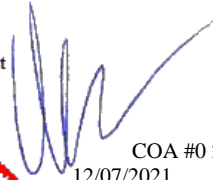


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12/07/2021



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6750 Forum Drive, Suite 305
Orlando, FL 32821
Phone: (800)755-6001
www.alpineitw.com



Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 21-6343
Job Description: Mitchell	
Address:	

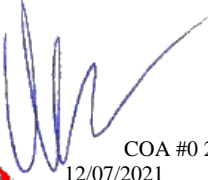
Job Engineering Criteria:	
Design Code: FBC 7th Ed. 2020 Res.	IntelliVIEW Version: 21.01.01A JRef #: 1Xb52150003
Wind Standard: ASCE 7-16 Wind Speed (mph): 130	Design Loading (psf): 40.00
Building Type: Closed	

This package contains general notes pages, 50 truss drawing(s) and 7 detail(s).

Item	Drawing Number	Truss
1	341.21.1202.20056	A01
3	341.21.1202.22665	A03
5	341.21.1202.24025	A05
7	341.21.1202.21212	A07
9	341.21.1202.23681	A09
11	341.21.1202.21337	A11
13	341.21.1202.23931	A13
15	341.21.1202.24478	A15
17	341.21.1202.24321	A17
19	341.21.1202.21571	A19
21	341.21.1202.21540	A21
23	341.21.1202.23571	A23
25	341.21.1202.20134	A25
27	341.21.1202.20775	A27
29	341.21.1238.30060	B02
31	341.21.1202.22743	J01
33	341.21.1202.23946	J02
35	341.21.1202.23009	J03
37	341.21.1202.24040	J05
39	341.21.1202.23181	J07
41	341.21.1202.24306	J09
43	341.21.1202.20415	PB01
45	341.21.1202.19462	PB03
47	341.21.1202.20384	V02
49	341.21.1202.19321	V04
51	A14015ENC160118	

Item	Drawing Number	Truss
2	341.21.1202.19415	A02
4	341.21.1202.22666	A04
6	341.21.1202.24556	A06
8	341.21.1202.23603	A08
10	341.21.1202.23493	A10
12	341.21.1202.22401	A12
14	341.21.1202.23103	A14
16	341.21.1202.21024	A16
18	341.21.1202.23399	A18
20	341.21.1202.22759	A20
22	341.21.1202.22587	A22
24	341.21.1202.19322	A24
26	341.21.1202.20321	A26
28	341.21.1202.20650	B01
30	341.21.1238.26440	B03
32	341.21.1202.22681	J01HJ
34	341.21.1202.23385	J02HJ
36	341.21.1202.24165	J04
38	341.21.1202.24603	J06
40	341.21.1202.21228	J08
42	341.21.1202.24181	J10
44	341.21.1202.20196	PB02
46	341.21.1202.19196	V01
48	341.21.1202.19306	V03
50	341.21.1202.20822	V05
52	A14030ENC160118	

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Address:	

Item	Drawing Number	Truss
53	BRCLBSUB0119	
55	PB160160118	
57	VALTN160118	

Item	Drawing Number	Truss
54	GBLLETIN0118	
56	VAL180160118	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

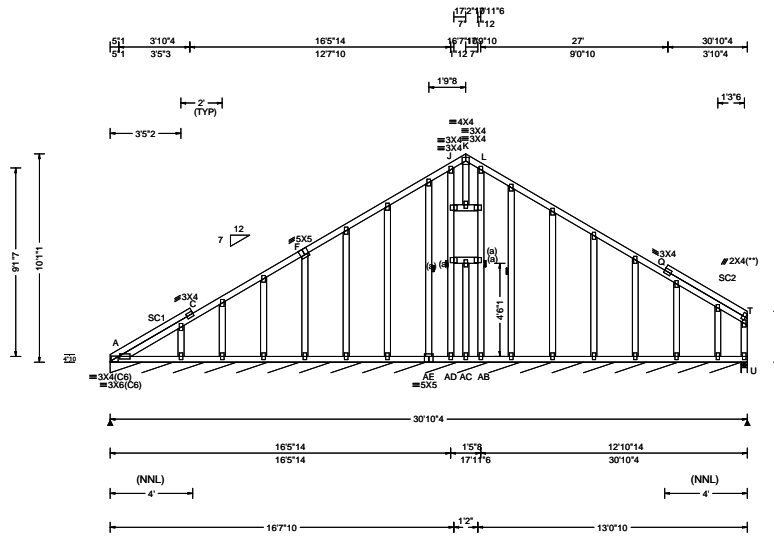
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoclin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 514 Earth City Expressway, Suite 242, Earth City, MO 63045; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com.

SEQN: 56065 / FROM:	GABL Ply: 1 Qty: 1	Job Number: 21-6343 Mitchell Truss Label: A01	Cust: R 215 JRef: 1Xb52150003 T9 DrwNo: 341.21.1202.20056 / YK 12/07/2021
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Loading Criteria (psf) TCLL: 20.00 TC DL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TC DL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.09 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg, Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.003 Q 999 240 VERT(CL): 0.006 Q 999 180 HORZ(LL): 0.004 Q - - HORZ(TL): 0.006 Q - - Creep Factor: 2.0 Max TC CSI: 0.331 Max BC CSI: 0.081 Max Web CSI: 0.173 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs), or *=PLF <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A*</td> <td>79</td> <td>-</td> <td>-</td> <td>/53</td> <td>-</td> <td>/6</td> </tr> <tr> <td>U</td> <td>141</td> <td>-</td> <td>-</td> <td>/89</td> <td>/59</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS A Brg Wid = 366 Min Req = - U Brg Wid = 3.5 Min Req = 1.5 Bearings A & U are a rigid surface. Members not listed have forces less than 375#	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A*	79	-	-	/53	-	/6	U	141	-	-	/89	/59	-
Loc	Gravity			Non-Gravity																											
	R+	/R-	/Rh	/Rw	/U	/RL																									
A*	79	-	-	/53	-	/6																									
U	141	-	-	/89	/59	-																									

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;
 Stack Chord: SC1 2x4 SP #2;
 Stack Chord: SC2 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

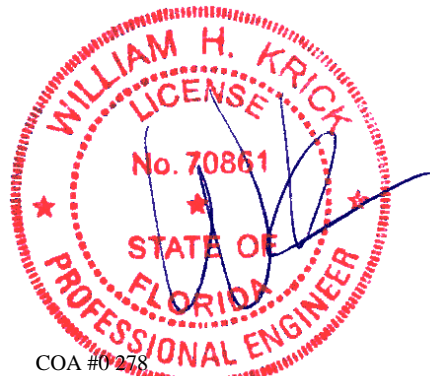
All plates are 2X4 except as noted.
 (**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
 Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

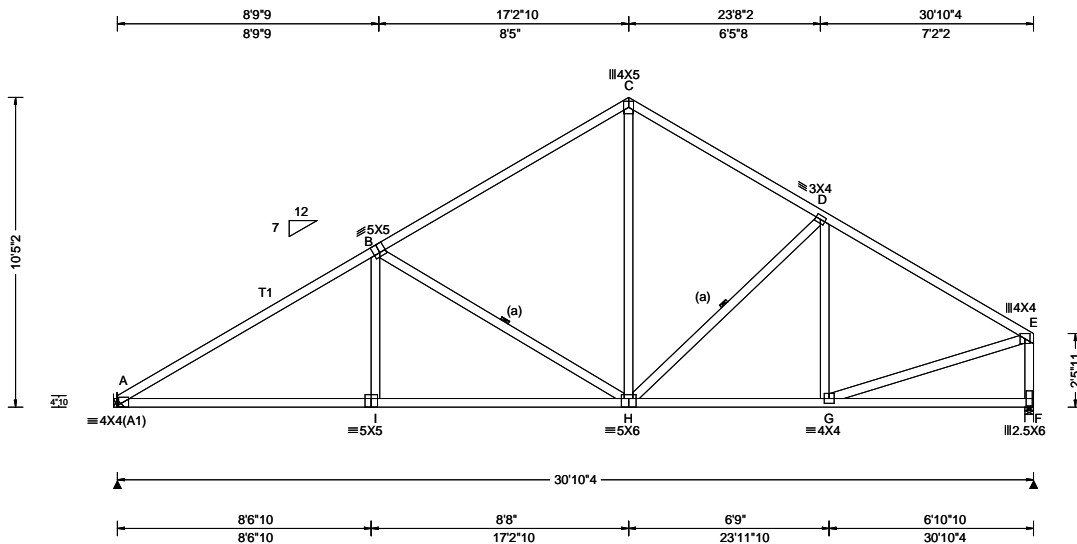


COA #0278

12/07/2021

****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**
 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.
 Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
 For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org





Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.09 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.077 I 999 240 VERT(CL): 0.146 I 999 180 HORZ(LL): 0.034 F - - HORZ(TL): 0.063 F - - Creep Factor: 2.0 Max TC CSI: 0.866 Max BC CSI: 0.809 Max Web CSI: 0.603 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>1433</td> <td>-</td> <td>-</td> <td>756</td> <td>299</td> <td>258</td> </tr> <tr> <td>F</td> <td>1394</td> <td>-</td> <td>-</td> <td>704</td> <td>302</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS A Brg Wid = - Min Req = - F Brg Wid = 3.5 Bearing F is a rigid surface. Members not listed have forces less than 375#</p> Maximum Top Chord Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>A - B</td> <td>687 -2290</td> <td>C - D</td> <td>571 -1381</td> </tr> <tr> <td>B - C</td> <td>554 -1420</td> <td>D - E</td> <td>514 -1592</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	1433	-	-	756	299	258	F	1394	-	-	704	302	-	Chords	Tens.Comp.	Chords	Tens. Comp.	A - B	687 -2290	C - D	571 -1381	B - C	554 -1420	D - E	514 -1592
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Lumber
 Top chord: 2x4 SP #2; T1 2x4 SP M-31;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Hangers / Ties
 (J) Hanger Support Required, by others

Loading
 Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.



Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
A - I	1875 -562	H - G	1297 -356
I - H	1870 -563		

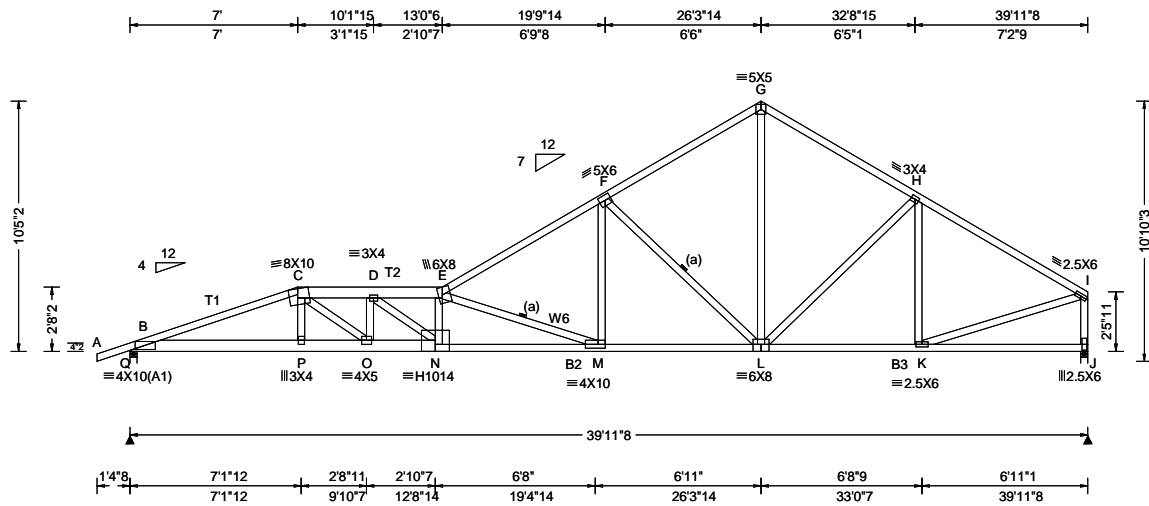
Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - H	412 -902	G - E	1317 -347
C - H	837 -306	E - F	454 -1340

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2 Complete Trusses Required



Loading Criteria (psf) TCLL: 20.00 TC DL: 10.00 BC LL: 0.00 BC DL: 10.00 Des Ld: 40.00 NCBC LL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TC DL: 4.2 psf BC DL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 4.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.419 E 999 240 VERT(CL): 0.848 E 563 180 HORZ(LL): -0.081 G - - HORZ(TL): 0.164 G - - Creep Factor: 2.0 Max TC CSI: 0.565 Max BC CSI: 0.732 Max Web CSI: 0.924 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>Q</td> <td>4047</td> <td>-</td> <td>-</td> <td>-</td> <td>/985</td> <td>-</td> </tr> <tr> <td>J</td> <td>2256</td> <td>-</td> <td>-</td> <td>-</td> <td>/536</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS Q Brg Wid = 3.5 Min Req = 1.7 J Brg Wid = 3.5 Min Req = 1.5 Bearings Q & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>1393 - 5832</td> <td>F - G</td> <td>340 - 1388</td> </tr> <tr> <td>C - D</td> <td>1594 - 6749</td> <td>G - H</td> <td>340 - 1388</td> </tr> <tr> <td>D - E</td> <td>1573 - 6649</td> <td>H - I</td> <td>329 - 1351</td> </tr> <tr> <td>E - F</td> <td>669 - 2784</td> <td></td> <td></td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	Q	4047	-	-	-	/985	-	J	2256	-	-	-	/536	-	Chords	Tens.Comp.	Chords	Tens. Comp.	B - C	1393 - 5832	F - G	340 - 1388	C - D	1594 - 6749	G - H	340 - 1388	D - E	1573 - 6649	H - I	329 - 1351	E - F	669 - 2784		
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Lumber
 Top chord: 2x4 SP #2; T1 2x4 SP M-31;
 T2 2x6 SP 2400f-2.0E;
 Bot chord: 2x6 SP 2400f-2.0E; B2 2x4 SP M-31;
 B3 2x4 SP #2;
 Webs: 2x4 SP #3; W6 2x4 SP #2;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Nailnote
 Nail Schedule: 0.128"x3", min. nails
 Top Chord: 1 Row @12.00" o.c.
 Bot Chord: 1 Row @12.00" o.c.
 Webs : 1 Row @ 4" o.c.
 Use equal spacing between rows and stagger nails in each row to avoid splitting.

Wind
 Wind loads and reactions based on MWFRS.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes
 The overall height of this truss excluding overhang is 10'-5-2.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - P	5516 - 1312	N - M	6647 - 1576
P - O	5551 - 1315	M - L	2307 - 548
O - N	6752 - 1596	L - K	1134 - 270

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
C - P	534 - 55	F - L	380 - 1600
C - O	1523 - 354	G - L	1152 - 237
E - M	1080 - 4552	K - I	1158 - 274
M - F	1587 - 334	I - J	278 - 1097

Special Loads
 -----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From 61 plf at -1.38 to 61 plf at 7.00	TC: From 31 plf at 7.00 to 31 plf at 9.04
TC: From 61 plf at 9.04 to 61 plf at 13.03	TC: From 63 plf at 13.03 to 63 plf at 39.96
BC: From 4 plf at -1.38 to 4 plf at 0.00	BC: From 20 plf at 0.00 to 20 plf at 7.03
BC: From 10 plf at 7.03 to 10 plf at 9.04	BC: From 20 plf at 9.04 to 20 plf at 39.96
TC: 476 lb Conc. Load at 7.03	BC: 462 lb Conc. Load at 7.03
BC: 2062 lb Conc. Load at 9.04	



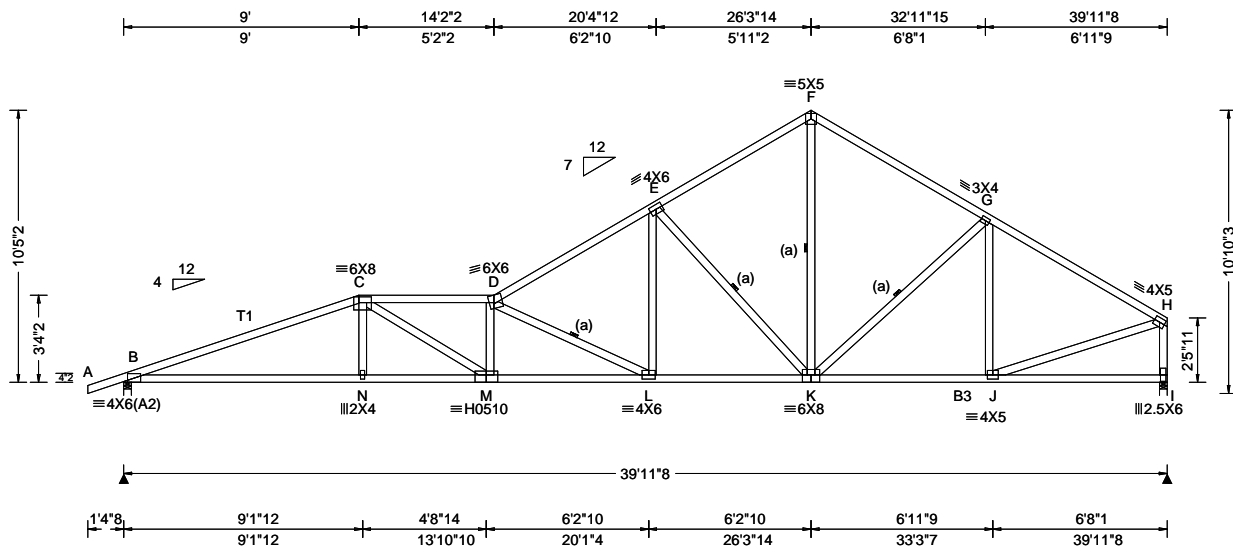
COA #0278
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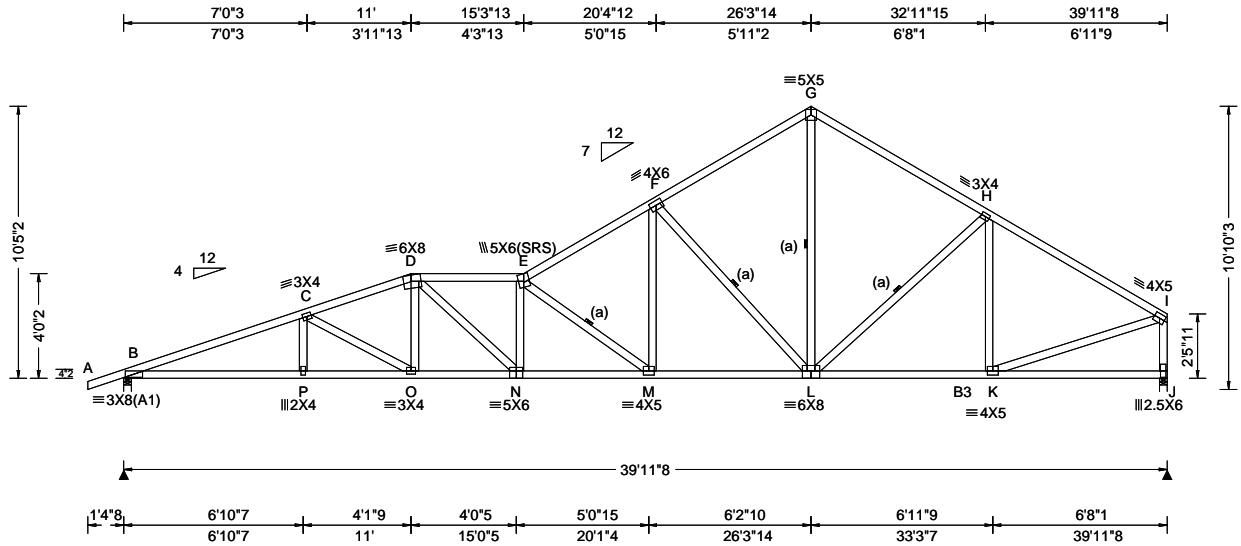
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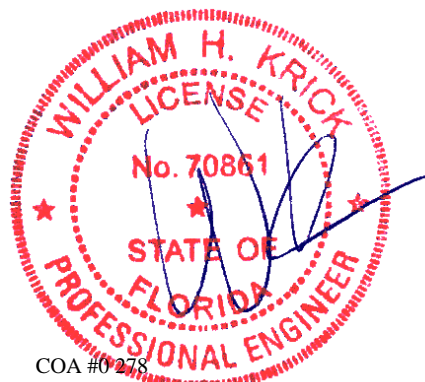
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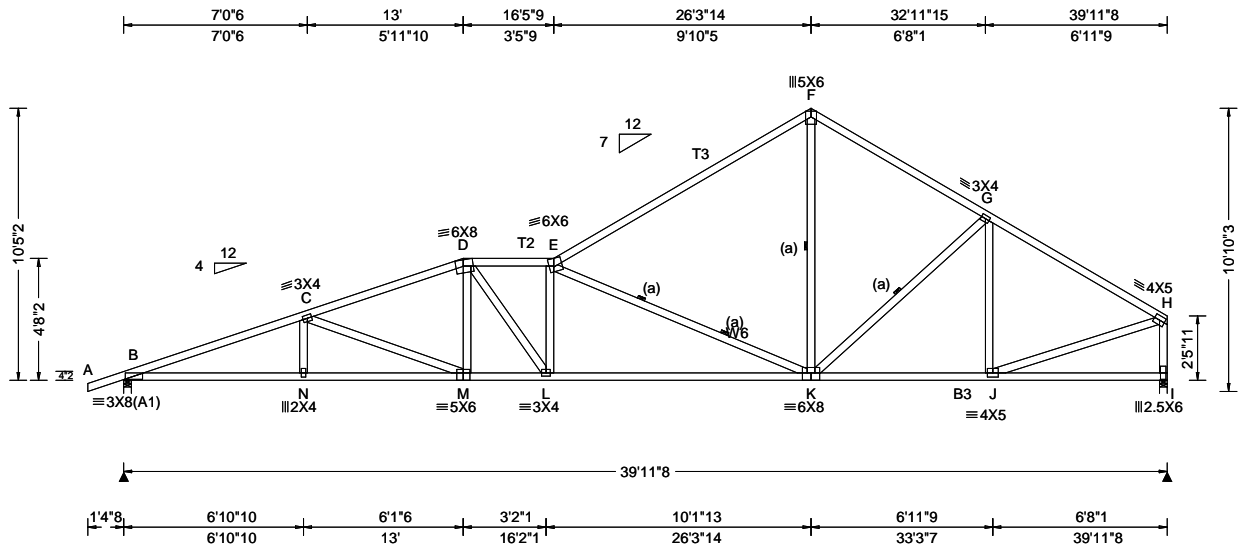
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 4.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.280 E 999 240 VERT(CL): 0.572 E 834 180 HORZ(LL): 0.066 I - - HORZ(TL): 0.135 I - - Creep Factor: 2.0 Max TC CSI: 0.860 Max BC CSI: 0.777 Max Web CSI: 0.972 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>1732</td> <td>-</td> <td>-</td> <td>/1072</td> <td>/430</td> <td>/274</td> </tr> <tr> <td>I</td> <td>1646</td> <td>-</td> <td>-</td> <td>/897</td> <td>/393</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 I Brg Wid = 3.5 Min Req = 1.9 Bearings B & I are a rigid surface. Members not listed have forces less than 375#</p> Maximum Top Chord Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>1829 - 4214</td> <td>E - F</td> <td>826 - 1900</td> </tr> <tr> <td>C - D</td> <td>1589 - 3507</td> <td>F - G</td> <td>858 - 1842</td> </tr> <tr> <td>D - E</td> <td>1678 - 3674</td> <td>G - H</td> <td>745 - 1893</td> </tr> </tbody> </table> Maximum Bot Chord Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - N</td> <td>3940 - 1806</td> <td>L - K</td> <td>3700 - 1595</td> </tr> <tr> <td>N - M</td> <td>3935 - 1809</td> <td>K - J</td> <td>1565 - 560</td> </tr> <tr> <td>M - L</td> <td>3267 - 1470</td> <td></td> <td></td> </tr> </tbody> </table> Maximum Web Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Webs</th> <th>Tens.Comp.</th> <th>Webs</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>C - M</td> <td>364 - 718</td> <td>F - K</td> <td>1211 - 531</td> </tr> <tr> <td>D - L</td> <td>692 - 258</td> <td>J - H</td> <td>1595 - 560</td> </tr> <tr> <td>L - E</td> <td>271 - 409</td> <td>H - I</td> <td>638 - 1587</td> </tr> <tr> <td>E - K</td> <td>1230 - 2399</td> <td></td> <td></td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	1732	-	-	/1072	/430	/274	I	1646	-	-	/897	/393	-	Chords	Tens.Comp.	Chords	Tens. Comp.	B - C	1829 - 4214	E - F	826 - 1900	C - D	1589 - 3507	F - G	858 - 1842	D - E	1678 - 3674	G - H	745 - 1893	Chords	Tens.Comp.	Chords	Tens. Comp.	B - N	3940 - 1806	L - K	3700 - 1595	N - M	3935 - 1809	K - J	1565 - 560	M - L	3267 - 1470			Webs	Tens.Comp.	Webs	Tens. Comp.	C - M	364 - 718	F - K	1211 - 531	D - L	692 - 258	J - H	1595 - 560	L - E	271 - 409	H - I	638 - 1587	E - K	1230 - 2399		
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Lumber

Top chord: 2x4 SP #2; T2,T3 2x4 SP M-31;
 Bot chord: 2x4 SP M-31; B3 2x4 SP #2;
 Webs: 2x4 SP #3; W6 2x4 SP #2;

Bracing

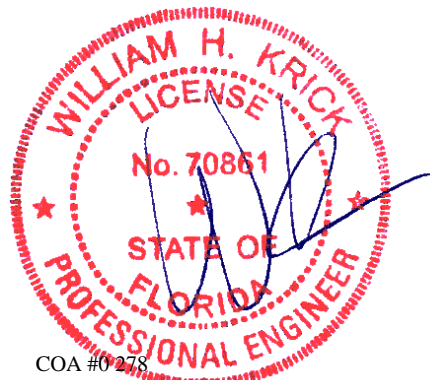
(a) Continuous lateral restraint equally spaced on member.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 10-5-2.



