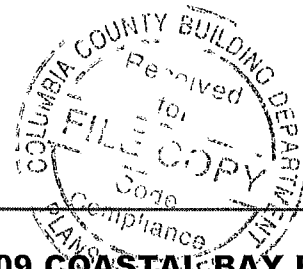


**JULIUS LEE PE.**



RE 509593 - WOODMAN PARK - ROPER RES

**1109 COASTAL BAY BLVD,  
BOYNTON BEACH, FL 33435**

**Site Information:**

Project Customer WOODMAN PARK Project Name 509593 Model Roper Res  
Lot/Block Subdivision  
Address 453 SW Long Leaf Dr  
City Columbia Cty State FL

**Name Address and License # of Structural Engineer of Record, If there is one, for the building.**

Name MARK E HADDOX License # CRC1329442  
Address 4816 W US HWY 90 STE 100  
City LAKE CITY, State FL

**General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):**

Design Code FBC2010/TPI2007 Design Program: MiTek 20/20 7 3  
Wind Code ASCE 7-10 Wind Speed 130 mph Floor Load: N/A psf  
Roof Load 32.0 psf

This package includes 48 individual, dated Truss Design Drawings and 0 Additional Drawings  
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet  
conforms to 61G15-31 003, section 5 of the Florida Board of Professional Engineers Rules  
This document processed per section 16G15-23 003 of the Florida Board of Professionals Rules

**In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany this coversheet. The latest approval dates supersede and replace the previous drawings.**

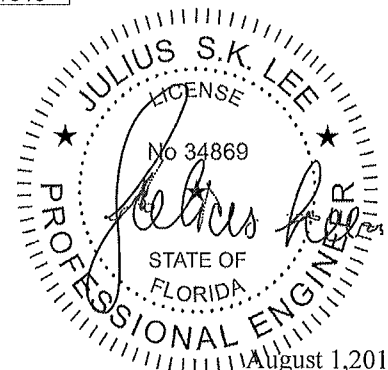
No	Seal#	Truss Name	Date	No	Seal#	Truss Name	Date
1	I7071111	CJ01	8/1/013	18	I7071128	PB06	8/1/013
2	I7071112	CJ02	8/1/013	19	I7071129	T01G	8/1/013
3	I7071113	CJ03	8/1/013	20	I7071130	T02	8/1/013
4	I7071114	CJ04	8/1/013	21	I7071131	T02G	8/1/013
5	I7071115	EJ01	8/1/013	22	I7071132	T03	8/1/013
6	I7071116	EJ01T	8/1/013	23	I7071133	T04	8/1/013
7	I7071117	EJ02	8/1/013	24	I7071134	T05	8/1/013
8	I7071118	EJ03	8/1/013	25	I7071135	T06	8/1/013
9	I7071119	EJ04	8/1/013	26	I7071136	T07	8/1/013
10	I7071120	EJ05	8/1/013	27	I7071137	T08	8/1/013
11	I7071121	EJ06	8/1/013	28	I7071138	T09	8/1/013
12	I7071122	HJ01	8/1/013	29	I7071139	T10	8/1/013
13	I7071123	PB01	8/1/013	30	I7071140	T11	8/1/013
14	I7071124	PB02	8/1/013	31	I7071141	T12	8/1/013
15	I7071125	PB03	8/1/013	32	I7071142	T13	8/1/013
16	I7071126	PB04	8/1/013	33	I7071143	T14	8/1/013
17	I7071127	PB05	8/1/013	34	I7071144	T15	8/1/013

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Jax).

Truss Design Engineer's Name Julius Lee

My license renewal date for the state of Florida is February 28, 2015.

**NOTE:** The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.



Job 509593	Truss CJ01	Truss Type Jack-Open Truss	Qty 2	Ply 1	WOODMAN PARK ROPER RES.	17071111
Builders FirstSource Lake City FL 32055		Job Reference (optional) ID 4ZXLndQASI7BhXBfnFLaYytlv_S-P17uyDAIRx0Tvp?LpABK5E742aMsChTleS8oAyySbk0				

7 350 s Sep 27 2012 MiTek Industries Inc. Thu Aug 01 14:40:59 2013 Page 1

LOADING (psf) TCLL 20.0 TCOL 7.0 BCLL 0.0 BCOL 5.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2010/TPI2007	CSI TC 0.32 BC 0.02 WB 0.04 (Matrix)	DEFL in (loc) l/def L/d Vert(LL) 0.02 1 n/r 120 Vert(TL) -0.00 1 n/r 120 Horz(TL) -0.00 4 n/a n/a	PLATES GRIP MT20 244/190  Weight 11 lb FT = 20%
--	--	--	--	--

**LUMBER**  
 TOP CHORD 2x4 SP No.2  
 BOT CHORD 2x4 SP No.2  
 WEBS 2x4 SP No.3

**BRACING**  
 TOP CHORD Structural wood sheathing directly applied or 1 11 13 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

**REACTIONS** (lb/size) 4=77/1 11 13 (min. 0-1-8) 2=196/1 11 13 (min 0-1-8) 5=-9/1 11 13 (min. 0-1-8) 6=-68/1 11 13 (min 0-1-8)  
 Max Horz 2=92(LC 12)  
 Max Uplift 4=-72(LC 16) 2=-147(LC 12) 5=-27(LC 3) 6=-89(LC 2)  
 Max Grav 4=94(LC 2) 2=239(LC 2) 6=116(LC 16)

**FORCES** (lb) Max. Comp./Max Ten. All forces 250 (lb) or less except when shown

**NOTES** (10-12)  
 1) Wind ASCE 7 10 Vult=130mph (3-second gust) Vasd=101mph TCCL=4 2psf BCDL=3 0psf h=20ft, Cat. II Exp C, Encl GCpi=0 18 MWFRS (envelope) gable end zone and C-C Exterior(2) zone C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60  
 2) Truss designed for wind loads in the plane of the truss only For studs exposed to wind (normal to the face) see Standard Industry Gable End Details as applicable or consult qualified building designer as per ANSI/TPI 1  
 3) Gable requires continuous bottom chord bearing  
 4) Gable studs spaced at 2-0-0 oc.  
 5) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads  
 6) \* This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members  
 7) All bearings are assumed to be SP No 2 crushing capacity of 565 psi  
 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 72 lb uplift at joint 4 147 lb uplift at joint 2 27 lb uplift at joint 5 and 89 lb uplift at joint 6.  
 9) Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.  
 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code  
 11) Note: Visually graded lumber designation SPp represents new lumber design values as per SPIB  
 12) Truss Design Engineer: Julius Lee, PE, Florida P E License No. 34869 Address: 1109 Coastal Bay Blvd Boynton Beach FL 33435

**LOAD CASE(S)** Standard

August 1,2013

**WARNING** Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.  
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.  
 Applicability of design parameters and proper incorporation of component is responsibility of building designer not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive Madison, WI 53719

Julius Lee PE,  
 1109 Coastal Bay  
 Boynton Beach, FL 33435

Job 509593	Truss CJ03	Truss Type Jack-Open Truss	Qty 2	Ply 1	WOODMAN PARK ROPER RES  Job Reference (optional)	17071113																																													
Builders FirstSource Lake City FL 32055		7 350 s Sep 27 2012 MiTek Industries, Inc. Thu Aug 01 14:41:01 2013 Page 1																																																	
<p>ID 4ZXLIIndQASi7BhXBFnFLaYytv_S-LQFeNvByZYHB969kwbEoAfCLuO?Wgbc25mdvErysBKm</p> <p style="text-align: center;">-2-0-0      2-5-9 2-0-0      2-5-9</p> <p style="text-align: right;">Scale = 1/26.3</p>																																																			
<p>Plate Offsets (X,Y). [2-0-3-0-0-1 12]</p> <table border="1" style="width:100%; border-collapse: collapse;"> <tr> <td style="width:15%;">LOADING (psf)</td> <td style="width:15%;">SPACING</td> <td style="width:15%;">CSI</td> <td style="width:15%;">DEFL</td> <td style="width:15%;">in (loc)</td> <td style="width:15%;">l/defl</td> <td style="width:15%;">L/d</td> <td style="width:15%;">PLATES</td> <td style="width:15%;">GRIP</td> </tr> <tr> <td>TCLL 20.0</td> <td>Plates Increase 1 25</td> <td>TC 0.69</td> <td>Vert(LL) -0.01</td> <td>4-5</td> <td>&gt;999</td> <td>240</td> <td>MT20</td> <td>244/190</td> </tr> <tr> <td>TCDL 7.0</td> <td>Lumber Increase 1 25</td> <td>BC 0.20</td> <td>Vert(TL) -0.01</td> <td>4-5</td> <td>&gt;999</td> <td>180</td> <td></td> <td></td> </tr> <tr> <td>BCLL 0.0 *</td> <td>Rep Stress Incr YES</td> <td>WB 0.00</td> <td>Horz(TL) 0.03</td> <td>3</td> <td>n/a</td> <td>n/a</td> <td></td> <td></td> </tr> <tr> <td>BCDL 5.0</td> <td>Code FBC2010/TPI2007</td> <td>(Matrix-M)</td> <td></td> <td></td> <td></td> <td></td> <td>Weight 15 lb</td> <td>FT = 20%</td> </tr> </table>							LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP	TCLL 20.0	Plates Increase 1 25	TC 0.69	Vert(LL) -0.01	4-5	>999	240	MT20	244/190	TCDL 7.0	Lumber Increase 1 25	BC 0.20	Vert(TL) -0.01	4-5	>999	180			BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) 0.03	3	n/a	n/a			BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)					Weight 15 lb	FT = 20%
LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP																																											
TCLL 20.0	Plates Increase 1 25	TC 0.69	Vert(LL) -0.01	4-5	>999	240	MT20	244/190																																											
TCDL 7.0	Lumber Increase 1 25	BC 0.20	Vert(TL) -0.01	4-5	>999	180																																													
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BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)					Weight 15 lb	FT = 20%																																											
<p><b>LUMBER</b></p> <p>TOP CHORD 2x4 SP No.2</p> <p>BOT CHORD 2x4 SP No 2</p> <p>WEBS 2x4 SP No.3</p> <p><b>BRACING</b></p> <p>TOP CHORD Structural wood sheathing directly applied or 2-5-9 oc purlins except end verticals.</p> <p>BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing</p> <p>MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.</p>																																																			
<p><b>REACTIONS</b> (lb/size) 5=200/0-5-8 (min 0-1-8) 3=17/Mechanical 4=-1/Mechanical</p> <p>Max Horz 5=184(LC 12)</p> <p>Max Uplift 5=-37(LC 12) 3=-87(LC 12) 4=-29(LC 12)</p> <p>Max Grav 5=243(LC 2) 3=47(LC 21) 4=28(LC 3)</p>																																																			
<p><b>FORCES</b> (lb) Max Comp./Max Ten All forces 250 (lb) or less except when shown.</p>																																																			
<p><b>NOTES</b> (7 g)</p> <p>1) Wind ASCE 7 10; Vult=130mph (3-second gust) Vasd=101mph TCDL=4 2psf BCDL=3.0psf h=20ft, Cat. II Exp C Encl GCpi=0.18 MWFRS (envelope) gable end zone and C-C Exterior(2) zone end vertical left exposed C-C for members and forces &amp; MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60</p> <p>2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.</p> <p>3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.</p> <p>4) All bearings are assumed to be SP No 2 crushing capacity of 565 psi</p> <p>5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 lb uplift at joint 5 87 lb uplift at joint 3 and 29 lb uplift at joint 4.</p> <p>6) Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss.</p> <p>7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code</p> <p>8) Note Visually graded lumber designation SPP represents new lumber design values as per SPIB.</p> <p>9) Truss Design Engineer Julius Lee PE Florida P E License No. 34869 Address: 1109 Coastal Bay Blvd Boynton Beach FL 33435</p>																																																			
<p><b>LOAD CASE(S)</b> Standard</p>																																																			



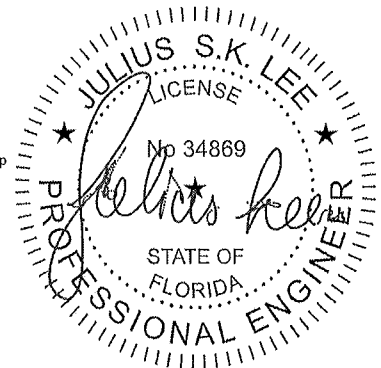
August 1,2013

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Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component! Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive Madison, WI 53719

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Boynton Beach, FL 33435

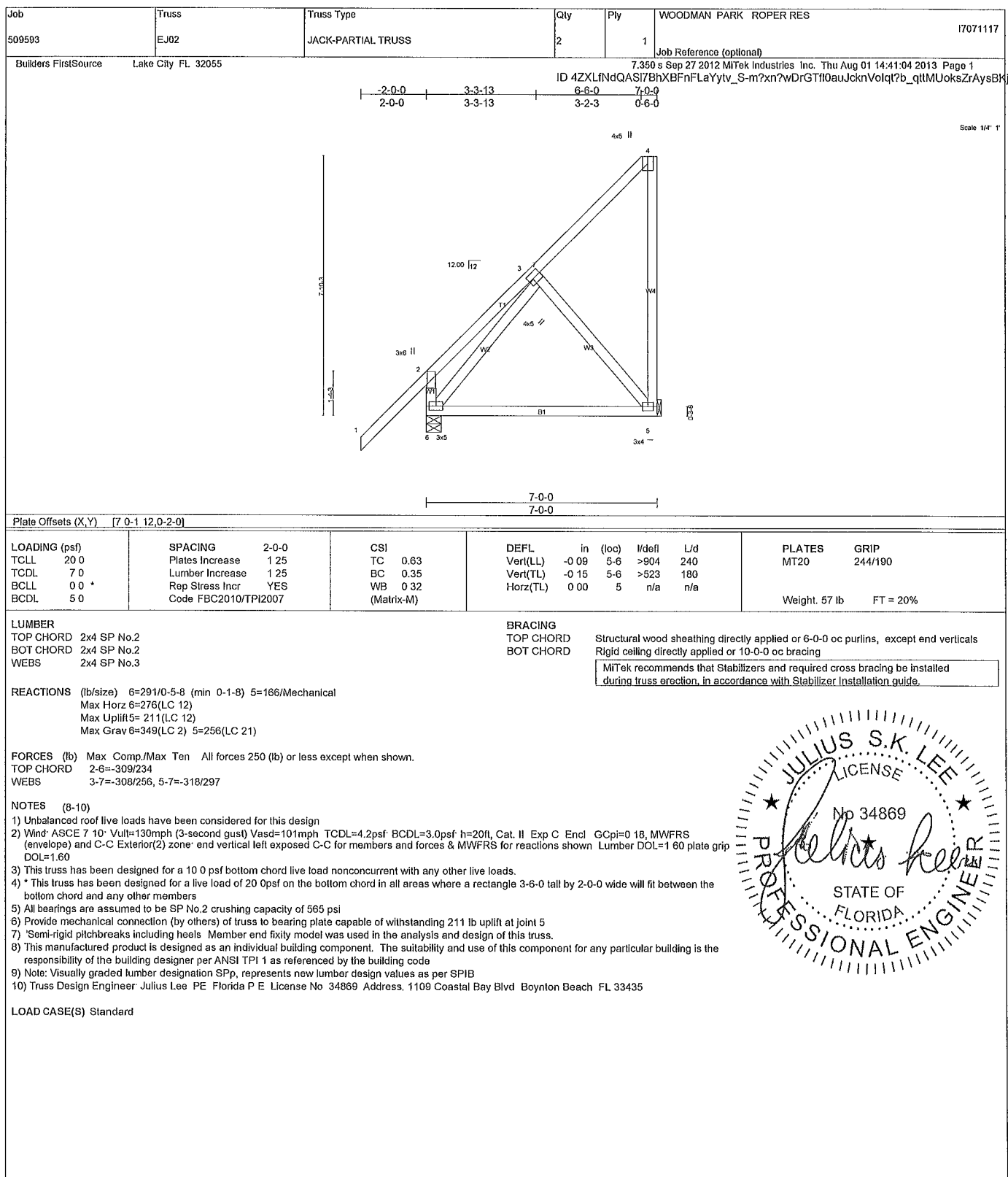
Job 509593	Truss EJ01	Truss Type JACK-PARTIAL TRUSS	Qty 19	Ply 1	WOODMAN PARK ROPER RES.  Job Reference (optional)	17071115	
Builders FirstSource, Lake City FL 32065		7.350 s Sep 27 2012 MITek Industries Inc. Thu Aug 01 14:41:03 2013 Page 1					
<p>ID 4ZXLInDQASi7BhXBFnFLaYytv_S-lpNOobDDV9XvOQJ620GGG4li_Bcv8RWKZ460JjysBKk</p> <p style="text-align: right;">Scale 1:40.3</p>							
<b>LOADING (psf)</b> TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 5.0		<b>SPACING</b> 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2010/TPI2007		<b>CSI</b> TC 0.58 BC 0.46 WB 0.29 (Matrix-M)		<b>DEFL</b> in (loc) l/defl L/d Vert(LL) -0.13 6-7 >640 240 Vert(TL) -0.23 6-7 >358 180 Horz(TL) -0.01 4 n/a n/a	<b>PLATES</b> GRIP MT20 244/190 Weight: 46 lb FT = 20%
<b>LUMBER</b> TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3							
<b>BRACING</b> TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing <div style="border: 1px solid black; padding: 2px; margin-top: 5px;">MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.</div>							
<b>REACTIONS</b> (lb/size) 4=72/Mechanical 5=97/Mechanical 7=293/0-5-8 (min 0-1-8) Max Horz 7=284(LC 12) Max Uplift 4=-95(LC 12) 5=-124(LC 12) Max Grav 4=108(LC 21) 5=153(LC 21) 7=352(LC 2)							
<b>FORCES (lb)</b> Max Comp./Max Ten. All forces 250 (lb) or less except when shown TOP CHORD 2-7=-429/536 2-3=-271/398 WEBS 3-7=-568/346 3-6=-332/312							
<b>NOTES</b> (7-9) 1) Wind ASCE 7 10 Vult=130mph (3-second gust) Vasd=101mph TCDL=4 2psf BCDL=3 0psf h=20ft Cat. II Exp C Encl GCpi=0 18 MWFRS (envelope) and C-C Exterior(2) zone end vertical left exposed C-C for members and forces & MWFRS for reactions shown Lumber DOL=1 60 plate grip DOL=1.60 2) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads. 3) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 95 lb uplift at joint 4 and 124 lb uplift at joint 5 6) Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code 8) Note Visually graded lumber designation SPP represents new lumber design values as per SPIB 9) Truss Design Engineer: Julius Lee PE Florida P.E. License No. 34869 Address 1109 Coastal Bay Blvd Boynton Beach FL 33435							
<b>LOAD CASE(S)</b> Standard							



