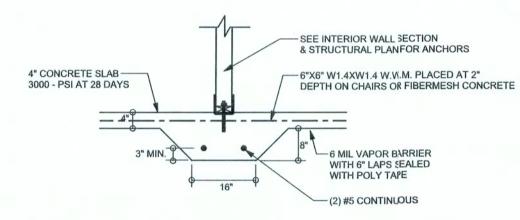


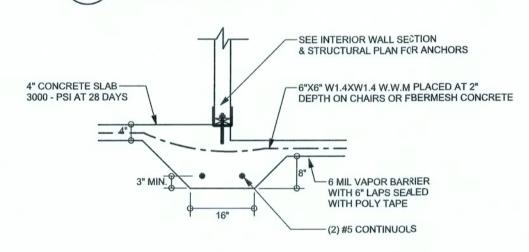
STEM WALL FOOTING — PORCH POST SEE STRUCTURAL PLAN SEE STRUCTURAL PLAN FOR CAST IN 4" CONCRETE SLAB -3000 - PSI AT 28 DAYS PLACE ANCHORS -(1) #5 CONT, IN HDR. BLOCK BOND BEAM @ SLAB EDGE INTERSECTION W/ STEMWALL HOUSE SLAB #5 STEEL DCWEL WITH 24" HOOK BENT INTO SLAB AND 6" HOOK IN FOOTING AT EACH CORNER AND AT 96" O.C. 6"X6" W1.4XW1.4 W.W.M. PLACED AT 2" — DEPTH ON CHAIRS OR FIBERMESH -8X8X16, RUNNING BOND, CMU STEM WALL, MIN 2, MAX 5 COURSES 6 MIL VAPOR BARRIER -WITH 6" LAPS SEALED WITH POLY TAPE TERMITE TREATED FILL, -EACH LIFT COMPACTED TO MIN. 95% MOD. PROCTOR (2) #5 REBAR CONTINOUS GRADE 40

F12\ ALT. STEM WALL PORCH FOOTING S-2 SCALE: 1/2" = 1'-0"

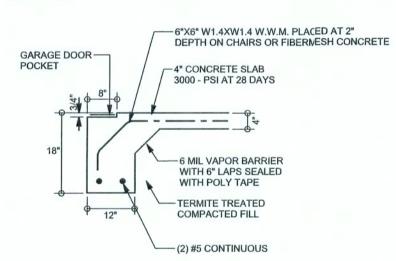
- 20" X 10" POURED CONCRETE STRIP FOOTING (MINIMUM 3000-PSI AT 28 DAYS)



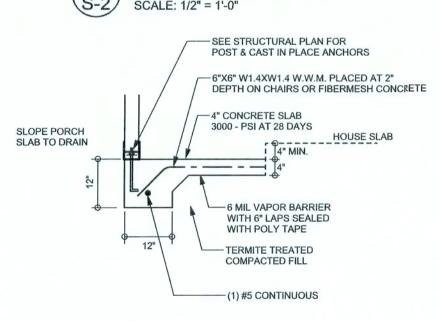
F2 INTERIOR BEARING FOOTING



F3 INTERIOR BEARING STEP FOOTING S-2 SCALE: 1/2" = 1'-0"



GARAGE DOOR FOOTING S-2 SCALE: 1/2" = 1'-0"