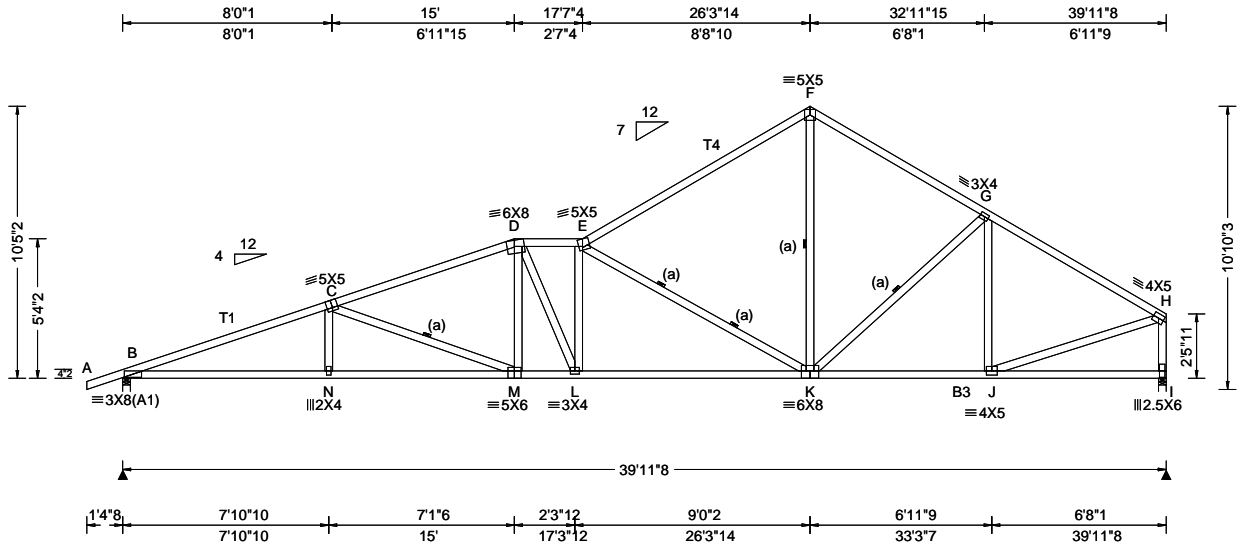
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6750 Forum Drive
 Suite 305
 Orlando FL, 32821



Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 4.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.244 E 999 240 VERT(CL): 0.498 E 960 180 HORZ(LL): 0.063 I - - HORZ(TL): 0.130 I - - Creep Factor: 2.0 Max TC CSI: 0.796 Max BC CSI: 0.684 Max Web CSI: 0.979 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1728 - / - / - /1079 /430 /273 I 1645 - / - / - /894 /393 -/ Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 I Brg Wid = 3.5 Min Req = 1.9 Bearings B & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.					
				B - C 1769 -4157 E - F 839 - 1865 C - D 1473 -3281 F - G 864 - 1835 D - E 1496 -3220 G - H 751 - 1894					

Lumber

Top chord: 2x4 SP #2; T1,T4 2x4 SP M-31;
 Bot chord: 2x4 SP M-31; B3 2x4 SP #2;
 Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Wind

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Additional Notes

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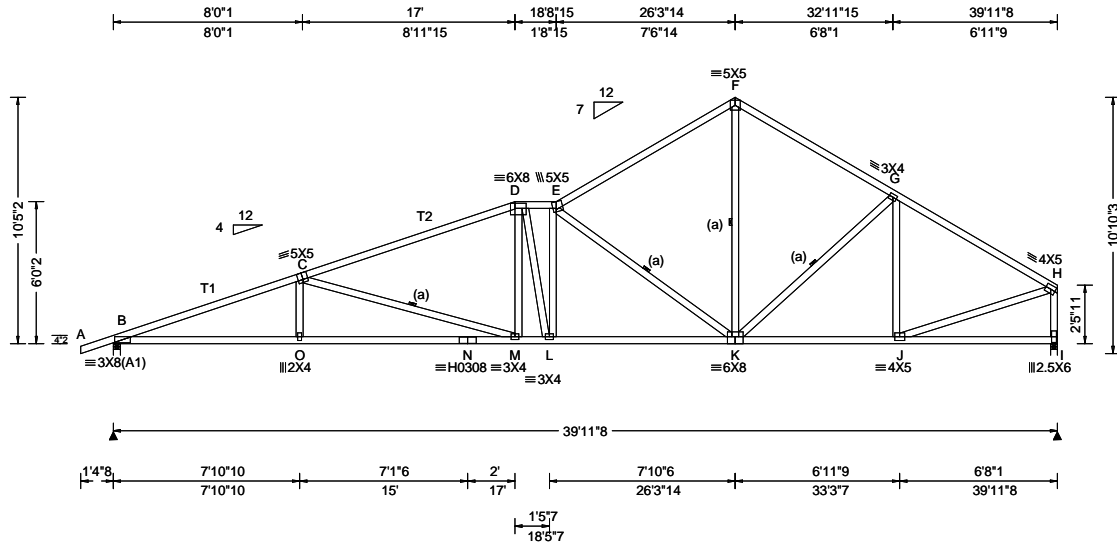


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Lumber
Top chord: 2x4 SP #2; T1,T2 2x4 SP M-31;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Wind
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Wind loading based on both gable and hip roof types.

Additional Notes
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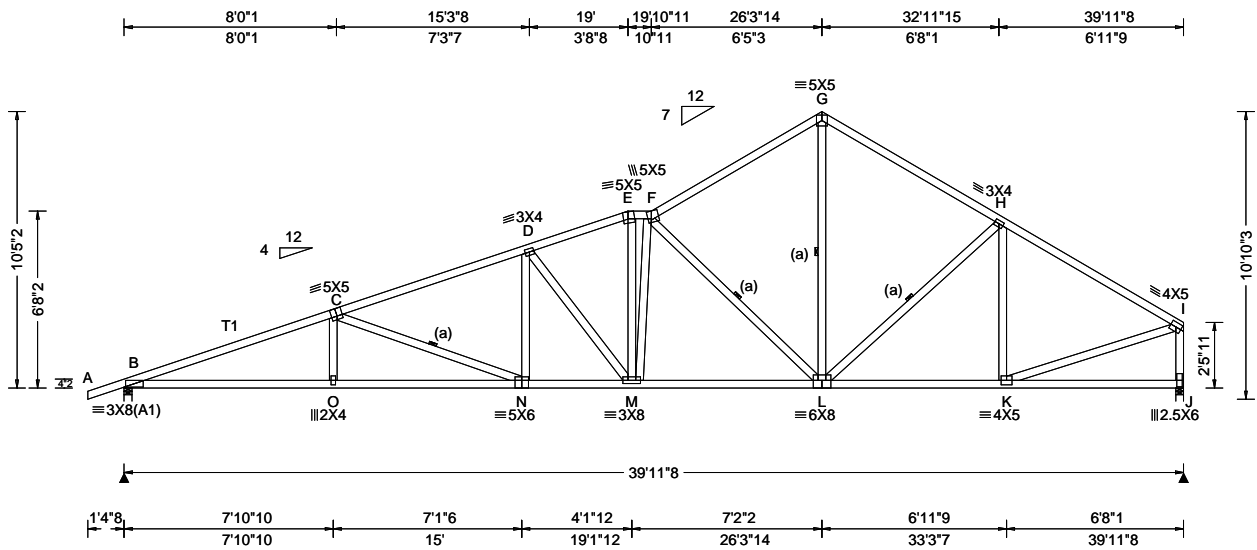
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Lumber
 Top chord: 2x4 SP #2; T1 2x4 SP M-31;
 Bot chord: 2x4 SP M-31;
 Webs: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Right end vertical not exposed to wind pressure.
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Additional Notes
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Maximum Bot Chord Forces Per Ply (lbs)

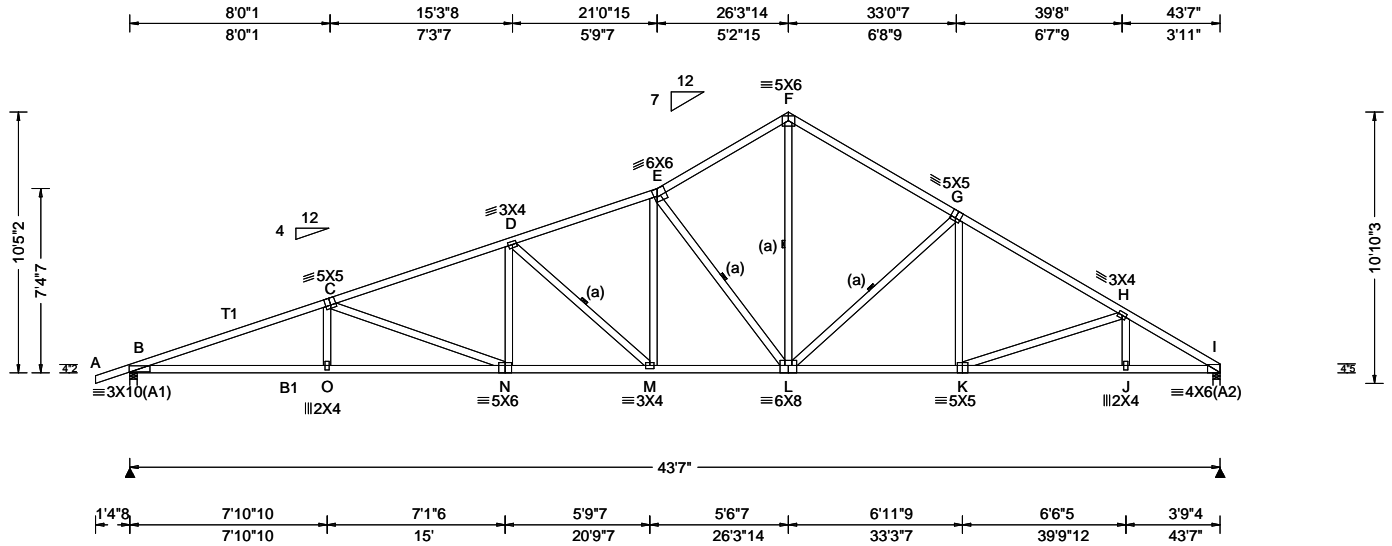
Chords	Tens.Comp.	Chords	Tens. Comp.
B - O	3872 - 1644	M - L	2565 - 1050
O - N	3867 - 1646	L - K	1569 - 573
N - M	3017 - 1291		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
C - N	438 - 888	F - L	817 - 1527
N - D	458 - 129	G - L	1304 - 629
D - M	423 - 803	K - I	1603 - 575
E - M	864 - 382	I - J	645 - 1585

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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 4.36 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.308 N 999 240 VERT(CL): 0.628 N 826 180 HORZ(LL): 0.099 I - - HORZ(TL): 0.202 I - - Creep Factor: 2.0 Max TC CSI: 0.852 Max BC CSI: 0.849 Max Web CSI: 0.990 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>1868</td> <td>-</td> <td>-</td> <td>/1184</td> <td>/468</td> <td>/290</td> </tr> <tr> <td>I</td> <td>1799</td> <td>-</td> <td>-</td> <td>/1016</td> <td>/429</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 I Brg Wid = 3.5 Min Req = 1.8 Bearings B & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs)</p> <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>1817 -4574</td> <td>F - G</td> <td>1048 -2249</td> </tr> <tr> <td>C - D</td> <td>1579 -3725</td> <td>G - H</td> <td>1140 -2803</td> </tr> <tr> <td>D - E</td> <td>1340 -2975</td> <td>H - I</td> <td>1216 -3162</td> </tr> <tr> <td>E - F</td> <td>1060 -2210</td> <td></td> <td></td> </tr> </tbody> </table> <p>Maximum Bot Chord Forces Per Ply (lbs)</p> <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - O</td> <td>4273 -1694</td> <td>L - K</td> <td>2330 -792</td> </tr> <tr> <td>O - N</td> <td>4268 -1696</td> <td>K - J</td> <td>2680 -1000</td> </tr> <tr> <td>N - M</td> <td>3446 -1358</td> <td>J - I</td> <td>2681 -998</td> </tr> <tr> <td>M - L</td> <td>2733 -1009</td> <td></td> <td></td> </tr> </tbody> </table> <p>Maximum Web Forces Per Ply (lbs)</p> <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Webs</th> <th>Tens.Comp.</th> <th>Webs</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>C - N</td> <td>428 -858</td> <td>E - L</td> <td>802 -1489</td> </tr> <tr> <td>N - D</td> <td>499 -118</td> <td>F - L</td> <td>1766 -846</td> </tr> <tr> <td>D - M</td> <td>460 -930</td> <td>L - G</td> <td>354 -657</td> </tr> <tr> <td>M - E</td> <td>735 -279</td> <td>G - K</td> <td>383 -38</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	1868	-	-	/1184	/468	/290	I	1799	-	-	/1016	/429	-	Chords	Tens.Comp.	Chords	Tens. Comp.	B - C	1817 -4574	F - G	1048 -2249	C - D	1579 -3725	G - H	1140 -2803	D - E	1340 -2975	H - I	1216 -3162	E - F	1060 -2210			Chords	Tens.Comp.	Chords	Tens. Comp.	B - O	4273 -1694	L - K	2330 -792	O - N	4268 -1696	K - J	2680 -1000	N - M	3446 -1358	J - I	2681 -998	M - L	2733 -1009			Webs	Tens.Comp.	Webs	Tens. Comp.	C - N	428 -858	E - L	802 -1489	N - D	499 -118	F - L	1766 -846	D - M	460 -930	L - G	354 -657	M - E	735 -279	G - K	383 -38
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Lumber

Top chord: 2x4 SP #2; T1 2x4 SP M-31;
 Bot chord: 2x4 SP #2; B1 2x4 SP M-31;
 Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 10'-5-2".

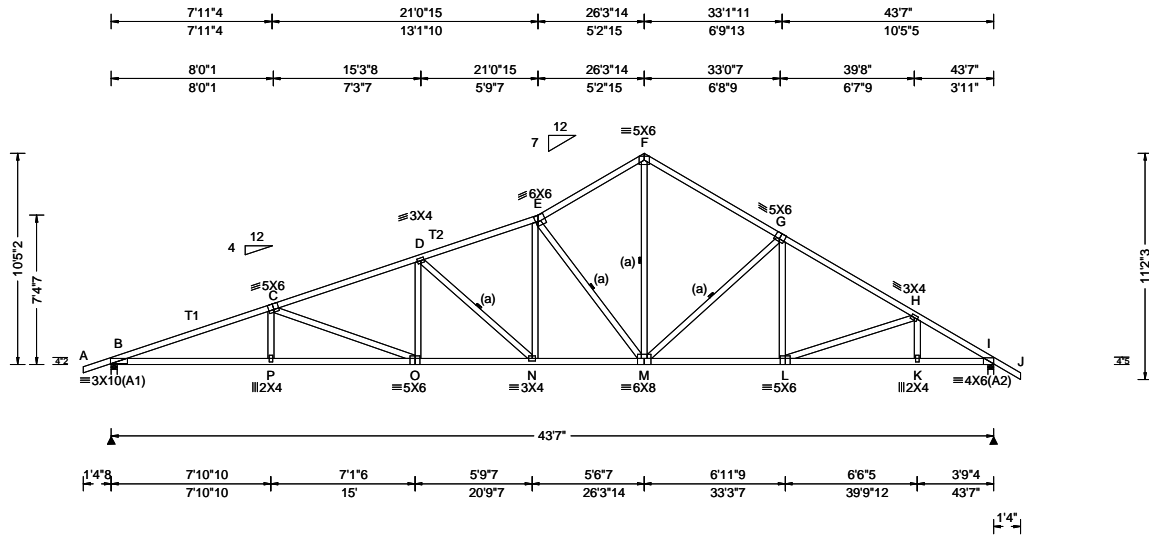


COA #0278

12/07/2021

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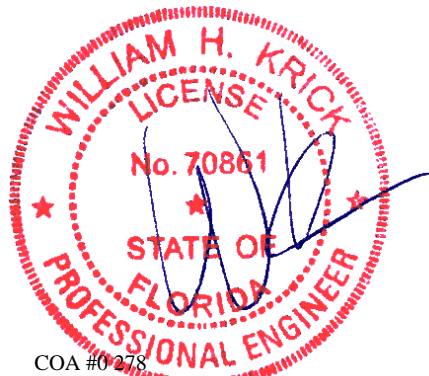
Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 4.36 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.276 O 999 240 VERT(CL): 0.560 O 928 180 HORZ(LL): 0.083 I - - HORZ(TL): 0.168 I - - Creep Factor: 2.0 Max TC CSI: 0.625 Max BC CSI: 0.449 Max Web CSI: 0.987 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 1866 /- /- /1184 /467 /305 I 1891 /- /- /1094 /454 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 I Brg Wid = 3.5 Min Req = 1.5 Bearings B & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.					
				Lumber Top chord: 2x4 SP #2; T1,T2 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP #3;	B - C 1812 -4566 F - G 1044 -2244 C - D 1575 -3721 G - H 1129 -2793 D - E 1335 -2969 H - I 1181 -3123 E - F 1055 -2205				

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes
 The overall height of this truss excluding overhang is 105-2.

Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.			
B - P	4265	-1637	M - L 2321 -731
P - O	4260	-1639	L - K 2645 -914
O - N	3442	-1302	K - I 2645 -912
N - M	2727	-951	
Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp.			
C - O	428	-854	E - M 800 -1487
O - D	500	-118	F - M 1761 -840
D - N	462	-933	M - G 350 -651
N - E	735	-280	G - L 380 -34



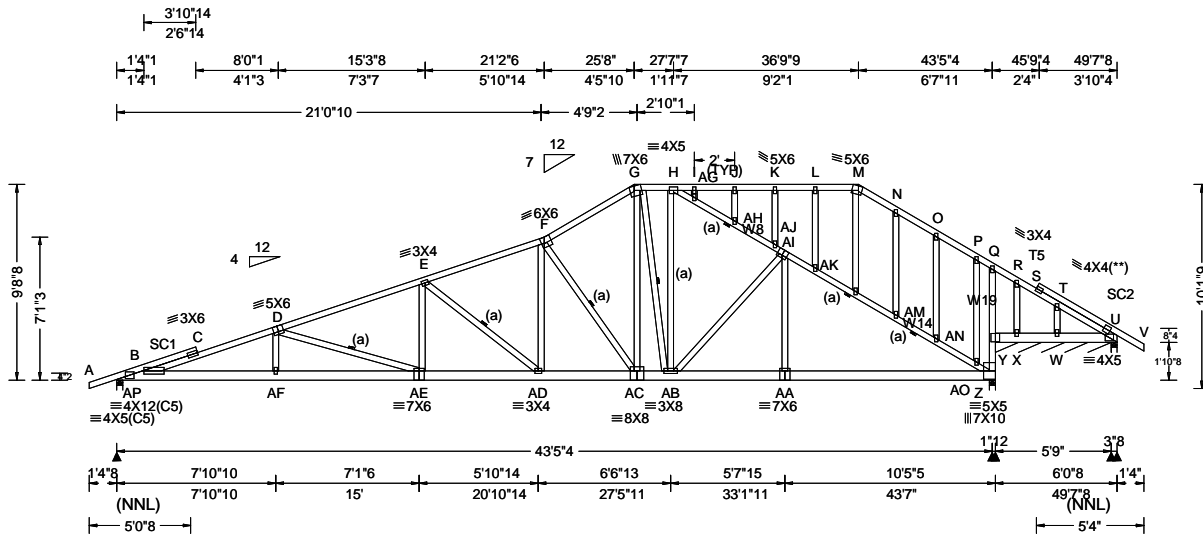
COA #0278
 12/07/2021

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Loading Criteria (psf)

TCLL: 20.00
 TC DL: 10.00
 BC LL: 0.00
 BC DL: 10.00
 Des Ld: 40.00
 NCBC LL: 10.00
 Soffit: 2.00
 Load Duration: 1.25
 Spacing: 24.0 "

Wind Criteria

Wind Std: ASCE 7-16
 Speed: 130 mph
 Enclosure: Closed
 Risk Category: II
 EXP: C Kzt: NA
 Mean Height: 15.00 ft
 TC DL: 4.2 psf
 BC DL: 3.0 psf
 MWFRS Parallel Dist: 0 to h/2
 C&C Dist a: 4.96 ft
 Loc. from endwall: Any
 GCp: 0.18
 Wind Duration: 1.60

Snow Criteria (Pg, Pf in PSF)

Pg: NA Ct: NA CAT: NA
 Pf: NA Ce: NA
 Lu: NA Cs: NA
 Snow Duration: NA

Building Code:
 FBC 7th Ed. 2020 Res.
 TPI Std: 2014
 Rep Fac: Yes
 FT/RT: 20(0)/10(0)
 Plate Type(s):
 WAVE

Defl/CSI Criteria

PP Deflection in loc L/defl L/#
 VERT(LL): 0.287 C 999 240
 VERT(CL): 0.581 C 888 180
 HORZ(LL): 0.106 R - -
 HORZ(TL): 0.214 R - -
 Creep Factor: 2.0
 Max TC CSI: 0.795
 Max BC CSI: 0.545
 Max Web CSI: 0.973
 VIEW Ver: 21.01.01A.0521.20

▲ Maximum Reactions (lbs), or * = PLF

Loc	Gravity			Non-Gravity		
	R+	/R-	/Rh	/Rw	/U	/RL
AP	1885	-	-	/1066	/9	/139
Z	693	-/642	-	/321	/342	-
U*	443	-	-	/182	-	-
U	444	-	-	/262	/76	-
X	-/207					

Wind reactions based on MWFRS
 AP Brg Wid = 3.5 Min Req = 1.6
 Z Brg Wid = 3.5 Min Req = 1.5
 U Brg Wid = 69.0 Min Req = -
 U Brg Wid = 3.5 Min Req = 1.5

Bearings AP, Z, Y, & U are a rigid surface.
 Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP M-31; T5 2x4 SP #2;
 Bot chord: 2x6 SP 2400f-2.0E;
 Webs: 2x4 SP #3; W8 2x4 SP #2; W14,
 W19 2x4 SP M-31;
 Stack Chord: SC1 2x4 SP #2;
 Stack Chord: SC2 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

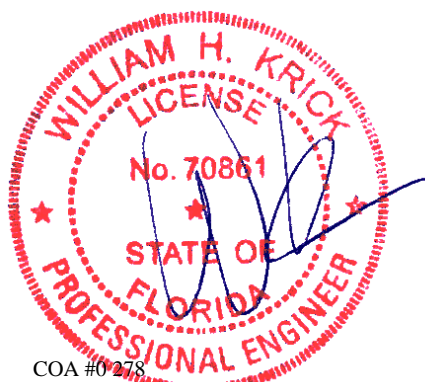
All plates are 2X4 except as noted.
 (**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.



COA #0 278

12/07/2021

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - C	365 - 5309	F - G	623 - 2404
C - D	353 - 5237	G - H	618 - 1963
D - E	414 - 4015	P - Q	338 - 483
E - F	537 - 3136	S - T	159 - 385

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - AF	4992 - 219	AC - AB	2008 - 348
AF - AE	4981 - 220	AB - AA	2269 - 486
AE - AD	3709 - 263	AA - Z	2271 - 485
AD - AC	2883 - 388		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
AF - D	384 0	AI - K	254 - 402
D - AE	0 - 1310	AI - AJ	446 - 2160
AE - E	569 0	AJ - AK	445 - 2335
E - AD	0 - 1033	AK - AM	473 - 2356
F - AC	72 - 1577	AM - AN	473 - 2381
AB - H	775 0	AN - AO	483 - 2413
AB - AJ	164 - 471	AO - Z	439 - 2309
H - AG	332 - 1994	Z - Y	1849 - 821
AG - AH	338 - 2001	Y - Q	244 - 752
AH - AI	322 - 1963		

Maximum Gable Forces Per Ply (lbs)

Gables	Tens.Comp.	Gables	Tens. Comp.
AD - F	789 0	G - AC	1245 - 45

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Additional Notes

Negative reaction(s) of -642# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

The overall height of this truss excluding overhang is 9-8-8.