August 1, 2013

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 Applicability of design parameters and proper incorporation of component is responsibility of building designer. Not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute 583 D'Onofrio Drive Madison, WI 53719

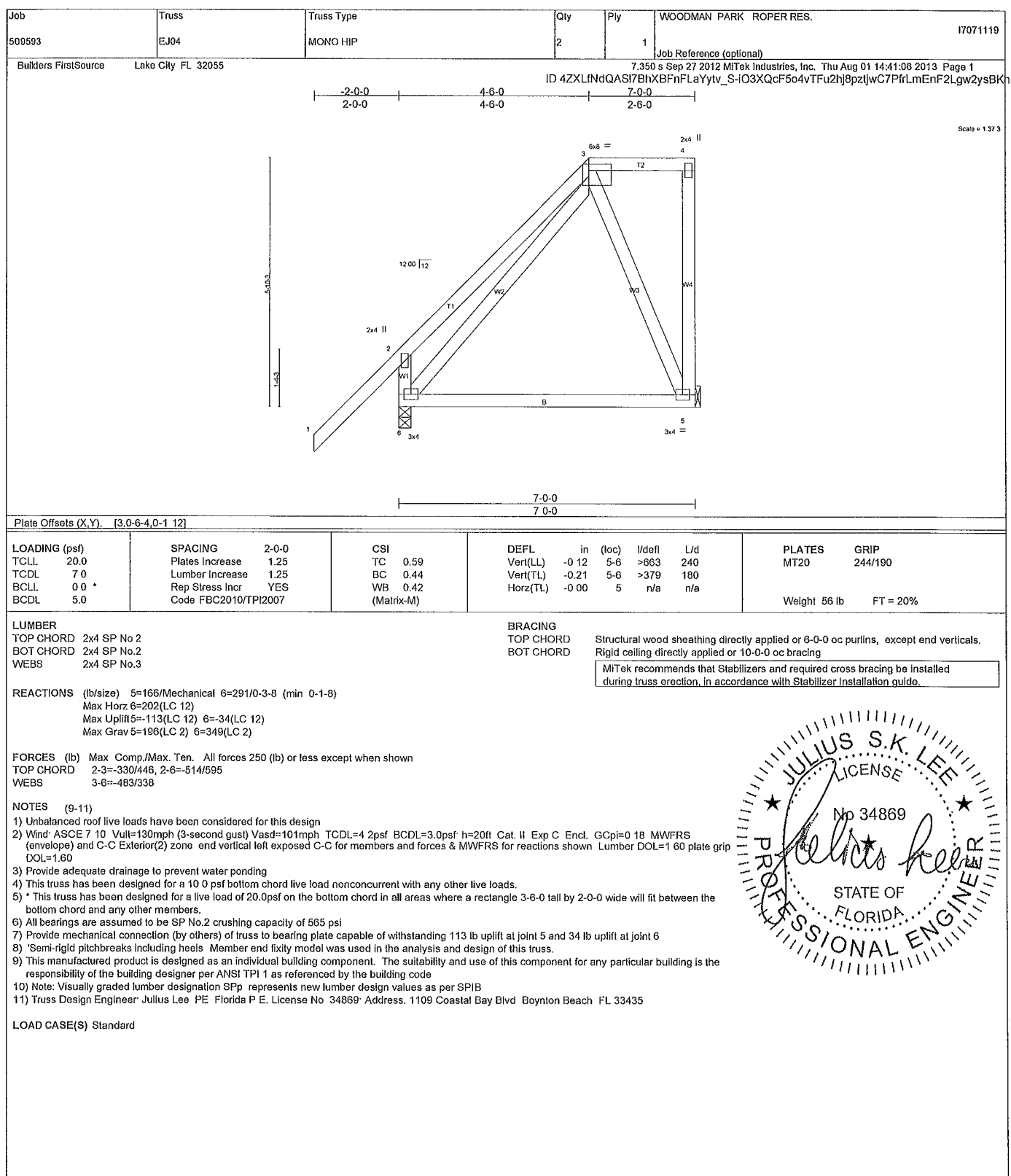
Julius Lee PE,  
 1109 Coastal Bay  
 Boynton Beach, FL 33435



August 1, 2013

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Boynton Beach, FL 33435

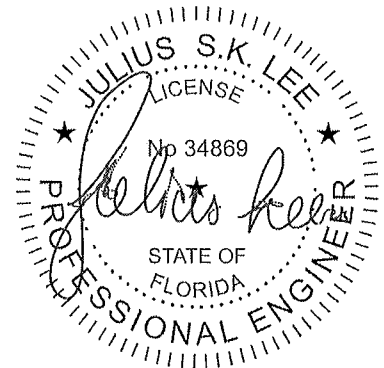


August 1, 2013

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Julius Lee PE  
1109 Coastal Bay  
Boynton Beach, FL 33435

Job 509593	Truss EJ08	Truss Type Jack-Open Truss	Qty 4	Ply 1	WOODMAN PARK ROPER RES	I7071121
Builders FirstSource Lake City FL 32055		Job Reference (optional) ID 4ZXLFndQASi7BhXBFnFLaYytv_S-AacveyGjZO1Kt1duHsLCQwSRxozy4GSwUi4ESUysBKg 7,350 s Sep 27 2012 MITek Industries Inc. Thu Aug 01 14:41:07 2013 Page 1				
Plate Offsets (X,Y) [2 0-2-9,0-1-8]						
<b>LOADING (psf)</b> TCCL 20 0 TCCL 7 0 BCLL 0 0 * BCDL 5 0	<b>SPACING</b> 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2010/TPI2007	<b>CSI</b> TC 0.33 BC 0.52 WB 0.17 (Matrix-M)	<b>DEFL</b> in (loc) l/defl L/d Vert(LL) -0 15 6-9 >694 240 Vert(TL) -0 28 6-9 >385 180 Horz(TL) -0 01 5 n/a n/a	<b>PLATES</b> GRIP MT20 244/190  Weight 39 lb FT = 20%		
<b>LUMBER</b> TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3			<b>BRACING</b> TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 9-11-3 oc bracing <div style="border: 1px solid black; padding: 2px; margin-top: 5px;">         MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.       </div>			
<b>REACTIONS (lb/size)</b> 4=79/Mechanical 2=348/0-5-8 (min. 0-1-8) 5=144/Mechanical Max Horz 2=182(LC 12) Max Uplift 4=63(LC 12) 2=-91(LC 12) 5=64(LC 12) Max Grav 4=97(LC 2) 2=415(LC 2) 5=168(LC 2)						
<b>FORCES (lb)</b> Max Comp./Max Ten All forces 250 (lb) or less except when shown TOP CHORD 2-3=-688/153 BOT CHORD 2-6=-351/850 WEBS 3-6=-369/398						
<b>NOTES (7-9)</b> 1) Wind ASCE 7 10 Vult=130mph (3-second gust) Vasd=101mph TCCL=4.2psf BCDL=3.0psf h=20ft Cat. II Exp C Encl GCpi=0.18, MWFRS (envelope) and C-C Exterior(2) zone C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60 2) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads. 3) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 63 lb uplift at joint 4 91 lb uplift at joint 2 and 64 lb uplift at joint 5. 6) Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss. 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code. 8) Note Visually graded lumber designation SPp represents new lumber design values as per SPIB. 9) Truss Design Engineer Julius Lee PE, Florida P E License No 34869 Address, 1109 Coastal Bay Blvd Boynton Beach FL 33435						
<b>LOAD CASE(S)</b> Standard						



August 1,2013

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Julius Lee PE  
 1109 Coastal Bay  
 Boynton Beach, FL 33435

Job 509593	Truss PB01	Truss Type PIGGYBACK TRUSS	Qty 15	Ply 1	WOODMAN PARK ROPER RES.	17071123																																				
Builders FirstSource Lake City FL 32055		7.350 s Sep 27 2012 MiTek Industries Inc. Thu Aug 01 14:41:09 2013 Page 1 ID.4ZXLfNdQASi7BhXBfFLaYytv_S-6ykf3eH_57H26LmGOHNgVLYqddCYBMDx0ZKWNysBKe																																								
<div style="display: flex; justify-content: space-between; margin: 0;"> <span>8-0-6</span> <span>8-0-6</span> <span>16-0-12</span> </div> <div style="display: flex; justify-content: space-between; margin: 0;"> <span>8-0-6</span> <span>8-0-6</span> <span>8-0-6</span> </div>																																										
Scale 1/264																																										
<table border="1" style="width:100%; border-collapse: collapse;"> <tr> <td style="width:15%;">LOADING (psf)</td> <td style="width:15%;">SPACING 2-0-0</td> <td style="width:15%;">CSI</td> <td style="width:15%;">DEFL</td> <td style="width:15%;">PLATES</td> <td style="width:15%;">GRIP</td> </tr> <tr> <td>TCLL 20.0</td> <td>Plates Increase 1.25</td> <td>TC 0.19</td> <td>in (loc) l/defl L/d</td> <td>MT20</td> <td>244/190</td> </tr> <tr> <td>TCDL 7.0</td> <td>Lumber Increase 1.25</td> <td>BC 0.08</td> <td>Vert(LL) 0.01 6 &gt;999 240</td> <td></td> <td></td> </tr> <tr> <td>BCLL 0.0 *</td> <td>Rep Stress Incr YES</td> <td>WB 0.08</td> <td>Vert(TL) -0.01 6-8 &gt;999 180</td> <td></td> <td></td> </tr> <tr> <td>BCDL 5.0</td> <td>Code FBC2010/TPI2007</td> <td>(Matrix)</td> <td>Horz(TL) 0.00 7 n/a n/a</td> <td></td> <td></td> </tr> <tr> <td colspan="4"></td> <td>Weight: 56 lb</td> <td>FT = 20%</td> </tr> </table>							LOADING (psf)	SPACING 2-0-0	CSI	DEFL	PLATES	GRIP	TCLL 20.0	Plates Increase 1.25	TC 0.19	in (loc) l/defl L/d	MT20	244/190	TCDL 7.0	Lumber Increase 1.25	BC 0.08	Vert(LL) 0.01 6 >999 240			BCLL 0.0 *	Rep Stress Incr YES	WB 0.08	Vert(TL) -0.01 6-8 >999 180			BCDL 5.0	Code FBC2010/TPI2007	(Matrix)	Horz(TL) 0.00 7 n/a n/a							Weight: 56 lb	FT = 20%
LOADING (psf)	SPACING 2-0-0	CSI	DEFL	PLATES	GRIP																																					
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				Weight: 56 lb	FT = 20%																																					
<div style="display: flex; justify-content: space-between;"> <div style="width:45%;"> <p><b>LUMBER</b></p> <p>TOP CHORD 2x4 SP No.2</p> <p>BOT CHORD 2x4 SP No.2</p> <p>WEBS 2x4 SP No.3</p> <p>OTHERS 2x4 SP No.3</p> <p><b>REACTIONS</b> All bearings 0-3-8 except (if=length) 1=0-5-8 7=0-5-8</p> <p>(lb) Max Horz 1=-54(LC 8)</p> <p>Max Uplift All uplift 100 lb or less at joint(s) 1 7 9 except 10=-131(LC 12) 8=-128(LC 13)</p> <p>Max Grav All reactions 250 lb or less at joint(s) 1 7 except 9=300(LC 2) 10=292(LC 27)</p> <p>8=292(LC 28)</p> <p><b>FORCES</b> (lb) Max Comp./Max Ten All forces 250 (lb) or less except when shown</p> <p>WEBS 4-9=-266/134 3-10=-271/269 5-8=-271/269</p> <p><b>NOTES</b> (10-12)</p> <p>1) Unbalanced roof live loads have been considered for this design</p> <p>2) Wind: ASCE 7 10' Vult=130mph (3-second gust) Vasd=101mph TCDL=4.2psf BCDL=3.0psf h=20ft: Cat. II Exp C Encl GCpi=0.18 MWFRS (envelope) and C-C Exterior(2) zone C-C for members and forces &amp; MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60</p> <p>3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.</p> <p>4) * This truss has been designed for a live load of 20.0 psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.</p> <p>5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi</p> <p>6) Bearing at joint(s) 1 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula Building designer should verify capacity of bearing surface</p> <p>7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 7 9 except (if=lb) 10=131 8=128</p> <p>8) 'Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss</p> <p>9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable or consult qualified building designer</p> <p>10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code</p> <p>11) Note: Visually graded lumber designation SPp represents new lumber design values as per SPIB.</p> <p>12) Truss Design Engineer Julius Lee PE, Florida P.E. License No 34869 Address: 1109 Coastal Bay Blvd Boynton Beach FL 33435</p> </div> <div style="width:50%;"> <p><b>BRACING</b></p> <p>TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.</p> <p>BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing</p> <div style="border: 1px solid black; padding: 5px; margin-top: 10px;"> <p>MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.</p> </div> </div> </div>																																										



August 1, 2013

**WARNING** Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MI-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE,  
1109 Coastal Bay  
Boynton Beach, FL 33435



