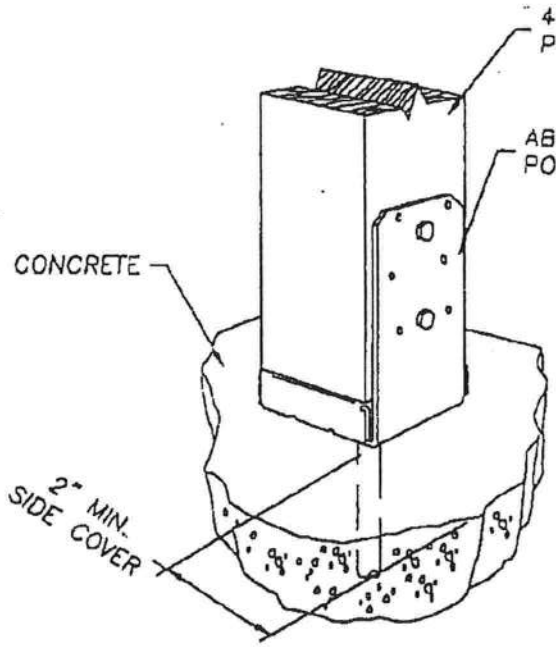
TABLE OF CONTENTS

SHEET #	DESCRIPTION
1	ABU ELEVATED POST BASES, NOTES & CHART
2	EPB ELEVATED POST BASES, NOTES & CHART
3	H10 SEISMIC & HURRICANE TIE, NOTES & CHART
4	HETA/HETAL EMBEDDED TRUSS ANCHORS, NOTES & CHART
5	PB44 POST BASE, NOTES & CHART

MODEL NO.	NOMINAL POST SIZE	MATERIAL		DIMENSIONS					FASTENER	
		BASE	STRAP	W1	W2	D	L	HB	ANCHOR DIA.[3]	NAILS
ABU44	4 x 4	16GA.	12GA.	3 9/16"	3 1/2"	–	3"	1 3/4"	5/8"	12-16d
ABU46	4 x 6	12GA.	12GA.	3 9/16"	5 3/8"	–	5"	2 5/8"	5/8"	12-16d
ABU66	6 x 6	12GA.	10GA.	5 1/2"	5 3/8"	–	5"	1 3/4"	5/8"	12-16d

Mark
State Of Florida
Marc
Mark C.





ABU44 ELEVATED POST BASE
TYPICAL INSTALLATION

BASE

	ALLOWABLE LOADS		
	UPLIFT (133)		
TS DIA.	NAILS	BOLTS	DOWN (100)
1/2"	1340	1250	6400
1/2"	2300	1665	9635
1/2"	1980	1700	10000

thennan
ional Engineer #40116
01

MAY 10 2001

00-0512.11

SIMPSON STRONG-TIE CO., INC.
4637 CHABOT DRIVE, SUITE 200
PLEASANTON, CA. 94588
PH. 800.925.5099
FAX. 925.847.3068

PRODUCT: POST BASES, SEISMIC & HURRICANE TIES & EMBEDDED TRUSS ANCHORS

PART OR ASSEMBLY: ABU ELEVATED POST BASES NOTES & CHART

REVISIONS

NO	DATE		BY
1	3/08/01	GENERAL REVISION	TJH

RW BUILDING CONSULTANTS, INC.
813.684.3831

DATE: 10/23/00
SCALE: N.T.S.
DWG. BY: TJH
CHK. BY: RW
DRAWING NO.: S-2068
SHEET 1 OF 5

Residential System Sizing Calculation

Summary

Rosewood Model

Lake City, FL 32025-

Project Title:
Greg Talley

Code Only
Professional Version
Climate: North

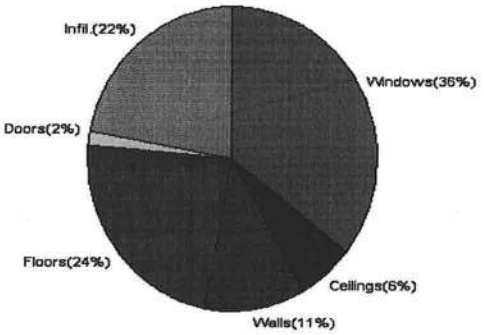
1/4/2007

Location for weather data: Gainesville - Defaults: Latitude(29) Altitude(152 ft.) Temp Range(M) Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(54gr.)					
Winter design temperature	33	F	Summer design temperature	92	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	37	F	Summer temperature difference	17	F
Total heating load calculation	**33349**	**Btuh**	**Total cooling load calculation**	**40409**	**Btuh**
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh
Total (Electric Heat Pump)	119.9	40000	Sensible (SHR = 0.75)	89.5	30000
Heat Pump + Auxiliary(0.0kW)	119.9	40000	Latent	144.9	10000
			Total (Electric Heat Pump)	99.0	40000