COA #0278

12/07/2021

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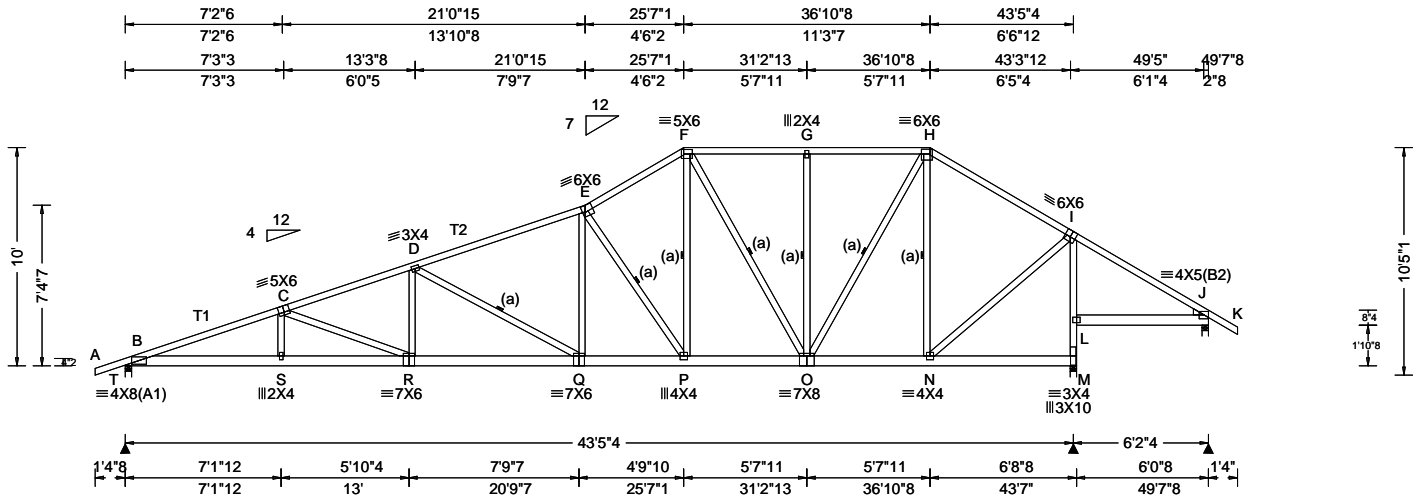
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Suite 305
Orlando FL, 32821

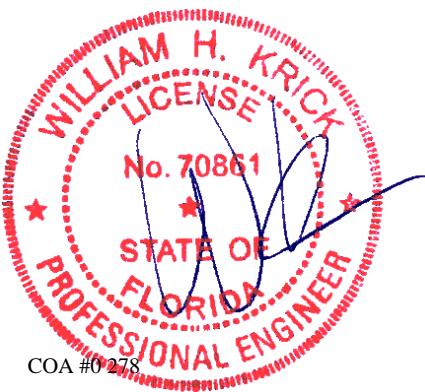


Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCDL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 4.96 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.220 R 999 240 VERT(CL): 0.446 R 999 180 HORZ(LL): 0.042 N - - HORZ(TL): 0.085 N - - Creep Factor: 2.0 Max TC CSI: 0.481 Max BC CSI: 0.326 Max Web CSI: 0.735 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity T 1850 - / - / /1223 /473 /293 M 2111 - / - / /1141 /483 - J 316 - / - / /255 /100 - Wind reactions based on MWFRS T Brg Wid = 3.5 Min Req = 1.5 M Brg Wid = 3.5 Min Req = 1.7 J Brg Wid = 3.5 Min Req = 1.5 Bearings T, M, & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.
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Lumber Top chord: 2x4 SP #2; T1,T2 2x4 SP M-31; Bot chord: 2x6 SP 2400f-2.0E; Webs: 2x4 SP #3; Rt Wedge: 2x4 SP #3;	▲ Maximum Reactions (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 2073 -4656 F - G 1026 -1589 C - D 1900 -3979 G - H 1026 -1589 D - E 1537 -2980 H - I 769 -1270 E - F 1310 -2287
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Bracing (a) Continuous lateral restraint equally spaced on member. Wind Wind loads based on MWFRS with additional C&C member design. Wind loading based on both gable and hip roof types.	Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - S 4365 -1991 Q - P 2724 -1262 S - R 4359 -1992 P - O 1906 -788 R - Q 3704 -1744 O - N 1007 -352
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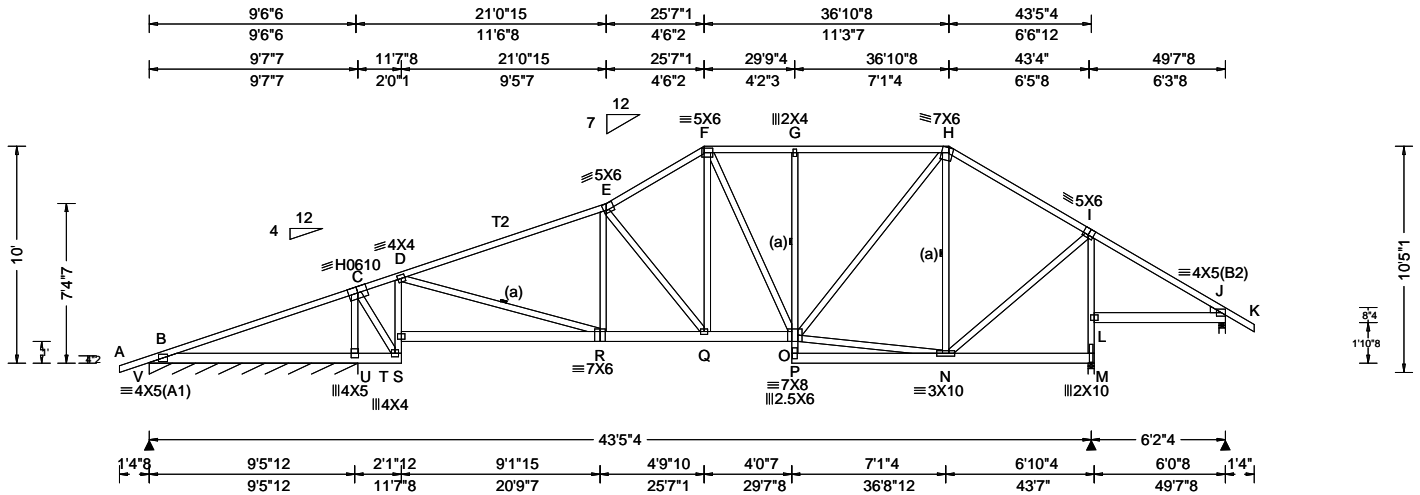
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COA #0278
12/07/2021

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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 4.96 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.069 E 999 240 VERT(CL): 0.139 E 999 180 HORZ(LL): 0.026 L - - HORZ(TL): 0.053 L - - Creep Factor: 2.0 Max TC CSI: 0.888 Max BC CSI: 0.234 Max Web CSI: 0.989 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL V* 241 /- /- /161 /62 /30 M 1597 /- /- /877 /349 /- J 337 /- /- /273 /113 /- Wind reactions based on MWFRS V Brg Wid = 115 Min Req = - M Brg Wid = 3.5 Min Req = 1.5 J Brg Wid = 3.5 Min Req = 1.5 Bearings V, M, & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 850 -447 F - G 835 -1197 D - E 881 -1649 G - H 835 -1196 E - F 909 -1455 H - I 611 -941
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Lumber

Top chord: 2x4 SP #2; T2 2x4 SP M-31;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;
Rt Wedge: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 3X4 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

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The overall height of this truss excluding overhang is 10'-0".

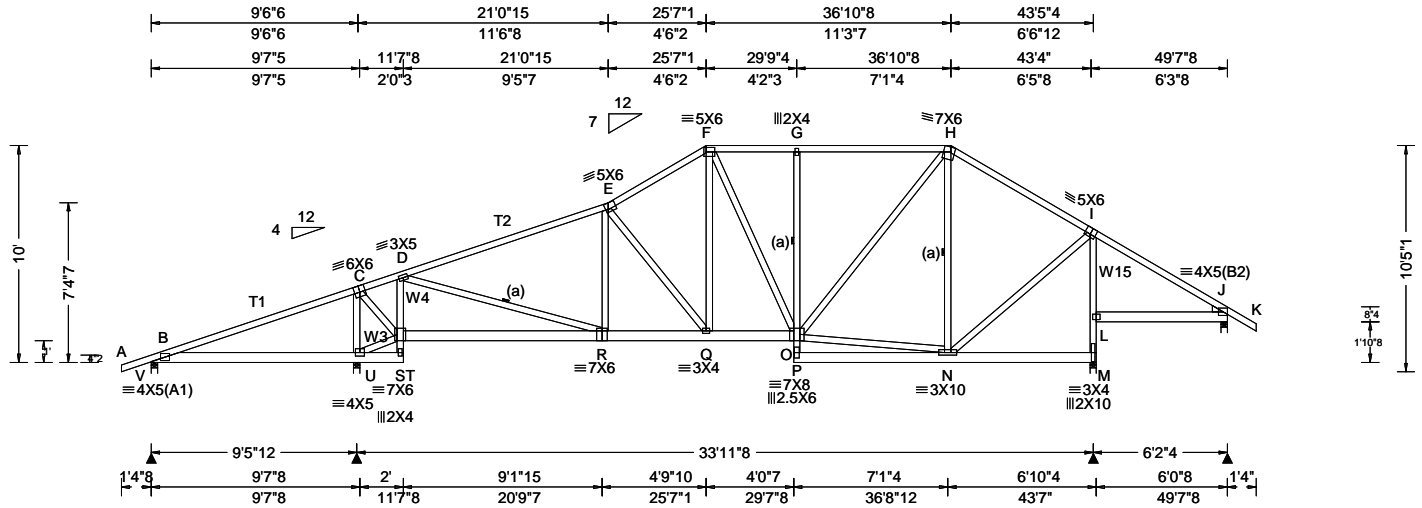


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Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 4.96 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.063 Q 999 240 VERT(CL): 0.125 Q 999 180 HORZ(LL): 0.017 L - - HORZ(TL): 0.034 L - - Creep Factor: 2.0 Max TC CSI: 0.561 Max BC CSI: 0.216 Max Web CSI: 0.899 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL V 355 - / - / - /155 /54 /293 U 2056 - / - / - /1372 /542 -/ M 1699 - / - / - /878 /352 -/ J 339 - / - / - /271 /111 -/ Wind reactions based on MWFRS V Brg Wid = 3.5 Min Req = 1.5 U Brg Wid = 3.5 Min Req = 1.5 M Brg Wid = 3.5 Min Req = 1.5 J Brg Wid = 3.5 Min Req = 1.5 Bearings V, U, M, & J are a rigid surface. Members not listed have forces less than 375#																									
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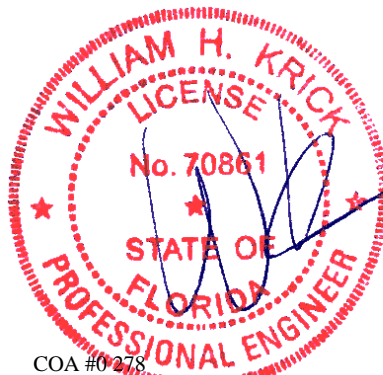
Lumber
 Top chord: 2x4 SP #2; T1,T2 2x4 SP M-31;
 Bot chord: 2x6 SP 2400f-2.0E;
 Webs: 2x4 SP #3; W3,W4,W15 2x4 SP M-31;
 Rt Wedge: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Loading
 Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes
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COA #0218

12/07/2021

Maximum Bot Chord Forces Per Ply (lbs)

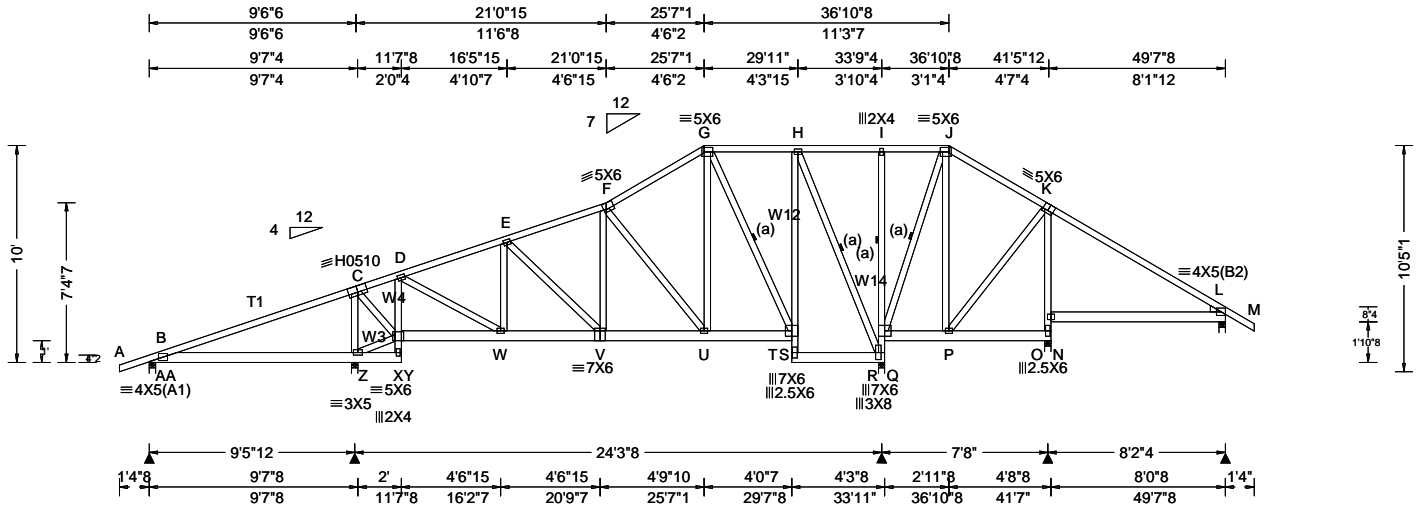
Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - U	311	-545	Q - O	1217	-437
R - Q	1563	-691			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - U	719	-1358	G - O	327	-381
C - S	1300	-636	O - N	717	-207
U - S	380	-626	O - H	769	-381
S - D	742	-1314	N - H	286	-583
D - R	1288	-573	N - I	990	-306
E - Q	422	-577	I - L	655	-1496
F - Q	496	-306	L - M	654	-1581

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Lumber

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 Bot chord: 2x6 SP 2400f-2.0E;
 Webs: 2x4 SP #3; W3,W12 2x4 SP #2; W4,
 W14 2x4 SP M-31;
 Rt Wedge: 2x4 SP #3;

Bracing

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Plating Notes

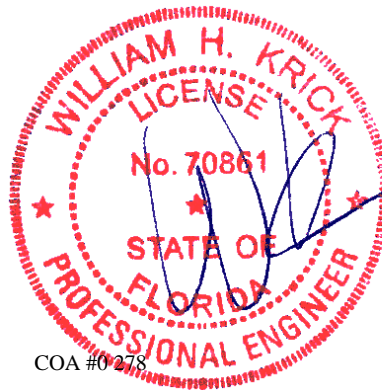
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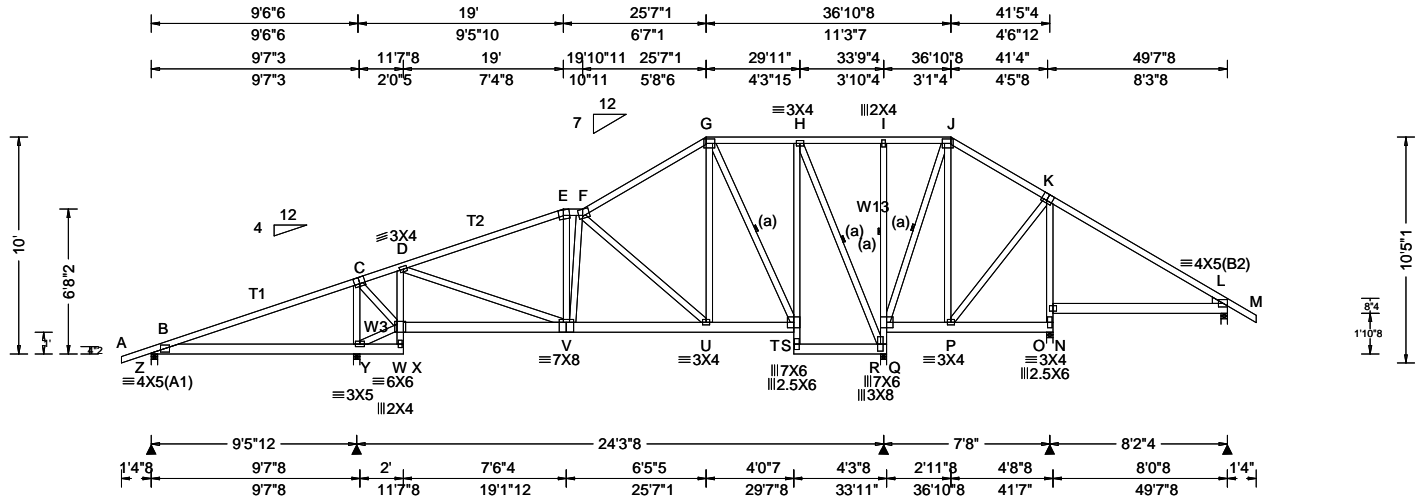
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 4.96 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.035 U 999 240 VERT(CL): 0.072 U 999 180 HORZ(LL): 0.041 R - - HORZ(TL): 0.086 R - - Creep Factor: 2.0 Max TC CSI: 0.597 Max BC CSI: 0.207 Max Web CSI: 0.774 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Z 376 - / - / - /178 /72 /294 Y 1551 - / - / - /1063 /443 -/ R 1509 - / - / - /924 /248 -/ O 555 - / - / - /394 /137 -/ L 430 - / - / - /347 /156 -/ Non-Gravity Wind reactions based on MWFRS Z Brg Wid = 3.5 Min Req = 1.5 Y Brg Wid = 3.5 Min Req = 1.5 R Brg Wid = 3.5 Min Req = 1.5 O Brg Wid = 3.5 Min Req = 1.5 L Brg Wid = 3.5 Min Req = 1.5 Bearings Z, Y, R, O, & L are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 455 -248 F - G 529 -584 D - E 610 -907 G - H 427 -192 E - F 649 -788 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - Y 208 -399 U - S 428 -141 V - U 795 -416 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. C - Y 586 -933 G - S 207 -596 C - W 746 -417 S - H 618 -172 Y - W 256 -464 H - R 393 -903 W - D 509 -833 R - Q 245 -644 D - V 757 -356 K - N 183 -390 F - U 376 -501 N - O 191 -508 G - U 467 -224
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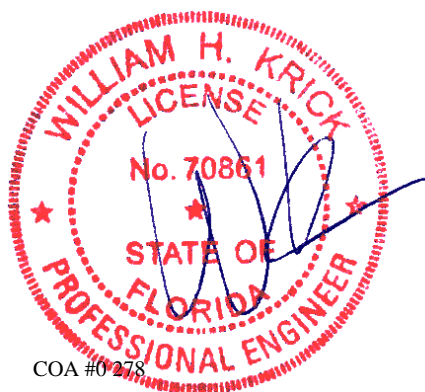
Lumber
Top chord: 2x4 SP #2; T1,T2 2x4 SP M-31;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3; W3 2x4 SP #2; W13 2x4 SP M-31;
Rt Wedge: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 5X6 except as noted.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

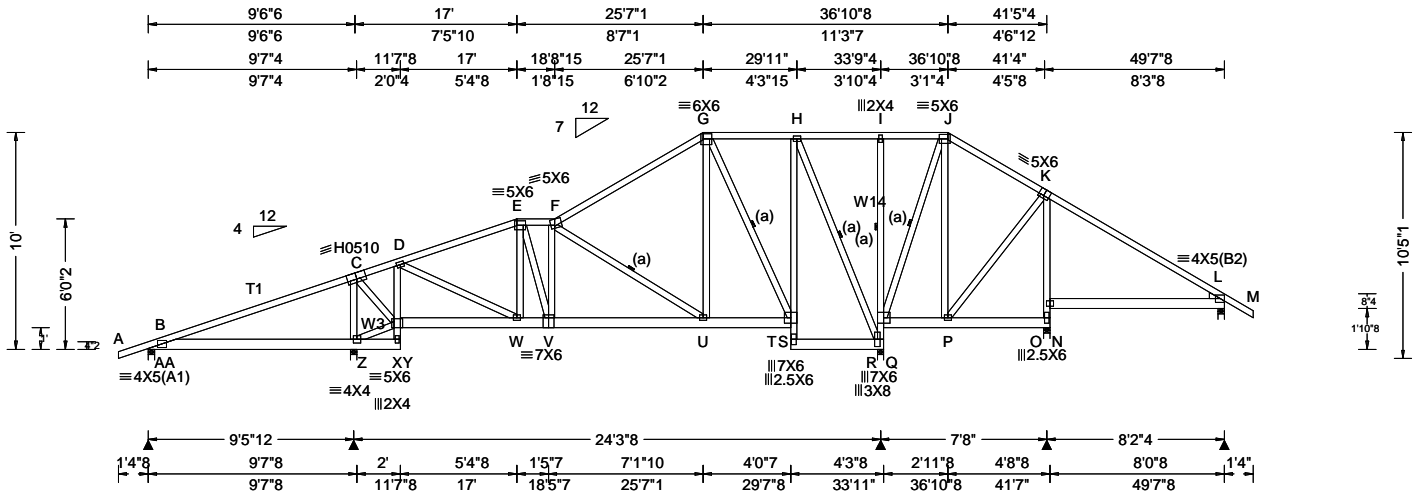
Additional Notes
WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.
The overall height of this truss excluding overhang is 10-0-0.



COA #0 278
12/07/2021

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Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 4.96 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.036 U 999 240 VERT(CL): 0.073 U 999 180 HORZ(LL): 0.040 R - - HORZ(TL): 0.083 R - - Creep Factor: 2.0 Max TC CSI: 0.670 Max BC CSI: 0.210 Max Web CSI: 0.747 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs)																																																																																																													
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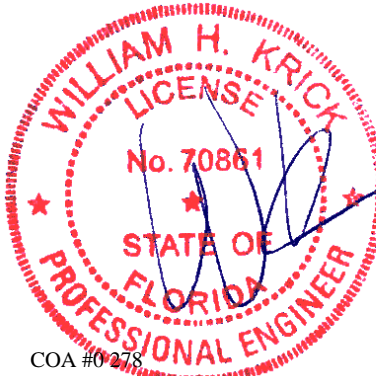
Lumber
 Top chord: 2x4 SP #2; T1 2x4 SP M-31;
 Bot chord: 2x6 SP 2400f-2.0E;
 Webs: 2x4 SP #3; W3 2x4 SP #2; W14 2x4 SP M-31;
 Rt Wedge: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Plating Notes
 All plates are 3X4 except as noted.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

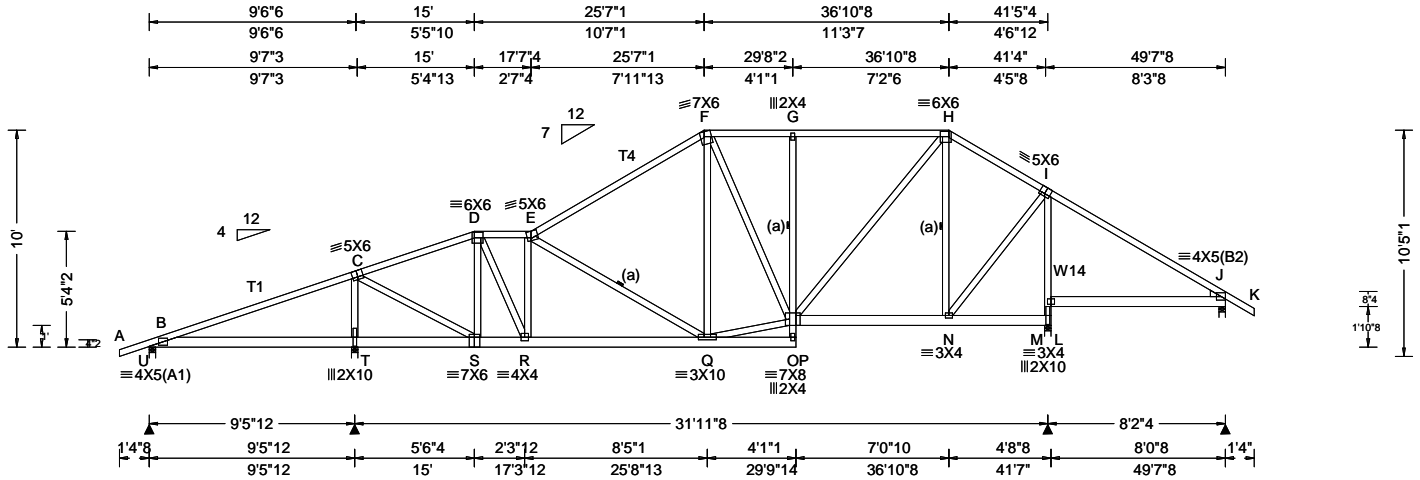
Additional Notes
 WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.
 The overall height of this truss excluding overhang is 10'-0".