Job 509593	Truss PB03	Truss Type Piggyback Truss	Qty 1	Ply 1	WOODMAN PARK ROPER RES	I7071125
Builders FirstSource Lake City FL 32055					Job Reference (optional) 7 350 s Sep 27 2012 MITek Industries Inc. Thu Aug 01 14:41 11 2013 Page 1	
ID 4ZXLIIndQASI7BhXBFnFLaYyiv_S-3LsQTKJEddXmMfwWfP8amdAgQR705?WPK2RbGysBK6						
<div style="display: flex; justify-content: space-between; margin: 0;"> <span>7-0-12 7-0-12</span> <span>9-0-0 1 11-4</span> <span>16-0-12 7-0-12</span> </div>						
Scale 1:267						
<div style="display: flex; justify-content: space-between; margin: 0;"> <span>4-0-0 4-0-0</span> <span>7-0-12 3-0-12</span> <span>8-0-6 0-11 10</span> <span>12-0-12 4-0-6</span> <span>16-0-12 4-0-0</span> </div>						
<b>LOADING (psf)</b> TCCL 20.0 TCCL 7.0 BCCL 0.0 * BCDL 5.0	<b>SPACING</b> 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2010/TPI2007	<b>CSI</b> TC 0.15 BC 0.12 WB 0.07 (Matrix)	<b>DEFL</b> in (loc) l/defl L/d Vert(LL) 0.01 2 >999 240 Vert(TL) -0.01 2 >999 180 Horiz(TL) 0.00 9 n/a n/a		<b>PLATES</b> MT20 <b>GRIP</b> 244/190  Weight, 64 lb FT = 20%	
<b>LUMBER</b> TOP CHORD 2x4 SP No 2 BOT CHORD 2x4 SP No 2 WEBS 2x4 SP No 3 OTHERS 2x4 SP No 3						
<b>BRACING</b> TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing <div style="border: 1px solid black; padding: 2px; margin-top: 5px;">           MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.         </div>						
<b>REACTIONS</b> All bearings 0-3-8 except (jt=length) 1=0-5-8, 9=0-5-8 (lb) Max Horz 1=47(LC 9) Max Uplift All uplift 100 lb or less at joint(s) 1 9 12 except 14=120(LC 12) 10=116(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1 9 except 14=282(LC 27) 10=282(LC 28) 12=313(LC 2)						
<b>FORCES</b> (lb) Max Comp./Max Ten All forces 250 (lb) or less except when shown.						
<b>NOTES</b> (12 14) 1) Unbalanced roof live loads have been considered for this design 2) Wind, ASCE 7 10' Vult=130mph (3-second gust) Vasd=101mph TCCL=4.2psf BCDL=3.0psf h=20ft Cat. II Exp C Encl GCpi=0.18 MWFRS (envelope) and C-C Exterior(2) zone C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60 3) Provide adequate drainage to prevent water ponding 4) All plates are 2x4 MT20 unless otherwise indicated 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads 6) * This truss has been designed for a live load of 20.0 psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members 7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi 8) Bearing at joint(s) 1 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula Building designer should verify capacity of bearing surface 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 9 12 except (jt=lb) 14=120 10=116 10) Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss. 11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable or consult qualified building designer 12) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code 13) Note Visually graded lumber designation SPP represents new lumber design values as per SPIB 14) Truss Design Engineer Julius Lee PE Florida P E License No 34869 Address, 1109 Coastal Bay Blvd Boynton Beach FL 33435						
<b>LOAD CASE(S)</b> Standard						



August 1, 2013

**WARNING** Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.  
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component.  
 Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE,  
 1109 Coastal Bay  
 Boynton Beach, FL 33435

Job 509593	Truss PB05	Truss Type Piggyback Truss	Qty 2	Ply 1	WOODMAN PARK ROPER RES.	17071127
Builders FirstSource, Lake City FL 32055					Job Reference (optional) 7,350 s Sep 27 2012 MiTek Industries Inc. Thu Aug 01 14:41 13 2013 Page 1 ID:4ZXLfNdQASi7BhXBfnFLaYytlv_S-7k_Au?KU8EnUbz41d7RcfBiW6D7gU?hpseXYf8ysBKa	

LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 5.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2010/TPI2007	CSI TC 0 16 BC 0 11 WB 0 12 (Matrix)	DEFL in (loc) l/defl L/d Vert(LL) -0.01 2-6 >999 240 Vert(TL) -0.01 2-6 >999 180 Horz(TL) 0.00 6 n/a n/a	PLATES GRIP MT20 244/190  Weight: 26 lb FT = 20%
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**LUMBER**

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

OTHERS 2x4 SP No.3

**REACTIONS** (lb/size) 1=25/0-5-8 (min. 0-3-4) 5=25/0-5-8 (min. 0-3-4) 6=382/0-3-8 (min 0-1-8)

Max Horz 1=28(LC 9)

Max Uplift 1=19(LC 28) 5=-19(LC 27) 6=-115(LC 12)

Max Grav 1=61(LC 27) 5=61(LC 28) 6=454(LC 2)

**FORCES** (lb) Max Comp/Max Ten All forces 250 (lb) or less except when shown

TOP CHORD 2-3=-220/286 3-4=-220/286

BOT CHORD 2-6=-245/271 4-6=-245/271

WEBS 3-6=-449/408

**NOTES** (10-12)

- 1) Unbalanced roof live loads have been considered for this design
- 2) Wind ASCE 7 10: Vuft=130mph (3-second gust) Vasd=101mph TCDL=4.2psf BCDL=3.0psf h=20ft; Cat. II Exp C End GCPI=0.18 MWFRS (envelope) and C-C Exterior(2) zone-C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- 5) All bearings are assumed to be SP No 2 crushing capacity of 565 psi
- 6) Bearing at joint(s) 1 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula Building designer should verify capacity of bearing surface
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 5 except (jt=lb) 6=115
- 8) Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss.
- 9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable or consult qualified building designer
- 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
- 11) Note Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
- 12) Truss Design Engineer Julius Lee PE Florida P.E. License No 34869 Address: 1109 Coastal Bay Blvd Boynton Beach FL 33435

**LOAD CASE(S)** Standard

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins

BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

August 1,2013

**WARNING** Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D5B 89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE,  
1109 Coastal Bay  
Boynton Beach, FL 33435

Job 509593	Truss T01G	Truss Type COMMON TRUSS	Qty 1	Ply 1	WOODMAN PARK ROPER RES.  Job Reference (optional) ID:4ZXLFndQASI7BhXBFnFLaYyiv_S-x65xJhMkg1CqGEQXU4lcolf1nlyw86Jy0fk1ysBK	17071129
Builders FirstSource, Lake City FL 32055			7.350 s Sep 27 2012 MiTek Industries, Inc. Thu Aug 01 14:41 15 2013 Page 1			

LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 5.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2010/TPI2007	CSI TC 0.60 BC 0.13 WB 0.06 (Matrix-M)	DEFL in (loc) l/defl L/d Vert(LL) -0.01 9 >999 240 Vert(TL) -0.01 9 >999 180 Horz(TL) -0.01 2 n/a n/a	PLATES GRIP MT20 244/190  Weight 109 lb FT = 20%
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**LUMBER**

TOP CHORD 2x6 SP No 2 \*Except\*  
T1 2x4 SP No 2

BOT CHORD 2x6 SP No 2

WEBS 2x4 SP No 3

OTHERS 2x4 SP No 3

WEDGE  
Left 2x4 SYP No.3, Right 2x4 SYP No.3

**REACTIONS** All bearings 3-3-8  
(lb) Max Horz 2=238(LC 10)  
Max Uplift All uplift 100 lb or less at joint(s) 10 8 except 2=163(LC 13) 6=155(LC 12)  
Max Grav All reactions 250 lb or less at joint(s) 10 8 except 2=396(LC 2) 6=396(LC 2)  
2=365(LC 1) 6=365(LC 1)

**FORCES (lb)** Max Comp./Max Ten All forces 250 (lb) or less except when shown  
TOP CHORD 2-3=-405/517 3-4=-348/134 4-5=-348/133 5-6=-415/509  
BOT CHORD 2-10=-171/296 10-25=-171/296 9-25=-171/296 9-26=-171/296 8-26=-171/296 6-8=-171/296

**NOTES (12 14)**

- Unbalanced roof live loads have been considered for this design
- Wind ASCE 7 10' Vult=130mph (3-second gust) Vasd=101mph TCDL=4 2psf BCDL=3 0psf h=20ft, Cat. II Exp C Encl GCpf=0.18 MWFRS (envelope) gable end zone and C-C Exterior(2) zone end vertical left and right exposed C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only For studs exposed to wind (normal to the face) see Standard Industry Gable End Details as applicable or consult qualified building designer as per ANSI/TPI 1
- All plates are 2x4 MT20 unless otherwise indicated
- The Fabrication Tolerance at joint 2 = 12% joint 6 = 12%
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members with BCDL = 5 0psf
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 10 8 except (it=lb) 2=163 6=155, 2=163, 6=155
- 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
- Note Visually graded lumber designation SPp, represents new lumber design values as per SPIB
- Truss Design Engineer Julius Lee PE Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd Boynton Beach FL 33435

**LOAD CASE(S)** Standard

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

August 1,2013



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Julius Lee PE,  
1109 Coastal Bay  
Boynton Beach, FL 33435

Job 509593	Truss T02G	Truss Type GABLE	Qty 1	Ply 1	WOODMAN PARK ROPER RES  Job Reference (optional) 7 350 s Sep 27 2012 MITek Industries, Inc. Thu Aug 01 14:41 19 2013 Page 1 ID 4ZXLFndQASi7BhXBfnFLaYytlv_S-quLR93PFk4YdJuXB_NY0vSyS8e8UueohEa_stoysBKJ	17071131
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Builders FirstSource Lake City FL 32055

Job 509593	Truss T03	Truss Type COMMON TRUSS	Qty 3	Ply 1	WOODMAN PARK ROPER RES Job Reference (optional)	17071132
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Builders FirstSource Lake City FL 32055
7.350 s Sep 27 2012 MiTek Industries Inc. Thu Aug 01 14:41:20 2013 Page 1
ID 4ZXLFndQASi7BhXBFnFLaYytv\_S-I4vqMPQIVogUx16OY54FSgVZU2lpd35rTEkQPEysBK

Plate Offsets (X,Y) [2 0-3-8,Edge], [3 0-3-0,0-3-4], [5 0-3-0,0-3-4], [6 0-1 12,0-1-8], [7,Edge,0-3-8]									
LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL 20.0	Plates Increase 1.25	TC 0.79	Ver(LL) -0.28	8-10	>938	240	MT20	244/190	
TCDL 7.0	Lumber Increase 1.25	BC 0.98	Ver(TL) -0.52	8-10	>505	180			
BCLL 0.0 *	Rep Stress Incr NO	WB 0.53	Horz(TL) 0.01	7	n/a	n/a			
BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)					Weight 179 lb	FT = 20%	

**LUMBER**

TOP CHORD 2x4 SP No.2

BOT CHORD 2x6 SP No.2

WEBS 2x4 SP No.3 \*Except\*

W4 2x4 SP No 2

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 3-1 15 oc purlins, except end verticals

BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing

WEBS 1 Row at midpt 4-8, 4-10

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

**REACTIONS** (lb/size) 11=1076/0-5-8 (min 0-1 14) 7=972/0-5-8 (min 0-1 11)

Max Horz 11=357(LC 9)

Max Uplift 11=292(LC 12) 7=260(LC 12)

Max Grav 11=1175(LC 21) 7=1120(LC 21)

**FORCES** (lb) Max Comp./Max Ten All forces 250 (lb) or less except when shown

TOP CHORD 2-3=-1776/634 3-4=-2005/1005 4-5=-2025/1011 5-6=-1777/627 2-11=-1747/720 6-7=-1587/570

BOT CHORD 10-11=-343/340 10-12=-132/667 9-12=-132/667 9-13=-132/667 8-13=-132/667

WEBS 4-8=-673/1298 5-8=-607/512 4-10=-658/1273 3-10=-591/488 2-10=-273/1114 6-8=-264/1123

**NOTES** (9-11)

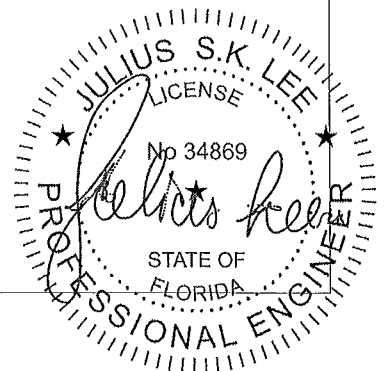
- Unbalanced roof live loads have been considered for this design
- Wind ASCE 7 10- Vult=130mph (3-second gust) Vasd=101mph TCDL=4.2psf BCDL=3.0psf h=20ft; Cat. II Exp C Encl GCp=0.18 MWFRS (envelope) and C-C Exterior(2) zone end vertical left and right exposed C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0 psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (if=lb) 11=292, 7=260
- \*Semi-rigid pitchbreaks including heels Member and fixity model was used in the analysis and design of this truss.
- In the LOAD CASE(S) section loads applied to the face of the truss are noted as front (F) or back (B)
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
- Note Visually graded lumber designation SPp represents new lumber design values as per SPIB
- Truss Design Engineer Julius Lee PE Florida P.E. License No 34869 Address 1109 Coastal Bay Blvd Boynton Beach FL 33435

**LOAD CASE(S)** Standard

1) Regular Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert. 1 2=-44, 2-4=-44 4-6=-44 10-11=-10 10-12=-61(F=-51), 12-13=-91(F=-51), 8-13=-61(F=-51) 7-8=-10



August 1, 2013

**WARNING** Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MI-7473 BEFORE USE.

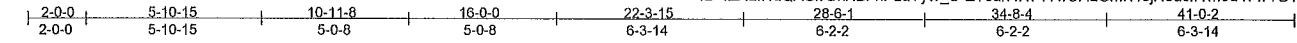
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D5B 89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE  
1109 Coastal Bay  
Boynton Beach, FL 33435

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK ROPER RES.	17071134
509593	T05	Half Hip Truss	1	1		

Builders FirstSource Lake City FL 32055

7.350 s Sep 27 2012 MITek Industries, Inc. Thu Aug 01 14:41:22 2013 Page 1  
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Scale 1/32

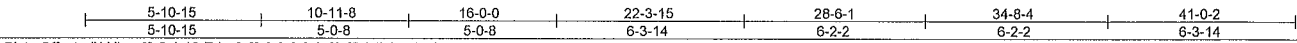
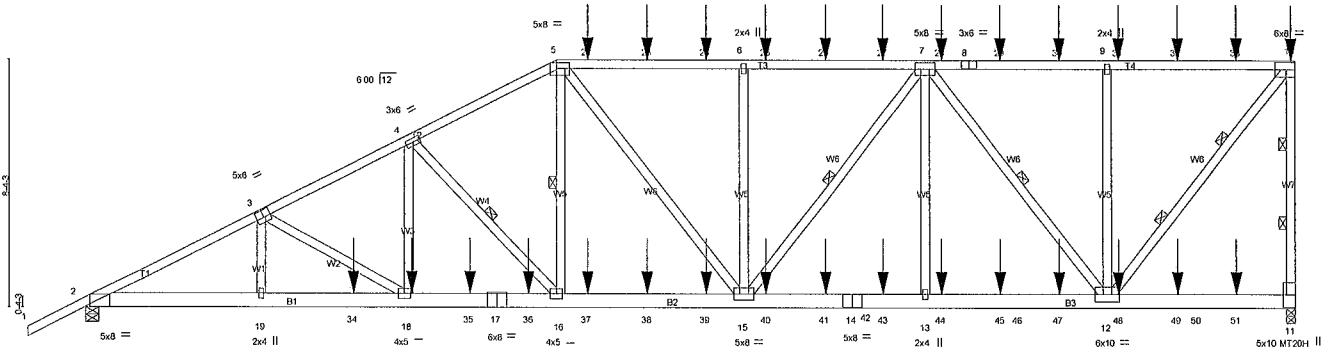


