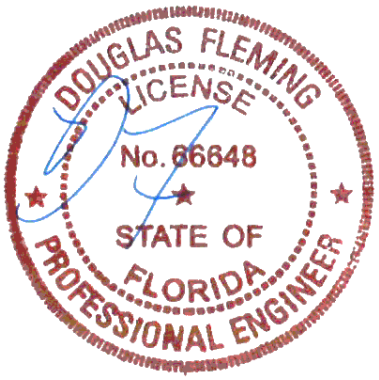

Alpine, an ITW Company
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025
Phone: (800)755-6001
www.alpineitw.com

COA #0 278
Florida Certificate of Product Approval #FL1999

06/24/2024

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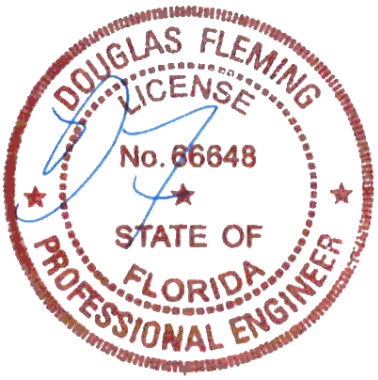
Site Information:	**Page 1:**
Customer: W. B. Howland Company, Inc.	*Job Number:* 24-1285
Job Description: Hedrick	
Address:	

Job Engineering Criteria:	
Design Code: FBC 8th Ed. 2023 Res.	*IntelliVIEW Version:* 23.02.01A through 24.01.02 *JRef #:* 1Y0W2150008
Wind Standard: ASCE 7-22 *Wind Speed (mph):* 130 *Building Type:* Closed	*Design Loading (psf):* 40.00

This package contains general notes pages, 65 truss drawing(s) and 7 detail(s).

Item	Drawing Number	Truss	Item	Drawing Number	Truss
1	176.24.0951.30440	A01	2	176.24.0951.55437	A02
3	176.24.1034.26650	A03	4	176.24.0952.36717	A04
5	176.24.0952.50517	A05	6	176.24.0953.00080	A06
7	176.24.0953.08510	A07	8	176.24.0953.18777	A08
9	176.24.1034.44910	A09	10	176.24.1034.48917	A10
11	176.24.0953.45597	A11	12	176.24.0953.52110	A12
13	176.24.0953.58257	A13	14	176.24.1034.10847	A14
15	176.24.1000.07233	B01	16	176.24.1000.12130	B02
17	176.24.1000.14647	B03	18	176.24.1000.53177	B04
19	173.24.1518.48422	C01	20	176.24.1000.54683	C02
21	176.24.1000.56980	C03	22	173.24.1518.48271	C04
23	173.24.1518.48302	D01	24	176.24.1001.00080	D02
25	176.24.1001.01473	D03	26	176.24.1001.02673	D04
27	176.24.1001.04190	D05	28	176.24.1001.35000	D06
29	173.24.1518.48224	E01	30	173.24.1518.48162	E03
31	176.24.1001.39987	J01	32	176.24.1001.43977	J01HJ
33	176.24.1001.47700	J02	34	176.24.1001.51767	J02HJ
35	176.24.1001.54670	J03	36	176.24.1001.57687	J03HJ
37	176.24.1002.00983	J04	38	176.24.1002.04980	J05
39	176.24.1002.07323	J06	40	176.24.1002.09547	J07
41	176.24.1002.12543	J08	42	173.24.1518.48427	J09
43	173.24.1518.48208	J10	44	176.24.1002.16027	J11
45	176.24.1002.18280	J12	46	173.24.1518.48444	J13
47	176.24.0951.01433	PB01	48	176.24.0951.02617	PB02
49	176.24.0951.04763	PB03	50	176.24.0951.23930	PB04



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Site Information:	**Page 2:**
Customer: W. B. Howland Company, Inc.	*Job Number:* 24-1285
Job Description: Hedrick	
Address:	

Item	Drawing Number	Truss
51	176.24.1002.20990	V01
53	176.24.1002.23547	V03
55	176.24.1003.51183	V05
57	176.24.1003.56050	V07
59	176.24.1003.59853	V09
61	176.24.1004.12467	V11
63	176.24.1004.14680	V13
65	176.24.1004.18633	V15
67	BRCLBSUB0119	
69	PB160220723	
71	VALTN220723	

Item	Drawing Number	Truss
52	176.24.1002.22150	V02
54	176.24.1003.49680	V04
56	176.24.1003.54010	V06
58	176.24.1003.57320	V08
60	176.24.1004.09157	V10
62	176.24.1004.13513	V12
64	176.24.1004.16837	V14
66	160TL	
68	DEFLCAMB1014	
70	VAL180220723	
72	CNNAILSP1014	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:
The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high-quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:
Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:
The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

Connector Plate Information:
Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:
Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:
Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.
BCLL = Bottom Chord standard design Live Load in pounds per square foot.
C = Coated lumber.
C-AT = AtTEK coated lumber.
C-FX = FX Lumber Guard coated lumber.
C -TW = TechWood 4400 coated lumber.
CL = Certified lumber.
Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.
FRT = Fire Retardant Treated lumber.
FRT-DB = D-Blaze Fire Retardant Treated lumber.
FRT-DC = Dricon Fire Retardant Treated lumber.
FRT-FP = FirePRO Fire Retardant Treated lumber.
FRT-FL = FlamePRO Fire Retardant Treated lumber.
FRT-FT = FlameTech Fire Retardant Treated lumber.
FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.
FRT-PR = ProWood Fire Retardant Treated lumber.
g = green lumber.
HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.
HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.
HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.
Ic = Incised lumber.
FJ = Finger Jointed lumber.
L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.
L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.
Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.
Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for all load cases.
Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for all load cases.
Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for all load cases.
NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.
PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.
PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds
PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.
PP = Panel Point.
R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).
Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).
TCDL = Top Chord standard design Dead Load in pounds per square foot.
TCLL = Top Chord standard design Live Load in pounds per square foot.
U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.
VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.
VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.
VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.
W = Width of non-hanger bearing, in inches.

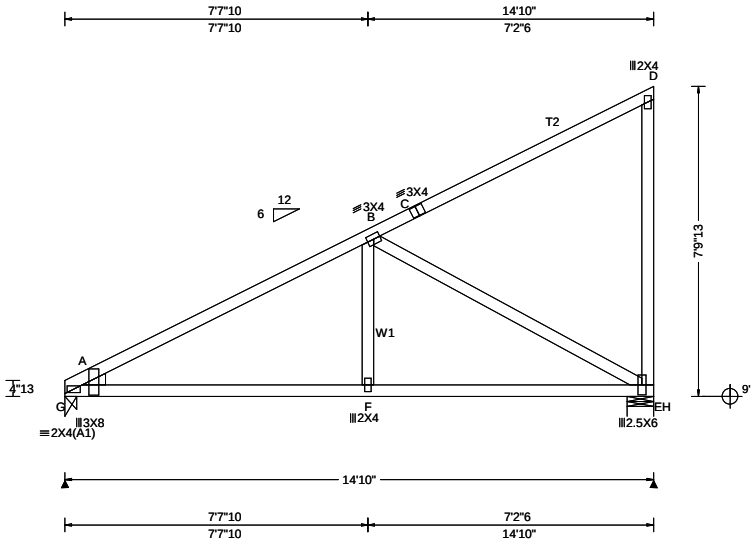
General Notes (continued)

Refer to ASCE-7 for Wind and Seismic abbreviations.
Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www. sbcacomponents.com

SEQN: 26145 SPEC FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** A01	Cust: R 215 JRef:1Y0W2150008 T11 DrwNo: 176.24.0951.30440 NW / DF 06/24/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.026	A	999	240
VERT(CL): 0.054	A	999	180
HORZ(LL): 0.014	A	-	-
HORZ(TL): 0.029	A	-	-

Creep Factor: 2.0
Max TC CSI: 0.576
Max BC CSI: 0.706
Max Web CSI: 0.892

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
G	617	/-	/-	/373	/93	/336
H	605	/-	/-	/450	/120	/-

Wind reactions based on MWFRS
G Brg Wid = 3.5 Min Req = 1.5 (Truss)
H Brg Wid = 8.0 Min Req = 1.5 (Truss)
Bearings G & H are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
A - B	272	- 850

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - F	697	- 420	F - E	693	- 421

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.
B - E	358	- 793

Lumber

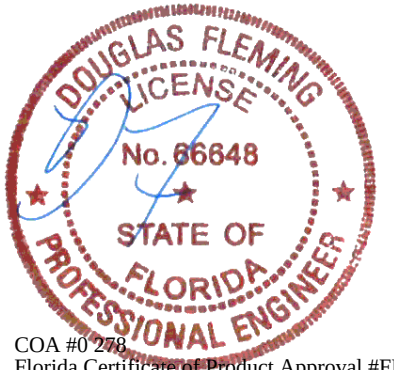
Top chord: 2x4 SP M-31; T2 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2; W1 2x4 SP #3;
Lt Wedge: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



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06/24/2024

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

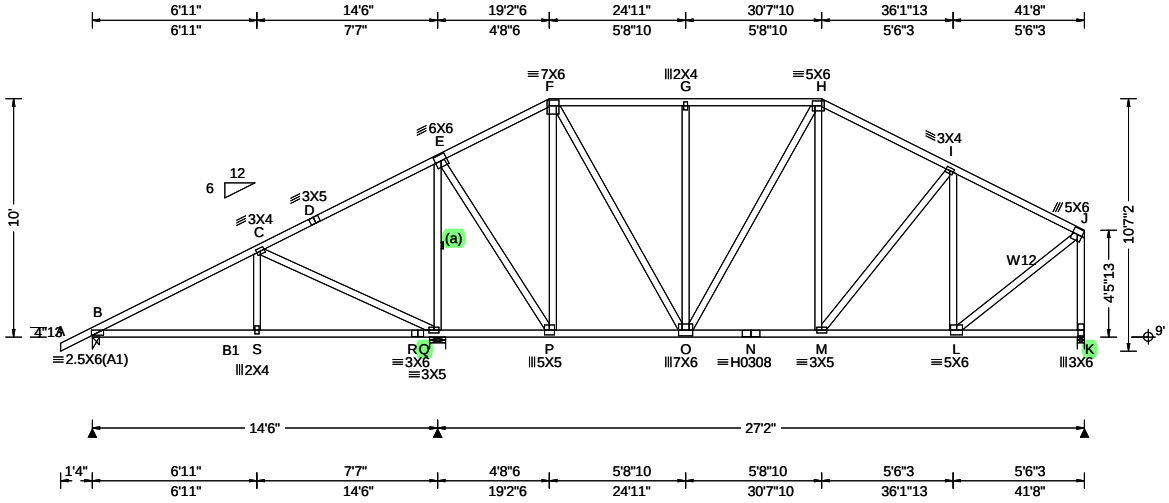
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26464 SPEC FROM:	Ply: 2 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** A02	Cust: R 215 JRef:1Y0W2150008 T2 DrwNo: 176.24.0951.55437 NW / DF 06/24/2024	

2 Complete Trusses Required



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	0.00
Soffit:	2.00
Load Duration:	1.25
Spacing:	24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.30 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 4.17 ft ft
Loc. from endwall: not in 6.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE, HS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.074	M	999	240
VERT(CL): 0.146	M	999	180
HORZ(LL): 0.020	L	-	-
HORZ(TL): 0.039	L	-	-

Creep Factor: 2.0
Max TC CSI: 0.383
Max BC CSI: 0.655
Max Web CSI: 0.808

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	620	/-	/-	/-	/154	/131
Q	6317	/-	/-	/-	/1023	/-
K	5862	/-	/-	/-	/966	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
Q Brg Wid = 8.0 Min Req = 2.6 (Truss)
K Brg Wid = 3.5 Min Req = 2.4 (Truss)
Bearings B, Q, & K are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
E - F	252	- 1351	H - I	394	- 2221
F - G	334	- 1915	I - J	376	- 2104
G - H	334	- 1915			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
P - O	1171	- 148	N - M	1949	- 274
O - N	1949	- 274	M - L	1865	- 267

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
Q - E	459	- 2606	H - M	869	- 113
E - P	2120	- 335	L - J	2348	- 372
F - P	155	- 812	J - K	419	- 2439
F - O	1463	- 240			

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP M-31; B1 2x4 SP #2;
Webs: 2x4 SP #3; W12 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Nailnote

Nail Schedule:0.131"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @ 6.75" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	62 plf at	-1.33 to	62 plf at	41.67
BC: From	4 plf at	-1.33 to	4 plf at	0.00
BC: From	20 plf at	0.00 to	20 plf at	13.67
BC: From	10 plf at	13.67 to	10 plf at	41.67

BC: 683 lb Conc. Load at 15.40,17.40,19.40,21.40 23.40,25.40,27.40,29.40,31.40,33.40,35.40,37.40 39.40,41.27

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads and reactions based on MWFRS.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

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06/24/2024

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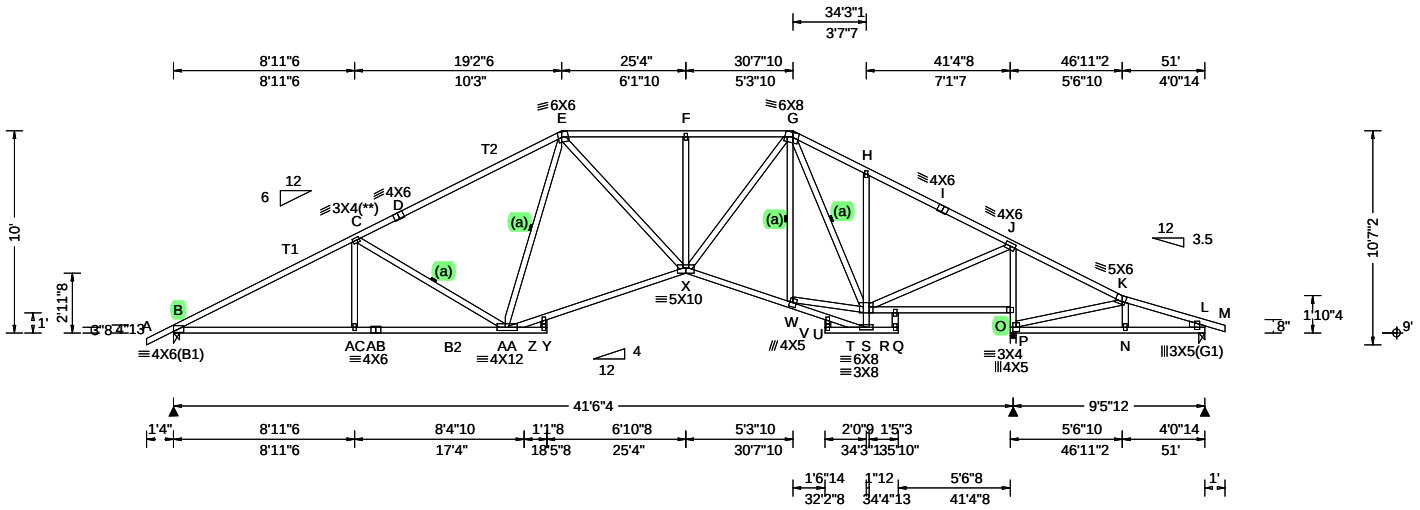
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 34735 COMN FROM:	Ply: 1 Qty: 2	**Job Number:** 24-1285 Hedrick **Truss Label:** A03	Cust: R 215 JRef:1Y0W2150008 T13 DrwNo: 176.24.1034.26650 NW / DF 06/24/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.30 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 5.10 ft ft
Loc. from endwall: not in 13.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.193	Y	999	240
VERT(CL): 0.390	Y	999	180
HORZ(LL): 0.077	S	-	-
HORZ(TL): 0.158	S	-	-

Creep Factor: 2.0
Max TC CSI: 0.864
Max BC CSI: 0.882
Max Web CSI: 0.820

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs)

Loc	Gravity R+	/ R-	/ Rh	Non-Gravity / Rw	/ U	/ RL
B	1790	/-	/-	/1135	/-	/292
O	2269	/-	/-	/1295	/-	/-
L	364	/-	/-	/220	/51	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 2.1 (Truss)
O Brg Wid = 3.5 Min Req = 2.7 (Truss)
L Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B, O, & L are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	347	-3086	G - H	322	-1689
C - D	214	-2356	H - I	182	-1632
D - E	255	-2298	I - J	163	-1727
E - F	116	-2450	J - K	386	-3
F - G	116	-2450	K - L	176	-474

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B -AC	2663	-271	Z - X	2124	0
AC-AB	2660	-272	X - W	1652	0
AB-AA	2660	-272	O - N	410	-130
AA- Z	2070	0	N - L	420	-125

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C -AA	339	-807	S - J	1772	0
E - X	685	0	J - P	196	-2002
X - G	1458	0	P - O	168	-2084
W - G	0	-420	O - K	174	-593
W - S	1546	0			

Lumber
Top chord: 2x4 SP #2; T1,T2 2x4 SP M-31;
Bot chord: 2x4 SP #2; B2 2x4 SP M-31;
Webs: 2x4 SP #3;
Rt Stub Wedge: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

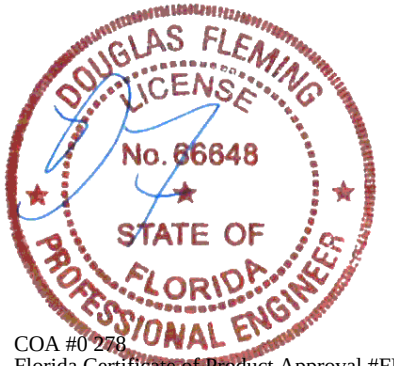
Plating Notes
All plates are 2X4 except as noted.
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes
WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Laterally brace top chord below filler and bottom chord above filler at 24" o.c.,including a lateral brace at chord ends (If no rigid diaphragm exists at that point)



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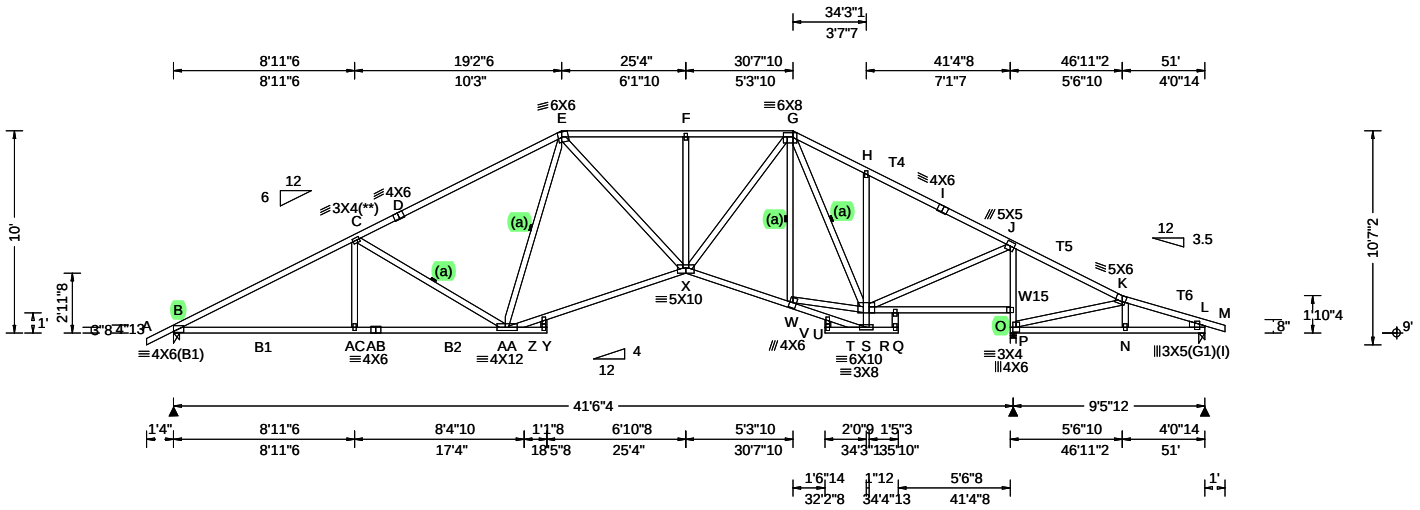
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 34737 COMN FROM:	Ply: 1 Qty: 3	**Job Number:** 24-1285 Hedrick **Truss Label:** A04	Cust: R 215 JRef:1Y0W2150008 T29 DrwNo: 176.24.0952.36717 NW / DF 06/24/2024



Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00		Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00		Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.199 Y 999 240
BCLL: 0.00		Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.406 Y 999 180
BCDL: 10.00		Risk Category: II	Snow Duration: NA	HORZ(LL): 0.079 S - -
Des Ld: 40.00		EXP: C Kzt: NA		HORZ(TL): 0.162 S - -
NCBCLL: 10.00		Mean Height: 15.30 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00		TCDL: 5.0 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.824
Load Duration: 1.25		BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.812
Spacing: 24.0 "		MWFRS Parallel Dist: h to 2h	Rep Fac: Varies by Ld Case	Max Web CSI: 0.814
		C&C Dist a: 5.10 ft ft	FT/RT:20(0)/10(0)	
		Loc. from endwall: not in 13.00 ft	Plate Type(s):	VIEW Ver: 23.02.01A.1204.18
		GCpi: 0.18	WAVE	
		Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1899	/-	/-	/1136	/343	/292
O	2795	/-	/-	/1293	/501	/-
L	527	/-	/-	/222	/96	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.6 (Truss)
O Brg Wid = 3.5 Min Req = 3.3 (Truss)
L Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B, O, & L are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	604	- 3318	G - H	339	- 1967
C - D	459	- 2596	H - I	322	- 1932
D - E	436	- 2538	I - J	360	- 2027
E - F	485	- 2867	J - K	486	- 112
F - G	485	- 2868	K - L	175	- 937

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B -AC	2869	- 501	Z - X	2375	- 425
AC-AB	2867	- 503	X - W	2021	- 347
AB-AA	2867	- 503	O - N	858	- 129
AA- Z	2319	- 420	N - L	864	- 124

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C -AA	334	- 797	G - S	106	- 499
E - X	950	- 134	S - J	2136	- 365
X - G	1567	- 260	J - P	501	- 2365
F - X	145	- 405	P - O	475	- 2452
W - G	127	- 531	O - K	186	- 1143
W - S	1894	- 310			

Lumber

Top chord: 2x4 SP M-31; T4,T5,T6 2x4 SP #2;
Bot chord: 2x4 SP #2; B1,B2 2x4 SP M-31;
Webs: 2x4 SP #3; W15 2x4 SP #2;
Rt Stub Wedge: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Special Loads

------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	62 plf at	-1.33 to	62 plf at	46.93
TC: From	61 plf at	46.93 to	61 plf at	52.00
BC: From	4 plf at	-1.33 to	4 plf at	0.00
BC: From	20 plf at	0.00 to	20 plf at	18.46
BC: From	21 plf at	18.46 to	21 plf at	33.33
BC: From	20 plf at	33.33 to	20 plf at	51.00
BC: From	4 plf at	51.00 to	4 plf at	52.00

TC: 400 lb Conc. Load at 29.48,46.40

Plating Notes

All plates are 2X4 except as noted.

(I) - plates so marked were sized using 0% Fabrication Tolerance, 0 degrees Rotational Tolerance, and/or zero Positioning Tolerance.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

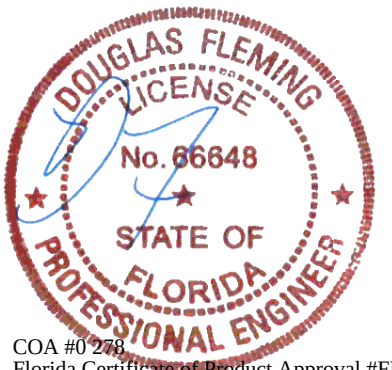
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Laterally brace top chord below filler and bottom chord above filler at 24" o.c.,including a lateral brace at chord ends (If no rigid diaphragm exists at that point)



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06/24/2024

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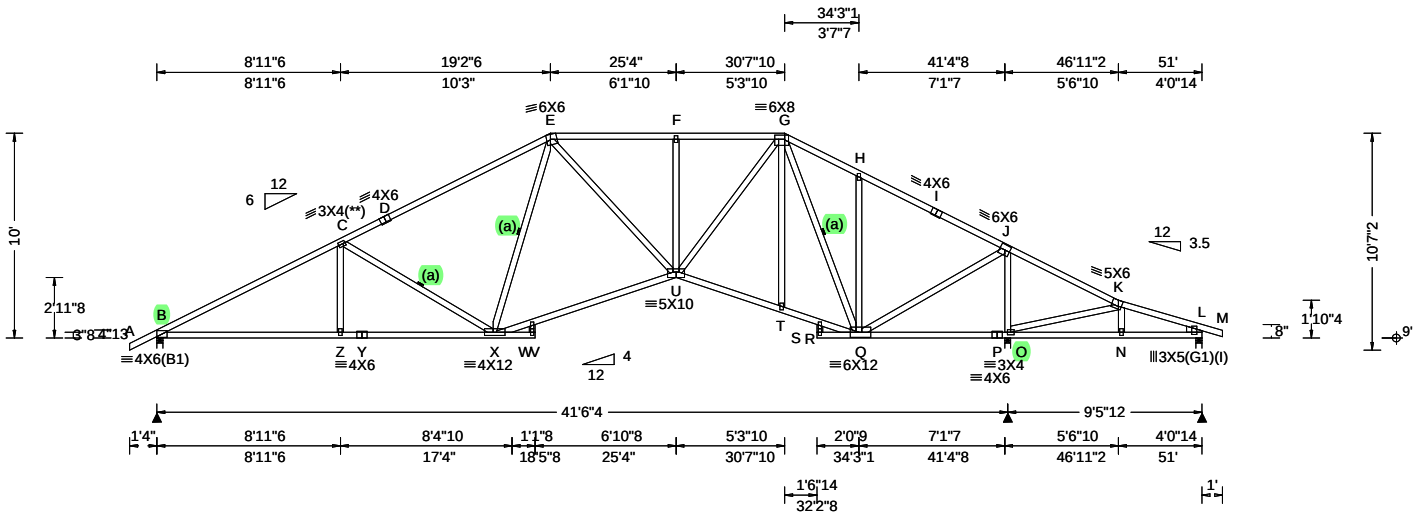
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 34739 COMN FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** A05	Cust: R 215 JRef:1Y0W2150008 T19 DrwNo: 176.24.0952.50517 NW / DF 06/24/2024



Loading Criteria (psf)	
TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00
Load Duration:	1.25
Spacing:	24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.30 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 5.10 ft ft
Loc. from endwall: not in 13.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	0.164 V	999	240
VERT(CL):	0.337 V	999	180
HORZ(LL):	0.067 Q	-	-
HORZ(TL):	0.138 Q	-	-

Creep Factor: 2.0
Max TC CSI: 0.677
Max BC CSI: 0.383
Max Web CSI: 0.561

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1808	/-	/-	/1096	/328	/292
O	3333	/-	/-	/1503	/588	/-
L	200	/-144	/-	/62	/114	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
O Brg Wid = 3.5 Min Req = 2.4 (Truss)
L Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B, O, & L are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	570	- 3126	G - H	298	- 1240
C - D	425	- 2395	H - I	199	- 1198
D - E	402	- 2337	I - J	236	- 1292
E - F	425	- 2531	J - K	1554	- 296
F - G	426	- 2532	K - L	612	- 83

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - Z	2698	- 471	T - R	1661	- 283
Z - Y	2696	- 472	R - Q	1641	- 280
Y - X	2696	- 472	Q - P	199	- 1144
X - W	2112	- 384	P - O	199	- 1144
W - U	2168	- 389	O - N	58	- 573
U - T	1680	- 287	N - L	63	- 563

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - X	334	- 808	G - Q	262	- 1460
E - U	744	- 97	Q - J	2515	- 436
U - G	1549	- 257	J - O	605	- 2927
F - U	144	- 405	O - K	180	- 1024

Lumber

Top chord: 2x4 SP M-31;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP M-31;
Rt Stub Wedge: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Special Loads

------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	62 plf at	-1.33 to	62 plf at	46.93
TC: From	61 plf at	46.93 to	61 plf at	52.00
BC: From	4 plf at	-1.33 to	4 plf at	0.00
BC: From	20 plf at	0.00 to	20 plf at	18.46
BC: From	21 plf at	18.46 to	21 plf at	33.33
BC: From	20 plf at	33.33 to	20 plf at	51.00
BC: From	4 plf at	51.00 to	4 plf at	52.00

TC: 400 lb Conc. Load at 29.48,46.40

Plating Notes

All plates are 2X4 except as noted.