COA #0218
 12/07/2021

****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
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 Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
 For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org





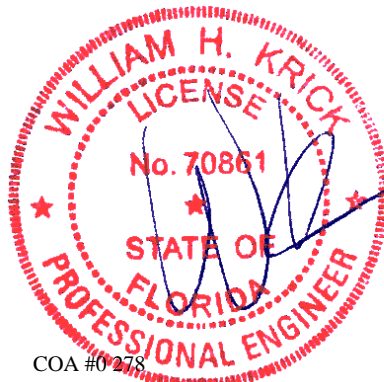
Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCCL: 0.00 BCCL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 4.2 psf BCCL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 4.96 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.051 G 999 240 VERT(CL): 0.105 G 999 180 HORZ(LL): 0.012 J - - HORZ(TL): 0.024 J - - Creep Factor: 2.0 Max TC CSI: 0.645 Max BC CSI: 0.197 Max Web CSI: 0.851 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL
				U 358 - / - / - /185 /59 /295 T 1941 - / - / - /1254 /513 -/ M 1621 - / - / - /860 /342 -/ J 423 - / - / - /334 /140 -/ Wind reactions based on MWFRS U Brg Wid = 3.5 Min Req = 1.5 T Brg Wid = 3.5 Min Req = 1.5 M Brg Wid = 3.5 Min Req = 1.5 J Brg Wid = 3.5 Min Req = 1.5 Bearings U, T, M, & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber
 Top chord: 2x4 SP #2; T1,T4 2x4 SP M-31;
 Bot chord: 2x6 SP 2400f-2.0E;
 Webs: 2x4 SP #3; W14 2x4 SP #2;
 Rt Wedge: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes
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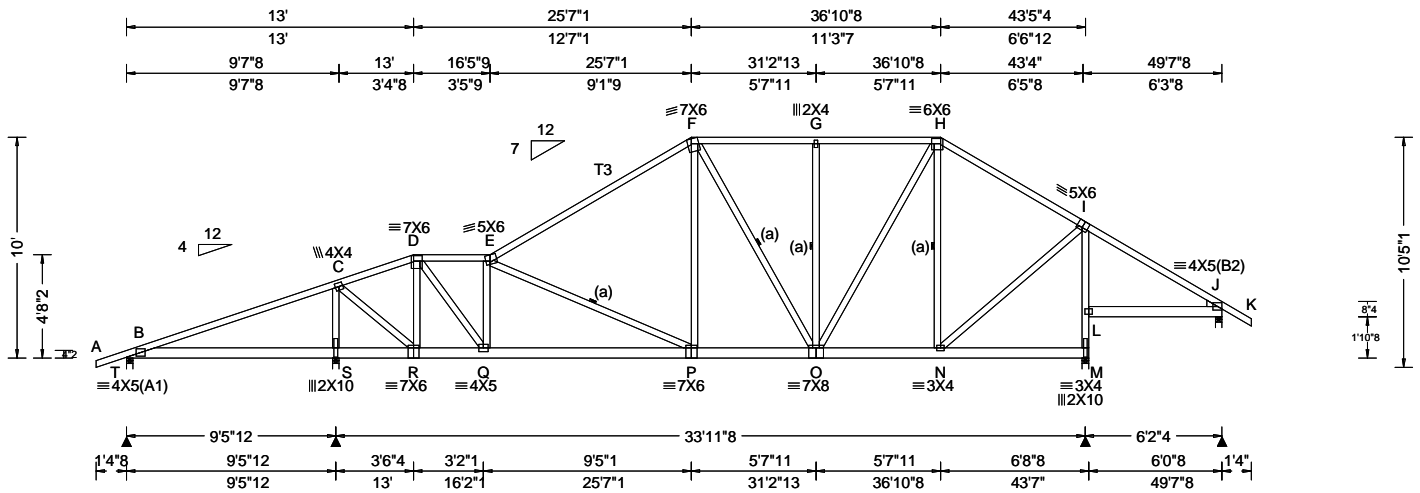


COA #0 278
 12/07/2021

Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.			
B - T	244	-447	R - Q 1361 -599
T - S	217	-399	O - N 566 -150
S - R	899	-392	
Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp.			
T - C	964	-1635	Q - O 1036 -330
C - S	1501	-701	O - H 780 -366
D - S	384	-747	H - N 270 -630
D - R	1024	-466	N - I 960 -280
R - E	474	-797	I - L 621 -1470
E - Q	314	-413	L - M 630 -1592

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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 4.96 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg, Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.055 P 999 240 VERT(CL): 0.112 P 999 180 HORZ(LL): 0.013 D - - HORZ(TL): 0.026 D - - Creep Factor: 2.0 Max TC CSI: 0.853 Max BC CSI: 0.211 Max Web CSI: 0.670 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>T</td> <td>331</td> <td>/-1</td> <td>/-</td> <td>/162</td> <td>/51</td> <td>/296</td> </tr> <tr> <td>S</td> <td>2085</td> <td>/-</td> <td>/-</td> <td>/1317</td> <td>/543</td> <td>/-</td> </tr> <tr> <td>M</td> <td>1615</td> <td>/-</td> <td>/-</td> <td>/872</td> <td>/350</td> <td>/-</td> </tr> <tr> <td>J</td> <td>342</td> <td>/-</td> <td>/-</td> <td>/275</td> <td>/114</td> <td>/-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS T Brg Wid = 3.5 Min Req = 1.5 S Brg Wid = 3.5 Min Req = 1.5 M Brg Wid = 3.5 Min Req = 1.5 J Brg Wid = 3.5 Min Req = 1.5 Bearings T, S, M, & J are a rigid surface. Members not listed have forces less than 375#</p>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	T	331	/-1	/-	/162	/51	/296	S	2085	/-	/-	/1317	/543	/-	M	1615	/-	/-	/872	/350	/-	J	342	/-	/-	/275	/114	/-
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J	342	/-	/-	/275	/114	/-																																							

Lumber
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 Bot chord: 2x6 SP 2400f-2.0E;
 Webs: 2x4 SP #3;
 Rt Wedge: 2x4 SP #3;

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 (a) Continuous lateral restraint equally spaced on member.

Wind
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 Wind loading based on both gable and hip roof types.

Additional Notes
 WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.
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Maximum Top Chord Forces Per Ply (lbs)

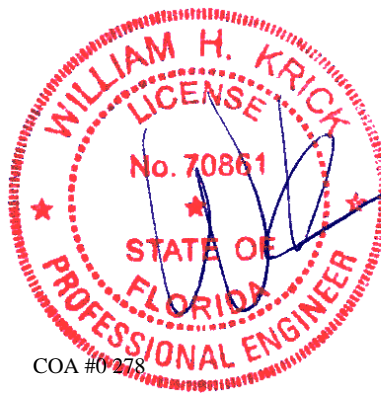
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B - C	661 -329	F - G	728 -1045
C - D	349 -624	G - H	728 -1045
D - E	723 -1393	H - I	594 -953
E - F	753 -1465		

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - S	292 -573	Q - P	1461 -616
S - R	264 -522	P - O	1141 -372
R - Q	546 -200	O - N	731 -210

Maximum Web Forces Per Ply (lbs)

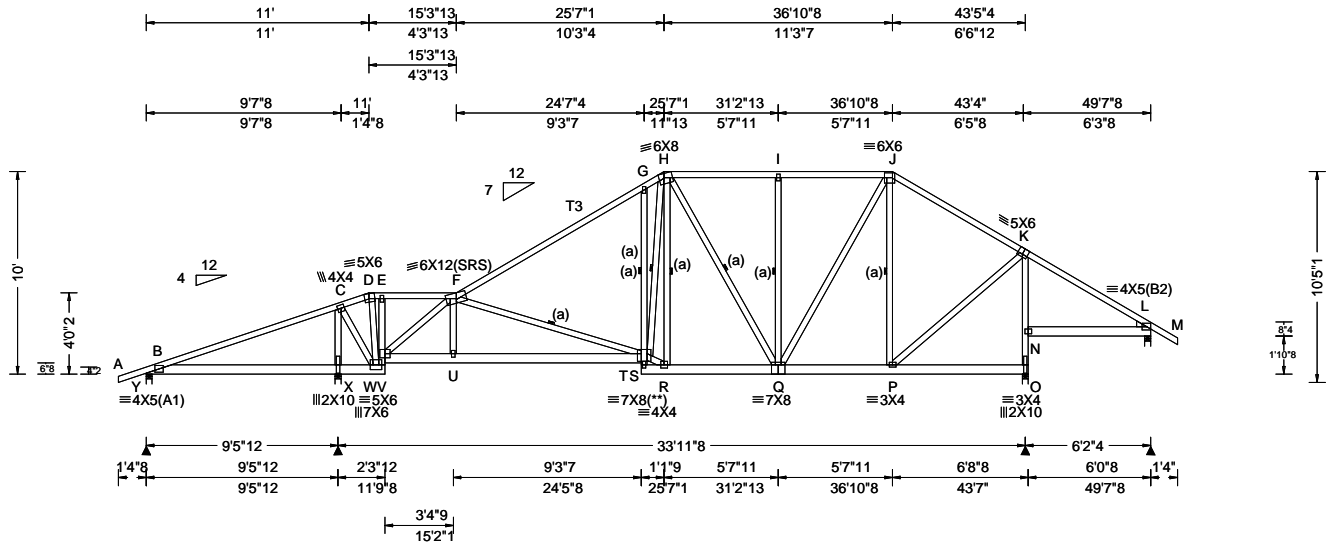
Webs	Tens.Comp.	Webs	Tens. Comp.
S - C	1005 -1739	O - H	621 -291
C - R	1453 -616	H - N	226 -494
D - R	456 -1034	N - I	962 -281
D - Q	1458 -645	I - L	631 -1479
Q - E	580 -1021	L - M	632 -1561
F - P	438 -74		



COA #0278
 12/07/2021

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 For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org





Loading Criteria (psf)	
TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00
Load Duration:	1.25
Spacing:	24.0 "

Wind Criteria	
Wind Std:	ASCE 7-16
Speed:	130 mph
Enclosure:	Closed
Risk Category:	II
EXP:	C Kzt: NA
Mean Height:	15.00 ft
TCDL:	4.2 psf
BCDL:	3.0 psf
MWFRS Parallel Dist:	0 to h/2
C&C Dist a:	4.96 ft
Loc. from endwall:	Any
GCpi:	0.18
Wind Duration:	1.60

Snow Criteria (Pg, Pf in PSF)	
Pg:	NA Ct: NA CAT: NA
Pf:	NA Ce: NA
Lu:	NA Cs: NA
Snow Duration:	NA
Building Code:	
FBC 7th Ed. 2020 Res.	
TPI Std: 2014	
Rep Fac: Yes	
FT/RT: 20(0)/10(0)	
Plate Type(s):	
WAVE	

Defl/CSI Criteria	
PP Deflection in loc L/defl L/#	
VERT(LL):	0.075 G 999 240
VERT(CL):	0.154 G 999 180
HORZ(LL):	0.019 N - -
HORZ(TL):	0.042 N - -
Creep Factor: 2.0	
Max TC CSI: 0.935	
Max BC CSI: 0.233	
Max Web CSI: 0.921	
VIEW Ver: 21.01.01A.0521.20	

▲ Maximum Reactions (lbs)						
Gravity			Non-Gravity			
Loc	R+ / R-	/ Rh	/ Rw	/ U	/ RL	
Y	306	-/97	-/	/85	/62	/297
X	2191	-/	-/	/1401	/553	-/
O	1597	-/	-/	/874	/349	-/
L	339	-/	-/	/274	/112	-/
Wind reactions based on MWFRS						
Y	Brg Wid = 3.5	Min Req = 1.5				
X	Brg Wid = 3.5	Min Req = 1.5				
O	Brg Wid = 3.5	Min Req = 1.5				
L	Brg Wid = 3.5	Min Req = 1.5				
Bearings Y, X, O, & L are a rigid surface.						
Members not listed have forces less than 375#						

Lumber

Top chord: 2x4 SP #2; T3 2x4 SP M-31;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;
Rt Wedge: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

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The overall height of this truss excluding overhang is 10'-0".

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - C	922 -384	H - I	710 -1009
F - G	773 -1580	I - J	710 -1009
G - H	1017 -1584	J - K	582 -930

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - X	358 -844	U - S	1466 -608
X - W	330 -792	R - Q	1090 -352
V - U	1469 -605	Q - P	713 -199

Maximum Web Forces Per Ply (lbs)

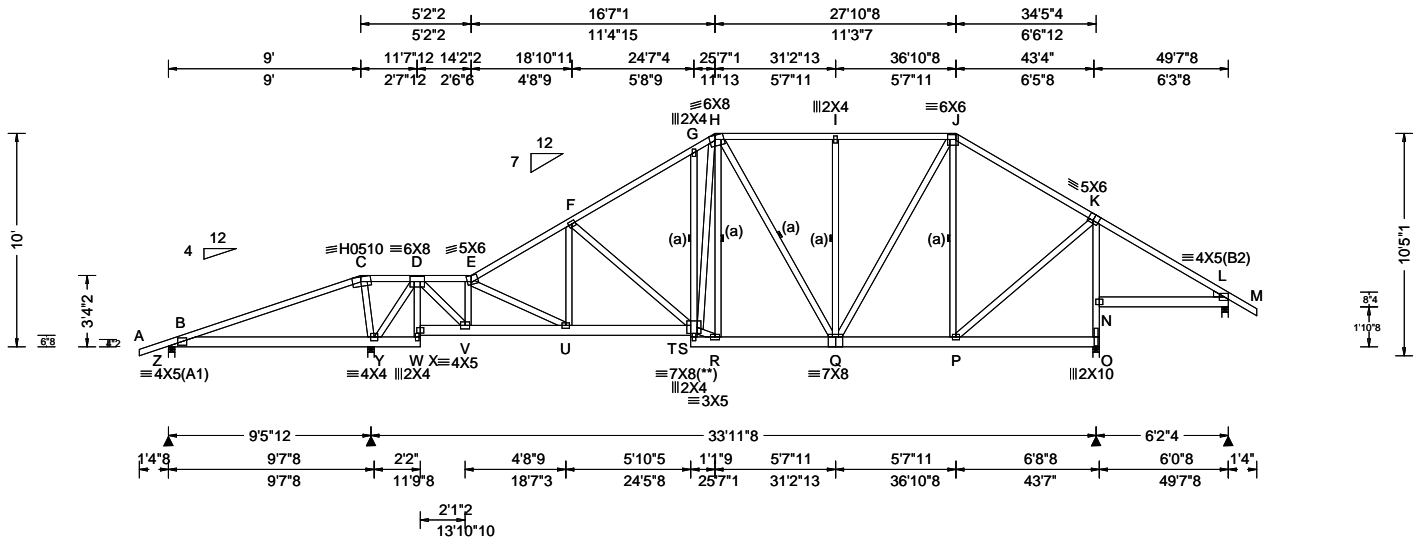
Webs	Tens.Comp.	Webs	Tens. Comp.
X - C	984 -1741	S - R	1288 -371
C - W	1328 -461	Q - J	591 -278
W - V	613 -1358	J - P	211 -471
V - F	862 -1988	P - K	946 -270
G - S	481 -603	K - N	627 -1461
S - H	1300 -648	N - O	628 -1544



COA #0278
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Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 4.96 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.070 G 999 240 VERT(CL): 0.143 G 999 180 HORZ(LL): 0.015 N - - HORZ(TL): 0.031 N - - Creep Factor: 2.0 Max TC CSI: 0.785 Max BC CSI: 0.240 Max Web CSI: 0.683 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs)																																				
				Gravity Non-Gravity <table border="1"> <thead> <tr> <th>Loc</th> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>Z</td> <td>295</td> <td>-122</td> <td>-</td> <td>/84</td> <td>/60</td> <td>/297</td> </tr> <tr> <td>Y</td> <td>2222</td> <td>-</td> <td>/0</td> <td>/1388</td> <td>/562</td> <td>/0</td> </tr> <tr> <td>O</td> <td>1590</td> <td>-</td> <td>-</td> <td>/865</td> <td>/347</td> <td>-</td> </tr> <tr> <td>L</td> <td>339</td> <td>-</td> <td>-</td> <td>/273</td> <td>/112</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS Z Brg Wid = 3.5 Min Req = 1.5 Y Brg Wid = 3.5 Min Req = 1.5 O Brg Wid = 3.5 Min Req = 1.5 L Brg Wid = 3.5 Min Req = 1.5 Bearings Z, Y, O, & L are a rigid surface. Members not listed have forces less than 375#						Loc	R+	/R-	/Rh	/Rw	/U	/RL	Z	295	-122	-	/84	/60	/297	Y	2222	-	/0	/1388	/562	/0	O	1590	-	-	/865	/347	-	L	339	-
Loc	R+	/R-	/Rh	/Rw	/U	/RL																																		
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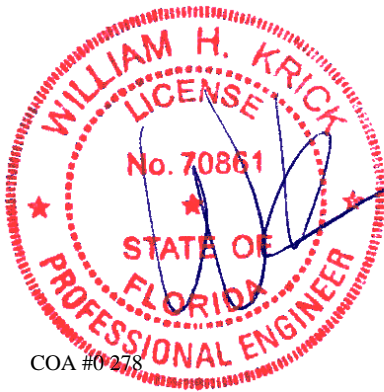
Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x6 SP 2400f-2.0E;
 Webs: 2x4 SP #3;
 Rt Wedge: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Plating Notes
 All plates are 3X4 except as noted.
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Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes
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COA #0278
 12/07/2021

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - C	982 -397	G - H	861 -1379
C - D	1007 -390	H - I	704 -1002
D - E	475 -992	I - J	704 -1002
E - F	766 -1682	J - K	577 -925
F - G	775 -1467		

Maximum Bot Chord Forces Per Ply (lbs)

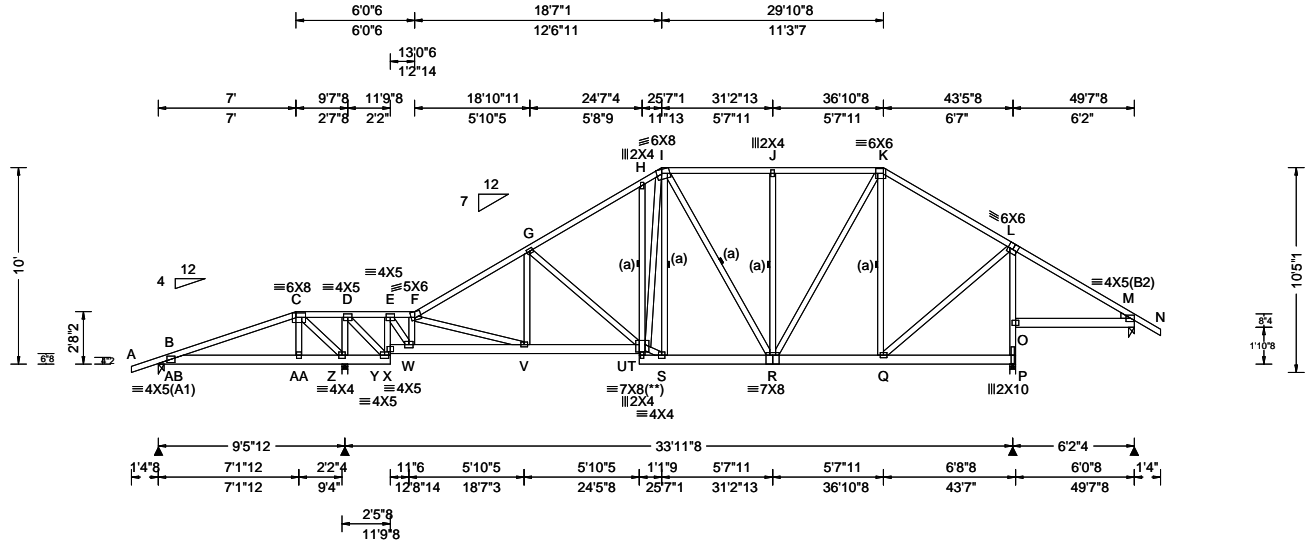
Chords	Tens.Comp.	Chords	Tens. Comp.
B - Y	756 -1810	R - Q	1071 -338
V - U	1132 -462	Q - P	709 -196
U - S	1408 -531		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
C - Y	526 -703	Q - J	587 -274
Y - D	700 -1616	J - P	209 -470
D - V	1634 -701	P - K	941 -265
V - E	565 -1190	K - N	622 -1455
S - R	1214 -364	N - O	623 -1538
S - H	963 -423		

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Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x6 SP 2400f-2.0E;
 Webs: 2x4 SP #3;
 Rt Wedge: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Special Loads
 -----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

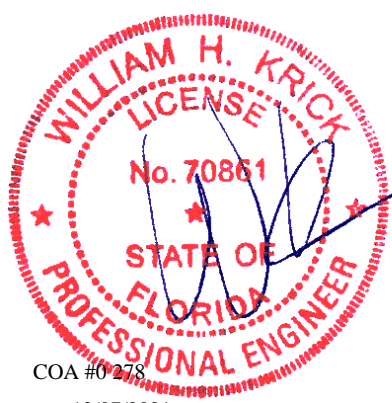
TC: From 61 plf at -1.38 to 61 plf at 7.00	TC: From 31 plf at 7.00 to 31 plf at 9.06
TC: From 61 plf at 9.06 to 61 plf at 13.03	TC: From 63 plf at 13.03 to 63 plf at 50.96
BC: From 4 plf at -1.38 to 4 plf at 0.00	BC: From 20 plf at 0.00 to 20 plf at 7.03
BC: From 10 plf at 7.03 to 10 plf at 9.06	BC: From 20 plf at 9.06 to 20 plf at 49.63
BC: From 5 plf at 49.63 to 5 plf at 50.96	
TC: 471 lb Conc. Load at 7.03	BC: 467 lb Conc. Load at 7.03
BC: 267 lb Conc. Load at 9.06	

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COA #0278
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Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B-AA	322	-583	V-T	1414	-342
AA-Z	361	-567	S-R	1079	-261
Z-Y	317	-1305	R-Q	717	-169
W-V	806	-192			

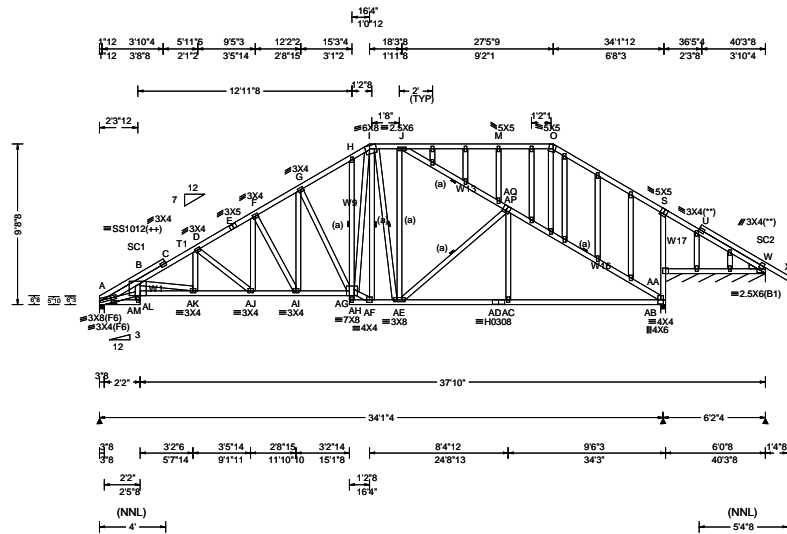
Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C-AA	618	-15	T-I	983	-248
C-Z	448	-1848	T-S	1250	-290
Z-D	381	-1445	I-S	138	-393
D-Y	1622	-389	R-K	593	-148
Y-X	347	-1386	K-Q	192	-476
X-E	318	-1322	Q-L	952	-221
E-W	1482	-351	L-O	425	-1466
W-F	367	-1301	O-P	413	-1549
F-V	646	-151			

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 Suite 305
 Orlando FL, 32821



Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 4.03 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, 18SS, HS	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.202 P 999 240 VERT(CL): 0.419 P 968 180 HORZ(LL): 0.141 T - - HORZ(TL): 0.292 T - - Creep Factor: 2.0 Max TC CSI: 0.608 Max BC CSI: 0.952 Max Web CSI: 0.768 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs), or *=PLF <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>1440</td> <td>-</td> <td>-</td> <td>/776</td> <td>/15</td> <td>/87</td> </tr> <tr> <td>AB</td> <td>526</td> <td>-454</td> <td>-</td> <td>/252</td> <td>/304</td> <td>-</td> </tr> <tr> <td>W*</td> <td>406</td> <td>-</td> <td>-</td> <td>/156</td> <td>-</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS A Brg Wid = 3.5 Min Req = 1.6 AB Brg Wid = 3.5 Min Req = 1.5 W Brg Wid = 72.5 Min Req = - Bearings A, AB, & AA are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>A - B</td> <td>204 -2060</td> <td>E - F</td> <td>342 -2357</td> </tr> <tr> <td>A - C</td> <td>190 -1657</td> <td>F - G</td> <td>356 -2084</td> </tr> <tr> <td>B - C</td> <td>138 -1357</td> <td>G - H</td> <td>398 -1795</td> </tr> <tr> <td>C - D</td> <td>333 -2891</td> <td>H - I</td> <td>392 -1725</td> </tr> <tr> <td>D - E</td> <td>336 -2427</td> <td>I - J</td> <td>401 -1312</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	1440	-	-	/776	/15	/87	AB	526	-454	-	/252	/304	-	W*	406	-	-	/156	-	-	Chords	Tens.Comp.	Chords	Tens. Comp.	A - B	204 -2060	E - F	342 -2357	A - C	190 -1657	F - G	356 -2084	B - C	138 -1357	G - H	398 -1795	C - D	333 -2891	H - I	392 -1725	D - E	336 -2427	I - J	401 -1312
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Lumber
 Top chord: 2x4 SP #2; T1 2x4 SP M-31;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3; W1,W9,W16 2x4 SP M-31; W13, W17 2x4 SP #2;
 Filler 2x4 SP #2;
 Stack Chord: SC1 2x4 SP #2;
 Stack Chord: SC2 2x4 SP #2;
 Rt Stub Wedge: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Plating Notes
 All plates are 2X4 except as noted.
 (++) - This plate works for both joints covered.
 (**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading
 Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Purlins
 Laterally brace BC at 24" oc in lieu of rigid ceiling.
 Laterally brace BC above filler at 24" oc.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes
 Negative reaction(s) of -454# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.
 See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
 Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.
 Shim all supports to solid bearing.

