

Piggyback Detail - ASCE 7-22: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

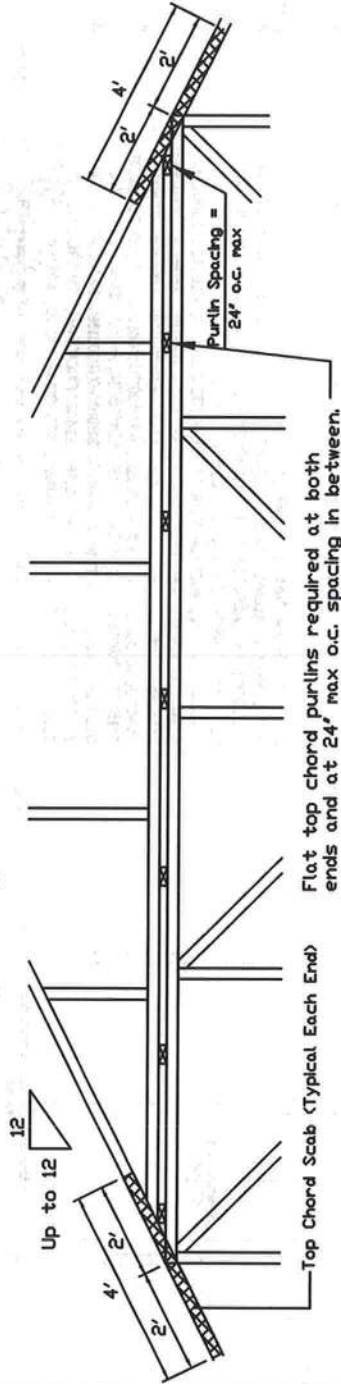
160 mph Wind, 30.00 ft Mean Hgt, ASCE 7-22, Enclosed Bldg, located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0.
Dr- 140 mph wind, 30.00 ft Mean Hgt, ASCE 7-22, Enclosed Bldg, located anywhere in roof, Exp D, Wind DL= 5.0 psf (min), Kzt=1.0.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building designer shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

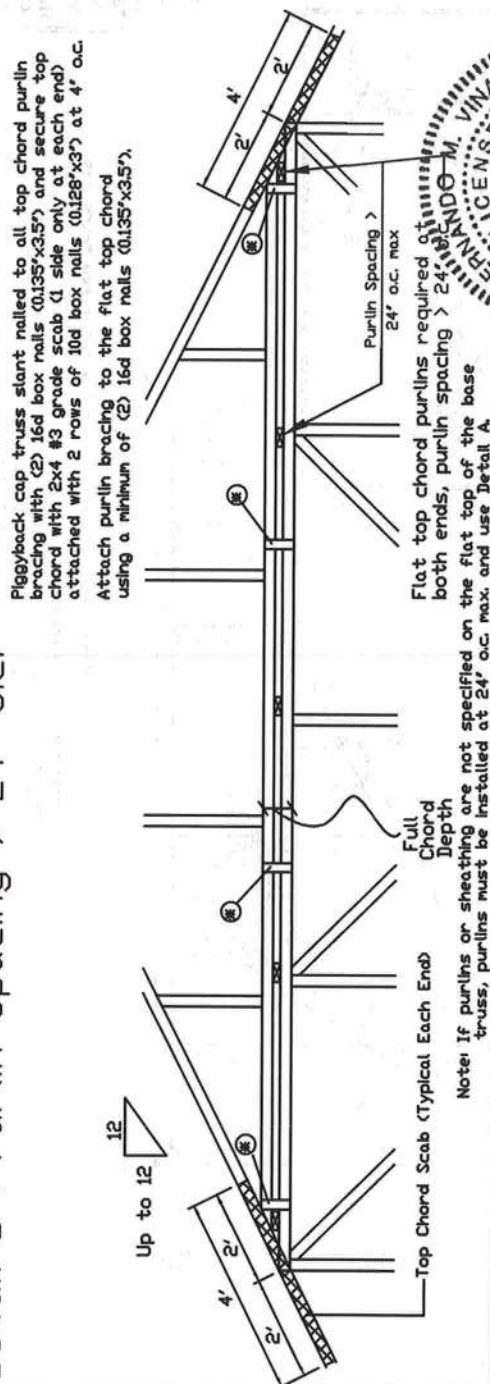
Maximum truss spacing is 24' o.c. Detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Detail A : Purlin Spacing = 24" o.c. or less



Detail B : Purlin Spacing > 24" o.c.



