GENERAL:

THE DRAWINGS ARE INTENDED TO SHOW THE GENERAL ARRANGEMENT, DESIGN AND EXTENT OF THE WORK AND ARE PARTIALLY DIAGRAMMATIC. THEY ARE NOT INTENDED TO BE SCALED FOR ROUGH-IN MEASUREMENTS, OR TO SERVE AS SHOP DRAWINGS OR PORTIONS THEREOF.

2. ALL DETAILS AND SECTIONS SHOWN ON THE DRAWINGS ARE INTENDED TO BE TYPICAL AND SHALL BE CONSTRUED TO APPLY TO ANY SIMILAR SITUATION ELSEWHERE ON THE PROJECT, EXCEPT WHERE A DIFFERENT DETAIL OR SECTION IS SHOWN.

. PRIOR TO START OF CONSTRUCTION, THE CONTRACTOR AND ALL THE SUBCONTRACTORS SHALL VERIFY ALL GRADES, LINES, LEVELS, DIMENSIONS AND COORDINATE EXISTING CONDITIONS AT THE JOB SITE WITH THE PLANS AND SPECIFICATIONS. THEY SHALL REPORT ANY INCONSISTENCIES OR ERRORS IN THE ABOVE TO THE ARCHITECT/ENGINEER BEFORE COMMENCING WORK. THE CONTRACTOR AND HIS SUBCONTRACTORS SHALL LAY OUT THEIR WORK FROM ESTABLISHED REFERENCE POINTS AND BE RESPONSIBLE FOR ALL LINES, ELEVATIONS AND MEASUREMENTS IN CONNECTION WITH THEIR WORK.

4. IF ANY ERRORS OR OMISSIONS APPEAR IN THE DRAWINGS, GENERAL NOTES OR OTHER DOCUMENTS, THE CONTRACTOR SHALL NOTIFY THE ARCHITECT IN WRITING OF SUCH OMISSION OR ERROR PRIOR TO PROCEEDING WITH ANY WORK WHICH APPEARS IN QUESTION. IN THE EVENT OF THE CONTRACTOR'S FAILING TO GIVE SUCH AN ADVANCED NOTICE, HE SHALL BE HELD RESPONSIBLE FOR THE RESULTS OF ANY SUCH ERRORS OR OMISSIONS AND THE COST OF RECTIFYING THE SAME.

5. THE CONTRACTOR SHALL USE THE STRUCTURAL DRAWINGS AND SPECIFICATIONS TOGETHER WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL AND OTHER TRADE DRAWINGS AND SHOP DRAWINGS, TO LOCATE DEPRESSED SLABS, SLOPES, DRAINS, OUTLETS, RECESSES, OPENINGS, BOLT SETTING, SLEEVES, DIMENSIONS, ETC. NOTIFY ARCHITECT/ENGINEER, IN WRITING, OF ANY POTENTIAL CONFLICTS BEFORE PROCEEDING WITH THE

SHOP DRAWINGS AND DELEGATED ENGINEERING:

I. ALL SHOP DRAWINGS SHALL BE SUBMITTED FOR ARCHITECT'S REVIEW ONLY AFTER THEY HAVE BEEN THOROUGHLY REVIEWED BY THE CONTRACTOR FOR CONSTRUCTION METHODS, DIMENSIONS AND OTHER TRADE REQUIREMENTS. AND STAMPED WITH THE CONTRACTOR'S APPROVAL STAMP. THE ARCHITECT ASSUMES NO RESPONSIBILITY FOR DIMENSIONS. QUANTITIES. ENGINEERING DESIGN BY DELEGATED ENGINEERS, ERRORS OR OMISSIONS AS A RESULT OF REVIEWING ANY SHOP DRAWINGS. ANY ERRORS OR OMISSIONS MUST BE MADE GOOD BY THE CONTRACTOR, IRRESPECTIVE OF RECEIPT, CHECKING OR REVIEW OF DRAWINGS BY THE ENGINEER AND EVEN THOUGH WORK IS DONE IN ACCORDANCE WITH SUCH DRAWINGS.

2. BEFORE STRUCTURAL INSPECTIONS CAN BE MADE ON A PORTION OF THE STRUCTURE, ALL RELATED SHOP DRAWINGS, DELEGATED ENGINEERING, PRODUCT APPROVAL, MANUFACTURER'S DATA AND OTHER RELATED INFORMATION, MUST BE REVIEWED AND ACCEPTED BY THE ARCHITECT-OF-RECORD AND APPROVED BY THE BUILDING DEPARTMENT.

3. SHOP DRAWINGS SHALL CONTAIN ALL INFORMATION SHOWN ON THE STRUCTURAL PLANS (RELATED TO THE DELEGATED DESIGN) INCLUDING ALL DESIGN LOADS, IN ADDITION TO THE INFORMATION REQUIRED BY THE DELEGATED ENGINEER'S DESIGN.

4. ARCHITECT WILL REVIEW ALL SUBMITTED SHOP DRAWINGS, PREPARED AND SIGNED AND SEALED BY THE CONTRACTOR'S DELEGATED ENGINEER, ONLY FOR GENERAL COMPLIANCE WITH THE DESIGN INTENT, REQUIRED LOADING AND COORDINATION WITH THE STRUCTURAL DESIGN.

. CONTRACTOR SHALL SUBMIT TO THE ARCHITECT TWO SETS OF BLUE PRINTS OF THE STRUCTURAL SHOP DRAWINGS FOR ARCHITECT REVIEW, BEFORE STARTING FABRICATION. THE ARCHITECT WILL RETURN ONE MARKED UP AND STAMPED COPY TO THE CONTRACTOR. THE MARKED-UP COPY SHALL BE USED TO MAKE THE PRINTS REQUIRED FOR SHOP DRAWING DISTRIBUTION.

CONSTRUCTION MEANS AND METHODS:

THE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCE OR PROCEDURES, SAFETY PRECAUTIONS, SHORES, RESHORES, LATERAL BRACING AND PROGRAMS IN CONNECTION WITH THE PROJECT, ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. OUR SERVICES DO NOT GUARANTEE NOR ASSURE LIABILITY FOR THE JOB SAFETY, TEMPORARY SHORING AND BRACING AND THE PERFORMANCE OF THE CONTRACTOR.

2. THE CONTRACTOR IS RESPONSIBLE AND SHALL COMPLY WITH THE SAFETY REQUIREMENTS OF THE 2004 FLORIDA BUILDING CODE AND APPLICABLE LOCAL, STATE AND FEDERAL LAWS.

3. PROVIDE ALL SHORING, BRACING AND SHEETING AS REQUIRED FOR SAFETY, STRUCTURAL STABILITY AND FOR THE PROPER EXECUTION OF THE WORK. REMOVE WHEN WORK IS COMPLETED.

4. PROVIDE AND MAINTAIN GUARD LIGHTS AT ALL BARRICADES, RAILINGS, OBSTRUCTIONS IN THE STREETS, ROADS OR SIDEWALKS AND ALL TRENCHES OR PITS ADJACENT TO PUBLIC WALKS OR ROADS.

5. AT ALL TIMES, PROVIDE PROTECTION AGAINST WEATHER (RAIN, WIND, STORMS OR THE SUN), SO AS TO MAINTAIN ALL WORK, MATERIALS, APPARATUS AND FIXTURES FREE FROM INJURY OR DAMAGE.

6. AT THE END OF THE DAYS WORK, COVER ALL WORK LIKELY TO BE DAMAGED. ANY WORK DAMAGED BY FAILURE TO PROVIDE PROTECTION SHALL BE REMOVED AND REPLACED WITH NEW WORK AT THE CONTRACTOR'S

7. THE CONTRACTOR SHALL PAY FOR ALL DAMAGES TO ADJACENT STRUCTURES, SIDEWALKS AND TO STREETS OR OTHER PUBLIC PROPERTY OR PUBLIC UTILITIES.

STRUCTURAL DESIGN CRITERIA:

THE DESIGN COMPLIES WITH THE REQUIREMENTS OF THE 2007 FLORIDA BUILDING CODE - SECTION 1609 AND OTHER REFERENCED CODES AND SPECIFICATIONS. ALL CODES AND SPECIFICATIONS SHALL BE LATEST EDITION

AT TIME OF PERMIT.

RESIDENTIAL

BALCONIES

2. WIND LOAD CRITERIA: BASED ON ANSI/ASCE 7-03. BASIC WIND VELOCITY IIO MPH,

ROOF DESIGN LOADS: SUPERIMPOSED DEAD LOADS: SUPERIMPOSED LIVE LOADS: 20 PSF 4. FLOOR DESIGN LOADS: SUPERIMPOSED DEAD LOADS: 25 PSF SUPERIMPOSED LIVE LOADS:

. 60 PSF 5. WIND NET UPLIFT: ARE AS INDICATED ON PLANS

FOUNDATIONS: SPREAD FOOTINGS)

FOUNDATIC ARE DESIGNED TO BEAR ON WELL COMPACTED GRADE OR CLE FILL OF AN ALLOWABLE BEARING CAPACITY OF 1.000 PSF MINMUM. FOR QUIRED SOIL BEARING CAPASITIES GREATER THAN 1,000 PSF, A CERTIP TESTING LABORATORY SHALL BE ENGAGED BY THE OWNER TO VER THAT THE REQUIRED BEARING CAPACITY WAS OBTAINED. SAID SOIL CAPTY SHALL BE CERTIFIED AND TESTED BY A FLORIDA REGISTERED FIDATION ENGINEER, PRIOR TO CASTING OF CONCRETE IN

. NATURAL ADE (OR FILL) BELOW FOOTINGS SHALL BE COMPACTED TO % MODIFIED PROCTOR (ASTM D-1557).

3. TOP OF WL FOOTINGS TO BE AT THE SAME ELEVATION AS TOP OF COLUMN PAD FTINGS. STEP WALL FOOTING FROM HIGHER COLUMN FOOTING TO THE LOWERIE (AS DETAILED ON THE PLANS).

4. BOTTOM CALL FOOTINGS TO BE A MINIMUM 1'-6" BELOW THE TOP OF CONCRETE SLADN GRADE (UNLESS OTHERWISE NOTED) OR MINIMUM 1'-0" BELOW FINISHERADE, WHICHEVER IS LOWER. IN THE EVENT THAT THE SLAB STEPS ON EAGIDE OF THE FOOTING, THE FOOTING SHALL BE 1'-6" BELOW TOP

REINFORCI IN THE CONTINUOUS WALL FOOTINGS (MONOLITHIC AND NON-MONCHIC) SHALL BE SPLICED 40 BAR DIAMETERS MINIMUM AND SHALL EXTENDINTINUOUSLY THRU ALL FOOTING PADS.

ALL LONGDINAL REBARS IN THE CONTINUOUS WALL FOOTINGS, SHALL BE CONUED AT BENTS AND CORNERS BY BENDING THE REBARS 48 BAR DIAMETERAROUND THE CORNERS OR ADDING MATCHING CORNER BARS, EXTENDING 48 R-DIAMETERS INTO FOOTING EACH SIDE OF CORNER OR BENT.

7. ALL FOOTS SHALL BE 12" MINIMUM THICKNESS.

CONCRETE SLA ON GRADE:

ALL INTER! AND EXTERIOR SLABS AND WALKWAYS AS SHOWN ON THE STRUCRAL OR ARCHITECTURAL PLANS, SHALL BE FOUR INCHES THICK MINIMUMEINFORCED WITH 6 X 6 - WI.4 X WI.4 WELDED WIRE FABRIC (UNLESS OTHEISE NOTED).

ALL SLABIN GRADE TO BE CONSTRUCTED IN ACCORDANCE WITH LATEST A.C.I GUIDE FOR CONCRETE FLOOR AND SLAB CONSTRUCTION" (A.C.I.

