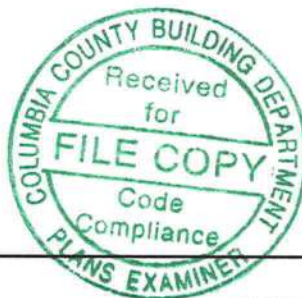


Julius Lee



73

RE: 497392 - GIEBEIG - Lot 44 Mayfair

**1109 Coastal Bay Blvd.
Boynton Beach, FL 33435**

Site Information:

Project Customer: GIEBEIG HOMES Project Name: 497392 Model: ST. JOHNS MODIFIED
Lot/Block: 44 Subdivision: MAYFAIR
Address:
City: COLUMBIA CTY State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: BRIAN TRENT GIEBEIG License #: RR282811523
Address: 462 SW FAIRLINGTON CT
City: LAKE CITY State: FL

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

FBC 2010/TPI 2007 Design Program: MiTek 20/20 7.3
ASCE 7-10 Wind Speed: 130 mph Floor Load: N/A psf
Roof Load: 32.0 psf

This package includes 29 individual, dated Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany this coversheet. The latest approval dates supersede and replace the previous drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	I6822107	CJ1	6/5/013	18	I6822124	T11	6/5/013
2	I6822108	CJ3	6/5/013	19	I6822125	T12	6/5/013
3	I6822109	CJ5	6/5/013	20	I6822126	T13	6/5/013
4	I6822110	EJ01	6/5/013	21	I6822127	T14	6/5/013
5	I6822111	EJ7	6/5/013	22	I6822128	T15	6/5/013
6	I6822112	HJ01	6/5/013	23	I6822129	T15A	6/5/013
7	I6822113	HJ9	6/5/013	24	I6822130	T16	6/5/013
8	I6822114	T01	6/5/013	25	I6822131	T17	6/5/013
9	I6822115	T02	6/5/013	26	I6822132	T18	6/5/013
10	I6822116	T03	6/5/013	27	I6822133	T19	6/5/013
11	I6822117	T04	6/5/013	28	I6822134	T20	6/5/013
12	I6822118	T05	6/5/013	29	I6822135	T21	6/5/013
13	I6822119	T06	6/5/013				
14	I6822120	T07	6/5/013				
15	I6822121	T08	6/5/013				
16	I6822122	T09	6/5/013				
17	I6822123	T10	6/5/013				

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Jax).

Truss Design Engineer's Name: Julius Lee

My license renewal date for the state of Florida is February 28, 2015.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.



June 5, 2013

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 497392	Truss CJ5	Truss Type Jack-Open Truss	Qty 14	Ply 1	GIEBEIG - Lot 44 Mayfair Job Reference (optional)	16822109
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Builders FirstSource, Lake City, FL 32055

7.350 s Sep 27 2012 MiTek Industries, Inc. Wed Jun 05 12:18:21 2013 Page 1
ID: 9B5QRIZPhUL0yMYqzVn3hhzz67b-B1nGon0kV6PeXcX0HJPuJdx9Jd7sRHqI3AAxGiz9KsW

Scale = 1/20

Plate Offsets (X,Y): [2-0-0,0-1-2]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.25	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.18	Vert(LL) 0.05 4-7 >999 240		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.00	Vert(TL) 0.04 4-7 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix-M)	Horz(TL) -0.00 2 n/a n/a		
	Code FBC2010/TPI2007			Weight: 19 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING
TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=79/Mechanical, 2=253/0-3-8 (min. 0-1-8), 4=23/Mechanical
Max Horz 2=128(LC 12)
Max Uplift 3=70(LC 12), 2=105(LC 12), 4=26(LC 9)
Max Grav 3=97(LC 2), 2=304(LC 2), 4=56(LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-281/457
BOT CHORD 2-4=-740/436

NOTES (7-9)
1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; End.; GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 70 lb uplift at joint 3, 105 lb uplift at joint 2 and 26 lb uplift at joint 4.
6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
8) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

June 5, 2013

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 497392	Truss EJ7	Truss Type Jack-Partial Truss	Qty 29	Ply 1	GIEBEIG - Lot 44 Mayfair Job Reference (optional)	16822111
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Builders FirstSource, Lake City, FL 32055

7.350 s Sep 27 2012 MiTek Industries, Inc. Wed Jun 05 12:18:23 2013 Page 1
ID:9B5QRtZPhUL0yMYqzVn3hhzz6?b-7Qv0DT1_2jMmvpPPkRyiA1RSRmXvAKaWUf1Ldz9KsU

Scale = 1/25.3

Plate Offsets (X,Y): [2-0-5-0-0-4]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.46	Vert(LL)	0.19	4-7	>436	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.36	Vert(TL)	0.16	4-7	>527	180		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.01	2	n/a	n/a		
BCDL 5.0	Code	FBC2010/TPI2007	(Matrix-M)							

Weight: 26 lb FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=113/Mechanical, 2=318/0-3-8 (min. 0-1-8), 4=32/Mechanical
Max Horz 2=115(LC 12)
Max Uplift 3=64(LC 12), 2=96(LC 9), 4=34(LC 9)
Max Grav 3=139(LC 2), 2=380(LC 2), 4=79(LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-685/1162
BOT CHORD 2-4=-1754/1060

NOTES (7-9)
1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; End.; GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 64 lb uplift at joint 3, 96 lb uplift at joint 2 and 34 lb uplift at joint 4.
6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
8) Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB.
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

June 5,2013

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 497392	Truss HJ9	Truss Type Diagonal Hip Girder	Qty 7	Ply 1	GIEBEIG - Lot 44 Mayfair Job Reference (optional)	16822113
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7 350 s Sep 27 2012 MiTek Industries, Inc. Wed Jun 05 12:18:26 2013 Page 1
ID:9B5QRtZPhUL0yMYqzVn3hhzz67b-X?b9rV3sKe2xdNQz4s7fKpfx9elm6Sz1CSuixyz9KsR

Builders FirstSource, Lake City, FL 32055

Scale = 1/24"

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.56	Vert(LL)	0.08	6-7	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.45	Vert(TL)	-0.10	6-7	>999	180		
BCLL 0.0 *	Rep Stress Incr	NO	WB 0.33	Horz(TL)	0.01	5	n/a	n/a		
BCDL 5.0	Code FBC2010/TP12007		(Matrix-M)						Weight: 44 lb	FT = 20%

Plate Offsets (X,Y): [2-0-0.7, Edge]