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3x8 Trulox plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (1) 2x8P wave piggyback plate attached to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

■ In addition, provide connection with one of the following methods:

Trulox
Use 3x8 Trulox plates for 2x4 chord member, and 3x10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.

APA Rated Gusset
8"x8"x7/16" (min) APA rated sheathing gussets (each face), Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gusset; (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.

2x4 Vertical Scabs
2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.

2x8P Wave Piggyback Plate
Use 2x8P wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at line of base truss top chord to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.

IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of the Building Components Group Inc. (BCGI) Truss Installation Manual. Trusses are to be installed in accordance with the BCGI Truss Installation Manual. Trusses shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 1004-2 for standard plate positions.

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

For more information see this job's general notes page and these web sites:
ALPINE: www.alpinecorp.com TFS: www.trusscomponents.com ICC: www.iccsafe.org



REF PIGGYBACK
DATE 07/03/2023
DRWG PB160220723

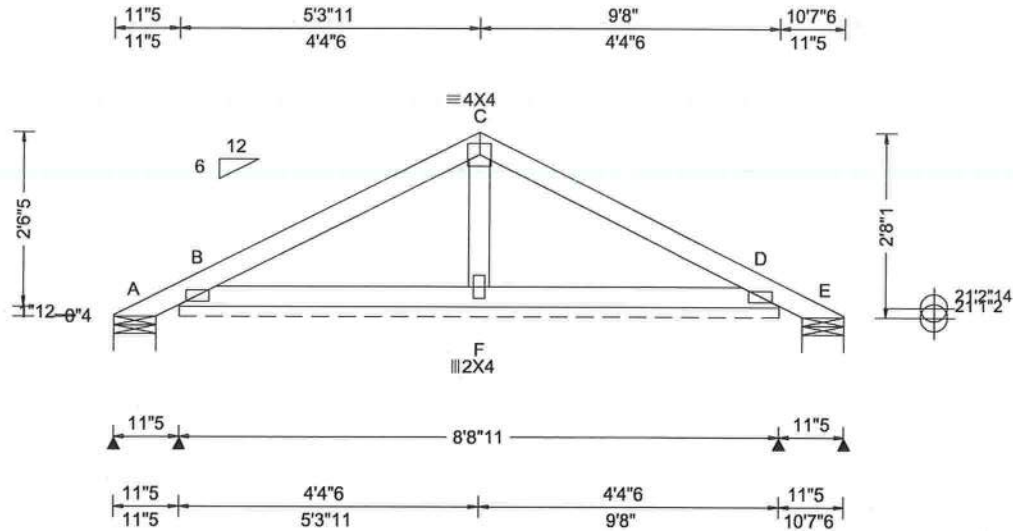
10/02/2024

SPACING 24.0'

COA#0-278

Florida Certificate of Product Approval #FL1999

SEQN: 780270 FROM: CDM	GABL Qty: 1	Ply: 1 Truss Label: PB02	Job Number: 24-1632 VANMESBURGER	Cust: R 215 JRef: 1Y3R2150002 T2 DrwNo: 276.24.1139.13367 SSB / FV 10/02/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 22.44 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.003 B 999 240 VERT(CL): 0.006 B 999 180 HORZ(LL): -0.002 D - - HORZ(TL): 0.003 D - - Creep Factor: 2.0 Max TC CSI: 0.252 Max BC CSI: 0.154 Max Web CSI: 0.079 VIEW Ver: 23.02.04.0123.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A - /-86 /- /52 /88 /64 B* 111 /- /- /57 /40 /- E - /-86 /- /45 /60 /- Wind reactions based on MWFRS A Brg Wid = 7.3 Min Req = 1.5 (Truss) B Brg Wid = 104 Min Req = - E Brg Wid = 7.3 Min Req = 1.5 (Truss) Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/999.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

The overall height of this truss excluding overhang is 2-8-1.

Refer to DWG PB160220723 for piggyback details.



