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ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
 Document ID:1UNU487-Z0116155310

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

Truss Fabricator:	Anderson Truss Company
Job Identification:	12-146--Fill in later Smith Garage -- Falling Creek, FL
Truss Count:	2
Model Code:	Florida Building Code
Truss Criteria:	FBC2010Res/TPI-2007(STD)
Engineering Software:	Alpine Software,Version 10.03.
Structural Engineer of Record:	The identity of the structural EOR did not exist as of
Address:	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads:	Roof - 40.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 120 MPH ASCE 7-10 -Closed

- 07/16/2012

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

Walter P. Finn
-Truss Design Engineer-

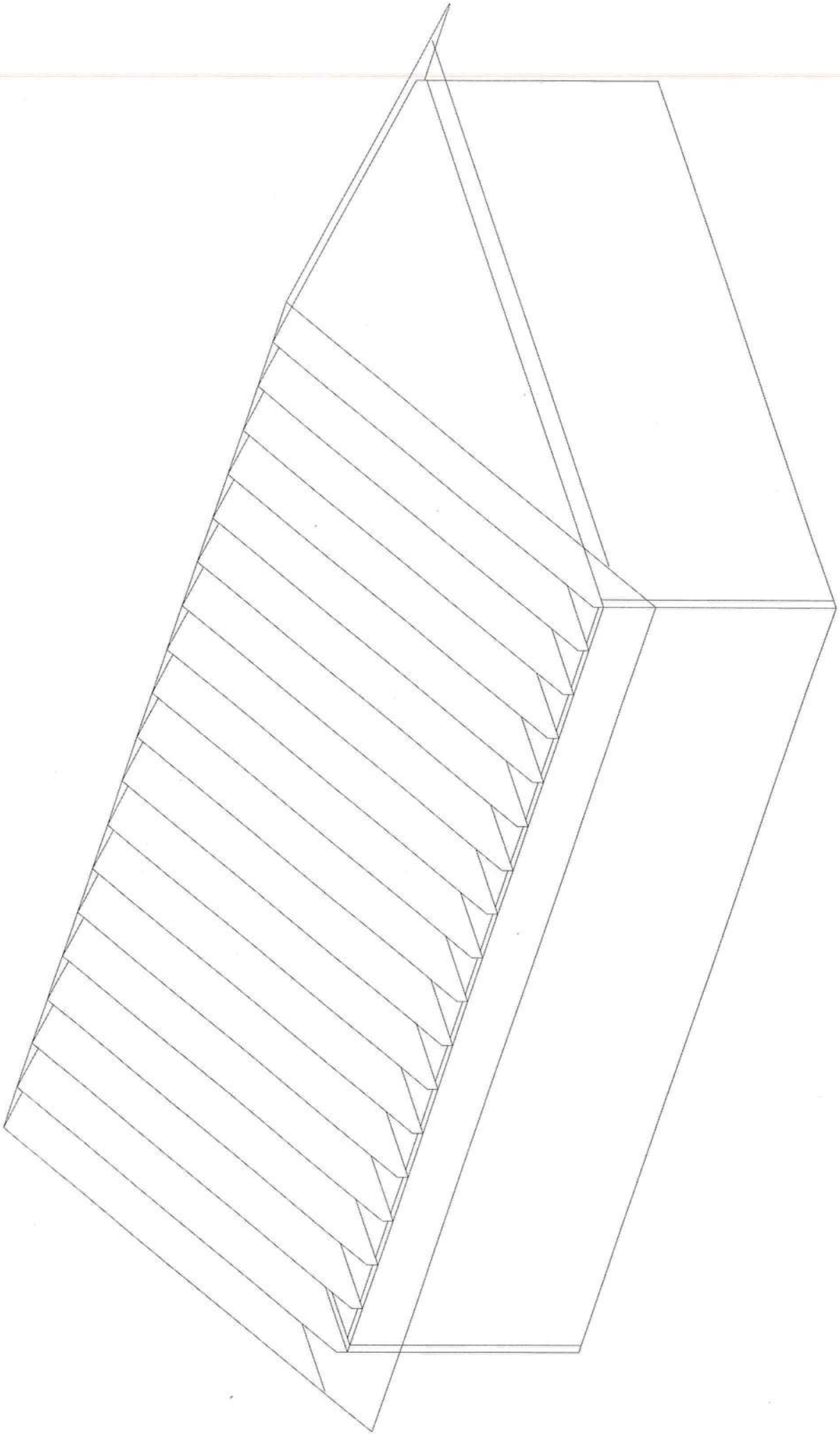
1950 Marley Drive
Haines City, FL 33844

Details: 12015EC1-GBLLETIN-GABRST10-

#	Ref	Description	Drawing#	Date
1	96991--A	24' Attic	12198001	07/16/12
2	96992--AGE	24' Gable	12198002	07/16/12

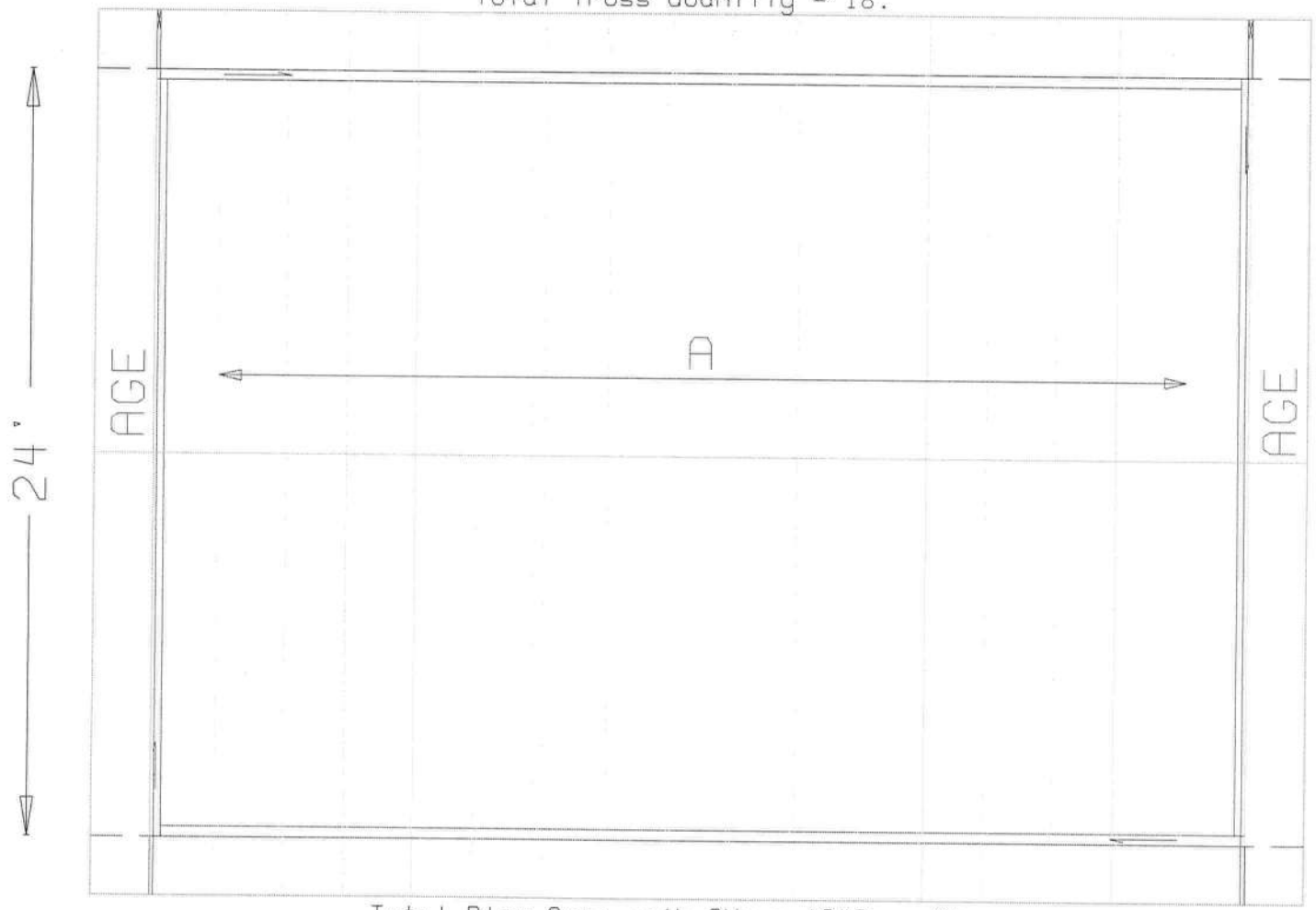
ALPINE

COLUMBIA COUNTY BUILDING DEPARTMENT Received for FILE COPY Code Compliance PLANS EXAMINER



