



BUILDING CODE COMPLIANCE OFFICE (BCCO)
PRODUCT CONTROL DIVISION

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

Ceco Door Products
9159 Telecom Drive
Milan, TN 38358

out swing

SCOPE:

This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by Miami-Dade County Product Control Division and accepted by the Board of Rules and Appeals (BORA) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Division (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BORA reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Division that this product or material fails to meet the requirements of the applicable building code.

This product is approved as described herein, and has been designed to comply with the High Velocity Hurricane Zone of the Florida Building Code.

DESCRIPTION: Series "Regent" & "Omega" 18 ga. 3⁰-7⁰ Outswing Commercial Steel Door

APPROVAL DOCUMENT: Drawing No. RD0087, titled "3-0 x 7-0 Series", sheets 1 through 7 of 7, dated 5/30/97 with revision C dated 2/24/00, prepared by the manufacturer, bearing the Miami-Dade County Product Control Renewal stamp with the Notice of Acceptance number and expiration date by the Miami-Dade County Product Control Division.

MISSILE IMPACT RATING: Large and Small Missile Impact

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

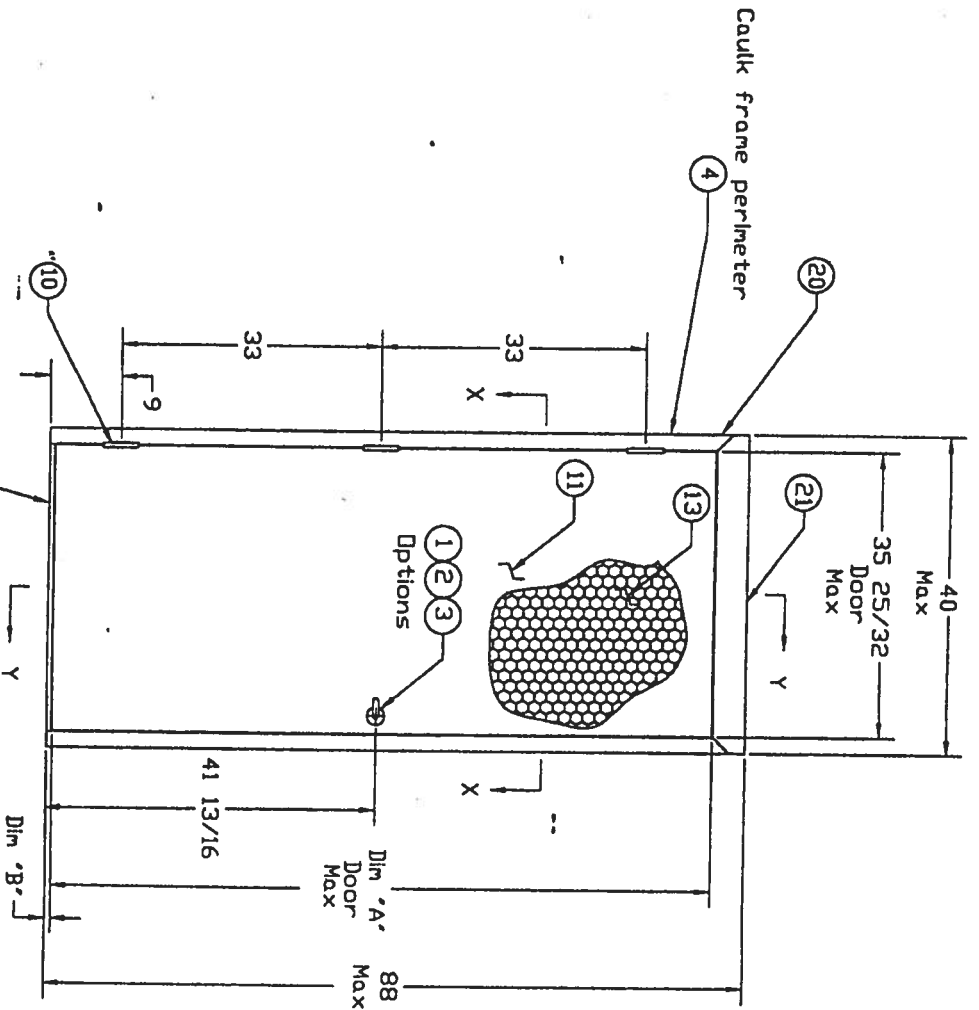
ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.

This NOA renews NOA # 00-0315.03 and consists of this page 1 as well as approval document mentioned above. The submitted documentation was reviewed by Manuel Perez, P.E.