Maximum Bot Chord Forces Per Ply (lbs)

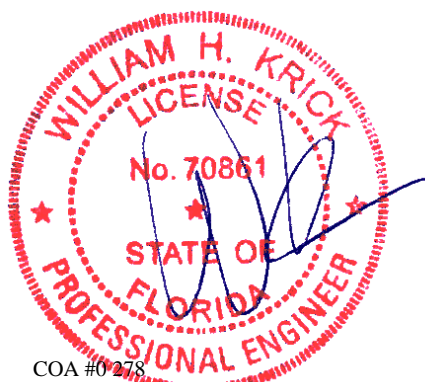
Chords	Tens.Comp.	Chords	Tens. Comp.
AL-AK	3512 -187	AF-AE	1312 -114
AK-AJ	2525 -75	AE-AD	1693 -301
AJ-AI	2013 -57	AD-AC	1693 -301
AI-AG	1759 -78	AC-AB	1696 -300

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
A - AL	2695 -134	AG - I	1424 0
A - AM	554 -36	AG-AF	1450 -132
AL - B	470 -11	AE-AQ	174 -512
B - AK	115 -983	J -AQ	263 -1474
D -AJ	23 -640	AQ-AC	375 0
AJ - F	467 0	AQ-AB	327 -1813
F - AI	0 -521	AB-AA	1496 -533
AI - G	439 0	AA - S	123 -469
G - AG	0 -569		

Maximum Gable Forces Per Ply (lbs)

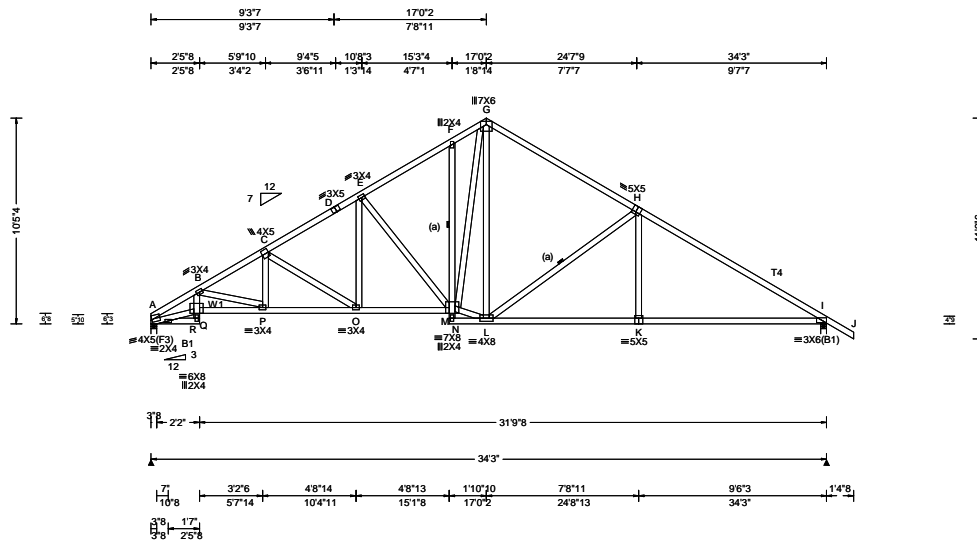
Gables	Tens.Comp.	Gables	Tens. Comp.
I - AF	55 -612	AP - M	247 -387
AE - J	407 0		



COA #0 278
 12/07/2021

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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.42 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.157 O 999 240 VERT(CL): 0.302 O 999 180 HORZ(LL): 0.091 I - - HORZ(TL): 0.175 I - - Creep Factor: 2.0 Max TC CSI: 0.844 Max BC CSI: 0.939 Max Web CSI: 0.716 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 1475 -/ - /818 /335 /289 I 1675 -/ - /909 /362 -/ Wind reactions based on MWFRS A Brg Wid = 3.5 Min Req = 1.5 I Brg Wid = 3.5 Min Req = 2.0 Bearings A & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 992 -3057 E - F 724 -1845 B - C 916 -2757 F - G 776 -1744 C - D 794 -2274 G - H 659 -1647 D - E 800 -2158 H - I 759 -2495
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Lumber
Top chord: 2x4 SP #2; T4 2x4 SP M-31;
Bot chord: 2x4 SP #2; B1 2x4 SP M-31;
Webs: 2x4 SP #3; W1 2x4 SP #2;
Filler: 2x4 SP #2;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins
Laterally brace BC at 24" oc in lieu of rigid ceiling.
Laterally brace BC above filler at 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

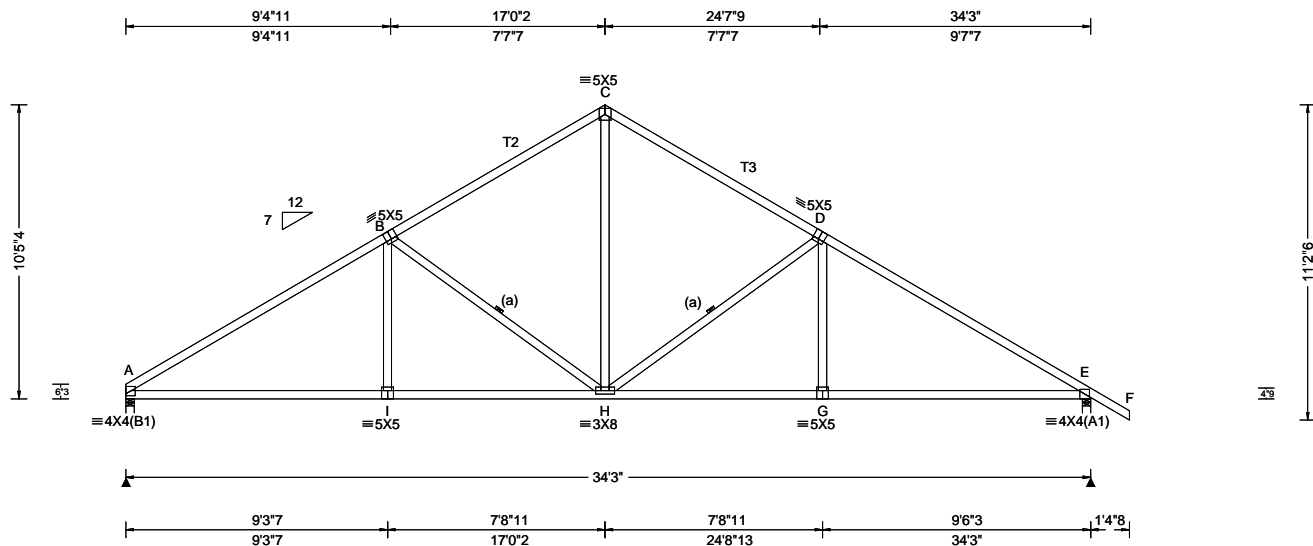
Maximum Bot Chord Forces Per Ply (lbs)			
Chords	Tens.Comp.	Chords	Tens. Comp.
Q - P	2758 -830	L - K	2030 -481
P - O	2365 -654	K - I	2035 -480
O - M	1888 -430		
Maximum Web Forces Per Ply (lbs)			
Webs	Tens.Comp.	Webs	Tens. Comp.
A - Q	1938 -577	E - M	303 -574
A - R	594 -182	M - G	1172 -406
B - P	184 -407	M - L	1375 -162
C - O	263 -555	L - H	397 -904
O - E	390 -94	H - K	387 0



COA #0278
12/07/2021

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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.42 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg, Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.084 H 999 240 VERT(CL): 0.174 H 999 180 HORZ(LL): 0.047 E - - HORZ(TL): 0.096 E - - Creep Factor: 2.0 Max TC CSI: 0.813 Max BC CSI: 0.895 Max Web CSI: 0.811 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>1420</td> <td>-</td> <td>-</td> <td>/825</td> <td>/333</td> <td>/289</td> </tr> <tr> <td>E</td> <td>1522</td> <td>-</td> <td>-</td> <td>/909</td> <td>/362</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS A Brg Wid = 3.5 Min Req = 1.7 E Brg Wid = 3.5 Min Req = 1.8 Bearings A & E are a rigid surface. Members not listed have forces less than 375#</p> Maximum Top Chord Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>A - B</td> <td>751 -2195</td> <td>C - D</td> <td>658 -1540</td> </tr> <tr> <td>B - C</td> <td>658 -1538</td> <td>D - E</td> <td>757 -2216</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	1420	-	-	/825	/333	/289	E	1522	-	-	/909	/362	-	Chords	Tens.Comp.	Chords	Tens. Comp.	A - B	751 -2195	C - D	658 -1540	B - C	658 -1538	D - E	757 -2216
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Lumber

Top chord: 2x4 SP M-31; T2,T3 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
A - I	1772 -473	H - G	1792 -479
I - H	1769 -474	G - E	1794 -478

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - H	385 -690	H - D	391 -718
C - H	978 -411		

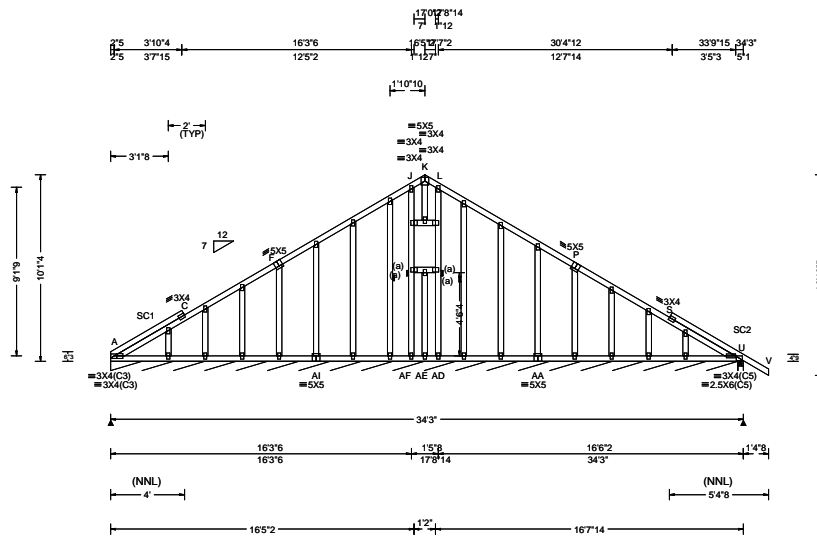


COA #0278
 12/07/2021

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SEQN: 56039 / FROM:	GABL Ply: 1 Qty: 1	Job Number: 21-6343 Mitchell Truss Label: A27	Cust: R 215 JRef: 1Xb52150003 T46 / DrwNo: 341.21.1202.20775 / YK 12/07/2021
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Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.42 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.003 C 999 240 VERT(CL): 0.006 C 999 180 HORZ(LL): 0.002 S - - HORZ(TL): 0.003 S - - Creep Factor: 2.0 Max TC CSI: 0.277 Max BC CSI: 0.083 Max Web CSI: 0.144 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs), or *=PLF <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A*</td> <td>78</td> <td>-</td> <td>-</td> <td>/42</td> <td>-</td> <td>/2</td> </tr> <tr> <td>U</td> <td>289</td> <td>-</td> <td>-</td> <td>/209</td> <td>/66</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS A Brg Wid = 407 Min Req = - U Brg Wid = 3.5 Min Req = 1.5 Bearings A & U are a rigid surface. Members not listed have forces less than 375#	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A*	78	-	-	/42	-	/2	U	289	-	-	/209	/66	-
Loc	Gravity			Non-Gravity																											
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U	289	-	-	/209	/66	-																									

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;
 Stack Chord: SC1 2x4 SP #2;
 Stack Chord: SC2 2x4 SP #2;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Plating Notes
 All plates are 2X4 except as noted.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

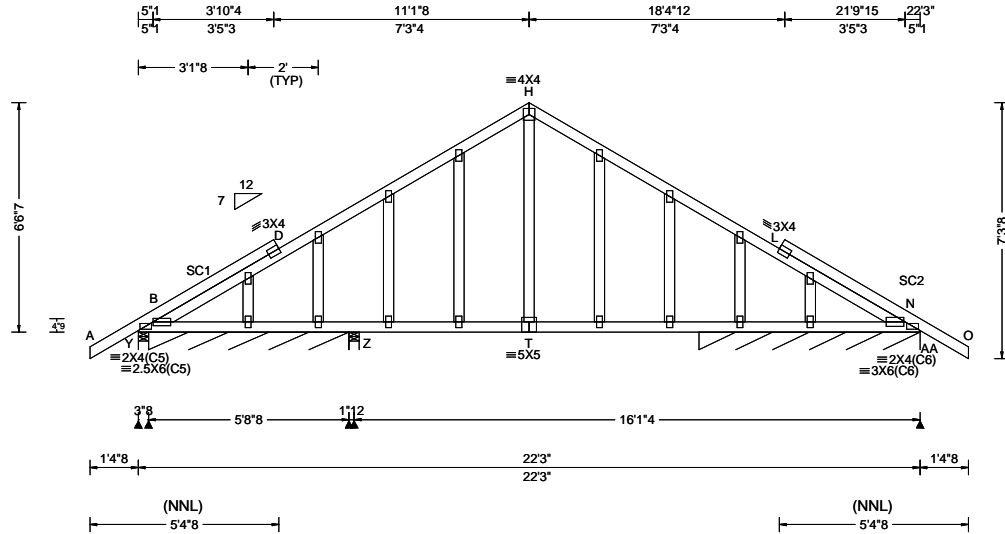
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.024 G 999 240 VERT(CL): 0.049 G 999 180 HORZ(LL): 0.011 G - - HORZ(TL): 0.022 G - - Creep Factor: 2.0 Max TC CSI: 0.256 Max BC CSI: 0.330 Max Web CSI: 0.067 VIEW Ver: 21.01.01A.0521.20	Gravity Loc R+ / R- / Rh / Rw / U / RL Y 484 /- /- /301 /11 /98 Y* 45 /- /- /23 /8 /- Z 324 /- /- /174 /- /- AA* 162 /- /- /97 /- /- Non-Gravity Wind reactions based on MWFRS Y Brg Wid = 3.5 Min Req = 1.5 Y Brg Wid = 68.5 Min Req = - Z Brg Wid = 3.5 Min Req = 1.5 AA Brg Wid = 75.5 Min Req = - Bearings Y, Y, Z, & R are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. D - H 86 -464 H - L 42 -465 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - T 724 0 T - N 724 0

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Plating Notes
All plates are 2X4 except as noted.

Loading
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes
See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

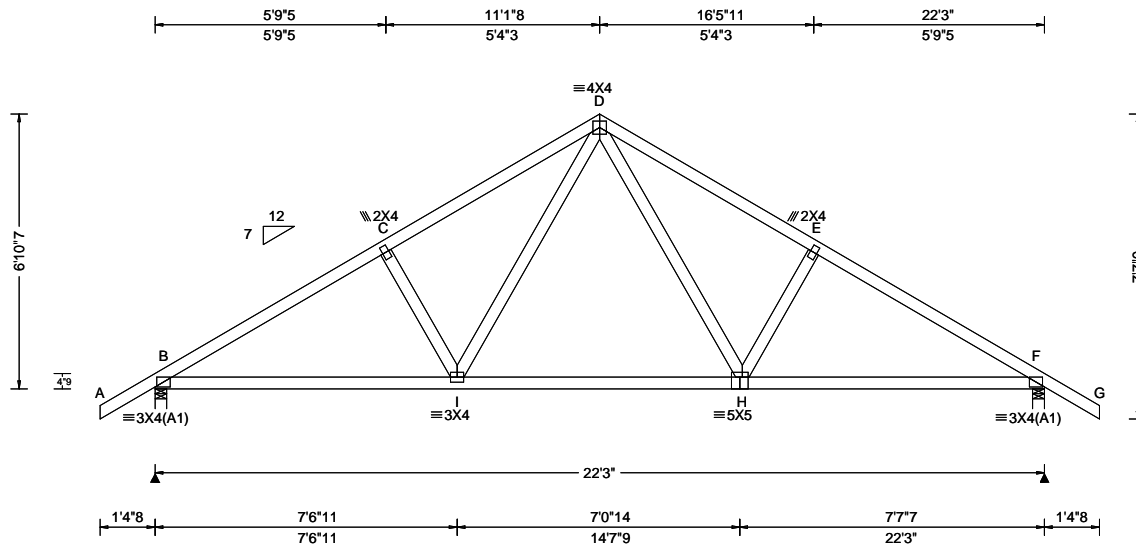


COA #0278
12/07/2021

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SEQN: 408811 FROM:	SPEC Ply: 1 Qty: 11	Job Number: 21-6343 Mitchell Truss Label: B02	Cust: R 215 JRef: 1Xb52150003 T35 DrwNo: 341.21.1238.30060 KD / WHK 12/07/2021
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Loading Criteria (psf) TCLL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.041 I 999 240 VERT(CL): 0.079 I 999 180 HORZ(LL): 0.020 F - - HORZ(TL): 0.039 F - - Creep Factor: 2.0 Max TC CSI: 0.124 Max BC CSI: 0.561 Max Web CSI: 0.226 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1074 /- /- /611 /242 /208 F 1073 /- /- /611 /242 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 F Brg Wid = 3.5 Min Req = 1.5 Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 542 - 1511 D - E 576 - 1347 C - D 576 - 1349 E - F 542 - 1509					
				Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - I 1230 - 331 H - F 1229 - 342 I - H 838 - 117 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. I - D 530 - 210 D - H 526 - 211					

Lumber

Top chord: 2x4 SP M-31;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 6-10-7.



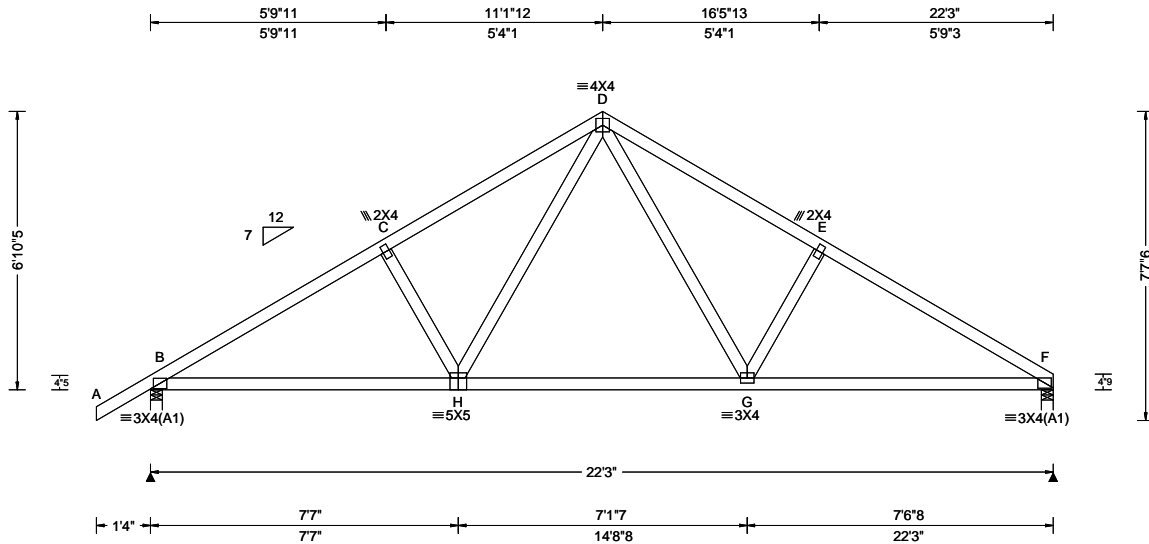
COA #0278

12/07/2021

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SEQN: 408814 FROM:	COMN Ply: 1 Qty: 3	Job Number: 21-6343 Mitchell Truss Label: B03	Cust: R 215 JRef: 1Xb52150003 T13 DrwNo: 341.21.1238.26440 KD / WHK 12/07/2021
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Loading Criteria (psf) TCLL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.048 H 999 240 VERT(CL): 0.093 H 999 180 HORZ(LL): 0.020 F - - HORZ(TL): 0.039 F - - Creep Factor: 2.0 Max TC CSI: 0.307 Max BC CSI: 0.553 Max Web CSI: 0.237 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>1074</td> <td>/-</td> <td>/-</td> <td>/608</td> <td>/243</td> <td>/191</td> </tr> <tr> <td>F</td> <td>978</td> <td>/-</td> <td>/-</td> <td>/531</td> <td>/215</td> <td>/-</td> </tr> </tbody> </table>						Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	1074	/-	/-	/608	/243	/191	F	978	/-	/-	/531	/215	/-
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Webs	Tens.Comp.	Webs	Tens. Comp.																																	
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Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

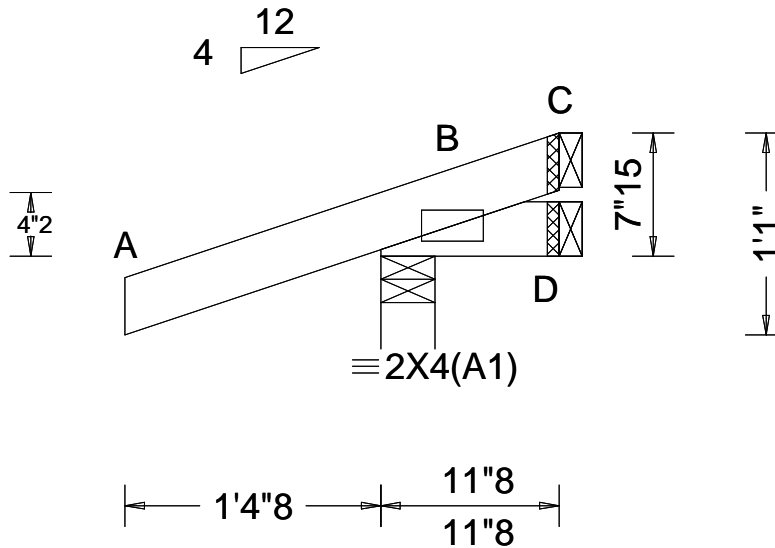
The overall height of this truss excluding overhang is 6-10-5.



COA #0278
 12/07/2021

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.000 D - - HORZ(TL): 0.000 D - - Creep Factor: 2.0 Max TC CSI: 0.223 Max BC CSI: 0.028 Max Web CSI: 0.000 VIEW Ver: 21.01.01A.0521.20	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 228 /- /- /169 /93 /26 D 3 /-15 /- /16 /15 /- C - /-45 /- /32 /39 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 D Brg Wid = 1.5 C Brg Wid = 1.5 Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

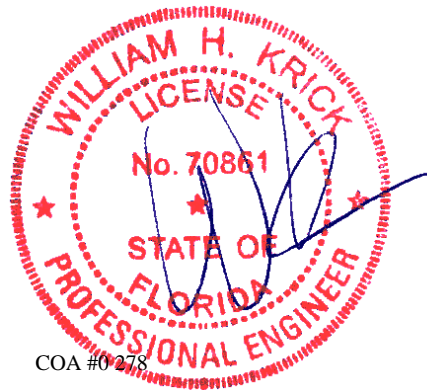
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

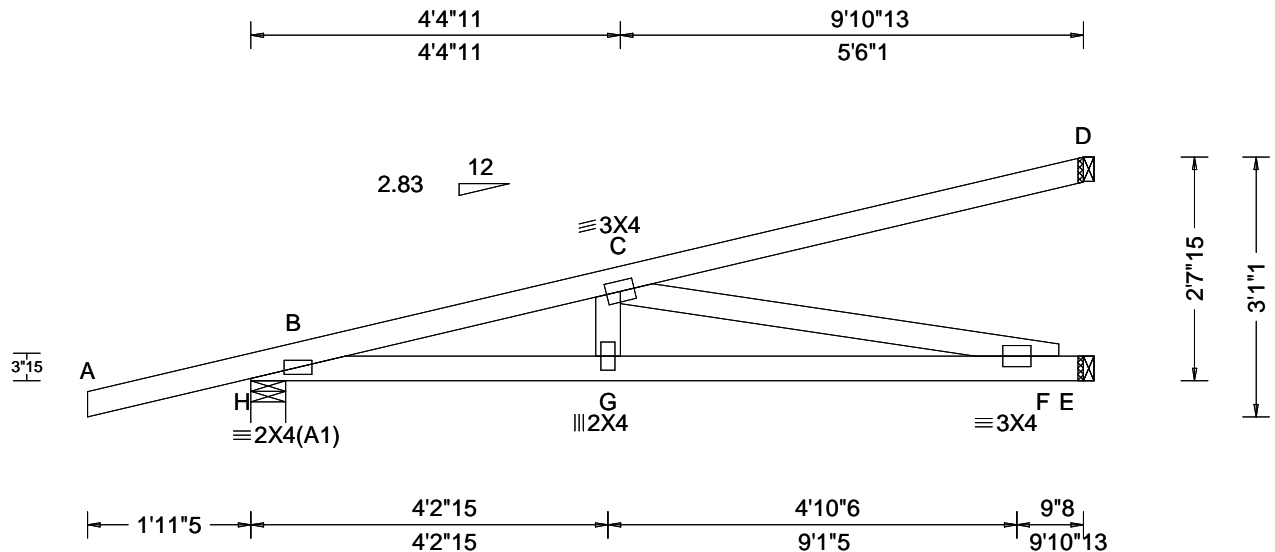
The overall height of this truss excluding overhang is 0'-7-15.



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Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.052 C 999 240 VERT(CL): 0.105 C 999 180 HORZ(LL): 0.009 F - - HORZ(TL): 0.019 F - - Creep Factor: 2.0 Max TC CSI: 0.890 Max BC CSI: 0.739 Max Web CSI: 0.752 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>H</td> <td>360</td> <td>-</td> <td>-</td> <td>-</td> <td>/95</td> <td>-</td> </tr> <tr> <td>E</td> <td>335</td> <td>-</td> <td>-</td> <td>-</td> <td>/34</td> <td>-</td> </tr> <tr> <td>D</td> <td>292</td> <td>-</td> <td>-</td> <td>-</td> <td>/124</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS H Brg Wid = 4.9 Min Req = 1.5 E Brg Wid = 1.5 D Brg Wid = 1.5 Bearing H is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>314 - 1258</td> </tr> </tbody> </table> Maximum Bot Chord Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - G</td> <td>1224 -307</td> <td>G - F</td> <td>1211 -313</td> </tr> </tbody> </table> Maximum Web Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Webs</th> <th>Tens.Comp.</th> </tr> </thead> <tbody> <tr> <td>C - F</td> <td>319 - 1236</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	H	360	-	-	-	/95	-	E	335	-	-	-	/34	-	D	292	-	-	-	/124	-	Chords	Tens.Comp.	B - C	314 - 1258	Chords	Tens.Comp.	Chords	Tens. Comp.	B - G	1224 -307	G - F	1211 -313	Webs	Tens.Comp.	C - F	319 - 1236
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C - F	319 - 1236																																																					

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Loading
 Hipjack supports 7-0-0 setback jacks with no webs.

Wind
 Wind loads and reactions based on MWFRS.
 Wind loading based on both gable and hip roof types.

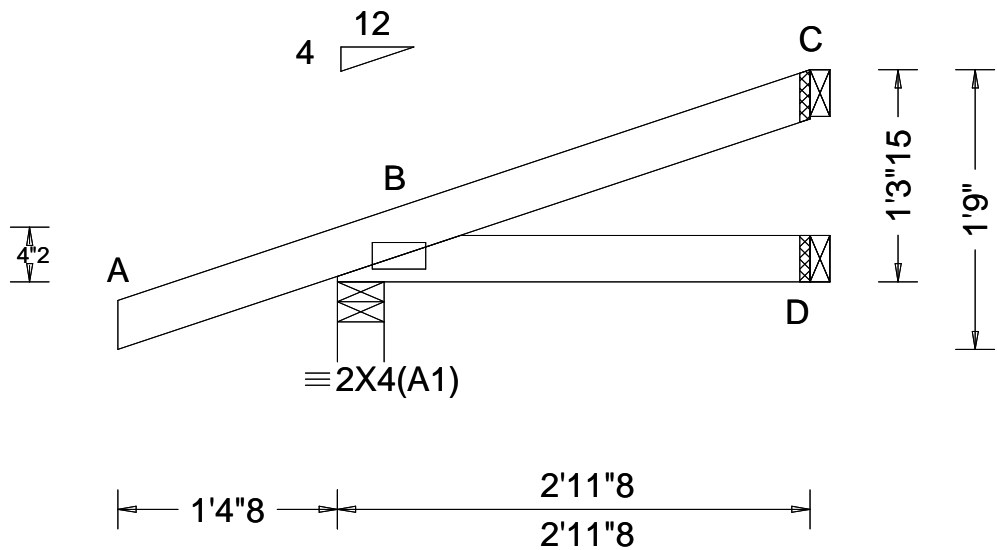
Additional Notes
 The overall height of this truss excluding overhang is 2'-7-15.