Plate Offsets (X,Y) [2.0-1 10,Edge], [3.0-3.0-0-3-0], [5.0-5-8,0-2-4], [11 Edge,0-3-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	I/def	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.94	Vert(LL)	0.47 15-16	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.88	Vert(TL)	-0.57 15-16	>863	180	MT20H	187/143
BCDL 0.0 *	Rep Stress Incr	NO	WB 0.88	Horz(TL)	0.15 11	n/a	n/a		
BCDL 5.0	Code FBC2010/TPI2007		(Matrix-M)					Weight 307 lb	FT = 20%

LUMBER	BRACING	
TOP CHORD 2x4 SP No.2 *Except*	TOP CHORD	Structural wood sheathing directly applied except end verticals
T3 2x4 SP No 1	BOT CHORD	Rigid ceiling directly applied or 4-1 1 cc bracing
BOT CHORD 2x6 SP No.2 *Except*	WEBS	1 Row at midpt 4-16 5-16 7 15, 7 12
B1 2x6 SP SS		2 Rows at 1/3 pts 10-11 10-12
WEBS 2x4 SP No.3 *Except*		
W7 W6, 2x4 SP No 2		

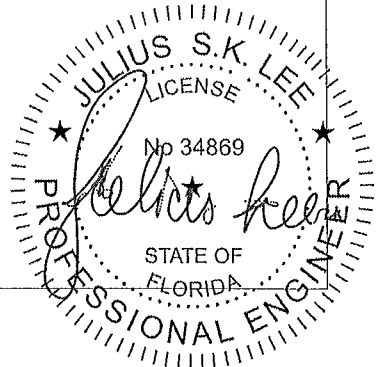
MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 11=2614/0-3-8 (min. 0-3-5) 2-2367/0-5-8 (min 0-3-3)  
Max Horz 2=305(LC 8)  
Max Uplift 11=2369(LC 5) 2=1702(LC 8)  
Max Grav 11=2790(LC 2) 2=2679(LC 2)

FORCES (lb) Max Comp./Max Ten All forces 250 (lb) or less except when shown  
TOP CHORD 2-3=-5457/3647 3-4=-4926/3480 4-5=-3989/3018 5-22=-3626/2940 22-23=-3626/2940  
23-24=-3626/2940 6-24=-3626/2940 6-25=-3626/2940 25-26=-3626/2940 26-27=-3626/2940  
7 27=-3626/2940 7-28=-1875/1586 8-28= 1875/1586 8-29= 1875/1586 29-30=-1875/1586  
9-30= 1875/1586 9-31=-1875/1586 31-32=-1875/1586 32-33=-1875/1586 10-33=-1875/1586  
10-11=-2664/2261  
BOT CHORD 2 19=-3473/4828 19-34=-3473/4827 16-34=-3473/4827 18-35=-3236/4353 17 35=-3236/4353  
17-36=-3236/4353 16-36=-3236/4353, 16-37=-2750/3532 37-38= 2750/3532 38-39=-2750/3532  
15-39=-2750/3532 15-40=-2541/3057 40-41= 2541/3057 41-42= 2541/3057 14-42=-2541/3057  
14-43=-2541/3057 13-43=-2541/3057 13-44=-2541/3057 44-45=-2541/3057 45-46=-2541/3057  
46-47=-2541/3057 12-47=-2541/3057  
WEBS 3-19=-80/296, 3-18=-548/274 4-18=-595/1020 4-16=-1223/729 5-16=-941/1314 5-15=-445/187  
6-15=-476/415 7 15=-681/929 7 13=-358/468 7 12=-1929/1558 9-12=-478/409  
10-12=-2570/3039

#### NOTES (11 13)

- 1) Wind ASCE 7 10, Vult=130mph (3-second gust) Vasd=101mph TCDL=4 2psf BCDL=3 0psf h=20ft, Cat. II Exp C, End GCp=0.18 MWFRS (envelope) Lumber DOL=1.60 plate grip DOL=1.60
- 2) Provide adequate drainage to prevent water ponding
- 3) All plates are MT20 plates unless otherwise indicated
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jit=lb) 11=2369 2=1702
- 8) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.

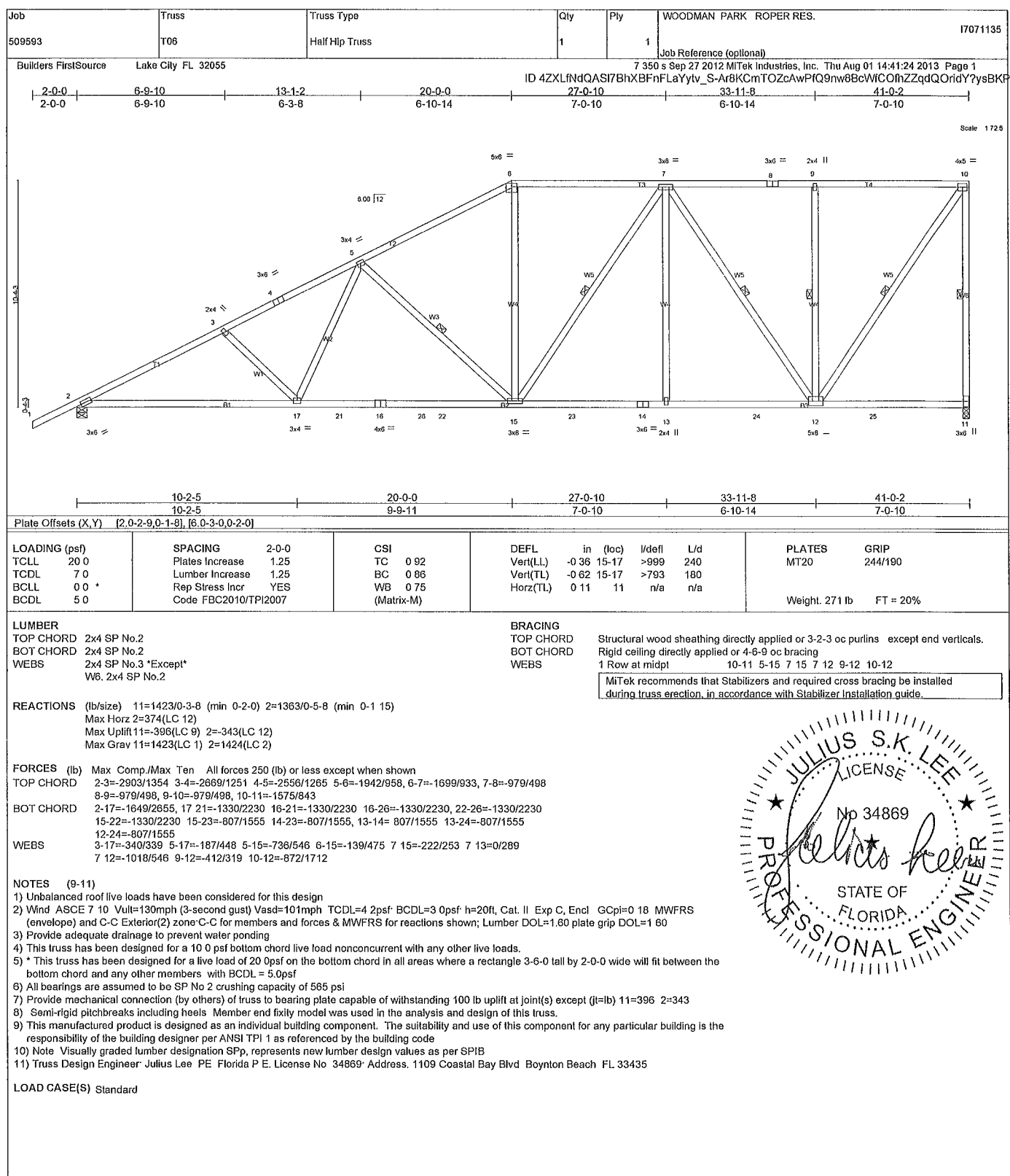


August 1, 2013

Continued on page 2

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Julius Lee PE,  
1109 Coastal Bay  
Boynton Beach, FL 33435



August 1, 2013

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Julius Lee PE  
1109 Coastal Bay  
Boynton Beach, FL 33435

Job 509593	Truss T08	Truss Type Piggyback Base Truss	Qty 3	Ply 1	WOODMAN PARK ROPER RES	17071137																																													
Builders FirstSource Lake City FL 32055		7 350 s Sep 27 2012 MiTek Industries, Inc. Thu Aug 01 14:41:27 2013 Page 1 ID 4ZXLfNdQASi7BhXBFnFLaYytlv_S-bQqTqoVGrXYVG68kS3iuE8HoZsjQm6vt4pwh9KysBKM																																																	
<div style="display: flex; justify-content: space-between;"> <div> <p>13-1-2 20-0-0 25-4-4 30-8-8 36-0-12 41-0-2</p> <p>2-0-0 6-9-10 6-9-10 6-3-8 6-10-14 5-4-4 5-4-4 5-4-4 4-11-6</p> </div> <div style="text-align: right;">Scale = 1/32</div> </div>																																																			
<table border="1" style="width:100%; border-collapse: collapse;"> <tr> <th style="width:20%;">LOADING (psf)</th> <th style="width:20%;">SPACING</th> <th style="width:10%;">CSI</th> <th style="width:10%;">DEFL</th> <th style="width:10%;">in (loc)</th> <th style="width:10%;">l/defl</th> <th style="width:10%;">L/d</th> <th style="width:10%;">PLATES</th> <th style="width:10%;">GRIP</th> </tr> <tr> <td>TCLL 20 0</td> <td>Plates Increase 1.25</td> <td>TC 0.81</td> <td>Vert(LL) -0.35</td> <td>16-18</td> <td>&gt;999</td> <td>240</td> <td>MT20</td> <td>244/190</td> </tr> <tr> <td>TCDL 7 0</td> <td>Lumber Increase 1.25</td> <td>BC 0.85</td> <td>Vert(TL) -0.60</td> <td>16-18</td> <td>&gt;822</td> <td>180</td> <td></td> <td></td> </tr> <tr> <td>BCLL 0 0 *</td> <td>Rep Stress Incr YES</td> <td>WB 0.97</td> <td>Horz(TL) 0 11</td> <td>12</td> <td>n/a</td> <td>n/a</td> <td></td> <td></td> </tr> <tr> <td>BCDL 5 0</td> <td>Code FBC2010/TPI2007</td> <td>(Matrix-M)</td> <td></td> <td></td> <td></td> <td></td> <td>Weight 277 lb</td> <td>FT = 20%</td> </tr> </table>							LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP	TCLL 20 0	Plates Increase 1.25	TC 0.81	Vert(LL) -0.35	16-18	>999	240	MT20	244/190	TCDL 7 0	Lumber Increase 1.25	BC 0.85	Vert(TL) -0.60	16-18	>822	180			BCLL 0 0 *	Rep Stress Incr YES	WB 0.97	Horz(TL) 0 11	12	n/a	n/a			BCDL 5 0	Code FBC2010/TPI2007	(Matrix-M)					Weight 277 lb	FT = 20%
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<table border="1" style="width:100%; border-collapse: collapse;"> <tr> <td style="width:50%;"> <b>LUMBER</b>            TOP CHORD 2x4 SP No 2            BOT CHORD 2x4 SP No 2            WEBS 2x4 SP No 3         </td> <td style="width:50%;"> <b>BRACING</b>            TOP CHORD            BOT CHORD            WEBS            Structural wood sheathing directly applied or 3-2-2 oc purlins except end verticals            Rigid ceiling directly applied or 4-7 14 oc bracing            1 Row at midpt 5-16 7 16 7 14 9-13, 10-13 11 12            MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.         </td> </tr> </table>							<b>LUMBER</b> TOP CHORD 2x4 SP No 2 BOT CHORD 2x4 SP No 2 WEBS 2x4 SP No 3	<b>BRACING</b> TOP CHORD BOT CHORD WEBS Structural wood sheathing directly applied or 3-2-2 oc purlins except end verticals Rigid ceiling directly applied or 4-7 14 oc bracing 1 Row at midpt 5-16 7 16 7 14 9-13, 10-13 11 12 MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.																																											
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<b>REACTIONS</b> (lb/size) 2=1361/0-5-8 (min 0-1 15) 12=1398/0-3-8 (min 0-1 15) Max Horz 2=321(LC 12) Max Uplift 2=355(LC 12) 12=283(LC 9) Max Grav 2=1424(LC 2) 12=1398(LC 1)																																																			
<b>FORCES</b> (lb) Max Comp./Max Ten All forces 250 (lb) or less except when shown TOP CHORD 2-3=-2915/1420 3-4=-2682/1318, 4-5=-2568/1332 5-6=-1957/1024 6-7=-1711/992 7-8=-1486/833, 8-9= 1486/833, 9-10=-758/473, 10-11=-871/461 11 12=-1607/825 BOT CHORD 2-18=-1575/2631 18-22=-1257/2208 17-22=-1257/2208, 17 29=-1257/2208, 23-29= 1257/2208 16-23=-1257/2208 16-24=-794/1580 15-24=-794/1580 15-25=-794/1580 14-25= 794/1580 14-26=-620/1259 26-27=-620/1259 13-27=-620/1259 WEBS 3-18=-339/338, 5-18=-186/444 5-16=-738/549 6-16=-197/519 7 14=-498/362 9-14=-314/749 9-13=-1165/609 11 13=-616/1316																																																			
<b>NOTES</b> (9-11) 1) Unbalanced roof live loads have been considered for this design 2) Wind: ASCE 7 10, Vult=130mph (3-second gust) Vasd=101mph TCDL=4 2psf BCDL=3 0psf h=20ft, Cat. II Exp C, Encl GCp=0 18 MWFRS (envelope) and C-C Exterior(2) zone C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1 60 3) Provide adequate drainage to prevent water ponding 4) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads. 5) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members with BCDL = 5 0psf 6) All bearings are assumed to be SP No 2 crushing capacity of 565 psi 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (l=lb) 2=355 12=283. 8) 'Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code 10) Note: Visually graded lumber designation SPp represents new lumber design values as per SPIB 11) Truss Design Engineer Julius Lee PE, Florida P E License No. 34869 Address: 1109 Coastal Bay Blvd Boynton Beach FL 33435																																																			
<b>LOAD CASE(S)</b> Standard																																																			



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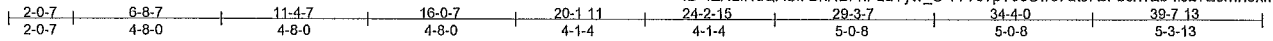
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 1109 Coastal Bay  
 Boynton Beach, FL 33435



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK ROPER RES.	17071139
509593	T10	Half Hip Truss	1	1	Job Reference (optional)	

Builders FirstSource, Lake City FL 32055

7 350 s Sep 27 2012 MITek Industries Inc. Thu Aug 01 14:41:30 2013 Page 1  
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Scale = 1/8" = 1'-0"

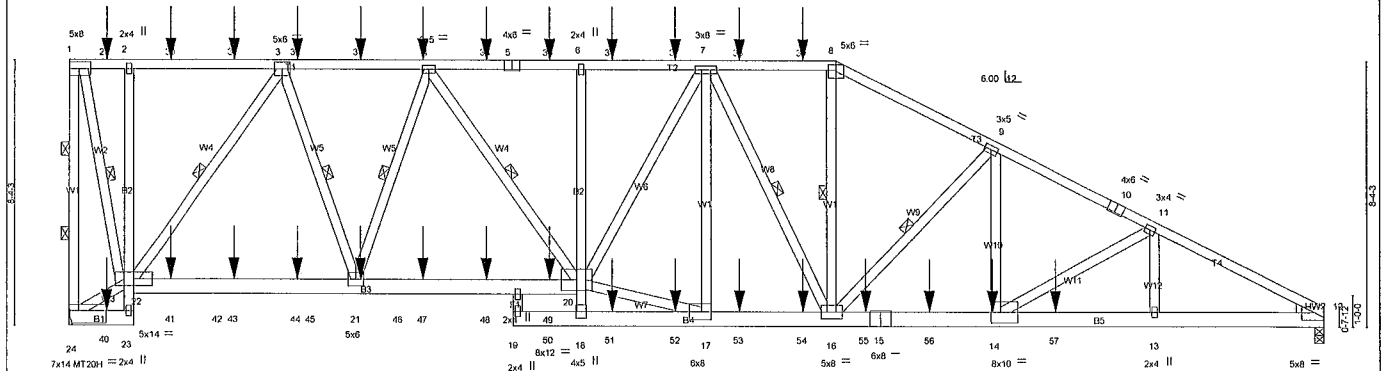


Plate Offsets (X,Y)	[5.0-3.0,Edge], [8.0-3.0-0.2-0], [10.0-3.0-Edge], [12.0-0.4-0-1-3], [14.0-3.8-0.4-0], [17.0-3.8-0.3-0], [24.Edge-0.4-12]
---------------------	--