Smith Garage

Total Truss Quantity = 18.



Total Plan Area with OHs = 1042 sq.ft

Roof Plane Sheathing Area = 1252 sq. ft

Location:
JOB DESCRIPTION:: Fill in later
/: Smith Garage

JOB NO:
12-146

PAGE NO:
1 OF 1

(12-146--Fill in later Smith Garage -- Falling Creek, FL - A 24' Attic)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Calculated horizontal deflection is 0.10" due to live load and 0.26" due to dead load.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

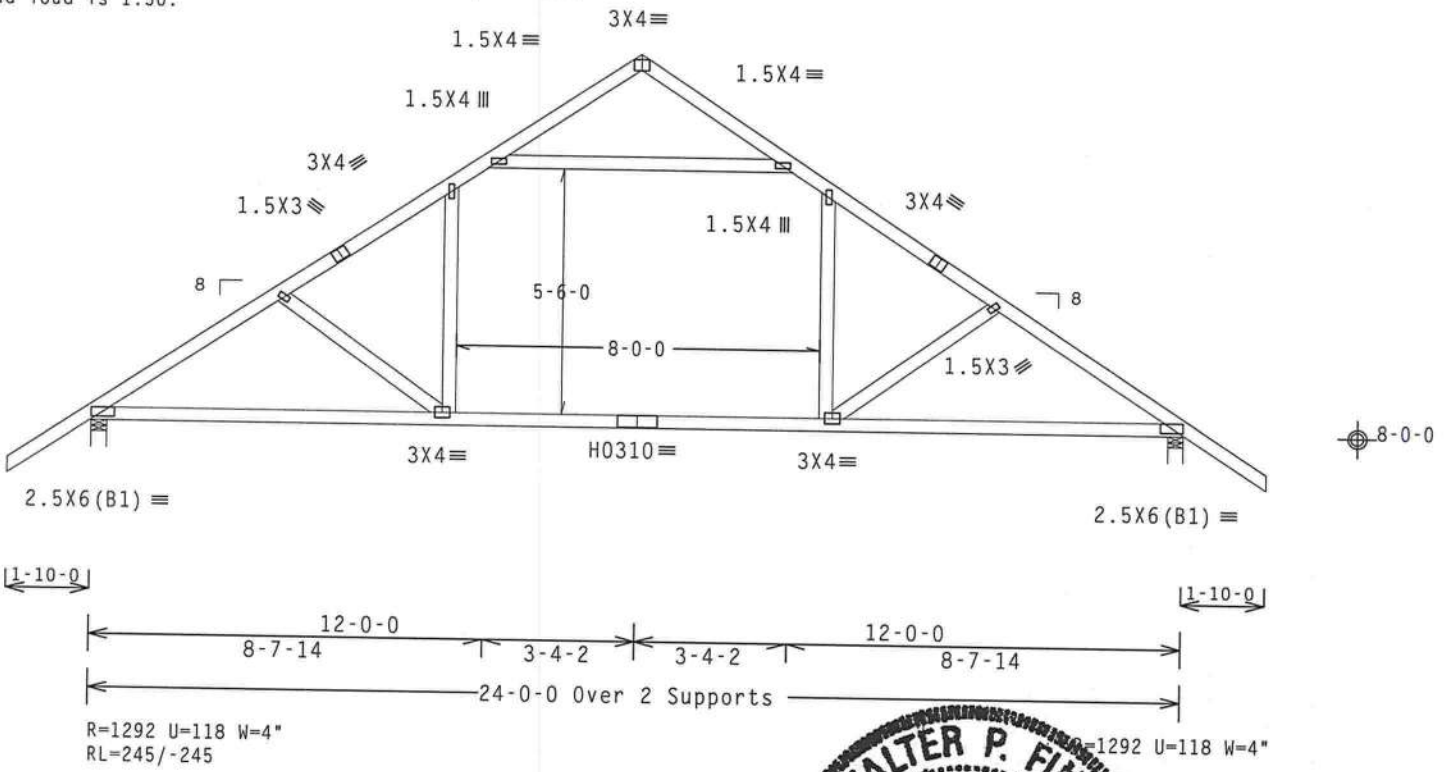
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