LUMBER
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3

BRACING
 TOP CHORD
 BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.
 Rigid ceiling directly applied or 8-3-13 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 4=120/Mechanical, 2=370/0-4-15 (min. 0-1-8), 5=194/Mechanical
 Max Horz 2=186(LC 4)
 Max Uplift 4=-105(LC 4), 2=-285(LC 4), 5=-194(LC 4)
 Max Grav 4=147(LC 2), 2=454(LC 2), 5=225(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-11=-623/380, 11-12=-571/398, 3-12=-571/387
 BOT CHORD 2-14=-463/571, 14-15=-463/571, 7-15=-463/571, 7-16=-463/571, 6-16=-463/571
 WEBS 3-6=-605/491

NOTES (9-11)
 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; End.; GCpi=0.18; MWFRS (envelope) gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 105 lb uplift at joint 4, 285 lb uplift at joint 2 and 194 lb uplift at joint 5.
 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 27 lb down and 49 lb up at 1-5-12, 27 lb down and 49 lb up at 1-5-12, 3 lb down and 41 lb up at 4-3-11, 3 lb down and 41 lb up at 4-3-11, and 43 lb down and 73 lb up at 7-1-10, and 43 lb down and 73 lb up at 7-1-10 on top chord, and 15 lb down and 28 lb up at 1-5-12, 15 lb down and 28 lb up at 1-5-12, 3 lb down and 25 lb up at 4-3-11, 3 lb down and 25 lb up at 4-3-11, and 26 lb down and 36 lb up at 7-1-10, and 26 lb down and 36 lb up at 7-1-10 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 10) Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB.
 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-4=-44, 5-8=-10

Continued on page 2



June 5, 2013

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719.

Julius Lee
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

A circular professional engineer seal for Julius S.K. Lee, State of Florida, License No. 34869. The seal features the text "JULIUS S.K. LEE" at the top, "LICENSE" below it, "No. 34869" in the center, and "STATE OF FLORIDA" below that. The outer ring of the seal reads "PROFESSIONAL ENGINEER". The seal is surrounded by a decorative border of small stars.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 497392	Truss T02	Truss Type HIP	Qty 1	Ply 1	GIEBEIG - Lot 44 Mayfair Job Reference (optional)	16822115																																				
Builders FirstSource, Lake City, FL 32055		7.350 s Sep 27 2012 MiTek Industries, Inc. Wed Jun 05 12:18:29 2013 Page 1																																								
ID.9B5QRtZPhUL0yMYqzVn3hhzz6?b-yaGHUW6idZQWUq9Yl_YMyRHxHsnrJsbTvQ6MYH29KsO																																										
Scale = 1/32" = 1'-0"																																										
<table border="1" style="width:100%; border-collapse: collapse;"> <tr> <td style="width:25%;">LOADING (psf)</td> <td style="width:25%;">SPACING</td> <td style="width:10%;">CSI</td> <td style="width:10%;">DEFL</td> <td style="width:10%;">PLATES</td> <td style="width:10%;">GRIP</td> </tr> <tr> <td>TCLL 20.0</td> <td>2-0-0</td> <td>TC 0.25</td> <td>in (loc) l/defl L/d</td> <td>MT20</td> <td>244/190</td> </tr> <tr> <td>TCDL 7.0</td> <td>Plates Increase 1.25</td> <td>BC 0.49</td> <td>Vert(LL) -0.11 9-14 >999 240</td> <td></td> <td></td> </tr> <tr> <td>BCLL 0.0 *</td> <td>Lumber Increase 1.25</td> <td>WB 0.14</td> <td>Vert(TL) -0.21 9-14 >999 180</td> <td></td> <td></td> </tr> <tr> <td>BCDL 5.0</td> <td>Rep Stress Incr YES</td> <td>(Matrix-M)</td> <td>Horz(TL) 0.03 7 n/a n/a</td> <td></td> <td></td> </tr> <tr> <td></td> <td>Code FBC2010/TP12007</td> <td></td> <td></td> <td>Weight: 103 lb</td> <td>FT = 20%</td> </tr> </table>							LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP	TCLL 20.0	2-0-0	TC 0.25	in (loc) l/defl L/d	MT20	244/190	TCDL 7.0	Plates Increase 1.25	BC 0.49	Vert(LL) -0.11 9-14 >999 240			BCLL 0.0 *	Lumber Increase 1.25	WB 0.14	Vert(TL) -0.21 9-14 >999 180			BCDL 5.0	Rep Stress Incr YES	(Matrix-M)	Horz(TL) 0.03 7 n/a n/a				Code FBC2010/TP12007			Weight: 103 lb	FT = 20%
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<table border="1" style="width:100%; border-collapse: collapse;"> <tr> <td style="width:50%;"> LUMBER TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 </td> <td style="width:50%;"> BRACING TOP CHORD BOT CHORD Structural wood sheathing directly applied or 5-8-1 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing. MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. </td> </tr> </table>							LUMBER TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3	BRACING TOP CHORD BOT CHORD Structural wood sheathing directly applied or 5-8-1 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing. MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.																																		
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REACTIONS (lb/size) 7=628/0-3-8 (min. 0-1-8), 2=628/0-3-8 (min. 0-1-8) Max Horz 2=-59(LC 10) Max Uplift 7=-117(LC 13), 2=-117(LC 12) Max Grav 7=748(LC 2), 2=748(LC 2)																																										
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1066/497, 3-4=-815/384, 4-5=-703/381, 5-6=-814/383, 6-7=-1066/496 BOT CHORD 2-11=-339/910, 10-11=-151/668, 9-10=-151/668, 7-9=-345/912 WEBS 3-11=-291/221, 6-9=-292/221																																										
NOTES (9-11) 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Provide adequate drainage to prevent water ponding. 4) This truss has been designed for a live load of 10.0 psf bottom chord live load nonconcurrent with any other live loads. 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi. 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 117 lb uplift at joint 7 and 117 lb uplift at joint 2. 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss. 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code. 10) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package) 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435																																										
LOAD CASE(S) Standard																																										