COA #0278
 12/07/2021

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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.001 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.225 Max BC CSI: 0.059 Max Web CSI: 0.000 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>243</td> <td>/-</td> <td>/-</td> <td>/168</td> <td>/65</td> <td>/47</td> </tr> <tr> <td>D</td> <td>48</td> <td>/-</td> <td>/-</td> <td>/26</td> <td>/-</td> <td>/-</td> </tr> <tr> <td>C</td> <td>61</td> <td>/-</td> <td>/-</td> <td>/32</td> <td>/30</td> <td>/-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	243	/-	/-	/168	/65	/47	D	48	/-	/-	/26	/-	/-	C	61	/-	/-	/32	/30	/-
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Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 D Brg Wid = 1.5 C Brg Wid = 1.5 Bearing B is a rigid surface. Members not listed have forces less than 375#																																						

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

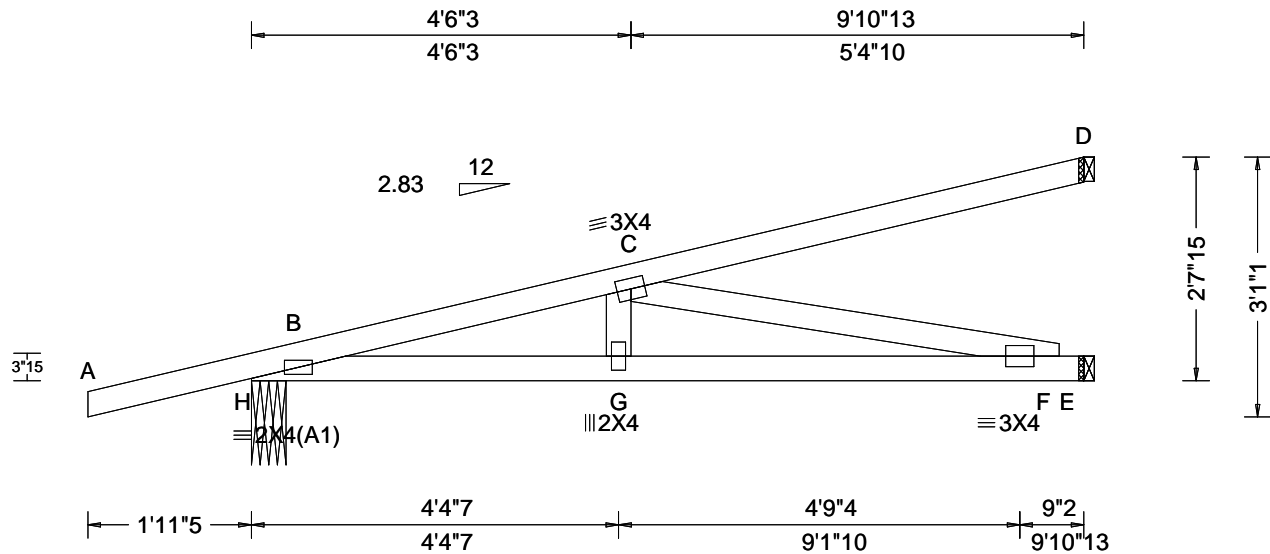
The overall height of this truss excluding overhang is 1-3-15.



COA #0278
 12/07/2021

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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.050 C 999 240 VERT(CL): 0.101 C 999 180 HORZ(LL): 0.009 F - - HORZ(TL): 0.018 F - - Creep Factor: 2.0 Max TC CSI: 0.855 Max BC CSI: 0.725 Max Web CSI: 0.710 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>H</td> <td>360</td> <td>-</td> <td>-</td> <td>-</td> <td>/95</td> <td>-</td> </tr> <tr> <td>E</td> <td>340</td> <td>-</td> <td>-</td> <td>-</td> <td>/37</td> <td>-</td> </tr> <tr> <td>D</td> <td>287</td> <td>-</td> <td>-</td> <td>-</td> <td>/122</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS H Brg Wid = 4.9 Min Req = 1.5 E Brg Wid = 1.5 D Brg Wid = 1.5 Bearing H is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>309 - 1236</td> </tr> </tbody> </table> Maximum Bot Chord Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - G</td> <td>1201 - 301</td> <td>G - F</td> <td>1188 - 307</td> </tr> </tbody> </table> Maximum Web Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Webs</th> <th>Tens.Comp.</th> </tr> </thead> <tbody> <tr> <td>C - F</td> <td>314 - 1215</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	H	360	-	-	-	/95	-	E	340	-	-	-	/37	-	D	287	-	-	-	/122	-	Chords	Tens.Comp.	B - C	309 - 1236	Chords	Tens.Comp.	Chords	Tens. Comp.	B - G	1201 - 301	G - F	1188 - 307	Webs	Tens.Comp.	C - F	314 - 1215
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B - G	1201 - 301	G - F	1188 - 307																																																			
Webs	Tens.Comp.																																																					
C - F	314 - 1215																																																					

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Loading

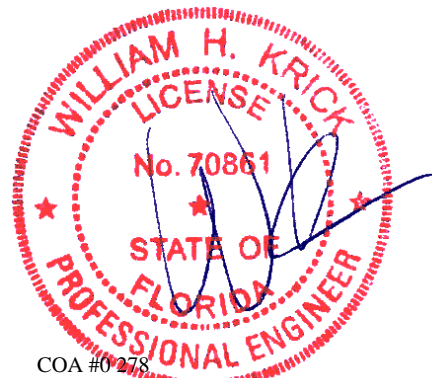
Hipjack supports 7-0-0 setback jacks with no webs.

Wind

Wind loads and reactions based on MWFRS.
 Wind loading based on both gable and hip roof types.

Additional Notes

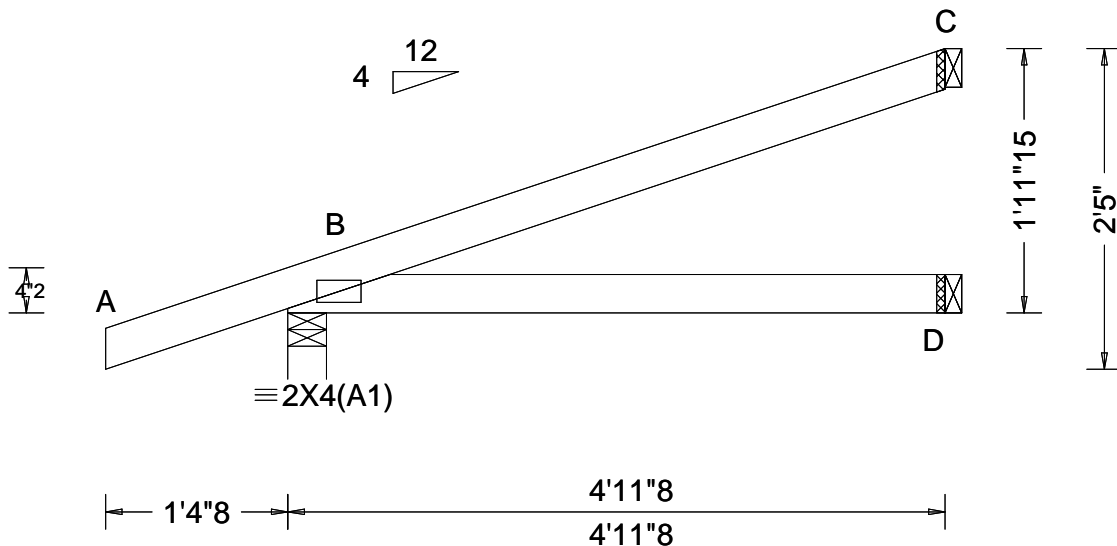
The overall height of this truss excluding overhang is 2'-7-15.



COA #0278
 12/07/2021

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.004 B - - HORZ(TL): 0.009 B - - Creep Factor: 2.0 Max TC CSI: 0.421 Max BC CSI: 0.221 Max Web CSI: 0.000 VIEW Ver: 21.01.01A.0521.20	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 312 /- /- /210 /73 /71 D 87 /- /- /48 /- /- C 124 /- /- /68 /58 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 D Brg Wid = 1.5 C Brg Wid = 1.5 Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

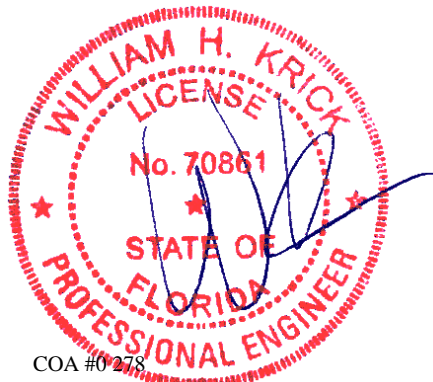
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

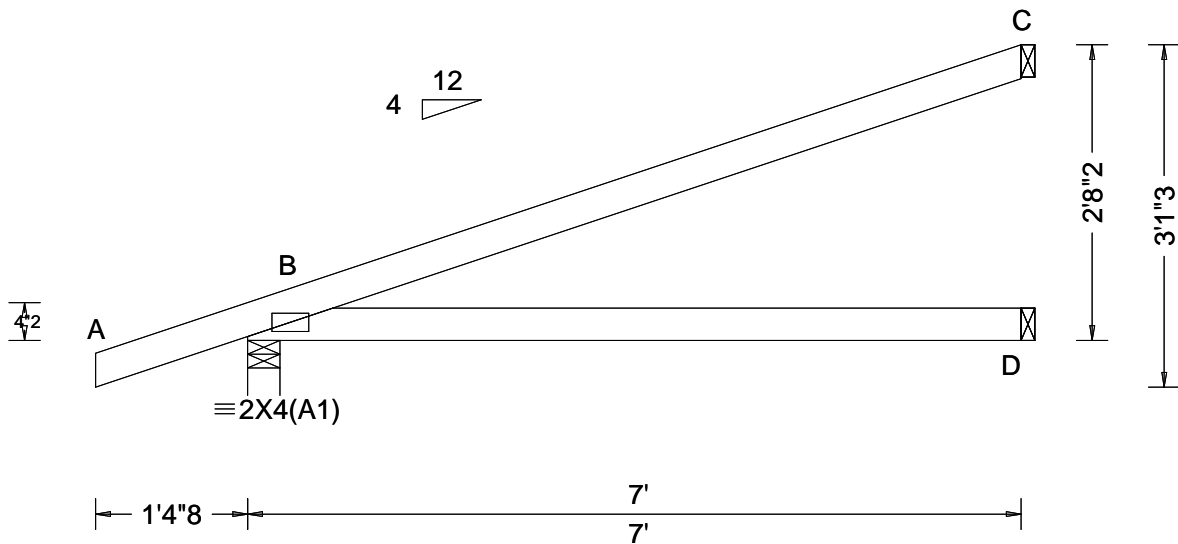
The overall height of this truss excluding overhang is 1-11-15.



COA #0 278
12/07/2021

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Loading Criteria (psf) TCLL: 20.00 TC DL: 10.00 BC LL: 0.00 BC DL: 10.00 Des Ld: 40.00 NC BCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TC DL: 4.2 psf BC DL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GC Cpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.015 B - - HORZ(TL): 0.029 B - - Creep Factor: 2.0 Max TC CSI: 0.880 Max BC CSI: 0.497 Max Web CSI: 0.000 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>391</td> <td>-</td> <td>-</td> <td>/258</td> <td>/88</td> <td>/95</td> </tr> <tr> <td>D</td> <td>127</td> <td>-</td> <td>-</td> <td>/71</td> <td>-</td> <td>-</td> </tr> <tr> <td>C</td> <td>183</td> <td>-</td> <td>-</td> <td>/102</td> <td>/86</td> <td>-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	391	-	-	/258	/88	/95	D	127	-	-	/71	-	-	C	183	-	-	/102	/86	-
				Loc		Gravity			Non-Gravity																													
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Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 D Brg Wid = 1.5 C Brg Wid = 1.5 Bearing B is a rigid surface. Members not listed have forces less than 375#																																						

Lumber

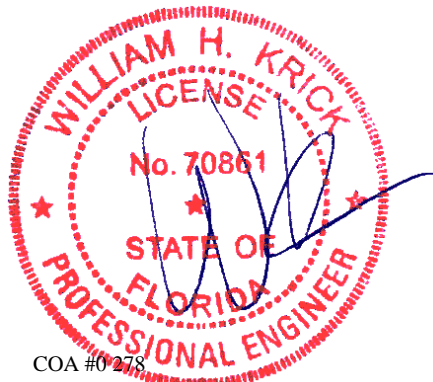
Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

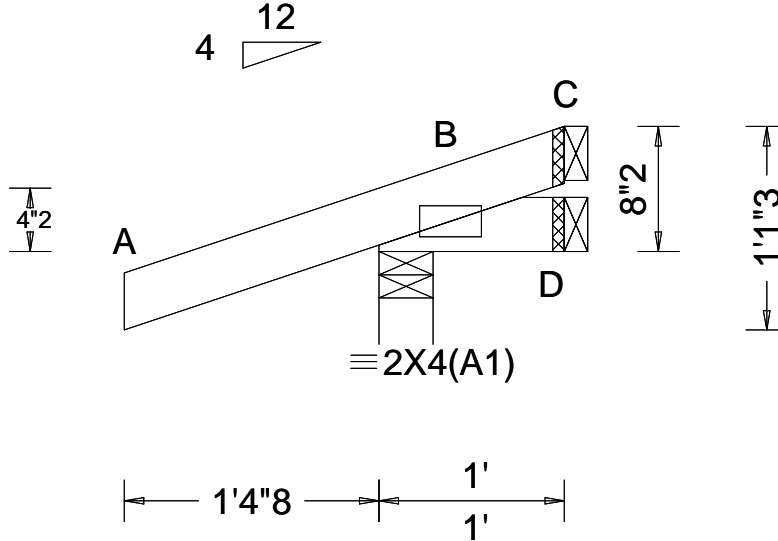
The overall height of this truss excluding overhang is 2-8-2.



COA #0 278
 12/07/2021

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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.000 D - - HORZ(TL): 0.000 D - - Creep Factor: 2.0 Max TC CSI: 0.223 Max BC CSI: 0.028 Max Web CSI: 0.000 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 225 /- /- /167 /91 /26 D 4 /-14 /- /16 /14 /- C - /-40 /- /30 /35 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 D Brg Wid = 1.5 C Brg Wid = 1.5 Bearing B is a rigid surface. Members not listed have forces less than 375#
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Lumber

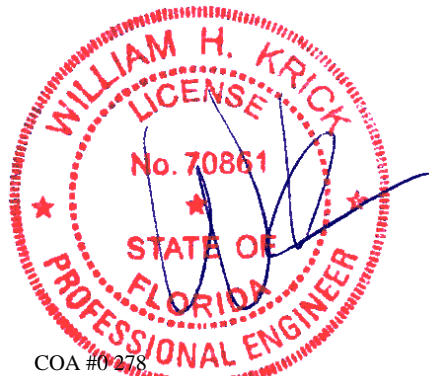
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 0-8-2.

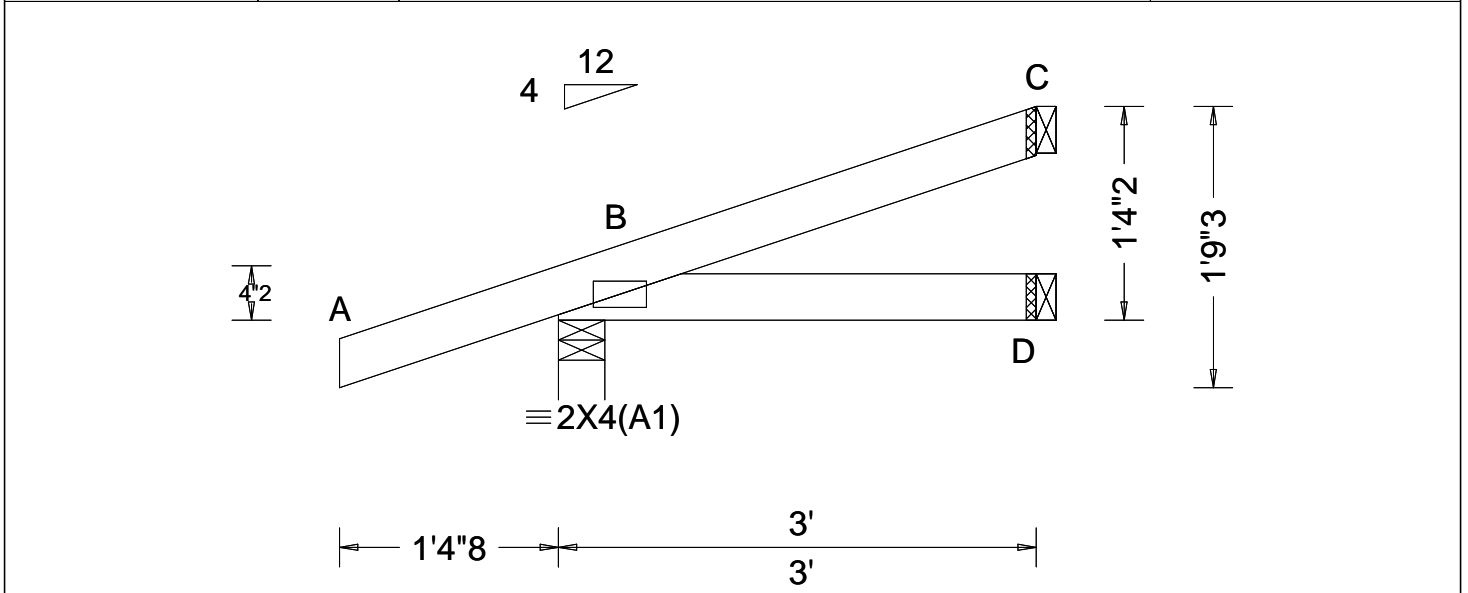


COA #0278

12/07/2021

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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp1: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.001 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.225 Max BC CSI: 0.061 Max Web CSI: 0.000 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 244 /- /- /169 /65 /48 D 48 /- /- /26 /- /- C 63 /- /- /33 /31 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 D Brg Wid = 1.5 C Brg Wid = 1.5 Bearing B is a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

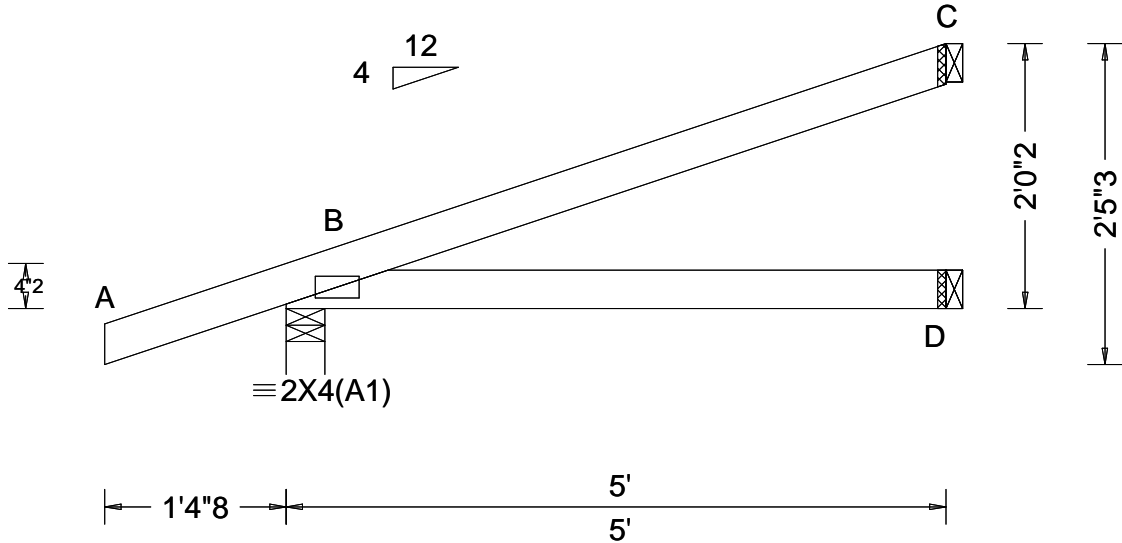
The overall height of this truss excluding overhang is 1-4-2.



COA #0278
12/07/2021

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/def L/#	Loc	R+	/R-	/Rh	/Rw	/U	/RL
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): NA	B	314	/-	/-	/211	/73	/72
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA	D	88	/-	/-	/48	/-	/-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.005 B - -	C	125	/-	/-	/69	/59	/-
Des Ld: 40.00	EXP: C Kzt: NA	Building Code:	HORZ(TL): 0.009 B - -	Wind reactions based on MWFRS						
NCBCLL: 10.00	Mean Height: 15.00 ft	FBC 7th Ed. 2020 Res.	Creep Factor: 2.0	B Brg Wid = 3.5 Min Req = 1.5						
Soffit: 2.00	TCDL: 4.2 psf	TPI Std: 2014	Max TC CSI: 0.430	D Brg Wid = 1.5						
Load Duration: 1.25	BCDL: 3.0 psf	Rep Fac: Yes	Max BC CSI: 0.225	C Brg Wid = 1.5						
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	FT/RT:20(0)/10(0)	Max Web CSI: 0.000	Bearing B is a rigid surface.						
	C&C Dist a: 3.00 ft	Plate Type(s):	VIEW Ver: 21.01.01A.0521.20	Members not listed have forces less than 375#						
	Loc. from endwall: Any	WAVE								
	GCp1: 0.18									
	Wind Duration: 1.60									

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

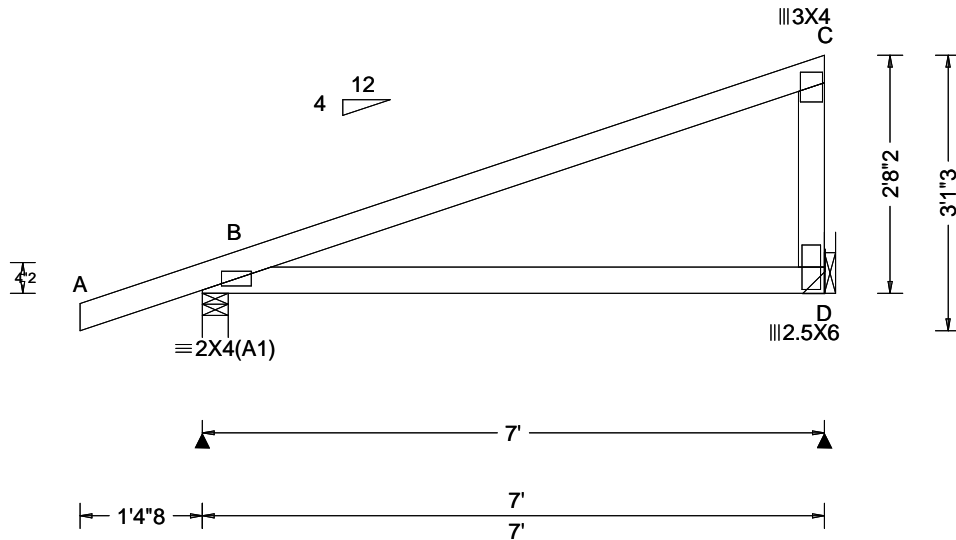
The overall height of this truss excluding overhang is 2'-0"-2."



COA #0278

12/07/2021

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.013 B - - HORZ(TL): 0.026 B - - Creep Factor: 2.0 Max TC CSI: 0.793 Max BC CSI: 0.453 Max Web CSI: 0.372 VIEW Ver: 21.01.01A.0521.20	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 391 /- /- /258 /88 /95 D 267 /- /- /173 /81 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 D Brg Wid = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Hangers / Ties
Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.
Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.
Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.
Bearing at location x=6'9" uses the following support conditions: 6'9"
Bearing D (6'9", 9'1'8") LUS26
Supporting Member: (1)2x6 SP 2400f-2.0E
(4) 0.148"x3" nails into supporting member,
(3) 0.148"x3" nails into supported member.

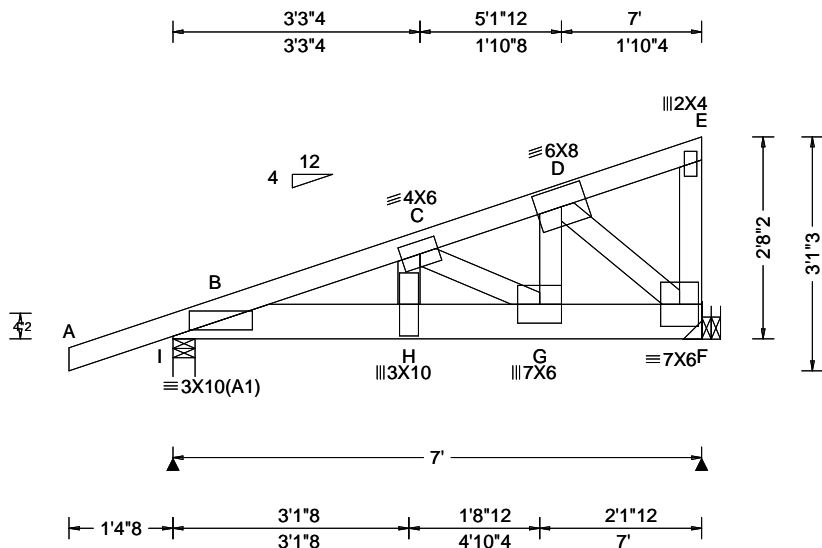
Additional Notes
The overall height of this truss excluding overhang is 2-8-2.

Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



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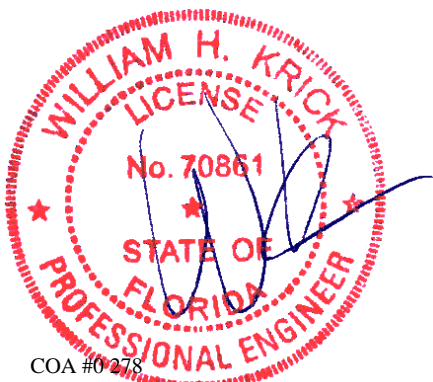
Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.050 H 999 240 VERT(CL): 0.099 H 834 180 HORZ(LL): -0.017 E - - HORZ(TL): 0.034 E - - Creep Factor: 2.0 Max TC CSI: 0.649 Max BC CSI: 0.596 Max Web CSI: 0.961 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL I 2824 /- /- /- /646 /- F 2062 /- /- /- /461 /- Wind reactions based on MWFRS I Brg Wid = 3.5 Min Req = 2.3 F Brg Wid = - Bearing I is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 994 -4532 C - D 573 -2595 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - H 4304 -939 G - F 2081 -460 H - G 4157 -910 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. H - C 1532 -305 G - D 2522 -526 C - G 441 -2021 D - F 610 -2756
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Special Loads
----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 61 plf at -1.38 to 61 plf at 7.00
BC: From 4 plf at -1.38 to 4 plf at 0.00
BC: From 10 plf at 0.00 to 10 plf at 7.00
BC: 1433 lb Conc. Load at 1.06, 3.06, 5.06

Wind
Wind loads and reactions based on MWFRS.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes
The overall height of this truss excluding overhang is 2-8-2.



COA #0278
12/07/2021

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Hangers / Ties

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Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=6'9" uses the following support conditions: 6'9"

Bearing F (6'9", 9'1"8) HUS26

Supporting Member: (2)2x6 SP 2400f-2.0E

(14) 0.148"x3" nails into supporting member,

(6) 0.148"x3" nails into supported member.



COA #0278

12/07/2021

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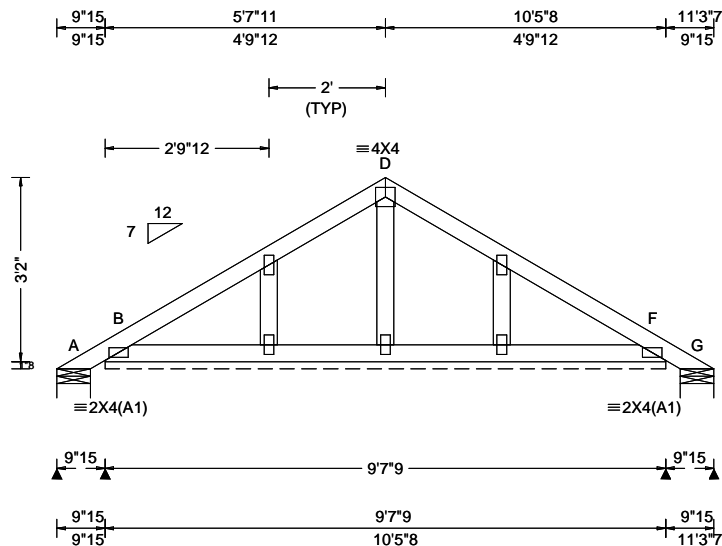
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6750 Forum Drive
Suite 305
Orlando FL, 32821



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 20.77 ft TCDL: 4.2 psf BCDL: 2.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.000 F 999 240 VERT(CL): 0.001 F 999 180 HORZ(LL): 0.001 F - - - HORZ(TL): 0.001 F - - - Creep Factor: 2.0 Max TC CSI: 0.065 Max BC CSI: 0.030 Max Web CSI: 0.035 VIEW Ver: 21.01.01A.0521.20	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity A - /-4 /- /49 /48 /85 B* 76 /- /- /54 /26 /- G - /-4 /- /7 /5 /- Wind reactions based on MWFRS A Brg Wid = 6.9 Min Req = 1.5 B Brg Wid = 115 Min Req = - G Brg Wid = 6.9 Min Req = 1.5 Bearings A, B, & G are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

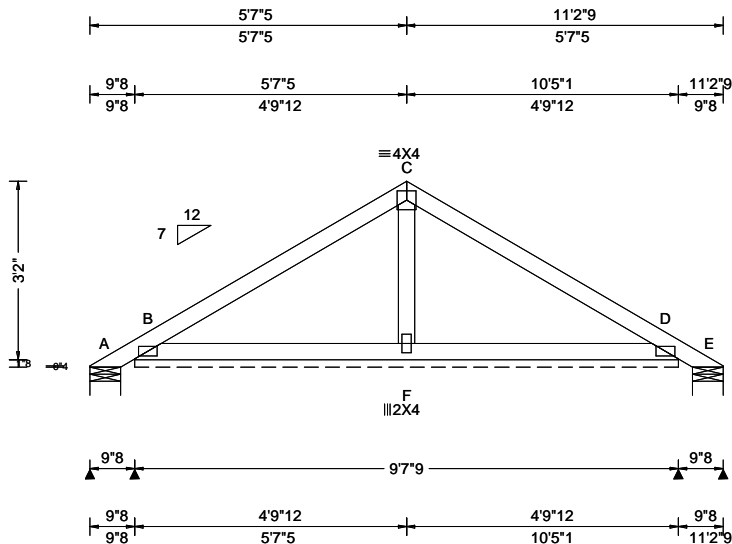
See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
Refer to DWG PB160160118 for piggyback details.



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12/07/2021

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF																																																
TCLL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 20.78 ft TCCL: 4.2 psf BCDL: 2.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.76 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.002 D 999 240 VERT(CL): 0.004 D 999 180 HORZ(LL): 0.002 D - - HORZ(TL): 0.003 D - - Creep Factor: 2.0 Max TC CSI: 0.282 Max BC CSI: 0.104 Max Web CSI: 0.030 VIEW Ver: 21.01.01A.0521.20	<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>-</td> <td>/-134</td> <td>/-</td> <td>/102</td> <td>/154</td> <td>/86</td> </tr> <tr> <td>B*</td> <td>101</td> <td>/-</td> <td>/-</td> <td>/65</td> <td>/53</td> <td>/-</td> </tr> <tr> <td>E</td> <td>-</td> <td>/-134</td> <td>/-</td> <td>/97</td> <td>/112</td> <td>/-</td> </tr> <tr> <td>B</td> <td></td> <td>/-144</td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>D</td> <td></td> <td>/-126</td> <td></td> <td></td> <td></td> <td></td> </tr> </tbody> </table> Wind reactions based on MWFRS A Brg Wid = 6.5 Min Req = 1.5 B Brg Wid = 115 Min Req = - E Brg Wid = 6.5 Min Req = 1.5 Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	-	/-134	/-	/102	/154	/86	B*	101	/-	/-	/65	/53	/-	E	-	/-134	/-	/97	/112	/-	B		/-144					D		/-126				
Loc	Gravity			Non-Gravity																																																
	R+	/R-	/Rh	/Rw	/U	/RL																																														
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B*	101	/-	/-	/65	/53	/-																																														
E	-	/-134	/-	/97	/112	/-																																														
B		/-144																																																		
D		/-126																																																		

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

Refer to DWG PB160160118 for piggyback details.

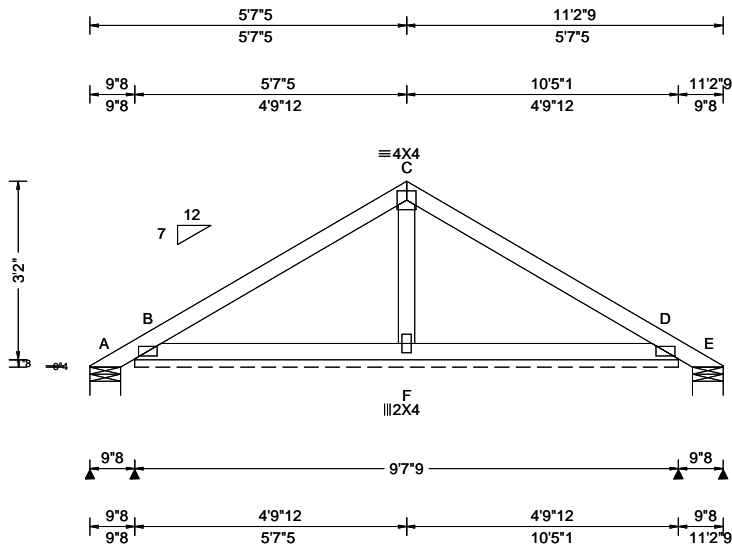


COA #0278

12/07/2021

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 20.78 ft TCDL: 4.2 psf BCDL: 2.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.76 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.002 D 999 240 VERT(CL): 0.004 D 999 180 HORZ(LL): 0.002 D - - HORZ(TL): 0.003 D - - Creep Factor: 2.0 Max TC CSI: 0.282 Max BC CSI: 0.104 Max Web CSI: 0.030 VIEW Ver: 21.01.01A.0521.20	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity A - /-134 /- /102 /154 /86 B* 101 /- /- /65 /53 /- E - /-134 /- /97 /112 /- B /-144 D /-126 Wind reactions based on MWFRS A Brg Wid = 6.5 Min Req = 1.5 B Brg Wid = 115 Min Req = - E Brg Wid = 6.5 Min Req = 1.5 Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Wind

Wind loads based on MWFRS.
Wind loading based on both gable and hip roof types.

Additional Notes

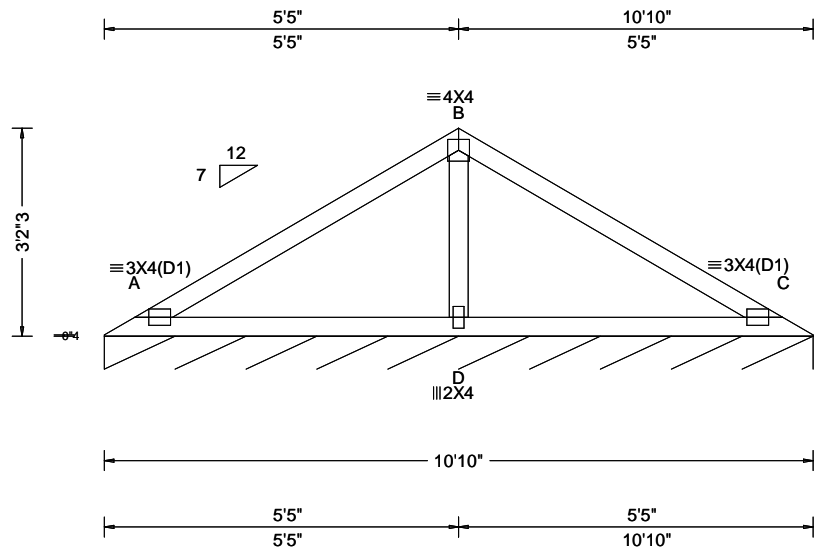
Refer to DWG PB160160118 for piggyback details.



COA #0278
12/07/2021

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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.017 A 999 240 VERT(CL): 0.035 A 999 180 HORZ(LL): -0.008 C - - HORZ(TL): 0.016 C - - Creep Factor: 2.0 Max TC CSI: 0.405 Max BC CSI: 0.340 Max Web CSI: 0.152 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL A* 83 /- /- /42 /17 /7 Wind reactions based on MWFRS A Brg Wid = 130 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 406 -274 B - C 406 -284 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. B - D 512 -609
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

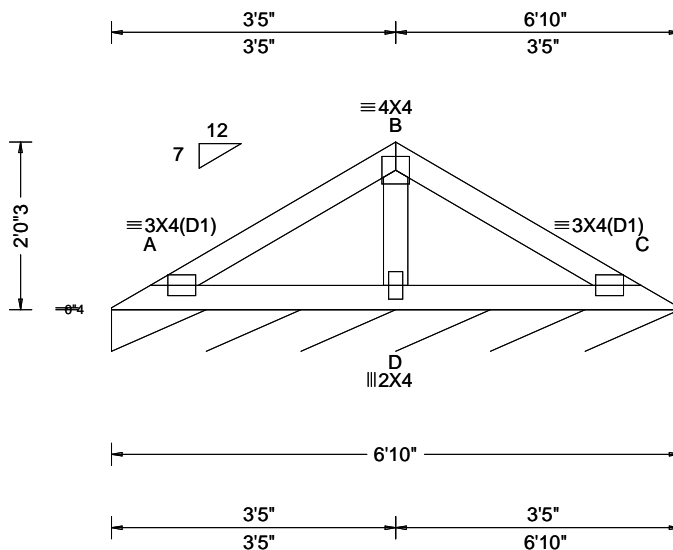
Additional Notes
See DWGS VALTN160118 and VAL180160118 for valley details.



COA #0 278
12/07/2021

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.16 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.004 A 999 240 VERT(CL): 0.008 A 999 180 HORZ(LL): -0.002 C - - HORZ(TL): 0.004 C - - Creep Factor: 2.0 Max TC CSI: 0.139 Max BC CSI: 0.120 Max Web CSI: 0.086 VIEW Ver: 21.01.01A.0521.20	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A* 83 /- /- /41 /15 /6 Wind reactions based on MWFRS A Brg Wid = 82.0 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

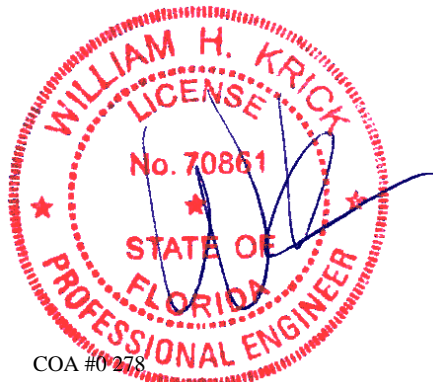
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

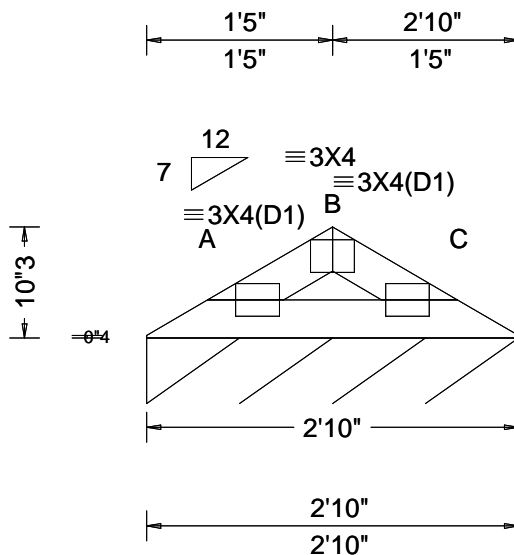
See DWGS VALTN160118 and VAL180160118 for valley details.



COA #0 278
12/07/2021

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.74 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.001 A 999 240 VERT(CL): 0.003 A 999 180 HORZ(LL): -0.001 A - - HORZ(TL): 0.001 A - - Creep Factor: 2.0 Max TC CSI: 0.033 Max BC CSI: 0.053 Max Web CSI: 0.000 VIEW Ver: 21.01.01A.0521.20	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A* 82 /- /- /36 /9 /4 Wind reactions based on MWFRS A Brg Wid = 34.0 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

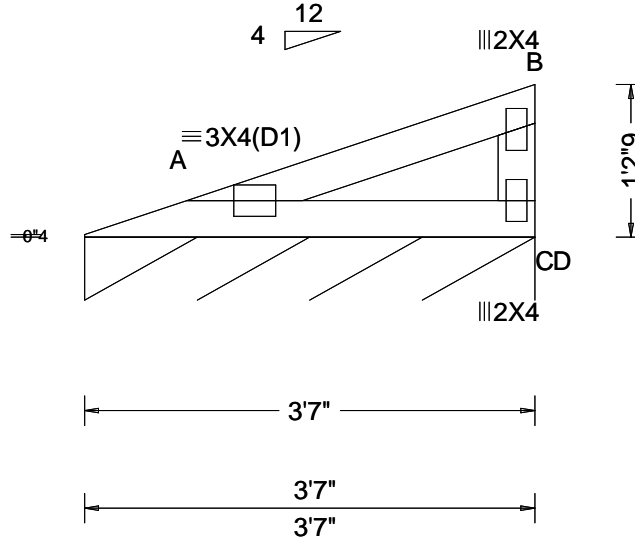
See DWGS VALTN160118 and VAL180160118 for valley details.



COA #0278
12/07/2021

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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.002 A - - HORZ(TL): 0.003 A - - Creep Factor: 2.0 Max TC CSI: 0.164 Max BC CSI: 0.155 Max Web CSI: 0.080 VIEW Ver: 21.01.01A.0521.20	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL D* 81 /- /- /44 /13 /9 Wind reactions based on MWFRS D Brg Wid = 43.0 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

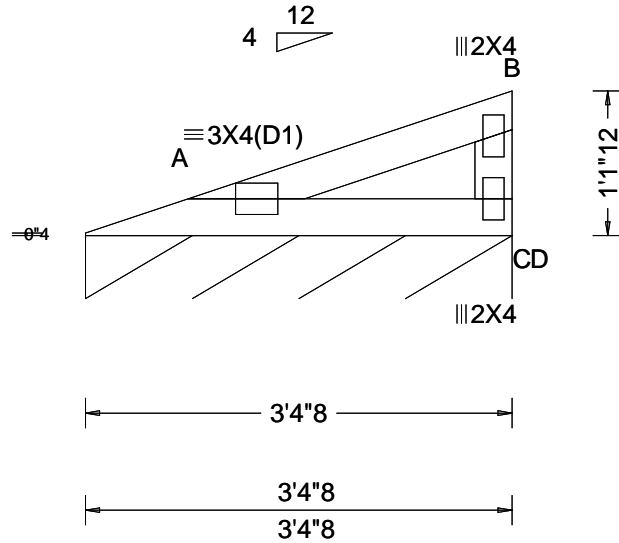
Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.



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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TC DL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TC DL: 4.2 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.001 A - - HORZ(TL): 0.003 A - - Creep Factor: 2.0 Max TC CSI: 0.136 Max BC CSI: 0.134 Max Web CSI: 0.070 VIEW Ver: 21.01.01A.0521.20	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL D* 81 /- /- /44 /12 /8 Wind reactions based on MWFRS D Brg Wid = 40.5 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN160118 and VAL180160118 for valley details.



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Gable Stud Reinforcement Detail

ASCE 7-16: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

- Or: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
- Or: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00
- Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

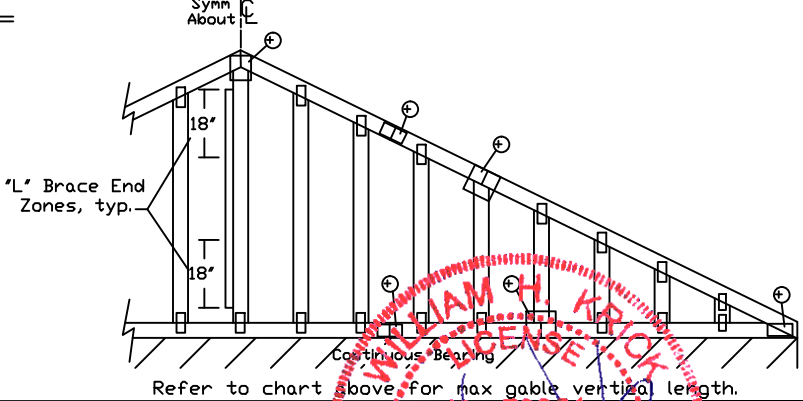
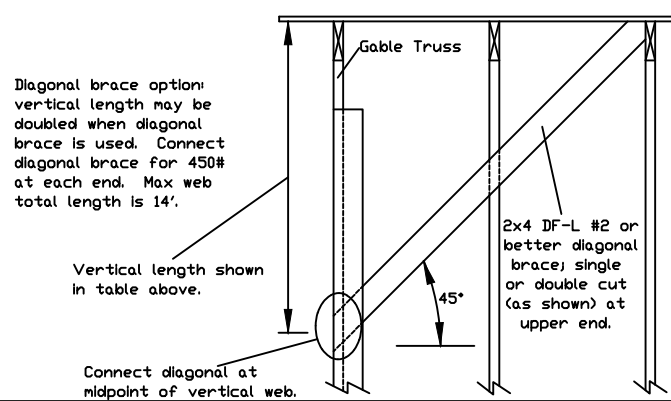
Max Gable Vertical Length	2x4 Gable Vertical		Brace Grade	No Braces	(1) 1x4 'L' Brace *		(1) 2x4 'L' Brace *		(2) 2x4 'L' Brace **		(1) 2x6 'L' Brace *		(2) 2x6 'L' Brace **	
	Spacing	Species			Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
	24" o.c.	SPF	#1 / #2	#1 / #2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"
#3				4' 1"	6' 7"	7' 1"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
Stud				4' 1"	6' 7"	7' 0"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
HF			#1	4' 1"	5' 8"	6' 0"	7' 7"	8' 1"	10' 1"	10' 6"	11' 10"	12' 8"	14' 0"	14' 0"
			#2	4' 6"	7' 4"	7' 8"	8' 8"	9' 0"	10' 4"	10' 9"	13' 8"	14' 0"	14' 0"	14' 0"
			Standard	4' 1"	5' 8"	6' 0"	7' 7"	8' 1"	10' 1"	10' 6"	11' 10"	12' 8"	14' 0"	14' 0"
SP		DFL	#1	4' 6"	7' 4"	7' 8"	8' 8"	9' 0"	10' 4"	10' 9"	13' 8"	14' 0"	14' 0"	14' 0"
			#2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"
			#3	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"
		Standard	#1	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"
			#2	4' 0"	5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	10' 2"	11' 0"	11' 10"	14' 0"	14' 0"
			#3	4' 0"	5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	10' 2"	11' 0"	11' 10"	14' 0"	14' 0"
16" o.c.	SPF	#1 / #2	#1 / #2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 8"	8' 1"	8' 8"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 8"	8' 1"	8' 6"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		HF	#1	5' 1"	8' 5"	8' 9"	9' 11"	10' 4"	11' 10"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
			Standard	4' 8"	6' 11"	7' 5"	9' 3"	9' 11"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	DFL	#1	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
			Standard	4' 8"	6' 5"	6' 10"	8' 7"	9' 2"	11' 7"	12' 1"	13' 6"	14' 0"	14' 0"	14' 0"
		#1 / #2	#1 / #2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	11' 8"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
SP	DFL	#1	5' 8"	9' 3"	9' 8"	10' 11"	11' 4"	13' 0"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"	
		#2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	12' 11"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	
		#3	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	
	Standard	#1	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	
		#2	5' 1"	7' 5"	7' 11"	9' 11"	10' 7"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	
		#3	5' 1"	7' 5"	7' 11"	9' 11"	10' 7"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	

Bracing Group Species and Grades:

Group A:			
Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard
Douglas Fir-Larch		Southern Pine***	
#3		#3	
Stud		Stud	
Standard		Standard	
Group B:			
Hem-Fir			
#1 & Btr			
#1			
Douglas Fir-Larch		Southern Pine***	
#1		#1	
#2		#2	

1x4 Braces shall be SRB (Stress-Rated Board).
 ***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:
 Wind Load deflection criterion is L/240.
 Provide uplift connections for 55 plf over continuous bearing (5 psf TC Dead Load).
 Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12' plywood overhang.