JOINTS SIL BE PROVIDED IN ALL INTERIOR SLABS ON GRADE AT LOC. INDICATEDN THE PLANS DIVIDING THE SLAB INTO SQUARE PANELS NOT TO EXCEED 20 X 2T. IN SIZE. CAST SLAB IN LONG ALTERNATE STRIPS. PROVIDE A CONTRACTIOIOINT BETWEEN EACH STRIP. SEE PLAN FOR SAW-CUT, CONTRACTION () ISOLATION JOINT DETAILS.

4. PROVIDE SI-CUT JOINTS AT ALL SIDEWALKS AT A MAXIMUM SPACING OF FINFEET ON CENTERS AND ISOLATION JOINTS AT 20 FEET O.C.

5. FILL MATEAL SHALL BE PLACED IN LIFTS NOT EXCEEDING 12" AND COMPACTETO 98 % MODIFIED PROCTOR (ASTM D-1557) WITHIN A DISTANCE OF EET BEYOND ALL FOOTING EDGES. TAKE AT LEAST ONE DENSITY TEST R EACH 1,600 SQ.FT. OF AREA AND 12" BELOW SURFACE. SEND RESULTS OF TITEST TO OWNER, ARCHITECT AND ENGINEER.

CONCRETE AND ENFORCING:

CONCRETE SIGN AND REINFORCEMENT IN ACCORDANCE WITH "BUILDING CODEEQUIREMENTS FOR REINFORCED CONCRETE" (A.C.I. 318 -LATEST EDITIO AND WITH "DETAILS AND DETAILING OF CONCRETE REINFORCEMENT- (A.C.I. 315 - LATEST EDITION).

ALL CONCTE WORK IN ACCORDANCE WITH "SPECIFICATIONS FOR STRUCTURAL CRETE FOR BUILDING" (A.C.I. 301 - LATEST EDITION). PRODUCTION OF ONCRETE, DELIVERY, PLACING AND CURING TO BE IN ACCORDANCE W "HOT WEATHER CONCRETING" (A.C.I. 305R - LATEST

3. ALL CONCTE TO BE REGULAR WEIGHT WITH A DESIGN STRENGTH OF 3,000 P.S.I. T 28 DAYS. MAXIMUM SLUMP 5".

4. ALL REINFCING TO BE NEW BILLET STEEL CONFORMING TO THE LATEST A.S.T.IA-615 GRADE 60, FABRICATED IN ACCORDANCE WITH C.R.S.I. MANUAL OF STDARD PRACTICE AND PLACED IN ACCORDANCE WITH A.C.I. 315 AND C.R.S.I. MJAL OF STANDARD PRACTICE.

5. CONCRETEDVER UNLESS OTHERWISE DETAILED ON DRAWINGS:

FOOTINGS:

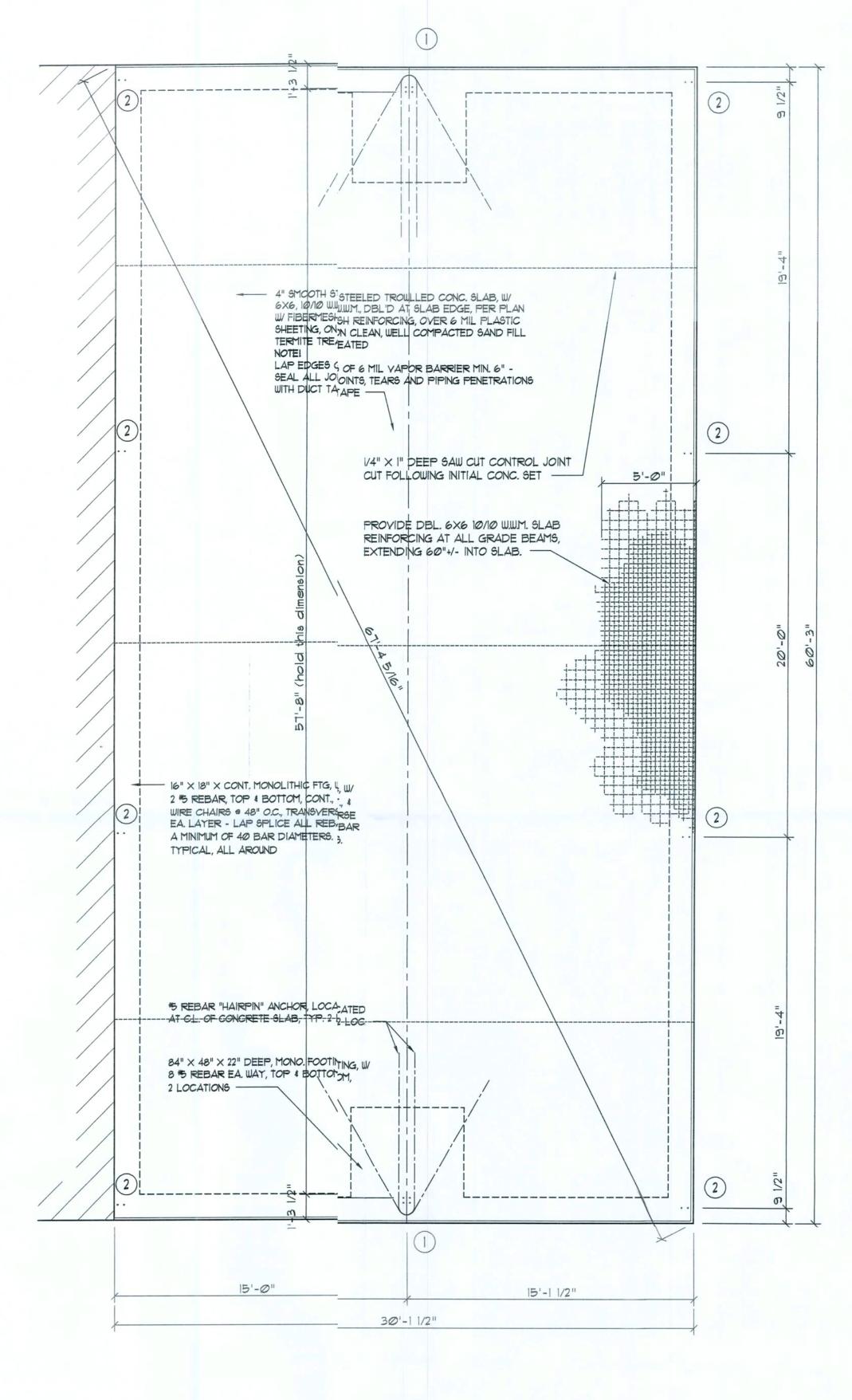
(TOP \$ SIDES) 2" CENTERED W/SLAB

SLABS ON GRA

6. BEAM REINCEMENT: LAPPED 36 BAR DIAMETER OR MINIMUM 18 INCHES. BOTTIBARS SPLICED ONLY AT SUPPORTS, TOP BARS SPLICED ONLY AT MID-IN. ALL TOP BARS HOOKED AT NONCONTINUOUS EDGES (U.O.N.). ALL OKS TO BE STANDARD 90 DEGREE HOOKS AS REQUIRED

ADDED REORCEMENT: PROVIDE ADDITIONAL CORNER BARS BENT 36 INCHE1INIMUM EACH WAY AT "L" AND "T" CORNERS IN OUTER FACES OF ALL BEAMS) MATCH ALL HORIZONTAL BAR (TOP, BOTTOM AND INTERMEDIATE BARS).

8. SEE PLAN R MINIMUM SIZE CONCRETE TIE BEAM REQUIREMENTS.



Foundation PLAN

SCALE: 1/4" = 1'-0"

THE ANCHOR BOLT DIAMETERS AND DEVELOPED LENGTHS INDICATED IN THIS DRAWING WERE DETERMININED USING SHEAR FRICTION THEORY AS DESCRIBED IN AISC DESIGN GUIDE NO.T, SECTION 9.2, ASSUMING AN ANCHOR BOLT MATERIAL OF ASTM A301 OR A36. THE COMBINED FORCES ACTING AT THE BASE OF THE STEEL FRAME RESULTING IN A VERTICAL REACTION ACTING UPON THE FOUNDATION WERE DEVELOPED AS FOLLOWS:

ANCHOR BOLT / FOUNDATION SIZING:

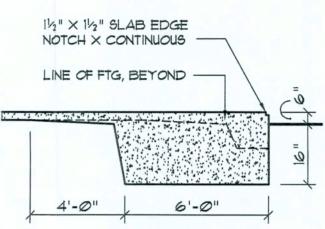
T = Td + Tsf

T = TOTAL TENSILE FORCE PER BOLT Td = TENSILE FORCE PER BOLT DUE TO DIRECTLY APPLIED LOAD = PN Tef = TENSILE FORCE PER BOLT DUE TO SHEAR FRICTION = Y / (n X u)

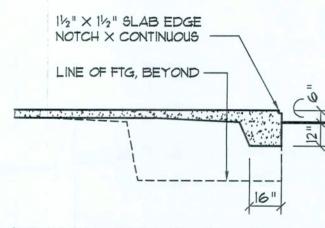
P = P = TOTAL UPLIFT TO BE RESISTED BY ANCHOR BOLT GROUP V = V = TOTAL SHEAR FORCE TO BE RESISTED BY ANCHOR BOLT GROUP

u = u = COEFFICIENT OF FRICTION (TAKEN AS 0.7 FOR UNGROUTED BASE

n = n = NUMBER OF ANCHOR BOLTS PLATES OR 0.9 FOR GROUTED BASE PLATES)



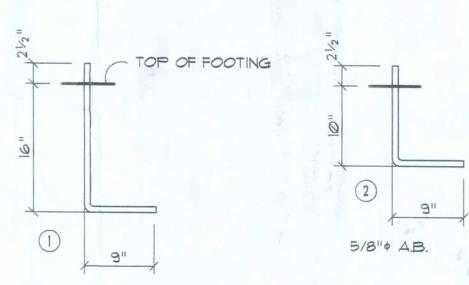
FOOTING @ MAIN FRAME 3CALE: 1/4" = 1'-0"



REFER TO THE METAL BUILDING SHOP DRAWINGS PREPARED BY BSX - BUILDING SYSTEMS EXPRESS, INC., FOR EXACT LOCATION OF ALL EMBEDDED ANCHOR BOLTS.

ADDED FILL SHALL BE APPLIED IN 12" LIFTS -EA. LIFT SHALL BE CONPACTED TO 98% DRY COMPACTION PER THE "MODIFIED PROCTOR" METHOD.

NOTE! THE DESIGN WIND SPEED FOR THIS PROJECT IS 100 MPH PER 2004 FBC 1606 AND LOCAL JURISDICTION REQUIREMENTS



3/4" \$ A.B.

ALL ANCHOR BOLTS ARE ASTM GRADE A36 STEEL ROD, THREADED 3", OR GRADE A30T, BLACK, AND FREE FROM RUST AND SCALE

Anchor Bolt DETAILS

SCALE: |" = |'-0"

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DRAWN

Sin OR OC OC

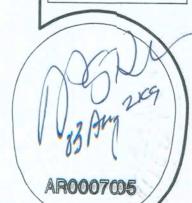
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DATE: 01 AUG 2009

2K942

SHEET:

1 Œ 1



GENERAL NOTES

- ALL LATEST EDITIONS USED.
- CODES AND AUTHORITIES HAVING JURISDICTION OVER THE WORK.
- COMMENCING CONSTRUCTION.
- DETAILS FOUND WITHIN THESE DRAWINGS SHALL BE ASSUMED TO BE TYPICAL DETAILS FOR THIS JOB ONLY. DETAILS SHALL GOVERN CONSTRUCTION FOR THIS JOB UNLESS NOTED OTHERWISE ON THE PLANS.
- SUBSURFACE SOIL CONDITIONS WERE NOT AVAILABLE AT THE TIME OF THIS DESIGN. THE OWNER SHALL PROVIDE TO THE CONTRACTOR A REPORT OF THE SUBSURFACE CONDITIONS. 4. ALL-THREASHALL BE DRILLED AND EPOXIED INTO FOOTING WITH SIMPSON SET EPOXY SOIL PREPARATIONS NOTED IN SAID REPORT SHALL BE FOLLOWED UNLESS MORE STRINGENT DESIGN IS SPECIFIED WITHIN THESE PLANS.