NOA No 03-0411.01
Expiration Date August 14, 2008
Approval Date: May 15, 2003
Page 1



Design Pressure		
Tested For Water Penetration		
With Overhang	+85 psf	-60 psf
Without Overhang	+60 psf	-60 psf

	Dim 'A'	Dim 'B'
3/4" Undercut	83 1/8	3/4
3/8" Undercut	83 1/2	3/8

4
Caulk Underneath Threshold

Sheet 2	Frame Anchor Installation
Sheet 3	Threshold Installation
Sheet 3	Weatherstrip Installation
Sheet 4	Door Latch Reinforcement
Sheet 5-6	Cross Section View
Sheet 7	Bill Of Material

PRODUCT REVIEWED
as compliant with the Florida
Building Code
Acceptance No. 03-0411-01
Expiration Date 06/15/2008
By: Michael J. J. J.
Michael J. J. J.
Florida Product Control

MATERIAL SPECIFICATIONS:

Finish: Rust Inhibitive Primer

3-0 x 7-0 Series
Elevation Drawing

CECD DOOR PRODUCTS
Millon, Tennessee 38358

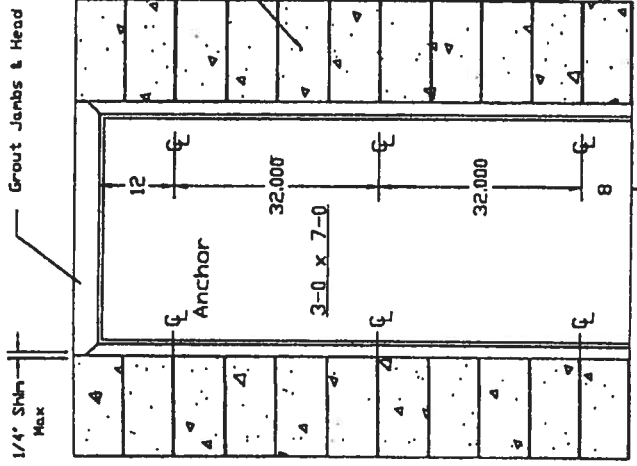
APPROVED AS COMPLYING WITH THE
SOUTH FLORIDA BUILDING CODE
DATE: 02/02/00
BY: Michael J. J. J.
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315-03

2/2/00 Revised Form, Transferred
Information from NDA
7/22/97 Revised Sheet Numbers

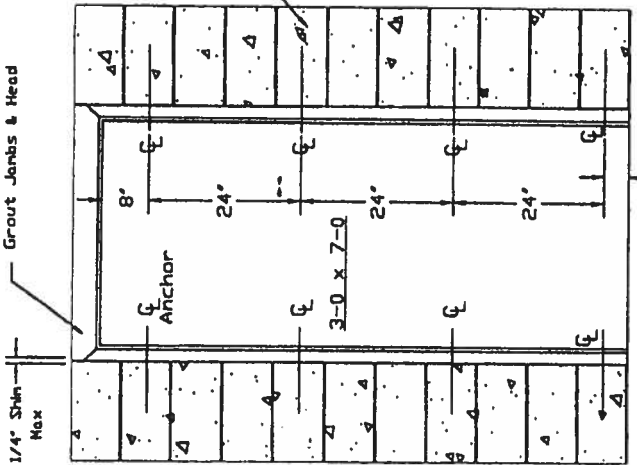
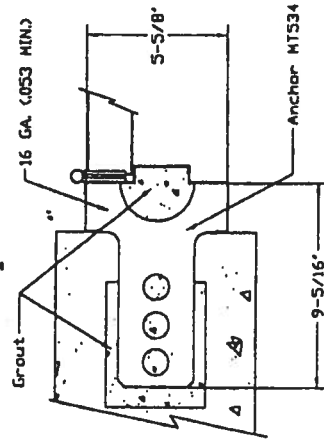
DRAWN BY: DATE:
GWS 5/30/97

ISSUE: REVISIONS

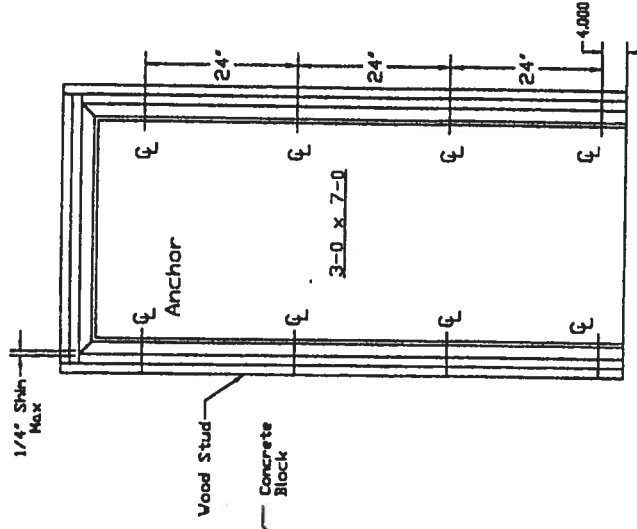
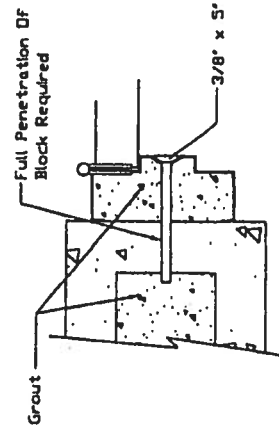
DRAWING NUMBER:
RD00087
Sheet 1 of 7