This design is based on lumber values in effect prior to June 1, 2012 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

BC attic loading: LL = 10.00 psf; DL = 3.00 psf; from 8-0-0 to 16-0-0.



PLT TYP. 20 Gauge HS,Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/10(0)

10.03.11.0300.30

QTY 16 FL/-/4/-/-/R/-

Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. [illegible] follow the latest edition of BCSI (Building Component Safety Information, by TPI and WTCA) [illegible] practices prior to performing these functions. Installers shall provide temporary bracing [illegible] BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bott[illegible] chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restrain[illegible] shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from th[illegible] any failure to build the trusses in conformance with ANSI/TPI 1, or for handling, shipping, instal[illegible] bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joi[illegible] Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. A seal on drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This job's general notes page: ITW-BCG: www.itwbcg.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

07/16/2012

TC LL	20.0 PSF	REF	R487-- 96991
TC DL	10.0 PSF	DATE	07/16/12
BC DL	10.0 PSF	DRW	HCUSR487 12198001
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD.	40.0 PSF	SEQN-	12768
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UNU487_Z01

(12-146--Fill in later Smith Garage -- Falling Creek, FL - AGE 24' Gable)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP #1::Stack Chord SC2 2x4 SP #1:

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Gable end supports 8" max rake overhang.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

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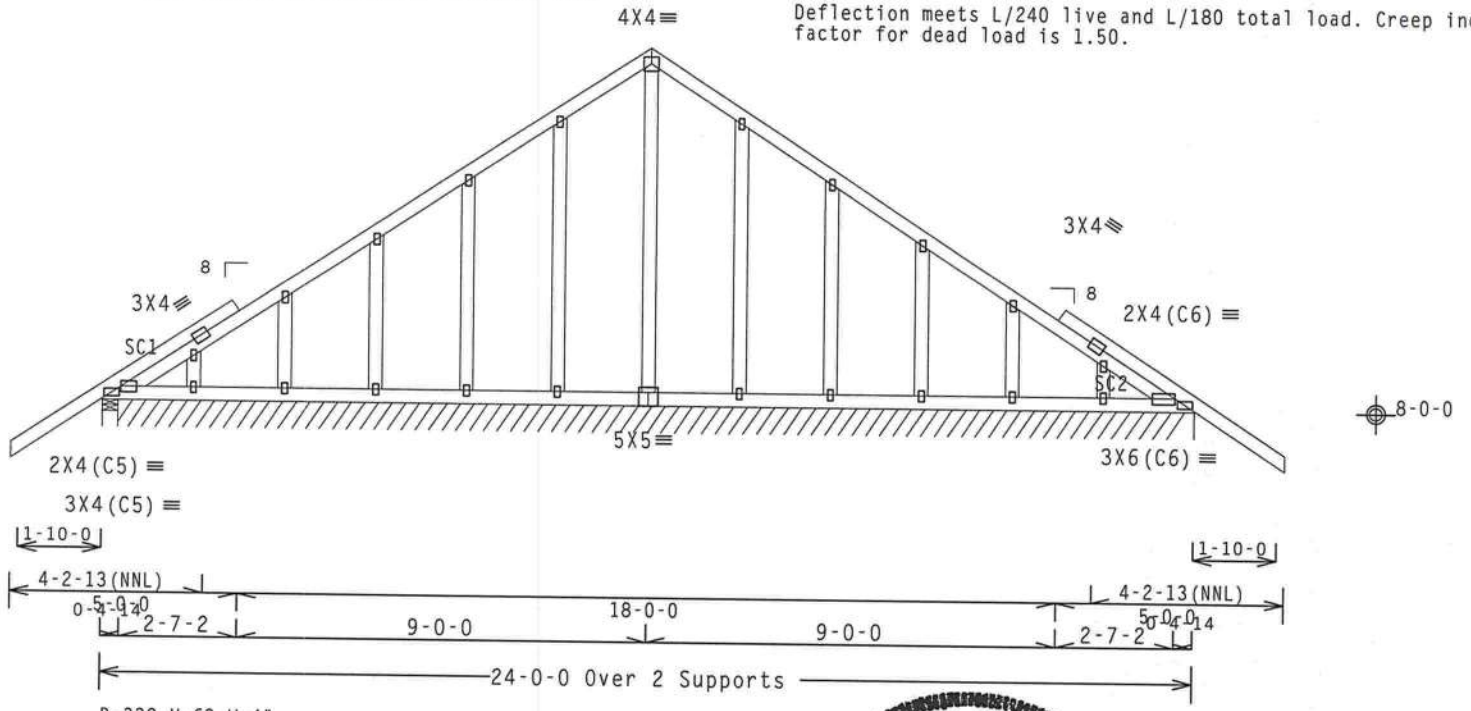
Wind loads and reactions based on MWFRS with additional C&C member design.