(I) - plates so marked were sized using 0% Fabrication Tolerance, 0 degrees Rotational Tolerance, and/or zero Positioning Tolerance.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

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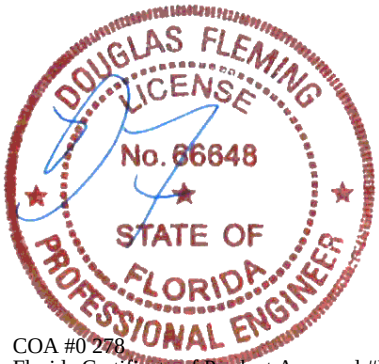
Laterally brace top chord below filler and bottom chord above filler at 24" o.c.,including a lateral brace at chord ends (If no rigid diaphragm exists at that point)



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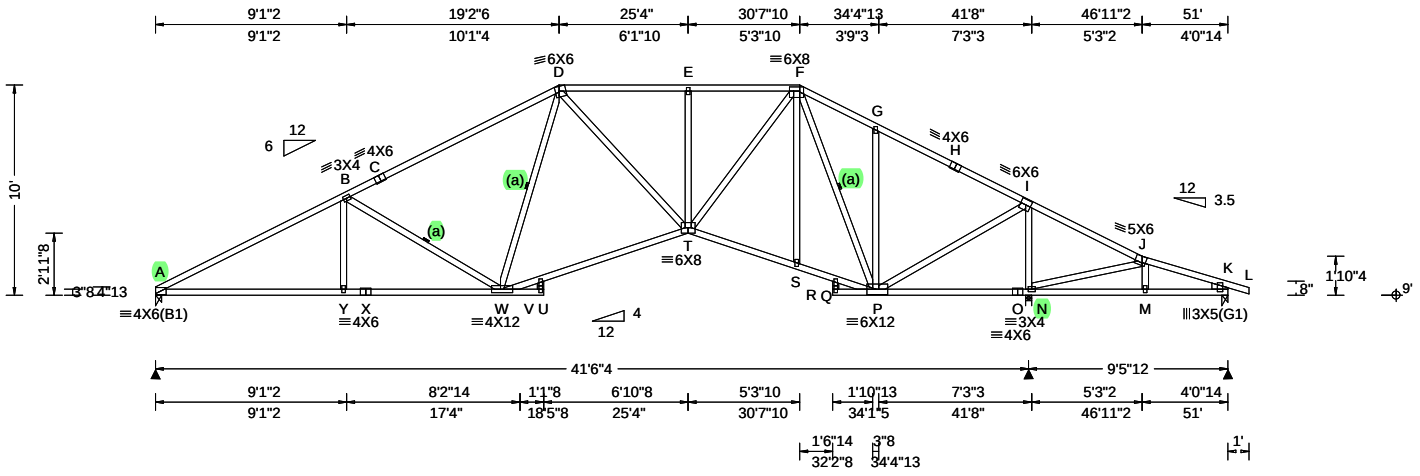
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 34730 SPEC FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** A06	Cust: R 215 JRef:1Y0W2150008 T43 DrwNo: 176.24.0953.00080 NW / DF 06/24/2024



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.163 U 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.335 U 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.066 P - -
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.137 P - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.669
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.382
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Varies by Ld Case	Max Web CSI: 0.559
	C&C Dist a: 5.10 ft ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 13.00 ft	Plate Type(s):	VIEW Ver: 23.02.01A.1204.18
	GCpi: 0.18	WAVE	
	Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	1712	/-	/-	/1013	/298	/268
N	3320	/-	/-	/1491	/590	/-
K	203	/-139	/-	/74	/98	/-

Wind reactions based on MWFRS
A Brg Wid = 3.5 Min Req = 1.5 (Truss)
N Brg Wid = 3.5 Min Req = 2.4 (Truss)
K Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings A, N, & K are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	839	- 3131	F - G	590	- 1238
B - C	721	- 2390	G - H	453	- 1196
C - D	764	- 2344	H - I	434	- 1291
D - E	864	- 2519	I - J	1542	- 294
E - F	864	- 2520	J - K	599	- 259

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - Y	2704	- 708	S - Q	1655	- 289
Y - X	2702	- 710	Q - P	1636	- 285
X - W	2702	- 710	P - O	303	- 1152
W - V	2104	- 494	O - N	303	- 1152
V - T	2160	- 500	N - M	258	- 560
T - S	1675	- 293	M - K	261	- 550

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - W	312	- 843	F - P	267	- 1558
D - T	831	- 121	P - I	2501	- 538
T - F	1538	- 434	I - N	771	- 2913
E - T	267	- 451	N - J	178	- 1025

Lumber

Top chord: 2x4 SP M-31;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP M-31;
Rt Stub Wedge: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Special Loads

------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	62 plf at	0.00 to	62 plf at	19.20
TC: From	61 plf at	19.20 to	61 plf at	30.64
TC: From	62 plf at	30.64 to	62 plf at	46.93
TC: From	61 plf at	46.93 to	61 plf at	52.00
BC: From	20 plf at	0.00 to	20 plf at	18.46
BC: From	21 plf at	18.46 to	21 plf at	33.33
BC: From	20 plf at	33.33 to	20 plf at	51.00
BC: From	4 plf at	51.00 to	4 plf at	52.00

TC: 400 lb Conc. Load at 29.48,46.41

Plating Notes

All plates are 2X4 except as noted.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

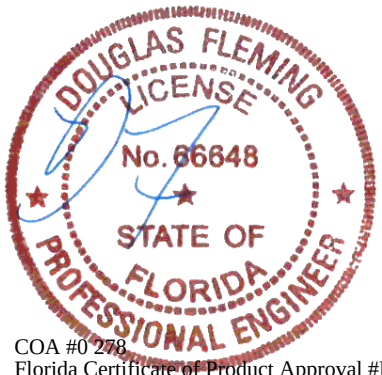
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Laterally brace top chord below filler and bottom chord above filler at 24" o.c.,including a lateral brace at chord ends (If no rigid diaphragm exists at that point)



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06/24/2024

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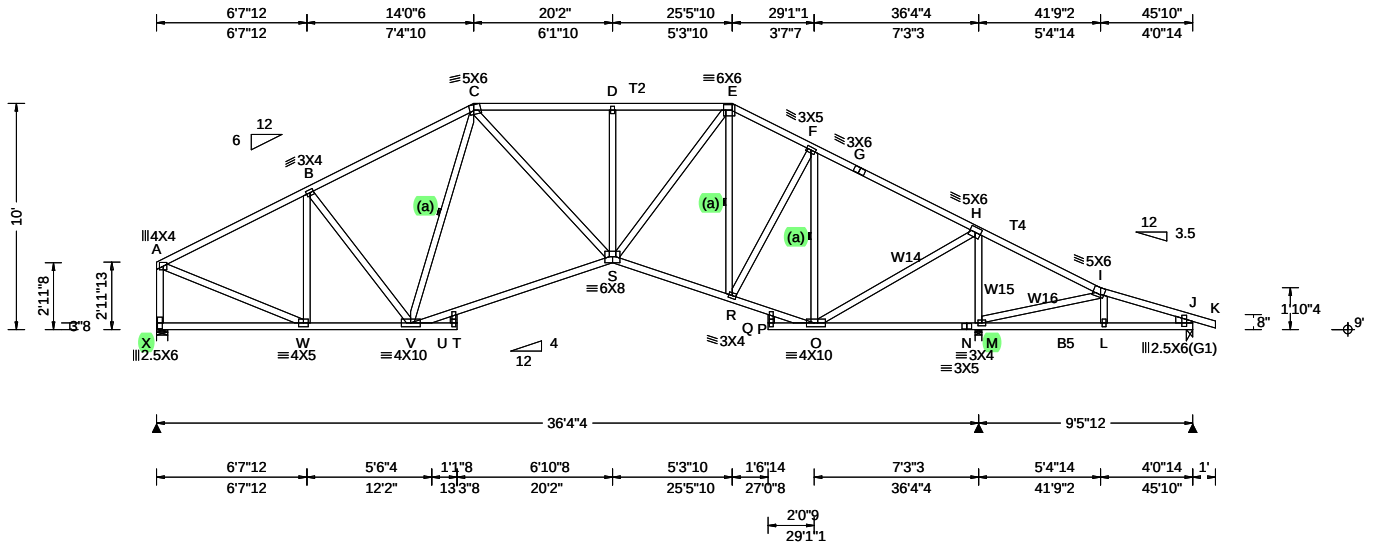
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 34725 SPEC FROM:	Ply: 1 Qty: 3	**Job Number:** 24-1285 Hedrick **Truss Label:** A07	Cust: R 215 JRef:1Y0W2150008 T51 DrwNo: 176.24.0953.08510 NW / DF 06/24/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 4.58 ft ft
Loc. from endwall: not in 13.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	0.131 T	999	240
VERT(CL):	0.308 T	999	180
HORZ(LL):	0.053 O	-	-
HORZ(TL):	0.109 O	-	-

Creep Factor: 2.0
Max TC CSI: 0.746
Max BC CSI: 0.740
Max Web CSI: 0.583

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
X	1531	/-	/-	/850	/267	/256
M	2923	/-	/-	/1327	/517	/-
J	289	/-	/-	/108	/53	/-

Wind reactions based on MWFRS
X Brg Wid = 6.0 Min Req = 1.8 (Truss)
M Brg Wid = 3.5 Min Req = 2.0 (Truss)
J Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings X, M, & J are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	498	- 1683	E - F	603	- 1639
B - C	619	- 1723	F - G	433	- 1093
C - D	759	- 2147	G - H	423	- 1238
D - E	759	- 2147	H - I	1156	- 223

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
W - V	1456	- 337	R - P	1097	- 187
V - U	1597	- 337	P - O	1082	- 185
U - S	1651	- 340	O - N	214	- 831
S - R	1514	- 266	N - M	214	- 831

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
A - X	445	- 1470	R - E	190	- 901
A - W	1530	- 405	R - F	925	- 152
W - B	209	- 480	F - O	325	- 1265
V - C	146	- 393	O - H	2063	- 414
C - S	934	- 170	H - M	655	- 2504
D - S	295	- 475	M - I	184	- 1035
S - E	1182	- 334			

Lumber

Top chord: 2x4 SP #2; T2,T4 2x4 SP M-31;
Bot chord: 2x4 SP #2; B5 2x4 SP M-31;
Webs: 2x4 SP #3; W14,W15,W16 2x4 SP M-31;
Rt Stub Wedge: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Special Loads

------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	62 plf at	0.00 to	62 plf at	14.03	
TC: From	61 plf at	14.03 to	61 plf at	25.47	
TC: From	62 plf at	25.47 to	62 plf at	41.76	
TC: From	61 plf at	41.76 to	61 plf at	46.83	
BC: From	20 plf at	0.00 to	20 plf at	13.29	
BC: From	21 plf at	13.29 to	21 plf at	28.17	
BC: From	20 plf at	28.17 to	20 plf at	45.83	
BC: From	4 plf at	45.83 to	4 plf at	46.83	

TC: 400 lb Conc. Load at 24.31,41.25

Plating Notes

All plates are 2X4 except as noted.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Laterally brace top chord below filler and bottom chord above filler at 24" o.c.,including a lateral brace at chord ends (If no rigid diaphragm exists at that point)

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

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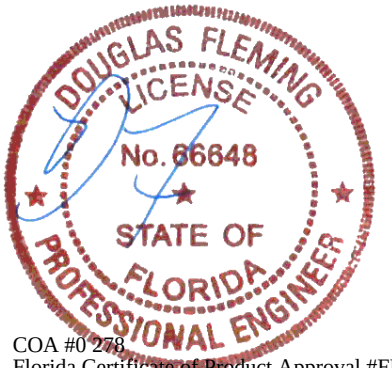

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06/24/2024

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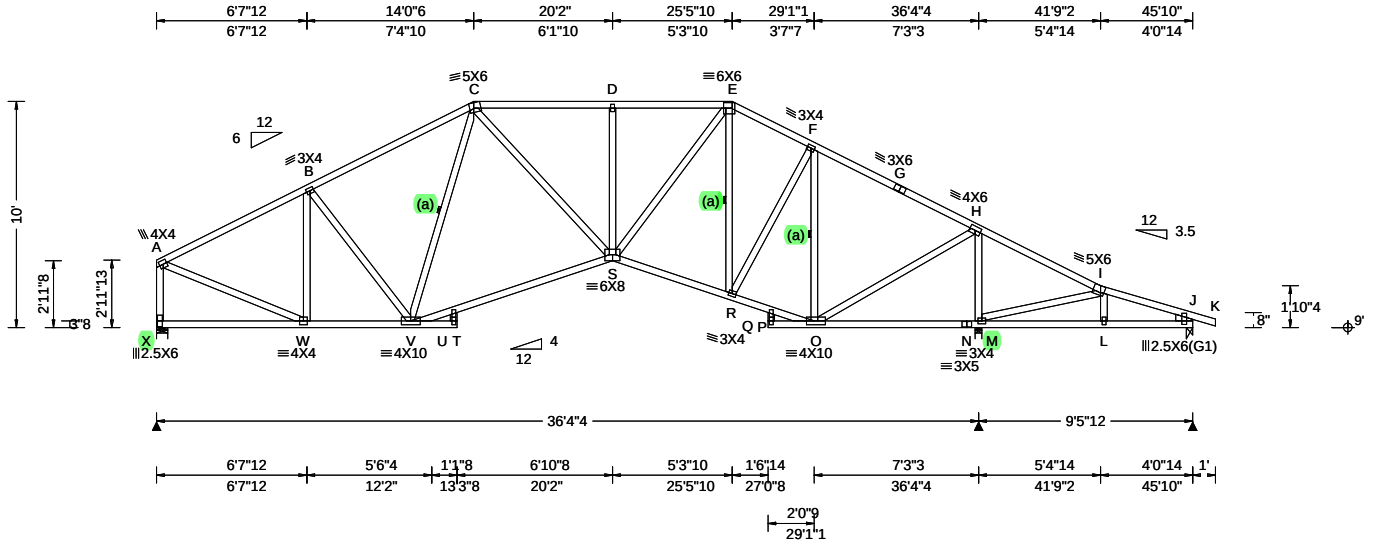
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 34741 SPEC FROM:	Ply: 1 Qty: 2	**Job Number:** 24-1285 Hedrick **Truss Label:** A08	Cust: R 215 JRef:1Y0W2150008 T34 DrwNo: 176.24.0953.18777 NW / DF 06/24/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 4.58 ft ft
Loc. from endwall: not in 13.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.128	T	999	240
VERT(CL): 0.319	T	999	180
HORZ(LL): 0.047	O	-	-
HORZ(TL): 0.098	O	-	-

Creep Factor: 2.0
Max TC CSI: 0.653
Max BC CSI: 0.740
Max Web CSI: 0.749

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
X	1445	/-	/-	/856	/40	/256
M	2230	/-	/-	/1305	/113	/-
J	249	/-4	/-	/126	/42	/-

Wind reactions based on MWFRS
X Brg Wid = 6.0 Min Req = 1.7 (Truss)
M Brg Wid = 3.5 Min Req = 2.3 (Truss)
J Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings X, M, & J are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	501	-1577	E - F	611	-1414
B - C	622	-1592	F - G	442	-1061
C - D	766	-1877	G - H	423	-1155
D - E	766	-1877	H - I	702	-162

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
W - V	1359	-340	R - P	1019	-183
V - U	1453	-340	P - O	1005	-180
U - S	1506	-343	O - N	196	-473
S - R	1284	-235	N - M	196	-473

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
A - X	447	-1384	R - E	141	-588
A - W	1427	-408	R - F	624	-106
W - B	210	-441	F - O	324	-976
C - S	733	-174	O - H	1553	-403
D - S	294	-381	H - M	642	-1959
S - E	1099	-333	M - I	164	-530

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Rt Stub Wedge: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 2X4 except as noted.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes
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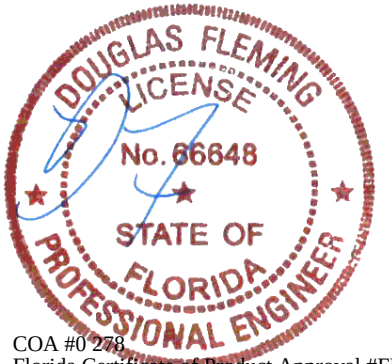

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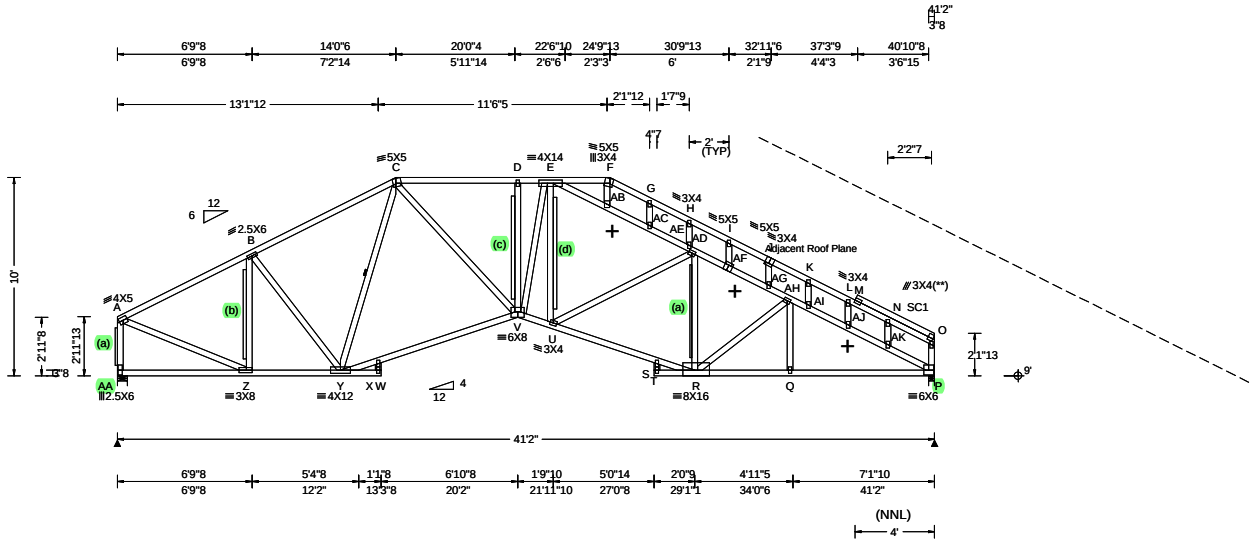
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26539 GABL FROM: Page 1 of 2	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** A09	Cust: R 215 JRef:1Y0W2150008 T48 DrwNo: 176.24.1034.44910 NW / DF 06/24/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 4.12 ft ft
Loc. from endwall: not in 13.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.234	F	999	240
VERT(CL): 0.484	F	999	180
HORZ(LL): 0.170	P	-	-
HORZ(TL): 0.351	P	-	-

Creep Factor: 2.0
Max TC CSI: 0.702
Max BC CSI: 0.856
Max Web CSI: 0.889

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
AA	1703	/-	/-	/976	/-	/265
P	1703	/-	/-	/1002	/-	/-

Wind reactions based on MWFRS
AA Brg Wid = 6.0 Min Req = 2.0 (Truss)
P Brg Wid = 3.5 Min Req = 2.0 (Truss)
Bearings AA & P are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	247	- 1893	C - D	219	- 2635
B - C	306	- 1983	D - E	219	- 2635

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
Z - Y	1646	- 22	U - S	2581	- 121
Y - X	1887	0	S - R	2530	- 121
X - V	1943	0	R - Q	2730	- 480
V - U	2572	0	Q - P	2735	- 479

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
A -AA	226	- 1642	AE- R	5	- 424
A - Z	1732	- 164	AE-AF	309	- 2636
Z - B	116	- 558	R -AH	504	- 402
Y - C	0	- 506	AF-AG	306	- 2663
C - V	1215	0	AG-AH	344	- 2706
V - E	858	0	AH-AI	526	- 2950
E -AB	9	- 2705	AI-AJ	583	- 2997
AB-AC	0	- 2729	AJ-AK	576	- 3030
AC-AD	8	- 2747	AK- P	563	- 3075
AD-AE	41	- 2837			

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins

In lieu of structural panels use purlins to brace TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

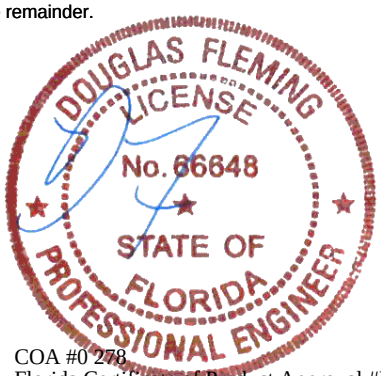
Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/566.

Laterally brace top chord below filler and bottom chord above filler at 24" o.c.,including a lateral brace at chord ends (If no rigid diaphragm exists at that point)

+ Member to be laterally braced for out of plane wind loads

Gable Reinforcement

(a) 1x4 "L" reinforcement. Any species and grade. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.
(b) 2x6 "L" reinforcement. Same species and grade as web. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.
(c) 2x4 "L" reinforcement. Any species and grade. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.
(d) 2x6 "L" reinforcement. Any species and grade. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.



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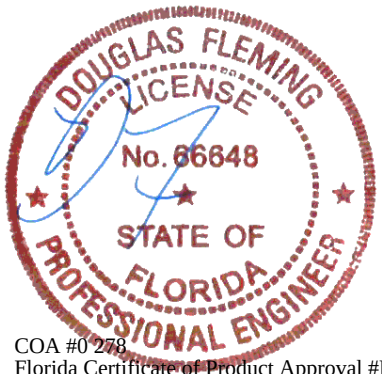
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

Laterally brace top chord below filler and bottom chord above filler at 24" o.c.,including a lateral brace at chord ends (If no rigid diaphragm exists at that point)



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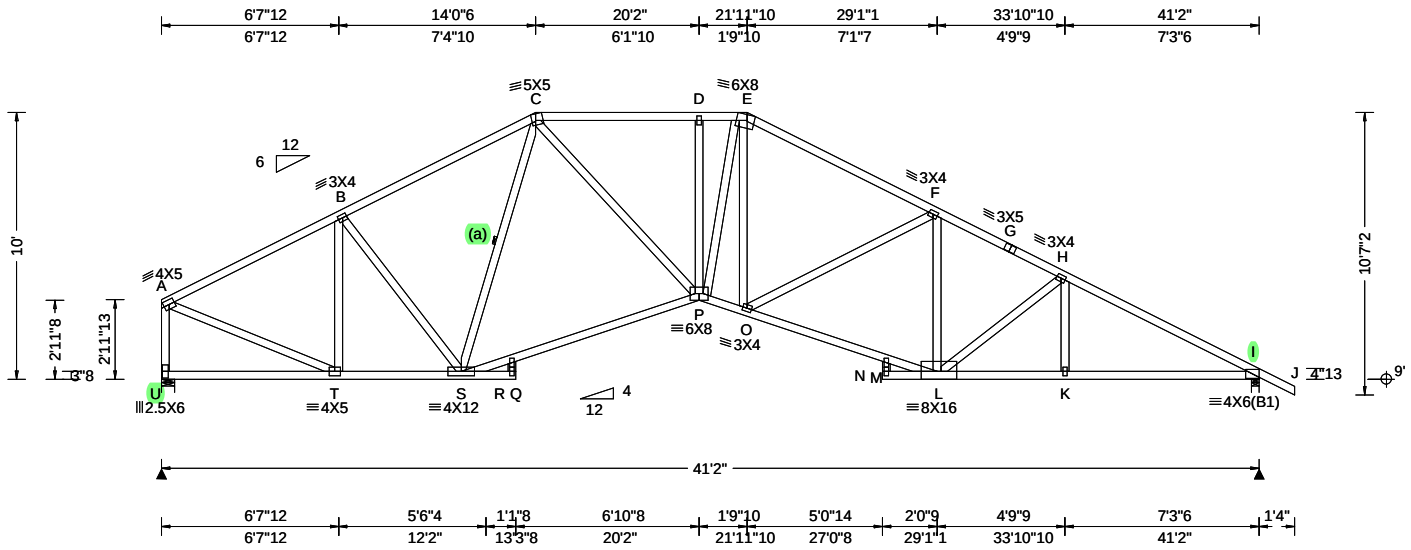
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26245 COMN FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** A10	Cust: R 215 JRef:1Y0W2150008 T39 DrwNo: 176.24.1034.48917 NW / DF 06/24/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 4.12 ft ft
Loc. from endwall: not in 13.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.191	Q	999	240
VERT(CL): 0.420	Q	999	180
HORZ(LL): 0.091	I	-	-
HORZ(TL): 0.188	I	-	-

Creep Factor: 2.0
Max TC CSI: 0.774
Max BC CSI: 0.841
Max Web CSI: 0.656

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs)

Loc	Gravity R+	/ R-	/ Rh	Non-Gravity / Rw	/ U	/ RL
U	1695	/-	/-	/963	/-	/270
I	1800	/-	/-	/1117	/-	/-

Wind reactions based on MWFRS
U Brg Wid = 6.0 Min Req = 2.0 (Truss)
I Brg Wid = 3.5 Min Req = 2.1 (Truss)
Bearings U & I are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	238	- 1883	E - F	265	- 2779
B - C	296	- 1971	F - G	366	- 2669
C - D	234	- 2611	G - H	352	- 2724
D - E	235	- 2611	H - I	379	- 3130

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
T - S	1637	- 51	O - M	2543	- 129
S - R	1874	0	M - L	2493	- 128
R - P	1930	0	L - K	2701	- 229
P - O	2536	0	K - I	2703	- 228

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
A - U	226	- 1634	C - P	1198	0
A - T	1722	- 156	P - E	832	0
T - B	113	- 555	F - L	11	- 398
S - C	0	- 499	L - H	154	- 410

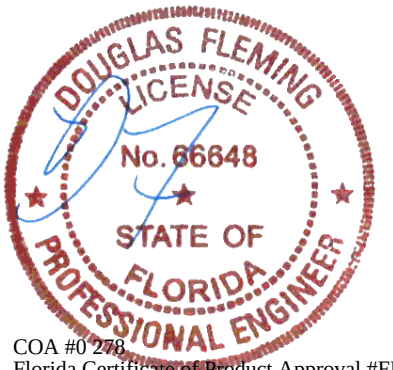
Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 2X4 except as noted.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.
Laterally brace top chord below filler and bottom chord above filler at 24" o.c.,including a lateral brace at chord ends (If no rigid diaphragm exists at that point)



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06/24/2024

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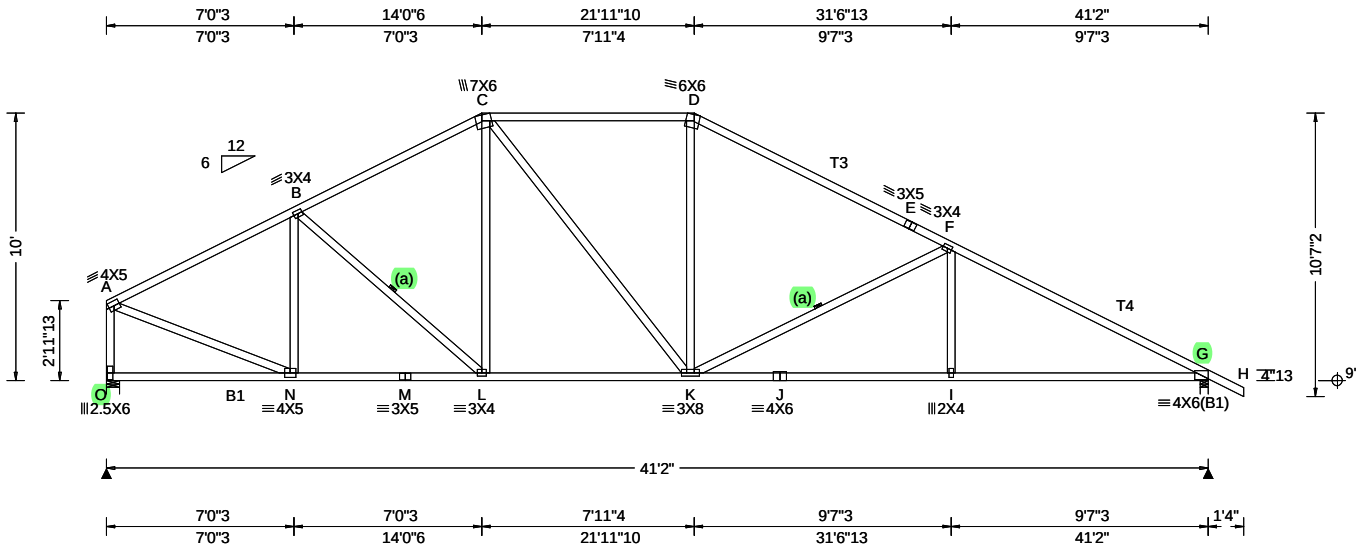
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26241 COMN FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** A11	Cust: R 215 JRef:1Y0W2150008 T40 DrwNo: 176.24.0953.45597 NW / DF 06/24/2024



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.114 K 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.232 K 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.045 G - -
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.093 G - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.810
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.629
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.657
	C&C Dist a: 4.12 ft ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 13.00 ft	Plate Type(s):	VIEW Ver: 23.02.01A.1204.18
	GCpi: 0.18	WAVE	
	Wind Duration: 1.60		

Lumber

Top chord: 2x4 SP #2; T3,T4 2x4 SP M-31;
Bot chord: 2x4 SP M-31; B1 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
O	1687	/-	/-	/950	/37	/270
G	1792	/-	/-	/1106	/92	/-

Wind reactions based on MWFRS
O Brg Wid = 6.0 Min Req = 2.0 (Truss)
G Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings O & G are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

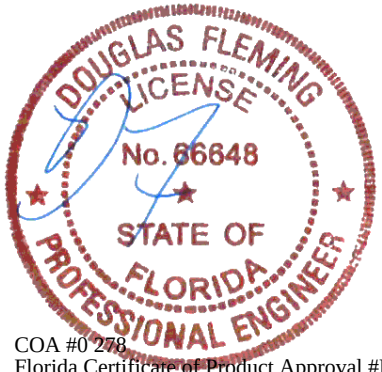
Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	576	- 1910	D - E	753	- 2131
B - C	714	- 1933	E - F	712	- 2172
C - D	762	- 1823	F - G	832	- 3059

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
N - M	1651	- 351	K - J	2623	- 615
M - L	1651	- 351	J - I	2623	- 615
L - K	1638	- 320	I - G	2626	- 613

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
A - O	498	- 1629	K - D	493	- 3
A - N	1723	- 467	K - F	291	- 904
N - B	248	- 475	F - I	398	0



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06/24/2024

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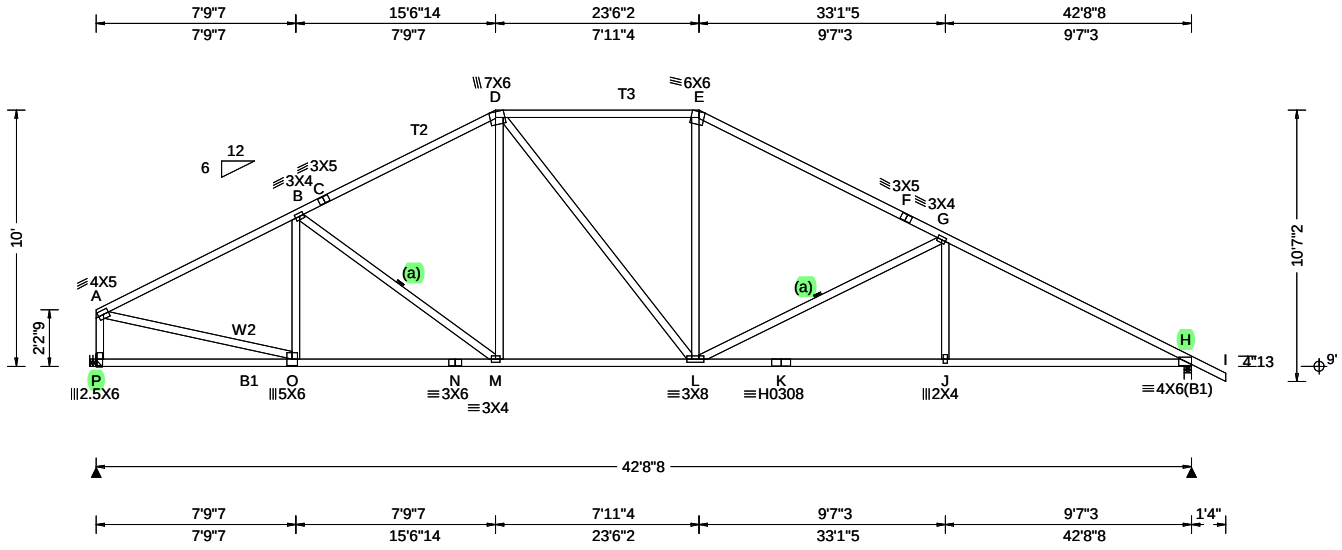
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26236 SPEC FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** A12	Cust: R 215 JRef:1Y0W2150008 T38 DrwNo: 176.24.0953.52110 NW / DF 06/24/2024	



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.128 L 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.263 L 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.051 H - -
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.105 H - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.812
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.780
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.821
	C&C Dist a: 4.27 ft ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: Any	Plate Type(s):	VIEW Ver: 23.02.01A.1204.18
	GCpi: 0.18	WAVE, HS	
	Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
P	1751	/-	/-	/1004	/309	/273
H	1856	/-	/-	/1138	/331	/-

Wind reactions based on MWFRS
P Brg Wid = - Min Req = -
H Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearing H is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	970	- 2318	E - F	1081	- 2270
B - C	1012	- 2148	F - G	1041	- 2310
C - D	1045	- 2120	G - H	1291	- 3195
D - E	1076	- 1948			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
O - N	2004	- 651	L - K	2745	- 980
N - M	2004	- 651	K - J	2745	- 980
M - L	1818	- 478	J - H	2748	- 979

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
A - P	736	- 1686	L - E	541	- 86
A - O	2008	- 767	L - G	506	- 901
D - M	400	- 57	G - J	397	0

Lumber

Top chord: 2x4 SP M-31; T2,T3 2x4 SP #2;
Bot chord: 2x4 SP M-31; B1 2x4 SP #2;
Webs: 2x4 SP #3; W2 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties

(J) Hanger Support Required, by others

Purlins

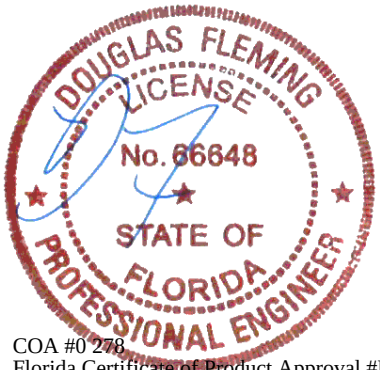
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



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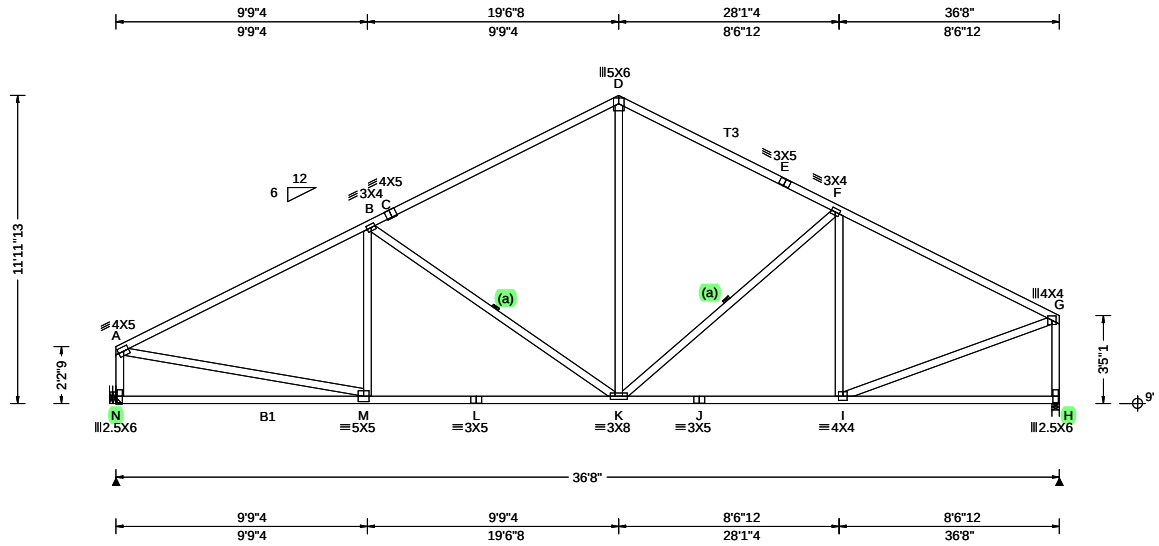
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26541 SPEC FROM: Page 1 of 2	Ply: 1 Qty: 5	**Job Number:** 24-1285 Hedrick **Truss Label:** A13		Cust: R 215 JRef:1Y0W2150008 T59 DrwNo: 176.24.0953.58257 NW / DF 06/24/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 16.10 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.67 ft ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.070	K	999	240
VERT(CL): 0.145	K	999	180
HORZ(LL): 0.022	H	-	-
HORZ(TL): 0.046	H	-	-

Creep Factor: 2.0
Max TC CSI: 0.789
Max BC CSI: 0.811
Max Web CSI: 0.634

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs)

Loc	Gravity R+	/ R-	/ Rh	Non-Gravity / Rw	/ U	/ RL
N	1510	/-	/-	/879	/272	/349
H	1609	/-	/-	/844	/292	/-

Wind reactions based on MWFRS
N Brg Wid = - Min Req = -
H Brg Wid = 3.5 Min Req = 1.9 (Truss)
Bearing H is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	481	- 2019	D - E	489	- 1435
B - C	461	- 1514	E - F	457	- 1491
C - D	505	- 1491	F - G	434	- 1723

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
M - L	1708	- 481	K - J	1423	- 373
L - K	1708	- 481	J - I	1423	- 373

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
A - N	377	- 1430	D - K	738	- 168
A - M	1663	- 310	I - G	1471	- 329
B - K	248	- 594	G - H	378	- 1538

Lumber

Top chord: 2x4 SP M-31; T3 2x4 SP #2;
Bot chord: 2x4 SP #2; B1 2x4 SP M-31;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Special Loads

------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	62 plf at	0.00 to	62 plf at	36.67
BC: From	20 plf at	0.00 to	20 plf at	36.67

TC: 100 lb Conc. Load at 36.47

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

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Glenview, IL 60025

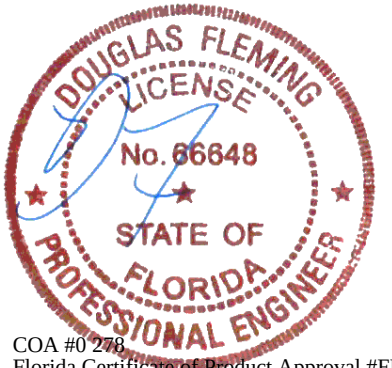
Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

(J) Hanger Support Required, by others
Bearing N (0', 9') HUS26
Supporting Member: (2)2x6 SP 2400f-2.0E
(14) 0.148"x3" nails into supporting member,
(4) 0.148"x3" nails into supported member.