CONCRETE NOTES

- MINIMUM CONCRETE COMPRESSIVE STRENGTH OF ALL CONCRETE IS 3000 PSI AT 28 DAYS.
- 6X6 W1.4 X W1.4 WWM TO BE PLACED IN THE CENTER OF THE SLAB. WWM SHALL BE LAPPED 8". THE USE OF FIBERMESH SHALL BE ALLOWED IN LIEU OF WWM. MINIMUM FIBER LENGTH = 1/2".
- SLAB THICKNESS IS 4", UNLESS NOTED OTHERWISE ON THE PLANS. SLAB SHOULD BE POURED OVER A 6 MIL. VAPOR BARRIER AND THE SOIL SHOULD BE TREATED WITH TERMITE POISON PRIOR TO POURING.
- THE FILL BELOW THE FOUNDATION SHOULD BE FREE OF DEBRIS, ORGANIC MATERIAL, COHESIVE SOILS OR ANY OTHER DELETERIOUS MATERIAL. SOIL MUST BE COMPACTED TO 95% MODIFIED PROCTOR MAXIMUM DRY DENSITY FOR TWO FEET BELOW THE BOTTOM OF THE FOOTING.
- ALL REINFORCEMENT SHALL BE GRADE 40 DEFORMED BARS CONFORMING TO ASTM-A615.
- VERTICAL AND HORIZONTAL REINFORCEMENT WILL BE LAPPED FOR 36 BAR DIAMETERS OR 24 WHICHEVER IS GREATER.
- . CORNER REINFORCEMENT SHALL BE LAPPED 25".
- REINFORCEMENT SHALL HAVE THE FOLLOWING COVER REQUIREMENTS; 8.1. 3" FOR CONCRETE CAST AND PERMANENTLY EXPOSED TO EARTH
- 8.2. 2" FOR CONCRETE EXPOSED TO EARTH AND WEATHER
- 8.3. 1 1/2" FOR CONCRETE NOT EXPOSED TO WEATHER OR EARTH FOR THE PRIMARY
- ONLY DIMENSIONS FOUND ON THE FOUNDATION PLAN BY GEOFF GARTNER, PE, SHOULD BE USED FOR FOUNDATION CONSTRUCTION. IF DIMENSIONS CAN NOT BE DETERMINED FROM FOUNDATION PLAN, CONTACT THE ENGINEER OF RECORD.
- 10. STEMWALL TO BE A MAXIMUM OF SIX (6) COURSES TALL. CONTACT ENGINEER OF RECORD IF STEMWALL WILL EXCEED SIX (6) COURSES IN HEIGHT.
- 11. WHERE THREADED RODS ARE EMBEDDED 12" INTO STEMWALLS, THE TOP TWO COURSES OF STEMWALL MUST BE FILLED.
- DESIGN OF WOOD COMPONENTS IN THIS STRUCTURE IS BASED ON THE 2007 FLORIDA BUILDING CODE, RESIDENTIAL EDITION AND THE NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION.
- DESIGN LOADING FOR THIS STRUCTURE IS FOUND IN THE LOAD TABLE ON THIS SHEET.
- ALL FRAMING ANCHORS SHOWN ON PLANS ARE SIMPSON. ALTERNATE CONNECTORS ARE ACCEPTABLE PROVIDED EQUAL OR GREATER CAPACITIES ARE ACHIEVED. CONTACT ENGINEER OF RECORD IF EQUAL CAPACITIES ARE NOT APPARENT.
- 4. ALL WOOD DIRECTLY EXPOSED TO CONCRETE, MASONRY OR SOIL SHALL BE PRESSURE
- 5. ALL WOOD DIRECTLY EXPOSED TO WEATHER SHALL BE PRESSURE TREATED.
- NAILS OR CONNECTORS EXPOSED TO WEATHER SHALL BE GALVANIZED.
- 7.1. ALL MEMBER SIZES GIVEN IN THE DRAWINGS ARE NOMINAL DIMENSIONS 7.2. WHERE POSTS ARE CALLED OUT, HEADERS SHALL BEAR FULLY ON POSTS.
- 7.3. ALL BEAMS AND JOISTS NOT BEARING ON SUPPORTING MEMBERS SHALL BE FRAMED WITH SIMPSON STRONG-TIE JOIST HANGERS OR EQUAL PER APPROVAL OF THE ENGINEER OF RECORD. THE JOIST HANGERS SHALL BE NAILED WITH NAILS MEETING THE DIAMETER AND
- 8. ALL NAILS SHALL BE COMMON NAILS, UNLESS OTHERWISE NOTED. NAIL SIZES ARE DEFINED
- 8d = 0.131" x 2-1/2" $10d = 0.148" \times 3"$ 12d = 0.148" x 3-1/4"

16d = 0.162" x 3-1/2"

- WHERE FRAMING DETAILS SHOW FOOTINGS, SEE FOOTING DETAILS ON THE FOUNDATION PLAN AND/OR THE FOOTING DETAILS SHEET.
- 10. CONVENTIONAL FRAMING LUMBER IS 2x No.2 SYP UNLESS NOTED OTHERWISE.

LOAD TABLE

| LIVE LOAD: DEAD LOAD: | 20.0 PSF 7.0 PSF | | | |
|--|----------------------------------|--|--|--|
| CEILING: LIVE LOAD: LIVE LOAD-STORAGE: DEAD LOAD: | 10.0 PSF 30.0 PSF 5.0 PSF | | | |
| FLOOR: LIVE LOAD: DEAD LOAD: DECK LIVE LOAD: | 40.0 PSF 10.0 PSF 50.0 PSF | | | |
| WIND LOADS: ASCE7-02, 120 MPH WIND, FLORIDA BUILDING CODE, RESIDENTIAL EDITION AND EXISTING BUILDING EDITION | | | | |
| EXPOSURE: IMPORTANCE: BUILDING CATEGORY: ENCLOSED BUILDING: | C 1.0 II | | | |
| INTERIOR PRESSURE COEFFICIENT: ROOF PITCH: | 0.18 MATCH EXISTING | | | |
| COMPONENT | AND CLADDII | | | |

COMPONENT AND CLADDING DESIGN PRESSURES

| EFFECTIVE AREA, SF | END ZONES, PSF | | INTERIOR ZONES, PSF | |
|-----------------------|----------------|-----|------------------------|-----|
| 0-20 | 35 | -45 | 35 | -38 |
| 20-50 | 33 | -41 | 33 | -36 |
| 50-100 | 31 | -39 | 31 | -34 |
| 100-200 | 29 | -35 | 29 | -33 |

ALL-THEAD NOTES

- CODES USED: 2007 FLORIDA BUILDING CODE, RESIDENTIAL EDITION, ACI, NDS, APA AND ASCE-7. 1. ALL-THREACONNECTORS TO BE 1/2" DIAMETER BARS PLACED AT 4' O.C. IN ALL EXTERIOR BEARING W.LS, 4' O.C. IN ALL INTERIOR WALLS, UNLESS NOTED OTHERWISE ON PLANS.
- 2. ALL DESIGN, CONSTRUCTION AND MATERIALS SHALL BE IN ACCORDANCE WITH APPLICABLE 2. ALL-THREAUSED IN SHEARWALL AND HIGH UPLIFT TRUSS HOLD-DOWN APPLICATIONS MAY BE COUNTED APART OF THE DESIGNATED O.C. SPACING.
- . CONTRACTOR SHALL VERIFY DIMENSIONS AND CONDITIONS AT THE JOB SITE PRIOR TO 3. PLACE ALL-IREAD RODS WITHIN 6" TO 10" OF EACH CORNER AND AT EACH INTERSECTION OF BEARING W.LS. RECOMMENDED INSTALLATION SEQUENCE IS:
 - A. PLACE JEARWALL RODS (SPECIFIED ON PLANS) B. PLACE GH UPLIFT RODS (SPECIFIED ON PLANS)
 - C. PLACE (RNER RODS D. PLACE IL OTHER RODS.
 - SYSTEM, OAPPROVED EQUIVALENT. ALL-THREAD EMBEDMENTS FOR SHEAR WALLS AND HIGH UPLIFT TRUS CONNECTIONS ARE SPECIFIED IN THEIR RESPECTIVE DETAILS.
 - WHERE NE(SSARY, ALL-THREAD COUPLERS SHALL BE RATED NOT LESS THAN 3200 LBS.
 - MINIMUM EXEDMENT OF ALL-THREAD SHALL BE:
 - 6" TYPIC, WALLS, MONOLITHIC FOOTING 7" - EXTERR WALLS, STEMWALL FOOTING 8" - MASORY WALLS
 - ALL-THREAEMBEDMENTS FOR SHEARWALLS AND HIGH UPLIFT CONNECTIONS ARE SHOWN IN THEIR RESICTIVE DETAILS ON THIS SHEET.
 - 8. USE 2" SQURE WASHERS ON ALL-THREAD AND ANCHOR BOLT LOCATIONS, U.N.O.
 - WHERE THIADED RODS ARE NOT ABLE TO BE INSTALLED DUE TO FIELD LIMITATIONS, IT IS ACCEPTABLTO PLACE A DOUBLE 2X STUD GROUP, FASTEN STUDS TOGETHER WITH (1) ROW OF 10d NAIL FASTEN THE STUDS TO THE TOP AND BOTTOM PLATES WITH (2) SPH. PLACE A 1/2" ANCHOR BIT WITH 6" EMBEDMENT WITHIN 6" OF DOUBLE STUD GROUP.

CONVEITIONAL FRAMING NOTES

- 1. ALL CONVETIONAL FRAMING LUMBER IS No.2 SYP.
- ALL RIDGEND VALLEY BEAM SIZES ARE NOTED ON THE PLANS.
- ALL ROOF IFTER SIZES ARE NOTED ON THE PLANS.
- 4. FASTEN RCF RAFTERS TO RIDGE BEAMS WITH (4) 16d TOE-NAILS.
- 5. FASTEN RCF RAFTERS TO BEARING WALLS WITH A SIMPSON H2.5A WITH (5) 8d NAILS IN THE RAFTER AND (5) 8d NAILS INTO THE TOP PLATE.
- 6. FASTEN RCF RAFTERS TO SLEEPER WITH SIMPSON H3 WITH (5) 8d NAILS INTO THE FFTER AND (5) 8d NAILS INTO THE SLEEPER.
- 7. 2X6 No.2 S' COLLAR TIES TO BE APPLIED. FASTEN EACH END WITH (6) 16d NAILS. BO'OM OF COLLAR TIES TO BE NO LOWER THAN 1/3 THE DISTANCE FROM THE PLATE TO THE PEAK.
- 8. ALL SLEEP'S ARE 2x6 No.2 SYP FASTENED TO EXISTING FRAMING WITH (4) 16d NAILS / EACH INTERSECTION OF SLEEPER AND EXISTING FRAMING.

ROOF RAFTER RIDGE BEAM ---- COLLAR TIE

(2) 2:SYP COLUMN FOR RIDGE/VALLEY BEAM SUPPORT, FASTENED TOGTHER WITH (1) ROW OF 8d NAILS AT 12" O.C. FASTEN RAFTER TO COLMN WITH 18 GA STRAP WITH (4) 8d NAILS IN RAFTER, (4) 8d NAILS IN COLIN. FASTEN COLUMN TO TOP PLATE OR HEADER WITH 18 GA STRAP WITH) 8d NAILS IN COLUMN, (4) 8d NAILS IN TOP PLATE/HEADER.

NOTE FOR BRICK VENEER SIDING

NO STORY

6'-0"

14'-0"

20'-0"

EXTEND NOT LESS THAN 8" INCHES INTO THE SUPPORT.