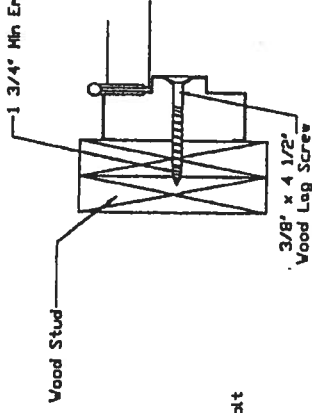
Masonry 7" Anchor



Existing Opening Anchor Into Block



Existing Opening Anchor Into Wood Stud



PRODUCT REVIEWED
as complying with the Florida
Building Code
Acceptance No. 03-041, 01
Expiration Date 03/31/08
By *Michael Dwyer*
Miami/Dade Product Control
Division

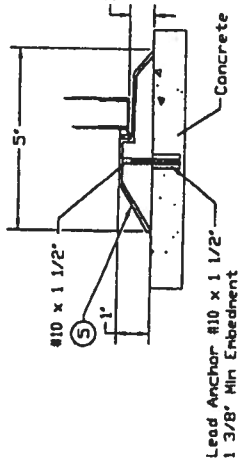
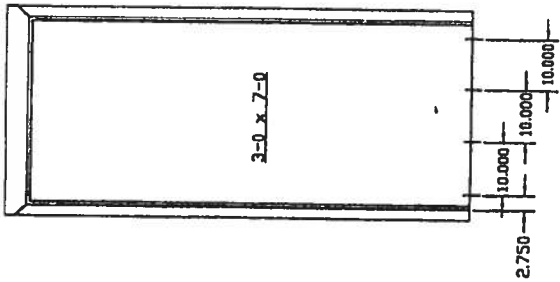
APPROVED AS COMPLYING WITH THE
SOUTH FLORIDA BUILDING CODE
DATE *June 08, 2008*
BY *Michael Dwyer*
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. *00-0315-03*

Revised Format, Transferred Information from NOA	Revised Sheet Number
7/23/07	7/23/07
ISSUE	REVISIONS
DRAWN BY: GWS	DATE: 5/30/97
DRAWING NUMBER: RD00087	Sheet 2 of 7

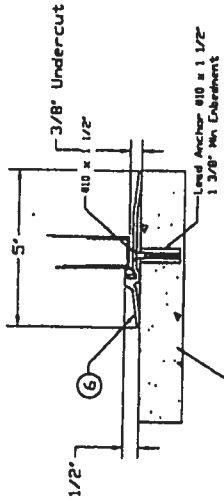
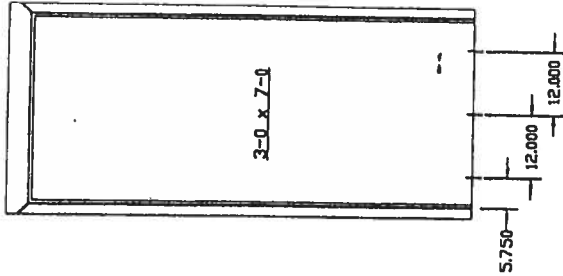
MATERIAL SPECIFICATIONS:

Frame Anchor
Installation Details
CECO DOOR PRODUCTS
Milton, Tennessee 38958

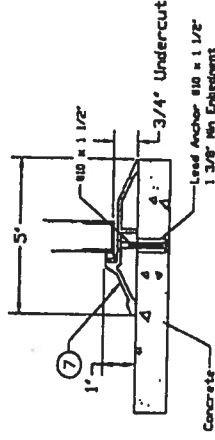
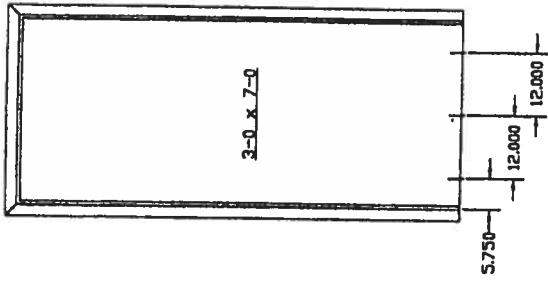
NOTES:
1. SEE SHEET 7 FOR BILL OF MATERIALS