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Note: All Plates Are 1.5X3 Except As Shown.

PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/10(0)

10.03 11.0209 20 QTY 2 FL/-/4/-/-/R/- Scale =.25"/Ft.

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07/16/2012

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Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 96992
TC DL	10.0 PSF	DATE	07/16/12
BC DL	10.0 PSF	DRW	HCUSR487 12198002
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD.	40.0 PSF	SEQN-	12779
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UNU487_Z01

Gable Stud Reinforcement Detail

ASCE 7-10: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Or: 100 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 "L" Brace *		(1) 2x4 "L" Brace *		(2) 2x4 "L" Brace **		(1) 2x6 "L" Brace *		(2) 2x6 "L" Brace **	
				Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" o.c.	SPF HF	#1 / #2	4' 10"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 7"	7' 9"	8' 3"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 7"	8' 1"	8' 4"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 7"	8' 1"	8' 4"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	4' 11"	8' 3"	8' 7"	9' 9"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 10"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 7"	6' 11"	7' 4"	9' 3"	9' 10"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 7"	6' 11"	7' 4"	9' 3"	9' 10"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 7"	6' 0"	6' 4"	8' 0"	8' 6"	10' 10"	11' 7"	12' 6"	13' 5"	14' 0"	14' 0"
16" o.c.	SPF HF	#1 / #2	5' 6"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 3"	9' 3"	9' 9"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 3"	9' 3"	9' 7"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 3"	9' 3"	9' 7"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 8"	9' 5"	9' 10"	11' 2"	11' 7"	13' 3"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 6"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 3"	8' 6"	9' 0"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 3"	8' 6"	9' 0"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 3"	7' 4"	7' 10"	9' 9"	10' 5"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
12" o.c.	SPF HF	#1 / #2	6' 1"	10' 4"	10' 8"	12' 2"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	6' 2"	10' 5"	10' 9"	12' 3"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	6' 1"	10' 4"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 9"	9' 9"	10' 5"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 9"	9' 9"	10' 5"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 9"	8' 6"	9' 0"	11' 3"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:

Group A:

Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard

Douglas Fir-Larch	Southern Pine***
#3	#3
Stud	Stud
Standard	Standard

Group B:

Hem-Fir: #1 & Btr, #1

Douglas Fir-Larch	Southern Pine***
#1	#1
#2	#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 35 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

So. Pine lumber design values based on the ALSC January, 2012 rulin

Attach "L" braces with 10d (0.128"x3.0" min) nails.

* For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones.