A. LONG LEG OF THE ANGLE SHALL BE PLACED IN A VERTICAL POSITION.

MEMBERS MEETING STRUCTURAL DESIGN REQUIREMENTS MAY BE USED.

ABOVE

ALLOWABLE SPANS FOR LINTELS SUPPORTING MASONRY VENEER a,b,c

ONE STORY

4'-6"

6'-0"

8'-0"

9'-6"

12'-0"

FOR SI: 1 INCH=25.4 MM, 1 FOOT=304.8 MM

B. DEPTH OF REINFORCED LINTELS SHALL NOT BE LESS THAN 8" AND ALL CELLS OF

HOLLOW MASONRY LINTELS SHALL BE GROUTED SOLID. REINFORCING BARS SHALL

C. STEEL MEMBERS INDICATED ARE ADEQUATE TYPICAL EXAMPLES; OTHER STEEL

ABOVE

TWO STORIES | No. OF 1/2"

3'-0"

4'-6"

6'-0"

7'-0"

9'-0"

EQUIVALENT

REINFORCING

2

3

4

1. HORIZONTAL TIES AT 24" O.C.

2. VERTICAL TIES AT 24" O.C.

3. WEEP HOLES AT 33" O.C.

SIZE OF STEEL

ANGLE

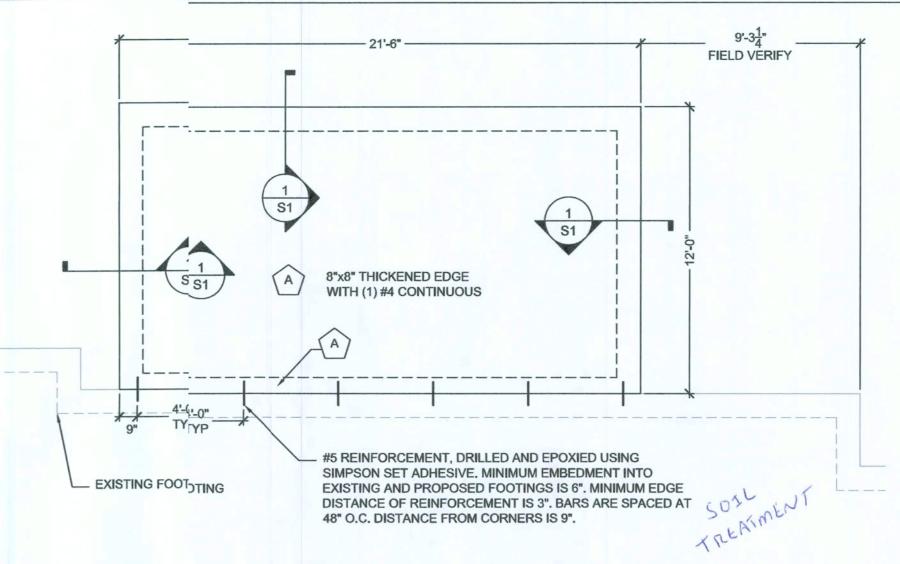
a,c (inches)

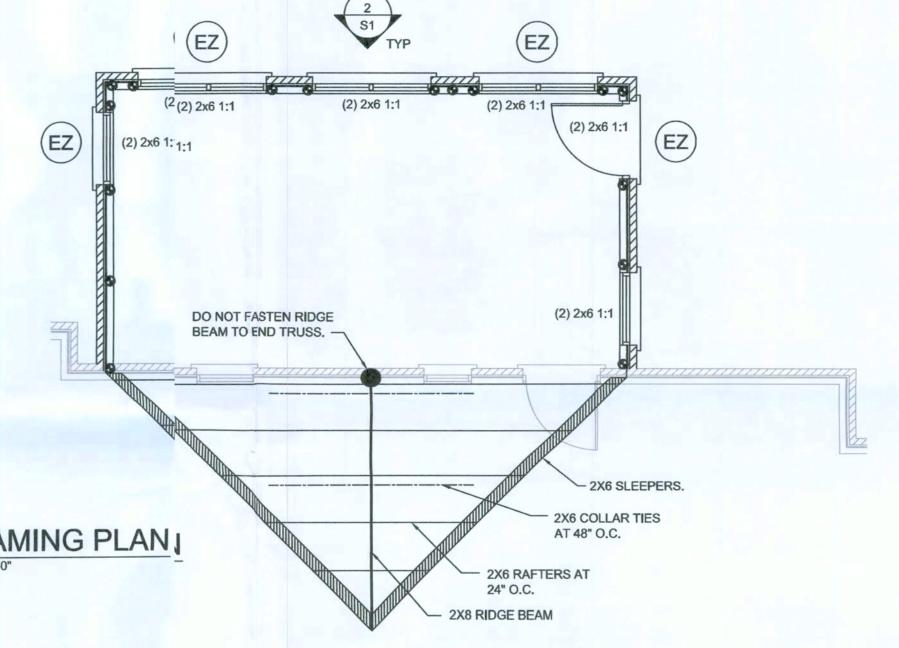
3x3x1/4

5x3 1/2x5/16

6x3 1/2x5/16

2-6x3 1/2x5/16





EDGE OF ALL THREAD WASHER MUST BE WITHIN 3" OF THE HEADER KING STUD, TYP BOTH SIDES OF OPENING. NO. 2 SYP HEADER, TYP. SEE PLANS FOR SIZE, PLIES, AND NUMBER OF KINGS AND CRIPPLES. KING STUDS & CRIPPLES: (6) 16d BETWEEN ADJACENT KING STUDS AND CRIPPLES. PLACE 1/2" ANCHOR BOLTS PER THE TYPICAL WALL SECTION.

TYPICAL HEADER FRAMING DETAIL

1. IF OPENINGS ARE LOCATED BETWEEN STANDARD ALL THREAD SPACING, NO ADDITIONAL CONNECTORS ARE REQUIRED FOR THE HEADER, CRIPPLES, KINGS OR TOP

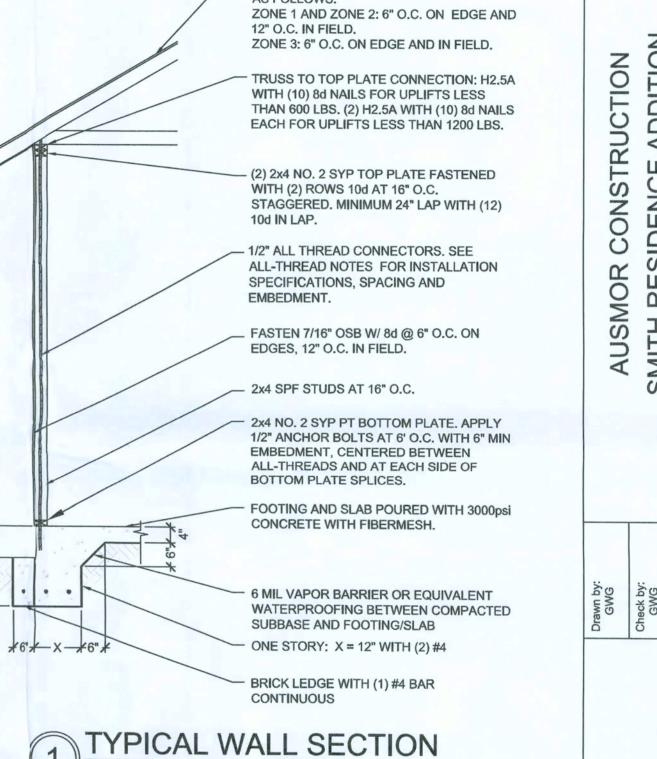
TOP AND BOTTOM PLATE MATERIAL: STUD MATERIAL: HEADERS: ALL HEADERS ARE (2) 2x6 No. 2 SYP WITH TWO KINGS AND ONE CRIPPLE, HEADER NOTATION IS: (EZ

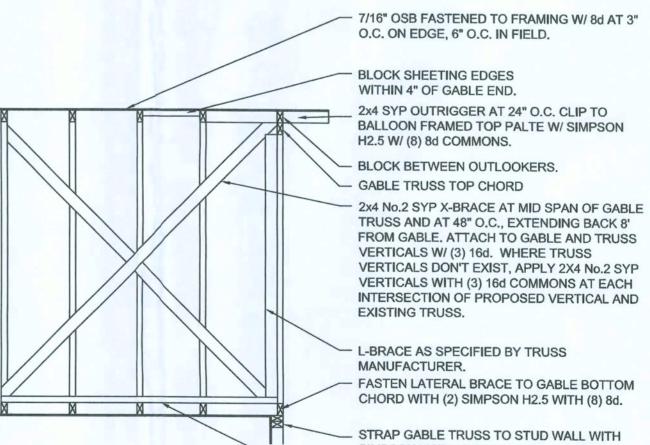
FINISHED GRADE

FRAMING NOTES AND LEGEND

UNO. FASTEN HEADER LUMBER TOGETHER USING (2) ROWS OF 16d (0.148" X 3.5") COMMON NAILS AT 12" ON CENTER, USE OSB SHIMS AS NECESSARY TO MAKE THE HEADER THICKNESS EQUAL TO THAT OF THE WALL IT IS IN. SEE THE HEADER DETAIL FOR FASTENING SPECIFICATIONS. NUMBER OF PLIES IN HEADER - NUMBER OF KING STUDS (2) 2x6 2:1 - NUMBER OF CRIPPLE STUDS SIZE OF DIMENSIONAL LUMBER 1-STORY THREADED ROD INDICATES GLAZING CLASSIFIED AS END ZONE. ALL OTHER GLAZING LOCATED ON EXTERIOR OF STRUCTURE WILL BE CLASSIFIED AS INTERIOR ZONE. SEE "COMPONENT AND CLADDING DESIGN PRESSURES," ON COVER SHEET. 7/16" OSB ROOF DECK FASTENED TO FRAMING WITH 8d RING SHANKS, SPACING AS FOLLOWS: ZONE 1 AND ZONE 2: 6" O.C. ON EDGE AND 12" O.C. IN FIELD. ZONE 3: 6" O.C. ON EDGE AND IN FIELD.

2x No. 2 SPF, UNO.

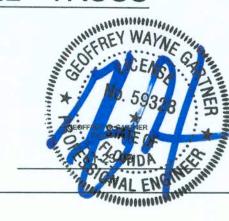




SIMPSON MSTA12 WITH (10) 10d AT 48" O.C.

2x4x8' SYP LATERAL BRACE AT 48". APPLY (2) 16d THROUGH DIAGONAL BRACE INTO EACH

GABLE BRACING DETAIL - TRUSS



1 OF

Sheet No.

ils document, together with the concepts and designs presented herein is intended only for the specific purpose and client for which it was prepared. Reuse of and improper reliance on this document without ven authorization by Alexander Grace Consulting, Inc., shall be without liability to Alexander Grace Consulting, Inc.