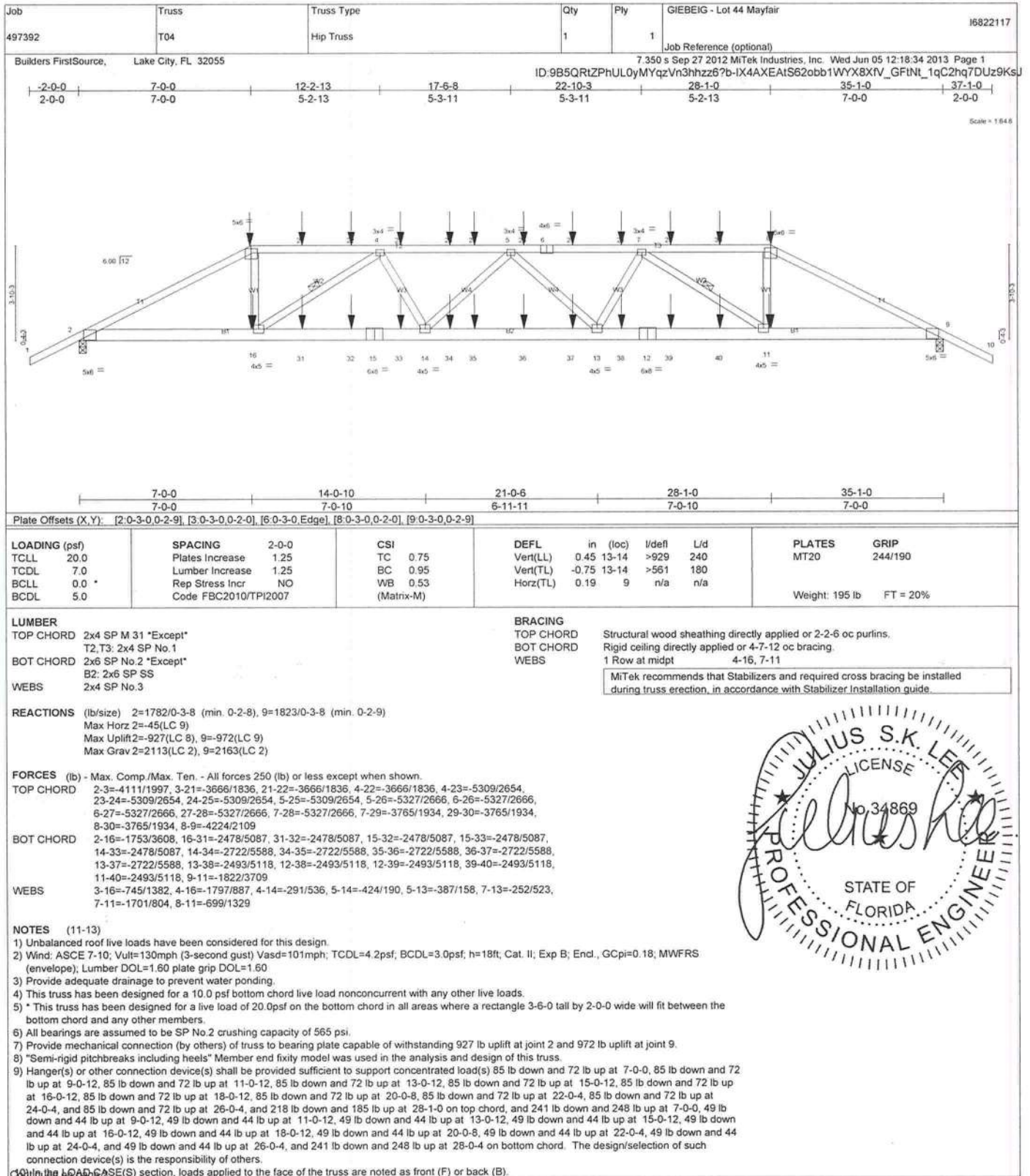
June 5, 2013



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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435



June 5, 2013



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 497392	Truss T05	Truss Type Hip Truss	Qty 1	Ply 1	GIEBEIG - Lot 44 Mayfair Job Reference (optional) 7.350 s Sep 27 2012 MiTek Industries, Inc. Wed Jun 05 12:18:36 2013 Page 1 ID:9B5QRtZPhUL0yMYqzVn3hhzz67b-FwBxywB8_jiWqvBugyA?kw3hBg8ASwaVW?JDINz9Ksh	16822118
Builders FirstSource, Lake City, FL 32055						

-2-0-0 4-9-5 9-0-0 14-7-9 20-5-7 26-1-0 30-3-11 35-1-0 37-1-0
2-0-0 4-9-5 4-2-11 5-7-9 5-9-14 5-7-9 4-2-11 4-9-5 2-0-0

Scale = 1/64" = 1'-0"

Plate Offsets (X,Y): [2-0-2-10,0-1-8], [4-0-3-0,0-2-0], [8-0-3-0,0-2-0], [10-0-2-10,0-1-8]	
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LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 5.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2010/TPI2007	CSI TC 0.34 BC 0.64 WB 0.64 (Matrix-M)	DEFL in (loc) l/defl L/d Vert(LL) -0.18 14 >999 240 Vert(TL) -0.38 14-16 >999 180 Horz(TL) 0.13 10 n/a n/a	PLATES GRIP MT20 244/190 Weight: 177 lb FT = 20%
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LUMBER TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3	BRACING TOP CHORD BOT CHORD <div style="border: 1px solid black; padding: 5px; font-size: x-small;"> Structural wood sheathing directly applied or 3-10-4 oc purlins. Rigid ceiling directly applied or 6-11-5 oc bracing. MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. </div>
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REACTIONS (lb/size) 2=1035/0-3-8 (min. 0-1-8), 10=1035/0-3-8 (min. 0-1-8)
 Max Horz 2=-59(LC 10)
 Max Uplift 2=-147(LC 9), 10=-147(LC 8)
 Max Grav 2=1231(LC 2), 10=1231(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-2080/897, 3-4=-1852/794, 4-5=-1643/755, 5-6=-2103/934, 6-7=-2103/934, 7-8=-1643/754, 8-9=-1852/794, 9-10=-2080/896
 BOT CHORD 2-16=-691/1806, 15-16=-719/2069, 14-15=-719/2069, 13-14=-720/2069, 12-13=-720/2069, 10-12=-698/1806
 WEBS 4-16=-200/552, 5-16=-649/256, 7-12=-649/256, 8-12=-200/552

NOTES (9-11)
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B, Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate gnp DOL=1.60
 3) Provide adequate drainage to prevent water ponding.
 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 147 lb uplift at joint 2 and 147 lb uplift at joint 10.
 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 10) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



June 5, 2013

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Julius Lee
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 497392	Truss T07	Truss Type Hip Truss	Qty 1	Ply 1	GIEBEIG - Lot 44 Mayfair Job Reference (optional) 7 350 s Sep 27 2012 MiTek Industries, Inc. Wed Jun 05 12:18:40 2013 Page 1 ID: 9B5QRIZPhUL0yMYqzVn3hhzz6?b-7hRRnHEe1ypyJWUgvoFxmEMclUUVOMy5RdHRR8z9KsD	16822120
Builders FirstSource, Lake City, FL 32055						

Plate Offsets (X,Y): [1-0-2-10,0-1-8], [3-0-3-0,0-2-0], [5-0-3-0,0-2-0], [8-0-2-10,0-1-8]				
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 5.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2010/TP12007	CSI TC 0.44 BC 0.68 WB 0.45 (Matrix-M)	DEFL in (loc) l/defl L/d Vert(LL) -0.25 12-13 >999 240 Vert(TL) -0.45 12-13 >929 180 Horz(TL) 0.11 8 n/a n/a	PLATES MT20 GRIP 244/190 Weight: 184 lb FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