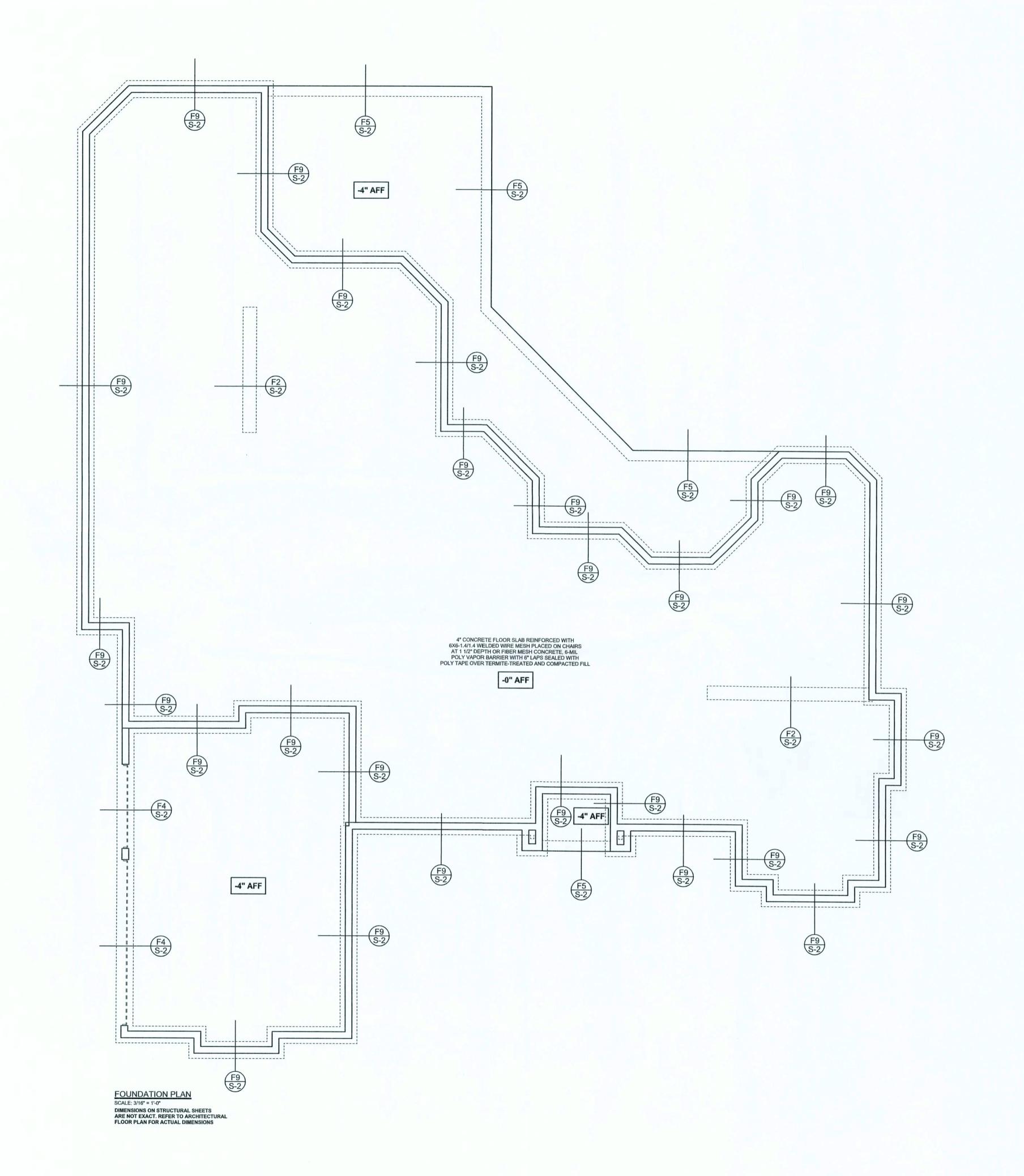
PORCH FOOTING

SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)		VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)			
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



REVISIONS

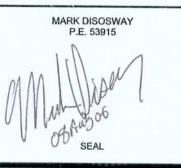
SOFTPIAN

PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419 DIMENSIONS:

Stated dimensions supercede scale dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification. COPYRIGHTS AND PROPERTY RIGHTS: Mark Disosway, P.E. hereby expressly reserve its common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any

form or manner without first the express written permission and consent of Mark Disosway. CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge

LIMITATION: This design is valid for one building, at specified location.