COA#0-278

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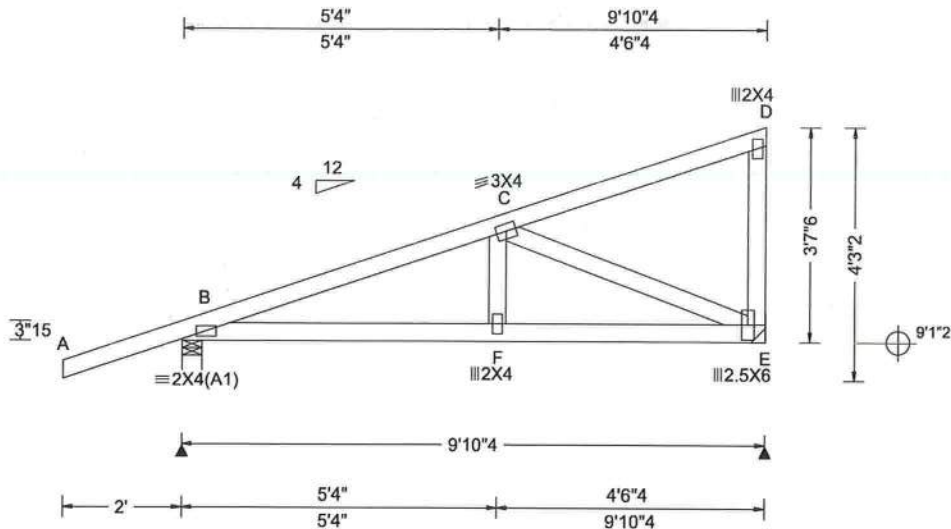
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCE: sbcecomponents.com; ICC: iccsafe.org; AWC: awc.org



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 780274 FROM: CDM	MONO Qty: 17	Job Number: 24-1632 VANMESBURGER Truss Label: J02	Cust: R 215 JRef: 1Y3R2150002 T10 DrwNo: 276.24.1139.10703 SSB / FV 10/02/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.014 F 999 240 VERT(CL): 0.027 F 999 180 HORZ(LL): 0.004 E - - HORZ(TL): 0.009 E - - Creep Factor: 2.0 Max TC CSI: 0.344 Max BC CSI: 0.266 Max Web CSI: 0.264 VIEW Ver: 23.02.04.0123.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 552 /- /- /328 /94 /135 E 378 /- /- /215 /88 /- Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 1.5 (Truss) E Brg Wid = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. B - C 323 -639

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 3-7-6.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens.Comp.
B - F	567 -490	F - E	561 -493

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.
C - E	529 -602



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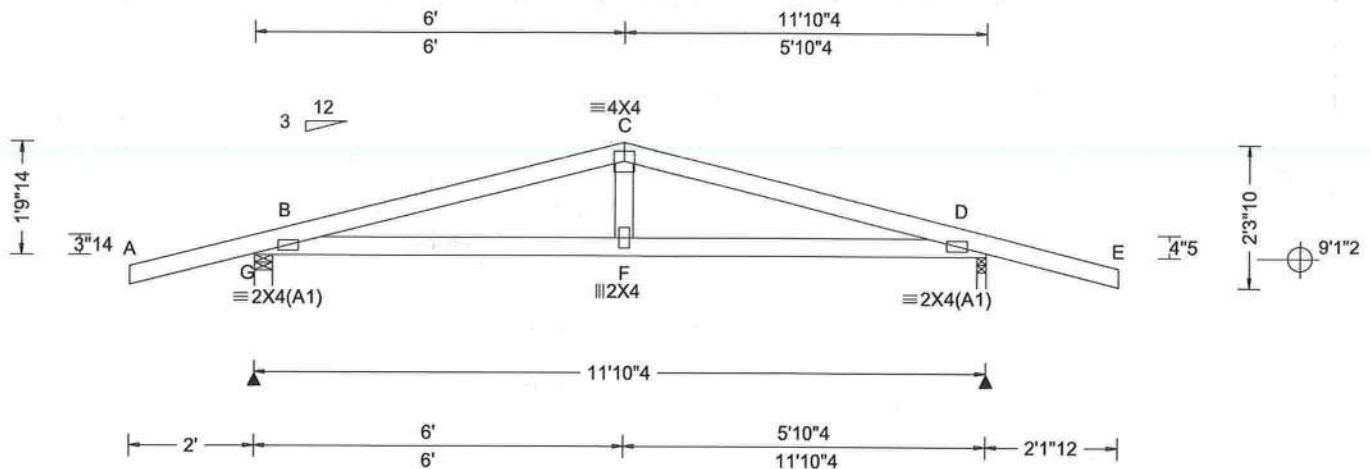
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 780280 FROM: CDM	COMN Qty: 5	Ply: 1 Job Number: 24-1632 VANMESBURGER Truss Label: B02	Cust: R215 JRef: 1Y3R2150002 T6 DrwNo: 276.24.1139.06270 SSB / FV 10/02/2024
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.035 F 999 240 VERT(CL): 0.066 F 999 180 HORZ(LL): 0.009 D - - HORZ(TL): 0.016 D - - Creep Factor: 2.0 Max TC CSI: 0.352 Max BC CSI: 0.410 Max Web CSI: 0.091 VIEW Ver: 23.02.04.0123.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL G 609 /- /- /320 /131 /34 D 615 /- /- /324 /137 /- Wind reactions based on MWFRS G Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 1.8 Min Req = 1.5 (Truss) Bearings G & D are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 690 -961 C - D 689 -962

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 1'-9-14.