APPROXIMATE

SITE LOCATION

110 MPH

DUVAL COUNTY WIND PEED MAP

CONTRACTOR MAY USE IMPACT GLAZINGS, ERMANENT SHUTTERS OR

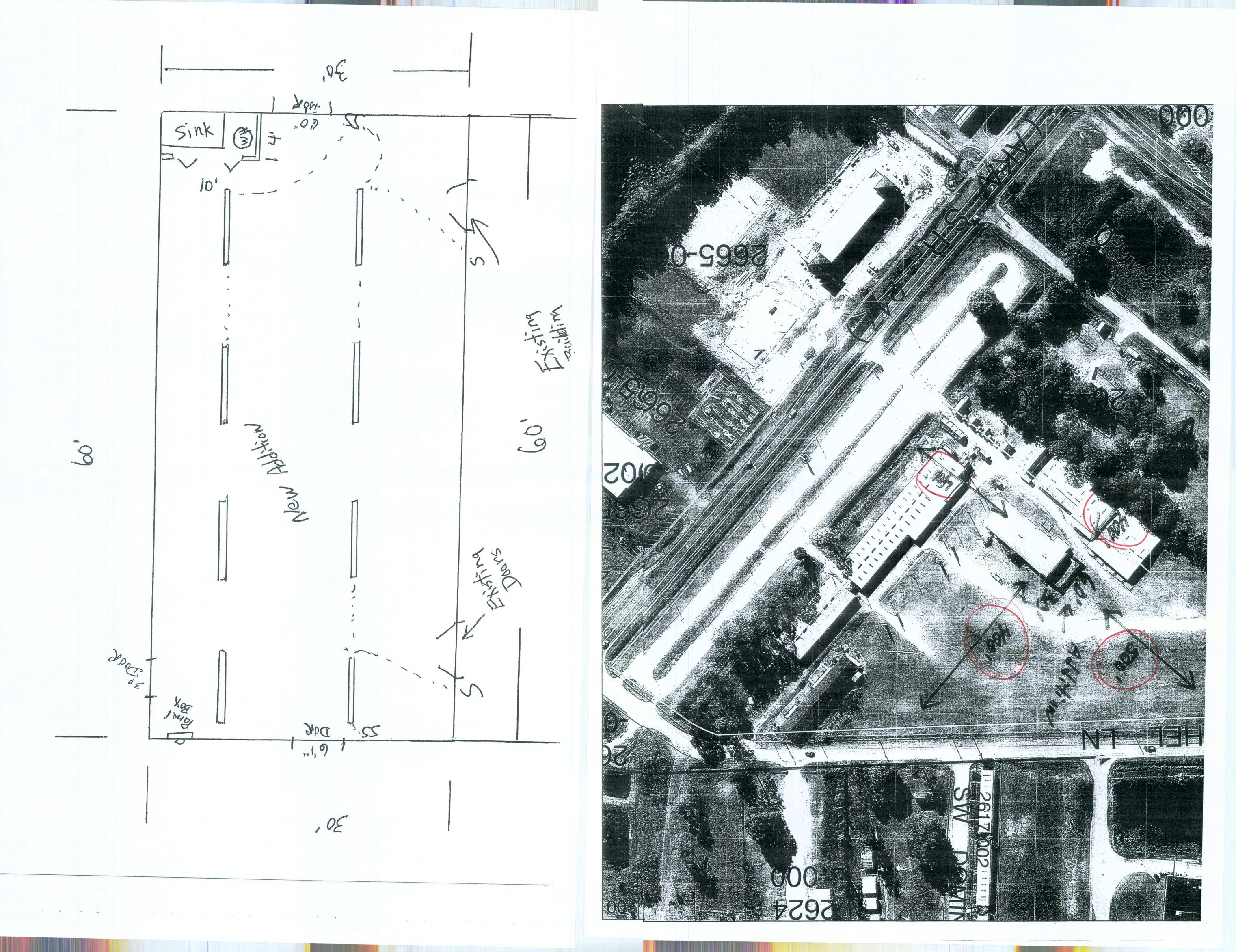
5/8" PLYWOOD, CUT TO FIT THE OPENINGS AD LABELED. PLYWOOD

AS THIS SITE LIES WITHIN THE HURRICANE IBRIS REGION,

PERMANENT OPENING PROTECTION WILL BREQUIRED. THE

MUST BE FASTENED WITH 1/4"X6" WOOD SCEWS AT 12" O.C.

105 MPH



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PAUL SEER 1758 NW Brown Rd. Lake City, FL 32055

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BASIC WIND SPEED:

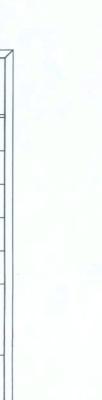
End ELEVATION

100 MPH

SCALE: 1/4" = 1'-0"

NOTE! HEIGHTS OF ALL ELEMENTS SHALL BE MATCHED TO EXISTING CONDITIONS

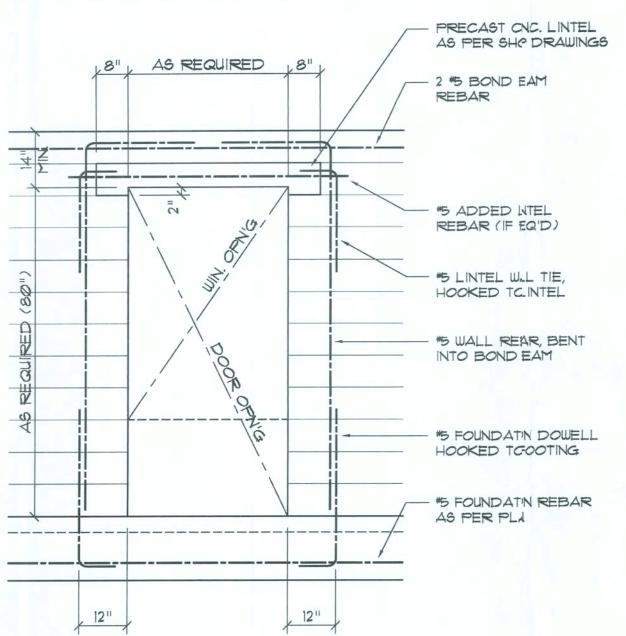
ALL MATERIALS SHALL MATCH EXISTING MATERIALS, COLOR, FININGH & STYLE.



1 = 1.00 WIND IMPORTANCE FACTOR (1): BUILDING CATAGORY CATAGORY II "B" WIND EXPOSURE: INTERNAL PRESSURE COEFFICIENT: +/- 0.18 MWFRS PER TABLE 1609.64 (FBC 2001) ROOF: - 19.1 PSF DESIGN WIND PRESSURES: WALLS: + 22.0 PSF EAVES: - 26.7 PSF COMPONENTS & CLADING PER TABLES | OP'NGS: + 18.0 / - 21 PSF 1609.6B \$ 1609.6C (FBC 2007) EAVES: - 56.4 PSF DESIGN WIND PRESSURES: ROOF: + 16.5 / - 20 PSF

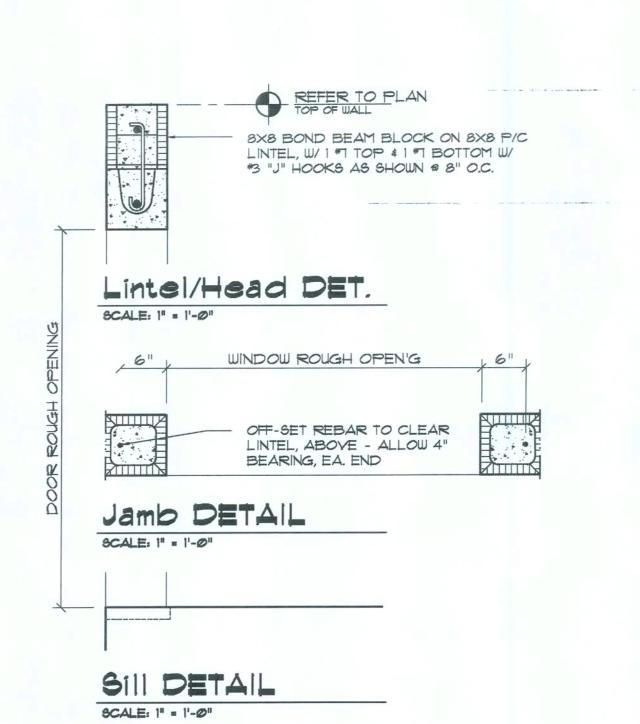
ALL WIND LOADS ARE IN ACCORDANCE WITH SECTION 169, FLORIDA BUILDING CODE, 2007 EDITION.





Typical Door/Window Opening Reinforcing DETAL SCALE: 1/2" = 1"-0"

NOTE! REFER TO GENERAL NOTES FOR LAP SPLICE AND HOOK MINIMUM LENGTH/SIZE - ALL PER ACI 318-LATEST



SIDE ELEVATION SCALE: 1/4" = 1'-0" OTHER SIDE: SIMILAR - OPP. HAND

THE FIRE MARSHALL, AND OTHER INSPECTION AUTHORITY. 2 AS REQUIRED BY THE BUILDING CODE, PERMITTING OF FIRE ALARM SYSTEMS SHALL BE BY DRAWINGS SIGNED AND SEALED BY A REGISTERED FIRE ALARM SYSTEM VENDOR/ INSTALLER. THE ELECTRICAL CONTRACTOR SHALL HAVE HIS VENDOR PREPARE THESE DRAWINGS FOR ENGINEERS APPROVAL AND SUBMIT THEM TO THE FIRE MARSHALL AND BUILDING DEPARTMENT, FOR PERMITTING

SPECIFY ALL ASPECTS OF, OR ALL REQUIRED EQUIPMENT. THE FIRE ALARM SYSTEM INSTALLED SHALL INCLUDE ALL DEVICES AND OTHER NECESSARY

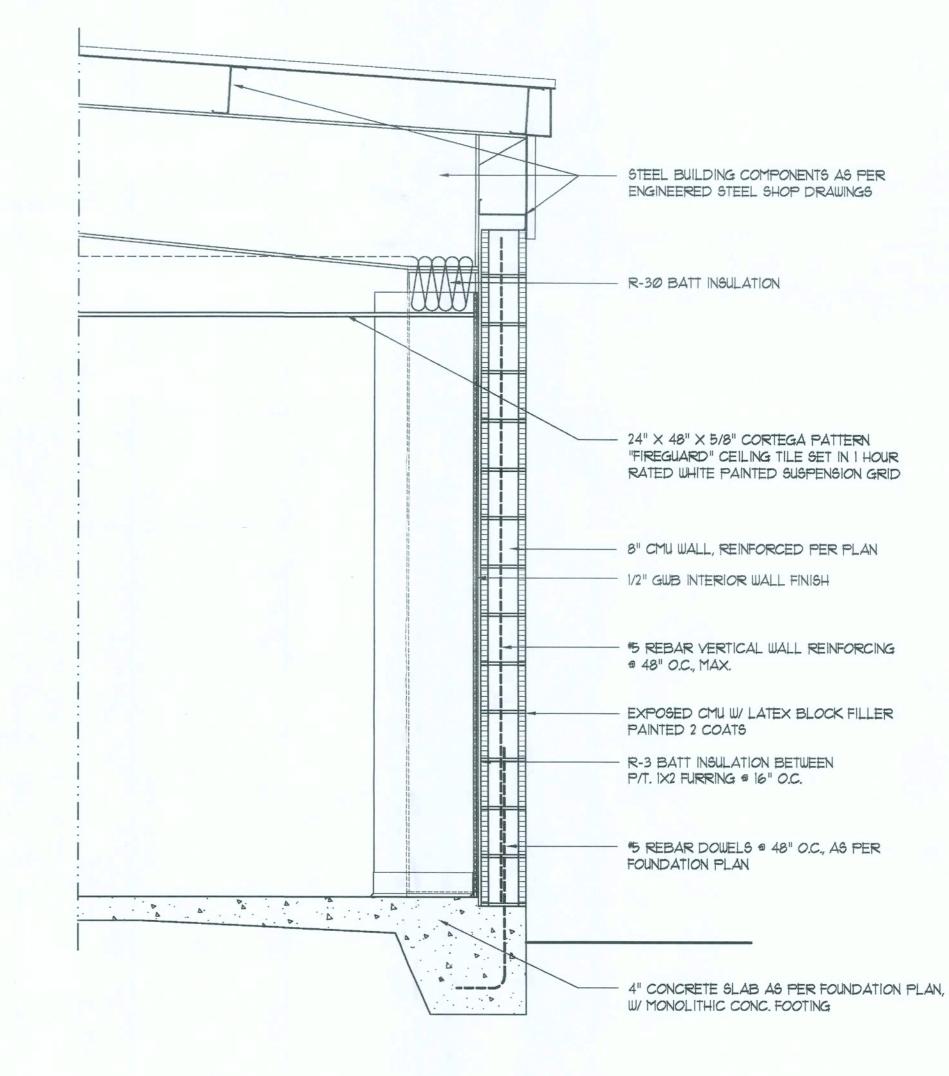
REQUIREMENTS TO COMPLY WITH ALL APPLICABLE CODES AND THE LOCAL