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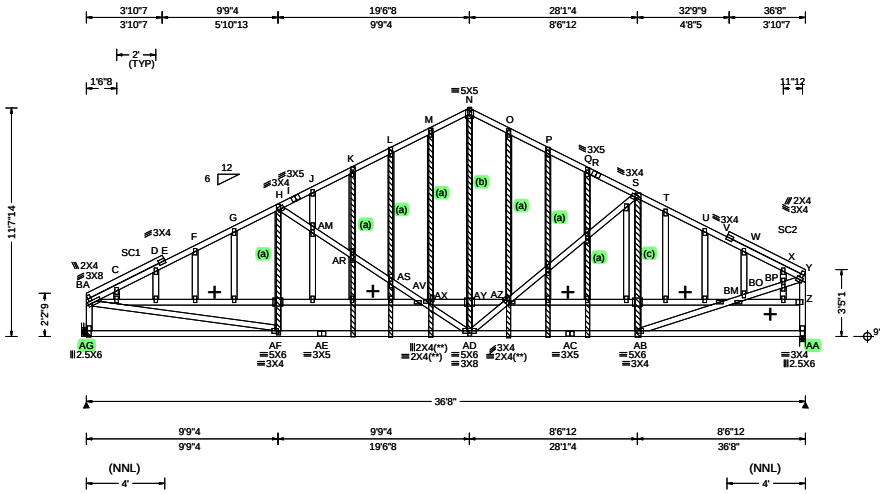
Glenview, IL 60025

SEQN: 26543 GABL	Ply: 2	**Job Number:** 24-1285	Cust: R 215 JRef:1Y0W2150008 T27
FROM:	Qty: 1	Hedrick	DrwNo: 176.24.1034.10847
Page 1 of 2		**Truss Label:** A14	NW / DF 06/24/2024

2 Complete Trusses Required



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.161 F 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.326 F 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.074 F - -
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.150 F - -
NCBCLL: 0.00	Mean Height: 15.77 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.527
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.564
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: No	Max Web CSI: 0.746
	C&C Dist a: 3.67 ft ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: Any	Plate Type(s):	VIEW Ver: 23.02.01A.1204.18
	GCpi: 0.18	WAVE	
	Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity	
Loc	R+ / R-	/ Rh	/ Rw	/ U	/ RL
AG	1776 /-	/-	/-	/391	/87
AA	1939 /-	/-	/-	/420	/-

Wind reactions based on MWFRS
AG Brg Wid = - Min Req = -
AA Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearing AA is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - C	291	-1296	N - O	213	-913
C - D	288	-1288	O - P	209	-910
D - E	295	-1303	P - Q	213	-918
E - F	278	-1268	Q - R	220	-929
F - G	283	-1279	R - S	225	-943
G - H	266	-1241	S - T	227	-1028
H - I	240	-975	T - U	242	-1058
I - J	238	-969	U - V	239	-1049
J - K	221	-933	V - W	250	-1077
K - L	218	-927	W - X	235	-1044
L - M	216	-925	X - Y	238	-1051
M - N	218	-920			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
AF-AE	861	-134	AD-AC	665	-86
AE-AD	861	-134	AC-AB	665	-86

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B -AF	828	-170	AD-AZ	144	-379
H -AM	83	-393	AY-AD	712	-150
AR-AS	79	-385	AY-AZ	450	-173
AS-AV	90	-408	AB-BM	668	-87
AV-AX	450	-172	BM-BO	986	-224
AV-AD	192	-592	BO-BP	1009	-234
AX-AY	450	-172	BP- Y	999	-229
N -AY	714	-153			

Maximum Gable Forces Per Ply (lbs)

Gables	Tens.	Comp.	Gables	Tens.	Comp.
B -AG	211	-827	Z -AA	226	-904
Z - Y	222	-897			

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Nailnote

Nail Schedule:0.131"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 31 plf at 0.00 to 31 plf at 36.67
BC: From 10 plf at 0.00 to 10 plf at 36.67
TC: 66 lb Conc. Load at 1.90, 3.90, 5.90, 7.90 9.90,11.90,13.90,15.90,17.90,19.90,21.90,23.90 25.90,27.90,29.90,31.90,33.90,35.90
TC: 100 lb Conc. Load at 36.50
BC: 51 lb Conc. Load at 1.90, 3.90, 5.90, 7.90 9.90,11.90,13.90,15.90,17.90,19.90,21.90,23.90 25.90,27.90,29.90,31.90,33.90,35.90

Plating Notes

All plates are 2X4 except as noted.

(**) 3 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

+ Member to be laterally braced for out of plane wind loads

Hangers / Ties

(J) Hanger Support Required, by others

Wind

Wind loads and reactions based on MWFRS.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

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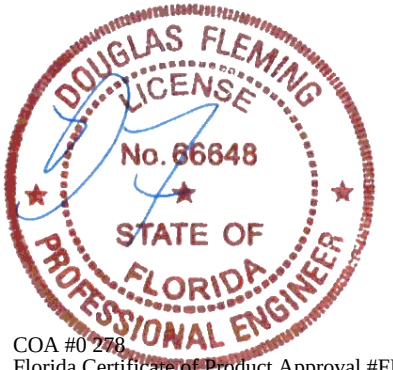
Additional Notes

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

Gable Reinforcement

(a) 2x3 "T" reinforcement. Any species and grade. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.
(b) 2x6 "T" reinforcement. Any species and grade. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.
(c) 2x4 "T" reinforcement. Any species and grade. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.

Laterally brace top chord below filler and bottom chord above filler at 24" o.c.,including a lateral brace at chord ends (If no rigid diaphragm exists at that point)



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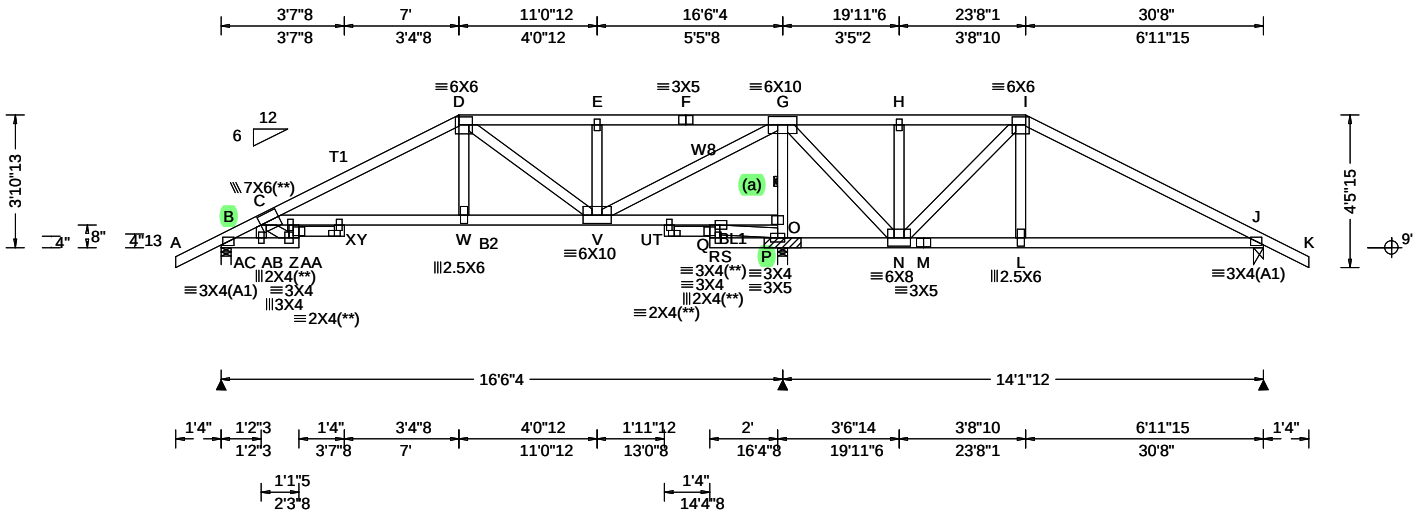
SEQN: 34745 HIPS FROM: Page 1 of 2	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** B01	Cust: R 215 JRef:1Y0W2150008 T3 DrwNo: 176.24.1000.07233 NW / DF 06/24/2024



Loading Criteria (psf)		**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)			**Defl/CSI Criteria**					
TCLL:	20.00	Wind Std: ASCE 7-22	Pg: NA	Ct: NA	CAT: NA	PP Deflection in	loc	L/defl	L/#		
TCDL:	10.00	Speed: 130 mph	Pf: NA		Ce: NA	VERT(LL):	0.199	Y	979	240	
BCLL:	0.00	Enclosure: Closed	Lu: NA	Cs: NA		VERT(CL):	0.400	Y	487	180	
BCDL:	10.00	Risk Category: II	Snow Duration: NA			HORZ(LL):	0.058	V	-	-	
Des Ld:	40.00	EXP: C Kzt: NA				HORZ(TL):	0.118	V	-	-	
NCBCLL:	10.00	Mean Height: 15.00 ft	Building Code:			Creep Factor: 2.0					
Soffit:	2.00	TCDL: 5.0 psf	FBC 8th Ed. 2023 Res.			Max TC CSI:	0.838				
Load Duration:	1.25	BCDL: 5.0 psf	TPI Std: 2014			Max BC CSI:	0.591				
Spacing:	24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Varies by Ld Case			Max Web CSI:	0.839				
		C&C Dist a: 3.07 ft ft	FT/RT:20(0)/10(0)								
		Loc. from endwall: not in 4.50 ft	Plate Type(s):								
		GCpi: 0.18	WAVE			VIEW Ver: 23.02.01A.1204.18					
		Wind Duration: 1.60									

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1116	/-	/-	/-	/259	/-
P	4262	/-	/-	/-	/926	/-
J	909	/-	/-	/-	/195	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
P Brg Wid = 3.5 Min Req = -
J Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B, P, & J are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	314	- 1242	E - F	261	- 811
C - D	504	- 2083	F - G	261	- 811
D - E	261	- 811	I - J	276	- 1251

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B -AC	959	- 261	V - T	294	- 1670
C - Z	1486	- 347	T - Q	243	- 1480
AC-AB	940	- 253	P - N	247	- 1500
Z - X	1519	- 364	N - M	1080	- 219
X - W	1813	- 425	M - L	1080	- 219
W - V	1880	- 436	L - J	1036	- 214

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C -AB	220	- 761	Q - P	221	- 1442
AB-AA	600	- 164	P - O	901	- 3440
Z -AA	494	- 131	G - O	947	- 3611
D - W	1338	- 219	G - N	2122	- 412
D - V	216	- 1358	N - I	254	- 1419
E - V	193	- 395	L - I	1072	- 131
V - G	2783	- 630			

Lumber

Top chord: 2x4 SP #2; T1 2x4 SP M-31;
Bot chord: 2x4 SP #2; B2 2x4 SP M-31;
Webs: 2x4 SP #3; W8 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Special Loads

------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	62 plf at	-1.33 to	62 plf at	7.00	
TC: From	31 plf at	7.00 to	31 plf at	23.67	
TC: From	62 plf at	23.67 to	62 plf at	32.00	
BC: From	4 plf at	-1.33 to	4 plf at	0.00	
BC: From	20 plf at	0.00 to	20 plf at	7.03	
BC: From	10 plf at	7.03 to	10 plf at	23.64	
BC: From	20 plf at	23.64 to	20 plf at	30.67	
BC: From	4 plf at	30.67 to	4 plf at	32.00	

TC: 94 lb Conc. Load at 9.06,11.06,13.06
TC: 190 lb Conc. Load at 15.06,15.60,17.61,19.61 21.61
BC: 897 lb Conc. Load at 7.03,23.64
BC: 181 lb Conc. Load at 9.06,11.06,13.06
BC: 130 lb Conc. Load at 15.06,15.60,17.61,19.61 21.61

Plating Notes

All plates are 2X4 except as noted.

(**) 6 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads and reactions based on MWFRS.

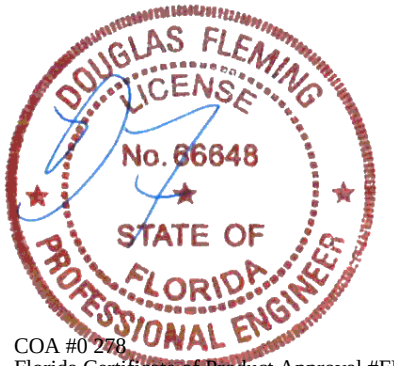
Wind loading based on both gable and hip roof types.

Bearing Block(s)

Brg blocks:0.131"x3", min. nails

brg	x-loc	#blocks	length/blk	#nails/blk	wall plate
2	16.375'	1	13"	8	Rigid Surface

Brg block to be same size and species as chord.
Refer to drawing CNNAILSP1014 for more information.



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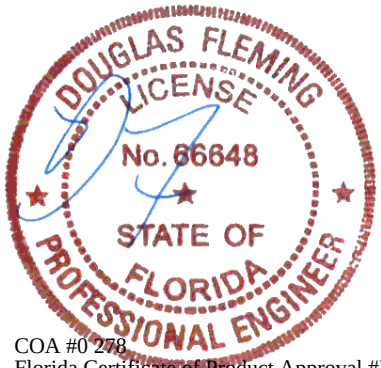
Laterally brace top chord below filler and bottom chord above filler at 24" o.c.,including a lateral brace at chord ends (If no rigid diaphragm exists at that point)



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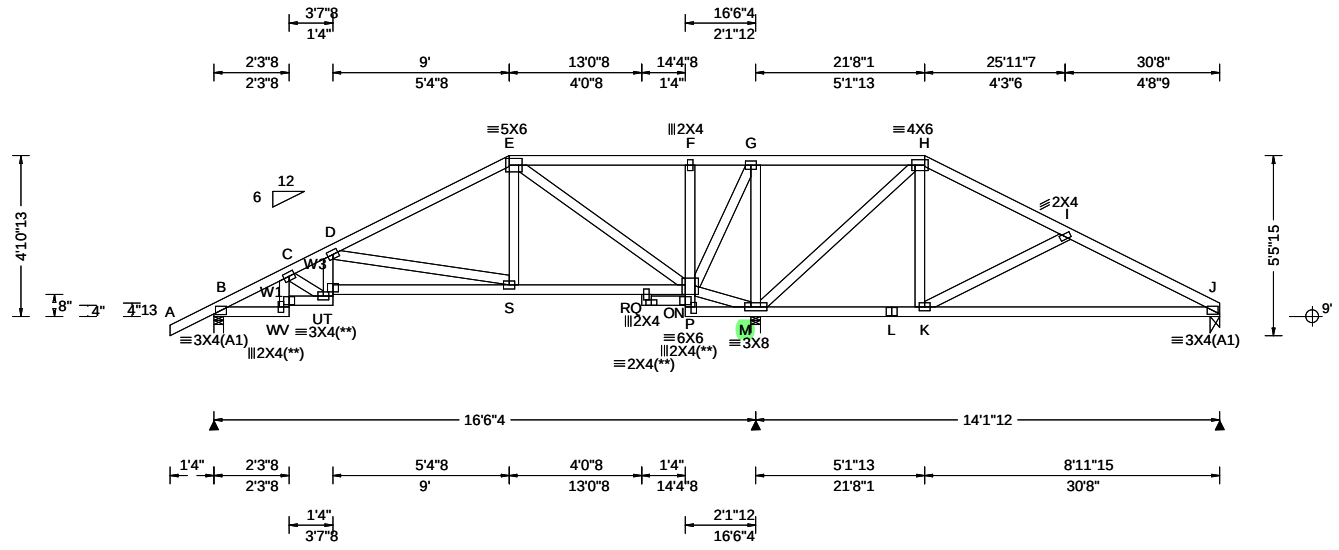
SEQN: 34754 HIPS FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** B02		Cust: R 215 JRef:1Y0W2150008 T26 DrwNo: 176.24.1000.12130 NW / DF 06/24/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.07 ft ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#	
VERT(LL):	0.043	D	999	240
VERT(CL):	0.093	D	999	180
HORZ(LL):	0.023	J	-	-
HORZ(TL):	0.049	P	-	-

Creep Factor: 2.0
Max TC CSI: 0.522
Max BC CSI: 0.555
Max Web CSI: 0.809

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	576	/-	/-	/365	/95	/137
M	1741	/-	/-	/921	/338	/-
J	432	/-	/-	/283	/66	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
M Brg Wid = 3.5 Min Req = 1.7 (Truss)
J Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B, M, & J are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	155	- 688	F - G	526	- 104
C - D	228	- 823	G - H	775	- 212
D - E	136	- 389	I - J	169	- 551
E - F	525	- 104			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - W	562	- 125	T - S	950	- 252
V - U	767	- 172	K - J	455	- 108

Maximum Web Forces Per Ply (lbs)

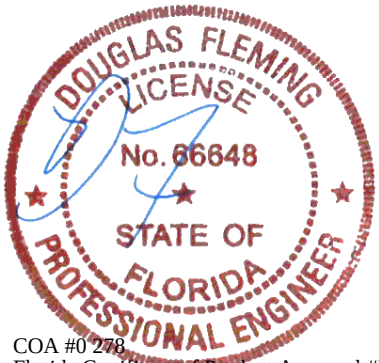
Webs	Tens.	Comp.	Webs	Tens.	Comp.
D - S	242	- 681	G - M	450	- 760
E - N	344	- 912	M - H	305	- 932
N - G	598	- 215	H - K	416	- 21
N - M	401	- 787	K - I	213	- 380

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; W1,W3 2x4 SP M-31;

Plating Notes
All plates are 3X4 except as noted.
(**) 4 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.
Laterally brace top chord below filler and bottom chord above filler at 24" o.c.,including a lateral brace at chord ends (If no rigid diaphragm exists at that point)



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06/24/2024

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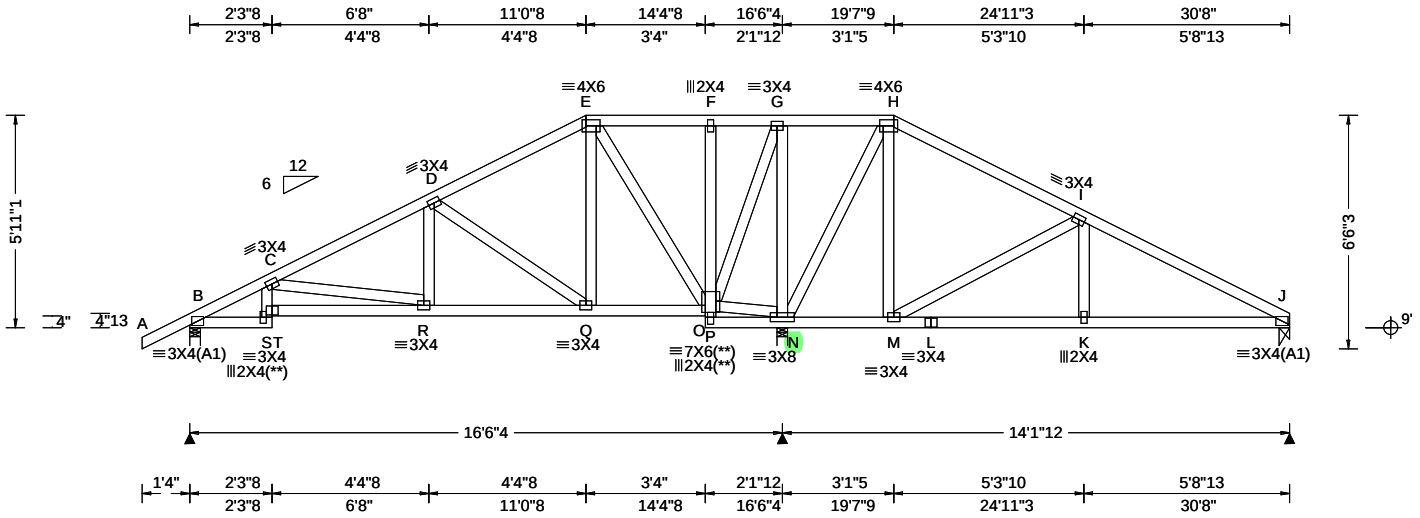
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 34756 HIPS FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** B03		Cust: R 215 JRef:1Y0W2150008 T20 DrwNo: 176.24.1000.14647 NW / DF 06/24/2024	



Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL:	20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL:	10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.028 C 999 240
BCLL:	0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.061 C 999 180
BCDL:	10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.015 J - -
Des Ld:	40.00	EXP: C Kzt: NA		HORZ(TL): 0.031 N - -
NCBCLL:	10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit:	2.00	TCDL: 5.0 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.446
Load Duration:	1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.312
Spacing:	24.0 "	MWFRS Parallel Dist: h/2 to h	Rep Fac: Yes	Max Web CSI: 0.759
		C&C Dist a: 3.07 ft ft	FT/RT:20(0)/10(0)	
		Loc. from endwall: not in 9.00 ft	Plate Type(s):	VIEW Ver: 23.02.01A.1204.18
		GCpi: 0.18	WAVE	
		Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	571	/-	/-	/357	/101	/164
N	1764	/-	/-	/963	/315	/-
J	411	/-	/-	/274	/80	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
N Brg Wid = 3.5 Min Req = 1.7 (Truss)
J Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B, N, & J are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	179	-729	G - H	679	-79
C - D	164	-563	H - I	401	-81
E - F	436	0	I - J	171	-514
F - G	436	0			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - T	613	-150	M - L	399	-105
S - R	832	-204	L - K	399	-105
R - Q	444	-90	K - J	403	-104

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - R	125	-383	O - N	284	-680
D - Q	169	-492	G - N	402	-803
E - Q	387	-59	N - H	235	-804
E - O	239	-744	M - I	211	-579
O - G	661	-244			

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

(**) 3 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

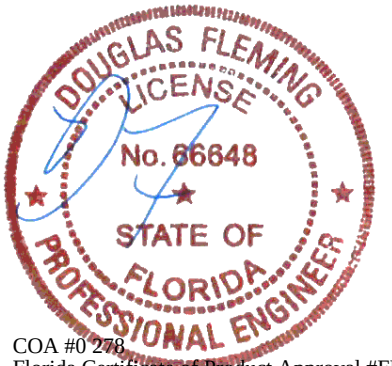
Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



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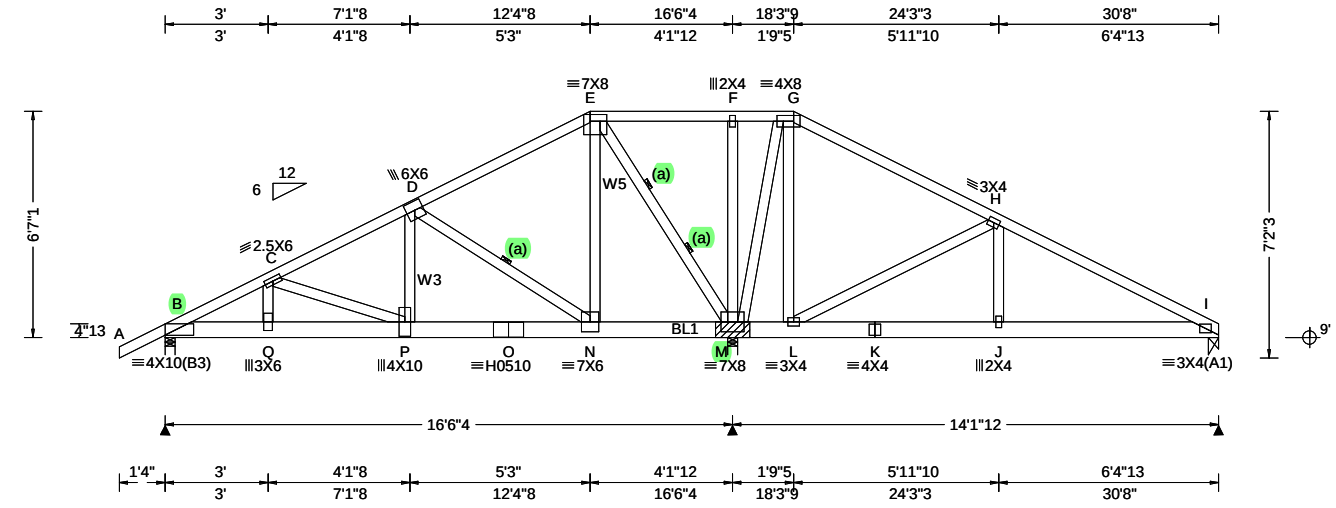
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26505 HIPS FROM:	Ply: 2 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** B04		Cust: R 215 JRef:1Y0W2150008 T41 DrwNo: 176.24.1000.53177 NW / DF 06/24/2024	

2 Complete Trusses Required



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	0.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.07 ft ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE, HS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.094	P	999	240
VERT(CL): 0.188	P	999	180
HORZ(LL): 0.022	C	-	-
HORZ(TL): 0.045	C	-	-

Creep Factor: 2.0
Max TC CSI: 0.583
Max BC CSI: 0.525
Max Web CSI: 0.892

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs)

		Gravity			Non-Gravity	
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	4715	/-	/-	/-	/912	/-
M	9687	/-	/-	/-	/1752	/-
I	438	/-712	/-	/138	/-	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 2.0 (Truss)
M Brg Wid = 3.5 Min Req = -
I Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B, M, & I are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	866	-4567	F - G	1343	-249
C - D	627	-3417	G - H	1386	-266
D - E	159	-872	H - I	1034	-273
E - F	1342	-249			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - Q	4053	-766	M - L	227	-1211
Q - P	4008	-757	L - K	212	-885
P - O	2888	-527	K - J	212	-885
O - N	2888	-527	J - I	214	-882
N - M	742	-127			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
Q - C	863	-164	E - N	3383	-582
C - P	219	-1039	E - M	677	-3751
P - D	2465	-423	M - G	80	-477
D - N	488	-2618			

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3; W3,W5 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Nailnote

Nail Schedule:0.131"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @ 4.25" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 62 plf at -1.33 to 62 plf at 30.67
BC: From 4 plf at -1.33 to 4 plf at 0.00
BC: From 20 plf at 0.00 to 20 plf at 30.67
BC: 1775 lb Conc. Load at 3.12
BC: 1510 lb Conc. Load at 5.06, 7.06, 9.06,11.06 13.06
BC: 1751 lb Conc. Load at 15.06

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads and reactions based on MWFRS.

Wind loading based on both gable and hip roof types.

Bearing Block(s)

Brg blocks:0.131"x3", min. nails

brg	x-loc	#blocks	length/blk	#nails/blk	wall plate
2	16.375'	1	12"	4	Rigid Surface

Brg block to be same size and species as chord.
Refer to drawing CNNAILSP1014 for more information.

Additional Notes

Negative reaction(s) of -712# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.

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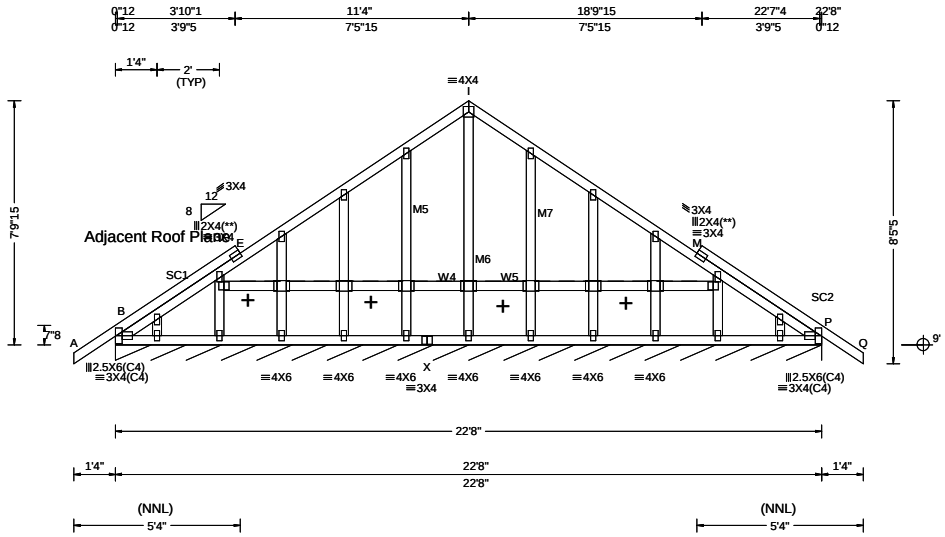
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 32922 / GABL FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** C01	Cust: R 215 JRef:1Y0W2150008 T9 / DrwNo: 173.24.1518.48422 NW / FV 06/21/2024



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.003 E 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.005 E 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.003 M - -
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.004 M - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.238
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.057
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.526
	C&C Dist a: 3.00 ft ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: Any	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 23.02.01A.1204.18
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
P*	92	/-	/-	/49	/15	/11

Wind reactions based on MWFRS
P Brg Wid = 272 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

+ Member to be laterally braced for out of plane wind loads

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; W4,W5,M5,M6,M7 2x4 SP M-31;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

Wind loads based on MWFRS with additional C&C member design.