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 2.0-0	TC 0.94	in (loc) I/defl	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.77	Vert(LL) 0.46 20-21 >999	MT20H	187/143
BCLL 0.0 *	Rep Stress Incr NO	WB 1.00	Vert(TL) -0.55 20-21 >864		
BCDL 5.0	Code FBC2010/TP12007	(Matrix-M)	Horz(TL) 0.21 12 n/a	Weight, 334 lb	FT = 20%

LUMBER	BRACING
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 1-9-15 oc purlins except end
BOT CHORD 2x4 SP No.3 *Except*	verticals
B1 B4 2x6 SP No.2 B3 B5 2x6 SP SS	Rigid ceiling directly applied or 4-4 15 oc bracing Except:
WEBS 2x4 SP No.3 *Except*	10-0-0 oc bracing 18-20
W7 2x4 SP No.2	1 Row at midpt 1-22 3-22 3-21 4-21 4-20 7 16, 8-16 9-16
WEDGE	2 Rows at 1/3 pts 1-24
Right, 2x4 SYP No.3	

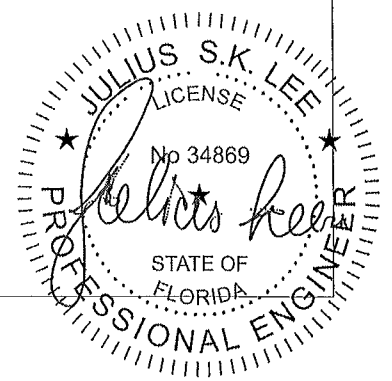
MITek recommends that Stabilizers and required cross bracing be installed during truss erection in accordance with Stabilizer Installation guide.

REACTIONS (lb/size)	24=2402/Mechanical 12=2168/0-3-8 (min 0-3-0)
Max Horz 24=-267(LC 9)	
Max Uplift 24=-2271(LC 4) 12=-1629(LC 9)	
Max Grav 24=2683(LC 2) 12=2525(LC 2)	

FORCES (lb)	Max Comp./Max Ten All forces 250 (lb) or less except when shown
TOP CHORD	1-24=-2578/2088 1 29=-638/517 2 29=-638/517 2-30=-657/532 30-31=-657/532 3-31=-657/532 3-32=-2732/2247 32-33=-2732/2247 4-33=-2732/2247 4-34=-3755/3005 5-34=-3755/3005 5-35=-3755/3005 6-35=-3755/3005 6-36=-3744/2995 6-37=-3744/2995 7-37=-3744/2995 7-38=-3329/2577 8-39=-3329/2577 8-39=-3329/2577 8-9=-3765/2826 9-10=-4581/3246 10-11=-4634/3231 11 12=-4800/3172
BOT CHORD	22-41=-1620/2120 41-42=-1620/2120 42-43=-1620/2120 43-44=-1620/2120 44-45=-1620/2120 21-45=-1620/2120 21-46=-2392/3130 46-47=-2392/3130 47-48=-2392/3130 48-49=-2392/3130 20-49=-2392/3130 6-20=-296/253 18-51=-187/254 51-52=-187/254 17-52=-187/254 17 53=-2538/3442 53-54=-2538/3442 16-54=-2538/3442 16-55=-2763/4097 15-55=-2763/4097 15-56=-2763/4097 14-56=-2763/4097 14-57=-2768/4221 13-57=-2768/4221 12-13=-2768/4221 22 24=-877/329 1-22=-2106/2600 3-22=-2576/2130 3-21=-1598/1930 4-21=-1254/946 4-20=-827/1100 17-20=-2425/3289 7-20=-590/612 7 17=-602/381 7 16=-261/468 8-16=-1111/1402 9-16=-1150/686 9-14=-530/915 11 14=-314/5
WEBS	

- NOTES (12-15)
- 1) Wind ASCE 7 10' Vult=130mph (3-second gust) Vasd=101mph TCDL=4.2psf BCDL=3.0psf h=20ft Cat. II Exp C, Encl GCpi=0.18 MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
  - 2) Provide adequate drainage to prevent water ponding
  - 3) All plates are MT20 plates unless otherwise indicated
  - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf
  - 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
  - 7) Provide metal plate or equivalent at bearing(s) 24 to support reaction chain.
  - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 24=2271 12=1629
  - 9) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.

Continued on page 2

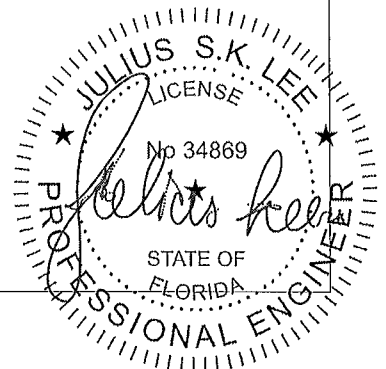


August 1, 2013

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Julius Lee PE,  
1109 Coastal Bay  
Boynton Beach, FL 33435

Job 509593	Truss T11	Truss Type Hip Truss	Qty 1	Ply 1	WOODMAN PARK ROPER RES.	I7071140																																													
Builders FirstSource, Lake City FL 32055					7.350 s Sep 27 2012 MITEK Industries, Inc. Thu Aug 01 14:41:32 2013 Page 1																																														
					ID: 4ZXLNdQASi7BhXBfnFLaYytv_S-xOdMVZPg4BnNt1hFcH3xC7bQIPiRUAcE5e2qYysBKlH																																														
Scale 1/8" = 1'-0"																																																			
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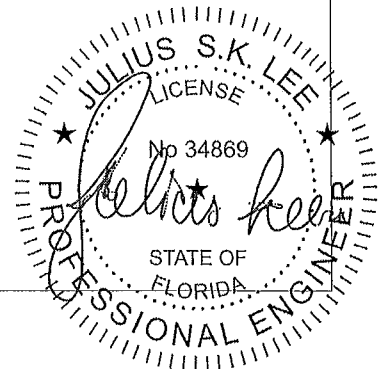


August 1, 2013

**WARNING** Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M17-7473 BEFORE USE.  
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroffo Drive Madison, WI 53719

Julius Lee PE  
 1109 Coastal Bay  
 Boynton Beach, FL 33435

Job 509593	Truss T13	Truss Type Piggyback Base Truss	Qty 1	Ply 1	WOODMAN PARK ROPER RES Job Reference (optional) 7 350 s Sep 27 2012 MITek Industries, Inc. Thu Aug 01 14:41:35 2013 Page 1 ID 4ZXLfNdQASi7BhXBFnFLaYytv_S-MzJvWxbHz?ZMELIGwkrMZqd6g5QSerw2w3siRtysBKE	17071142																														
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LOAD CASE(S) Standard																																				



August 1, 2013

**WARNING** Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.

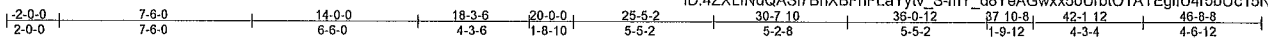
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE  
1109 Coastal Bay  
Boynton Beach, FL 33435

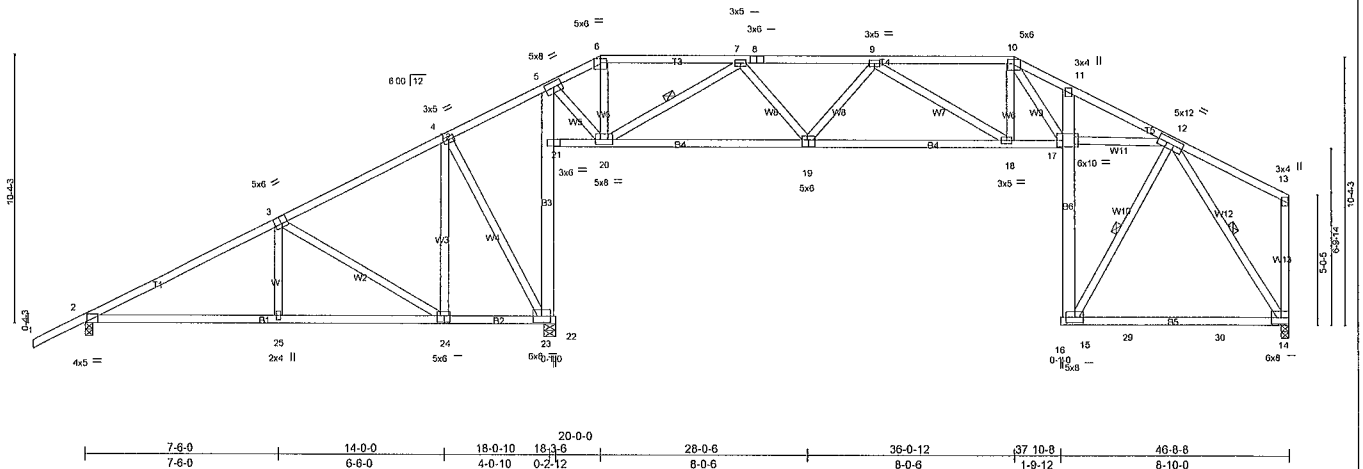
Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK ROPER RES	17071144
509593	T15	Piggyback Base Truss	2	1	Job Reference (optional)	

Builders FirstSource Lake City FL 32055

7.350 s Sep 27 2012 MITek Industries, Inc. Thu Aug 01 14:41:38 2013 Page 1  
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Scale 1/8"=1'-0"



Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK ROPER RES.	17071145
509593	T16	Piggyback Base Truss	4	1		

Builders FirstSource Lake City FL 32055

Job Reference (optional)  
7.350 s Sep 27 2012 MiTek Industries, Inc. Thu Aug 01 14:41:41 2013 Page 1  
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Scale 1/8" = 1'-0"

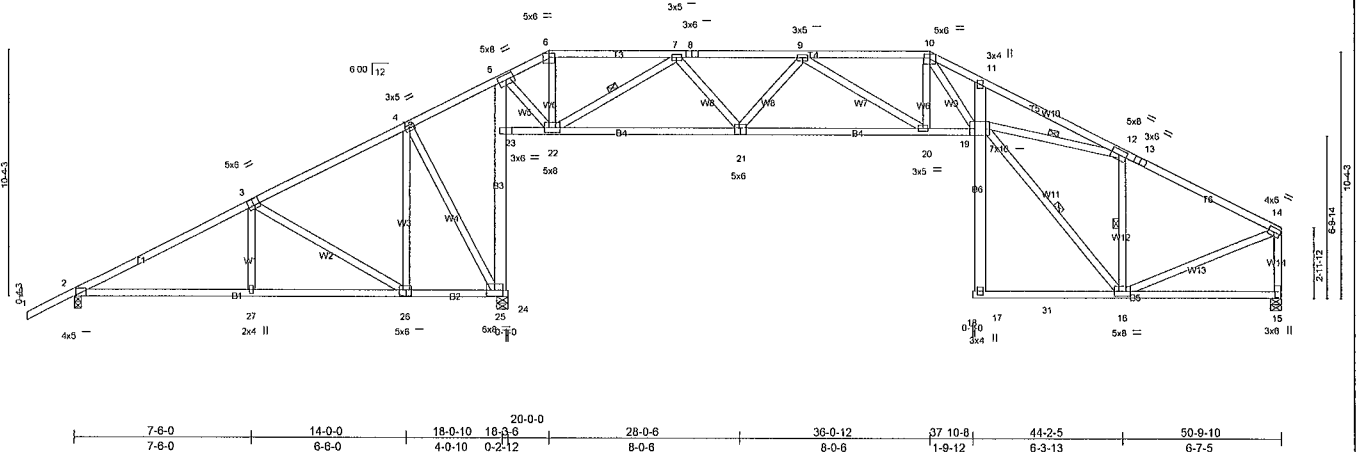


Plate Offsets (X, Y) [3-0-3-0,0-3-4], [6-0-3-0,0-2-0], [10-0-3-0,0-2-0], [19-0-2-8,Edge], [21-0-3-0,0-3-4], [26-0-2-8,0-3-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/def	l/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.90	Vert(LL)	0.55	18	>708	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.91	Vert(TL)	-0.77	18	>510	180		
BCLL 0.0	Rep Stress Incr	NO	WB 0.73	Horz(TL)	0.40	15	n/a	n/a		
BCDL 5.0	Code FBC2010/TPI2007		(Matrix-M)						Weight: 331 lb	FT = 20%

LUMBER  
TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.2 \*Except\*  
B3 B6 2x6 SP No 2  
WEBS 2x4 SP No.3 \*Except\*  
W10 2x4 SP No 2

BRACING  
TOP CHORD Structural wood sheathing directly applied or 2-3-2 oc purlins except end verticals.  
BOT CHORD Rigid ceiling directly applied or 4-8-13 oc bracing Except.  
5-7-0 oc bracing 23-25  
10-0-0 oc bracing 17 19  
WEBS 1 Row at midpt 7-22 16-19 12 19 12-16

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=554/0-3-8 (min 0-1-8) 15=982/0-5-8 (min 0-1-8) 25=1530/0-5-8 (min 0-2-3)  
Max Horz 2=214(LC 12)  
Max Uplift 2=299(LC 9) 15=297(LC 13) 25=640(LC 9)  
Max Grav 2=670(LC 27) 15=1076(LC 2) 25=1776(LC 2)

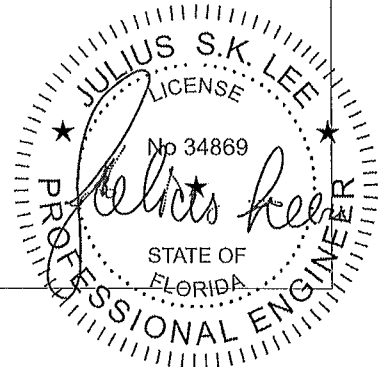
FORCES (lb) Max. Comp./Max Ten All forces 250 (lb) or less except when shown.  
TOP CHORD 2 3=-1231/1196 3-4=-359/579 4-5=-76/283 5-6=-752/653, 6-7=-669/616 7-8=-2591/1719  
8-9= 2591/1719 9-10=-3092/1905, 10-11=-4795/2855 11 12=-4790/2766 12-13=-1184/801  
13-14=-1299/787 14-15=-1166/736  
BOT CHORD 2-27=-1607/1603, 26-27=-996/698 25-26=-352/216, 23-25=-1369/713 5-23=-1374/718,  
22-23=-297/198 21-22=-1078/2011 20-21=-1503/2812 19-20=-1500/2983  
WEBS 3-27=-271/211 3-26=-559/748, 4-26=-604/361 4-25=-582/805 5-22=-578/1216, 7-22=-1698/1013  
7-21=-378/748, 9-21=-525/300 9-20=-119/262 10-19=-1245/2189 16-19=-908/1556,  
12-19=-1584/3089, 12-16=-1542/987 14-16=-646/1141