Attach 'L' braces with 10d (0.128"x3.0" min) nails.
 * For (1) 'L' brace: space nails at 2' o.c. in 18' end zones and 4' o.c. between zones.
 ** For (2) 'L' braces: space nails at 3' o.c. in 18' end zones and 6' o.c. between zones.
 'L' bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0"	3X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

514 Earth City Expressway
Suite 242
Earth City, MO 63045

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For more information see this Job's general notes page and these web sites:
 ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

No. 70861

WILLIAM H. KRIEGER
 STATE OF FLORIDA
 PROFESSIONAL ENGINEER
 License No. 70861
 COA #0-278 12/07/2021

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-16-GAB14015
DATE	01/26/2018
DRWG	A14015ENC160118

Gable Stud Reinforcement Detail

ASCE 7-16: 140 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

- Or: 120 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
- Or: 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00
- Or: 100 mph wind speed, 30' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length	2x4 Gable Vertical Spacing		Brace Grade	No Braces	(1) 1x4 'L' Brace *		(1) 2x4 'L' Brace *		(2) 2x4 'L' Brace **		(1) 2x6 'L' Brace *		(2) 2x6 'L' Brace **	
	Species	#1 / #2			Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
			24" o.c.	SPF	#1 / #2	#1	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"
#3	3' 10"	6' 2"				6' 7"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"
Stud	3' 10"	6' 2"				6' 6"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"
Standard	#1	3' 10"			5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	10' 0"	11' 0"	11' 10"	14' 0"	14' 0"
	#2	4' 2"			7' 0"	7' 3"	8' 3"	8' 7"	9' 10"	10' 3"	13' 0"	13' 6"	14' 0"	14' 0"
	Stud	4' 2"			7' 0"	7' 3"	8' 3"	8' 7"	9' 10"	10' 3"	13' 0"	13' 6"	14' 0"	14' 0"
SP	#2	#1		4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"
		#3		4' 0"	5' 7"	5' 11"	7' 5"	7' 11"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"
		Stud		4' 0"	5' 7"	5' 11"	7' 5"	7' 11"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"
	Standard	#1		3' 9"	4' 11"	5' 13"	6' 6"	7' 0"	8' 10"	9' 6"	10' 3"	11' 0"	13' 11"	14' 0"
		#2		4' 10"	8' 0"	8' 4"	9' 6"	9' 10"	11' 3"	11' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud		4' 10"	8' 0"	8' 4"	9' 6"	9' 10"	11' 3"	11' 9"	14' 0"	14' 0"	14' 0"	14' 0"
16" o.c.	SPF	#1 / #2	#1	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 5"	7' 6"	8' 3"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 5"	7' 6"	8' 0"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1	4' 10"	8' 0"	8' 4"	9' 6"	9' 10"	11' 3"	11' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#2	#1	4' 7"	6' 10"	7' 3"	8' 0"	8' 7"	10' 10"	11' 6"	12' 7"	13' 15"	14' 0"	14' 0"
			#3	4' 7"	6' 10"	7' 3"	8' 0"	8' 7"	10' 10"	11' 6"	12' 7"	13' 15"	14' 0"	14' 0"
			Stud	4' 7"	6' 10"	7' 3"	8' 0"	8' 7"	10' 10"	11' 6"	12' 7"	13' 15"	14' 0"	14' 0"
		Standard	#1 / #2	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	11' 2"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
12" o.c.	SPF	#1 / #2	#1	5' 4"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	12' 3"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	12' 3"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1	5' 4"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	12' 3"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	12' 3"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#2	#1	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1 / #2	4' 10"	6' 11"	7' 4"	9' 3"	9' 10"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 10"	6' 11"	7' 4"	9' 3"	9' 10"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 10"	6' 11"	7' 4"	9' 3"	9' 10"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"

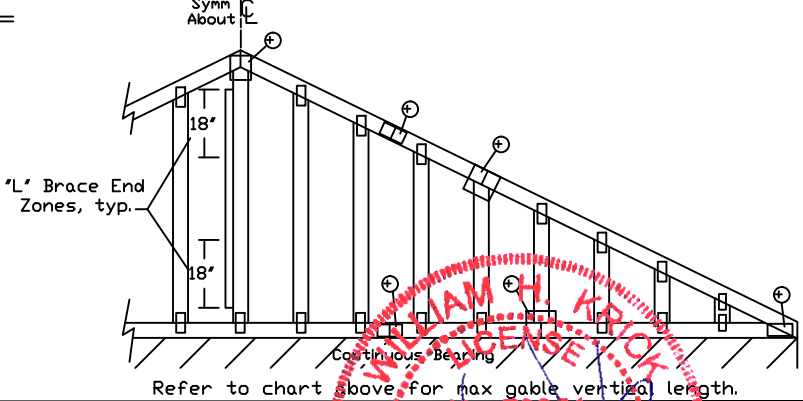
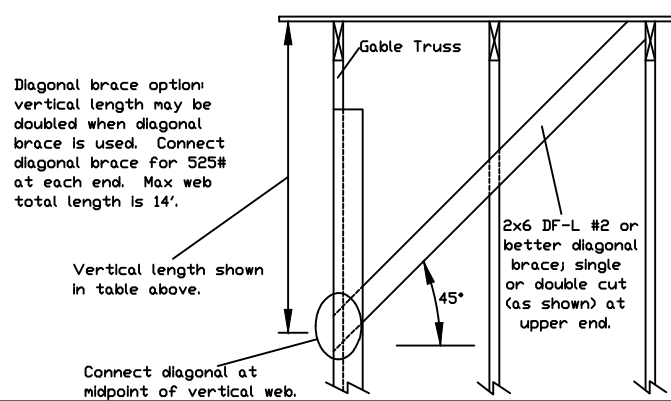
Bracing Group Species and Grades:

Group A:			
Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard
Douglas Fir-Larch		Southern Pine***	
#3		#3	
Stud		Stud	
Standard		Standard	

Group B:			
Hem-Fir			
#1 & Btr		#1	
Douglas Fir-Larch			
#1		#1	
#2		#2	

1x4 Braces shall be SRB (Stress-Rated Board).
 ***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:
 Wind Load deflection criterion is L/240.
 Provide uplift connections for 100 plf over continuous bearing (5 psf TC Dead Load).
 Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12' plywood overhang.



Attach 'L' braces with 10d (0.128"x3.0" min) nails.
 * For (1) 'L' brace: space nails at 2' o.c. in 18' end zones and 4' o.c. between zones.
 ** For (2) 'L' braces: space nails at 3' o.c. in 18' end zones and 6' o.c. between zones.

*'L' bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	2X4
Greater than 4' 0", but less than 11' 6"	3X4
Greater than 11' 6"	4X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

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For more information see this Job's general notes page and these web sites:
 ALPINE: www.alpineitw.com TPI: www.tpinst.org SBCA: www.sbcacomp.com ICC: www.iccsafe.org

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-16-GAB14030
DATE	01/26/2018
DRWG	A14030ENC160118

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

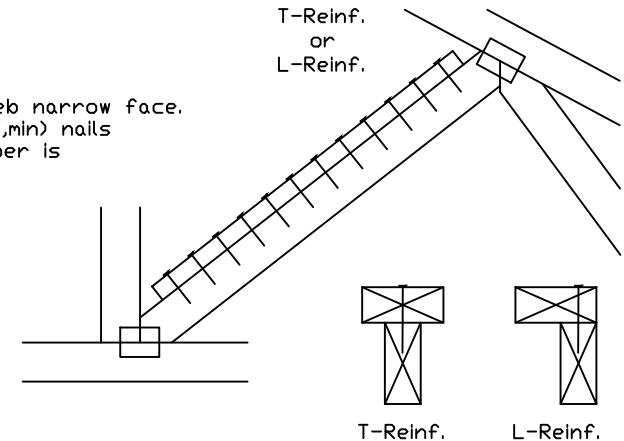
Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6 or 2x4	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.

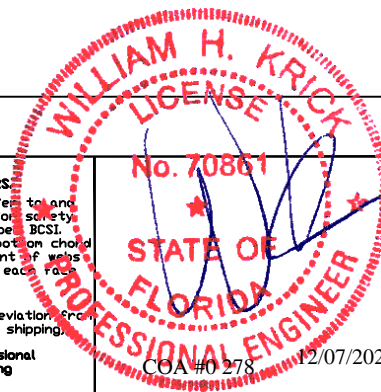
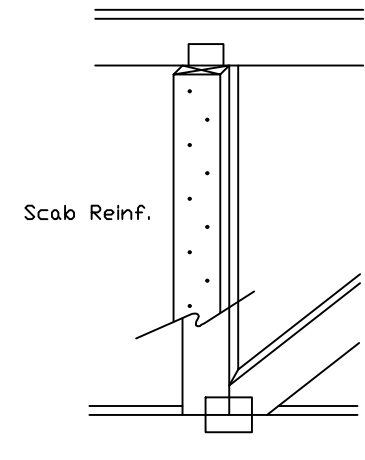
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

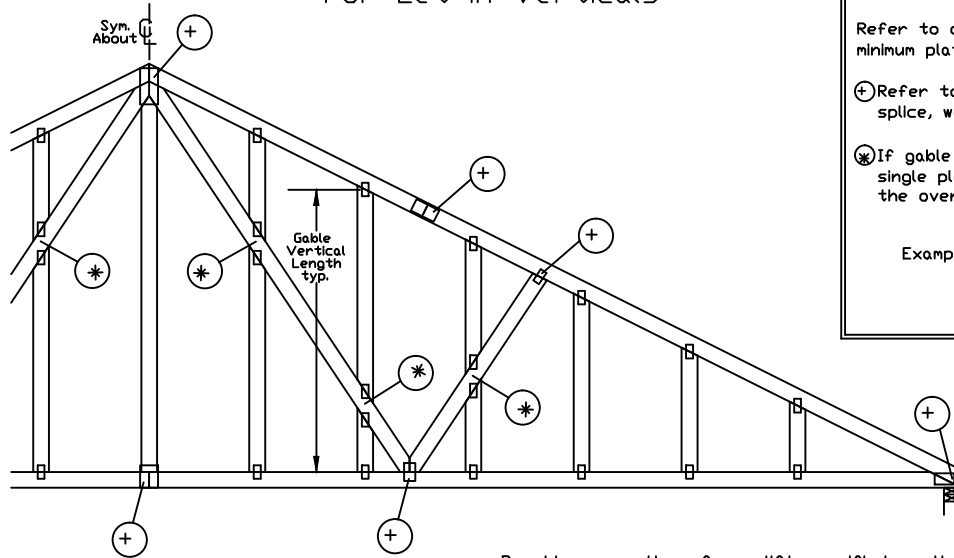
Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



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TC LL	PSF	REF CLR Subst.
TC DL	PSF	DATE 01/02/19
BC DL	PSF	DRWG BRCLBSUB0119
BC LL	PSF	
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

Gable Detail For Let-in Verticals



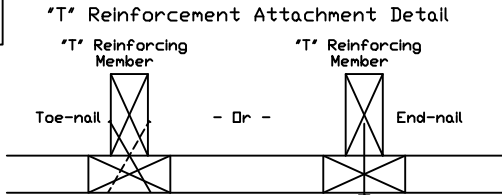
Gable Truss Plate Sizes

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

(+) Refer to Engineered truss design for peak, splice, web, and heel plates.

(x) If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



Provide connections for uplift specified on the engineered truss design.

Attach each 'T' reinforcing member with
End Driven Nails:
10d Common (0.148"x3",min) Nails at 4' o.c. plus
(4) nails in the top and bottom chords.

Toenailed Nails:
10d Common (0.148"x3",min) Toenails at 4' o.c. plus
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

- ASCE 7-05 Gable Detail Drawings
A13015051014, A12015051014, A11015051014, A10015051014, A14015051014,
A13030051014, A12030051014, A11030051014, A10030051014, A14030051014
- ASCE 7-10 & ASCE 7-16 Gable Detail Drawings
A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118,
A18015ENC100118, A20015ENC100118, A20015END100118, A20015PED100118,
A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118,
A18030ENC100118, A20030ENC100118, A20030END100118, A20030PED100118,
S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118,
S18015ENC100118, S20015ENC100118, S20015END100118, S20015PED100118,
S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118,
S18030ENC100118, S20030ENC100118, S20030END100118, S20030PED100118

See appropriate Alpine gable detail for maximum panel for Gable vertical length.

To convert from 'L' to 'T' reinforcing members, multiply 'T' increase by length (based on appropriate Alpine gable detail).

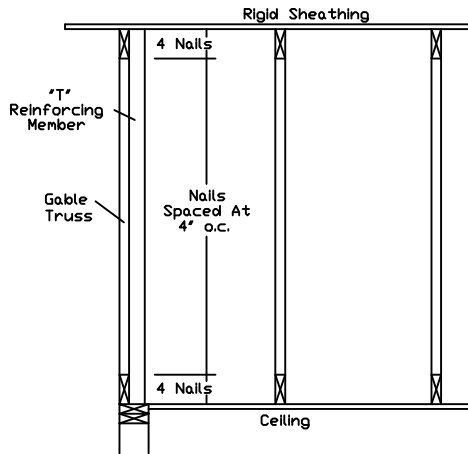
Maximum allowable 'T' reinforced gable vertical length is 14' from top to bottom chord.

'T' reinforcing member material must match size, specie, and grade of the 'L' reinforcing member.

Web Length Increase w/ 'T' Brace

'T' Reinf. Mbr. Size	'T' Increase
2x4	30 %
2x6	20 %

Example:
ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft, Kzt = 1.00
Gable Vertical = 24' o.c. SP #3
'T' Reinforcing Member Size = 2x4
'T' Brace Increase (From Above) = 30% = 1.30
(1) 2x4 'L' Brace Length = 8' 7"
Maximum 'T' Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"



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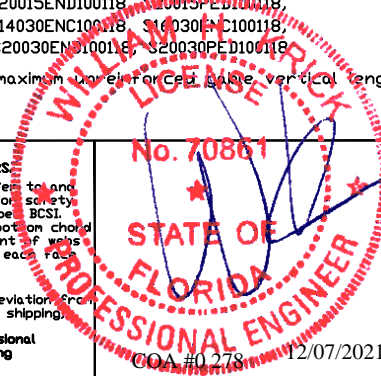
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514 Earth City Expressway
Suite 242
Earth City, MO 63045



REF LET-IN VERT

DATE 01/02/2018

DRWG GBLLETIN0118

MAX. TOT. LD. 60 PSF

DUR. FAC. ANY

MAX. SPACING 24.0"

Piggyback Detail - ASCE 7-16: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

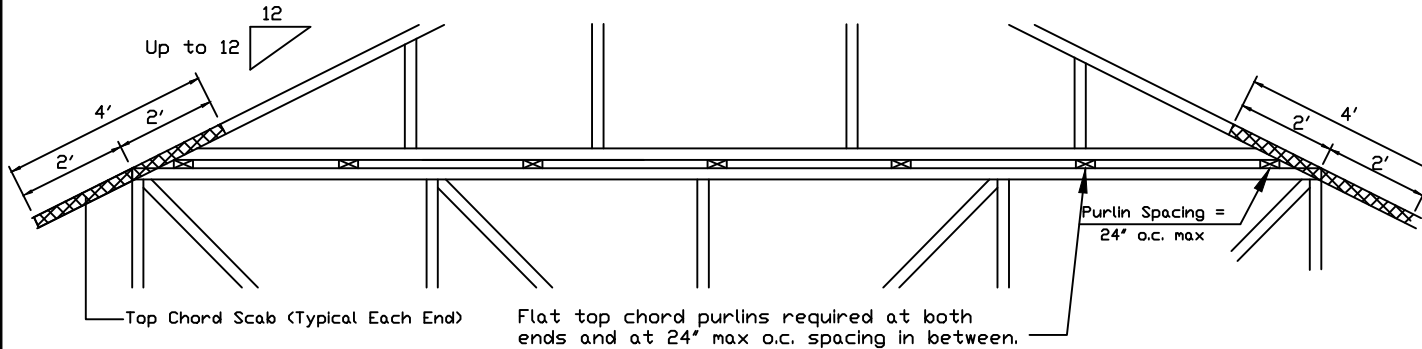
160 mph Wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0.
 Or 140 mph wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp D, wind DL= 5.0 psf (min), Kzt=1.0.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24' o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Detail A : Purlin Spacing = 24" o.c. or less

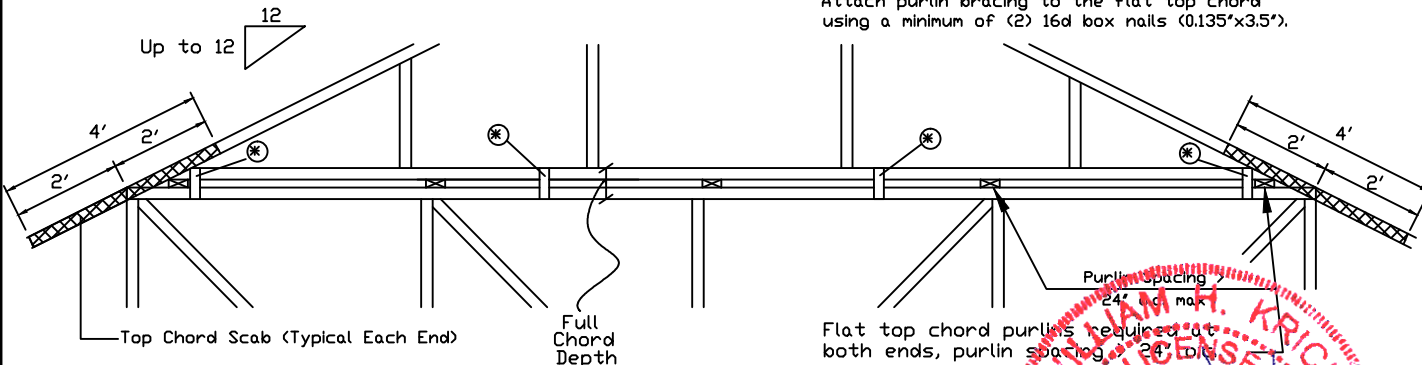


Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3X8 Trulox plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (1) 28PB wave piggyback plate plated to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

Detail B : Purlin Spacing > 24" o.c.



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").

Note: If purlins or sheathing are not specified on the flat top of the base truss, purlins must be installed at 24' o.c. max. and use Detail A.

* In addition, provide connection with one of the following methods:

- Trulox**
Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.
- APA Rated Gusset**
8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.
- 2x4 Vertical Scabs**
2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.
- 28PB Wave Piggyback Plate**
One 28PB wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.

514 Earth City Expressway
Suite 242
Earth City, MO 63045

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No. 70861
 STATE OF FLORIDA
 PROFESSIONAL ENGINEER
 COA # 0 278 12/07/2021

REF	PIGGYBACK
DATE	01/02/2018
DRWG	PB160160118
SPACING	24.0"

REF	PIGGYBACK
DATE	01/02/2018
DRWG	PB160160118
SPACING	24.0"

Valley Detail - ASCE 7-16: 180 mph, 30' Mean Height, Partially Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
 Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
 Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with:
 535# connection or with (1) Simpson H2.5A or equivalent connector for
 ASCE 7-16 180 mph. 30' Mean Height, Part. Enc. Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00
 Or
 ASCE 7-16 160 mph. 30' Mean Height, Part. Enc. Building, Exp. D, Wind TC DL=5 psf, Kzt = 1.00

Bottom chord may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

All plates shown are Alpine Wave Plates.

Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7-9" apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

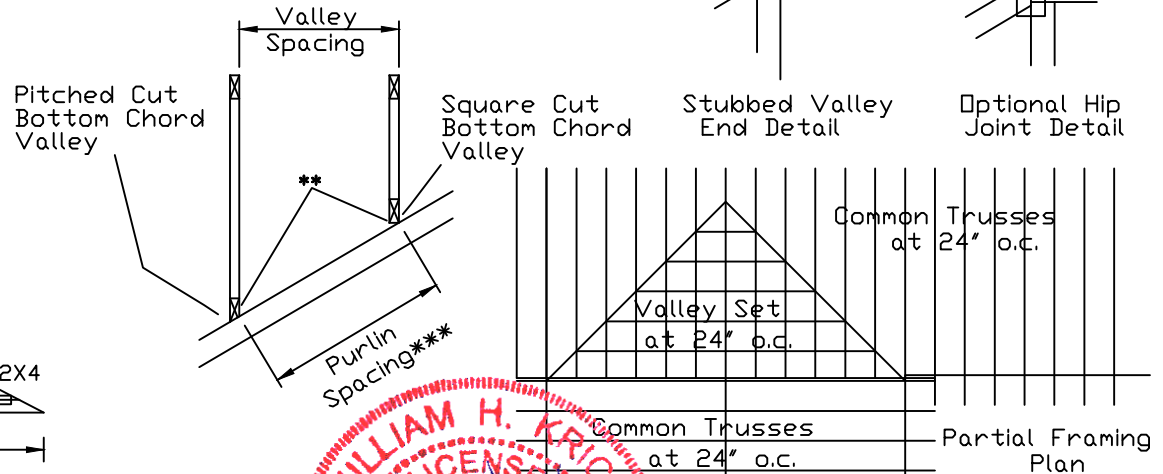
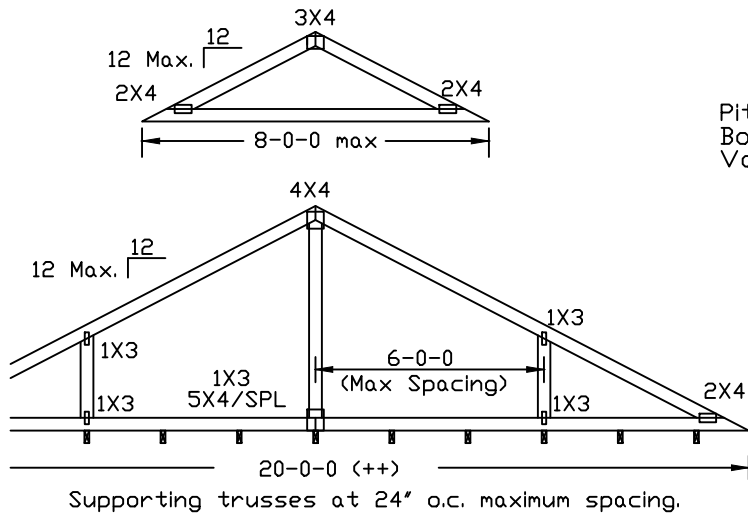
Top chord of truss beneath valley set must be braced with: properly attached, rated sheathing applied prior to valley truss installation.

Or
 Purlins at 24" o.c. or as otherwise specified on engineer's sealed design

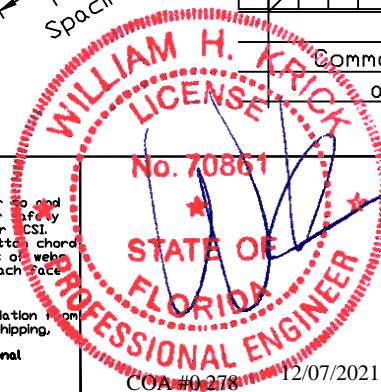
Or
 By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

*** Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.

++ Larger spans may be built as long as the vertical height does not exceed 14'-0".



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TC LL	30	30	40PSF	REF	VALLEY DETAIL
TC DL	20	15	7PSF	DATE	01/26/2018
BC DL	10	10	10 PSF	DRWG	VAL180160118
BC LL	0	0	0PSF		
TOT. LD.	60	55	57PSF		
DUR.FAC.1.25/1.33	1.15	1.15			
SPACING	24.0"				

Valley Detail - ASCE 7-16: 30' Mean Height, Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
 Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
 Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with:
 (2) 16d box (0.135" x 3.5") nails toe-nailed for
 ASCE 7-16, 30' Mean Height, Enclosed Building, Exp. C,
 Wind TC DL=5 psf, Kzt = 1.00, Max. Wind Speed based on
 supporting truss material at connection location:
 170 mph for SP (G = 0.55, min.),
 155 mph for DF-L (G = 0.50, min.), or
 120 mph for HF & SPF (G = 0.42, min.).

Maximum top chord pitch is 10/12 for supporting trusses
 below valley trusses.

Bottom chord of valley trusses may be square or
 pitched cut as shown.

Valleys short enough to be cut as solid triangular
 members from a single 2x6, or larger as required,
 shall be permitted in lieu of fabricating from
 separate 2x4 members.

All plates shown are Alpine Wave Plates.

Unless specified otherwise on engineer's sealed design, for vertical
 valley webs taller than 7'-9" apply 2x4 "T" reinforcement, 80% length of
 web, same species and grade or better, attached with 10d box
 (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous
 Lateral Restraint applied at mid-length of web is permitted with diagonal
 bracing as shown in DRWG BRCLBANC1014.

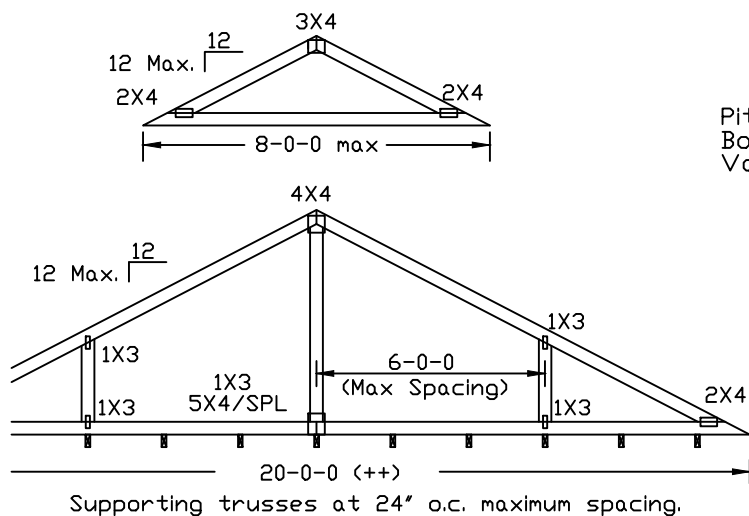
Top chord of truss beneath valley set must be braced with:
 properly attached, rated sheathing applied prior to valley truss
 installation.

Or
 Purlins at 24" o.c. or as otherwise specified on engineer's sealed design

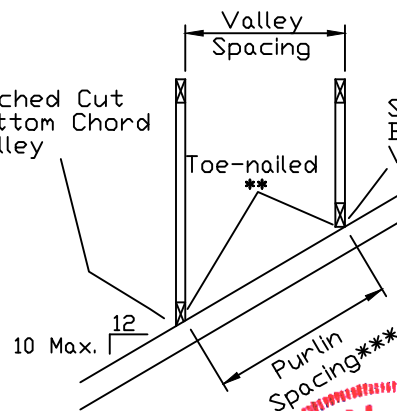
Or
 By valley trusses used in lieu of purlin spacing as specified on
 Engineer's sealed design.

*** Note that the purlin spacing for bracing the top chord of the truss
 beneath the valley is measured along the slope of the top chord.

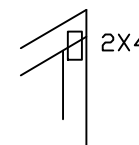
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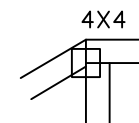
Pitched Cut
 Bottom Chord
 Valley



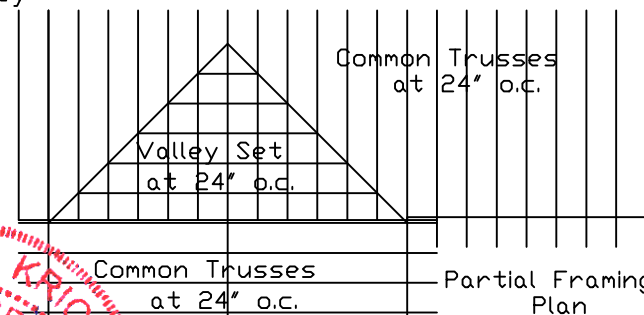
Square Cut
 Bottom Chord
 Valley



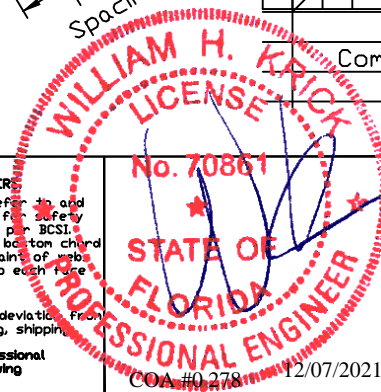
Stubbed Valley
 End Detail



Optional Hip
 Joint Detail



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