REACTIONS (lb/size) 1=997/0-3-8 (min. 0-1-8), 8=1087/0-3-8 (min. 0-1-8)
Max Horz 1=-86(LC 17)
Max Uplift 1=-145(LC 12), 8=-172(LC 13)
Max Grav 1=1122(LC 2), 8=1232(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-2150/867, 2-3=-1755/738, 3-4=-1534/713, 4-5=-1534/712, 5-6=-1688/737, 6-7=-1754/716, 7-8=-2145/863
BOT CHORD 1-15=-711/1856, 14-15=-653/1856, 13-14=-653/1856, 12-22=-443/1549, 22-23=-443/1549, 12-23=-443/1549, 11-12=-648/1851, 10-11=-648/1851, 8-10=-648/1851
WEBS 2-13=-452/294, 3-13=-159/483, 4-13=-275/96, 4-12=-276/96, 5-12=-158/483, 7-12=-447/289

NOTES (9-11)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; End.; GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 145 lb uplift at joint 1 and 172 lb uplift at joint 8.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB.
11) Truss Design Engineer: Julius Lee, PE, Florida P.E. License No. 34869, Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-3-12 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.



June 5,2013

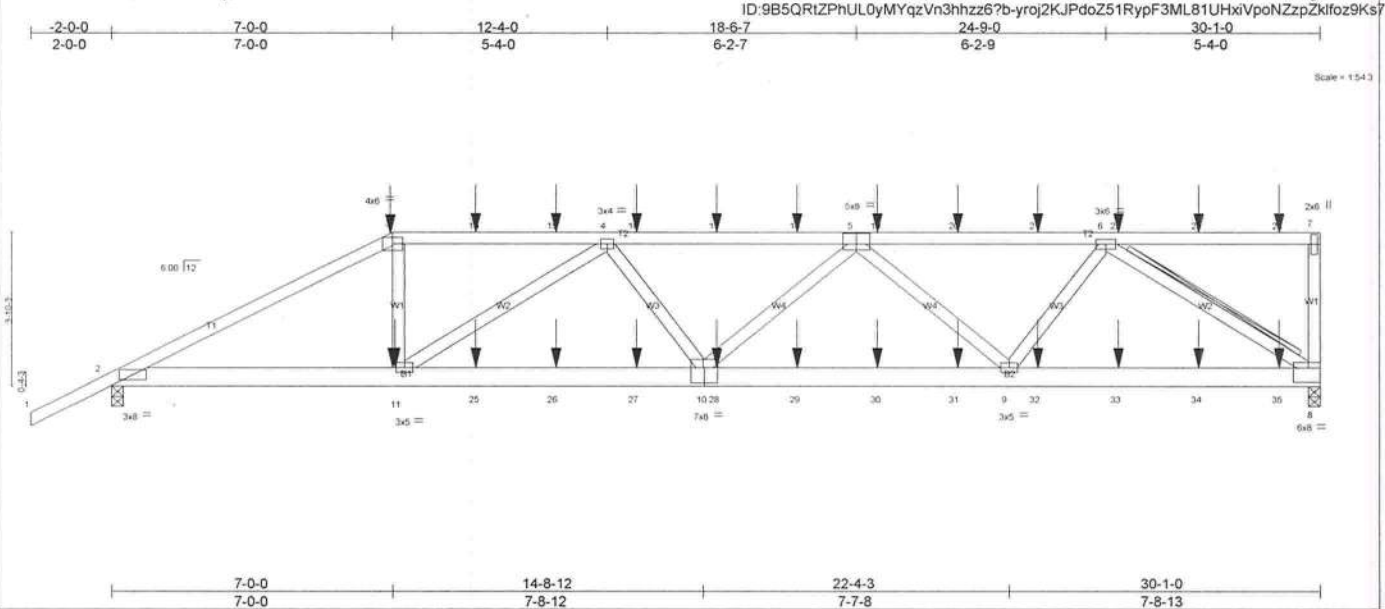
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
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Builders FirstSource, Lake City, FL 32055 7.350 s Sep 27 2012 MiTek Industries, Inc. Wed Jun 05 12:18:46 2013 Page 1

Builders FirstSource, Lake City, FL 32055

7.350 s Sep 27 2012 MiTek Industries, Inc. Wed Jun 05 12:18:46 2013 Page 1



Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in

REACTIONS (lb/size) 8=1517/0-3-8 (min. 0-2-2), 2=1507/0-3-8 (min. 0-2-2)

Continued on page 2

Continued on page 2



June 5, 2013

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BC511 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

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Job 497392	Truss T10	Truss Type MONO HIP	Qty 1	Ply 1	GIEBEIG - Lot 44 Mayfair	16822123
Builders FirstSource, Lake City, FL 32055		7:350 s Sep 27 2012 MiTek Industries, Inc. Wed Jun 05 12:18:48 2013 Page 1 ID:9B5QRIZPhUL0yMYqzVn3hhzz6?b-uEwTT0Kf9PppGi6CNUOpDSZjiWAGGNdGGIdsgz9Ks5				
<div style="display: flex; justify-content: space-between;"> <div style="width: 45%;"> <p> -2-0-0 4-9-5 9-0-0 15-11-9 2-0-0 4-9-5 4-2-11 6-11-9 </p> </div> <div style="width: 50%;"> <p> 23-1-7 30-1-0 7-1-14 6-11-9 </p> </div> </div> <p style="text-align: right;">Scale = 1/4" = 1'-0"</p>						
Plate Offsets (X, Y): [2-0-2-10, 0-1-8], [6-0-4-0, 0-3-0]						
LOADING (psf) TCCL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 5.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2010/TPI2007	CSI TC 0.49 BC 0.88 WB 0.51 (Matrix-M)	DEFL in (loc) l/defl L/d Vert(LL) -0.29 8-9 >999 240 Vert(TL) -0.53 8-9 >674 180 Horz(TL) 0.08 8 n/a n/a	PLATES MT20 GRIP 244/190 Weight: 155 lb FT = 20%		
LUMBER TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3		BRACING TOP CHORD BOT CHORD WEBS Structural wood sheathing directly applied or 4-3-6 oc purlins, except end verticals. Rigid ceiling directly applied or 6-8-6 oc bracing. 1 Row at midpt 5-11, 6-8 <div style="border: 1px solid black; padding: 2px; margin-top: 5px;"> MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. </div>				
REACTIONS (lb/size) 8=802/0-3-8 (min. 0-1-8), 2=903/0-3-8 (min. 0-1-8) Max Horz 2=143(LC 12) Max Uplift 8=183(LC 9), 2=134(LC 9) Max Grav 8=950(LC 2), 2=1074(LC 2)						
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1753/721, 3-4=-1523/620, 4-5=-1337/597, 5-6=-1411/571 BOT CHORD 2-11=-772/1546, 10-11=-697/1587, 9-10=-697/1587, 8-9=-501/1153 WEBS 4-11=-101/387, 5-11=-407/126, 5-9=-289/205, 6-9=-115/465, 6-8=-1349/593						
NOTES (9-11) 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; End., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Provide adequate drainage to prevent water ponding. 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi. 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 183 lb uplift at joint 8 and 134 lb uplift at joint 2. 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss. 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code. 10) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package) 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435						
LOAD CASE(S) Standard						