WINTER CALCULATIONS

Winter Heating Load (for 1672 sqft)

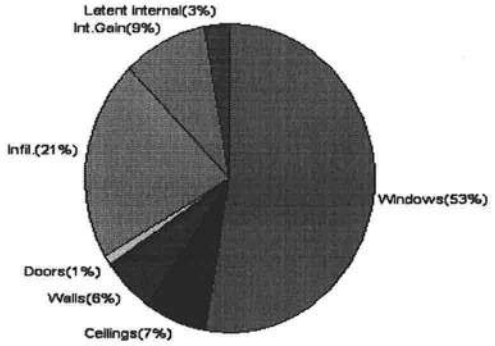
Load component			Load	
Window total	254	sqft	11921	Btuh
Wall total	1146	sqft	3765	Btuh
Door total	40	sqft	518	Btuh
Ceiling total	1750	sqft	2062	Btuh
Floor total	180	sqft	7859	Btuh
Infiltration	178	cfm	7224	Btuh
Duct loss			0	Btuh
Subtotal			**33349**	**Btuh**
Ventilation	0	cfm	0	Btuh
TOTAL HEAT LOSS			**33349**	**Btuh**



SUMMER CALCULATIONS

Summer Cooling Load (for 1672 sqft)

Load component			Load	
Window total	254	sqft	21239	Btuh
Wall total	1146	sqft	2292	Btuh
Door total	40	sqft	392	Btuh
Ceiling total	1750	sqft	2898	Btuh
Floor total			0	Btuh
Infiltration	156	cfm	2904	Btuh
Internal gain			3780	Btuh
Duct gain			0	Btuh
Sens. Ventilation	0	cfm	0	Btuh
Total sensible gain			**33506**	**Btuh**
Latent gain(ducts)			0	Btuh
Latent gain(infiltration)			5703	Btuh
Latent gain(ventilation)			0	Btuh
Latent gain(internal/occupants/other)			1200	Btuh
Total latent gain			**6903**	**Btuh**
TOTAL HEAT GAIN			**40409**	**Btuh**



Powered by ACCA MANUAL J

For Florida residences only

EnergyGauge® System Sizing
PREPARED BY: _____
DATE: 1-4-07

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

Rosewood Model

Lake City, FL 32025-

Project Title:
Greg Talley

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

1/4/2007

Component Loads for Whole House

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	1, Clear, Metal, 1.27	W	90.0	47.0	4229 Btuh
2	1, Clear, Metal, 1.27	W	30.0	47.0	1410 Btuh
3	1, Clear, Metal, 1.27	SW	20.0	47.0	940 Btuh
4	1, Clear, Metal, 1.27	S	15.0	47.0	705 Btuh
5	1, Clear, Metal, 1.27	N	30.0	47.0	1410 Btuh
6	1, Clear, Metal, 1.27	N	2.7	47.0	127 Btuh
7	1, Clear, Metal, 1.27	E	30.0	47.0	1410 Btuh
8	1, Clear, Metal, 1.27	E	16.0	47.0	752 Btuh
9	1, Clear, Metal, 1.27	S	20.0	47.0	940 Btuh
	Window Total		254(sqft)		11921 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	974	3.3	3200 Btuh
2	Frame - Wood - Adj(0.09)	13.0	172	3.3	565 Btuh
	Wall Total		1146		3765 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Adjacent		20	12.9	259 Btuh
2	Insulated - Exterior		20	12.9	259 Btuh
	Door Total		40		518Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin)	30.0	1750	1.2	2062 Btuh
	Ceiling Total		1750		2062Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab On Grade	0	180.0 ft(p)	43.7	7859 Btuh
	Floor Total		180		7859 Btuh
			Zone Envelope Subtotal:		26125 Btuh
Infiltration	Type	ACH X	Zone Volume	CFM=	
	Natural	0.80	13376	178.3	7224 Btuh
Ductload	Proposed leak free, R6.0, Supply(Attic), Return(Attic)			(DLM of 0.00)	0 Btuh
Zone #1			**Sensible Zone Subtotal**		**33349 Btuh**