Glenwood King Construction

Kevin Gray Residence

ADDRESS: Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: August 09, 2006

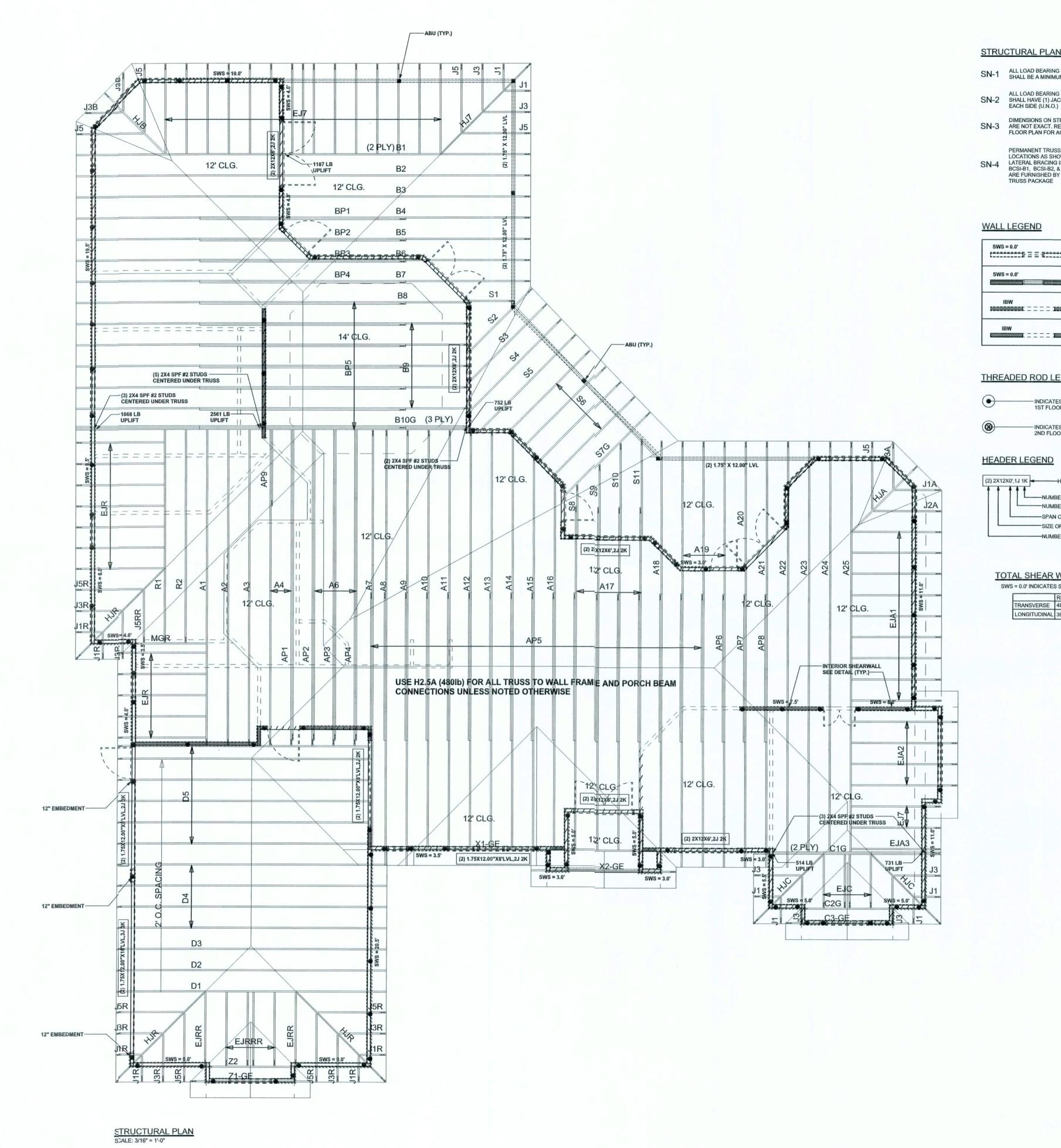
DRAWN BY: STRUCTURAL BY David Disosway

FINALS DATE: 08 / Aug / 06

JOB NUMBER: 608087 DRAWING NUMBER

S-2

OF 3 SHEETS



STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

SN-3
DIMENSIONS ON STRUCTURAL SHEETS
ARE NOT EXACT. REFER TO ARCHITECTURAL
FLOOR PLAN FOR ACTUAL DIMENSIONS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3, BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED

SM2 = 0.0,	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR
IBW \$55555553	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

THREADED ROD LEGEND

--- INDICATES LOCATION OF: 1ST FLOOR 1/2" A307 ALL THREADED ROD

- INDICATES LOCATION OF: 2ND FLOOR 1/2" A307 ALL THREADED ROD

HEADER LEGEND

(2) 2X12X0',1J 1K HEADER/BEAM CALL-OUT (U.N.O.) NUMBER OF KING STUDS (FULL LENGTH) ---NUMBER OF JACK STUDS (UNDER HEADER) SPAN OF HEADER SIZE OF HEADER MATERIAL ---NUMBER OF PLIES IN HEADER

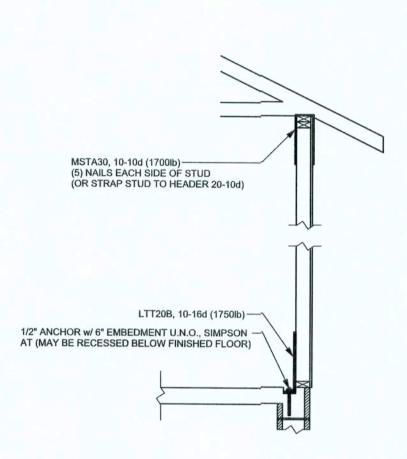
TOTAL SHEAR WALL SEGMENTS SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

REQUIRED ACTUAL
TRANSVERSE 48.5' 108.5'
LONGITUDINAL 39.0' 73.0'



REVISIONS





ALTERNATE WALL TIE CONNECTION WHERE THREADED ROD CANNOT BE PLACED IN WALL SCALE: 1/2" = 1'-0"

> VINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419 DIMENSIONS: tated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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examined this plan, and that the applicable portions of the plan, relating to wind engineerin comply with section R301.2.1, florida building code residential 2004, to the best of my

LIMITATION: This design is valid for one

building, at specified location. MARK DISOSWAY P.E. 53915

> Glenwood King Construction

> > Kevin Gray Residence

ADDRESS: Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

David Disosway

PRINTED DATE: August 09, 2006 STRUCTURAL BY: DRAWN BY:

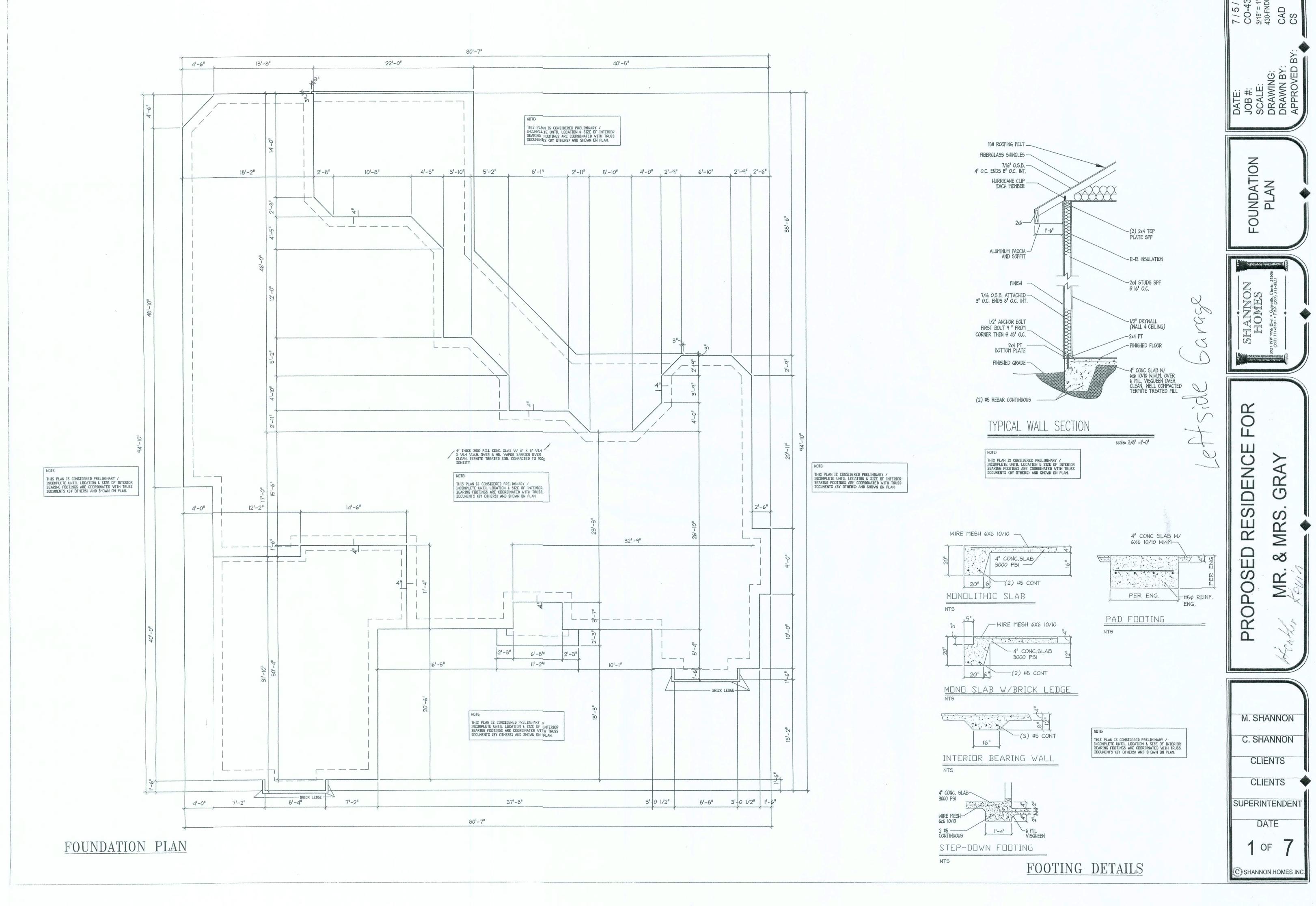
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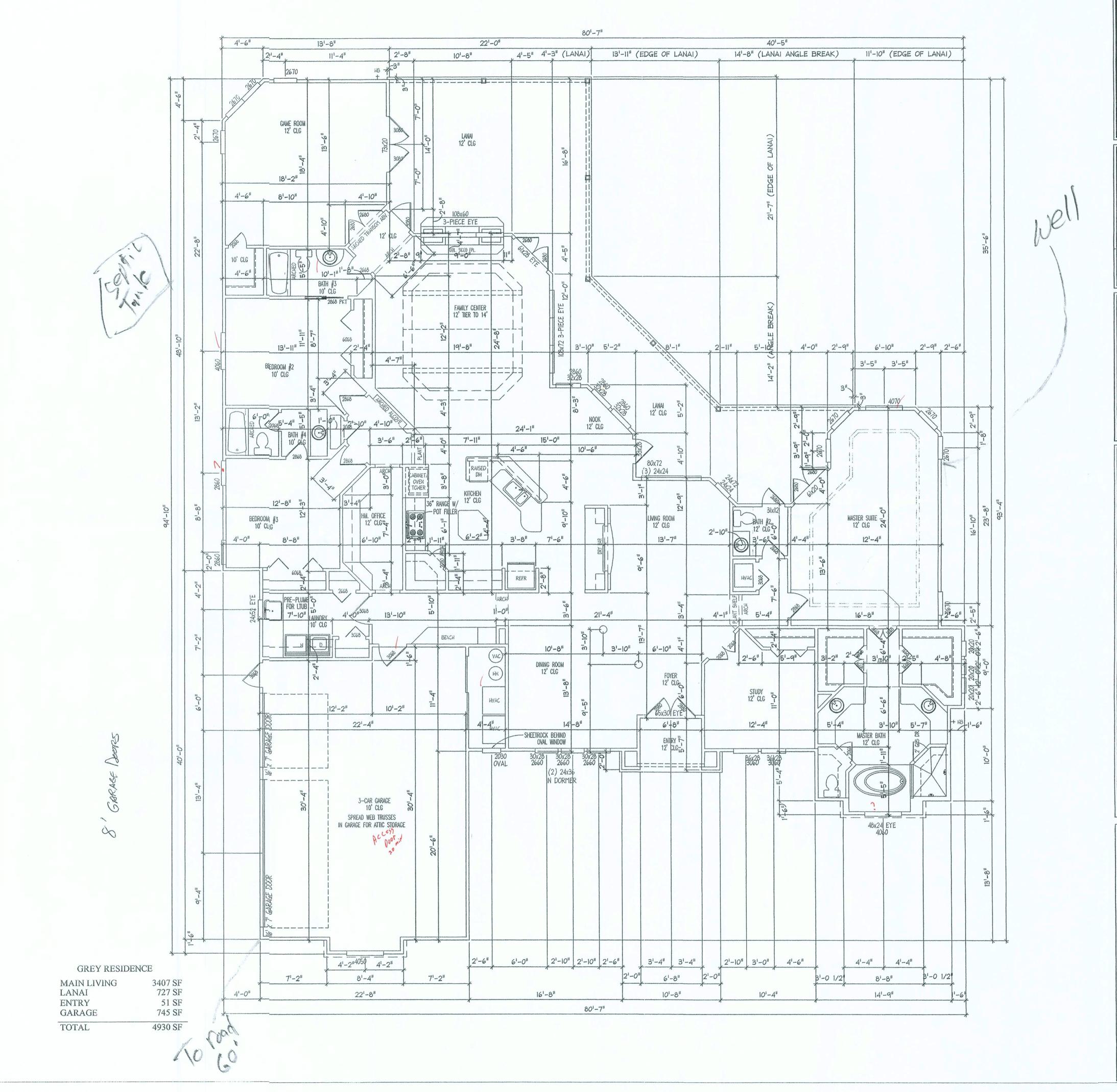
08 / Aug / 06 JOB NUMBER: 608087