June 5, 2013

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Job 497392	Truss T14	Truss Type SPECIAL	Qty 2	Ply 1	GIEBEIG - Lot 44 Mayfair Job Reference (optional) 7350 s Sep 27 2012 MiTek Industries, Inc. Wed Jun 05 12:18:54 2013 Page 1 ID:9B5QRtZPhUL0yMYqzVn3hhzz67b-jOHkk3PQkFZz_gZMjkVDTjpkIxFPg2z8fpgAxKz9Ks?	I6822127
Builders FirstSource, Lake City, FL 32055						

Scale = 1/62.1

Plate Offsets (X,Y): [4-0-3-8, 0-3-0], [16-0-2-0, 0-0-0]				
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 5.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2010/TP12007	CSI TC 0.49 BC 0.76 WB 0.64 (Matrix-M)	DEFL in (loc) l/defl L/d Vert(LL) 0.25 14-15 >999 240 Vert(TL) -0.50 14-15 >723 180 Horz(TL) 0.27 9 n/a n/a	PLATES GRIP MT20 244/190 Weight: 176 lb FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2 *Except*

B4: 2x4 SP No.3

WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-11-0 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 5-4-5 oc bracing. Except: 9-5-0 oc bracing: 10-12

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=943/0-3-8 (min 0-1-8), 9=859/0-3-8 (min 0-1-8)

Max Horz 2=130(LC 12)

Max Uplift 2=171(LC 12), 9=127(LC 13)

Max Grav 2=1074(LC 2), 9=957(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-3339/1354, 3-4=-3323/1460, 4-5=-1698/752, 5-6=-1707/778, 6-7=-1698/728

BOT CHORD 2-15=-1224/3011, 14-15=-733/1907, 14-20=-327/1114, 20-21=-327/1114, 13-21=-327/1114, 12-13=-545/1497, 10-12=-341/881, 9-10=-395/987

WEBS 4-15=-635/1403, 4-14=-739/429, 5-14=-290/739, 5-13=-248/556, 6-13=-284/227, 7-12=-585/1519, 7-10=-1171/520, 7-9=-1275/519

NOTES (9-11)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; End., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for bottom chord and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 171 lb uplift at joint 2 and 127 lb uplift at joint 9.
- 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TP1 1 as referenced by the building code.
- 10) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
- 11) Truss Design Engineer: Julius Lee, PE; Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



June 5, 2013

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Boynton, FL 33435

Job 497392	Truss T15A	Truss Type SPECIAL	Qty 1	Ply 1	GIEBEIG - Lot 44 Mayfair Job Reference (optional) 7350 s Sep 27 2012 MiTek Industries, Inc. Wed Jun 05 12:18:57 2013 Page 1 ID: 9B5QRIZPhUL0yMYqzVn3hhzz67b-7zztM5RJ1AyXr8ixOt3w4LRFc8HRIS_bLnurVWz9Kry	I6822129
Builders FirstSource, Lake City, FL 32055						

Plate Offsets (X,Y): [4:0-3-12,0-3-0], [6:0-3-0,0-3-4], [7:0-2-10,0-1-8]	
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LOADING (psf) TCLL 20.0 TCCL 7.0 BCLL 0.0 * BCDL 5.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2010/TP12007	CSI TC 0.48 BC 0.68 WB 0.50 (Matrix-M)	DEFL in (loc) l/defl L/d Vert(LL) 0.24 11-12 >999 240 Vert(TL) -0.49 11-12 >738 180 Horz(TL) 0.20 7 n/a n/a	PLATES GRIP MT20 244/190 Weight: 147 lb FT = 20%
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LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS (lb/size) 2=936/0-3-8 (min. 0-1-8), 7=955/0-3-8 (min. 0-1-8)

Max Horz 2=116(LC 12)

Max Uplift 2=-173(LC 12), 7=-159(LC 13)

Max Grav 2=1066(LC 2), 7=1075(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-3300/1288, 3-4=-3286/1396, 4-5=-1679/735, 5-6=-1654/740, 6-7=-1821/748

BOT CHORD 2-12=-1114/2978, 11-12=-667/1888, 11-19=-282/1115, 10-19=-282/1115, 10-20=-282/1115, 9-20=-282/1115, 7-9=-544/1560

WEBS 4-12=-593/1390, 4-11=-713/404, 5-11=-276/726, 5-9=-209/476, 6-9=-338/270

NOTES (9-11)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; End.; GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate gnp DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 173 lb uplift at joint 2 and 159 lb uplift at joint 7.
- 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TP1 1 as referenced by the Building code.
- 10) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
- 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

June 5,2013



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 497392	Truss T17	Truss Type SPECIAL	Qty 1	Ply 1	GIEBEIG - Lot 44 Mayfair Job Reference (optional)	I6822131
Builders FirstSource, Lake City, FL 32055		7.350 s Sep 27 2012 MiTek Industries, Inc. Wed Jun 05 12:19:00 2013 Page 1 ID:9B5QRizPhUL0yMYqzVn3hhzz67b-YYe?_7UBK5K6ib0W4?cdl_3mCLi4nn11k7V7_z9Krv				

Plate Offsets (X,Y): [3-0-3-0,0-3-0], [8-0-2-9,0-1-8]									
LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL 20.0	2-0-0	TC 0.46	Vert(LL)	-0.28	12-13	>999	240	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.70	Vert(TL)	-0.61	12-13	>588	180		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.56	Horz(TL)	0.21	8	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix-M)							
	Code FBC2010/TPI2007								
								Weight: 154 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS (lb/size) 1=807/0-3-8 (min. 0-1-8), 8=906/0-3-8 (min. 0-1-8)

Max Horz 1=88(LC 12)

Max Uplift 1=142(LC 12), 8=168(LC 13)

Max Grav 1=956(LC 2), 8=1077(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-3199/1341, 2-3=-3200/1455, 3-4=-1418/665, 4-5=-1379/676, 5-6=-1335/654, 6-7=-1510/684, 7-8=-1743/785

BOT CHORD 1-13=-1175/2894, 12-13=-699/1813, 11-12=-494/1453, 10-11=-494/1453, 8-10=-596/1506

WEBS 3-13=-628/1401, 3-12=-774/430, 4-12=-447/985, 5-12=-538/290, 5-10=-311/144, 6-10=-178/449, 7-10=-262/208

NOTES (10-12)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf, BCDL=3.0psf, h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 142 lb uplift at joint 1 and 168 lb uplift at joint 8.
- Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