#### NOTES (10-12)

- Unbalanced roof live loads have been considered for this design
- Wind ASCE 7 10 Vult=130mph (3-second gust) Vasd=101mph TCCL=4 psf BCDL=3 psf h=20ft; Cat. II Exp C Encl GCpi=0.18 MWFRS (envelope) and C-C Exterior(2) zone porch left exposed C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60
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- \* This truss has been designed for a live load of 20 psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=299 15=297 25=640.
- Semi-rigid pitchbreaks including heels Member and fixity model was used in the analysis and design of this truss.
- In the LOAD CASE(S) section loads applied to the face of the truss are noted as front (F) or back (B)
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Note Visually graded lumber designation SPp, represents new lumber design values as per SPIB
- Truss Design Engineer: Julius Lee PE, Florida P E License No 34869 Address: 1109 Coastal Bay Blvd Boynton Beach FL 33435

LOAD CASE(S) Standard

Continued on page 2



August 1, 2013

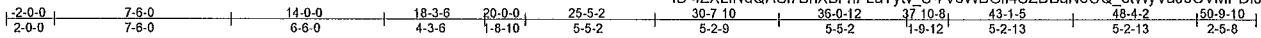
**WARNING** Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.  
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute 583 D'Oroff Drive, Madison, WI 53719

Julius Lee PE  
1109 Coastal Bay  
Boynton Beach, FL 33435

Job	Truss	Truss Type	Qty	Ply	WOODMAN PARK ROPER RES.	17071146
509593	T17	Piggyback Base Truss	2	1	Job Reference (optional)	

Builders FirstSource Lake City FL 32055

7.350 s Sep 27 2012 MiTek Industries Inc. Thu Aug 01 14:41:43 2013 Page 1  
ID 4ZXLNdQASi7BhXBFnFLaYytlv S-7VoWBGIM4SZDBaNoOQ\_etWyVaJ9OVMPDIJp8jPysBK3



Scale = 1/8" = 1'-0"

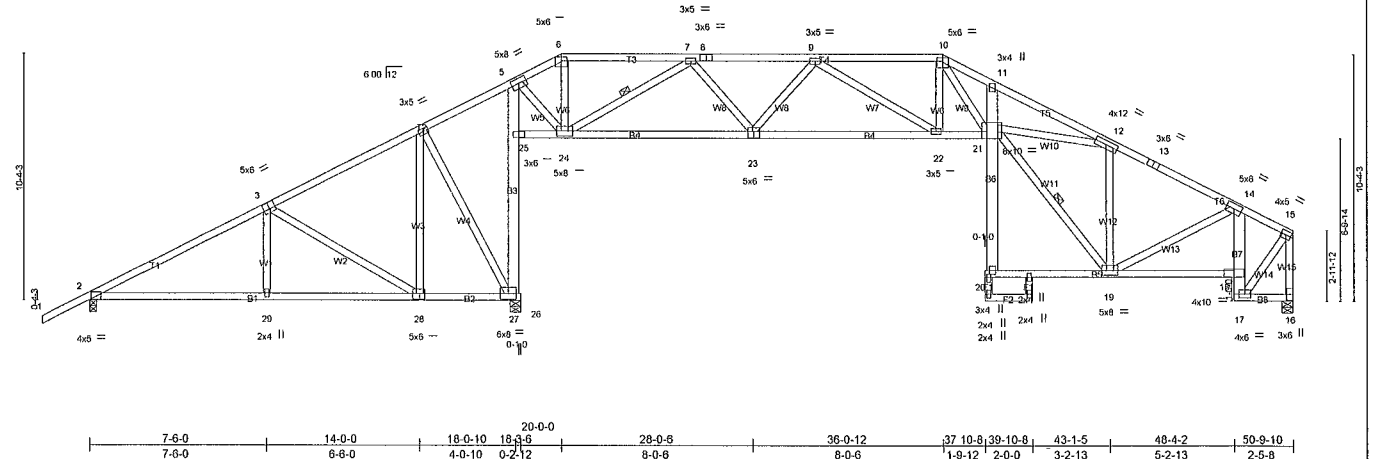


Plate Offsets (X, Y): [3 0-3-0,0-3-4], [6 0-3-0,0-2-0], [10 0-3-0,0-1 12], [20 0-1-8,0-1-0], [21 0-2-8,Edge], [23 0-3-0,0-3-4], [28 0-2-8,0-3-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20 0	Plates Increase	1.25	TC 0 73	Vert(LL)	0.51	21	>765	240	MT20	244/190
TCDL 7 0	Lumber Increase	1.25	BC 0 90	Vert(TL)	-0 71	21	>553	180		
BCLL 0 0 *	Rep Stress Incr	NO	WB 1 00	Horz(TL)	0.42	16	n/a	n/a		
BCDL 5 0	Code FBC2010/TP12007		(Matrix-M)						Weight 341 lb	FT = 20%

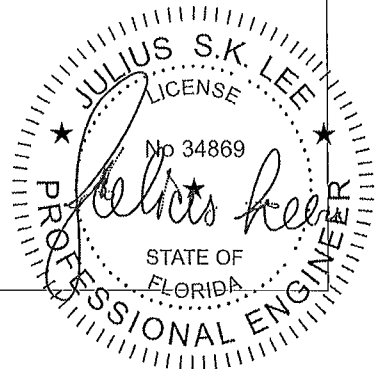
LUMBER	BRACING
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 2-5-14 oc purlins except end
BOT CHORD 2x4 SP No.2 *Except*	verticals
B3 B6,B7 2x6 SP No.2 F2 2x4 SP No.3	Rigid ceiling directly applied or 4-8-12 oc bracing Except:
WEBS 2x4 SP No.3	5-8-0 oc bracing 25-27
	10-0-0 oc bracing 20-21
	1 Row at midpt 7-24 19-21
	MiTek recommends that Stabilizers and required cross bracing be installed during truss erection in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2 557/0-3-8 (min. 0-1-8) 16=907/0-5-8 (min 0-1-8) 27=1496/0-5-8 (min 0-2-3)  
Max Horz 2=214(LC 12)  
Max Uplift 2=-299(LC 9) 16=-298(LC 13) 27=-641(LC 9)  
Max Grav 2=-671(LC 27) 16=1075(LC 2) 27=1773(LC 2)

FORCES (lb) Max Comp./Max. Ten All forces 250 (lb) or less except when shown  
TOP CHORD 2-3=-1233/1196 3-4=-364/580 4-5=-78/285 5-6=-747/658 6-7=-664/620 7-8=-2510/1724  
8-9=-2510/1724 9-10=-2935/1911 10-11=-4499/2859 11 12=-4487/2768 12-13=-1362/979  
13-14=-1458/968 14-15=-710/463 15-16= 1165/751  
BOT CHORD 2-29=-1607/1605 28-29=-997/703 27 28=-353/222 25-27=-1332/712 5-25=-1339/717  
24-25=-275/193 23-24=-1083/1957 22 23=-1509/2707 21 22=-1506/2826 18-19=-542/846  
17 18=-734/482 14-18=-714/500  
WEBS 3-29=-271/211 3-28=-558/747 4-28=-604/359 4-27=-584/805 5-24=-576/1187 7 24=-1638/1014  
7-23=-378/708 9-23=-485/300 10-21=-1243/1997 19-21=-1089/1806 12 21=-1455/2671  
12-19=-1514/966 14 19=-215/422 15-17=-623/978

- NOTES (10-12)
- Unbalanced roof live loads have been considered for this design
  - Wind ASCE 7 10 Vu1=130mph (3-second gust) Vasd=101mph TCDL=4 2psf BCDL=3 0psf h=20ft; Cat. II Exp C, Encl GCp=0 18; MWFRS (envelope) and C-C Exterior(2) zone: porch left exposed C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1 60
  - Provide adequate drainage to prevent water ponding
  - This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads
  - \* This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
  - All bearings are assumed to be SP No 2 crushing capacity of 565 psi
  - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 2=299 16=298 27=641
  - 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.
  - In the LOAD CASE(S) section loads applied to the face of the truss are noted as front (F) or back (B)
  - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
  - Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB.
  - Truss Design Engineer: Julius Lee PE, Florida P E License No. 34869 Address: 1109 Coastal Bay Blvd Boynton Beach FL 33435

LOAD CASES Standard

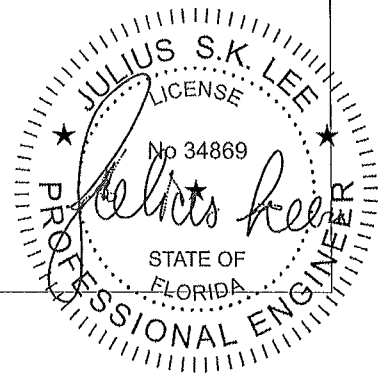


August 1, 2013

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Julius Lee PE  
1109 Coastal Bay  
Boynton Beach, FL 33435

Job 509593	Truss T18	Truss Type Piggyback Base Truss	Qty 3	Ply 1	WOODMAN PARK ROPER RES.  Job Reference (optional) 7 350 s Sep 27 2012 MITek Industries Inc. Thu Aug 01 14:41:45 2013 Page 1 ID:4ZXLFndQASi7BhXBfnFLaYyIv_S-3uvHcyjZc3pxRIWBVr08zx1uN7swzFHWdliEoHysBK4	17071147																																																					
Builders FirstSource Lake City FL 32055																																																											
<table border="1" style="width:100%; border-collapse: collapse;"> <tr> <td style="width:15%;">Plate Offsets (X,Y)</td> <td style="width:15%;">[3 0-4-0,0-3-0], [5-0-3-0,0-2-0], [9-0-3-0,0-2-0], [11-0-3-0,0-3-0], [23-0-1 12,0-3-0]</td> <td style="width:15%;">LOADING (psf)</td> <td style="width:15%;">SPACING</td> <td style="width:15%;">CSF</td> <td style="width:15%;">DEFLECT</td> <td style="width:15%;">PLATES</td> <td style="width:15%;">GRIP</td> </tr> <tr> <td>TCLL</td> <td>20 0</td> <td>Plates Increase</td> <td>1.25</td> <td>TC</td> <td>0.52</td> <td>in (loc)</td> <td>l/defl</td> <td>L/d</td> </tr> <tr> <td>TCDL</td> <td>7 0</td> <td>Lumber Increase</td> <td>1.25</td> <td>BC</td> <td>0.89</td> <td>0 23 24-27</td> <td>&gt;449</td> <td>240</td> </tr> <tr> <td>BCLL</td> <td>0 0 *</td> <td>Rep Stress Incr</td> <td>YES</td> <td>WB</td> <td>0.97</td> <td>Vert(TL)</td> <td>-0.50 16-17</td> <td>&gt;999</td> </tr> <tr> <td>BCDL</td> <td>5 0</td> <td>Code</td> <td>FBC2010/TPI2007</td> <td>(Matrix-M)</td> <td></td> <td>Horz(TL)</td> <td>0.21 14</td> <td>n/a</td> </tr> <tr> <td colspan="7"></td> <td>Weight: 356 lb</td> <td>FT = 20%</td> </tr> </table>							Plate Offsets (X,Y)	[3 0-4-0,0-3-0], [5-0-3-0,0-2-0], [9-0-3-0,0-2-0], [11-0-3-0,0-3-0], [23-0-1 12,0-3-0]	LOADING (psf)	SPACING	CSF	DEFLECT	PLATES	GRIP	TCLL	20 0	Plates Increase	1.25	TC	0.52	in (loc)	l/defl	L/d	TCDL	7 0	Lumber Increase	1.25	BC	0.89	0 23 24-27	>449	240	BCLL	0 0 *	Rep Stress Incr	YES	WB	0.97	Vert(TL)	-0.50 16-17	>999	BCDL	5 0	Code	FBC2010/TPI2007	(Matrix-M)		Horz(TL)	0.21 14	n/a								Weight: 356 lb	FT = 20%
Plate Offsets (X,Y)	[3 0-4-0,0-3-0], [5-0-3-0,0-2-0], [9-0-3-0,0-2-0], [11-0-3-0,0-3-0], [23-0-1 12,0-3-0]	LOADING (psf)	SPACING	CSF	DEFLECT	PLATES	GRIP																																																				
TCLL	20 0	Plates Increase	1.25	TC	0.52	in (loc)	l/defl	L/d																																																			
TCDL	7 0	Lumber Increase	1.25	BC	0.89	0 23 24-27	>449	240																																																			
BCLL	0 0 *	Rep Stress Incr	YES	WB	0.97	Vert(TL)	-0.50 16-17	>999																																																			
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							Weight: 356 lb	FT = 20%																																																			
<table border="1" style="width:100%; border-collapse: collapse;"> <tr> <td style="width:50%;"> <b>LUMBER</b>            TOP CHORD 2x4 SP No.2            BOT CHORD 2x4 SP No.2 *Except*            B4 2x4 SP No.3, B5: 2x4 SP No.1            WEBS 2x4 SP No.3         </td> <td style="width:50%;"> <b>BRACING</b>            TOP CHORD Structural wood sheathing directly applied or 3-11-9 oc purlins except end            BOT CHORD Rigid ceiling directly applied or 5-0-9 oc bracing            WEBS 1 Row at midpt 6-22 8-20 8-19 9-19  <div style="border: 1px solid black; padding: 2px; margin-top: 5px;">             MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.           </div> </td> </tr> </table>							<b>LUMBER</b> TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 *Except* B4 2x4 SP No.3, B5: 2x4 SP No.1 WEBS 2x4 SP No.3	<b>BRACING</b> TOP CHORD Structural wood sheathing directly applied or 3-11-9 oc purlins except end BOT CHORD Rigid ceiling directly applied or 5-0-9 oc bracing WEBS 1 Row at midpt 6-22 8-20 8-19 9-19 <div style="border: 1px solid black; padding: 2px; margin-top: 5px;">             MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.           </div>																																																			
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<b>REACTIONS</b> (lb/size) 2=162/0-3-8 (min 0-1 8) 14=1241/0-5-8 (min 0-1 13) 24=1784/0-5-8 (min 0-2-9) Max Horz 2=214(LC 12) Max Uplift 2=121(LC 8) 14=-294(LC 13) 24=-467(LC 9) Max Grav 2=274(LC 27) 14=1309(LC 2) 24=1810(LC 2)																																																											
<b>FORCES</b> (lb) Max Comp./Max. Ten All forces 250 (lb) or less except when shown TOP CHORD 2-3=-503/1029 3-4=-1103/573 4-5=-1525/875 5-6=-1329/848 6-7=-1705/1018 7-8=-1705/1018 8-9=-1559/985 9-10=-2056/1260 10-11=-2040/1126, 11 12=-1279/733 12 13=-1004/537 13-14=-1620/859 BOT CHORD 2-24=-1551/762 23-24=-882/436 23-28=-350/888 22-28=-350/888 22-29=-599/1525 21 29=-599/1525, 21-30=-599/1525, 20-30=-599/1525 20-31=-648/1606 31-32=-648/1606 19-32=-648/1606, 10-17=-286/283, 16-17=-822/1627 15-16=-918/493 WEBS 3-24=-2158/1173 3-23=-913/2054, 4-23=-944/545 4-22=-143/505 5-22=-155/358 6-22=-656/332 6-20=-67/303 8-19=-385/156 17 19=-554/1506 9-17=-478/678, 11 16=-872/444 13-15=-693/1336																																																											
<b>NOTES</b> (9-11) 1) Unbalanced roof live loads have been considered for this design 2) Wind ASCE 7 10' Vult=130mph (3-second gust) Vasd=101mph TCCL=4 2psf BCDL=3 0psf h=20ft; Cat. II Exp C End GCp=0.18 MWFRS (envelope) and C-C Exterior(2) zone porch left exposed-C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60 3) Provide adequate drainage to prevent water ponding 4) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads. 5) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5 0psf 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (lb=lb) 2=121 14=294 24=467 8) Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss. 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code 10) Note: Visually graded lumber designation SPp represents new lumber design values as per SPIB. 11) Truss Design Engineer Julius Lee, PE, Florida P E License No 34869 Address: 1109 Coastal Bay Blvd Boynton Beach FL 33435																																																											
LOAD CASE(S) Standard																																																											



August 1,2013

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Julius Lee PE,  
 1109 Coastal Bay  
 Boynton Beach, FL 33435

Job 509593	Truss T25	Truss Type COMMON TRUSS	Qty 4	Ply 1	WOODMAN PARK ROPER RES  Job Reference (optional) ID.4ZXLfNdQASi7BhXBFnFLaYytlv_S-?G111dIpBg4fgBgadG3a2M6EQwayRJEpgwnLsAysBK2	17071149
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Builders FirstSource Lake City FL 32055
7 350 s Sep 27 2012 MiTek Industries, Inc. Thu Aug 01 14:41:47 2013 Page 1

LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 BCDL 5.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2010/TPI2007	CSI TC 0.55 BC 0.41 WB 0.37 (Matrix-M)	DEFL in (loc) Vert(LL) -0.07 7-8 >999 240 Vert(TL) -0.13 8-9 >999 180 Horz(TL) 0.01 7 n/a n/a	PLATES GRIP MT20 244/190  Weight 108 lb FT = 20%
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**LUMBER**

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

**REACTIONS** (lb/size) 9=516/0-5-8 (min 0-1-8) 7=409/0-5-8 (min 0-1-8)

Max Horz 9=343(LC 9)

Max Uplift 9=257(LC 12) 7=195(LC 12)

Max Grav 9=616(LC 2) 7=484(LC 2)

**FORCES** (lb) Max Comp/Max Ten All forces 250 (lb) or less except when shown

TOP CHORD 3-4=-584/315 4-5=-584/317 2-9=-281/227

BOT CHORD 8-9=-223/406 7-8= 165/394

WEBS 4-8= 249/391 5-8=-238/274 3-8=-236/255 3-9=-632/244 5-7=-581/256

**NOTES** (8-10)

- 1) Unbalanced roof live loads have been considered for this design
- 2) Wind ASCE 7 10' Vult=130mph (3-second gust) Vasd=101mph TCDL=4.2psf BCDL=3.0psf h=20ft, Cat. II Exp C, Encl GCpi=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone end vertical left and right exposed C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 4) \* This truss has been designed for a live load of 20.0 psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (if=lb) 9=257 7=195
- 7) Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
- 9) Note: Visually graded lumber designation SPP represents new lumber design values as per SPIB.
- 10) Truss Design Engineer: Julius Lee PE Florida P.E. License No 34869 Address: 1109 Coastal Bay Blvd Boynton Beach FL 33435

**LOAD CASE(S)** Standard

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.