> DRAWING NUMBER **S-3**

> > OF 3 SHEETS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY ANDERSON TRUSS JOB #6-268





MR. & MRS. GRAY

M. SHANNON

C. SHANNON

CLIENTS

CLIENTS

SUPERINTENDENT
DATE

2 of 7



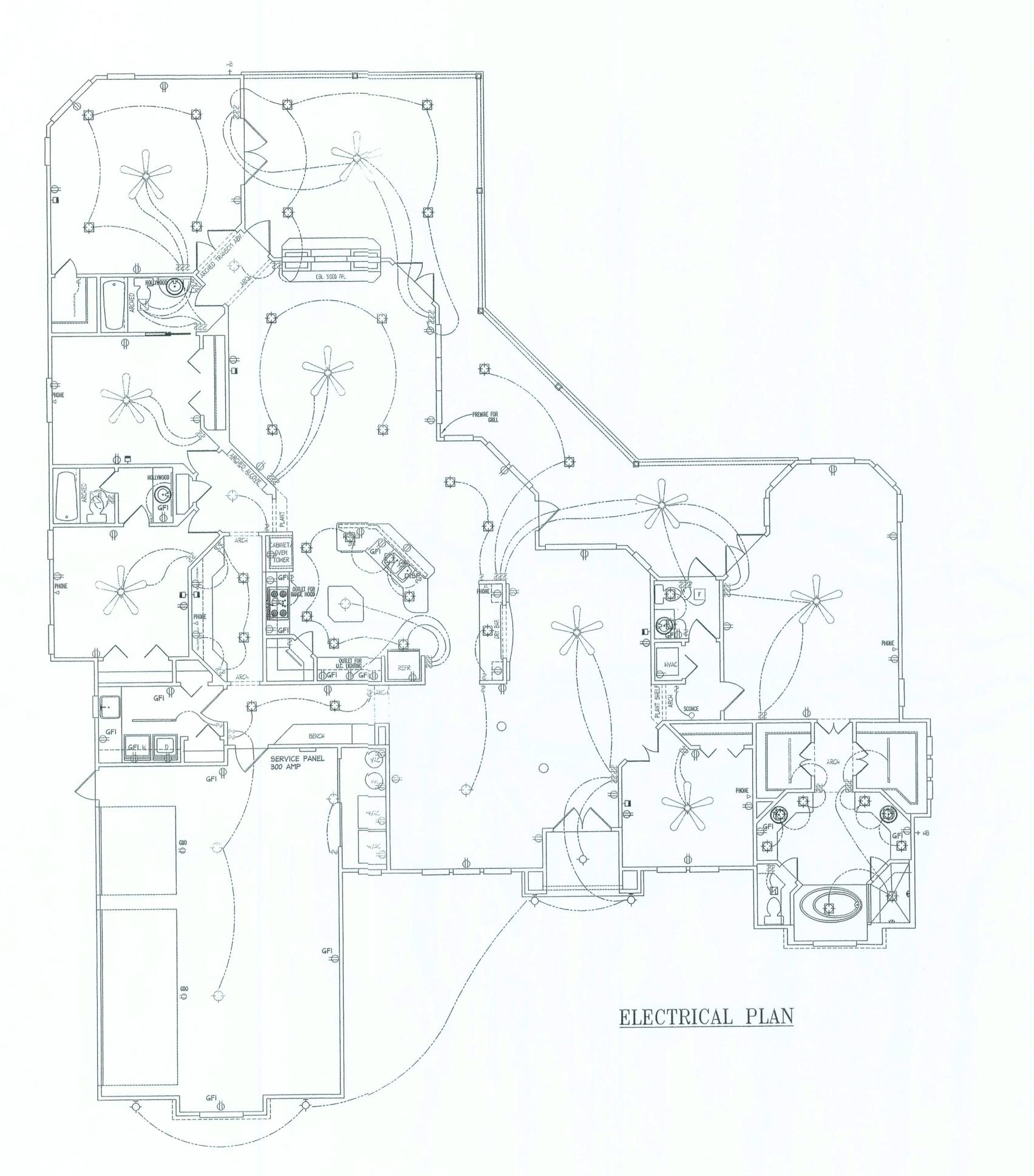
GRAY & MRS. M N

M. SHANNON C. SHANNON CLIENTS

CLIENTS SUPERINTENDENT

DATE

3 of 7



ELECTRICAL LEGEND SINGLE OUTLET) INSTALLED SWITCH OUTLET CEILING FAN WE WEATHER-PROOF OUTLET O INSTALLED CEILING € 230-VOLT OUTLET FAN W/LIGHT FIXT GFI OUTLET 42" AFF FLUORESCENT 220V GFI OUTLET 42" AFF