Manual J Winter Calculations

Residential Load - Component Details (continued)

Rosewood Model

Lake City, FL 32025-

Project Title:
Greg Talley

Code Only
Professional Version
Climate: North

1/4/2007

WHOLE HOUSE TOTALS

	Subtotal Sensible	33349 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	33349 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

For Florida residences only

System Sizing Calculations - Winter

Residential Load - Room by Room Component Details

Rosewood Model

Project Title:
Greg Talley

Code Only
Professional Version
Climate: North

Lake City, FL 32025-

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

1/4/2007

Component Loads for Zone #1: Main

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	1, Clear, Metal, 1.27	W	90.0	47.0	4229 Btuh
2	1, Clear, Metal, 1.27	W	30.0	47.0	1410 Btuh
3	1, Clear, Metal, 1.27	SW	20.0	47.0	940 Btuh
4	1, Clear, Metal, 1.27	S	15.0	47.0	705 Btuh
5	1, Clear, Metal, 1.27	N	30.0	47.0	1410 Btuh
6	1, Clear, Metal, 1.27	N	2.7	47.0	127 Btuh
7	1, Clear, Metal, 1.27	E	30.0	47.0	1410 Btuh
8	1, Clear, Metal, 1.27	E	16.0	47.0	752 Btuh
9	1, Clear, Metal, 1.27	S	20.0	47.0	940 Btuh
	Window Total		254(sqft)		11921 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	974	3.3	3200 Btuh
2	Frame - Wood - Adj(0.09)	13.0	172	3.3	565 Btuh
	Wall Total		1146		3765 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Adjacent		20	12.9	259 Btuh
2	Insulated - Exterior		20	12.9	259 Btuh
	Door Total		40		518Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin)	30.0	1750	1.2	2062 Btuh
	Ceiling Total		1750		2062Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab On Grade	0	180.0 ft(p)	43.7	7859 Btuh
	Floor Total		180		7859 Btuh
			Zone Envelope Subtotal:		26125 Btuh
Infiltration	Type	ACH X	Zone Volume	CFM=	
	Natural	0.80	13376	178.3	7224 Btuh
Ductload	Proposed leak free, R6.0, Supply(Attic), Return(Attic)			(DLM of 0.00)	0 Btuh
Zone #1			**Sensible Zone Subtotal**		**33349 Btuh**

Manual J Winter Calculations

Residential Load - Component Details (continued)

Rosewood Model

Lake City, FL 32025-

Project Title:
Greg Talley

Code Only
Professional Version
Climate: North

1/4/2007

WHOLE HOUSE TOTALS

	Subtotal Sensible	33349 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	33349 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

For Florida residences only

System Sizing Calculations - Summer

Residential Load - Whole House Component Details

Rosewood Model

Lake City, FL 32025-

Project Title:
Greg Talley

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F 1/4/2007

Component Loads for Whole House

Window	Type* Pn/SHGC/U/InSh/ExSh/IS	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load
1	1, Clear, 1.27, None,N,N	W	1.5ft	8ft.	90.0	0.0	90.0	37	94	8464 Btuh
2	1, Clear, 1.27, None,N,N	W	13.5f	8ft.	30.0	30.0	0.0	37	94	1124 Btuh
3	1, Clear, 1.27, None,N,N	SW	13.5f	8ft.	20.0	20.0	0.0	37	75	749 Btuh
4	1, Clear, 1.27, None,N,N	S	13.5f	8ft.	15.0	15.0	0.0	37	43	562 Btuh
5	1, Clear, 1.27, None,N,N	N	1.5ft	8ft.	30.0	0.0	30.0	37	37	1124 Btuh
6	1, Clear, 1.27, None,N,N	N	1.5ft	8ft.	2.7	0.0	2.7	37	37	101 Btuh
7	1, Clear, 1.27, None,N,N	E	7.5ft	8ft.	30.0	19.3	10.7	37	94	1726 Btuh
8	1, Clear, 1.27, None,N,N	E	1.5ft	8ft.	16.0	0.0	16.0	37	94	1505 Btuh
9	1, Clear, 1.27, None,N,N	S	1.5ft	8ft.	20.0	20.0	0.0	37	43	749 Btuh
	Excursion									5136 Btuh
	Window Total				254 (sqft)					21239 Btuh