FIRE ALARM NOTES

THE FIRE ALARM SYSTEM IN THE BID ONLY, IT IS NOT INTENDED TO

1. THESE FIRE ALARM NOTES ARE SHOWN TO INCLUDE

- 3. ALL CONDUCTORS SHALL BE INSTALLED IN METALLIC CONDUIT. P.Y.C. MAY BE USED FOR U.G. RUNS ONLY. ALL CONDUIT SHALL BE 3/4" TRADE SIZE, MIN. UNLESS SPECIFIED OTHERWISE. ALL CONDUCTORS SHALL BE COPPER WITH TITN OR EQUAL INSULATION. CONDUCTORS IN U.G. CONDUITS SHALL BE
- 4. THE SIZE AND NUMBER OF CONDUCTORS SHALL BE PROVIDED IN ACCORDANCE WITH THE SYSTEM REQUIREMENTS, AND MANUFACTURES SPECIFICATIONS.
- 5. THE FIRE ALARM SYSTEM SHALL BE A MULTIPLEXING DIGITAL SYSTEM
 AND ALL EQUIPMENT SHALL BE NEW AND OF CURRENT MODELS AVAILABLE.
 THE CONTRACTOR SHALL INCLUDE APPROPRIATEL SIZED BATTERY SYSTEM(S)
 AND PROVIDE BATTERY CALCULATIONS FOR THE SYSTEM ON
 THE SHOP DRAWINGS.
- 6. OUTLET BOX SIZES, AND OTHER REQUIREMENTS SHALL COMPLY WITH THE N.E.C. AND SYSTEM REQUIREMENTS



Typical SECTION SCALE: 3/4" = 1'-0"

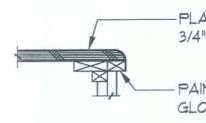
NOTE! HEIGHTS OF ALL ELEMENTS SHALL BE MATCHED TO EXISTING CONDITIONS

Floor PLAN

SCALE: 1/4" = 1'-0"

CABINETS, COUNTERS, SHELVES AND THE LIKE, SOWN ON THIS PLAN SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE STANDARDS OF QUALITY AS OUTLINED IN TENOTES TITLED "GENERAL MILLWORK NOTES", AND SHAL INCLUDE SUCH FEATURES, HARDWARE AND FINISHES AS DRECTED BY THE OWNER. THE PLAN YIEWS INDICATED ARE PR GENERAL LOCATION AND EXTENT OF THE WORK - UNLESS ETAILED CABINET PLANS ARE INCLUDED WITH THIS PLAN PACKAGE ALL OTHER PHYSICAL CHARACTERISTICS SHALIBE AS DIRECTED BY THE OWNER

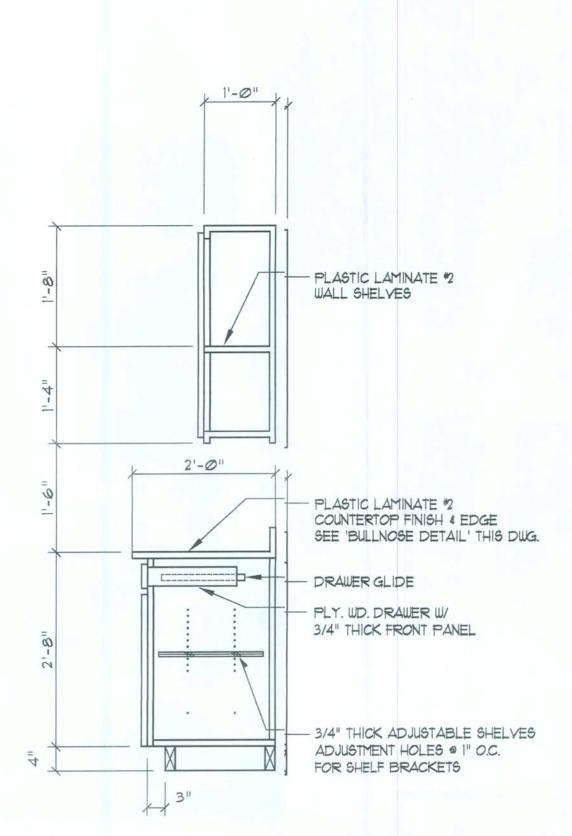
PROVIDE 2X6 BACKING AT ALL OVERHEAD CAINET LOCATIONS, FLUSH WITH FACE OF FRAMING - TOPOF BACKING TO BE T'-0" AFF.



-PLAST;TIC LAMINATE FINISH O/ 3/4" PPLY, WD. SUBSTRATE

- PAINT-T-OUT EXPOSED WOOD W/ GLOSS,S BLACK PAINT

-BULLNOOSE DETAIL-



GENERAL INTERIOR FINISH SCHEDULILE:

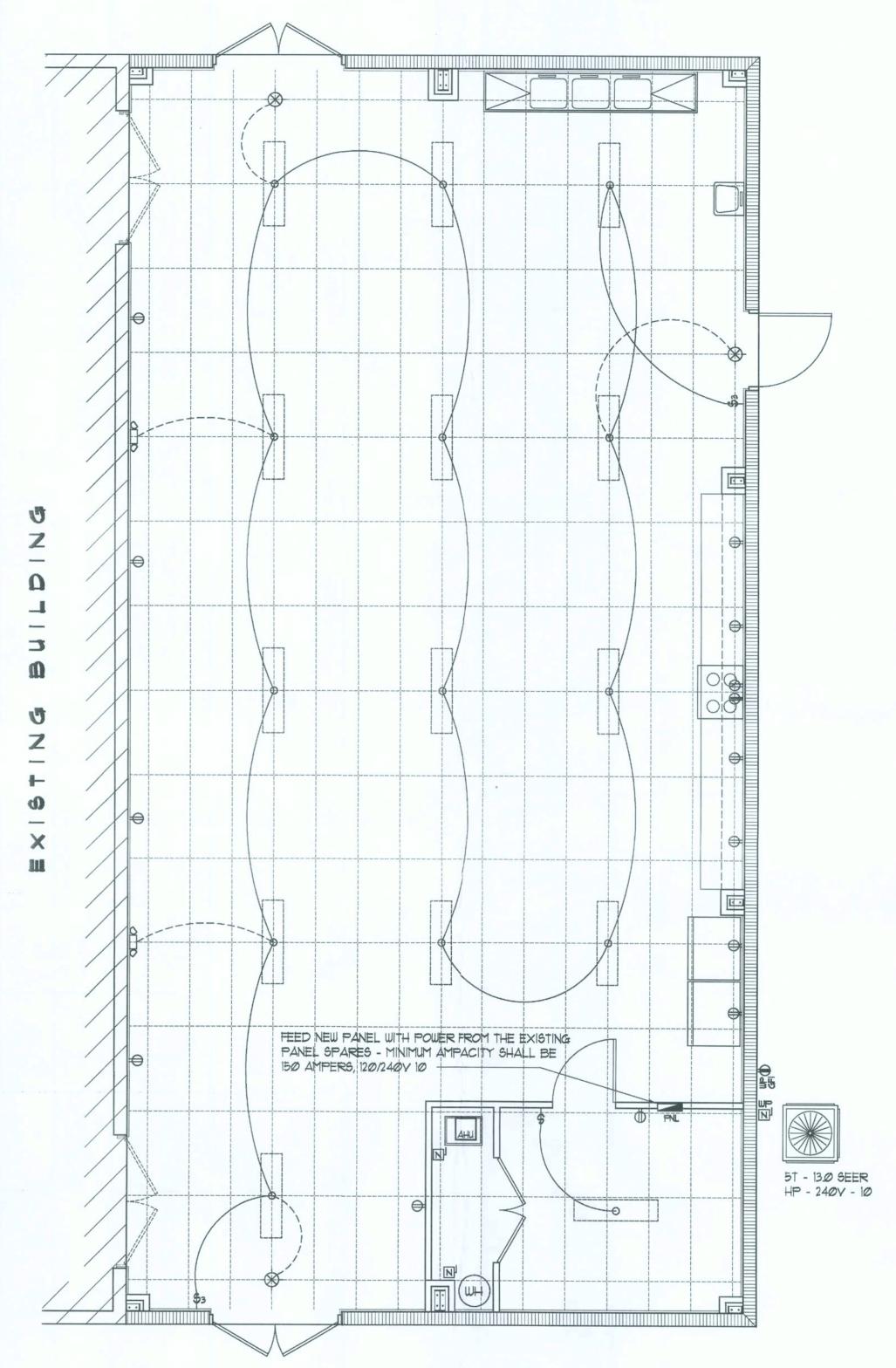
POLISHED SMOOTHTH CONCRETE SLAB WITH STAINED FINISH AS SELECTED BY THEHE OWNER 4" RUBBER COVE, E, COLOR AS SELECTED BY THE OWNER OR CERAMIC TILE 1/2" GWB, PRIMED 5 AND PAINTED 2 COATS LATEX WALL PAINT, WALLS: COLOR & GLOSS & AS SELECTED BY THE OWNER 24" × 48" × 5/8" (" CORTEGA PATTERN "FIREGUARD" CEILING TILE MAIN CEILING: SET IN I HR RATERED WHITE PANTED SUSPENSION GRID APPLIED FINISHES: APPLIED FINISHERD TO GIUB, Id: SPRAY, KNOCK-DOWN, SKIP-TROWEL AND SIMILAR TRESEATMENTS AS DIRECTED BY THE OWNER. AS SELECTED BY3Y THE OWNER MINIMUM API GRADE: "CUSTOM" - ALL COUNTERTOPS SH3HALL BE AS SELECTED BY THE OWNER CABNETS:

ELECTRICAL CONT'R SHALL PRREPARE "AS-BUILT" SHOP DWGS INDICATING ALL ELECTRIFICAL WORK, INCLUDING ANY CHANGES TO THE ELEC. PLAN, ADD'NS TO THE ELEC. PLAN, RISER DIAGRAM, AS-BUILT PANNEL SCHEDULE W/ ALL CKTS IDENTIFIED W/ CKT Nr., DESCRIPPTION & BRKR, SERVICE ENT. & ALL UNDERGROUND WIRE LOGICATIONS/ROUTING/DEPTH. RISER DIA. SHALL INCLUDE WIRDE SIZES/TYPE & EQUIPMENT TYPE W/ RATINGS & LOADS.

CONTRACTOR SHALL PROVIDE : 1 COPY OF AS-BUILT DWGS TO OWNER & I COPY TO THE PEERMIT ISSUING AUTHORITY.

H.V.A.C. CONTRACTOR SHALL PEREPARE "AS-BUILT" SHOP DRAWINGS INDICATING ALL H.V. A.C. WORK, INCLUDING ALL DUCTWORK LOC., SIZES, LINES, EEQUIPMENT SCH. & BALANCING REPORT - CONT'R SHALL PROYVIDE I COPY OF AS-BUILT DWGS TO OWNER & I COPY TO THE PEERMIT ISSUING AUTHORITY.

PLUMBING CONTRACTOR SHALL PREPARE "AS-BUILT" SHOP DRAWINGS INDICATING ALL PLUIUMBING WORK, INCLUDING ALL PLUMBING LINE LOCATIONS AND RISER DIAGRAM - CONT'R SHALL PROVIDE I COPY OF ASS-BUILT DWGS TO OWNER AND I COPY TO THE PERMIT ISSUING & AUTHORITY.



EMERGENCY LIGHTING AND EXIT SIGNS, SHALL BE PROVIDED AS DIRECTED BY THE FIRE MARSHAL, AND SHALL BE WIRED PER NEC 700-12F.