LIGHT FIXTURE ____ RECESSED FLUOR LIGHT FIXTURE HOLLYWOOD STRIP HOLLYWOOD STRIP

EXHAUST FAN

FOUR POLE SWITCH POLE SWITCH WIDIMMER

- LOW-VOLTAGE SWITCH RECESSED LIGHT FIXTURE

SINGLE POLE SWITCH

THREE POLE SWITCH

SURFACE MOUNT FIXTURE

O-HWALL MOUNT FIXTURE EYEBALL LIGHT

O UNDERCOUNTER REC CAN -DAPUSH BUTTON FOR ELEC GARAGE DOOR OPENER

SECURITY LIGHT LANDSCAPING GROUND

FAN/LIGHT COMBO

SURFACE MOUNT
SD SMOKE DETECTOR

LIGHTING 220 AMP PANEL BOX PHONE - TELEVISION

-ON DOORBELL CHIMES DOORBELL CHIMES

ELECTRICAL NOTES

OI. ELECTRICAL PLAN FOR DESIGN PURPOSES ONLY. 02. ALL ELECTRICAL OUTLETS IN THE BATHROOMS, KITCHEN, \$ UTILITIES NEAR SINKS \$ ANY OTHER PLUMBING ARE TO BE GROUND-FAULT CIRCUIT-INTERUPTER TYPE OR AS SPECIFIED BY LOCAL CODE; ALSO IN GARAGE AND EXTERIOR OUTLETS. 03. INSTALL LIGHT IN ATTIC WITH SWITCH AT ATTIC ACCESS UNLESS NOTED OTHERWISE.

04. GARAGE OUTLETS INSTALLED AT 42" ABOVE FINISHED FLOOR OR AS SPECIFIED BY LOCAL CODE.

05. ALL ELECTRICAL LOCATION AND QUANTITY TO BE VERIFIED \$ COORDINATED WITH OWNER PRIOR TO INSTALLATION. 06. COORDINATE POOL LIGHTS AND LANDSCAPE LIGHTS \$ SWITCH LOCATIONS WITH OWNER. 07. PREWIRE FOR STEREO - COORDINATE WITH OWNER (VERIFY W/BUILDER'S SPEC'S).

08. SECURITY ALARM SYSTEM - COORDINATE WITH OWNER VERIFY W/BUILDER'S SPECS).

09. OPTIONAL: PREWIRE INTERCOM AND SPEAKERS SYSTEM - COORDINATE WITH OWNER. 10. PROVIDE REQUIRED ELECTRICAL FOR POOL \$ POOL DECK. II. PROVIDE DISCONNECT SWITCH FOR AIR HANDLER IN SPACE REQ'D & LOCATE ON SITE W/ BUILDER & A/C CONTRACTOR. 12. PROVIDE OPTIONAL ELECTRIC TO MASTER BATH TUB (OR AS REQ'D FOR WHIRLPOOL HOOK UP PER OWNER.

13. PROVIDE REQUIRED BRACING IN ATTIC FOR HANGING CHANDALIERS. 14. PROVIDE & INSTALL SMOKE DETECTORS AS REQUIRED BY LOCAL CODE.