Walls	Type	R-Value/U-Value	Area(sqft)	HTM	Load
1	Frame - Wood - Ext	13.0/0.09	974.3	2.1	2032 Btuh
2	Frame - Wood - Adj	13.0/0.09	172.0	1.5	260 Btuh
	Wall Total		1146 (sqft)		2292 Btuh

Doors	Type	Area (sqft)	HTM	Load
1	Insulated - Adjacent	20.0	9.8	196 Btuh
2	Insulated - Exterior	20.0	9.8	196 Btuh
	Door Total	40 (sqft)		392 Btuh

Ceilings	Type/Color/Surface	R-Value	Area(sqft)	HTM	Load
1	Vented Attic/DarkShingle	30.0	1750.0	1.7	2898 Btuh
	Ceiling Total		1750 (sqft)		2898 Btuh

Floors	Type	R-Value	Size	HTM	Load
1	Slab On Grade	0.0	180 (ft(p))	0.0	0 Btuh
	Floor Total		180.0 (sqft)		0 Btuh

			Load
	Zone Envelope Subtotal:		26821 Btuh

Infiltration	Type	ACH	Volume(cuft)	CFM=	Load
	SensibleNatural	0.70	13376	156.1	2904 Btuh

Internal gain	Occupants	Btuh/occupant		Appliance	Load
	6	X 230	+	2400	3780 Btuh

Duct load	Proposed leak free, R6.0, Supply(Attic), Return(Attic)	DGM = 0.00	0.0 Btuh

	Sensible Zone Load	**33506 Btuh**

Manual J Summer Calculations

Residential Load - Component Details (continued)

Rosewood Model

Lake City, FL 32025-

Project Title:
Greg Talley

Code Only
Professional Version
Climate: North

1/4/2007

WHOLE HOUSE TOTALS

Whole House Totals for Cooling	**Sensible Envelope Load All Zones**	**33506 Btuh**
	Sensible Duct Load	0 Btuh
	Total Sensible Zone Loads	**33506 Btuh**
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
	Total sensible gain	**33506 Btuh**
	Latent infiltration gain (for 54 gr. humidity difference)	5703 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	0 Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200 Btuh
	Latent other gain	0 Btuh
	Latent total gain	**6903 Btuh**
	TOTAL GAIN	**40409 Btuh**

*Key: Window types (Pn - Number of panes of glass)
(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))
(Ornt - compass orientation)

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For Florida residences only

System Sizing Calculations - Summer

Residential Load - Room by Room Component Details

Rosewood Model

Lake City, FL 32025-

Project Title:
Greg Talley

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F 1/4/2007

Component Loads for Zone #1: Main

Window	Type* Pn/SHGC/U/InSh/ExSh/IS	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load
1	1, Clear, 1.27, None,N,N	W	1.5ft	8ft.	90.0	0.0	90.0	37	94	8464 Btuh
2	1, Clear, 1.27, None,N,N	W	13.5f	8ft.	30.0	30.0	0.0	37	94	1124 Btuh
3	1, Clear, 1.27, None,N,N	SW	13.5f	8ft.	20.0	20.0	0.0	37	75	749 Btuh
4	1, Clear, 1.27, None,N,N	S	13.5f	8ft.	15.0	15.0	0.0	37	43	562 Btuh
5	1, Clear, 1.27, None,N,N	N	1.5ft	8ft.	30.0	0.0	30.0	37	37	1124 Btuh
6	1, Clear, 1.27, None,N,N	N	1.5ft	8ft.	2.7	0.0	2.7	37	37	101 Btuh
7	1, Clear, 1.27, None,N,N	E	7.5ft	8ft.	30.0	19.3	10.7	37	94	1726 Btuh
8	1, Clear, 1.27, None,N,N	E	1.5ft	8ft.	16.0	0.0	16.0	37	94	1505 Btuh
9	1, Clear, 1.27, None,N,N	S	1.5ft	8ft.	20.0	20.0	0.0	37	43	749 Btuh
	Excursion									5136 Btuh
	Window Total				254 (sqft)					21239 Btuh