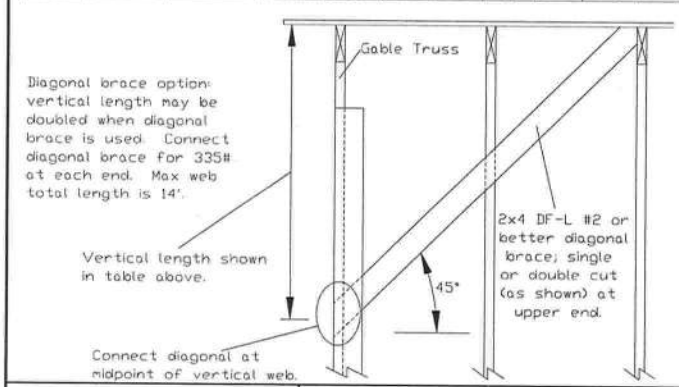
** For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

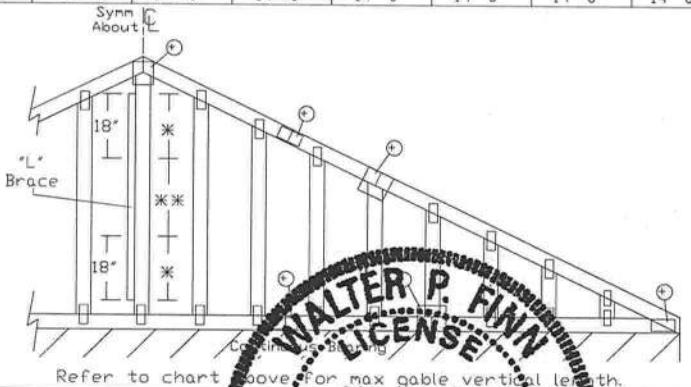
"L" bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0", but less than 11' 6"	2X4
Greather than 11' 6"	2.5X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.





ITW Building Components Group Inc.

Earth City, MO 63045

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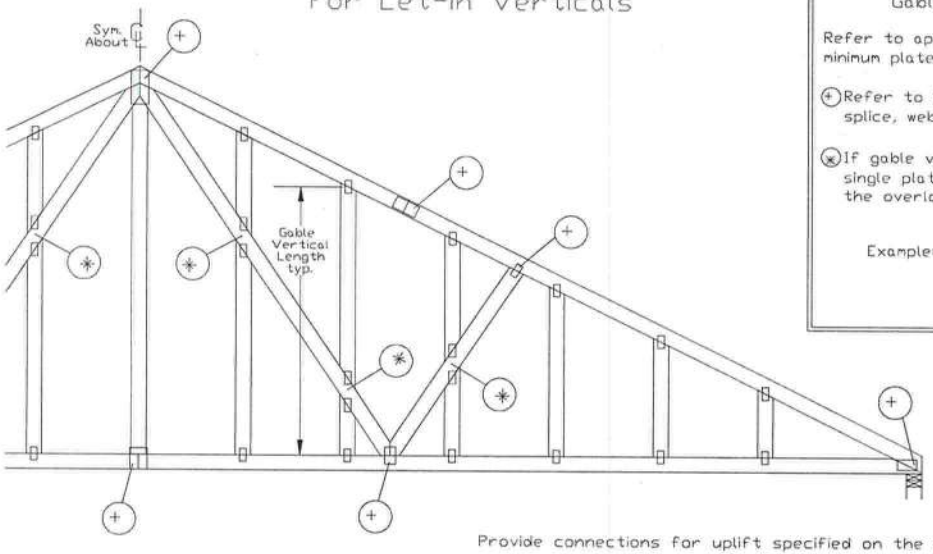
MAX. TOT. L.D. 60 PSF

MAX. SPACING 24.0"

REF ASCE7-10-GAB12015
DATE 2/14/12
DRWG A12015ENC100212

Gable Detail
For Let-in Verticals



Gable Truss Plate Sizes

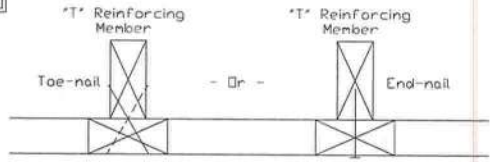
Refer to appropriate ITW gable detail for minimum plate sizes for vertical studs.

⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

⊗ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example: 2X4 2X4 2X8

"T" Reinforcement Attachment Detail

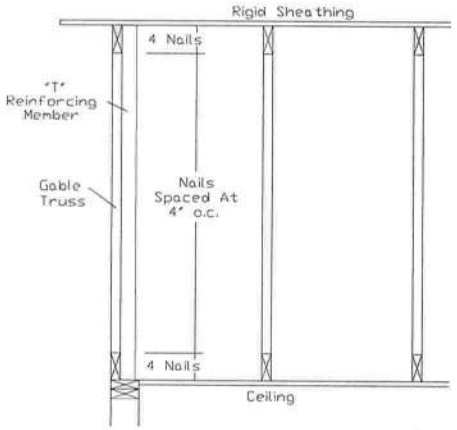


Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with

End Driven Nails:
- 10d Common (0.148"x 3.",min) Nails at 4" o.c. plus
- (4) nails in the top and bottom chords.

Toenailed Nails:
- 10d Common (0.148"x3",min) Toenails at 4" o.c. plus
- (4) toenails in the top and bottom chords.



This detail to be used with the appropriate ITW gable detail for ASCE wind load.

ASCE 7-98 Gable Detail Drawings
A13015980109, A12015980109, A11015980109, A10015980109, A13030980109, A12030980109, A11030980109, A10030980109

ASCE 7-02 Gable Detail Drawings
A13015020109, A12015020109, A11015020109, A10015020109, A14015020109, A13030020109, A12030020109, A11030020109, A10030020109, A14030020109

ASCE 7-05 Gable Detail Drawings
A13015050109, A12015050109, A11015050109, A10015050109, A14015050109, A13030050109, A12030050109, A11030050109, A10030050109, A14030050109

ASCE 7-10 Gable Detail Drawings
A11515ENC100212, A12015ENC100212, A14015ENC100212, A18015ENC100212, A20015ENC100212, A11530ENC100212, A12030ENC100212, A18030ENC100212, A20030ENC100212,

See appropriate ITW gable detail for maximum unreinforced gable vertical length.

To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate ITW gable detail).

Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member.

Web Length Increase w/ "T" Brace

"T" Reinf. Mbr. Size	"T" Increase
2x4	30 %
2x6	20 %

Example:
ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft, Kzt = 1.00
Gable Vertical = 24"o.c. SP #3
"T" Reinforcing Member Size = 2x4
"T" Brace Increase (From Above) = 30% = 1.30
(1) 2x4 "L" Brace Length = 8' 7"
Maximum "T" Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"

07/16/2012

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Earth City, MO 63045

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MAX. TOT. LD.	60 PSF
DUR. FAC.	ANY
MAX. SPACING	24.0"

REF	LET-IN VERT
DATE	2/16/12
DRWG	GBLLETIN0212

ASCE 7-10: 120 mph, 30' Mean Height, Closed, Exposure C
Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph, 30ft. Mean Hgt, ASCE 7-10, Enclosed, Exp C, or
100 mph, 30ft. Mean Hgt, ASCE 7-10, Enclosed, Exp D, or
100 mph, 30ft. Mean Hgt, ASCE 7-10, Part. Enclosed, Exp C,
Kzt = 1.00, Wind TC DL=5.0 psf, Wind BC DL=5.0 psf.

Lateral chord bracing requirements
Top: Continuous roof sheathing
Bot: Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail for lumber, plates, and other information not shown on this detail.