June 5,2013

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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 497392	Truss T19	Truss Type COMMON	Qty 2	Ply 1	GIEBEIG - Lot 44 Mayfair Job Reference (optional) ID:9B5QRiZPhUL0yMYqzVn3hhzz6?b-UwmmPpVSsiaqvAuBQe5nP8Ay98TXpGKU2ccCtZ9Krt	I6822133
Builders FirstSource, Lake City, FL 32055		7 350 s Sep 27 2012 MiTek Industries, Inc. Wed Jun 05 12:19:02 2013 Page 1				

Plate Offsets (X,Y): [2-0-6-0,0-0-14], [4-0-6-0,0-0-14]				
LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.25	Vert(LL) 0.02 5-8 >999 240	MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.11	Vert(TL) 0.02 5-8 >999 180	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.05	Horz(TL) -0.00 4 n/a n/a	
BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)		Weight: 33 lb FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS (lb/size) 4=221/0-3-8 (min. 0-1-8), 2=317/0-3-8 (min. 0-1-8)

Max Horz 2=68(LC 16)

Max Uplift 4=100(LC 8), 2=135(LC 12)

Max Grav 4=262(LC 2), 2=379(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=312/406, 3-4=325/476

BOT CHORD 2-5=321/247, 4-5=589/407

NOTES (8-10)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCDL=3.0psf; h=18ft, Cat. II; Exp B; Encl. GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint 4 and 135 lb uplift at joint 2.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.



June 5,2013



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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 497392	Truss T21	Truss Type COMMON	Qty 2	Ply 1	GIEBEIG - Lot 44 Mayfair Job Reference (optional) 7 350 s Sep 27 2012 MiTek Industries, Inc. Wed Jun 05 12:19:04 2013 Page 1 ID: 9B5QRtZPhUL0yMYqzVn3hhzz6?b-QJuWqUXiOKqYBCKHJrhatqDTtzm_?iBdyM5iGiz9Krr	I6822135
Builders FirstSource, Lake City, FL 32055						

Plate Offsets (X,Y): [2 0-2-10,0-1-8], [4 0-2-10,0-1-8]						
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.42	Vert(LL) 0.11	5-8 >999 240	MT20 244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.30	Vert(TL) 0.09	5-8 >999 180	
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.08	Horz(TL) -0.01	4 n/a n/a	
BCDL 5.0	Code FBC2010/TPI2007		(Matrix-M)			Weight: 53 lb FT = 20%

LUMBER TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 REACTIONS (lb/size) 4=376/0-3-8 (min. 0-1-8), 2=468/0-3-8 (min. 0-1-8) Max Horz 2=56(LC 12) Max Uplift 4=144(LC 8), 2=156(LC 9) Max Grav 4=446(LC 2), 2=558(LC 2) FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-615/917, 3-4=-797/1081 BOT CHORD 2-5=-1195/849, 4-5=-1424/1104 WEBS 3-5=-322/210 NOTES (8-10) 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-6" tall by 2'-0" wide will fit between the bottom chord and any other members. 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi. 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 144 lb uplift at joint 4 and 156 lb uplift at joint 2. 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss. 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code. 9) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package) 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435	BRACING TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6'-0" oc purlins. Rigid ceiling directly applied or 8'-2-15 oc bracing. MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.
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LOAD CASE(S) Standard

June 5, 2013



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

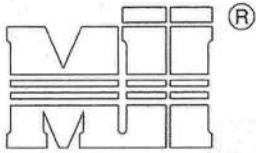
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D5B-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

August 10, 2010

T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

ST - T-BRACE 2



MiTek Industries, Inc.

MiTek Industries, Chesterfield, MO Page 1 of 1

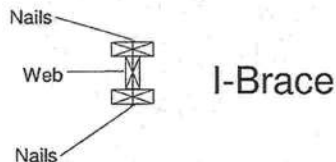
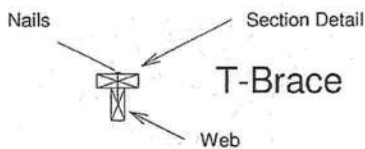
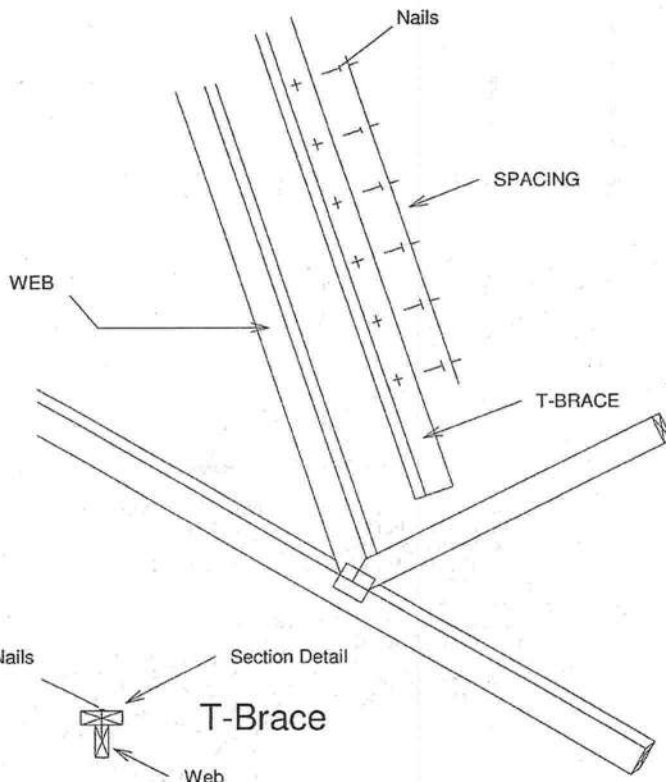
Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

Nailing Pattern		
T-Brace size	Nail Size	Nail Spacing
2x4 or 2x6 or 2x8	10d	6" o.c.
Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)		

Brace Size for One-Ply Truss		
Specified Continuous Rows of Lateral Bracing		
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

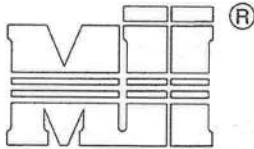
Brace Size for Two-Ply Truss		
Specified Continuous Rows of Lateral Bracing		
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace



T-Brace / I-Brace must be same species and grade (or better) as web member.



1109 COASTAL BAY
BOYNTON BC, FL 33435



MiTek Industries, Inc.