Walls	Type	R-Value/U-Value	Area(sqft)	HTM	Load
1	Frame - Wood - Ext	13.0/0.09	974.3	2.1	2032 Btuh
2	Frame - Wood - Adj	13.0/0.09	172.0	1.5	260 Btuh
	Wall Total		1146 (sqft)		2292 Btuh

Doors	Type	Area (sqft)	HTM	Load
1	Insulated - Adjacent	20.0	9.8	196 Btuh
2	Insulated - Exterior	20.0	9.8	196 Btuh
	Door Total	40 (sqft)		392 Btuh

Ceilings	Type/Color/Surface	R-Value	Area(sqft)	HTM	Load
1	Vented Attic/DarkShingle	30.0	1750.0	1.7	2898 Btuh
	Ceiling Total		1750 (sqft)		2898 Btuh

Floors	Type	R-Value	Size	HTM	Load
1	Slab On Grade	0.0	180 (ft(p))	0.0	0 Btuh
	Floor Total		180.0 (sqft)		0 Btuh

			Zone Envelope Subtotal:		26821 Btuh
Infiltration	Type	ACH	Volume(cuft)	CFM=	Load
	SensibleNatural	0.70	13376	156.1	2904 Btuh
Internal gain		Occupants	Btuh/occupant	Appliance	Load
		6	X 230 +	2400	3780 Btuh
Duct load	Proposed leak free, R6.0, Supply(Attic), Return(Attic)			DGM = 0.00	0.0 Btuh
			Sensible Zone Load		**33506 Btuh**

Manual J Summer Calculations

Residential Load - Component Details (continued)

Rosewood Model

Lake City, FL 32025-

Project Title:
Greg Talley

Code Only
Professional Version
Climate: North

1/4/2007

WHOLE HOUSE TOTALS

Whole House Totals for Cooling	**Sensible Envelope Load All Zones**	**33506**	**Btuh**
	Sensible Duct Load	0	Btuh
	Total Sensible Zone Loads	**33506**	**Btuh**
	Sensible ventilation	0	Btuh
	Blower	0	Btuh
	Total sensible gain	**33506**	**Btuh**
	Latent infiltration gain (for 54 gr. humidity difference)	5703	Btuh
	Latent ventilation gain	0	Btuh
	Latent duct gain	0	Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200	Btuh
	Latent other gain	0	Btuh
	Latent total gain	**6903**	**Btuh**
	TOTAL GAIN	**40409**	**Btuh**

*Key: Window types (Pn - Number of panes of glass)
(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))
(Ornt - compass orientation)