Threshold: National Guard 803S

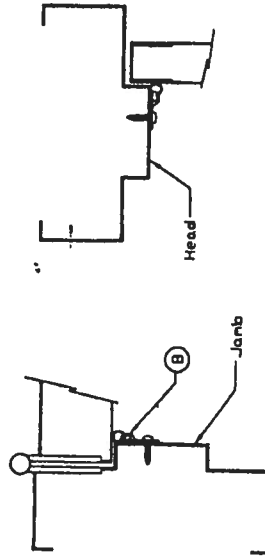


Threshold: Penko 2005AV



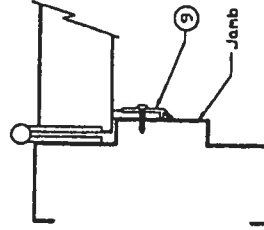
Threshold: Penko 181AX

NOTE: 1. All thresholds shown are made from extruded aluminum with slide-in vinyl weatherstrip insert.



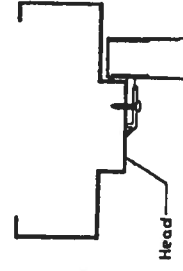
Weatherstrip: Penko 303AS

NOTE: 2. LOCATION: ALONG THE ENTIRE HEAD AND JAMB PERIMETER. ATTACHED WITH THIRTY FOUR (34) #8 X 3/4" PPH SMS SPACED AT 6" O/C.



Weatherstrip: National Guard 130NA

NOTE: 3. LOCATION: ALONG THE ENTIRE HEAD AND JAMB PERIMETER. ATTACHED WITH THIRTY FOUR (34) #8 X 3/4" PPH SMS SPACED AT 6" O/C.



MATERIAL SPECIFICATIONS:

Threshold & Weatherstrip Installation details



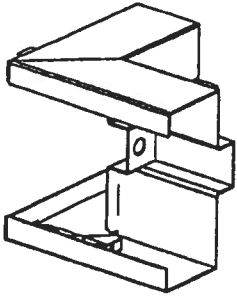
NOTE: 4. See Sheet 7 For Bill of Material

PRODUCT RENEWED
as complying with the Florida
Building Code
Applicants No. 03-041-D1
Certification Date 08/18/2008
By: [Signature]
Miami, Florida Product Counsel
Unitary

APPROVED AS COMPLYING WITH THE
SOUTH FLORIDA BUILDING CODE
DATE: June 08/2000
BY: [Signature]
PRODUCT COUNSEL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315-03

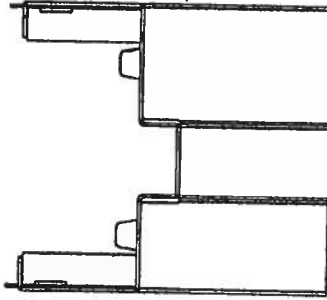
7/23/97	Revised Format, Transferred
7/23/97	Information from NOA
7/23/97	Revised Sheet Number
ISSUE	REVISIONS
DRAWN BY: GWS	DATE: 5/30/97