NOTES:

1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.)
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)

	DIAM.	SYP	DF	HF	SPF	SPF-S
3.5" LONG	.131	88.0	80.6	69.9	68.4	59.7
	.135	93.5	85.6	74.2	72.6	63.4
	.162	108.8	99.6	86.4	84.5	73.8
3.25" LONG	.128	74.2	67.9	58.9	57.6	50.3
	.131	75.9	69.5	60.3	59.0	51.1
	.148	81.4	74.5	64.6	63.2	52.5

VALUES SHOWN ARE CAPACITY PER TOE-NAIL.
APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

EXAMPLE:

(3) - 16d NAILS (.162" diam. x 3.5") WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1.15:

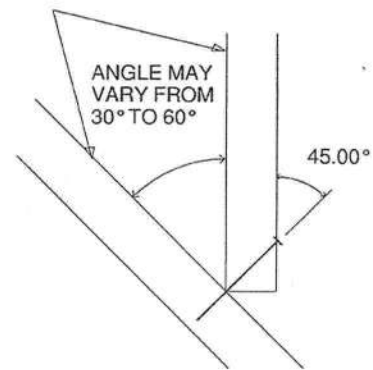
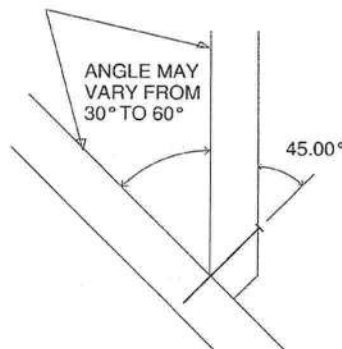
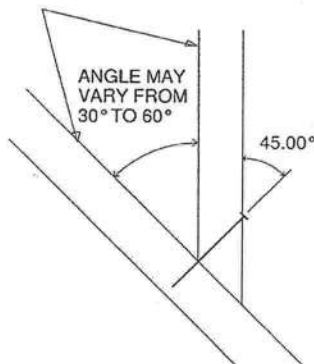
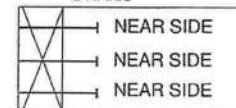
3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity

THIS DETAIL APPLICABLE TO THE
THREE END DETAILS SHOWN BELOW

VIEWS SHOWN ARE FOR
ILLUSTRATION PURPOSES ONLY

SIDE VIEW

3 NAILS

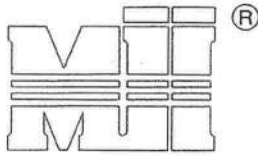


1109 COASTAL BAY
BOYNTON BC, FL 33435

FEBRUARY 14, 2012

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

ST-PIGGY-7-10



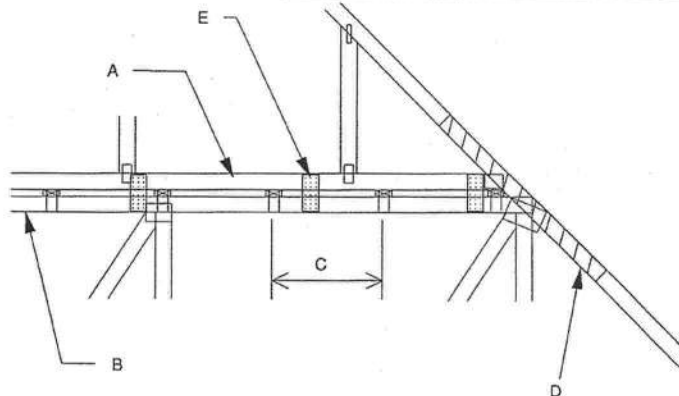
MiTek Industries, Inc.

MiTek Industries, Chesterfield, MO

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E
MAX MEAN ROOF HEIGHT = 30 FEET
MAX TRUSS SPACING = 24" O.C.
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-10
DURATION OF LOAD INCREASE : 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES
TRANSFERING DRAG LOADS (SHEAR TRUSSES).
ADDITIONAL CONSIDERATIONS BY BUILDING
ENGINEER/DESIGNER ARE REQUIRED.

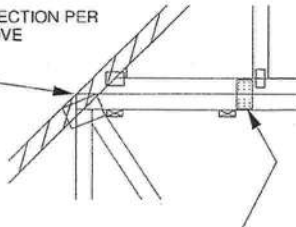
- A - PIGGYBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING. SHALL BE CONNECTED TO EACH PURLIN WITH (2) 0.131" X 3.5" TOE NAILED.
- B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
- C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) 0.131" X 3.5" NAILS EACH.
- D - 2 X 4-0" SCAB, SIZE AND GRADE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF 0.131" X 3" NAILS @ 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:
 - 1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
 - 2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.
- E - FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C. W/ (4) 0.131" X 1.5" PER MEMBER. STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5" EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)



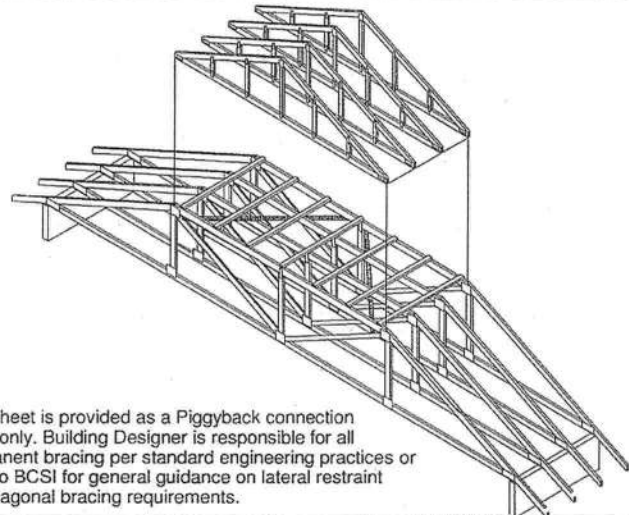
WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.

SCAB CONNECTION PER
NOTE D ABOVE

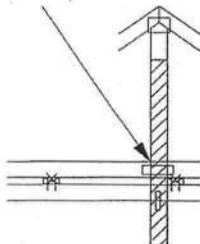


FOR ALL WIND SPEEDS, ATTACH MITEK 3X6 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) 0.131" X 1.5" PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE.



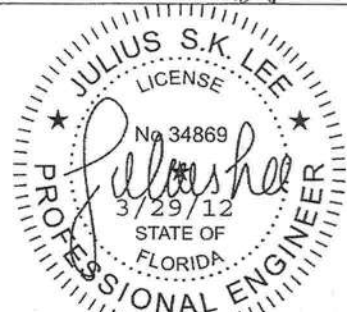
This sheet is provided as a Piggyback connection detail only. Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements.