For Florida residences only

Residential Window Diversity

MidSummer

Rosewood Model

Lake City, FL 32025-

Project Title:
Greg Talley

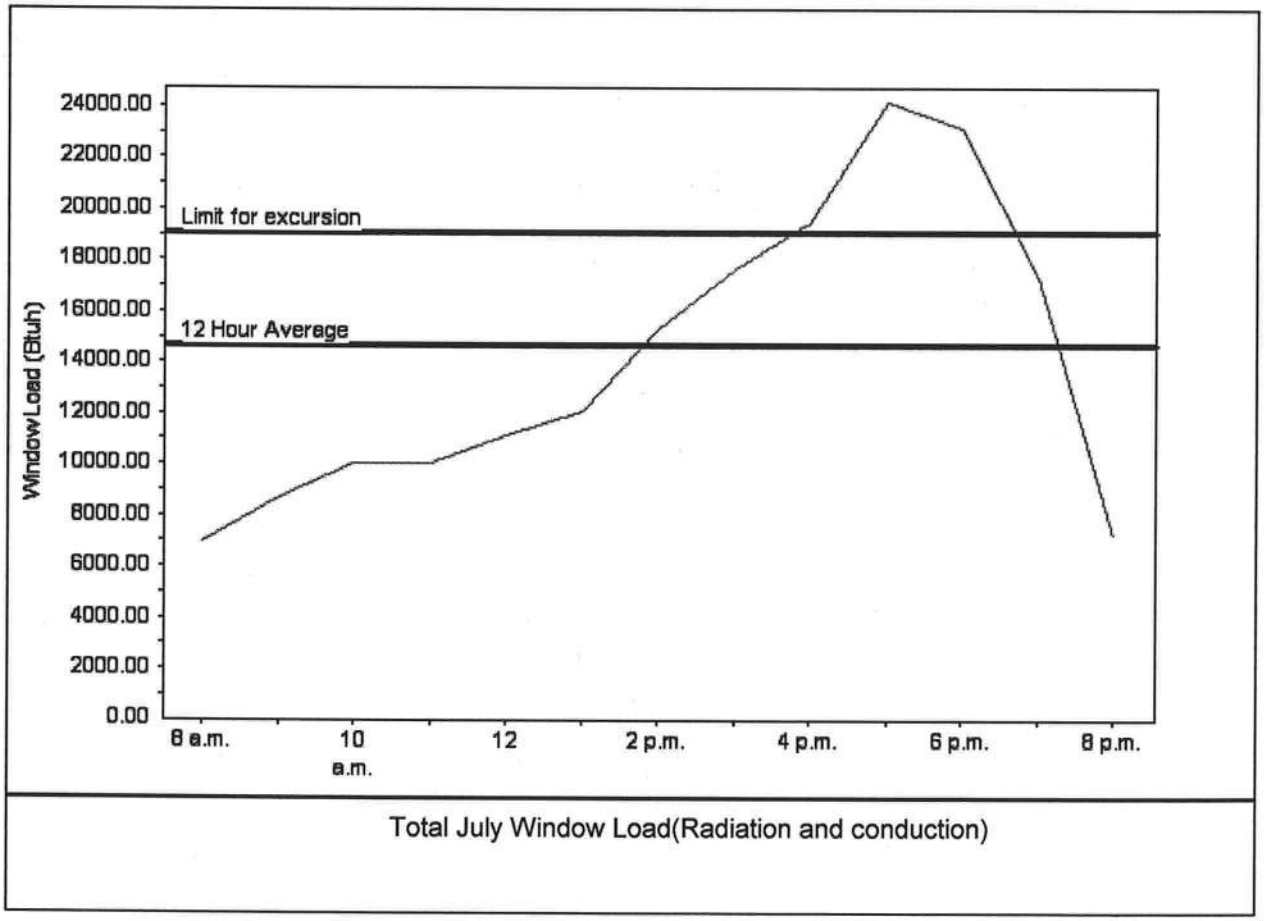
Code Only
Professional Version
Climate: North

1/4/2007

Weather data for: Gainesville - Defaults

Summer design temperature	92 F	Average window load for July	14659 Btu
Summer setpoint	75 F	Peak window load for July	24192 Btu
Summer temperature difference	17 F	Excusion limit(130% of Ave.)	19056 Btu
Latitude	29 North	Window excursion (July)	5136 Btuh