ELECTRICAL PLAN

RESIDENC MRS

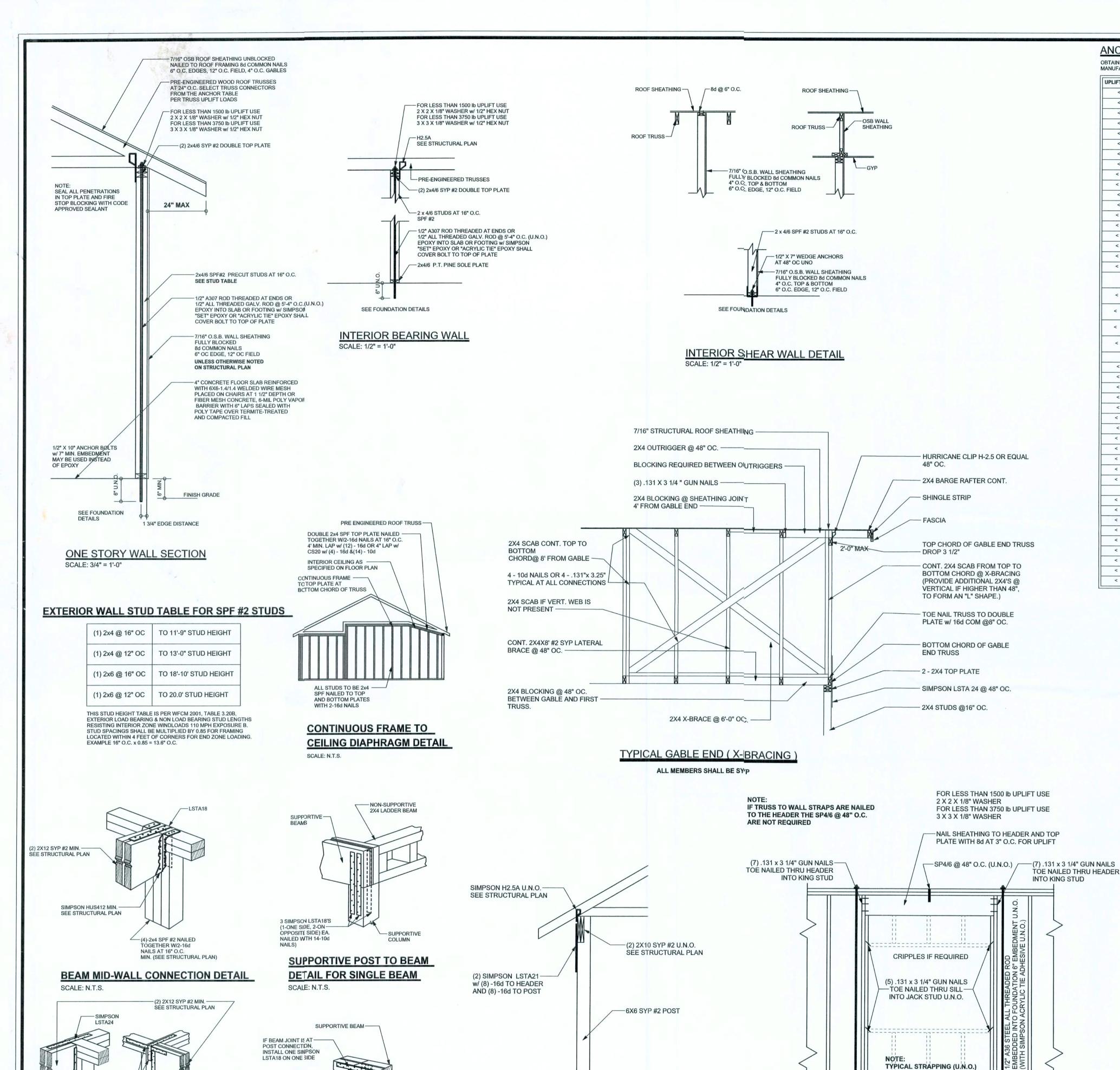
M. SHANNON C. SHANNON

> CLIENTS CLIENTS

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SIMPSON ABU POST BASE

w/ (12) - 16d & 5/8" x 10"

—SEE FOOTING DETAILS

TYPICAL PORCH POST DETAIL

ANCHOR BOLT

4-SIMPSON LSTA18 ----

└─3-1/2" P.T.

SUPPORTIVE CENTER POST TO BEAM DETAIL

(2-ONE SIDE, 2-ON

OTHER SIDE)

- SIMPSON HUS412 MIN

SCALE: N.T.S.

SEE STRUCTURAL PLAN

BEAM MAY BE ATTACHED IN

EITHER METHOD SHOWN ABOVE

BEAM CORNER CONNECTION. DETAIL

ANCHOR TABLE OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

PLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	10.0
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED RO 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB

GRADE & SPECIES TABLE

SYP #2

SYP #2

SYP #2

24F-V3 SP

MICROLAM

PARALAM

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT

DOOR WIDTH | 3/8" x 4" LAG | 16d | (2) ROWS OF STAGGER | .131 x 3 1/4" GN

18" O.C. 4" O.C.

5" O.C.

3" O.C.

5" O.C.

4" O.C.

3" O.C.

ATTACH GARAGE DOOR BUCK TO STUD PACK AT

SCREWS w/ 1" WASHER LAG SCREWS MAY BE

COUNTERSUNK, HORIZONTAL JAMBS DO NOT

GN PER TABLE BELOW:

11' - 15'

16' - 18'

2x6SYP #2 DOOR BUCK -

SCALE: N.T.S.

BRACKET -

(SEE STRUCTURAL PLAN)

(1) 2X6 SPF #2 SILL UP TO 7'-6" U.N.O.

(2) 2X4 SPF #2 SILL UP TO 7'-8" U.N.O.

(1) 2X4 SPF #2 SILL UP TO 5'-1" U.N.O.

(FOR: 120 MPH, 10'-0" WALL HEIGHT U.N.O.)

TYPICAL 1 STORY HEADER STRAPING DETAIL

RANSFER LOAD. CENTER LAG SCREWS OR

STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4"

24" O.C.

16" O.C.

GARAGE DOOR BUCK INSTALLATION DETAIL

EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG

IMBERSTRAND | 1700

Fb (psi) E (10⁶ psi)

1.6

1.6

1.6

1.8

1.7

2.0

1200

1050

975

2400

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET

GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" × 6" W1.4 × W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB \$HEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" \times 2" \times 9/64"; WITH 5/ δ " BOLTS TO BE 3" \times 3" \times 9/64"; WITH 3/4" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 7/8" BOLTS TO BE 3" \times 3" \times 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS		NER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH AR ART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.	E
REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL	CONFIRM SITE CONDITIONS BACKFILL HEIGHT, WIND SP	S, FOUNDATION BEARING CAPACITY, GRADE AND PEED AND DEBRIS ZONE, AND FLOOD ZONE.	_
BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL	PROVIDE MATERIALS AND CREQUIREMENTS FOR THE S	CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 STATED WIND VELOCITY AND DESIGN PRESSURES.	
DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL	BELIEVE THE PLAN OMITS A	A CONTINUOUS LOAD PATH CONNECTION, CALL	
BEAUTIONS.	DESIGN, PLACEMENT PLANS	S, TEMPORARY AND PERMANENT BRACING DETAILS,	

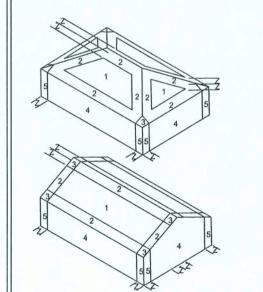
ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN ROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRE LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

ON.	ICLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; AN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% DPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS
	LDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE
BUI	LDING IS NOT IN THE WIND-BORNE DEBRIS REGION
1.)	BASIC WIND SPEED = 110 MPH
2.)	WIND EXPOSURE = B
3.)	WIND IMPORTANCE FACTOR = 1.0
4.)	BUILDING CATEGORY = II
5.)	ROOF ANGLE = 10-45 DEGREES
6.)	MEAN ROOF HEIGHT = <30 FT

7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING) 8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2)) Zone Effective Wind Area (ft2)



STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

SOIL BEARING CAPACITY 1000PSF

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

	1	U	100		
1	19.9	-21.B	18.1	-18.1	
2	19.9	-25.5	18.1	-21.8	
2 O'hg		-40.6		-40.6	
3	19.9	-25.5	18.1	-21.8	
3 O'hg		-68.3	9	-42.4	
4	21.8	-23.6	18.5	-20.4	
5	21.8	-29.1	18.5	-22.6	
	st Cas 5, 10				
8x7 Gar	age D	19.5	-22.9		
16x7 Garage Door			18.5	-21.0	