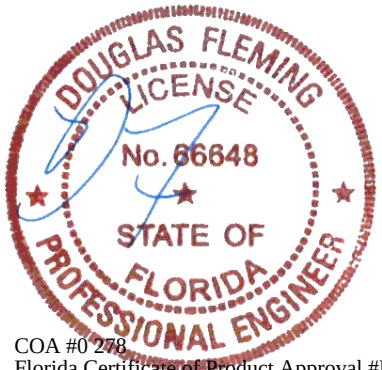
Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/999.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



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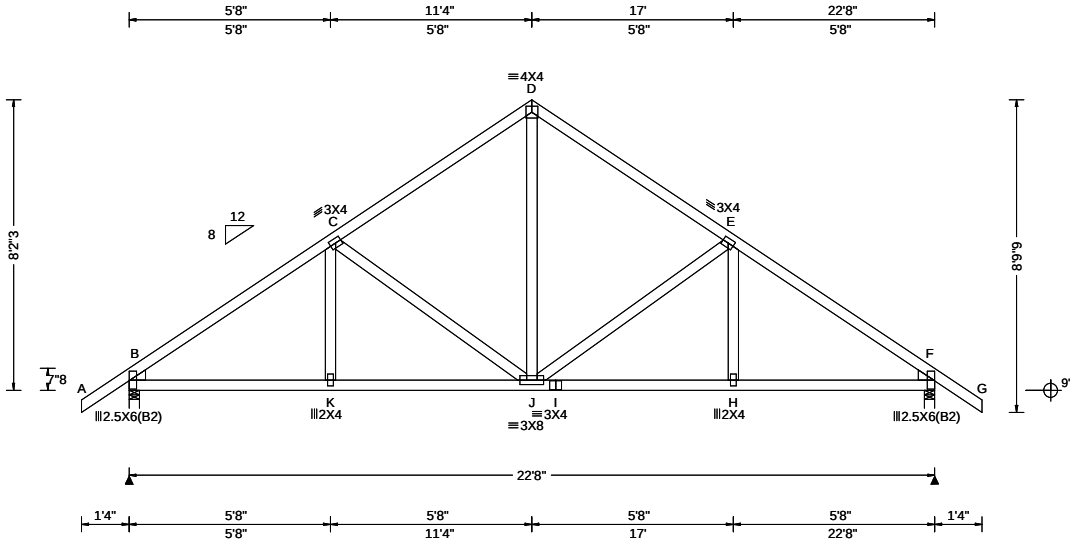
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 34758 COMN FROM:	Ply: 1 Qty: 7	**Job Number:** 24-1285 Hedrick **Truss Label:** C02		Cust: R 215 JRef:1Y0W2150008 T10 DrwNo: 176.24.1000.54683 NW / DF 06/24/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.041	J	999	240
VERT(CL): 0.084	J	999	180
HORZ(LL): 0.022	F	-	-
HORZ(TL): 0.045	F	-	-

Creep Factor: 2.0
Max TC CSI: 0.359
Max BC CSI: 0.401
Max Web CSI: 0.352

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1044	/-	/-	/642	/176	/262
F	1044	/-	/-	/642	/176	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
F Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B & F are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	253	-1319	D - E	266	-949
C - D	265	-949	E - F	252	-1318

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - K	1014	-94	I - H	1013	-91
K - J	1013	-95	H - F	1014	-90
J - I	1013	-91			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - J	167	-379	J - E	166	-379
D - J	583	-137			

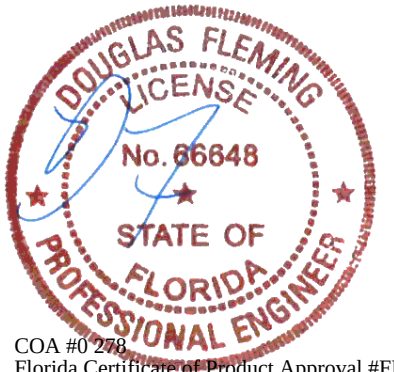
Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Wedge: 2x4 SP #3;Rt Wedge: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



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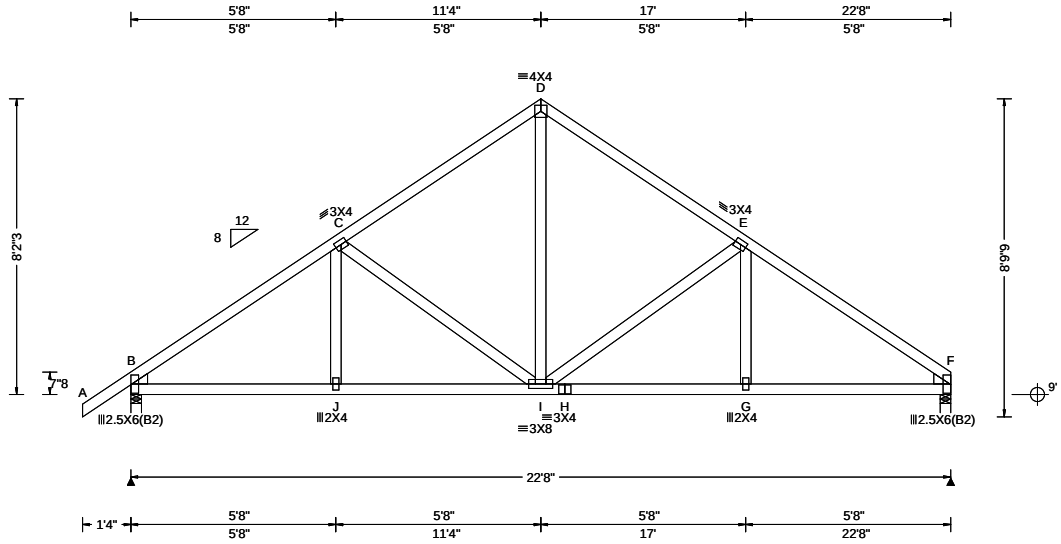
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 101321 SPEC FROM:	Ply: 1 Qty: 3	**Job Number:** 24-1285 Hedrick **Truss Label:** C03	Cust: R 215 JRef:1Y0W2150008 T18 DrwNo: 176.24.1000.56980 NW / DF 06/24/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.046	I	999	240
VERT(CL): 0.095	I	999	180
HORZ(LL): 0.025	F	-	-
HORZ(TL): 0.051	F	-	-

Creep Factor: 2.0
Max TC CSI: 0.459
Max BC CSI: 0.480
Max Web CSI: 0.363

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1047	/-	/-	/642	/177	/243
F	950	/-	/-	/557	/149	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
F Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B & F are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	250	-1332	D - E	274	-954
C - D	267	-953	E - F	264	-1341

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - J	1017	-128	H - G	1027	-133
J - I	1016	-128	G - F	1028	-132
I - H	1027	-133			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - I	165	-378	I - E	172	-392
D - I	589	-145			

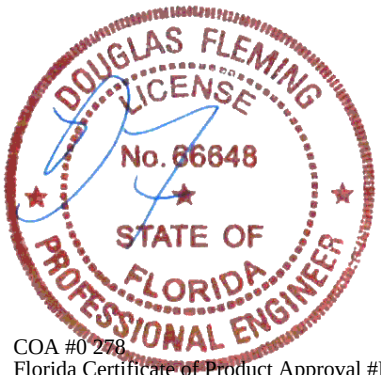
Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Wedge: 2x4 SP #3;Rt Wedge: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



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06/24/2024

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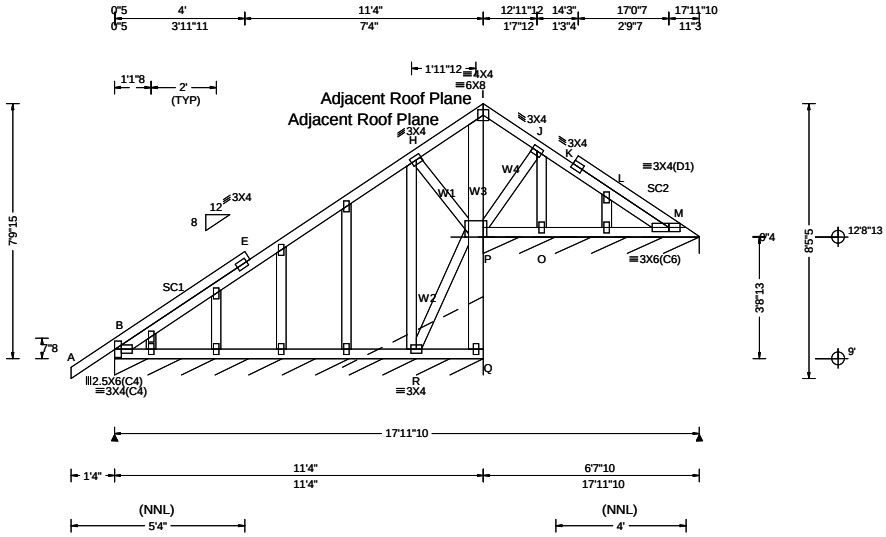
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 33498 / GABL FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** C04	Cust: R 215 JRef:1Y0W2150008 T16 DrwNo: 173.24.1518.48271 NW / FV 06/21/2024



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.002 K 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.004 K 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.003 E - -
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.003 K - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.093
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.020
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.254
	C&C Dist a: 3.00 ft ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 23.02.01A.1204.18
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B*	82	/-	/-	/45	/8	/19
P*	102	/-	/-	/66	/20	/-

Wind reactions based on MWFRS
B Brg Wid = 135 Min Req = -
P Brg Wid = 79.6 Min Req = -
Bearings B & P are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP M-31;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP M-31; W1,W2,W4 2x4 SP #3;
W3 2x6 SP 2400f-2.0E;
Stack Chord: SC1 2x4 SP M-31;
Stack Chord: SC2 2x4 SP M-31;

Plating Notes

All plates are 2X4 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/341.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

Bottom chord to be laterally braced for out of plane wind loads

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

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06/24/2024

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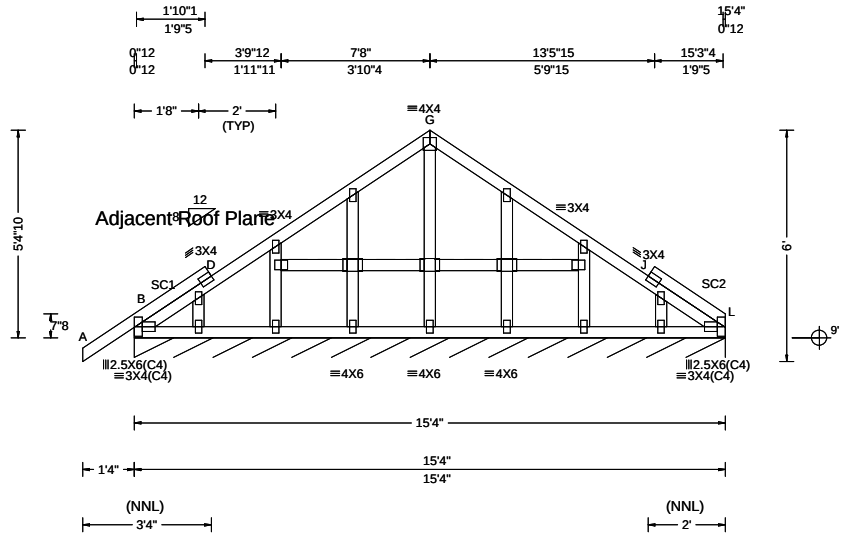
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 32948 / GABL FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** D01	Cust: R 215 JRef:1Y0W2150008 T33 DrwNo: 173.24.1518.48302 NW / FV 06/21/2024



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.001 J 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.002 J 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.002 D - -
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.002 D - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.157
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.054
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.487
	C&C Dist a: 3.00 ft ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: Any	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 23.02.01A.1204.18
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
L*	90	/-	/-	/50	/14	/11

Wind reactions based on MWFRS
L Brg Wid = 184 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.

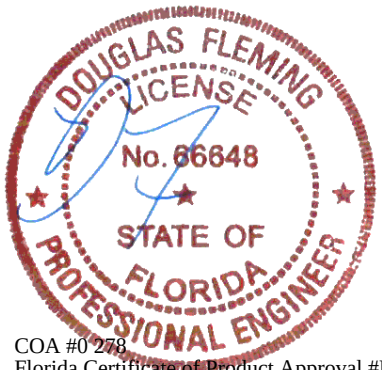
Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/999.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



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06/24/2024

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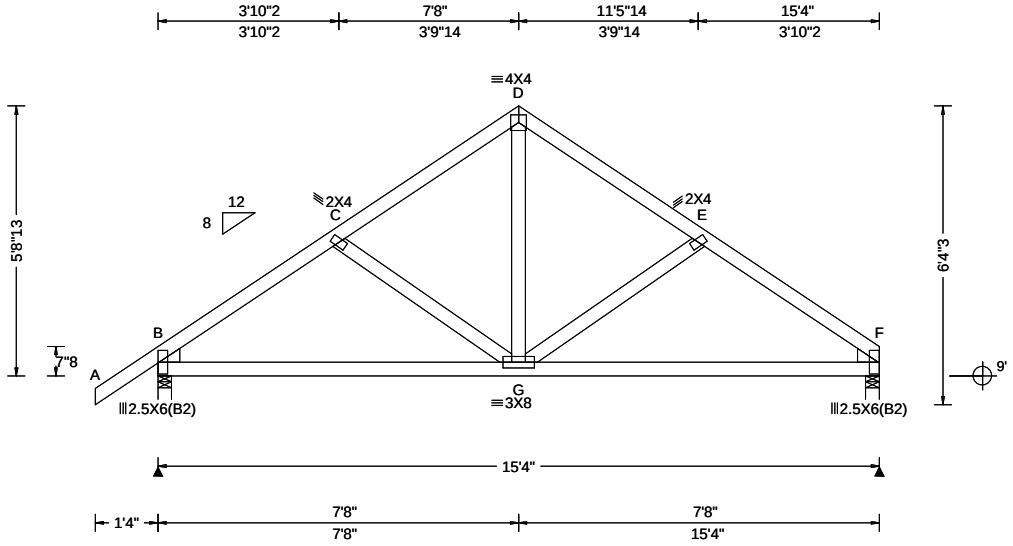
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26120 COMN FROM:	Ply: 1 Qty: 3	**Job Number:** 24-1285 Hedrick **Truss Label:** D02	Cust: R 215 JRef:1Y0W2150008 T44 DrwNo: 176.24.1001.00080 NW / DF 06/24/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.021	G	999	240
VERT(CL): 0.043	G	999	180
HORZ(LL): 0.010	F	-	-
HORZ(TL): 0.022	F	-	-

Creep Factor: 2.0
Max TC CSI: 0.281
Max BC CSI: 0.541
Max Web CSI: 0.162

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	740	/-	/-	/462	/128	/175
F	640	/-	/-	/376	/99	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
F Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B & F are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	382	- 843	D - E	331	- 645
C - D	328	- 643	E - F	392	- 850

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - G	634	- 226	G - F	647	- 243

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.
D - G	426	- 181

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Wedge: 2x4 SP #3;Rt Wedge: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

COA #0 278
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06/24/2024

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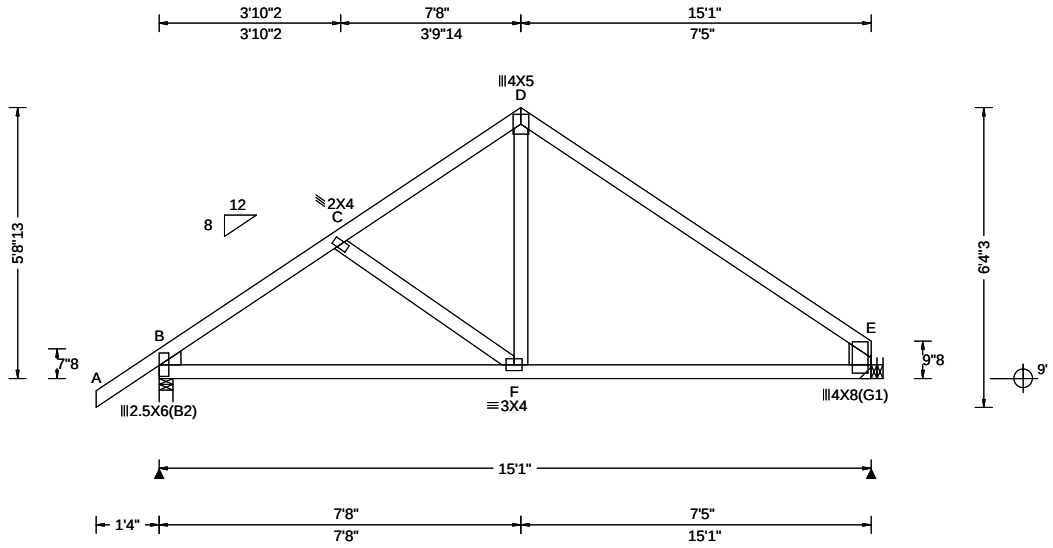
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26430 COMN FROM:	Ply: 1 Qty: 14	**Job Number:** 24-1285 Hedrick **Truss Label:** D03	Cust: R 215 JRef:1Y0W2150008 T47 DrwNo: 176.24.1001.01473 NW / DF 06/24/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.016	C	999	240
VERT(CL): 0.030	C	999	180
HORZ(LL): 0.011	E	-	-
HORZ(TL): 0.019	E	-	-

Creep Factor: 2.0
Max TC CSI: 0.635
Max BC CSI: 0.528
Max Web CSI: 0.139

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	766	/-	/-	/457	/126	/173
E	683	/-	/-	/367	/98	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
E Brg Wid = - Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	221	-889	D - E	192	-794
C - D	210	-712			

Maximum Bot Chord Forces Per Ply (lbs)

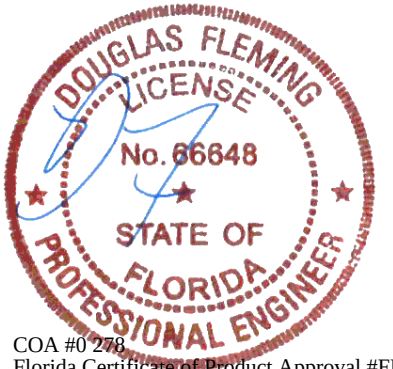
Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - F	664	-124	F - E	552	-34

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Rt Stub Wedge: 2x6 SP 2400f-2.0E;
Lt Wedge: 2x4 SP #3;

Hangers / Ties
(J) Hanger Support Required, by others

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



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06/24/2024

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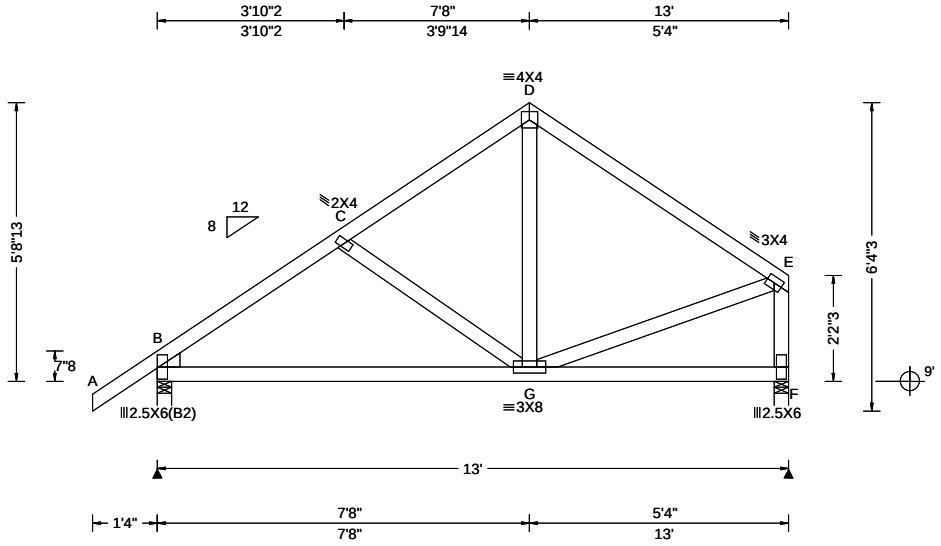
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26432 COMN FROM:	Ply: 1 Qty: 2	**Job Number:** 24-1285 Hedrick **Truss Label:** D04	Cust: R 215 JRef:1Y0W2150008 T7 DrwNo: 176.24.1001.02673 NW / DF 06/24/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.00 ft ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.011	C	999	240
VERT(CL): 0.023	C	999	180
HORZ(LL): 0.003	F	-	-
HORZ(TL): 0.007	F	-	-

Creep Factor: 2.0
Max TC CSI: 0.388
Max BC CSI: 0.471
Max Web CSI: 0.149

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs)

		Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
B	643	/-	/-	/412	/12	/206	
F	542	/-	/-	/295	/1	/-	

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
F Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B & F are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	229	-677	D - E	180	-507
C - D	200	-480			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
B - G	499	-238

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.
E - F	195	-502

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Wedge: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

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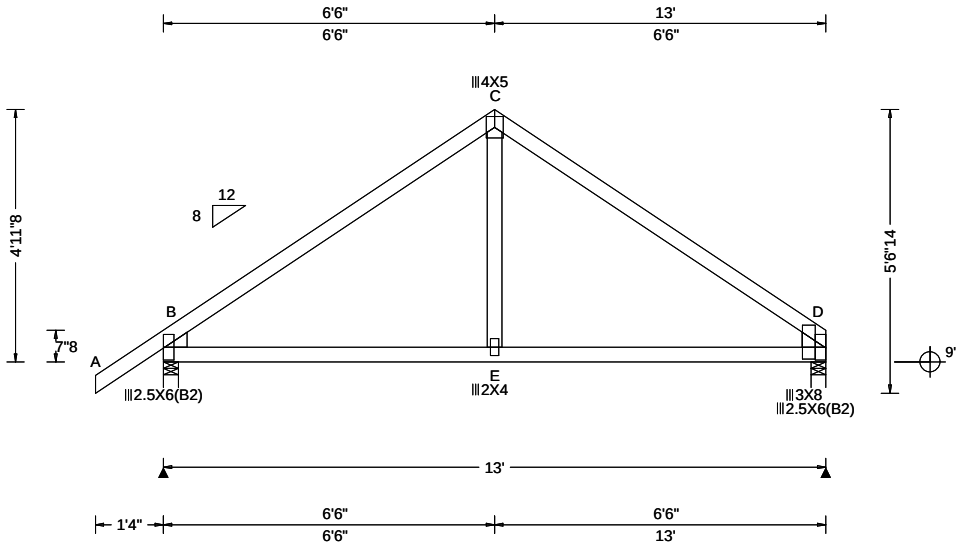
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26428 COMN FROM:	Ply: 1 Qty: 4	**Job Number:** 24-1285 Hedrick **Truss Label:** D05	Cust: R 215 JRef:1Y0W2150008 T57 DrwNo: 176.24.1001.04190 NW / DF 06/24/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.009	D	999	240
VERT(CL): 0.019	D	999	180
HORZ(LL): -0.009	D	-	-
HORZ(TL): 0.015	D	-	-

Creep Factor: 2.0
Max TC CSI: 0.436
Max BC CSI: 0.398
Max Web CSI: 0.109

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	643	/-	/-	/405	/112	/153
D	542	/-	/-	/318	/83	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
D Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B & D are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	205	-639	C - D	213	-637

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - E	447	-58	E - D	447	-58

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Wedge: 2x4 SP #3;Rt Wedge: 2x4 SP #3;

Wind
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

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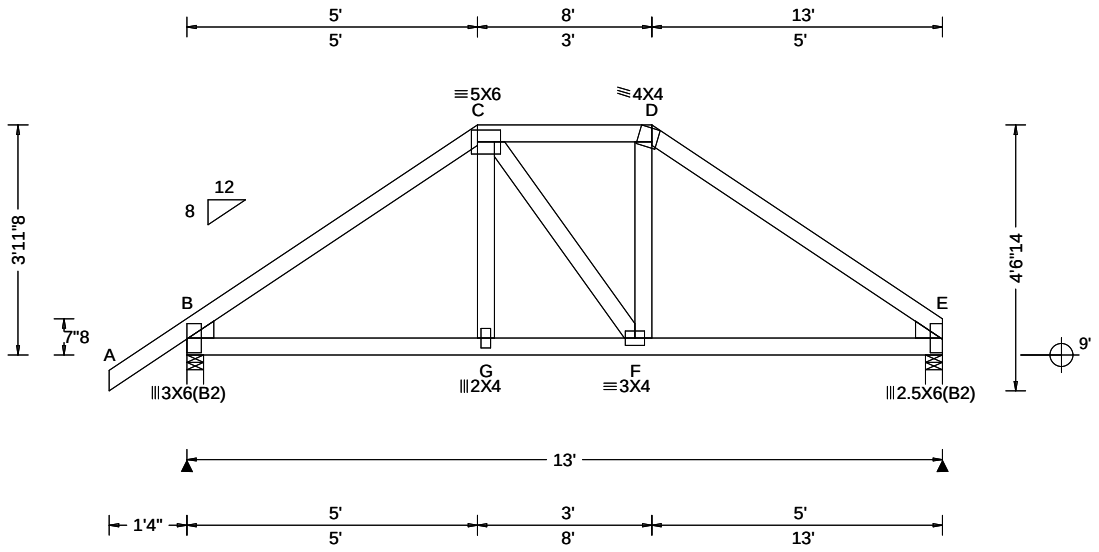
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26466 HIPS FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** D06		Cust: R 215 JRef:1Y0W2150008 T30 DrwNo: 176.24.1001.35000 NW / DF 06/24/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.040	G	999	240
VERT(CL): 0.082	G	999	180
HORZ(LL): 0.025	E	-	-
HORZ(TL): 0.050	E	-	-

Creep Factor: 2.0
Max TC CSI: 0.607
Max BC CSI: 0.802
Max Web CSI: 0.093

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1239	/-	/-	/-	/269	/-
E	1137	/-	/-	/-	/235	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
E Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B & E are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	369	-1640	D - E	372	-1644
C - D	272	-1297			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - G	1269	-275	F - E	1278	-279
G - F	1278	-271			

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Wedge: 2x4 SP #3;Rt Wedge: 2x4 SP #3;

Special Loads

------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	64 plf at	-1.33 to	64 plf at	5.00
TC: From	32 plf at	5.00 to	32 plf at	8.00
TC: From	64 plf at	8.00 to	64 plf at	13.00
BC: From	5 plf at	-1.33 to	5 plf at	0.00
BC: From	20 plf at	0.00 to	20 plf at	5.03
BC: From	10 plf at	5.03 to	10 plf at	7.97
BC: From	20 plf at	7.97 to	20 plf at	13.00

TC: 362 lb Conc. Load at 5.03, 7.97
TC: 138 lb Conc. Load at 6.50
BC: 181 lb Conc. Load at 5.03, 7.97
BC: 94 lb Conc. Load at 6.50

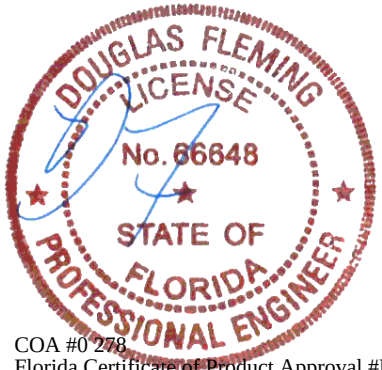
Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads and reactions based on MWFRS.

Wind loading based on both gable and hip roof types.



COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

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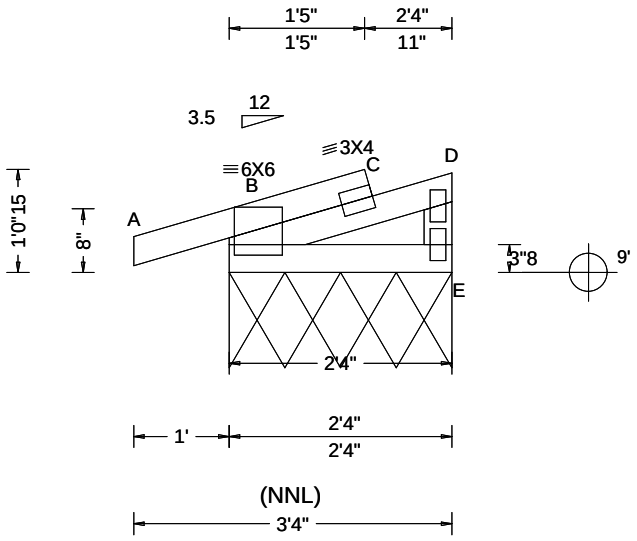
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 33367 / GABL FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** E01	Cust: R 215 JRef:1Y0W2150008 T61 DrwNo: 173.24.1518.48224 NW / FV 06/21/2024



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 C 999 240 VERT(CL): 0.001 C 999 180 HORZ(LL): 0.000 C - - HORZ(TL): 0.001 C - - Creep Factor: 2.0 Max TC CSI: 0.109 Max BC CSI: 0.040 Max Web CSI: 0.020 VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
E*	109	/-	/-	/56	/21	/13

Wind reactions based on MWFRS
E Brg Wid = 28.0 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Wind

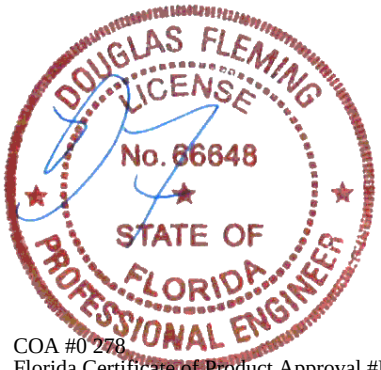
Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.



COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

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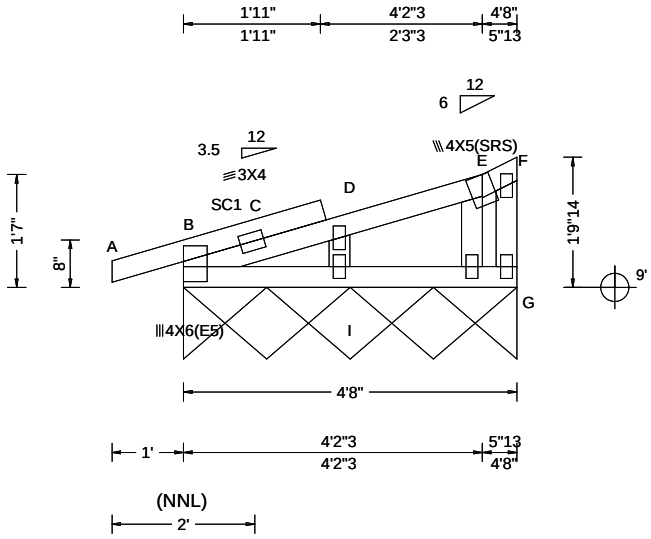
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 33373 / GABL FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** E03	Cust: R 215 JRef:1Y0W2150008 T53 / DrwNo: 173.24.1518.48162 NW / FV 06/21/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	0.001 C	999	240
VERT(CL):	0.002 C	999	180
HORZ(LL):	-0.000 E	-	-
HORZ(TL):	0.001 E	-	-

Creep Factor: 2.0
Max TC CSI: 0.068
Max BC CSI: 0.037
Max Web CSI: 0.043

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
G*	95	/-	/-	/53	/10	/10

Wind reactions based on MWFRS
G Brg Wid = 56.0 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

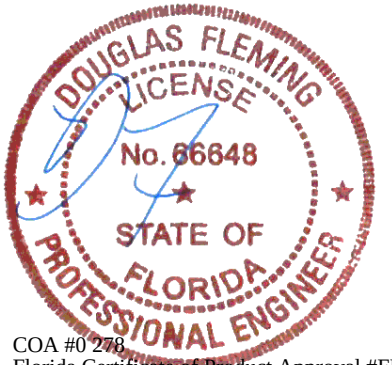
Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/999.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

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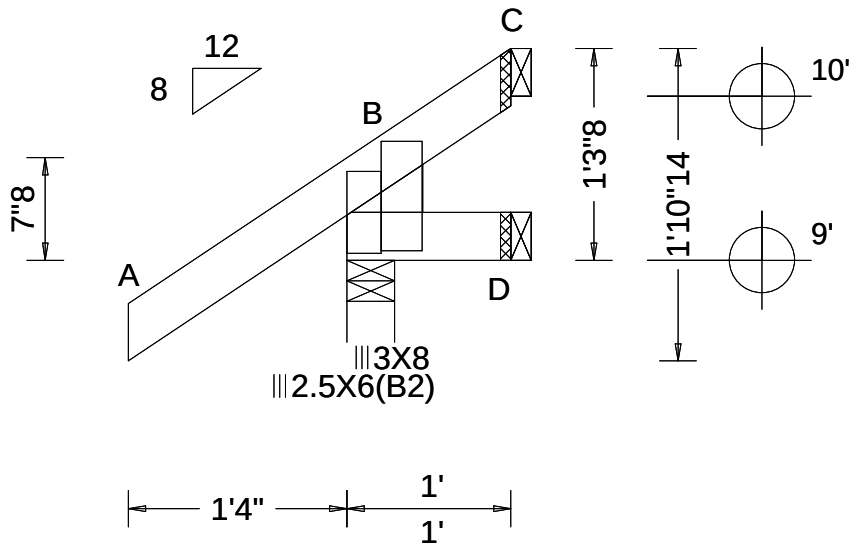
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26098 JACK FROM:	Ply: 1 Qty: 4	**Job Number:** 24-1285 Hedrick **Truss Label:** J01	Cust: R 215 JRef:1Y0W2150008 T32 DrwNo: 176.24.1001.39987 NW / DF 06/24/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	NA		
VERT(CL):	NA		
HORZ(LL):	-0.000 B	-	-
HORZ(TL):	0.001 B	-	-

Creep Factor: 2.0
Max TC CSI: 0.213
Max BC CSI: 0.024
Max Web CSI: 0.000

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	195	/-	/-	/160	/41	/50
D	8	/-11	/-	/14	/12	/-
C	-	/-8	/-	/17	/19	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
D Brg Wid = 1.5 Min Req = -
C Brg Wid = 1.5 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Lt Wedge: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Provide (2)16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2)16d common nails(0.162"x3.5"), toe nailed at Bot chord.

COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

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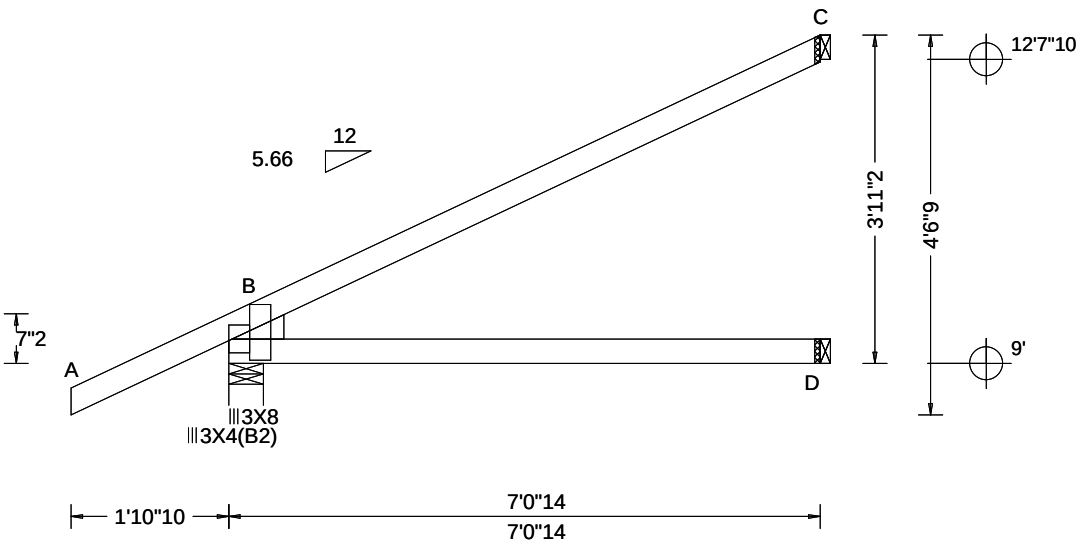
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26102 HIP_ FROM:	Ply: 1 Qty: 2	**Job Number:** 24-1285 Hedrick **Truss Label:** J01HJ	Cust: R 215 JRef:1Y0W2150008 T50 DrwNo: 176.24.1001.43977 NW / DF 06/24/2024



Loading Criteria (psf)
TCLL: 20.00
TCDL: 10.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 40.00
NCBCLL: 0.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft ft
Loc. from endwall: NA
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): NA			
VERT(CL): NA			
HORZ(LL): 0.017	B	-	-
HORZ(TL): 0.035	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.775
Max BC CSI: 0.383
Max Web CSI: 0.000

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	284	/-	/-	/-	/59	/-
D	87	/-	/-	/29	/-	/-
C	224	/-	/-	/-	/77	/-

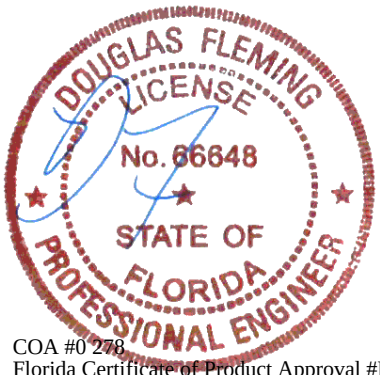
Wind reactions based on MWFRS
B Brg Wid = 4.9 Min Req = 1.5 (Truss)
D Brg Wid = 1.5 Min Req = -
C Brg Wid = 1.5 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Lt Wedge: 2x4 SP #3;

Loading
Hipjack supports 5-0-0 setback jacks with no webs.

Wind
Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

Provide (2)16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2)16d common nails(0.162"x3.5"), toe nailed at Bot chord.



COA #0 278
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06/24/2024

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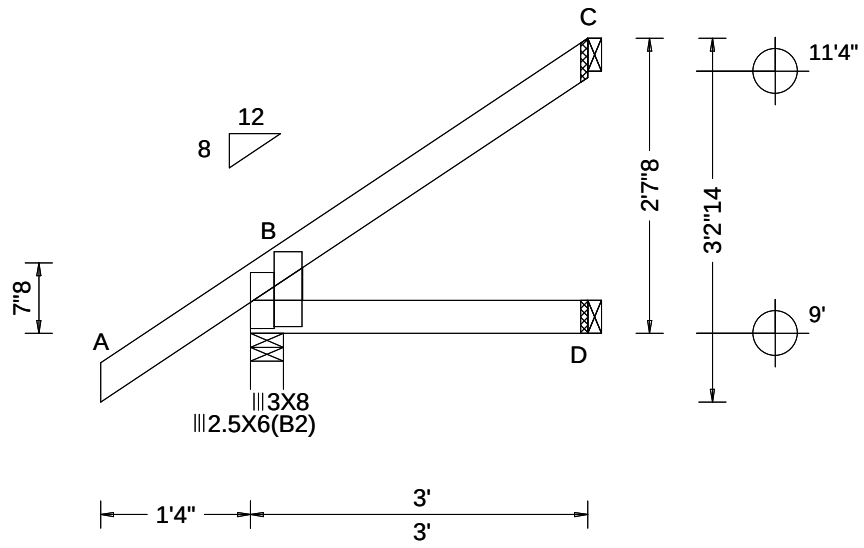
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26096 JACK FROM:	Ply: 1 Qty: 4	**Job Number:** 24-1285 Hedrick **Truss Label:** J02	Cust: R 215 JRef:1Y0W2150008 T28 DrwNo: 176.24.1001.47700 NW / DF 06/24/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	NA		
VERT(CL):	NA		
HORZ(LL):	0.001 B	-	-
HORZ(TL):	0.003 B	-	-

Creep Factor: 2.0
Max TC CSI: 0.213
Max BC CSI: 0.076
Max Web CSI: 0.000

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	238	/-	/-	/176	/21	/97
D	54	/-	/-	/33	/-	/-
C	75	/-	/-	/55	/47	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
D Brg Wid = 1.5 Min Req = -
C Brg Wid = 1.5 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber

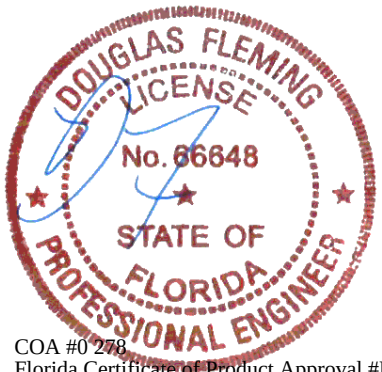
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Lt Wedge: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Provide (2)16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2)16d common nails(0.162"x3.5"), toe nailed at Bot chord.



COA #0 278
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06/24/2024

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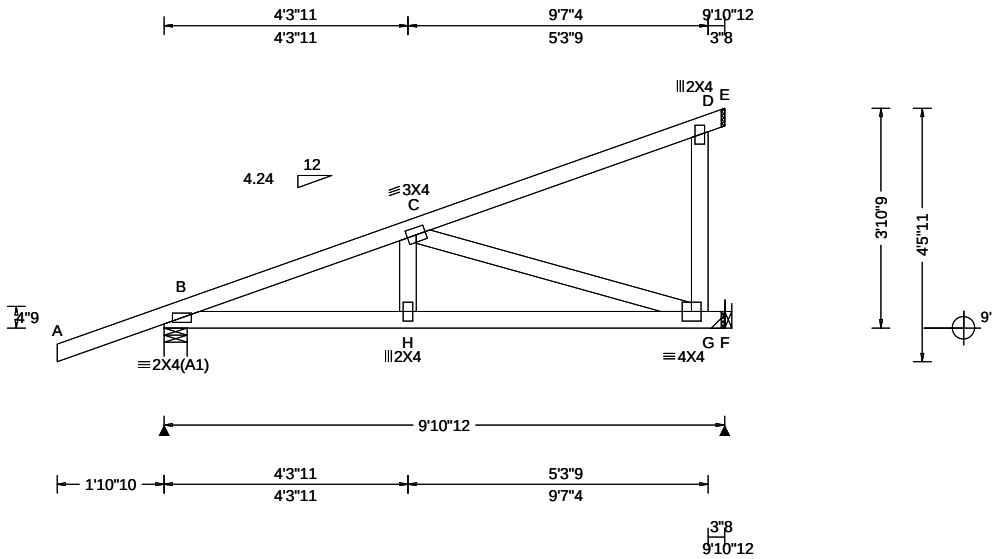
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26360 HIP_ FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** J02HJ	Cust: R 215 JRef:1Y0W2150008 T1 DrwNo: 176.24.1001.51767 NW / DF 06/24/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	0.00
Soffit:	2.00
Load Duration:	1.25
Spacing:	24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft ft
Loc. from endwall: NA
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.030	C	999	240
VERT(CL): 0.060	C	999	180
HORZ(LL): 0.006	G	-	-
HORZ(TL): 0.013	G	-	-

Creep Factor: 2.0
Max TC CSI: 0.711
Max BC CSI: 0.688
Max Web CSI: 0.431

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	439	/-	/-	/-	/90	/-
F	625	/-	/-	/-	/107	/-

Wind reactions based on MWFRS
B Brg Wid = 4.9 Min Req = 1.5 (Truss)
F Brg Wid = - Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
B - C	132	-824

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - H	764	-122	H - G	756	-126

Maximum Web Forces Per Ply (lbs)

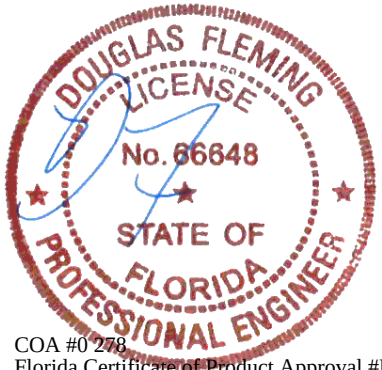
Webs	Tens.	Comp.
C - G	128	-766

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Hangers / Ties
(J) Hanger Support Required, by others

Loading
Hipjack supports 6-11-15 setback jacks with no webs.

Wind
Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.



COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

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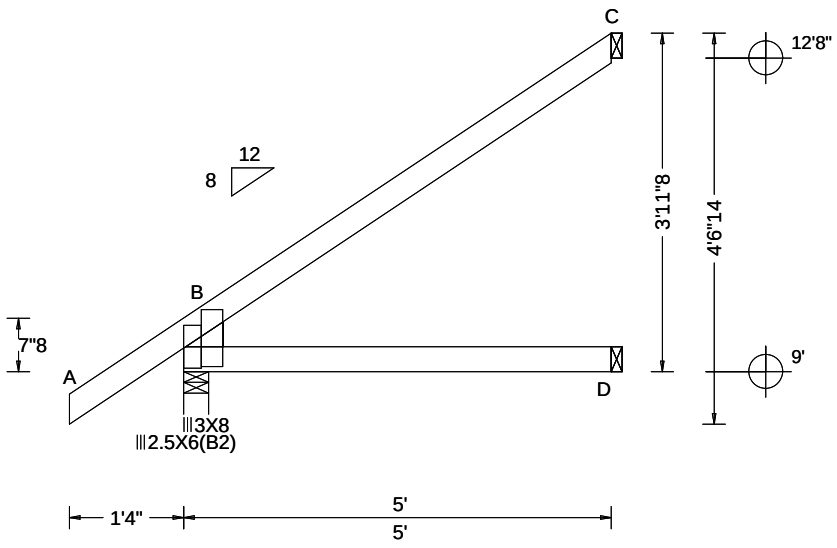
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26100 EJAC FROM:	Ply: 1 Qty: 3	**Job Number:** 24-1285 Hedrick **Truss Label:** J03		Cust: R 215 JRef:1Y0W2150008 T36 DrwNo: 176.24.1001.54670 NW / DF 06/24/2024	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): NA
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.009 B - -
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.017 B - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.446
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.259
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.000
	C&C Dist a: 3.00 ft ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 4.50 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 23.02.01A.1204.18
	Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	314	/-	/-	/222	/17	/145
D	94	/-	/-	/56	/-	/-
C	138	/-	/-	/101	/82	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
D Brg Wid = 1.5 Min Req = -
C Brg Wid = 1.5 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber

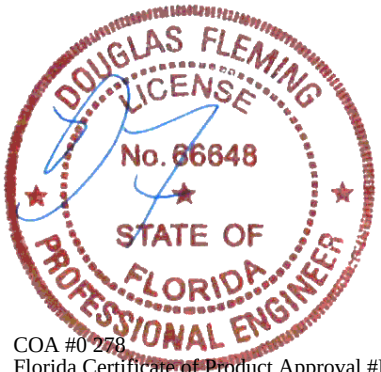
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Lt Wedge: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Provide (2)16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2)16d common nails(0.162"x3.5"), toe nailed at Bot chord.



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06/24/2024

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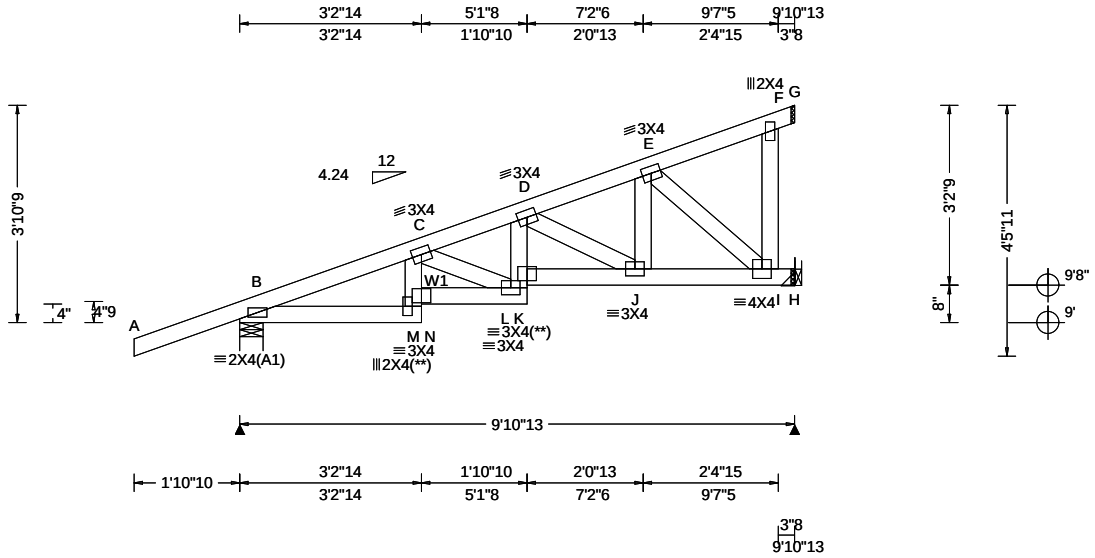
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26356 HIP_ FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** J03HJ		Cust: R 215 JRef:1Y0W2150008 T25 DrwNo: 176.24.1001.57687 NW / DF 06/24/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	0.00
Soffit:	2.00
Load Duration:	1.25
Spacing:	24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft ft
Loc. from endwall: NA
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.047	L	999	240
VERT(CL): 0.095	L	999	180
HORZ(LL): 0.022	I	-	-
HORZ(TL): 0.044	I	-	-

Creep Factor: 2.0
Max TC CSI: 0.308
Max BC CSI: 0.483
Max Web CSI: 0.925

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	439	/-	/-	/-	/90	/-
H	625	/-	/-	/-	/107	/-

Wind reactions based on MWFRS
B Brg Wid = 4.9 Min Req = 1.5 (Truss)
H Brg Wid = - Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	109	- 671	D - E	113	- 657
C - D	135	- 787			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - N	604	- 95	K - J	898	- 155
M - L	822	- 132	J - I	553	- 97

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
J - E	380	- 3	E - I	126	- 720

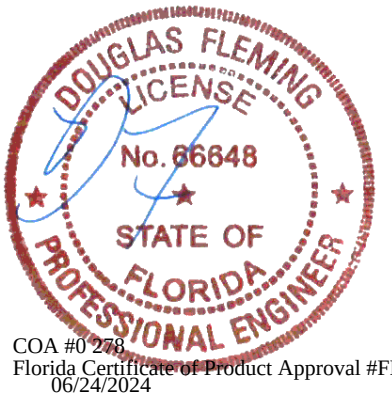
Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; W1 2x4 SP #2;

Plating Notes
(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Hangers / Ties
(J) Hanger Support Required, by others

Loading
Hipjack supports 7-0-0 setback jacks with no webs.

Wind
Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.



COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

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****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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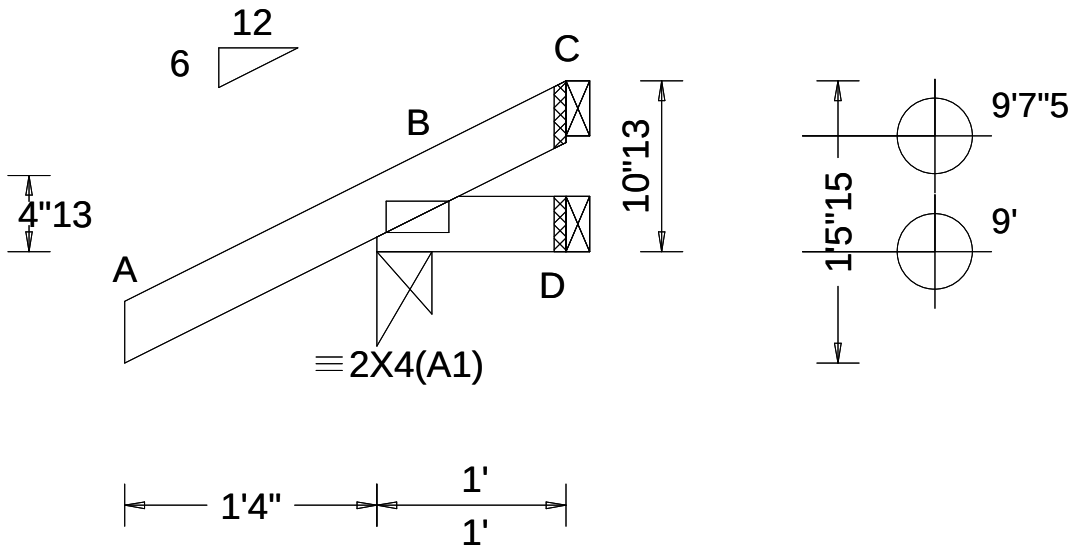
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26149 JACK FROM:	Ply: 1 Qty: 4	**Job Number:** 24-1285 Hedrick **Truss Label:** J04	Cust: R 215 JRef:1Y0W2150008 T24 DrwNo: 176.24.1002.00983 NW / DF 06/24/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	NA		
VERT(CL):	NA		
HORZ(LL):	-0.000 B	-	-
HORZ(TL):	0.001 B	-	-

Creep Factor: 2.0
Max TC CSI: 0.186
Max BC CSI: 0.028
Max Web CSI: 0.000

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	222	/-	/-	/174	/56	/36
D	8	/-10	/-	/12	/10	/-
C	-	/-41	/-	/28	/40	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
D Brg Wid = 1.5 Min Req = -
C Brg Wid = 1.5 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber

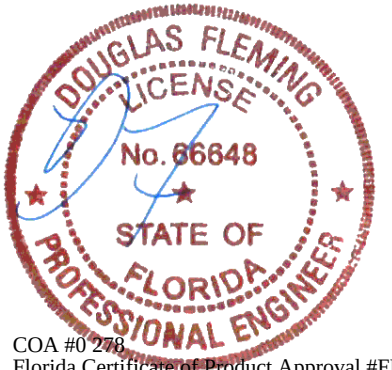
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Provide (2)16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2)16d common nails(0.162"x3.5"), toe nailed at Bot chord.



COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

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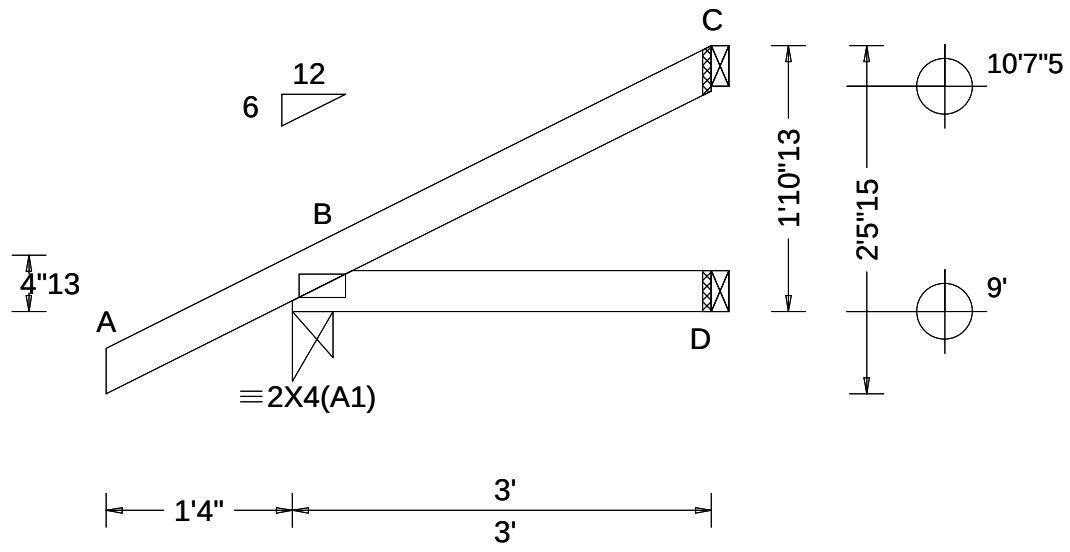
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26151 JACK FROM:	Ply: 1 Qty: 2	**Job Number:** 24-1285 Hedrick **Truss Label:** J05	Cust: R 215 JRef:1Y0W2150008 T23 DrwNo: 176.24.1002.04980 NW / DF 06/24/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	NA		
VERT(CL):	NA		
HORZ(LL):	0.001 B	-	-
HORZ(TL):	0.001 B	-	-

Creep Factor: 2.0
Max TC CSI: 0.195
Max BC CSI: 0.069
Max Web CSI: 0.000

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	244	/-	/-	/175	/36	/71
D	51	/-	/-	/27	/-	/-
C	66	/-	/-	/39	/37	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
D Brg Wid = 1.5 Min Req = -
C Brg Wid = 1.5 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber

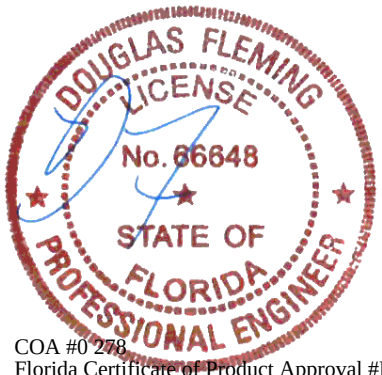
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Provide (2)16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2)16d common nails(0.162"x3.5"), toe nailed at Bot chord.



COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

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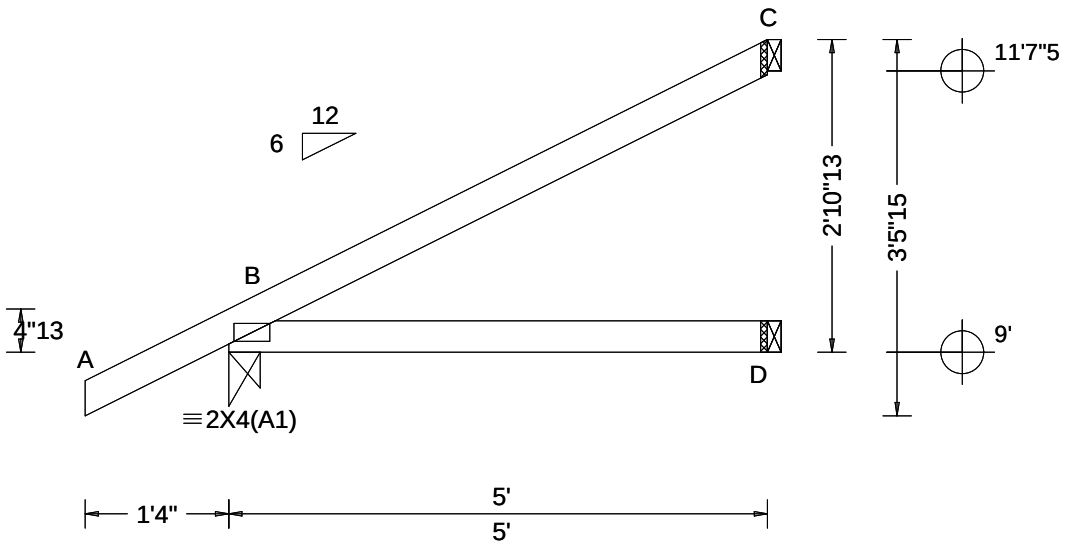
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26153 JACK FROM:	Ply: 1 Qty: 2	**Job Number:** 24-1285 Hedrick **Truss Label:** J06	Cust: R 215 JRef:1Y0W2150008 T21 DrwNo: 176.24.1002.07323 NW / DF 06/24/2024



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): NA
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.004 B - -
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.008 B - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.336
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.241
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.000
	C&C Dist a: 3.00 ft ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 4.50 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 23.02.01A.1204.18
	Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	316	/-	/-	/218	/39	/107
D	91	/-	/-	/49	/-	/-
C	130	/-	/-	/81	/67	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
D Brg Wid = 1.5 Min Req = -
C Brg Wid = 1.5 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber

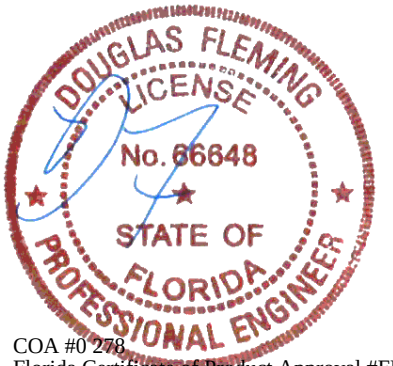
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Provide (2)16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2)16d common nails(0.162"x3.5"), toe nailed at Bot chord.



COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

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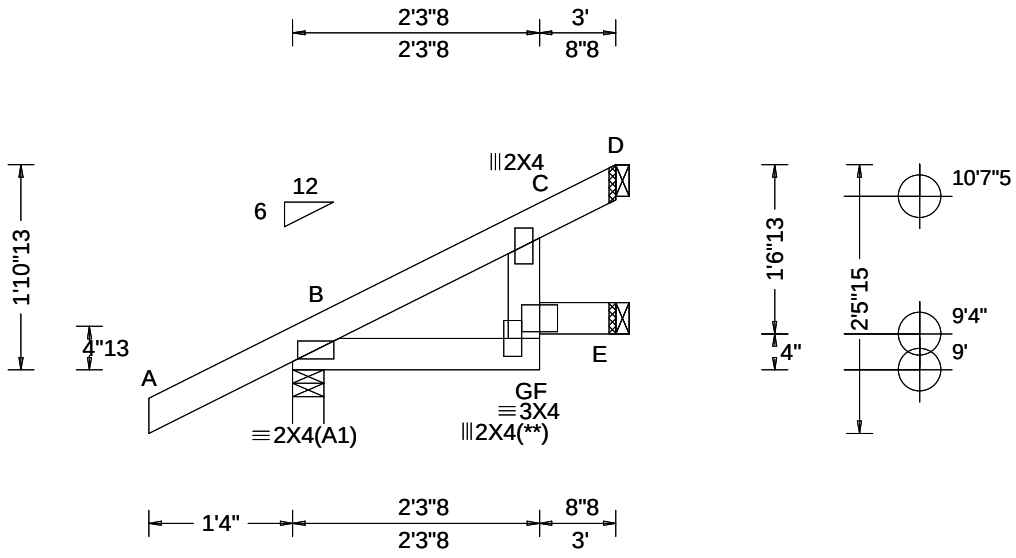
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26165 JACK FROM:	Ply: 1 Qty: 2	**Job Number:** 24-1285 Hedrick **Truss Label:** J07		Cust: R 215 JRef:1Y0W2150008 T17 DrwNo: 176.24.1002.09547 NW / DF 06/24/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.003	F	999	240
VERT(CL): 0.007	F	999	180
HORZ(LL): 0.002	C	-	-
HORZ(TL): 0.003	C	-	-

Creep Factor: 2.0
Max TC CSI: 0.190
Max BC CSI: 0.038
Max Web CSI: 0.032

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	244	/-	/-	/175	/36	/71
E	21	/-	/-	/13	/-	/-
D	76	/-	/-	/51	/28	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
E Brg Wid = 1.5 Min Req = -
D Brg Wid = 1.5 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

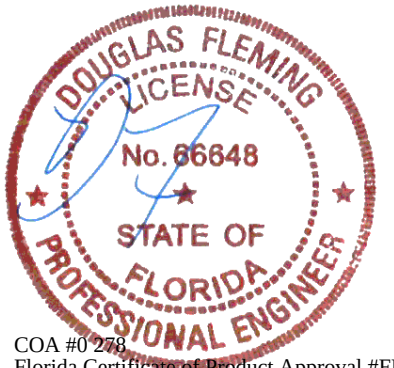
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Provide (2)16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2)16d common nails(0.162"x3.5"), toe nailed at Bot chord.



COA #0 278
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06/24/2024

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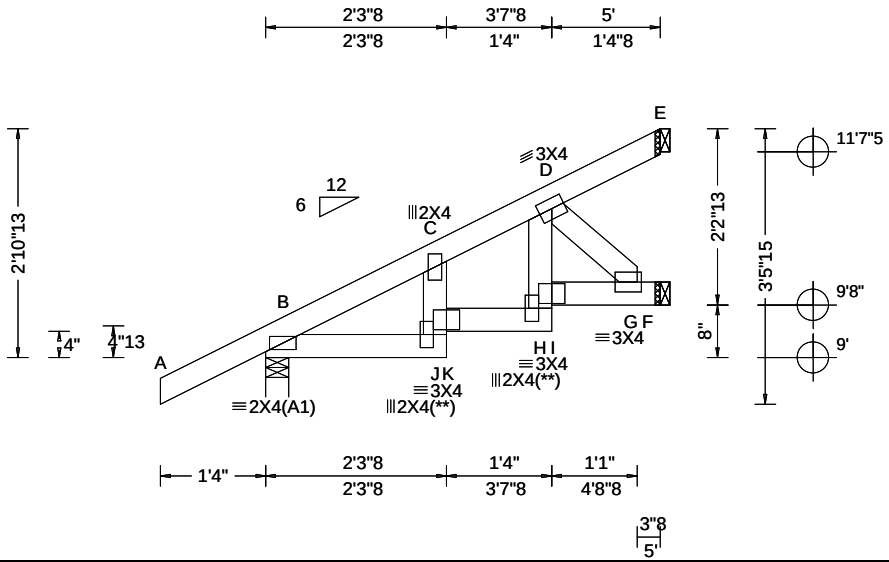
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26161 JACK FROM:	Ply: 1 Qty: 2	**Job Number:** 24-1285 Hedrick **Truss Label:** J08		Cust: R 215 JRef:1Y0W2150008 T45 DrwNo: 176.24.1002.12543 NW / DF 06/24/2024	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.012 J 999 240 VERT(CL): 0.023 J 999 180 HORZ(LL): 0.006 C - - HORZ(TL): 0.012 C - - Creep Factor: 2.0 Max TC CSI: 0.160 Max BC CSI: 0.122 Max Web CSI: 0.172 VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	316	/-	/-	/218	/39	/107
F	162	/-	/-	/117	/40	/-
E	34	/-5	/-	/14	/14	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
F Brg Wid = 1.5 Min Req = -
E Brg Wid = 1.5 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

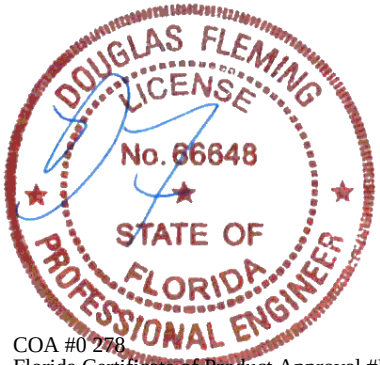
(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Provide (2)16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2)16d common nails(0.162"x3.5"), toe nailed at Bot chord.



COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

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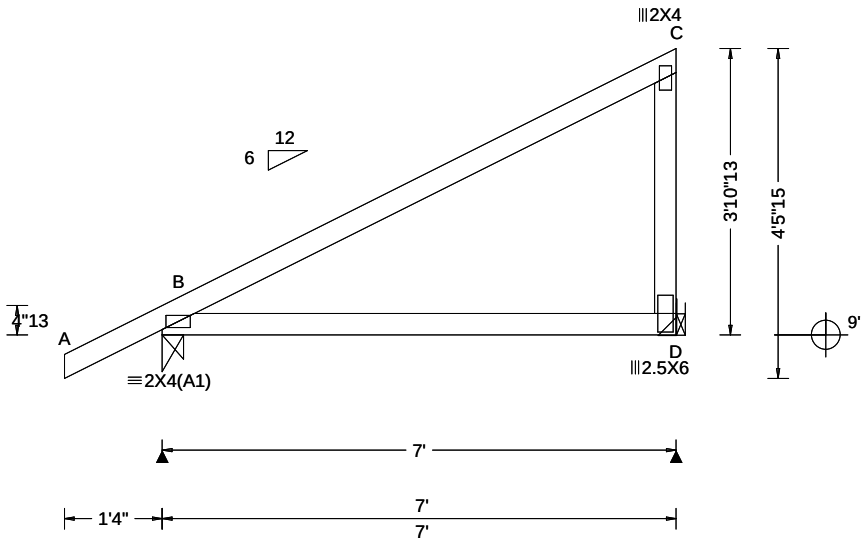
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26334 / EJAC FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** J09	Cust: R 215 JRef:1Y0W2150008 T6 DrwNo: 173.24.1518.48427 NW / FV 06/21/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	NA		
VERT(CL):	NA		
HORZ(LL):	0.012 B	-	-
HORZ(TL):	0.023 B	-	-

Creep Factor: 2.0
Max TC CSI: 0.659
Max BC CSI: 0.472
Max Web CSI: 0.266

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs)

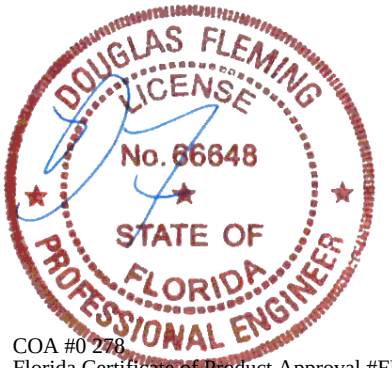
	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	394	/-	/-	/267	/43	/142
D	272	/-	/-	/193	/78	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
D Brg Wid = - Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Hangers / Ties
(J) Hanger Support Required, by others

Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

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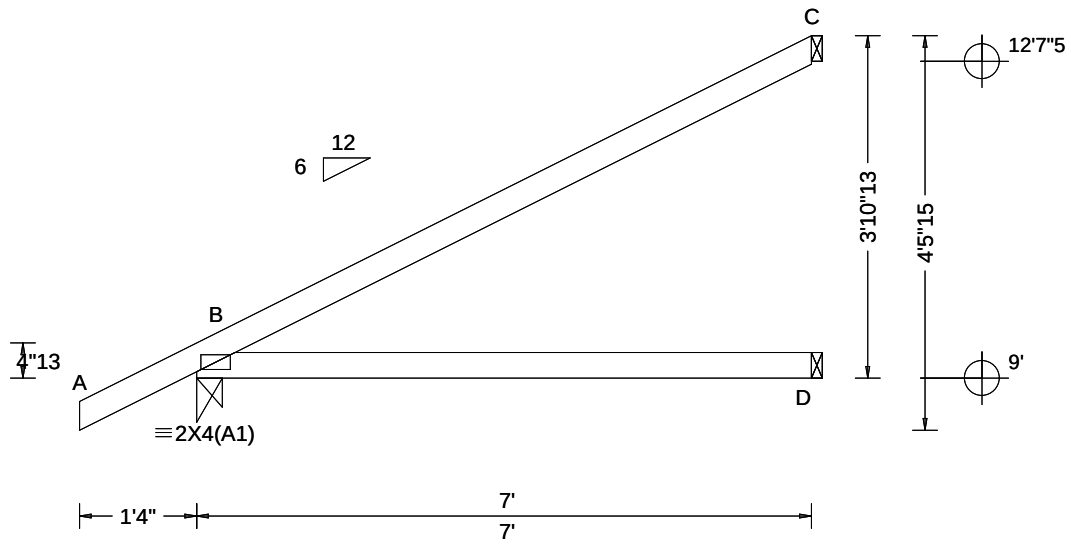
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26336 / EJAC FROM:	Ply: 1 Qty: 5	**Job Number:** 24-1285 Hedrick **Truss Label:** J10	Cust: R 215 JRef:1Y0W2150008 T83 DrwNo: 173.24.1518.48208 NW / FV 06/21/2024



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): NA
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.013 B - -
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.026 B - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.735
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.520
Spacing: 24.0 "	MWFRS Parallel Dist: h/2 to h	Rep Fac: Yes	Max Web CSI: 0.000
	C&C Dist a: 3.00 ft ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 4.50 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 23.02.01A.1204.18
	Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	394	/-	/-	/267	/43	/142
D	130	/-	/-	/73	/-	/-
C	190	/-	/-	/120	/96	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
D Brg Wid = 1.5 Min Req = -
C Brg Wid = 1.5 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber

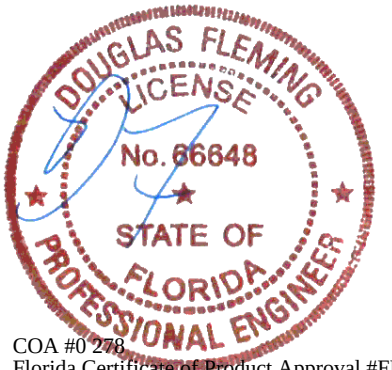
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Provide (2)16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2)16d common nails(0.162"x3.5"), toe nailed at Bot chord.



COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

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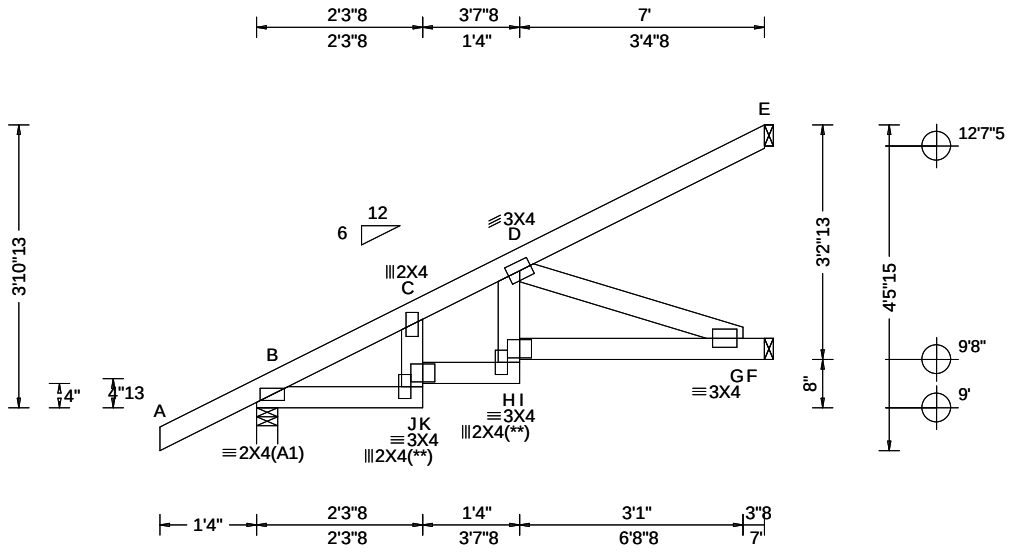
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26338 EJAC FROM:	Ply: 1 Qty: 3	**Job Number:** 24-1285 Hedrick **Truss Label:** J11		Cust: R 215 JRef:1Y0W2150008 T22 DrwNo: 176.24.1002.16027 NW / DF 06/24/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.017	J	999	240
VERT(CL): 0.032	J	999	180
HORZ(LL): 0.010	G	-	-
HORZ(TL): 0.020	G	-	-

Creep Factor: 2.0
Max TC CSI: 0.161
Max BC CSI: 0.246
Max Web CSI: 0.397

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	394	/-	/-	/267	/43	/142
F	181	/-	/-	/137	/31	/-
E	94	/-	/-	/57	/47	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
F Brg Wid = 1.5 Min Req = -
E Brg Wid = 1.5 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
H - G	412	- 311

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.
D - G	333	- 440

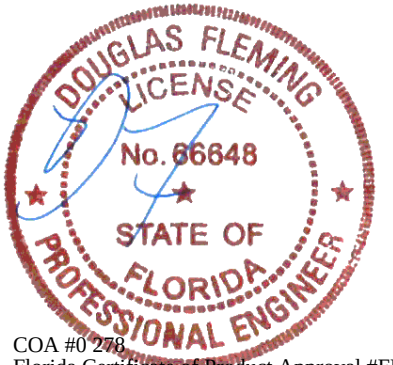
Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes
(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Provide (2)16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2)16d common nails(0.162"x3.5"), toe nailed at Bot chord.



COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

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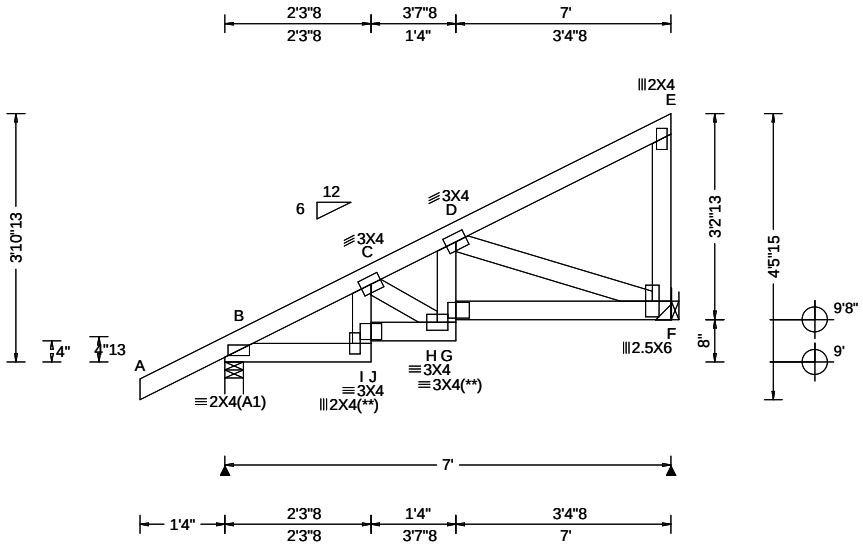
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26348 EJAC FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** J12	Cust: R 215 JRef:1Y0W2150008 T56 DrwNo: 176.24.1002.18280 NW / DF 06/24/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.014	H	999	240
VERT(CL): 0.029	H	999	180
HORZ(LL): 0.010	F	-	-
HORZ(TL): 0.019	F	-	-

Creep Factor: 2.0
Max TC CSI: 0.153
Max BC CSI: 0.202
Max Web CSI: 0.417

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	394	/-	/-	/267	/43	/142
F	272	/-	/-	/193	/78	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
F Brg Wid = - Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
G - F	398	-309

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.
D - F	324	-417

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes
(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Hangers / Ties
(J) Hanger Support Required, by others

Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

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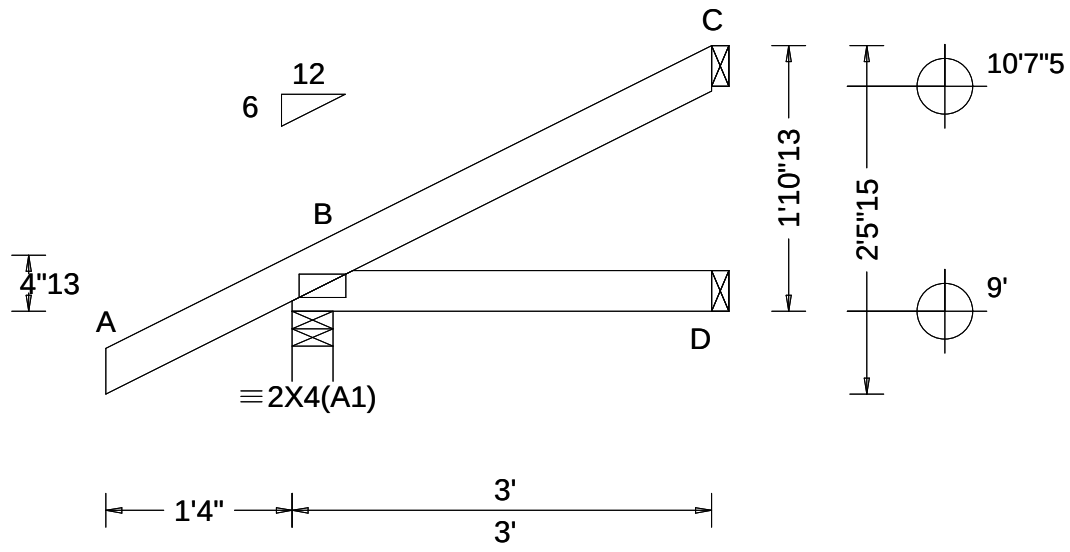
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26495 / EJAC FROM:	Ply: 1 Qty: 18	**Job Number:** 24-1285 Hedrick **Truss Label:** J13	Cust: R 215 JRef:1Y0W2150008 T4 / DrwNo: 173.24.1518.48444 NW / FV 06/21/2024



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): NA
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.001 B - -
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.001 B - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.195
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.068
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.000
	C&C Dist a: 3.00 ft ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: Any	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 23.02.01A.1204.18
	Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	244	/-	/-	/175	/37	/71
D	51	/-	/-	/27	/-	/-
C	66	/-	/-	/39	/37	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
D Brg Wid = 1.5 Min Req = -
C Brg Wid = 1.5 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber

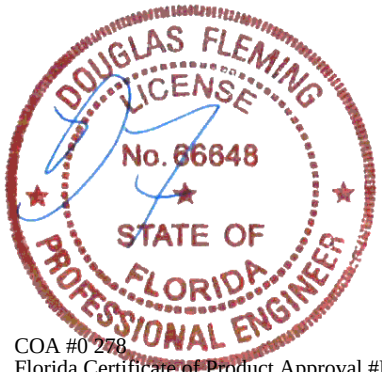
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Provide (2)16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2)16d common nails(0.162"x3.5"), toe nailed at Bot chord.



COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

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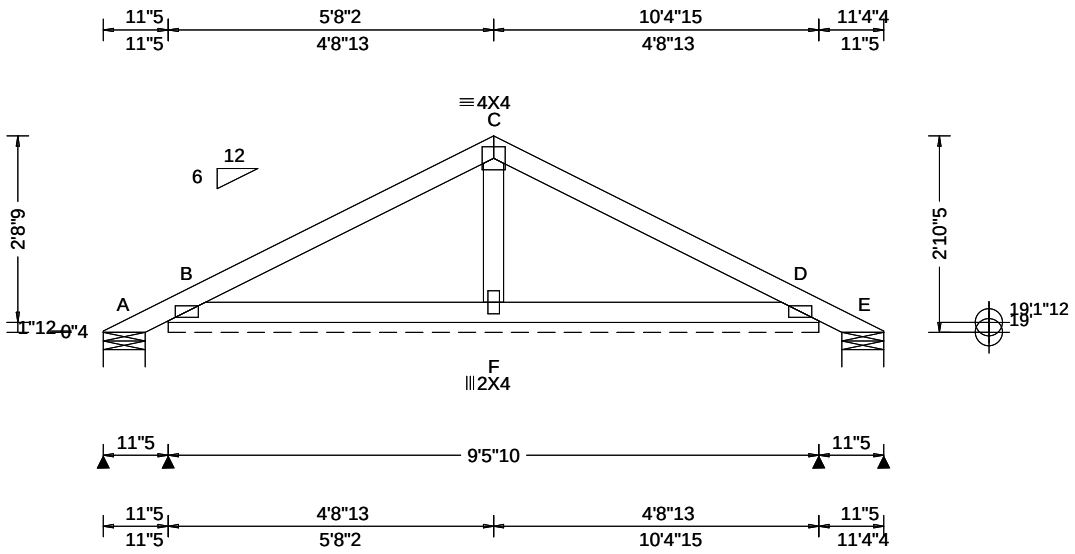
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26268 SPEC FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** PB01	Cust: R 215 JRef:1Y0W2150008 T12 DrwNo: 176.24.0951.01433 NW / DF 06/24/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	0.00
Soffit:	2.00
Load Duration:	1.25
Spacing:	24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.30 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.06 ft ft
Loc. from endwall: not in 6.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.003	D	999	240
VERT(CL): 0.005	D	999	180
HORZ(LL): -0.002	D	-	-
HORZ(TL): 0.003	D	-	-

Creep Factor: 2.0
Max TC CSI: 0.237
Max BC CSI: 0.104
Max Web CSI: 0.030

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	-	/-91	/-	/66	/107	/74
B*	95	/-	/-	/63	/33	/-
E	-	/-91	/-	/48	/75	/-

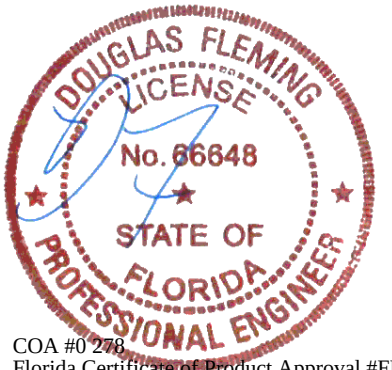
Wind reactions based on MWFRS
A Brg Wid = 7.3 Min Req = 1.5 (Truss)
B Brg Wid = 113 Min Req = -
E Brg Wid = 7.3 Min Req = 1.5 (Truss)
Bearings A, B, & E are a rigid surface.
Members not listed have forces less than 375#

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes
All plates are 2X4(A1) except as noted.

Wind
Wind loads based on MWFRS.
Wind loading based on both gable and hip roof types.

Additional Notes
Refer to DWG PB160220723 for piggyback details.



COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

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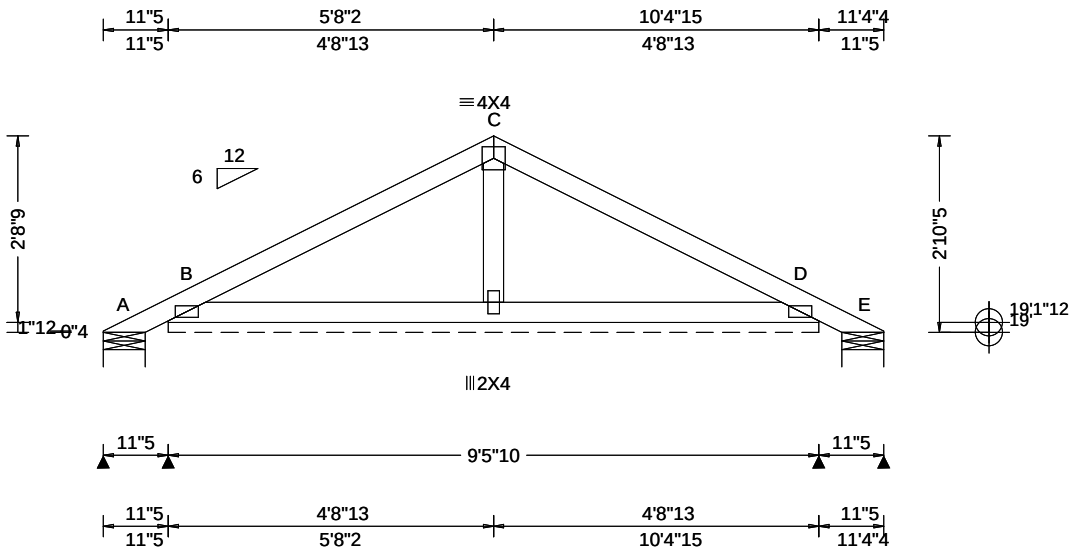
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 34760 COMN FROM:	Ply: 1 Qty: 12	**Job Number:** 24-1285 Hedrick **Truss Label:** PB02	Cust: R 215 JRef:1Y0W2150008 T31 DrwNo: 176.24.0951.02617 NW / DF 06/24/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.30 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.00 ft ft
Loc. from endwall: not in 13.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.003	D	999	240
VERT(CL): 0.007	D	999	180
HORZ(LL): -0.002	D	-	-
HORZ(TL): 0.004	D	-	-

Creep Factor: 2.0
Max TC CSI: 0.243
Max BC CSI: 0.178
Max Web CSI: 0.027

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	-	/-101	/-	/64	/110	/76
B*	116	/-	/-	/65	/6	/-
E	-	/-101	/-	/30	/77	/-

Wind reactions based on MWFRS
A Brg Wid = 7.3 Min Req = 1.5 (Truss)
B Brg Wid = 113 Min Req = -
E Brg Wid = 7.3 Min Req = 1.5 (Truss)
Bearings A, B, & E are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

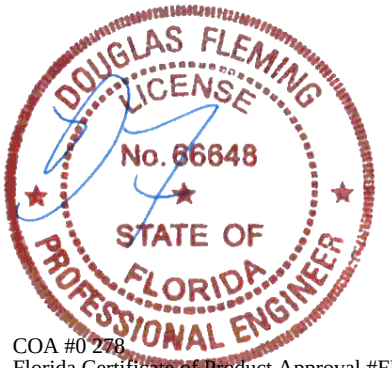
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Refer to DWG PB160220723 for piggyback details.



COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

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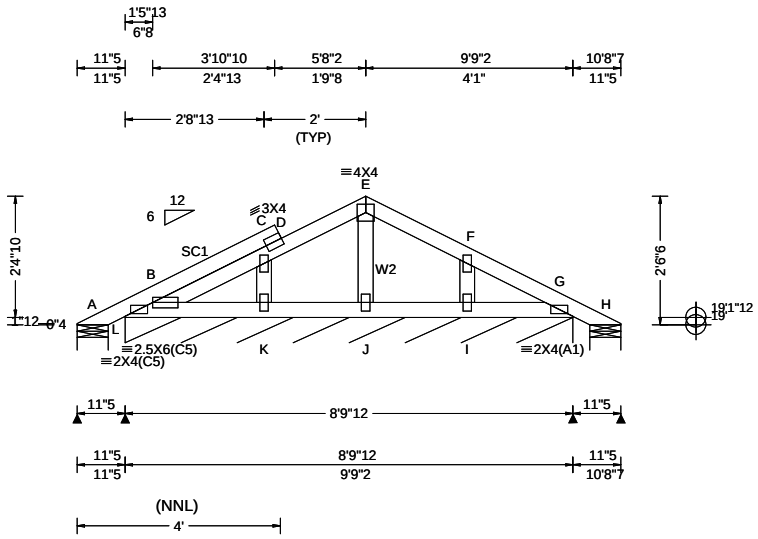
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 34707 GABL FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** PB03	Cust: R 215 JRef:1Y0W2150008 T5 DrwNo: 176.24.0951.04763 NW / DF 06/24/2024



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 20.28 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 D 999 240 VERT(CL): 0.003 D 999 180 HORZ(LL): 0.000 D - - HORZ(TL): 0.001 D - - Creep Factor: 2.0 Max TC CSI: 0.043 Max BC CSI: 0.013 Max Web CSI: 0.053 VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	12	/-30	/-	/34	/35	/67
L*	92	/-	/-	/49	/7	/-
H	14	/-	/-	/7	/5	/-

Wind reactions based on MWFRS
A Brg Wid = 7.3 Min Req = 1.5 (Truss)
L Brg Wid = 105 Min Req = -
H Brg Wid = 7.3 Min Req = 1.5 (Truss)
Bearings A, L, & H are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP M-31;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP #3; W2 2x4 SP M-31;
Stack Chord: SC1 2x4 SP M-31;

Plating Notes

All plates are 2X4 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

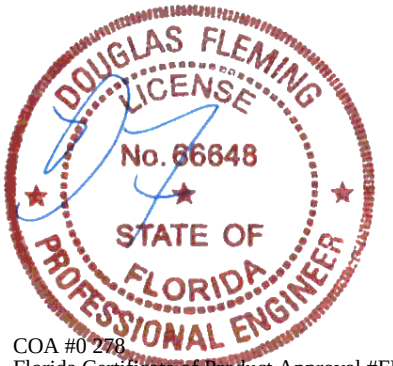
Additional Notes

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Refer to drawing PB160220723 for piggyback detail.



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06/24/2024

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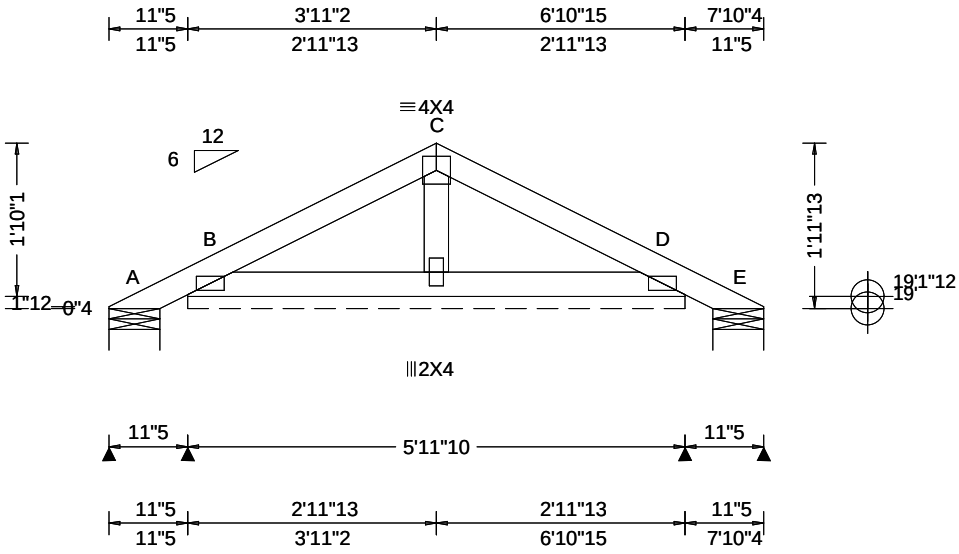
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26254 COMN FROM:	Ply: 1 Qty: 3	**Job Number:** 24-1285 Hedrick **Truss Label:** PB04	Cust: R 215 JRef:1Y0W2150008 T55 DrwNo: 176.24.0951.23930 NW / DF 06/24/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.00 ft ft
Loc. from endwall: not in 13.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	0.001 B	999	240
VERT(CL):	0.002 B	999	180
HORZ(LL):	-0.001 D	-	-
HORZ(TL):	0.001 D	-	-

Creep Factor: 2.0
Max TC CSI: 0.086
Max BC CSI: 0.074
Max Web CSI: 0.017

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	-	/-8	/-	/29	/29	/50
B*	105	/-	/-	/59	/3	/-
E	-	/-8	/-	/7	/7	/-

Wind reactions based on MWFRS
A Brg Wid = 7.3 Min Req = 1.5 (Truss)
B Brg Wid = 71.6 Min Req = -
E Brg Wid = 7.3 Min Req = 1.5 (Truss)
Bearings A, B, & E are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

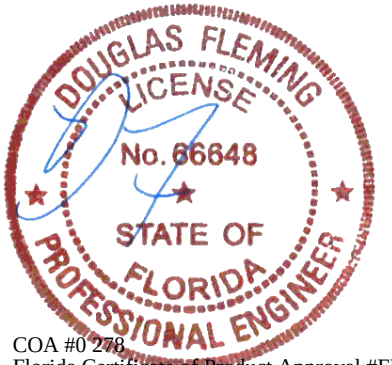
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Refer to DWG PB160220723 for piggyback details.



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06/24/2024

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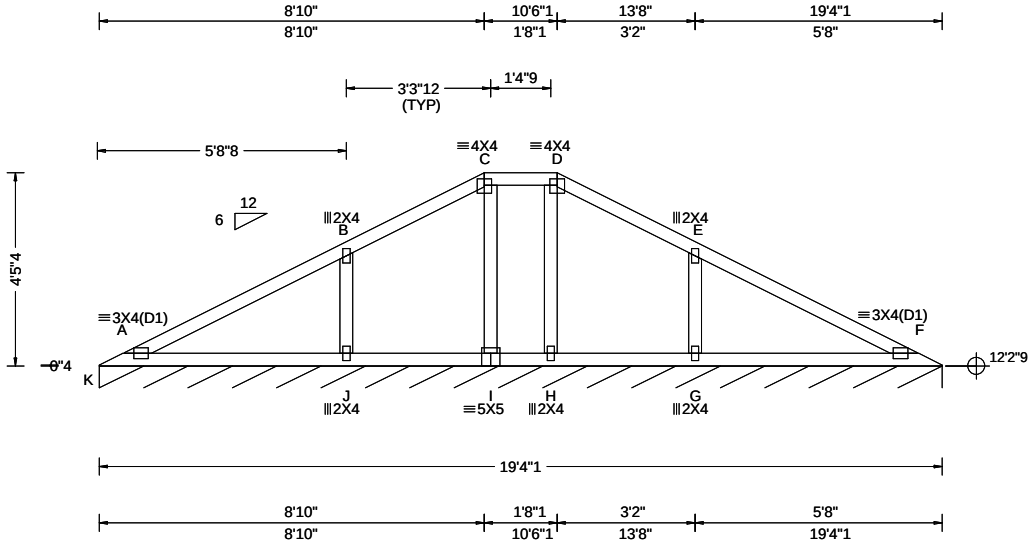
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26204 VAL FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** V01	Cust: R 215 JRef:1Y0W2150008 T60 DrwNo: 176.24.1002.20990 NW / DF 06/24/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.00 ft ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.019	F	999	240
VERT(CL): 0.039	F	999	180
HORZ(LL): -0.007	F	-	-
HORZ(TL): 0.014	F	-	-

Creep Factor: 2.0
Max TC CSI: 0.392
Max BC CSI: 0.246
Max Web CSI: 0.080

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
K*	82	/-	/-	/42	/1	/5

Wind reactions based on MWFRS
K Brg Wid = 232 Min Req = -
Bearing K is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

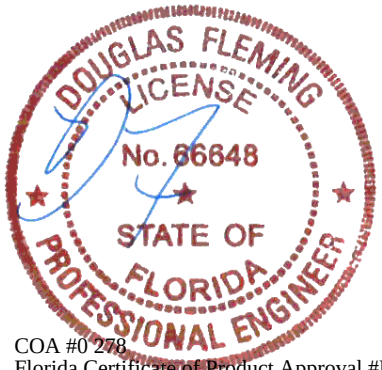
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.



COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

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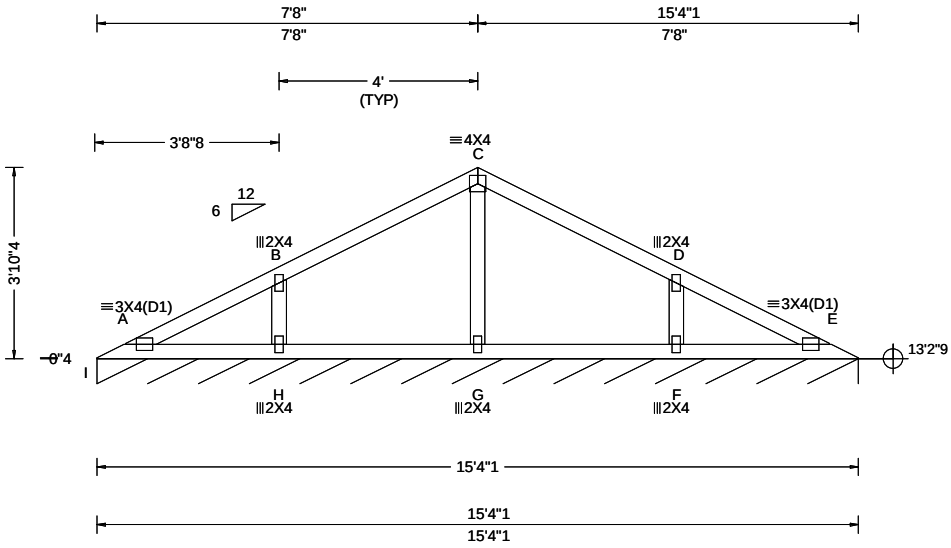
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26195 VAL FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** V02	Cust: R 215 JRef:1Y0W2150008 T62 DrwNo: 176.24.1002.22150 NW / DF 06/24/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.30 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.00 ft ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	0.004 E	999	240
VERT(CL):	0.008 E	999	180
HORZ(LL):	-0.001 E	-	-
HORZ(TL):	0.003 E	-	-

Creep Factor: 2.0
Max TC CSI: 0.275
Max BC CSI: 0.126
Max Web CSI: 0.067

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
I*	82	/-	/-	/42	/1	/6

Wind reactions based on MWFRS
I Brg Wid = 184 Min Req = -
Bearing I is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

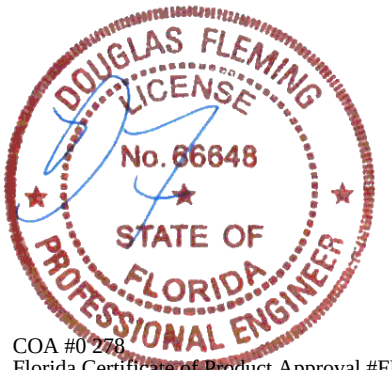
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.



COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

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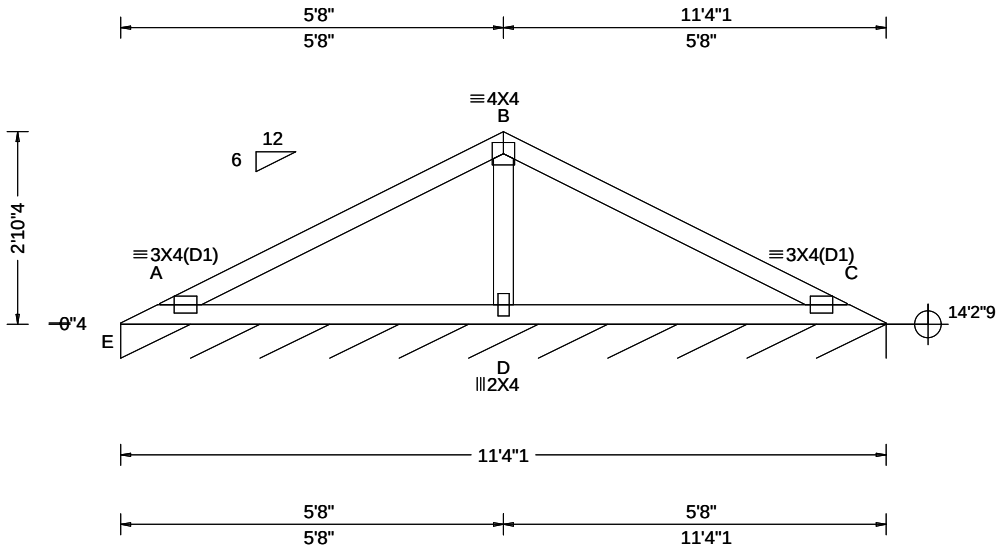
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26197 VAL FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** V03	Cust: R 215 JRef:1Y0W2150008 T63 DrwNo: 176.24.1002.23547 NW / DF 06/24/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.80 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.00 ft ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	0.021 C	999	240
VERT(CL):	0.044 C	999	180
HORZ(LL):	-0.009 C	-	-
HORZ(TL):	0.018 C	-	-

Creep Factor: 2.0
Max TC CSI: 0.434
Max BC CSI: 0.369
Max Web CSI: 0.133

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
E*	82	/-	/-	/41	/0	/6

Wind reactions based on MWFRS
E Brg Wid = 136 Min Req = -
Bearing E is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	455	- 213	B - C	455	- 227

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.
B - D	382	- 626

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.

COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

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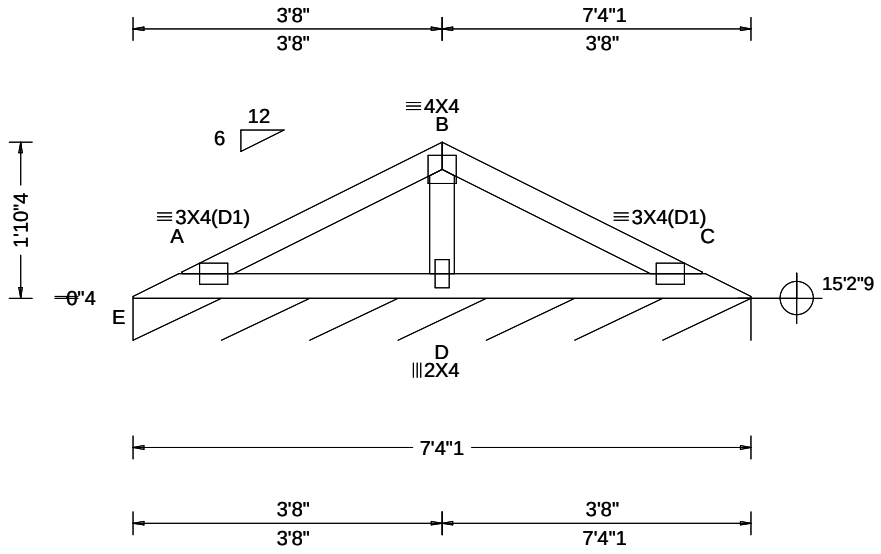
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26199 VAL FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** V04	Cust: R 215 JRef:1Y0W2150008 T46 DrwNo: 176.24.1003.49680 NW / DF 06/24/2024



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.005 C 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.011 C 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.002 C - -
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.005 C - -
NCBCLL: 10.00	Mean Height: 16.30 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.152
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.139
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.074
	C&C Dist a: 3.00 ft ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 23.02.01A.1204.18
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
E*	82	/-	/-	/40	/-	/5

Wind reactions based on MWFRS
E Brg Wid = 88.0 Min Req = -
Bearing E is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.

COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

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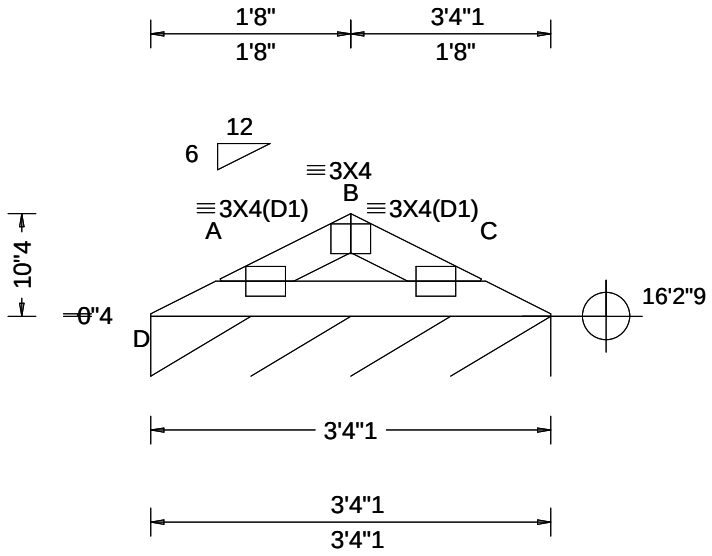
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26201 VAL FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** V05	Cust: R 215 JRef:1Y0W2150008 T42 DrwNo: 176.24.1003.51183 NW / DF 06/24/2024



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.80 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.002 C 999 240 VERT(CL): 0.005 C 999 180 HORZ(LL): -0.001 A - - HORZ(TL): 0.002 A - - Creep Factor: 2.0 Max TC CSI: 0.048 Max BC CSI: 0.074 Max Web CSI: 0.000 VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
D*	82	/-	/-	/35	/-	/4

Wind reactions based on MWFRS
D Brg Wid = 40.0 Min Req = -
Bearing D is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

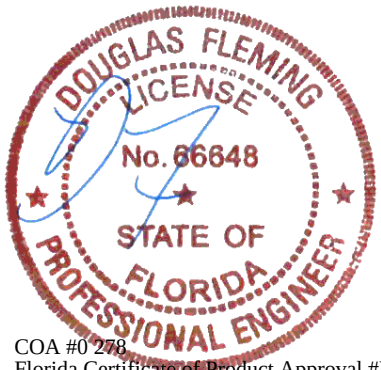
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.



COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

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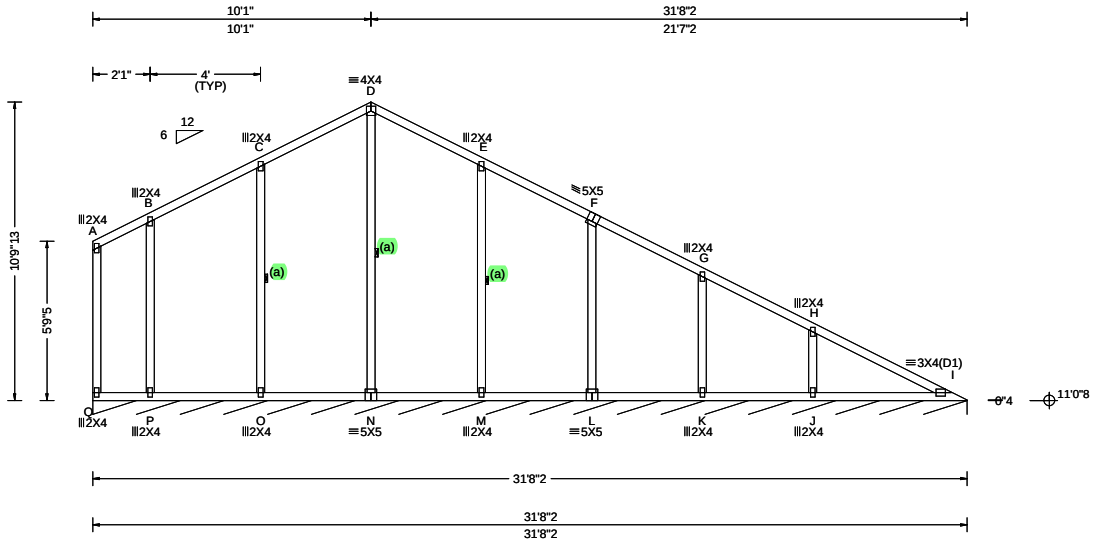
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26129 VAL FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** V06	Cust: R 215 JRef:1Y0W2150008 T65 DrwNo: 176.24.1003.54010 NW / DF 06/24/2024	



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.021 I 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.043 I 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.008 C - -
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.013 C - -
NCBCLL: 10.00	Mean Height: 16.61 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.336
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.251
Spacing: 24.0 "	MWFRS Parallel Dist: h/2 to h	Rep Fac: Yes	Max Web CSI: 0.203
	C&C Dist a: 3.17 ft ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 23.02.01A.1204.18
	Wind Duration: 1.60		

▲ Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
Q*	82	/-	/-	/47	/14	/9

Wind reactions based on MWFRS
Q Brg Wid = 380 Min Req = -
Bearing Q is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Wind

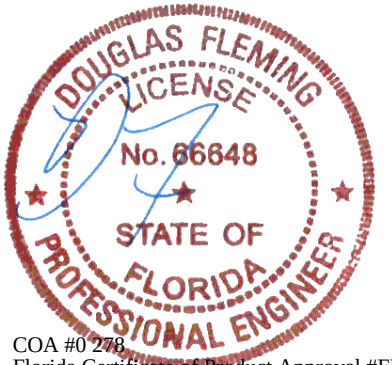
Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.



COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

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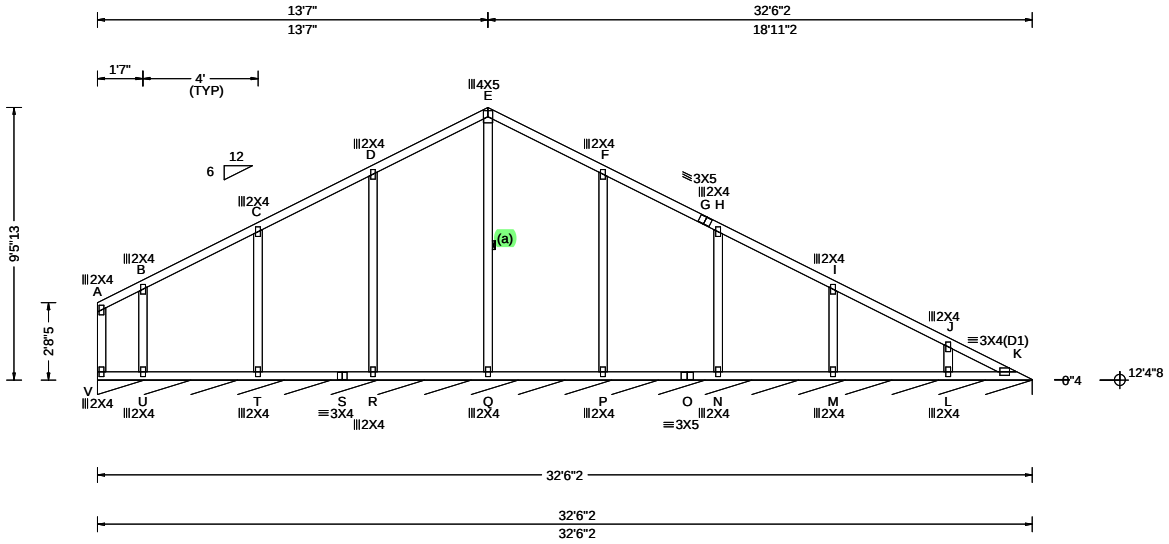
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26440 VAL FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** V07		Cust: R 215 JRef:1Y0W2150008 T69 DrwNo: 176.24.1003.56050 NW / DF 06/24/2024	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 17.27 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.26 ft ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.002	F	999	240
VERT(CL): 0.004	F	999	180
HORZ(LL): -0.007	B	-	-
HORZ(TL): 0.013	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.202
Max BC CSI: 0.113
Max Web CSI: 0.288

VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
V*	82	/-	/-	/44	/14	/7

Wind reactions based on MWFRS
V Brg Wid = 390 Min Req = -
Bearing V is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Wind

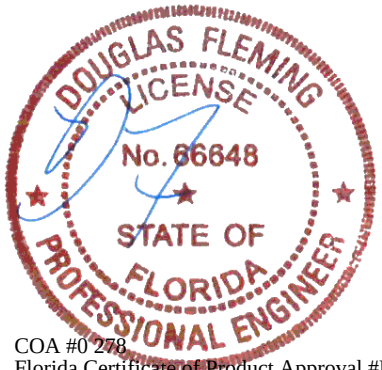
Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.



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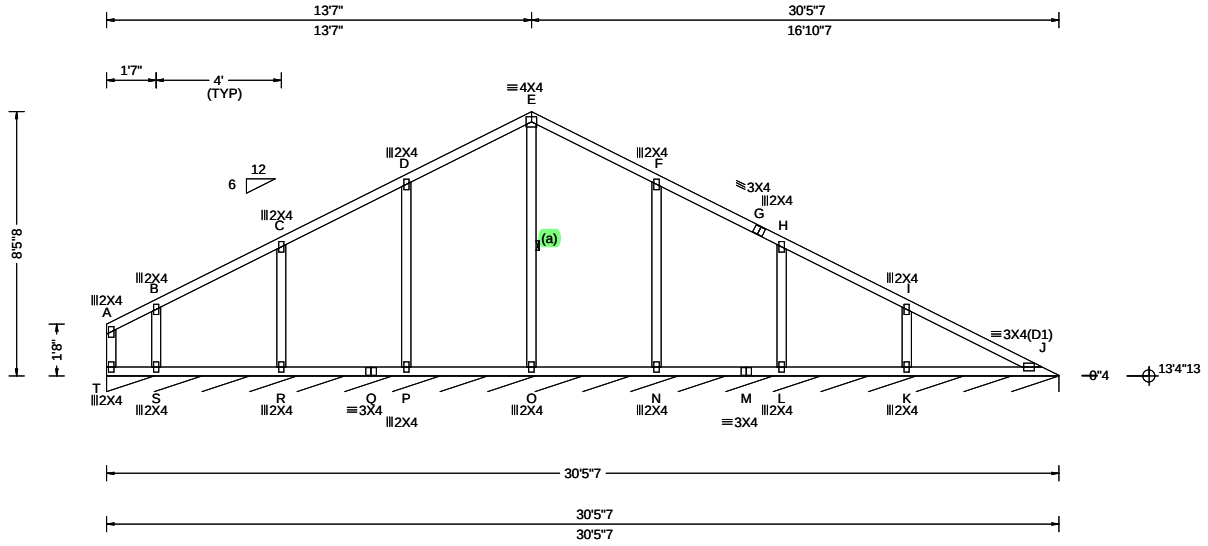
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26454 VAL FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** V08	Cust: R 215 JRef:1Y0W2150008 T54 DrwNo: 176.24.1003.57320 NW / DF 06/24/2024



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.013 J 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.026 J 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.006 A - -
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.011 A - -
NCBCLL: 10.00	Mean Height: 17.79 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.277
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.206
Spacing: 24.0 "	MWFRS Parallel Dist: h/2 to h	Rep Fac: Yes	Max Web CSI: 0.214
	C&C Dist a: 3.05 ft ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 23.02.01A.1204.18
	Wind Duration: 1.60		

▲ Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
T*	82	/-	/-	/43	/14	/7

Wind reactions based on MWFRS
T Brg Wid = 365 Min Req = -
Bearing T is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Wind

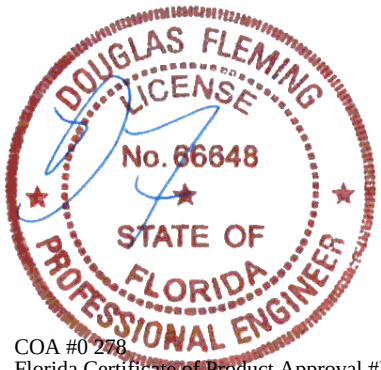
Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.



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06/24/2024

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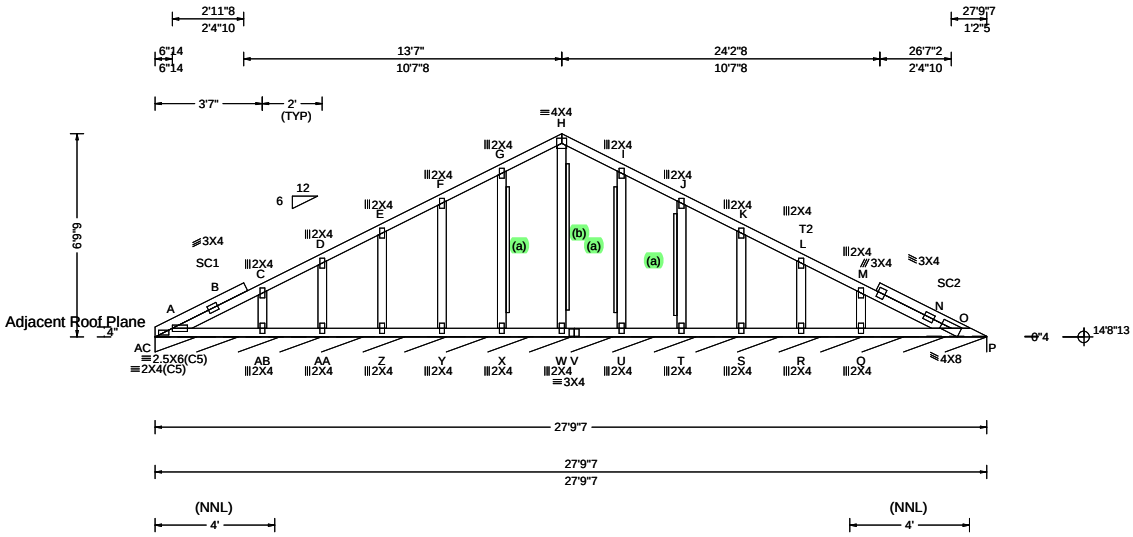
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 9969 VAL FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** V09		Cust: R 215 JRef:1Y0W2150008 T15 DrwNo: 176.24.1003.59853 NW / DF 06/24/2024	



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.004 B 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.009 B 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.002 G - -
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.003 B - -
NCBCLL: 10.00	Mean Height: 18.28 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.070
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.080
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.932
	C&C Dist a: 3.00 ft ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 24.01.02.0529.20
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
AC*82	/-	/-		/42	/13	/6

Wind reactions based on MWFRS
AC Brg Wid = 333 Min Req = -
Bearing AC is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP M-31; T2 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP M-31;
Stack Chord: SC2 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/287.

Gable Reinforcement

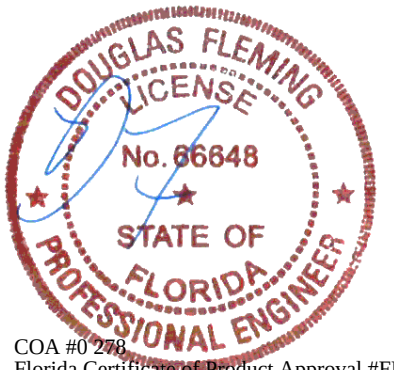
(a) 1x4 "L" reinforcement. Any species and grade. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.
(b) 1x4 "L" reinforcement. Same species and grade as web. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Stacked top chord must NOT be notched or cut in area (NNL). Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

See DWGS VALTN220723 and VAL180220723 for valley details.



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06/24/2024

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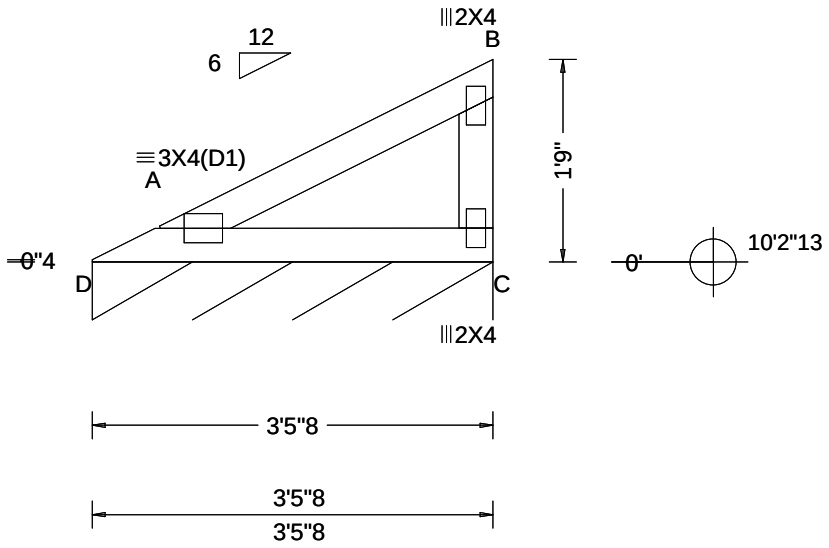
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26206 VAL FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** V10	Cust: R 215 JRef:1Y0W2150008 T35 DrwNo: 176.24.1004.09157 NW / DF 06/24/2024



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.003 A - - HORZ(TL): 0.005 A - - Creep Factor: 2.0 Max TC CSI: 0.138 Max BC CSI: 0.113 Max Web CSI: 0.068 VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
D*	82	/-	/-	/49	/2	/20

Wind reactions based on MWFRS
D Brg Wid = 41.5 Min Req = -
Bearing D is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

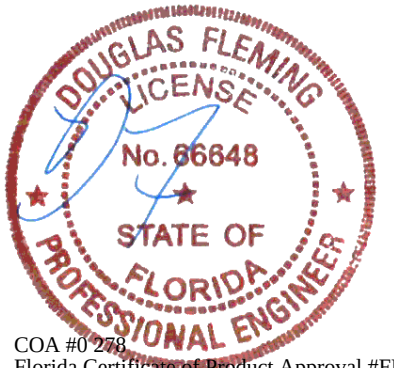
Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.



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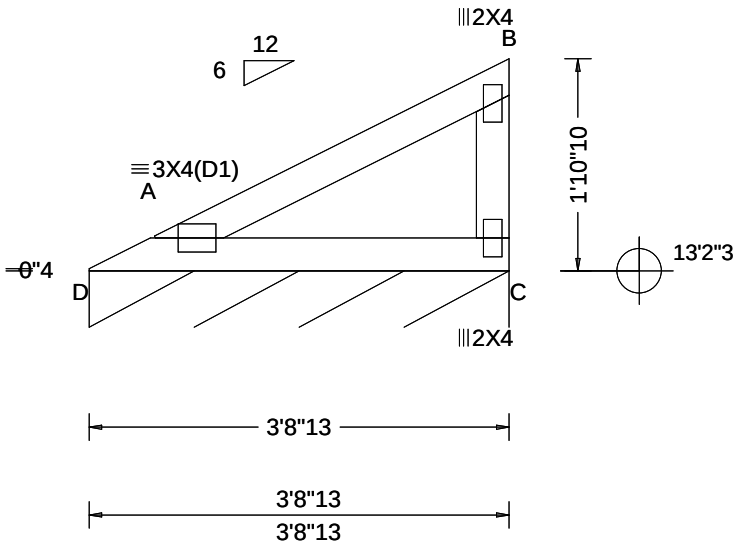
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26456 VAL FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** V11		Cust: R 215 JRef:1Y0W2150008 T37 DrwNo: 176.24.1004.12467 NW / DF 06/24/2024	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.002 A - - HORZ(TL): 0.005 A - - Creep Factor: 2.0 Max TC CSI: 0.166 Max BC CSI: 0.130 Max Web CSI: 0.079 VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
D*	82	/-	/-	/50	/10	/20

Wind reactions based on MWFRS
D Brg Wid = 44.8 Min Req = -
Bearing D is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

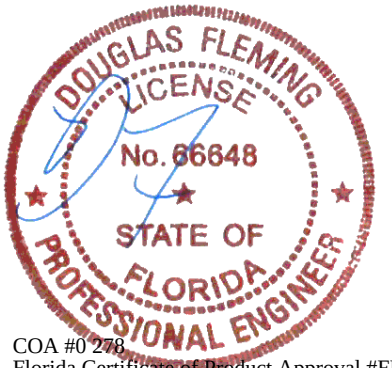
Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.



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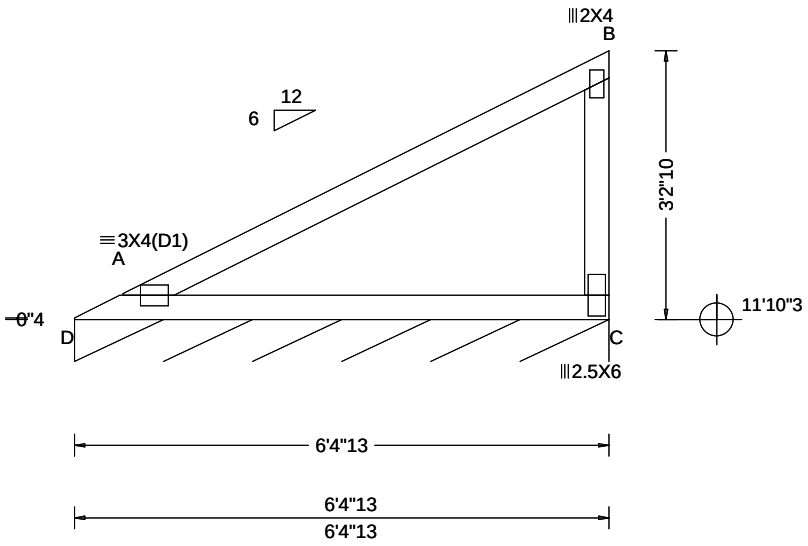
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26460 VAL FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** V12	Cust: R 215 JRef:1Y0W2150008 T64 DrwNo: 176.24.1004.13513 NW / DF 06/24/2024



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.012 A - - HORZ(TL): 0.025 A - - Creep Factor: 2.0 Max TC CSI: 0.528 Max BC CSI: 0.440 Max Web CSI: 0.238 VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
D*	82	/-	/-	/52	/12	/21

Wind reactions based on MWFRS
D Brg Wid = 76.8 Min Req = -
Bearing D is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.

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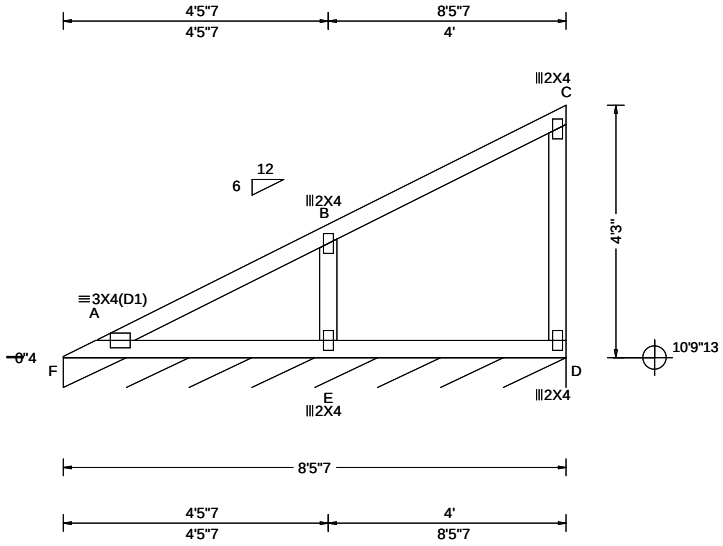
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26458 VAL FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** V13		Cust: R 215 JRef:1Y0W2150008 T66 DrwNo: 176.24.1004.14680 NW / DF 06/24/2024	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**	**▲Maximum Reactions (lbs), or *=PLF**
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.009 A 999 240 VERT(CL): 0.018 A 999 180 HORZ(LL): -0.002 C - - HORZ(TL): 0.005 A - - Creep Factor: 2.0 Max TC CSI: 0.295 Max BC CSI: 0.198 Max Web CSI: 0.319 VIEW Ver: 23.02.01A.1204.18	Gravity / Non-Gravity Loc R+ / R- / Rh / Rw / U / RL F* 82 /- /- /53 /12 /21 Wind reactions based on MWFRS F Brg Wid = 101 Min Req = - Bearing F is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

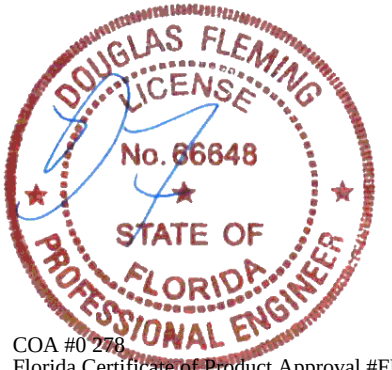
Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.