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Julius Lee PE  
1109 Coastal Bay  
Boynton Beach, FL 33435



Job 509593	Truss T26	Truss Type COMMON TRUSS	Qty 1	Ply 2	WOODMAN PARK ROPER RES.  Job Reference (optional) ID 4ZXLFndQASi7BhXBfnFLaYyiv_S-Qrj9fnnhRbSEXeP9IOclf?kod8jleXIFMu??TVysBK?	17071151
Builders FirstSource Lake City FL 32055		7.350 s Sep 27 2012 MiTek Industries, Inc. Thu Aug 01 14:41:50 2013 Page 1				

Plate Offsets (X,Y) [2 0-10-4,0-0-2], [8 0-10-4,0-0-2]					
LOADING (psf)	SPACING 2-0-0	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1 25	TC 0.36	in (loc)	MT20	244/190
TCDL 7.0	Lumber Increase 1 25	BC 0.24	Ver(LL) 0.08 10-11 >999 240		
BCLL 0.0 *	Rep Stress Incr NO	WB 0.90	Ver(TL) -0.12 9-10 >999 180		
BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)	Horz(TL) 0.02 8 n/a n/a		
			Weight: 276 lb	FT = 20%	

**LUMBER**

TOP CHORD 2x4 SP No.2

BOT CHORD 2x8 SP DSS

WEBS 2x4 SP No.3

SLIDER Left 2x8 SP DSS 1-6-0, Right 2x8 SP DSS 1-6-0

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 5-5-11 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

**REACTIONS** (lb/size) 8=5333/0-5-8 (min 0-3-4) 2=2981/0-5-8 (min 0-1 15)

Max Horz 2=234(LC 5)

Max Uplift 8= 1952(LC 8) 2=-1636(LC 8)

Max Grav 8=5555(LC 2) 2=3242(LC 2)

**FORCES** (lb) Max Comp./Max. Ten. All forces 250 (lb) or less except when shown

TOP CHORD 2-3= 2766/1453 3-4=-3426/1858 4-5=-3557/1899 5-6=-3557/1899 6-7=-4604/1847

7-8=-4001/1514

BOT CHORD 2-11=-1312/2333 11-20=-1312/2333 10-20= 1312/2333 10-21=-1231/3171 21-22=-1231/3171

9-22=-1231/3171 9-23=-1231/3171 8-23=-1231/3171

WEBS 5-10=-2562/4707 6-10=-1143/126 6-9=-60/1552 4-10=-208/340 4-11=-436/34

**NOTES** (11 13)

1) 2-ply truss to be connected together with 10d (0 131'x3') nails as follows

Top chords connected as follows 2x4 1 row at 0-9-0 oc.

Bottom chords connected as follows 2x8 2 rows staggered at 0-4-0 oc.

Webs connected as follows 2x4 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies except if noted as front (F) or back (B) face in the LOAD CASE(S) section Ply to ply connections have been provided to distribute only loads noted as (F) or (B) unless otherwise indicated

3) Unbalanced roof live loads have been considered for this design

4) Wind: ASCE 7 10' Vuft=130mph (3-second gust) Vasd=101mph TCCL=4 2psf BCDL=3 0psf h=20ft Cat. II Exp C, Encl GCpi=0 18 MWFRS (envelope); end vertical left exposed Lumber DOL=1.60 plate grip DOL=1.60

5) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (if=lb) 8=1952 2=1636

9) Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss

10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2673 lb down and 2281 lb up at 7-0-12 1267 lb down and 267 lb up at 9-0-12 1267 lb down and 267 lb up at 11-0-12 and 1267 lb down and 267 lb up at 13-0-12 and 1211 lb down and 269 lb up at 15-0-12 on bottom chord The design/selecion of such connection device(s) is the responsibility of others.

11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code

12) Note Visually graded lumber designation SPp represents new lumber design values as per SPIB.

13) Truss Design Engineer Julius Lee PE Florida P.E. License No 34869 Address: 1109 Coastal Bay Blvd Boynton Beach FL 33435

**LOAD CASE(S)** Standard

Continued on page 2



August 1,2013



**WARNING** Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oro Drive, Madison, WI 53719

Julius Lee PE.  
1109 Coastal Bay  
Boynton Beach, FL 33435

Job 509593	Truss T27G	Truss Type GABLE	Qty 1	Ply 1	WOODMAN PARK ROPER RES.	17071152
Builders FirstSource Lake City FL 32055					7 350 s Sep 27 2012 MiTek Industries Inc. Thu Aug 01 14:41:52 2013 Page 1	
					ID 4ZXLIIndQASI7BhXBfnFLaYytlv_S-MEqw4LpyzDlymyZXQpemiIQq5OxOs6a0YqCU6XNysBJz	

Job 509593	Truss T28G	Truss Type GABLE	Qty 1	Ply 1	WOODMAN PARK ROPER RES.  Job Reference (optional)	I7071154
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Builders FirstSource Lake City FL 32055
7,350 s Sep 27 2012 MITek Industries Inc. Thu Aug 01 14:41:55 2013 Page 1

ID: 4ZXLfndQASi7BhXBfnFLaYytv\_S-mpW2INrqF84WdQH65xCTM2Sd09NbJvd\_WAjm7IysBJW

Scale = 1/8" = 1'-0"

Plate Offsets (X, Y) [2,0-0-12,0-10-2], [9,0-3-5,0-1-0]								
LOADING (psf)	SPACING 2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20 0	Plates Increase 1.25	TC 0.45	Vert(LL) 0 11	9-11	>719	240	MT20	244/190
TCDL 7 0	Lumber Increase 1.25	BC 0.43	Vert(TL) 0 10	9-11	>812	180		
BCLL 0 0 *	Rep Stress Incr YES	WB 0.47	Horz(TL) 0 04	11	n/a	n/a		
BCDL 5 0	Code FBC2010/TPI2007	(Matrix-M)					Weight 209 lb	FT = 20%

**LUMBER**

TOP CHORD 2x4 SP No.2

BOT CHORD 2x6 SP No.2

WEBS 2x4 SP No.3

OTHERS 2x4 SP No.3

SLIDER Left 2x8 SP DSS 2-7 10 Right 2x8 SP DSS 1-6-0

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins

BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing

WEBS 1 Row at midpt 5-11

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

**REACTIONS** (lb/size) 2=430/0-5-8 (min 0-1-8) 9=259/0-3-8 (min 0-1-8) 11=659/0-5-8 (min 0-1-8)

Max Horz 2=380(LC 11)

Max Uplift 2=198(LC 13) 9=200(LC 8) 11=370(LC 12)

Max Grav 2=512(LC 2) 9=329(LC 28) 11=728(LC 21)

**FORCES** (lb) Max. Comp/Max Ten All forces 250 (lb) or less except when shown

TOP CHORD 2-3=-727/165, 3-4=-603/191 4-5=-674/209 5-6=-242/411

BOT CHORD 2-12=-323/653 11 12=-277/414 11-30=-255/228, 9-30=-251/229

WEBS 4-12=-389/336 5-12=-246/599 5-11=-636/257 6-11=-506/440

**NOTES** (12 14)

1) Unbalanced roof live loads have been considered for this design

2) Wind ASCE 7 10' Vult=130mph (3-second gust) Vasd=101mph TCDL=4 2psf BCDL=3 0psf h=20ft, Cat. II Exp C End GCpi=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone end vertical left and right exposed porch right exposed C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only For studs exposed to wind (normal to the face) see Standard Industry Gable End Details as applicable or consult qualified building designer as per ANSI/TPI 1

4) All plates are 2x4 MT20 unless otherwise indicated

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf

8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi

9) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula Building designer should verify capacity of bearing surface

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (j=lb) 2=198 9=200 11=370.

11) 'Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss.

12) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code

13) Note Visually graded lumber designation SPp represents new lumber design values as per SPIB.

14) Truss Design Engineer Julius Lee PE Florida P.E. License No 34869 Address 1109 Coastal Bay Blvd Boynton Beach, FL 33435



August 1, 2013

**WARNING** Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroffo Drive, Madison, WI 53719

Julius Lee PE  
1109 Coastal Bay  
Boynton Beach, FL 33435

Job 509593	Truss T30	Truss Type MONOPITCH TRUSS	Qty 6	Ply 1	WOODMAN PARK ROPER RES  Job Reference (optional) ID 4ZXLFndQASi7BhXBfnFLaYytlv_S-E74RwisS0RCNFZslffjivG_p?Ylk2TE8tqSKg9ysBJv	17071156
Builders FirstSource Lake City FL 32055		7 350 s Sep 27 2012 MiTek Industries Inc. Thu Aug 01 14:41 56 2013 Page 1				

Scale = 1/2" = 1'-0"

Plate Offsets (X,Y) [2-0-5-8,0-0-8]							
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d
TCLL 20 0	Plates Increase	1 25	TC 0 37	Vert(LL)	0 11	4-7	>620 240
TCDL 7 0	Lumber Increase	1 25	BC 0 30	Vert(TL)	0 09	4-7	>712 180
BCLL 0 0 *	Rep Stress Incr	YES	WB 0 00	Horz(TL)	-0 00	2	n/a n/a
BCDL 5 0	Code FBC2010/TPI2007		(Matrix-M)				
				PLATES		GRIP	
				MT20		244/190	
				Weight 25 lb		FT = 20%	

**LUMBER**

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

**REACTIONS** (lb/size) 4=116/0-5-8 (min 0-1-8) 2=273/0-5-8 (min 0-1-8)

Max Horz 2=124(LC 12)

Max Uplift 4=-89(LC 9) 2=-120(LC 9)

Max Grav 4=137(LC 2) 2=327(LC 2)

**FORCES** (lb) Max Comp./Max Ten. All forces 250 (lb) or less except when shown

TOP CHORD 2-3=-437/877

BOT CHORD 2-4=-1374/692

**NOTES** (7-9)

- 1) Wind ASCE 7 10 Vult=130mph (3-second gust) Vasd=101mph TCDL=4 2psf BCDL=3 0psf h=20ft, Cat II Exp C Encl GCpi=0 18 MWFRS (envelope) and C-C Exterior(2) zone porch left and right exposed C-C for members and forces & MWFRS for reactions shown Lumber DOL=1 60 plate grip DOL=1 80
- 2) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (l=lb) 2=120.
- 6) Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
- 8) Note Visually graded lumber designation SPP represents new lumber design values as per SPIB.
- 9) Truss Design Engineer Julius Lee PE Florida P E License No 34869 Address. 1109 Coastal Bay Blvd Boynton Beach FL 33435

**LOAD CASE(S)** Standard

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 5-8-10 oc purlins except end verticals

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.



August 1,2013

**WARNING** Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

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Julius Lee PE  
1109 Coastal Bay  
Boynton Beach, FL 33435

Job 509593	Truss T32	Truss Type KINGPOST	Qty 8	Ply 1	WOODMAN PARK ROPER RES	I7071158
Builders FirstSource Lake City FL 32055					Job Reference (optional) 7.350 s Sep 27 2012 MITek Industries, Inc. Thu Aug 01 14:41:58 2013 Page 1 ID:4ZXLfNdQASi7BhXBFnFLaYyiv_S-BOCBLOjY3S5Uiohm3IA_h4ERMVrWNURC8xQk1ysB...	

Plate Offsets (X,Y) [2 0-2-6,0-1-0], [4 0-2-6,0-1-0]					
LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20 0	2-0-0	TC 0.05	in (loc) l/defl L/d	MIT20	244/190
TCDL 7 0	Plates Increase 1.25	BC 0.03	Vert(L) -0 00 6 >999 240		
BCLL 0 0 *	Lumber Increase 1.25	WB 0.02	Vert(TL) -0 00 6 >999 180		
BCDL 5 0	Rep Stress Incr YES	(Matrix-M)	Horz(TL) 0 00 4 n/a n/a		
	Code FBC2010/TPI2007			Weight 16 lb	FT = 20%

**LUMBER**

TOP CHORD 2x4 SP No 2

BOT CHORD 2x4 SP No 2

WEBS 2x4 SP No 3

**REACTIONS** (lb/size) 2=110/0-3-8 (min 0-1-8) 4=110/0-3-8 (min 0-1-8)

Max Horz 2=55(LC 10)

Max Uplift 2=35(LC 12) 4=35(LC 13)

Max Grav 2=132(LC 2) 4=132(LC 2)

**FORCES** (lb) Max. Comp/Max Ten All forces 250 (lb) or less except when shown

**NOTES** (8-10)

- Unbalanced roof live loads have been considered for this design
- Wind: ASCE 7 10 Vult=130mph (3-second gust) Vasd=101mph TCCL=4 2psf BCDL=3.0psf h=20ft, Cat. II Exp C, Encl GCpi=0 18, MWFRS (envelope) and C-C Exterior(2) zone C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1 60
- This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- All bearings are assumed to be SP No 2 crushing capacity of 565 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 4
- 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Note Visually graded lumber designation SPp represents new lumber design values as per SPIB
- Truss Design Engineer Julius Lee PE, Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd Boynton Beach FL 33435

**LOAD CASE(S)** Standard

**BRACING**

TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

August 1,2013

**WARNING** Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 BEFORE USE.

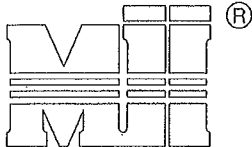
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oonofrio Drive, Madison, WI 53719