RD0087
Sheet 3 of 7

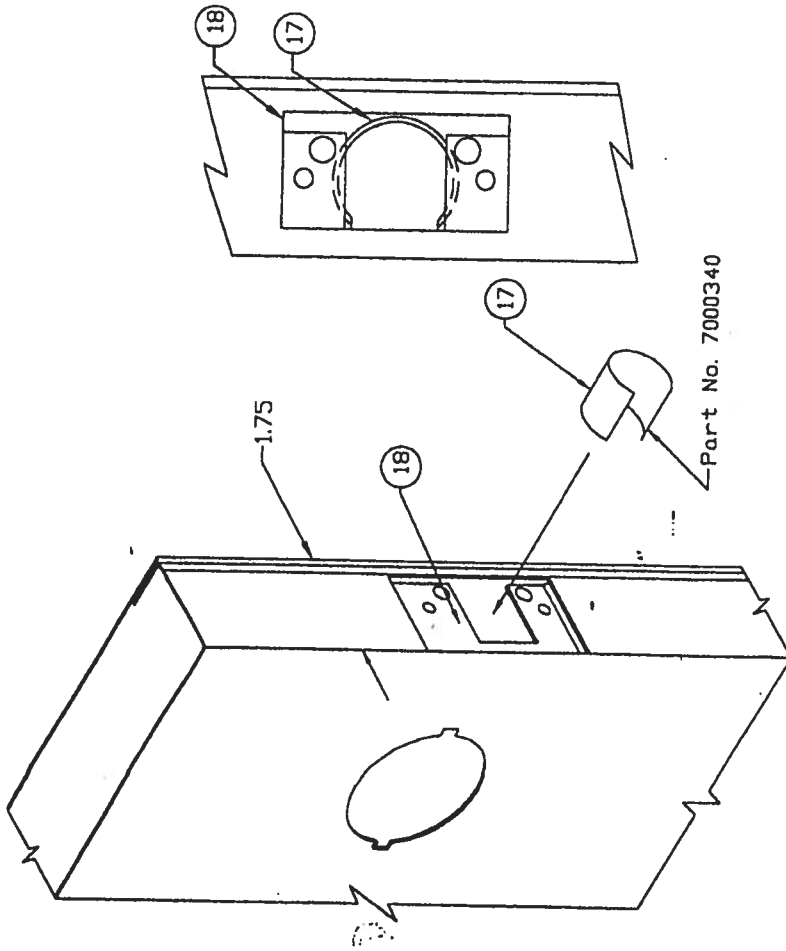


Interlocking Fold Over Tab

Frame Head



Frame Jamb



Note: 1. For Cylindrical Lock Only
2. See Sheet 7 For Bill Of Material

MATERIAL SPECIFICATIONS:

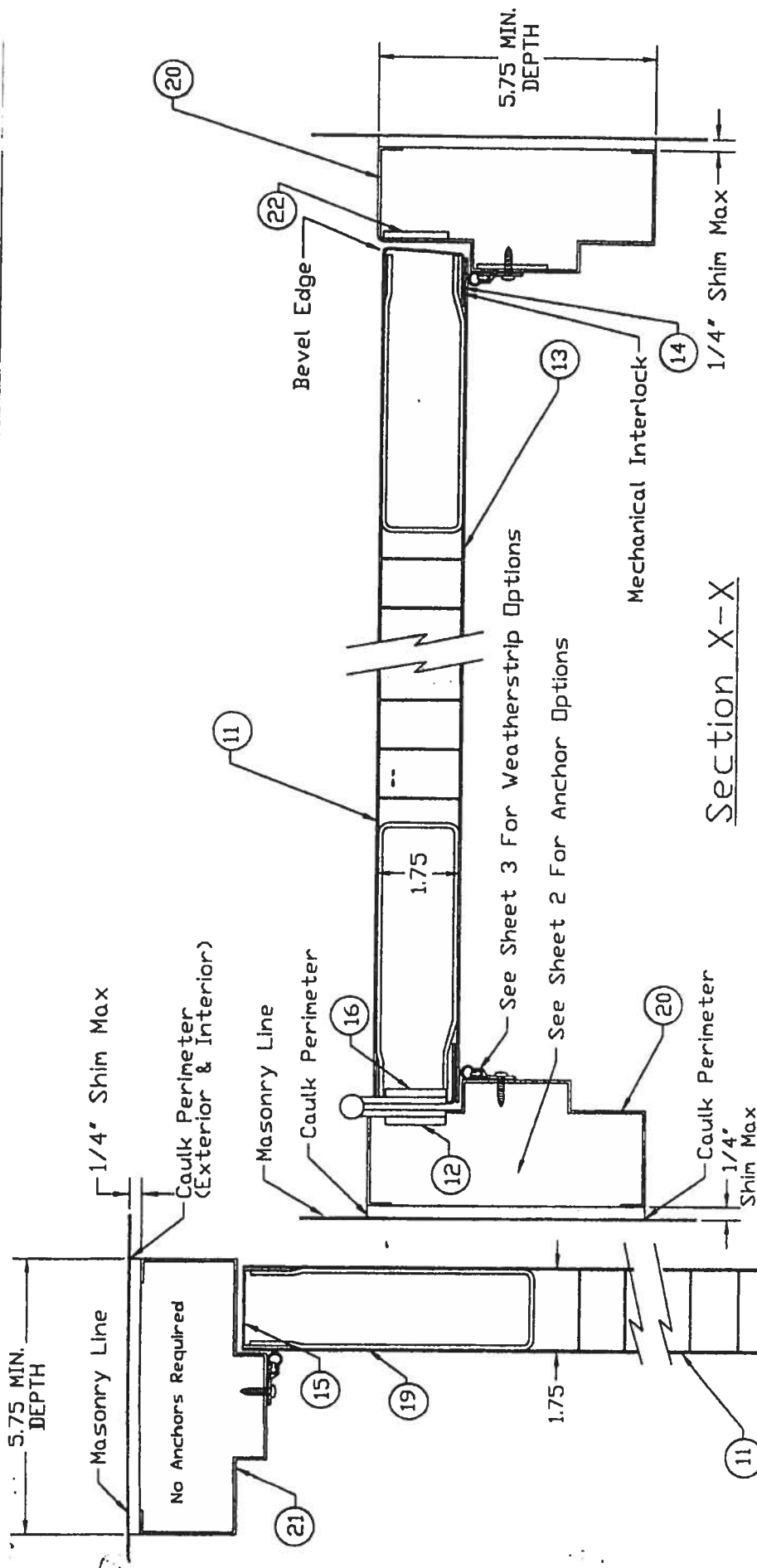
Cylindrical Lock Reinforcement
and "SF" Series Frame Corner
Installation Details