Electrical PLAN

SCALE: 1/4" = 1'-0"

FIRE ALARM SYSTEM

THIS FACILITY SHALL BE EQUIPTED WITH A SELF-CONTAINED FIRE ALARM ADVANCE WARNING SYSTEM. THE OPPERATION OF WHICH SHALL ALERT THE BUILDING OCCUPANTS AND NOTIFY THE 911 EMERGENCY RESPONSE SYSTEM. EQUIPMENT AND SERVICE PROVIDER SHALL BE AS SELECTED BY THE OWNER, DETAILS OF INSTALLATION SHALL BE VIA SHOP DRAWINGS AND OPPERATING FEATURES SHALL BE AS REQUIRED BY NFPA 101, 2009 EDITION, "LIFE SAFETY CODE" SECTION 40.3.4

FIRE EXTINGUISHER CABINETS

PROVIDE 10 LB ABC FIRE EXTINGUISHERS IN ALUM. SEMI-RECESSED WALL CABINETS W/ BREAKAWAY GLASS FRONT PANEL. LOCATE I CABINET AT EACH EXIT DOOR OF EACH SPACE, WITHIN 10 FEET OF THE DOOR ANY OTHER LOCATIONS SHALL BE AS DIRECTED BY THE FIRE MARSHAL AND/OR OWNER.

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GENERAL:

1. THE DRAWINGS ARE INTENDED TO SHOW THE GENERAL ARRANGEMENT, DESIGN AND EXTENT OF THE WORK AND ARE PARTIALLY DIAGRAMMATIC. THEY ARE NOT INTENDED TO BE SCALED FOR ROUGH-IN MEASUREMENTS, OR TO SERVE AS SHOP DRAWINGS OR PORTIONS THEREOF.

2. ALL DETAILS AND SECTIONS SHOWN ON THE DRAWINGS ARE INTENDED TO BE TYPICAL AND SHALL BE CONSTRUED TO APPLY TO ANY SIMILAR SITUATION ELSEWHERE ON THE PROJECT, EXCEPT WHERE A DIFFERENT DETAIL OR SECTION IS SHOWN.

3. PRIOR TO START OF CONSTRUCTION, THE CONTRACTOR AND ALL THE SUBCONTRACTORS SHALL VERIFY ALL GRADES, LINES, LEVELS, DIMENSIONS AND COORDINATE EXISTING CONDITIONS AT THE JOB SITE WITH THE PLANS AND SPECIFICATIONS. THEY SHALL REPORT ANY INCONSISTENCIES OR ERRORS IN THE ABOVE TO THE ARCHITECT/ENGINEER BEFORE COMMENCING WORK. THE CONTRACTOR AND HIS SUBCONTRACTORS SHALL LAY OUT THEIR WORK FROM ESTABLISHED REFERENCE POINTS AND BE RESPONSIBLE FOR ALL LINES, ELEVATIONS AND MEASUREMENTS IN CONNECTION WITH THEIR WORK.

4. IF ANY ERRORS OR OMISSIONS APPEAR IN THE DRAWINGS, GENERAL NOTES OR OTHER DOCUMENTS, THE CONTRACTOR SHALL NOTIFY THE ARCHITECT IN WRITING OF SUCH OMISSION OR ERROR PRIOR TO PROCEEDING WITH ANY WORK WHICH APPEARS IN QUESTION. IN THE EVENT OF THE CONTRACTOR'S FAILING TO GIVE SUCH AN ADVANCED NOTICE, HE SHALL BE HELD RESPONSIBLE FOR THE RESULTS OF ANY SUCH ERRORS OR OMISSIONS AND THE COST OF RECTIFYING THE SAME.

5. THE CONTRACTOR SHALL USE THE STRUCTURAL DRAWINGS AND SPECIFICATIONS TOGETHER WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL AND OTHER TRADE DRAWINGS AND SHOP DRAWINGS, TO LOCATE DEPRESSED SLABS, SLOPES, DRAINS, OUTLETS, RECESSES, OPENINGS, BOLT SETTING, SLEEVES, DIMENSIONS, ETC. NOTIFY ARCHITECT/ENGINEER, IN WRITING, OF ANY POTENTIAL CONFLICTS BEFORE PROCEEDING WITH THE

SHOP DRAWINGS AND DELEGATED ENGINEERING:

ALL SHOP DRAWINGS SHALL BE SUBMITTED FOR ARCHITECT'S REVIEW ONLY AFTER THEY HAVE BEEN THOROUGHLY REVIEWED BY THE CONTRACTOR FOR CONSTRUCTION METHODS, DIMENSIONS AND OTHER TRADE REQUIREMENTS, AND STAMPED WITH THE CONTRACTOR'S APPROVAL STAMP. THE ARCHITECT ASSUMES NO RESPONSIBILITY FOR DIMENSIONS, QUANTITIES ENGINEERING DESIGN BY DELEGATED ENGINEERS, ERRORS OR OMISSIONS AS A RESULT OF REVIEWING ANY SHOP DRAWINGS. ANY ERRORS OR OMISSIONS MUST BE MADE GOOD BY THE CONTRACTOR, IRRESPECTIVE OF RECEIPT, CHECKING OR REVIEW OF DRAWINGS BY THE ENGINEER AND EVEN THOUGH WORK IS DONE IN ACCORDANCE WITH SUCH DRAWINGS.

2. BEFORE STRUCTURAL INSPECTIONS CAN BE MADE ON A PORTION OF THE STRUCTURE, ALL RELATED SHOP DRAWINGS, DELEGATED ENGINEERING, PRODUCT APPROVAL, MANUFACTURER'S DATA AND OTHER RELATED INFORMATION, MUST BE REVIEWED AND ACCEPTED BY THE ARCHITECT-OF-RECORD AND APPROVED BY THE BUILDING DEPARTMENT.

3. SHOP DRAWINGS SHALL CONTAIN ALL INFORMATION SHOWN ON THE STRUCTURAL PLANS (RELATED TO THE DELEGATED DESIGN) INCLUDING ALL DESIGN LOADS, IN ADDITION TO THE INFORMATION REQUIRED BY THE DELEGATED ENGINEER'S DESIGN.

4. ARCHITECT WILL REVIEW ALL SUBMITTED SHOP DRAWINGS, PREPARED AND SIGNED AND SEALED BY THE CONTRACTOR'S DELEGATED ENGINEER, ONLY FOR GENERAL COMPLIANCE WITH THE DESIGN INTENT, REQUIRED LOADING AND COORDINATION WITH THE STRUCTURAL DESIGN.

5. CONTRACTOR SHALL SUBMIT TO THE ARCHITECT TWO SETS OF BLUE PRINTS OF THE STRUCTURAL SHOP DRAWINGS FOR ARCHITECT REVIEW, BEFORE STARTING FABRICATION. THE ARCHITECT WILL RETURN ONE MARKED UP AND STAMPED COPY TO THE CONTRACTOR. THE MARKED-UP COPY SHALL BE USED TO MAKE THE PRINTS REQUIRED FOR SHOP DRAWING DISTRIBUTION.

CONSTRUCTION MEANS AND METHODS:

THE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCE OR PROCEDURES, SAFETY PRECAUTIONS, SHORES, RESHORES, LATERAL BRACING AND PROGRAMS IN CONNECTION WITH THE PROJECT. ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. OUR SERVICES DO NOT GUARANTEE NOR ASSURE LIABILITY FOR THE JOB SAFETY, TEMPORARY SHORING AND BRACING AND THE PERFORMANCE OF THE CONTRACTOR.

2. THE CONTRACTOR IS RESPONSIBLE AND SHALL COMPLY WITH THE SAFETY REQUIREMENTS OF THE 2004 FLORIDA BUILDING CODE AND APPLICABLE LOCAL, STATE AND FEDERAL LAWS.

3. PROVIDE ALL SHORING, BRACING AND SHEETING AS REQUIRED FOR SAFETY, STRUCTURAL STABILITY AND FOR THE PROPER EXECUTION OF THE WORK, REMOVE WHEN WORK IS COMPLETED.

4. PROVIDE AND MAINTAIN GUARD LIGHTS AT ALL BARRICADES. RAILINGS. OBSTRUCTIONS IN THE STREETS, ROADS OR SIDEWALKS AND ALL

TRENCHES OR PITS ADJACENT TO PUBLIC WALKS OR ROADS. 5. AT ALL TIMES, PROVIDE PROTECTION AGAINST WEATHER (RAIN, WIND, STORMS OR THE SUN), SO AS TO MAINTAIN ALL WORK, MATERIALS,

APPARATUS AND FIXTURES FREE FROM INJURY OR DAMAGE. 6. AT THE END OF THE DAYS WORK, COVER ALL WORK LIKELY TO BE DAMAGED. ANY WORK DAMAGED BY FAILURE TO PROVIDE PROTECTION SHALL BE REMOVED AND REPLACED WITH NEW WORK AT THE CONTRACTOR'S

7. THE CONTRACTOR SHALL PAY FOR ALL DAMAGES TO ADJACENT STRUCTURES, SIDEWALKS AND TO STREETS OR OTHER PUBLIC PROPERTY OR PUBLIC UTILITIES.

STRUCTURAL DESIGN CRITERIA:

. THE DESIGN COMPLIES WITH THE REQUIREMENTS OF THE 2007 FLORIDA BUILDING CODE - SECTION 1609 AND OTHER REFERENCED CODES AND SPECIFICATIONS. ALL CODES AND SPECIFICATIONS SHALL BE LATEST EDITION AT TIME OF PERMIT.

..... 40 PSF

WIND LOAD CRITERIA:

RESIDENTIAL

BASED ON ANSI/ASCE 7-03. BASIC WIND VELOCITY 110 MPH,

3. ROOF DESIGN LOADS: SUPERIMPOSED DEAD LOADS: 20 PSF SUPERIMPOSED LIVE LOADS: 20 PSF 4. FLOOR DESIGN LOADS: SUPERIMPOSED DEAD LOADS: 25 PSF SUPERIMPOSED LIVE LOADS:

. 60 PSF BALCONIES 5. WIND NET UPLIFT: ARE AS INDICATED ON PLANS

FOUNDATIONS (SPREAD FOOTINGS)

FOUNDATIS ARE DESIGNED TO BEAR ON WELL COMPACTED GRADE OR CLN FILL OF AN ALLOWABLE BEARING CAPACITY OF 1,000 PSF MINMUM, FOREQUIRED SOIL BEARING CAPASITIES GREATER THAN 1,000 PSF, A CERTID TESTING LABORATORY SHALL BE ENGAGED BY THE OWNER TO VEY THAT THE REQUIRED BEARING CAPACITY WAS OBTAINED. SAID SOIL CACITY SHALL BE CERTIFIED AND TESTED BY A FLORIDA REGISTERED FINDATION ENGINEER, PRIOR TO CASTING OF CONCRETE IN

2. NATURALRADE (OR FILL) BELOW FOOTINGS SHALL BE COMPACTED T98 % MODIFIED PROCTOR (ASTM D-1557).

3. TOP OF LL FOOTINGS TO BE AT THE SAME ELEVATION AS TOP OF COLUMN PAD ITINGS. STEP WALL FOOTING FROM HIGHER COLUMN FOOTING TO THE LOWEDNE (AS DETAILED ON THE PLANS).

4. BOTTOM ALL FOOTINGS TO BE A MINIMUM I'-6" BELOW THE TOP OF CONCRETE SLION GRADE (UNLESS OTHERWISE NOTED) OR MINIMUM 1'-0" BELOW FINISHIGRADE, WHICHEVER IS LOWER. IN THE EVENT THAT THE SLAB STEPS ON EASIDE OF THE FOOTING, THE FOOTING SHALL BE 11-6" BELOW TOP OF THE LOWEBLAB.

5. REINFORG IN THE CONTINUOUS WALL FOOTINGS (MONOLITHIC AND NON-MONTHIC) SHALL BE SPLICED 40 BAR DIAMETERS MINIMUM AND SHALL EXTENDINTINUOUSLY THRU ALL FOOTING PADS.