Julius Lee PE,  
1109 Coastal Bay  
Boynton Beach, FL 33435

August 10, 2010

# T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

ST - T-BRACE 2



MiTek Industries, Inc

MiTek Industries, Chesterfield, MO Page 1 of 1

Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs

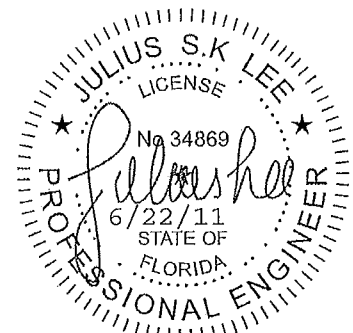
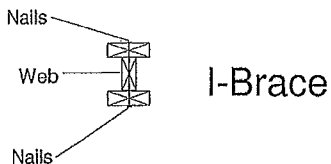
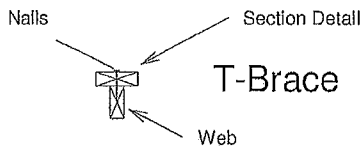
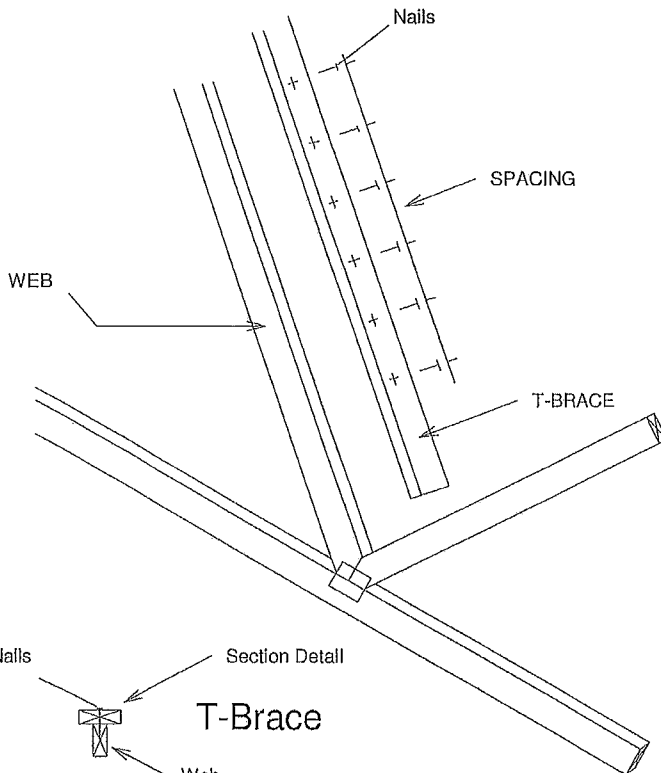
Nailing Pattern		
T-Brace size	Nail Size	Nail Spacing
2x4 or 2x6 or 2x8	10d	6" o.c.
Note Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)		

Brace Size for One-Ply Truss		
Specified Continuous Rows of Lateral Bracing		
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

Brace Size for Two-Ply Truss		
Specified Continuous Rows of Lateral Bracing		
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

Brace Size for Two-Ply Truss		
Specified Continuous Rows of Lateral Bracing		
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

T-Brace / I-Brace must be same species and grade (or better) as web member

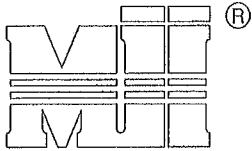


1109 COASTAL BAY  
BOYNTON BC, FL 33435

JANUARY 1, 2009

## LATERAL TOE-NAIL DETAIL

ST-TOENAIL\_SP



MITek Industries, Inc

MITek Industries, Chesterfield, MO Page 1 of 1

## NOTES

- 1 TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN)
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD
- 3 ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES

TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)

	DIAM.	SYP	DF	HF	SPF	SPF-S
3.5" LONG	131	88.0	80.6	69.9	68.4	59.7
	135	93.5	85.6	74.2	72.6	63.4
	162	108.8	99.6	86.4	84.5	73.8
3.25" LONG	128	74.2	67.9	58.9	57.6	50.3
	131	75.9	69.5	60.3	59.0	51.1
	148	81.4	74.5	64.6	63.2	52.5

THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

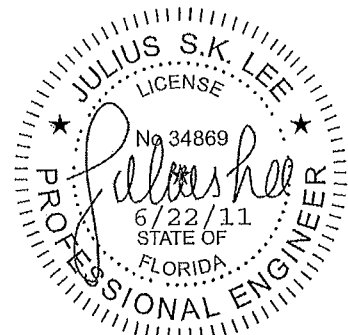
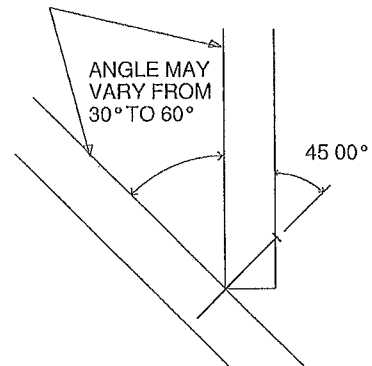
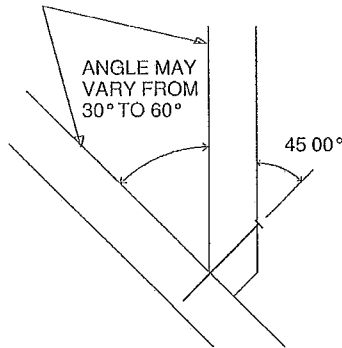
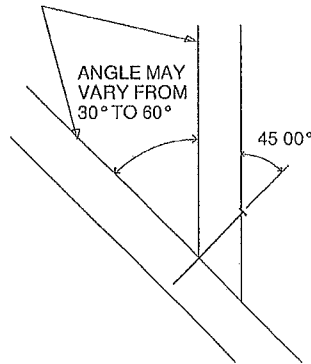
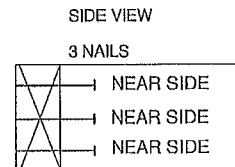
VALUES SHOWN ARE CAPACITY PER TOE-NAIL.  
 APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED

## EXAMPLE

(3) 16d NAILS (162 diam x 3.5") WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1.15

3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity

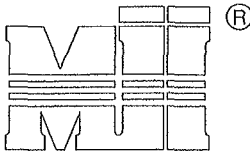


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FEBRUARY 14, 2012

# STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

ST-PIGGY-7-10



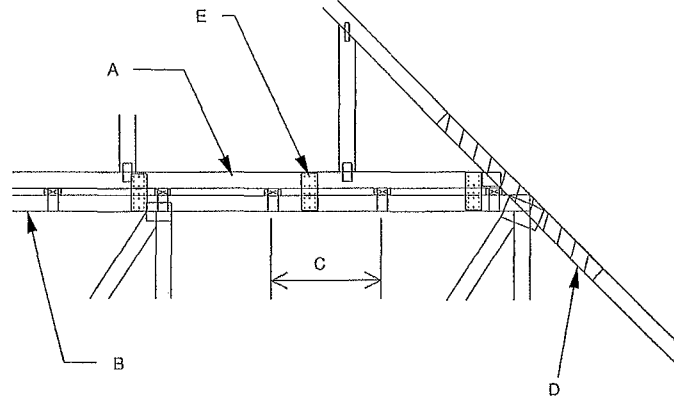
MITek Industries, Inc.

MITek Industries, Chesterfield, MO

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E  
MAX MEAN ROOF HEIGHT = 30 FEET  
MAX TRUSS SPACING = 24" O.C.  
CATEGORY II BUILDING  
EXPOSURE B or C  
ASCE 7-10  
DURATION OF LOAD INCREASE 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES  
TRANSFERING DRAG LOADS (SHEAR TRUSSES)  
ADDITIONAL CONSIDERATIONS BY BUILDING  
ENGINEER/DESIGNER ARE REQUIRED

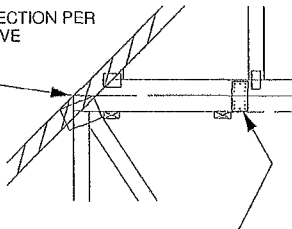
- A PIGGYBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING SHALL BE CONNECTED TO EACH PURLIN WITH (2) 0.131" X 3.5" TOE NAILED
- B BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING
- C PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) 0.131" X 3.5" NAILS EACH.
- D 2" X 4'-0" SCAB, SIZE AND GRADE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF 0.131" X 3" NAILS @ 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:
1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
  2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.
- E FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C. W/ (4) 0.131" X 1.5" PER MEMBER STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5" EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)



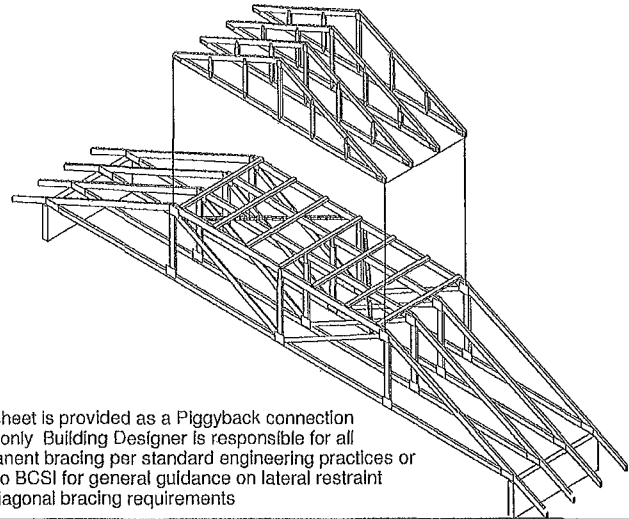
## WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.

SCAB CONNECTION PER NOTE D ABOVE

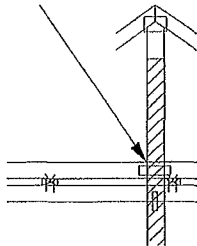


FOR ALL WIND SPEEDS ATTACH MITEK 3X6 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) 0.131" X 1.5" PER MEMBER STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE.



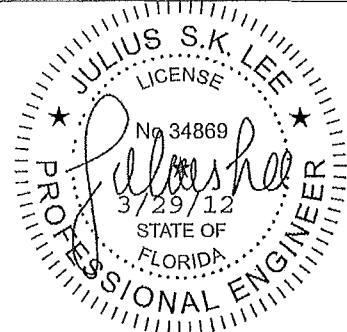
This sheet is provided as a Piggyback connection detail only. Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements.

VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK



## FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB

- 1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.
- 2) ATTACH 2" X 4'-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
- 3) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1'15") REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
- 4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
- 5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN

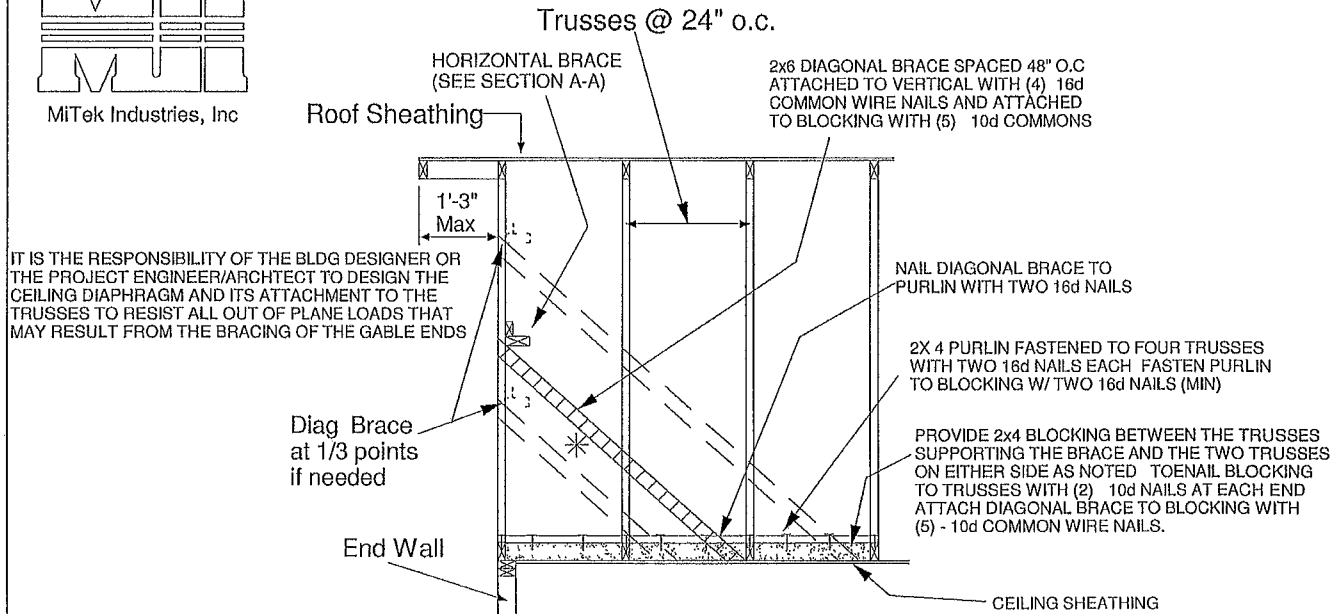


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## ALTERNATE DIAGONAL BRACING TO THE BOTTOM CHORD



## BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

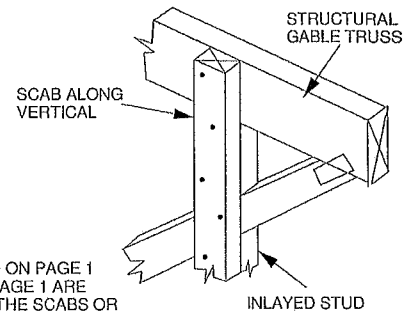
STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED

METHOD 1 ATTACH A MATCHING GABLE TRUSS TO THE INSIDE FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE FOLLOWING NAILING SCHEDULE

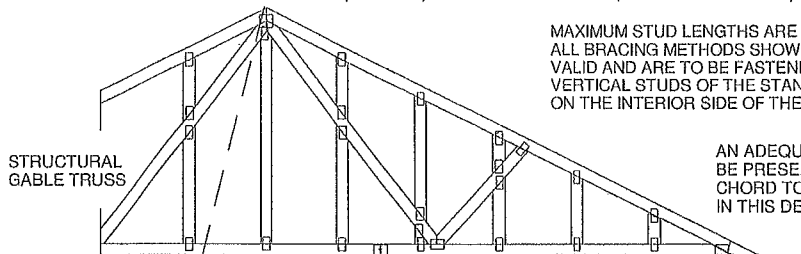
METHOD 2 ATTACH 2X SCABS TO THE FACE OF EACH VERTICAL MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE AND SPECIES AS THE TRUSS VERTICALS

## NAILING SCHEDULE

- FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7 10) OR LESS, NAIL ALL MEMBERS WITH ONE ROW OF 10d (131" X 3") NAILS SPACED 6' O.C.
- FOR WIND SPEEDS GREATER 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7 10) NAIL ALL MEMBERS WITH TWO ROWS OF 10d (131" X 3") NAILS SPACED 6' O.C. (2X 4 STUDS MINIMUM)

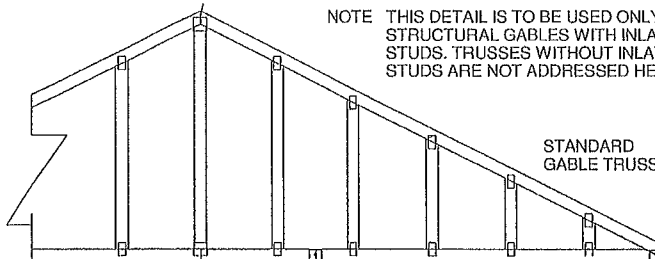


MAXIMUM STUD LENGTHS ARE LISTED ON PAGE 1. ALL BRACING METHODS SHOWN ON PAGE 1 ARE VALID AND ARE TO BE FASTENED TO THE SCABS OR VERTICAL STUDS OF THE STANDARD GABLE TRUSS ON THE INTERIOR SIDE OF THE STRUCTURE.



AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL STUDS ONLY.

NOTE: THIS DETAIL IS TO BE USED ONLY FOR STRUCTURAL GABLES WITH INLAVED STUDS. TRUSSES WITHOUT INLAVED STUDS ARE NOT ADDRESSED HERE.



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