COA #0 278
Florida Certificate of Product Approval #FL1999
06/24/2024

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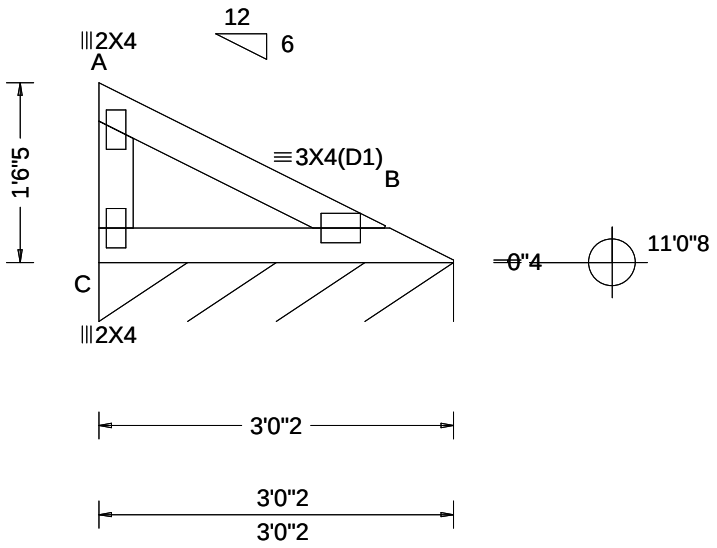
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 26489 VAL FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** V14	Cust: R 215 JRef:1Y0W2150008 T8 DrwNo: 176.24.1004.16837 NW / DF 06/24/2024



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.004 B 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.009 B 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.002 B - -
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.003 B - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.099
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.087
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.051
	C&C Dist a: 3.00 ft ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 23.02.01A.1204.18
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
C*	82	/-	/-	/48	/1	/10

Wind reactions based on MWFRS
C Brg Wid = 36.1 Min Req = -
Bearing C is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

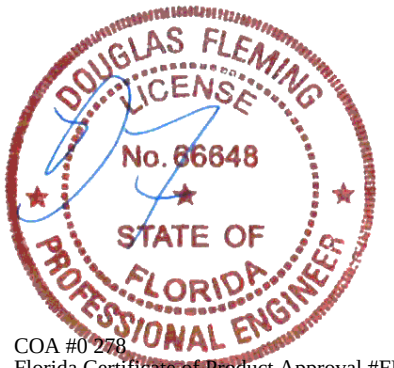
Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.



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06/24/2024

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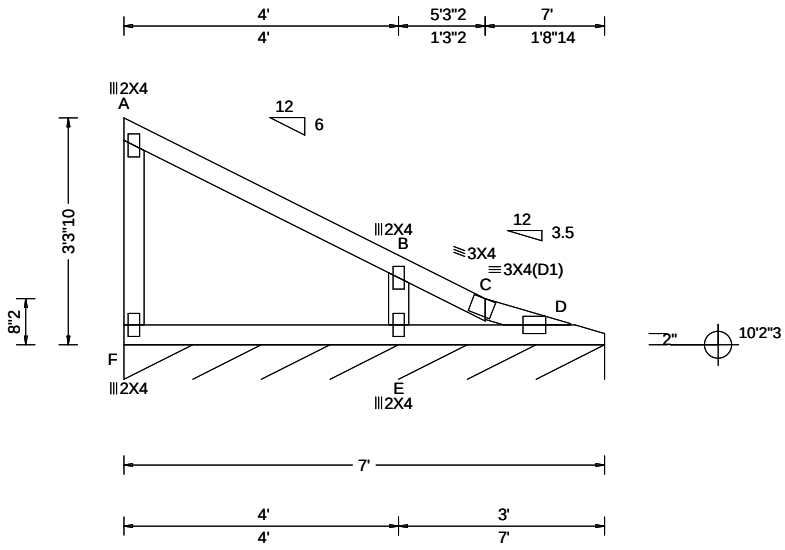
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 34716 VAL FROM:	Ply: 1 Qty: 1	**Job Number:** 24-1285 Hedrick **Truss Label:** V15		Cust: R 215 JRef:1Y0W2150008 T14 DrwNo: 176.24.1004.18633 NW / DF 06/24/2024	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 D 999 240 VERT(CL): 0.002 D 999 180 HORZ(LL): 0.001 A - - HORZ(TL): 0.001 A - - Creep Factor: 2.0 Max TC CSI: 0.198 Max BC CSI: 0.117 Max Web CSI: 0.091 VIEW Ver: 23.02.01A.1204.18

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
D*	82	/-	/-	/51	/3	/11

Wind reactions based on MWFRS
D Brg Wid = 84.0 Min Req = -
Bearing F is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

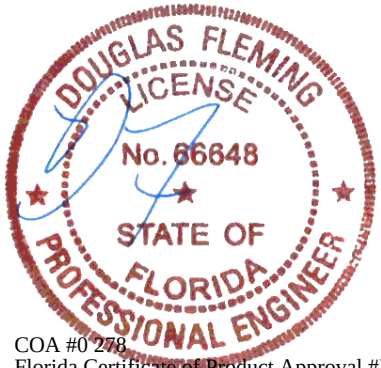
Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.



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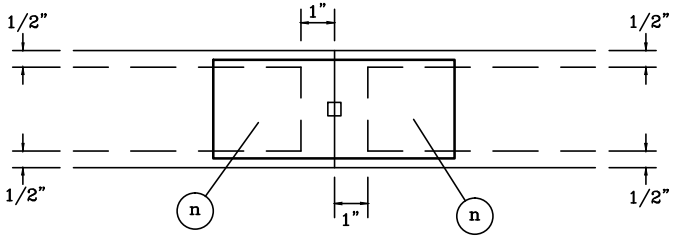
Glenview, IL 60025

TRULOX INFORMATION DETAIL

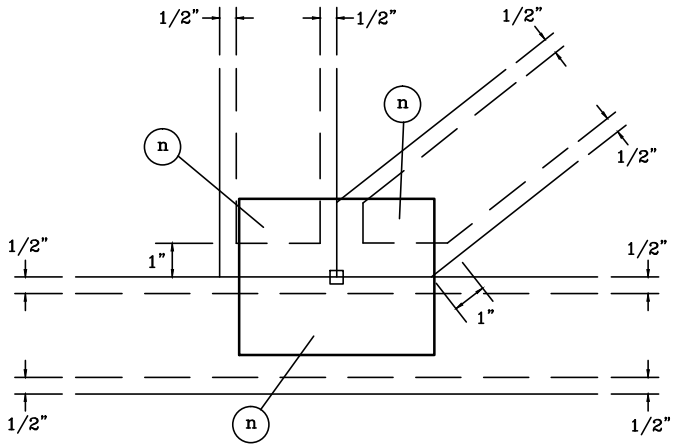
TYPICAL OFF PANEL SPLICE



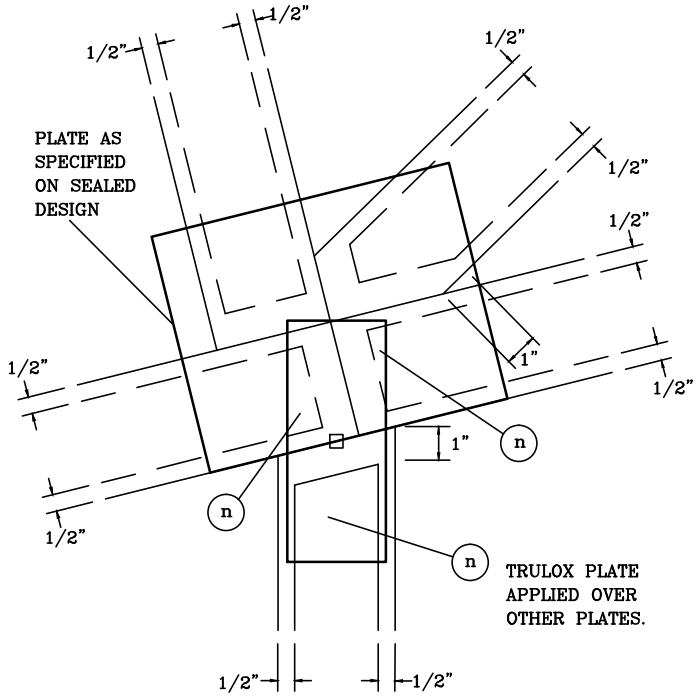
DO NOT APPLY NAILS WITHIN 1/2" OF LUMBER EDGES OR 1" OF LUMBER ENDS ON EACH FACE, AS SHOWN BY DASHED LINES.

NAILS MUST NOT SPLIT LUMBER.

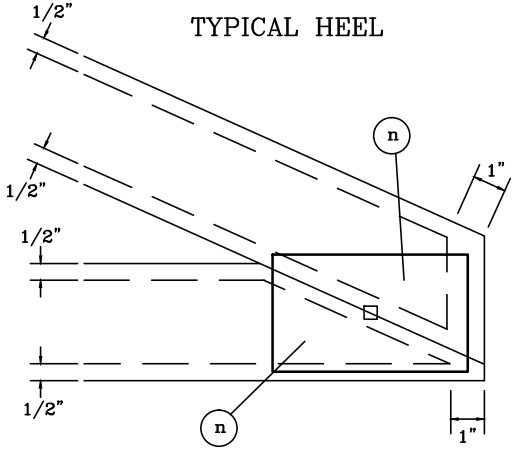
TYPICAL PANEL POINT WITHOUT SPLICE



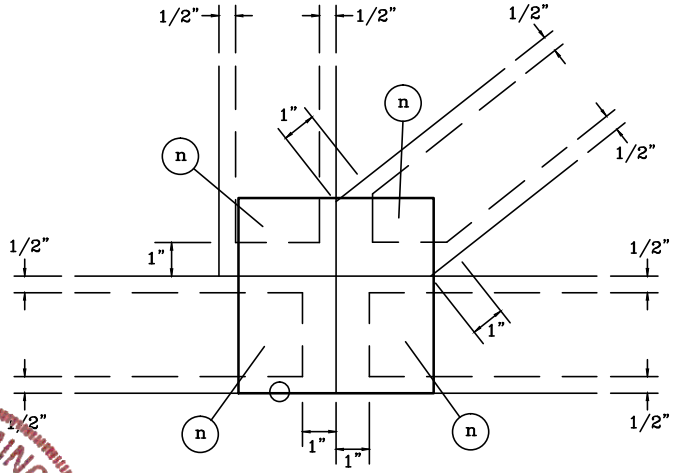
TYPICAL FILLER



TYPICAL HEEL



TYPICAL PANEL POINT SPLICE



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Glenview, IL 60025

NOTES:
(n) IS THE REQUIRED NUMBER OF 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY AS SPECIFIED ON THE SEALED DESIGN REFERENCING THIS DETAIL.
○ LOCATES PLATE CORNER OR FLUSH EDGE.
□ LOCATES PLATE CENTER.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

06/24/2024
COA #0 278

TRULOX PLATING

160 TL

PAGE 1 OF 1

DATE 10/01/14

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforecement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

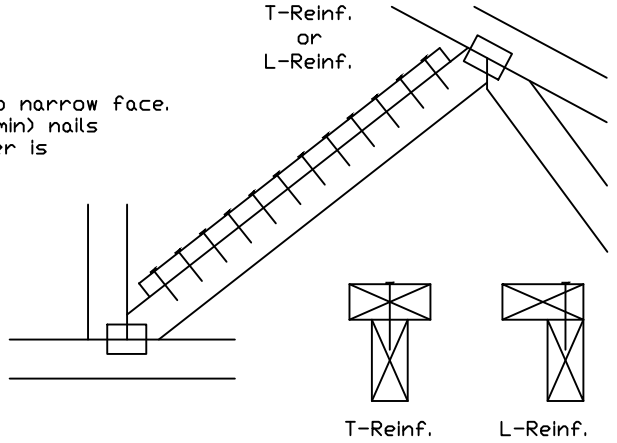
Web Member Size	Specified CLR Restraint	Alternative Reinforecement T- or L- Reinf.	Alternative Reinforecement Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.

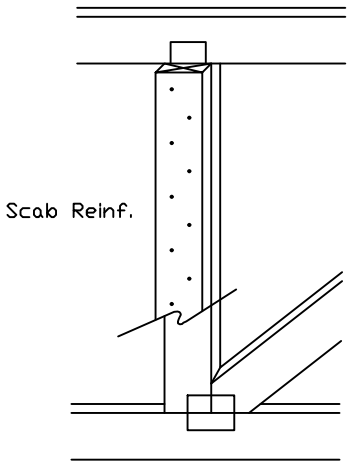
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



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06/24/2024

COA #0 278

ALPINE AN ITW COMPANY
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North Building, 4th Floor
Glenview, IL 60025

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TC LL		PSF
TC DL		PSF
BC DL		PSF
BC LL		PSF
TOT. LD.		PSF
DUR. FAC.		
SPACING		

REF	CLR Subst.
DATE	01/02/19
DRWG	BRCLBSUB0119

Commentary: Deflection and Camber

Camber may be built into trusses to compensate for the vertical deflection that results from the application of loads. Providing camber has the following advantages:

- Helps to ensure level ceilings and floors after dead loads are applied.
- Facilitates drainage to avoid ponding on flat or low slope roofs.
- Compensates for different deflection characteristics between adjacent trusses.
- Improves appearance of garage door headers and other long spans that can appear to "sag."
- Avoids "dips" in roof ridgelines at the transition from the gable to adjacent clear span trusses.

In accordance with ANSI/TPI 1 the Building Designer, through the Construction Documents, shall provide the location, direction, and magnitude of all loads attributable to ponding that may occur due to the design of the roof drainage system. The Building Designer shall also specify any dead load, live load, and in-service creep deflection criteria for flat or low-slope roofs subject to ponding loads.

The amount of camber is dependent on the truss type, span, loading, application, etceteras.

More restrictive limits for allowable deflection and slenderness ratio (L/D) may be required to help control vibration.

The following tables are provided as guidelines for limiting deflection and estimating camber. Conditions or codes may exist that require exceeding these recommendations, or past experience may warrant using more stringent limitations.

L = Span of Truss (inches)
D = Depth of Truss at Deflection Point (inches)

Recommended Truss Deflection Limits

Truss Type	L/D	Deflection Limits	
		Live Load	Total Load
Pitched Roof Trusses	24	L/240 (vertical)	L/180 (vertical)
Floor of Room-In-Attic Trusses	24	L/360 (vertical)	L/240 (vertical)
Flat or Shallow Pitched Roof Trusses	24	L/360 (vertical)	L/240 (vertical)
Residential Floor Trusses	24	L/360 (vertical)	L/240 (vertical)
Commercial Floor Trusses	20	L/480 (vertical)	L/240 (vertical)
Scissors Trusses	24	0.75" (horizontal)	1.25" (horizontal)

Truss Type	Recommended Camber
Pitched Trusses	1.00 x Deflection from Actual Dead Load
Sloping Parallel Chord Trusses	1.5 x Vertical Deflection from Actual Dead Load
Floor Trusses	(0.25 x Deflection from Live Load) + Actual Dead Load
Flat Roof Trusses	(0.25 x Deflection from Live Load) + (1.5 x Design Dead Load Deflection)

Note: The actual dead load may be considerably less than the design dead load.

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REF	DEFLEC/CAMB
DATE	10/01/14
DRWG	DEFLCAMB1014

Piggyback Detail - ASCE 7-22: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

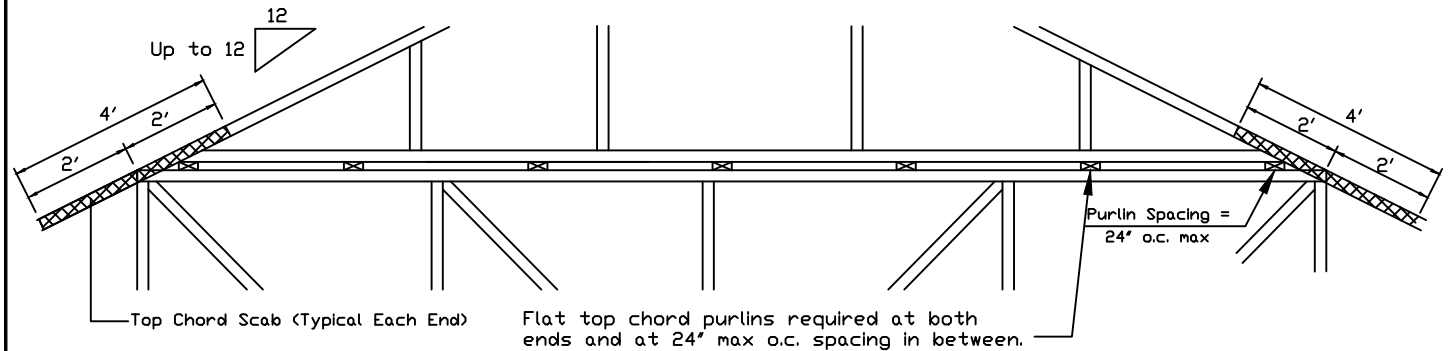
160 mph Wind, 30.00 ft Mean Hgt, ASCE 7-22, Enclosed Bldg. located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0.
Or 140 mph wind, 30.00 ft Mean Hgt, ASCE 7-22, Enclosed Bldg. located anywhere in roof, Exp D, Wind DL= 5.0 psf (min), Kzt=1.0.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building designer shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24" o.c. Detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Detail A : Purlin Spacing = 24" o.c. or less



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4" o.c.

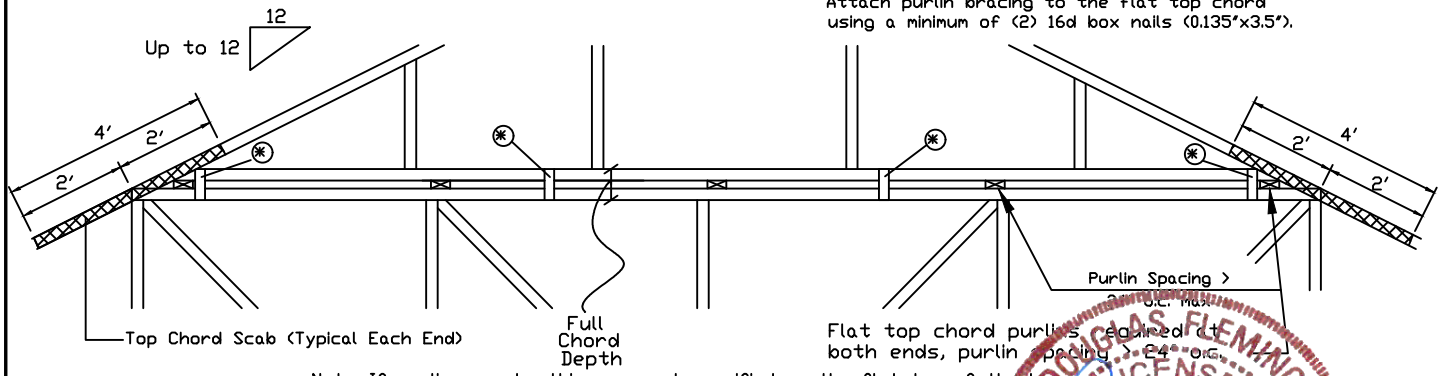
Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3X8 Trulox plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (1) 28PB wave piggyback plate plated to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

Detail B : Purlin Spacing > 24" o.c.

Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4" o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").



Note: If purlins or sheathing are not specified on the flat top of the base truss, purlins must be installed at 24" o.c. max. and use Detail

* In addition, provide connection with one of the following methods:

Trulox
Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.

APA Rated Gusset
8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.

2x4 Vertical Scabs
2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.

28PB Wave Piggyback Plate
One 28PB wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

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THOMAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
06/24/2024
COA #0 278

REF	PIGGYBACK
DATE	07/03/2023
DRWG	PB160220723
SPACING	24.0"

Valley Detail - ASCE 7-22: 180 mph, 30' Mean Height, Partially Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with:
535# connection or with (1) Simpson H2.5A or equivalent connector for
ASCE 7-22 180 mph. 30' Mean Height, Part. Enc. Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00
Or
ASCE 7-22 160 mph. 30' Mean Height, Part. Enc. Building, Exp. D, Wind TC DL=5 psf, Kzt = 1.00

Bottom chord may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

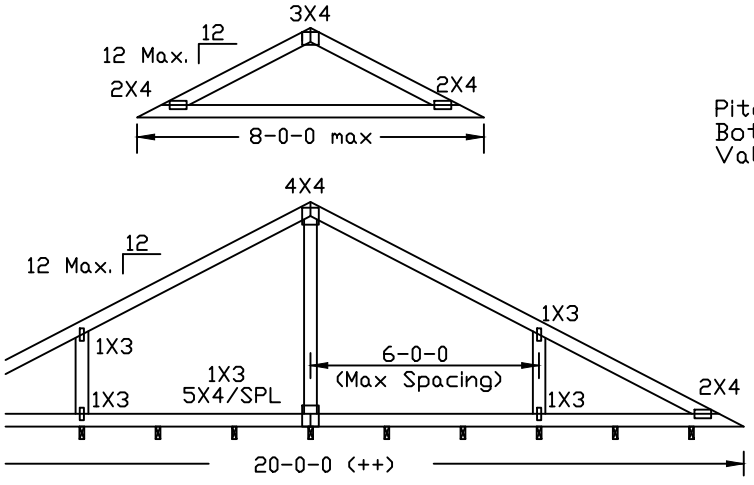
All plates shown are Alpine Wave Plates.

Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7-9" apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

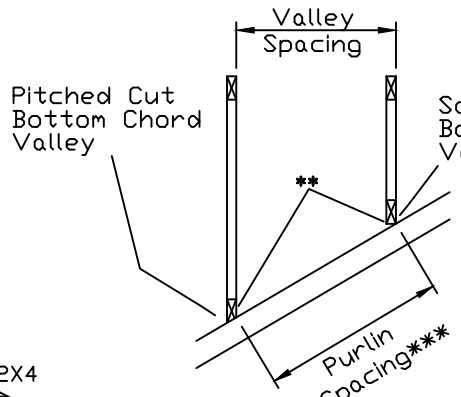
Top chord of truss beneath valley set must be braced with:
properly attached, rated sheathing applied prior to valley truss installation.
Or
Purlins at 24" o.c. or as otherwise specified on engineer's sealed design
Or
By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

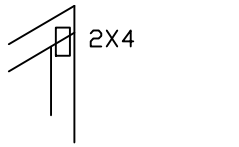
*** Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.

++ Larger spans may be built as long as the vertical height does not exceed 14'-0".



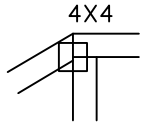
Supporting trusses at 24" o.c. maximum spacing.



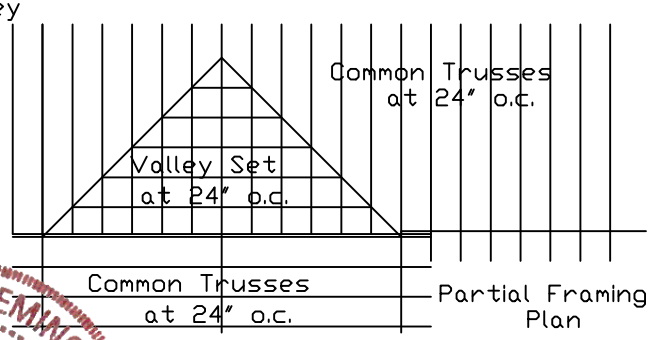


Square Cut Bottom Chord Valley

Stubbed Valley End Detail



Optional Hip Joint Detail



Partial Framing Plan

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

06/24/2024

COA #0 278

ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

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ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

TC LL	30	30	40	PSF
TC DL	20	15	7	PSF
BC DL	10	10	10	PSF
BC LL	0	0	0	PSF
TOT. LD.	60	55	57	PSF
DUR.FAC.	1.25	1.33	1.15	1.15
SPACING			24.0"	

REF	VALLEY DETAIL
DATE	07/03/2023
DRWG	VAL180220723

Valley Detail - ASCE 7-22: 30' Mean Height, Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with:
(2) 16d box (0.135" x 3.5") nails toe-nailed for ASCE 7-22, 30' Mean Height, Enclosed Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00, Max. Wind Speed based on supporting truss material at connection location:
140 mph for SP (G = 0.55, min.),
125 mph for DF-L (G = 0.50, min.), or
105 mph for HF & SPF (G = 0.42, min.).

Maximum top chord pitch is 10/12 for supporting trusses below valley trusses.

Bottom chord of valley trusses may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

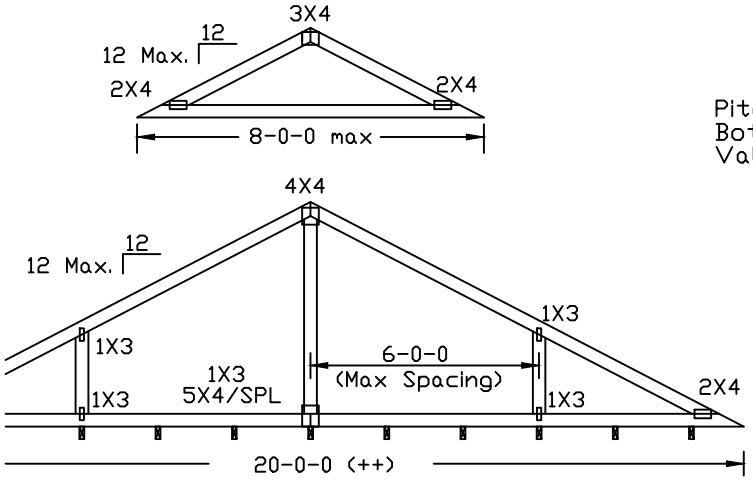
All plates shown are Alpine Wave Plates.

Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7-9" apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

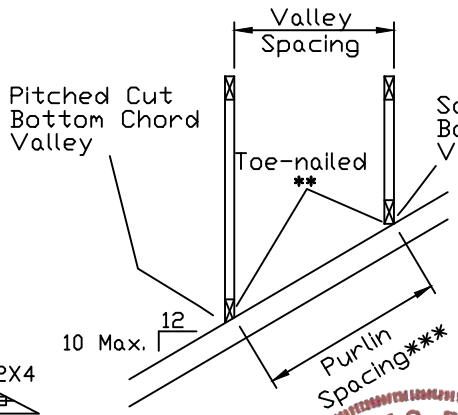
Top chord of truss beneath valley set must be braced with:
properly attached, rated sheathing applied prior to valley truss installation.
Or
Purlins at 24" o.c. or as otherwise specified on engineer's sealed design
Or
By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

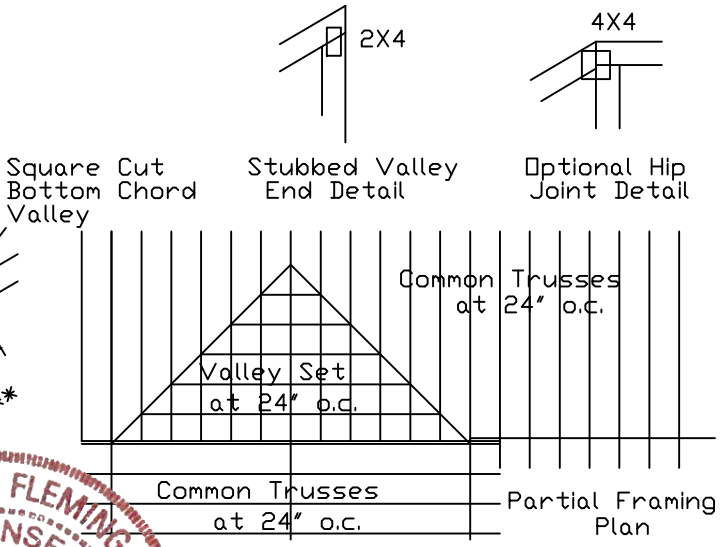
*** Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.

++ Larger spans may be built as long as the vertical height does not exceed 14'-0".



Supporting trusses at 24" o.c. maximum spacing.





DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
06/24/2024
COA #0 278

ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

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TC LL	30	30	40PSF
TC DL	20	15	7PSF
BC DL	10	10	10 PSF
BC LL	0	0	0PSF
TOT. LD.	60	55	57PSF
DUR.FAC.	1.25/1.33	1.15	1.15
SPACING		24.0"	

REF	VALLEY DETAIL
DATE	07/03/2023
DRWG	VALTN220723

NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

BLOCK LOCATION, SIZE, LENGTH,GRADE AND TOTAL NUMBER AND TYPE OF NAILS ARE TO BE SPECIFIED ON SEALED DESIGN REFERENCING THIS DETAIL.

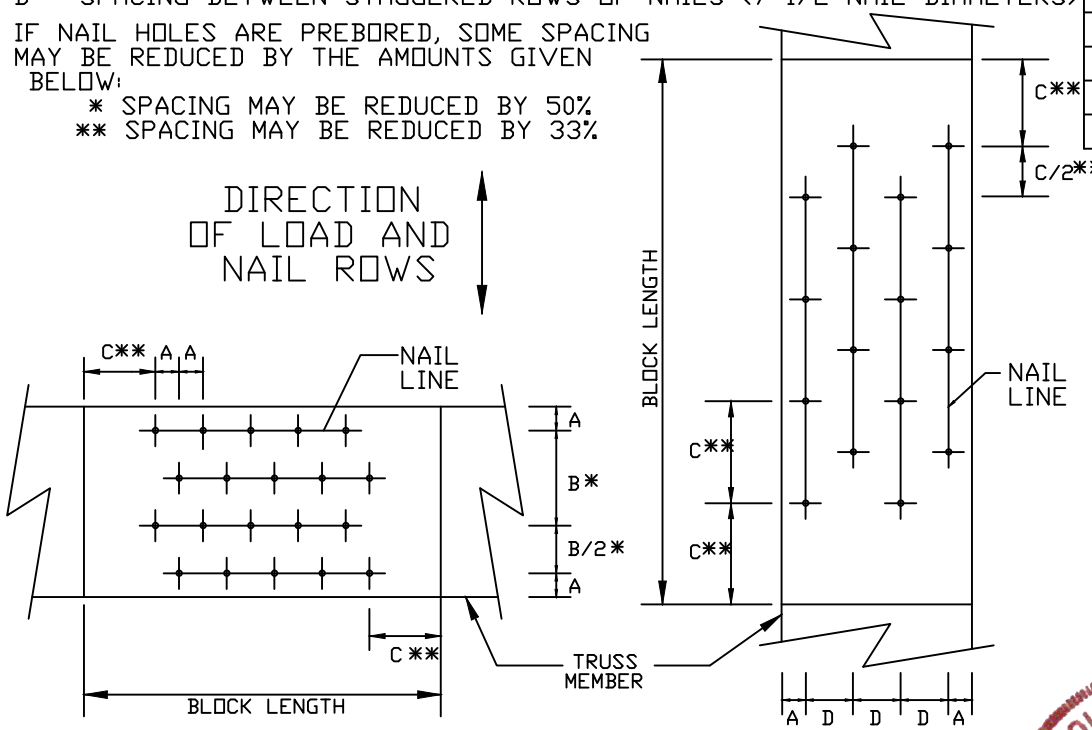
LOAD PERPENDICULAR TO GRAIN
A - EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)
B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)
C - END DISTANCE (15 NAIL DIAMETERS)

LOAD PARALLEL TO GRAIN
A - EDGE DISTANCE (6 NAIL DIAMETERS)
C - SPACING OF NAILS IN A ROW AND END DISTANCE (15 NAIL DIAMETERS)
D - SPACING BETWEEN STAGGERED ROWS OF NAILS (7 1/2 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW:
* SPACING MAY BE REDUCED BY 50%
** SPACING MAY BE REDUCED BY 33%

MINIMUM NAIL SPACING DISTANCES

NAIL TYPE	DISTANCES A	B*	C**	D
8d BOX (0.113"X 2.5",MIN)	3/4"	1 3/8"	1 3/4"	7/8"
10d BOX (0.128"X 3.",MIN)	7/8"	1 5/8"	2"	1"
12d BOX (0.128"X 3.25",MIN)	7/8"	1 5/8"	2"	1"
16d BOX (0.135"X 3.5",MIN)	7/8"	1 5/8"	2 1/8"	1 1/8"
20d BOX (0.148"X 4.",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
8d COMMON (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
10d COMMON (0.148"X 3.",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
12d COMMON (0.148"X 3.25",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
16d COMMON (0.162"X 3.5",MIN)	1'	2"	2 1/2"	1 1/4"
GUN (0.120"X 2.5",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
GUN (0.120"X 3.",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 3.",MIN)	7/8"	1 5/8"	2"	1"



LOAD APPLIED PERPENDICULAR TO GRAIN

LOAD APPLIED PARALLEL TO GRAIN

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06/24/2024

COA #0 278

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REF	NAIL SPACE
DATE	10/01/14
DRWG	CNNAILSP1014