 **CECO DOOR PRODUCTS**
Milan, Tennessee 38358

PRODUCT RENEWED
as complying with the Florida
Building Code
Acceptance No. 03-D41.01
Expiration Date Aug. 13, 2008
By M. M. M. M. M.
Milan, Tennessee
Division

APPROVED AS COMPLYING WITH THE
SOUTH FLORIDA BUILDING CODE
DATE June 08, 2000
BY M. M. M. M. M.
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315-03

2/24/04 GWS	Revised Format, Transferred Information from NOA
7/24/04 GWS	Revised Sheet Number
ISSUE	REVISIONS
DRAWN BY: GWS	DATE: 6/06/97
DRAWING NUMBER: RD0087	
Sheet 4 of 7	



Section X-X

Section Y-Y

Note: See Sheet 7 For Bill Of Material

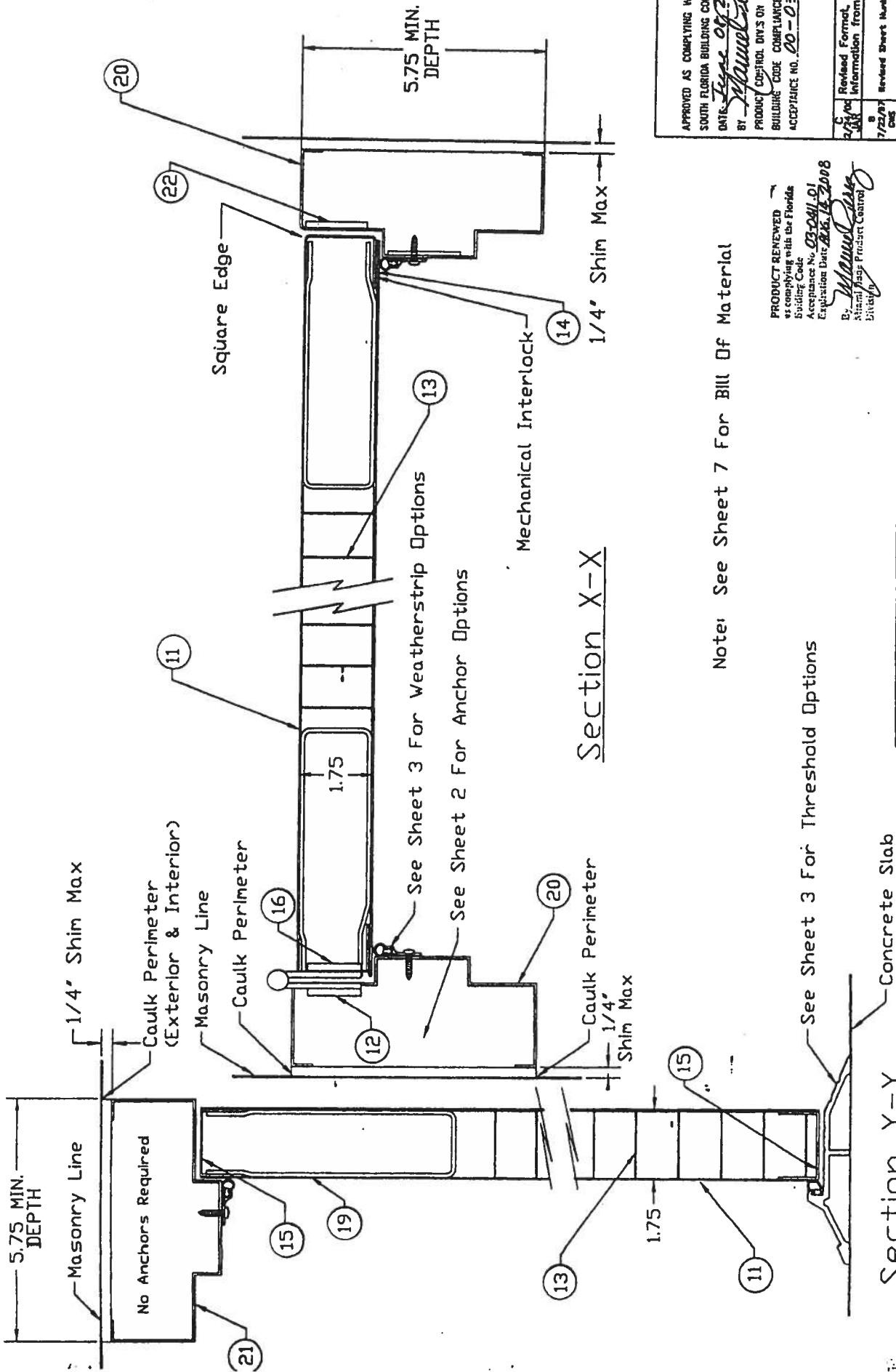
PRODUCT REVIEWED
as complying with the Florida
Building Code
Acceptance No. 03-0411.01
Expiration Date 03-19-2008
By: *Manuel*
Miami / Code Product Control