WINDOW Average and Peak Loads



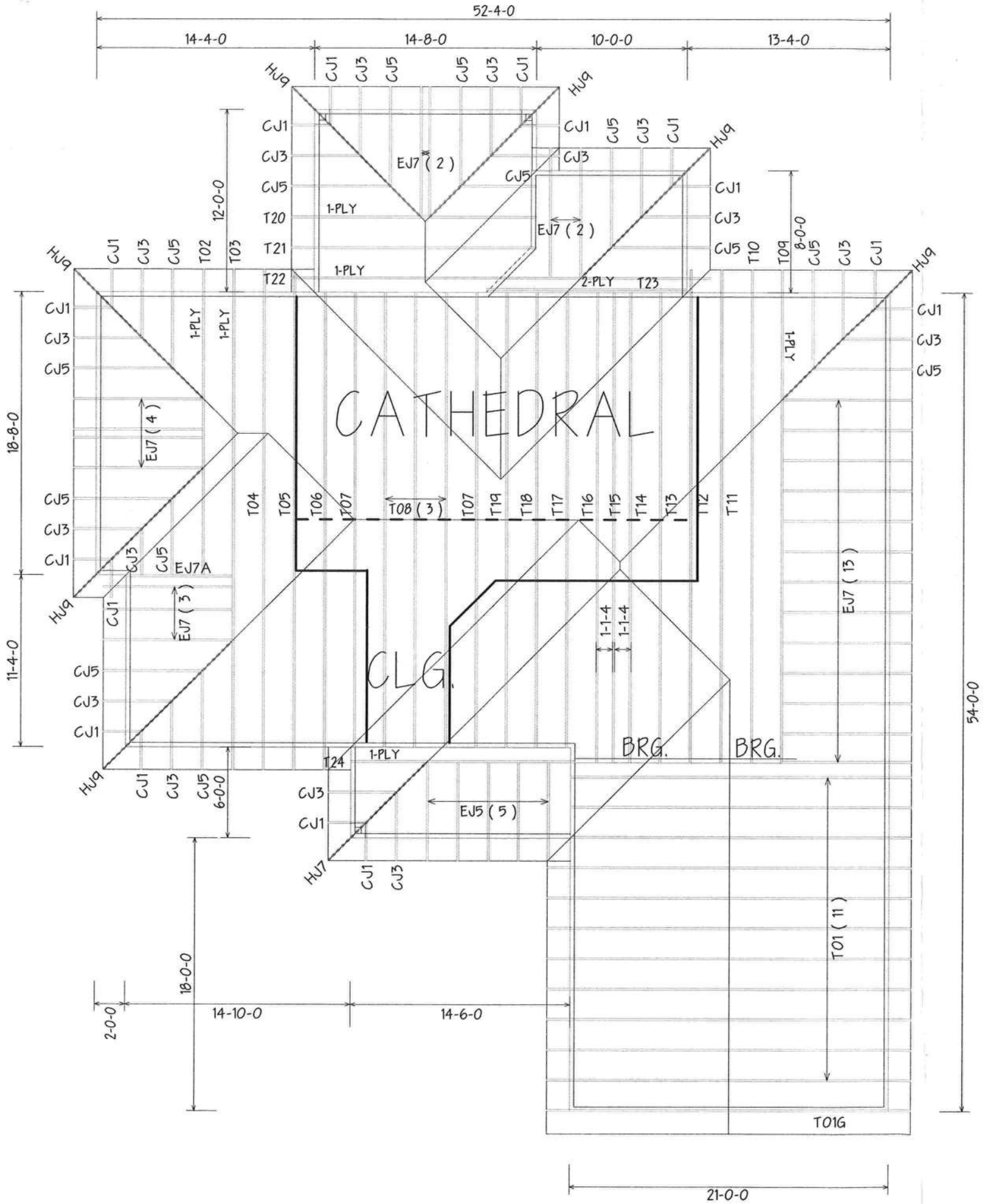
Total July Window Load(Radiation and conduction)

This application has glass areas that produce large heat gains for part of the day. Variable air volume devices are required to overcome spikes in solar gain for one or more rooms. Install a zoned system or provide zone control for problem rooms. Single speed equipment may not be suitable for the application.

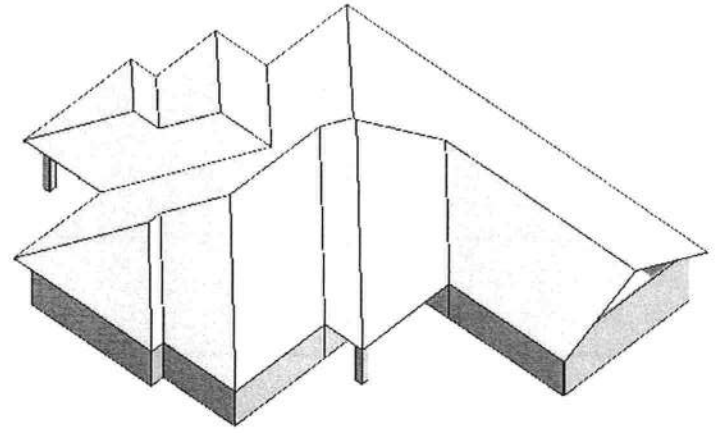
EnergyGauge® System Sizing for Florida residences only

PREPARED BY: _____

DATE: _____



6/12 PITCH
1'6" O/H



BEARING HEIGHT SCHEDULE

	8'-1 1/8"

NOTES:

1.) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

2.) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V105 FOR ALTERNATE BRACING REQUIREMENTS.

3.) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4.) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.

5.) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6.) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7.) ALL ROOF TRUSS HANGERS TO BE SIMPSON HTU26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8.) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date: _____

Approved by: _____ Date: _____

Builders FirstSource

Bunnell
PHONE: 904-437 3349 FAX: 904-437-3994

Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973

Lake City
PHONE: 904-755-6894 FAX: 904-755-7973

Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

BUILDER: G & J BLDR.

LEGAL ADDRESS: LOT 17 CANNON CREEK PL

MODEL:	REVISION:
CUSTOM	SCALE: NTS

DATE:	DRAWN BY:	JOB #:
1-18-07	K.L.H.	L223413