VERTICAL WEB TO
EXTEND THROUGH
BOTTOM CHORD
OF PIGGYBACK

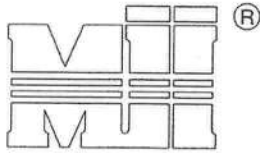


FOR LARGE CONCENTRATED LOADS APPLIED
TO CAP TRUSS REQUIRING A VERTICAL WEB:

- 1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.
- 2) ATTACH 2 x 4-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
- 3) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
- 4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
- 5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN.



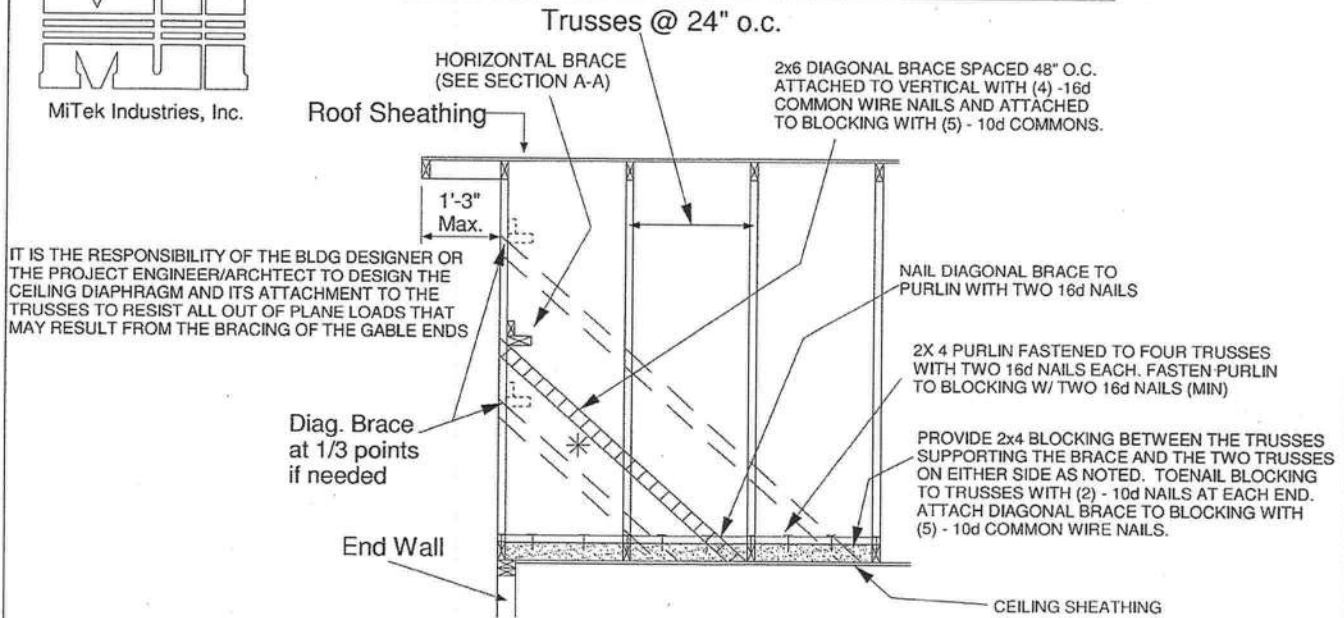
1109 COASTAL BAY
BOYNTON BC, FL 33435



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ALTERNATE DIAGONAL BRACING TO THE BOTTOM CHORD



BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

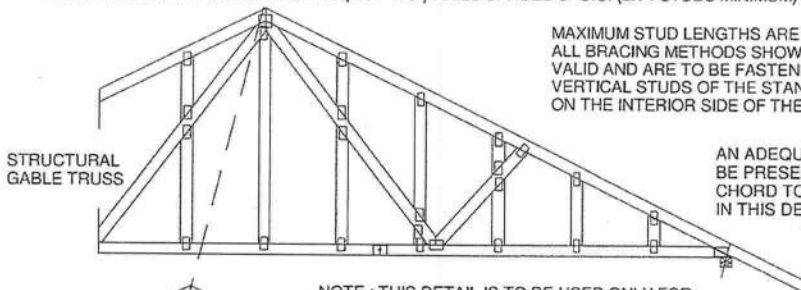
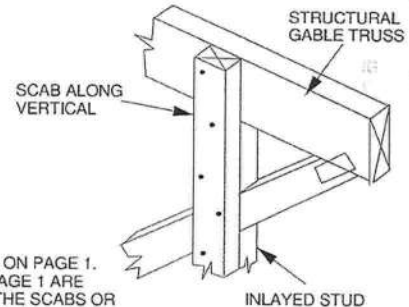
STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED:

METHOD 1 : ATTACH A MATCHING GABLE TRUSS TO THE INSIDE FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE FOLLOWING NAILING SCHEDULE.

METHOD 2 : ATTACH 2X SCABS TO THE FACE OF EACH VERTICAL MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE AND SPECIES AS THE TRUSS VERTICALS

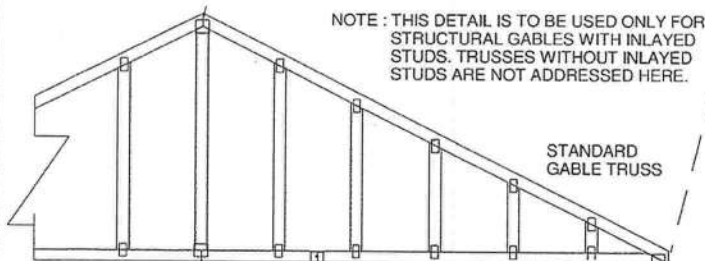
NAILING SCHEDULE:

- FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10) OR LESS, NAIL ALL MEMBERS WITH ONE ROW OF 10d (.131" X 3") NAILS SPACED 6" O.C.
- FOR WIND SPEEDS GREATER 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10) NAIL ALL MEMBERS WITH TWO ROWS OF 10d (.131" X 3") NAILS SPACED 6" O.C. (2X 4 STUDS MINIMUM)

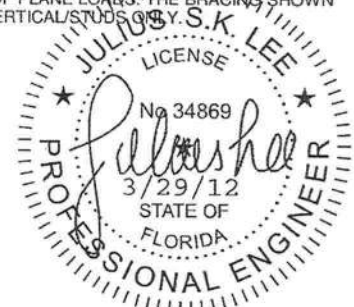


MAXIMUM STUD LENGTHS ARE LISTED ON PAGE 1. ALL BRACING METHODS SHOWN ON PAGE 1 ARE VALID AND ARE TO BE FASTENED TO THE SCABS OR VERTICAL STUDS OF THE STANDARD GABLE TRUSS ON THE INTERIOR SIDE OF THE STRUCTURE.

AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL STUDS ONLY.



NOTE : THIS DETAIL IS TO BE USED ONLY FOR STRUCTURAL GABLES WITH INLAVED STUDS. TRUSSES WITHOUT INLAVED STUDS ARE NOT ADDRESSED HERE.



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