	55				
DESIGN	LOADS				
FLOOR	R 40 PSF (ALL OTHER DWELLING ROOMS)				_
	30 PSF (SLEEPING ROOMS)				_

30 PSF (ATTICS WITH STORAGE) 10 PSF (ATTICS WITHOUT STORAGE, <3:12) ROOF 20 PSF (FLAT OR <4:12) 16 PSF (4:12 TO <12:12) 12 PSF (12:12 AND GREATER)

FINALS DATE: 08 / Aug / 06 JOB NUMBER:

> **S-1** OF 3 SHEETS

608087 DRAWING NUMBER

David Disosway

building, at specified location. P.E. 53915 Glenwood King Construction Kevin Gray Residence ADDRESS: Columbia County, Florida Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871 PRINTED DATE: August 09, 2006 STRUCTURAL BY DRAWN BY:

NDLOAD ENGINEER: Mark Disosway

PE No.53915, POB 868, Lake City, FL

tated dimensions supercede scaled

limensions. Refer all questions to

o not proceed without clarification

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CERTIFICATION: I hereby certify that I have

amined this plan, and that the applicable portions of the plan, relating to wind engine

comply with section R301.2.1, florida building

ode residential 2004, to the best of my

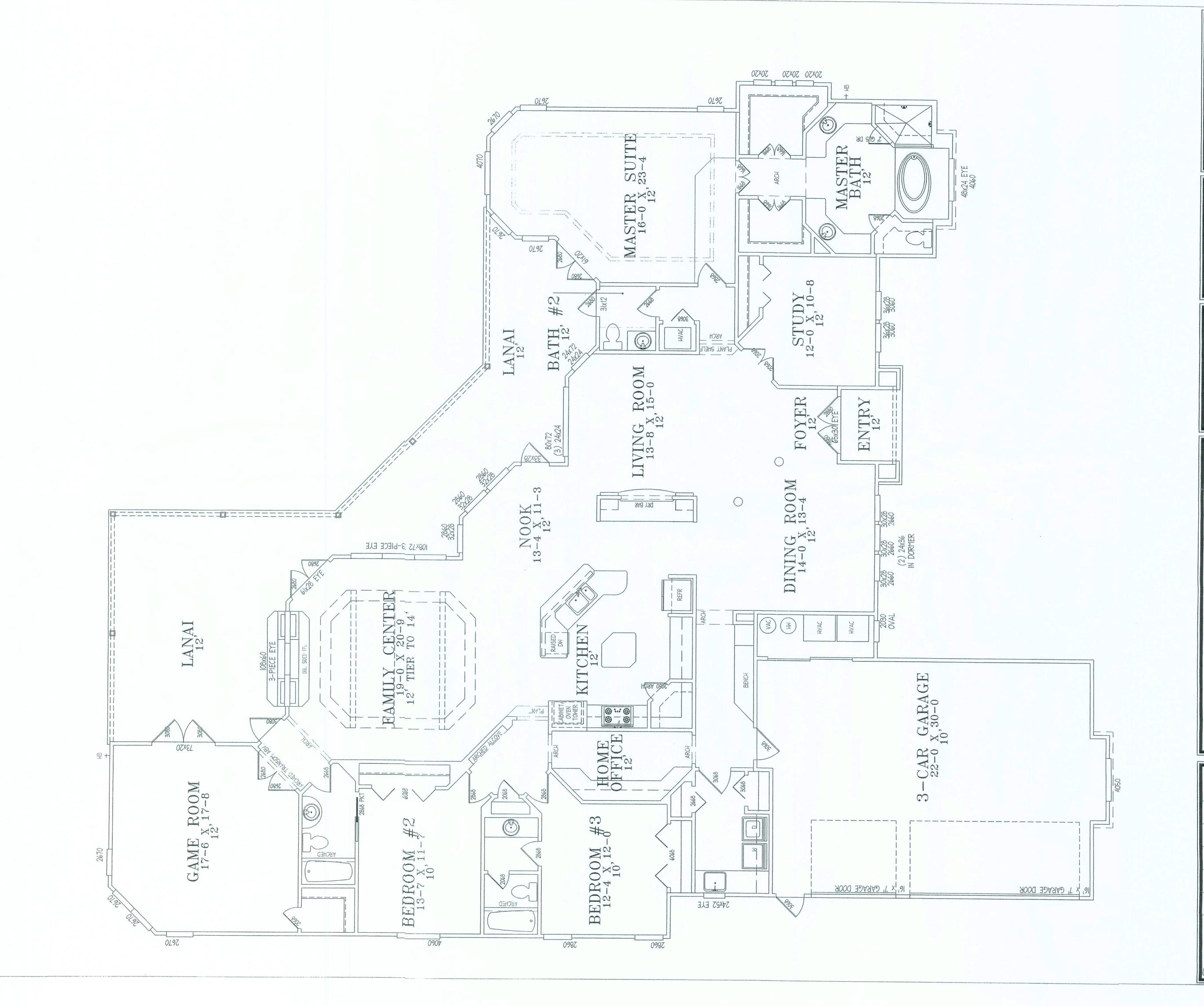
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32056, 386-754-5419



RESIDENCE MR. & MRS.

PROPOSED

M. SHANNON C. SHANNON CLIENTS

CLIENTS SUPERINTENDENT

DATE 5 OF 7