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Gable Reinforcement

- (a) 2x3 "T" reinforcement. Same species and grade as web. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.
- (b) 2x6 "T" reinforcement. Same species and grade as web. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.
- (c) 2x3 "T" reinforcement. Any species and grade. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.
- (d) 2x4 "T" reinforcement. Same species and grade as web. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.
- (e) 2x4 SP/DF #2 or better "T" reinforcement. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.
- (f) 2x6 "T" reinforcement. Any species and grade. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.



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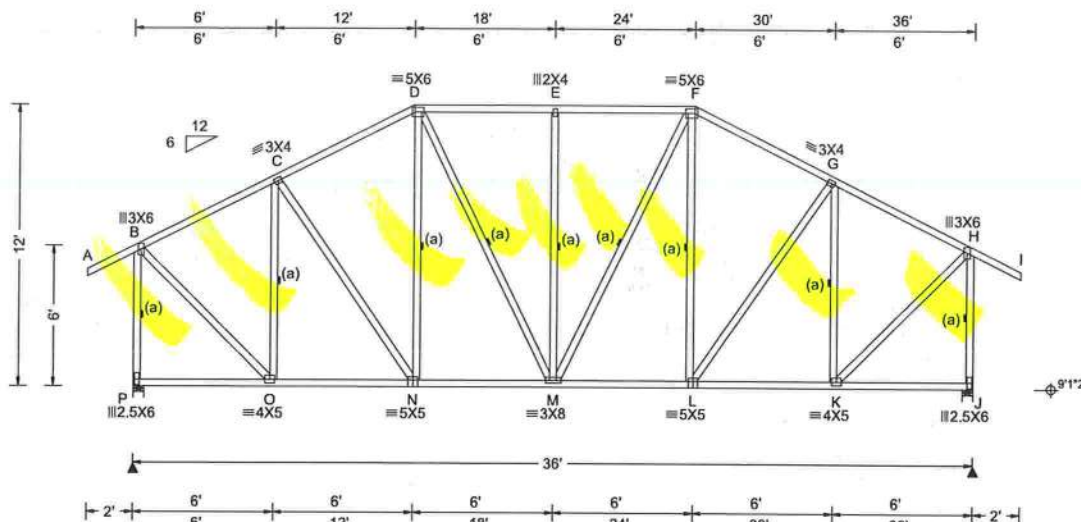
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity		Non-Gravity				
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.078 E 999 240	Loc	R+	/R-	/Rh	/Rw	/U	/RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.136 E 999 180	P	1893	/-	/-	/939	/113	/212
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.026 K - -	J	1892	/-	/-	/939	/113	/-
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.046 K - -	Wind reactions based on MWFRS						
NCBCLL: 10.00	Mean Height: 19.09 ft	Building Code:	Creep Factor: 2.0	P	Brg Wid = 5.5	Min Req = 2.2	(Truss)			
Soffit: 2.00	TCDL: 5.0 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.430	J	Brg Wid = 5.5	Min Req = 2.2	(Truss)			
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.545	Bearings P & J are a rigid surface.						
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.556	Members not listed have forces less than 375#						
	C&C Dist a: 3.60 ft	FT/RT:20(0)/10(0)		Maximum Top Chord Forces Per Ply (lbs)						
	Loc. from endwall: Any	Plate Type(s):		Chords	Tens.Comp.	Chords	Tens. Comp.			
	GCpi: 0.18	WAVE	VIEW Ver: 23.02.04.0123.14	B - C	265	-1251	E - F	314	-1420	
Lumber	Wind Duration: 1.60			C - D	333	-1501	F - G	333	-1500	
Top chords 2 x 12 @ 12'				D - E	314	-1420	G - H	265	-1251	

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 12'-0".



COA#0-278

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10/02/2024

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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

General Notes (continued)

Key to Terms (continued):

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high-quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Bearing Information:

The bearing area factor, C_b , is considered for the allowable capacity of solid sawn wood bearings supporting trusses that are located a minimum of 3" from the end of the lumber piece.