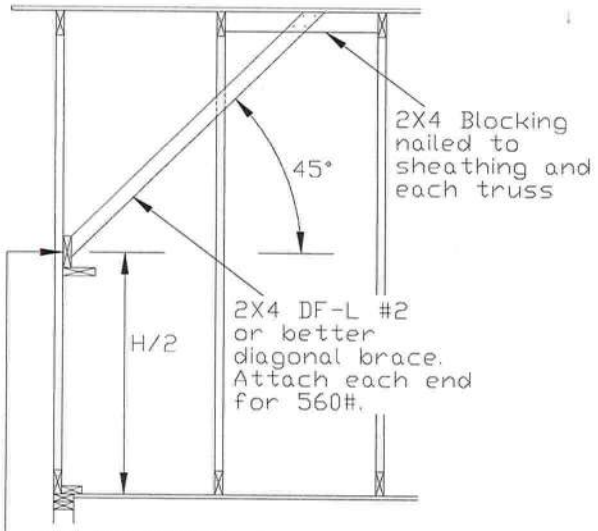
Nails: 10d box or gun (0.128"x3",min) nails.

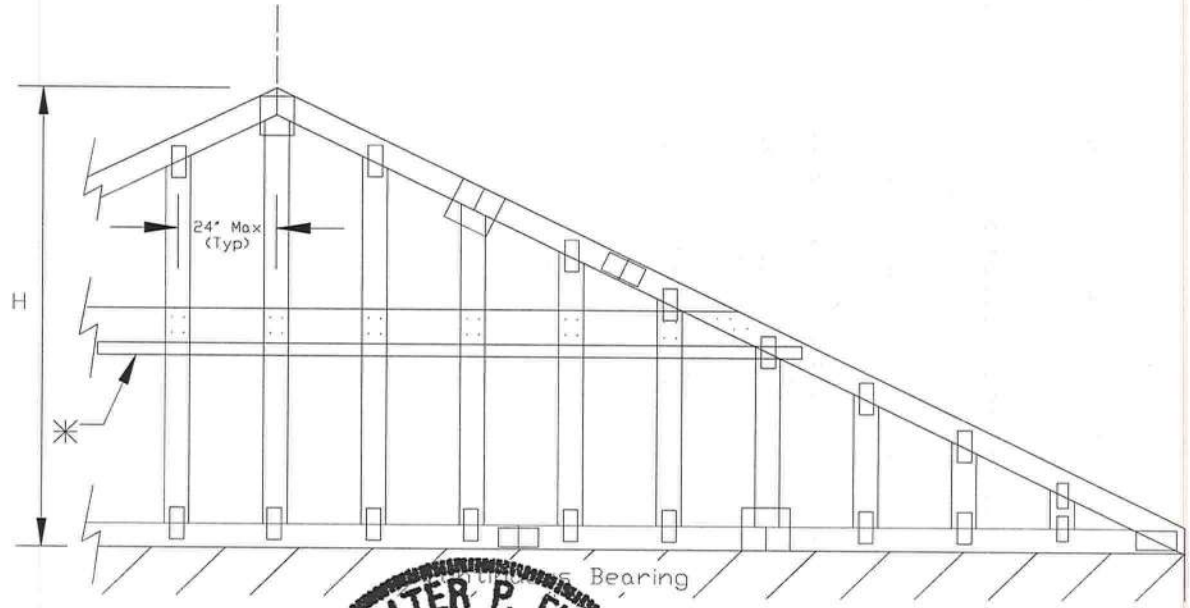
H Less than 4'6" - no stud bracing required

H Greater than 4'6" to 7'6" in length
provide a 2x6 stiffback at mid-height and brace stiffback to roof diaphragm every 6'0" (see detail below or refer to DRWG A12030ENC10).

H Greater than 7'6" to 12'0" max:
provide a 2x6 stiffback at mid-height and brace to roof diaphragm every 4'0" (see detail below or refer to DRWG A12030ENC10).

✳ Optional 2x L-reinforcement attached to stiffback with 10d box or gun (0.128" x 3", min.) nails @ 6" o.c.





WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
07/16/2012

ITW Building Components Group Inc.
Earth City, MO 63045

WARNING! READ AND FOLLOW ALL NOTES ON THIS DRAWING!
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see this job's general notes page and these web sites:
ITWBCG: www.itwbcg.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.org; ICC: www.iccsafe.or

MAX. TOT. LD. 60 PSF

MAX. SPACING

REF	GE WHALER
DATE	2/14/12
DRWG	GABRST100212