6. ALL LONUDINAL REBARS IN THE CONTINUOUS WALL FOOTINGS SHALL BE CONUED AT BENTS AND CORNERS BY BENDING THE REBARS 48 BAR DIAMETELAROUND THE CORNERS OR ADDING MATCHING CORNER BARS, EXTENDING 48AR-DIAMETERS INTO FOOTING EACH SIDE OF CORNER OR BENT.

7. ALL FOOTGS SHALL BE 12" MINIMUM THICKNESS.

CONCRETE SLD ON GRADE:

ALL INTER AND EXTERIOR SLABS AND WALKWAYS AS SHOWN ON THE STRUURAL OR ARCHITECTURAL PLANS, SHALL BE FOUR INCHES THICK MINIMUREINFORCED WITH 6 X 6 - WI.4 X WI.4 WELDED WIRE FABRIC (UNLESS OTHNISE NOTED).

2. ALL SLATON GRADE TO BE CONSTRUCTED IN ACCORDANCE WITH LATEST A.C.I "GUIDE FOR CONCRETE FLOOR AND SLAB CONSTRUCTION" (A.C.I.

3. JOINTS SLL BE PROVIDED IN ALL INTERIOR SLABS ON GRADE AT LOC. INDICATEON THE PLANS DIVIDING THE SLAB INTO SQUARE PANELS NOT TO EXCEED 20 X FT. IN SIZE. CAST SLAB IN LONG ALTERNATE STRIPS. PROVIDE A CONTRACTICJOINT BETWEEN EACH STRIP. SEE PLAN FOR SAW-CUT, CONTRACTION ID ISOLATION JOINT DETAILS.

4. PROVIDE W-CUT JOINTS AT ALL SIDEWALKS AT A MAXIMUM SPACING OF F. FEET ON CENTERS AND ISOLATION JOINTS AT 20 FEET O.C.

5. FILL MATIAL SHALL BE PLACED IN LIFTS NOT EXCEEDING 12" AND COMPACT TO 98 % MODIFIED PROCTOR (ASTM D-1557) WITHIN A DISTANCE OF FEET BEYOND ALL FOOTING EDGES. TAKE AT LEAST ONE DENSITY TESTOR EACH 1,600 SQ.FT. OF AREA AND 12" BELOW SURFACE. SEND RESULTS OF I TEST TO OWNER, ARCHITECT AND ENGINEER.

CONCRETE ANREINFORCING:

CONCRETEESIGN AND REINFORCEMENT IN ACCORDANCE WITH "BUILDING COTREQUIREMENTS FOR REINFORCED CONCRETE" (A.C.I. 318 -LATEST EDITION AND WITH "DETAILS AND DETAILING OF CONCRETE REINFORCEMEN - (A.C.I. 315 - LATEST EDITION).

2. ALL CONCTE WORK IN ACCORDANCE WITH "SPECIFICATIONS FOR STRUCTURAL ACRETE FOR BUILDING" (A.C.I. 301 - LATEST EDITION). PRODUCTION (CONCRETE, DELIVERY, PLACING AND CURING TO BE IN ACCORDANCE 'H 'HOT WEATHER CONCRETING" (A.C.I. 305R - LATEST

3. ALL CONCTE TO BE REGULAR WEIGHT WITH A DESIGN STRENGTH OF 3,000 P.S.AT 28 DAYS. MAXIMUM SLUMP 5.

4. ALL REINRCING TO BE NEW BILLET STEEL CONFORMING TO THE LATEST A.S.T A-615 GRADE 60, FABRICATED IN ACCORDANCE WITH C.R.S.I. MANUAL OF SYDARD PRACTICE AND PLACED IN ACCORDANCE WITH A.C.I. 315 AND C.R.S.I. NUAL OF STANDARD PRACTICE.

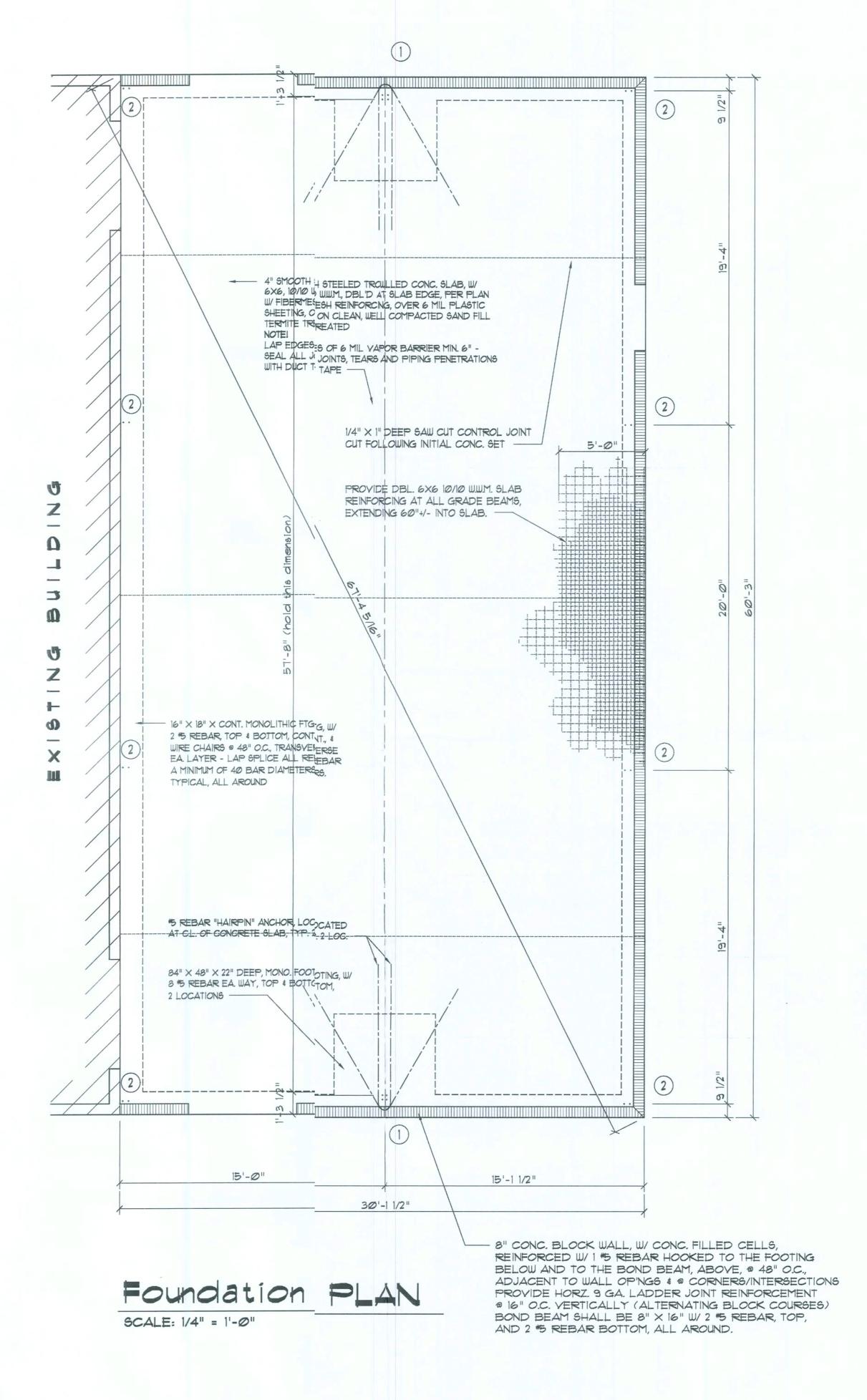
5. CONCRETIOVER UNLESS OTHERWISE DETAILED ON DRAWINGS:

CENTERED W/SLAB SLABS ON GRE:

6. BEAM REORCEMENT: LAPPED 36 BAR DIAMETER OR MINIMUM 18 INCHES. BOT'II BARS SPLICED ONLY AT SUPPORTS, TOP BARS SPLICED ONLY AT MID-AN. ALL TOP BARS HOOKED AT NONCONTINUOUS EDGES (U.O.N.). ALLOOKS TO BE STANDARD 90 DEGREE HOOKS AS REQUIRED

. ADDED RIFORCEMENT: PROVIDE ADDITIONAL CORNER BARS BENT 36 INCHIMINIMUM EACH WAY AT "L" AND "T" CORNERS IN OUTER FACES OF ALL BEAMSO MATCH ALL HORIZONTAL BAR (TOP, BOTTOM AND INTERMEDIATEEBARS).

8. SEE PLAN'R MINIMUM SIZE CONCRETE TIE BEAM REQUIREMENTS.



ANCHOR BOLT / FOUNDATION SIZING:

THE ANCHOR BOLT DIAMETERS AND DEVELOPED LENGTHS INDICATED IN THIS DRAWING WERE DETERMININED USING SHEAR FRICTION THEORY AS DESCRIBED IN AISC DESIGN GUIDE No.7, SECTION 9.2, ASSUMING AN ANCHOR BOLT MATERIAL OF ASTM A301 OR A36. THE COMBINED FORCES ACTING AT THE BASE OF THE STEEL FRAME RESULTING IN A VERTICAL REACTION ACTING UPON THE FOUNDATION WERE DEVELOPED AS FOLLOWS:

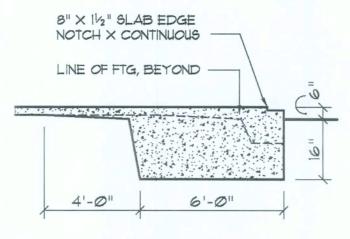
T = Td + Tsf

T = TOTAL TENSILE FORCE PER BOLT Td = TENSILE FORCE PER BOLT DUE TO DIRECTLY APPLIED LOAD = PM TOF = TENSILE FORCE PER BOLT DUE TO SHEAR FRICTION = Y / (n X u)

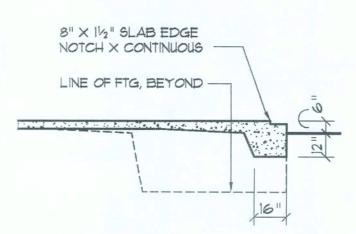
P = P = TOTAL UPLIFT TO BE RESISTED BY ANCHOR BOLT GROUP V = V = TOTAL SHEAR FORCE TO BE RESISTED BY ANCHOR BOLT GROUP

n = n = NUMBER OF ANCHOR BOLTS u = U = COEFFICIENT OF PRICTION (TAKEN AS Ø.7 FOR UNGROUTED BASE

PLATES OR 0.9 FOR GROUTED BASE PLATES)



FOOTING @ MAIN FRAME

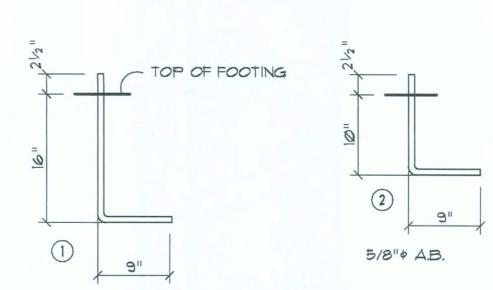


FOOTING @ SLAB EDGE

REFER TO THE METAL BUILDING SHOP DRAWINGS PREPARED BY BSX - BUILDING SYSTEMS EXPRESS, INC., FOR EXACT LOCATION OF ALL EMBEDDED ANCHOR BOLTS.

ADDED FILL SHALL BE APPLIED IN 12" LIFTS -EA. LIFT SHALL BE CONPACTED TO 98% DRY COMPACTION PER THE "MODIFIED PROCTOR" METHOD.

THE DESIGN WIND SPEED FOR THIS PROJECT IS 100 MPH PER 2004 FBC 1606 AND LOCAL JURISDICTION REQUIREMENTS



3/4" \$ A.B.

ALL ANCHOR BOLTS ARE ASTM GRADE A36 STEEL ROD, THREADED 3", OR GRADE A301, BLACK, AND FREE FROM RUST AND SCALE

Anchor Bolt DETAILS

SCALE: 1" = 1'-0"



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