APPROVED AS COMPLYING WITH THE
SOUTH FLORIDA BUILDING CODE
DATE *June 08 2000*
BY *Manuel*
FEDERAL CHURCH DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0311.03

Revised Form, Transferred
Information from NDA
7/22/97
Revised Sheet Number

ISSUE		REVISIONS	
DRAWN BY:	GWS	DATE:	5/30/97
DRAWING NUMBER:		RD00087	
		Sheet 5 of 7	

MATERIAL SPECIFICATIONS:		Cross Section View	
		Regent Door	
		CECO DOOR PRODUCTS	
		Milan, Tennessee 38358	



Note: See Sheet 7 For Bill Of Material

PRODUCT REVIEWED
as complying with the Florida
Building Code
Acceptance No. 03-041-01
Expiration Date 12/14/2008
By: *Manual*
Miami Pass Product Control
Division

APPROVED AS COMPLYING WITH THE
SOUTH FLORIDA BUILDING CODE
DATE: *May 09/2000*
BY: *Manual*
PRODUCT CONTROL DIV'S ON
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-031-03

7/22/97	7/22/97	7/22/97	7/22/97
Revised	Revised	Revised	Revised
Format	Format	Format	Format
Transferred	Transferred	Transferred	Transferred
Information	Information	Information	Information
from	from	from	from
ROA	ROA	ROA	ROA
Revised	Revised	Revised	Revised
Sheet	Sheet	Sheet	Sheet
Number	Number	Number	Number
ISSUE	ISSUE	ISSUE	ISSUE
REVISIONS	REVISIONS	REVISIONS	REVISIONS
DRAWN BY:	DRAWN BY:	DRAWN BY:	DRAWN BY:
GWS	GWS	GWS	GWS
DATE:	DATE:	DATE:	DATE:
5/30/97	5/30/97	5/30/97	5/30/97
DRAWING NUMBER:	DRAWING NUMBER:	DRAWING NUMBER:	DRAWING NUMBER:
RD0087	RD0087	RD0087	RD0087
Sheet 6 of 7	Sheet 6 of 7	Sheet 6 of 7	Sheet 6 of 7

MATERIAL SPECIFICATIONS:

Cross Section View
Omega Door
CECO DOOR PRODUCTS
Millen, Tennessee 38358

ITEM QTY	DESCRIPTION	MATERIAL	SIZE
1	SCHLAGE SERIES A53PD GRADE 2, LATCH LOCK, SINGLE LEVER OR KNOB OPERATED		
2	MARKS SERIES 170AB GRADE 2, LATCH LOCK, INSIDE/OUTSIDE LEVER OPERATED		
3	YALE SERIES A053070 GRADE 2 LATCH LOCK, SINGLE LEVER OR KNOB OPERATED.		
4	CALLK FOR INSTALLATION AND WEATHERSTRIP ADAPTER SCREWS FRAME PERIMETER (INSIDE & OUT) AND FRAME SILL CORNERS	GE SILICONE HOUSEHOLD SEALANT	
5	NATIONAL GUARD #803S		
6	PERKO #2005AV		
7	PERKO #181AV		
8	1 ROW PERKO #303AS HIGH SURFACE APPLIED EXTRUDED ALUMINUM WEATHERSTRIP ADAPTER WITH A SILICON (TND BULB INSERT		
9	1 ROW NATIONAL GUARD #130NA 1-1/4" WIDE X 0.188" SURFACE APPLIED EXTRUDED ALUMINUM WEATHERSTRIP ADAPT. WITH A FOAM INSERT		
10	3 HAGER #B1279, 4-1/2" X 4-1/2" X .0134" THICK STEEL HINGE EACH ATTACHED WITH EIGHT #12-24 X 1/2" FH MS		
11	1 FACE SHEET CONFORMING TO ASTM A366 AND ASTM-A568	COMMERCIAL QUALITY COLD ROLLED STEEL MINIMUM YIELD STR. OF Fy=36,000 PSD	18 GAUGE .0042" MIN. THICK
12	3 HINGE REINFORCING PLATE, PLATE SPOT WELDED TO FRAME JAMB AT EACH HINGE LOCATION	STEEL	1-1/4" X 9" X 7 GA
13	1 CORE FULL HONEYCOMB CORE PERMANENTLY BONDED TO THE INSIDE OF EACH FACE SKIN WITH NON-FLAMMABLE ADHESIVE	PHENOLIC RESIN-IMPREGNATED KRAFT PAPER	1-1/8" CELL
14	1 DEFLEX 3500 STRUCTURAL ADHESIVE EPOXY		
15	1 ROLL FORMED STEEL CHANNEL ON THE TOP AND BOTTOM OF THE DOOR SPOT WELDED TO EXTERIOR AND GLUED TO INTERIOR SKIN		1" X 1-3/4" X 1" X 16 GA .0053" MIN
16	3 DOOR HINGE REINFORCEMENT		1-1/4" X 9" X 7 GA
17	1 DOOR LATCH REINFORCEMENT	28 GA. GALV.	1-1/4" X 9" X 7 GA
18	1 DOOR LOCK REINFORCEMENT	STEEL	.015" THICK X 1.313 INSIDE DIAMETER
19	1 DOOR CLOSER REINFORCEMENT, ROLLED FORM CHANNELS TACK WELDED TO DOOR END CHANNELS	STEEL	16 GA .0053"
20	2 SERIES "SF", FRAME JAMB DOUBLE RABBIT PROFILE FACE SHEET, CONFORMING TO ASTM A366 AND ASTM-A653	16 GA. .0053" MINJ STEEL	2" FACE, 5-3/4" DEPTH MIN.
21	1 SERIES "SF", FRAME HEAD, DOUBLE RABBIT PROFILE FACE SHEET CONFORMING TO ASTM A366 AND ASTM-A653	COMMERCIAL QUALITY COLD ROLLED STEEL MINIMUM YIELD STR. OF Fy=40,000 PSD	2" FACE, 5-3/4" DEPTH MIN.
22	1 JAMB LOCK STRIKE REINFORCING PLATE	STEEL	1-1/8" X 2-1/2" X 12 GA.

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE DATA <i>Sign 08/2000</i> BY <i>Maunul</i> PRODUCT COMPLIANCE DIVISION BUILDING CODE COMPLIANCE OFFICE ACCEPTANCE NO. <i>00-0317-03</i>		PRODUCT RENEWED as complying with the Florida Building Code Acceptance No. <i>03-0411-01</i> Expiration Date <i>08/20/2008</i> BY <i>Maunul</i> Miami-Dade Fire-Rescue District	
Revised Formet, Transferred Information from NOA 2/2/00 JAK	Revised Sheet Number 2/2/00 GWS	ISSUE 2/2/00 GWS	REVISIONS DATE 6/02/97
MATERIAL SPECIFICATIONS:		3-0 x 7-0 Series Bill Of Materials	
 CECD DOOR PRODUCTS Milan, Tennessee 38358		DRAWING NUMBER: RD0087 Sheet 7 of 7	



BUILDING CODE COMPLIANCE OFFICE (BCCO)
PRODUCT CONTROL DIVISION

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

Ceco Door Products
9159 Telecom Drive
Milan, TN 38358

IN SWING

SCOPE:

This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by Miami-Dade County Product Control Division and accepted by the Board of Rules and Appeals (BORA) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

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This product is approved as described herein, and has been designed to comply with the High Velocity Hurricane Zone of the Florida Building Code.

DESCRIPTION: The Ceco Series Single Flush / Embossed Inswing Commercial Steel Doors -Impact

APPROVAL DOCUMENT: Drawing No RD0728, titled "3-0 x 7-0 , Series Regent, Omega, Imperial, Versa door", prepared by manufacturer, sheets 1 through 9 of 9 dated 05/22/02 and latest revised on 10-10-02, bearing the Miami-Dade County Product Control Approval stamp with the Notice of Acceptance number and approval date by the Miami-Dade County Product Control Division.

MISSILE IMPACT RATING: Large and Small Missile Impact

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

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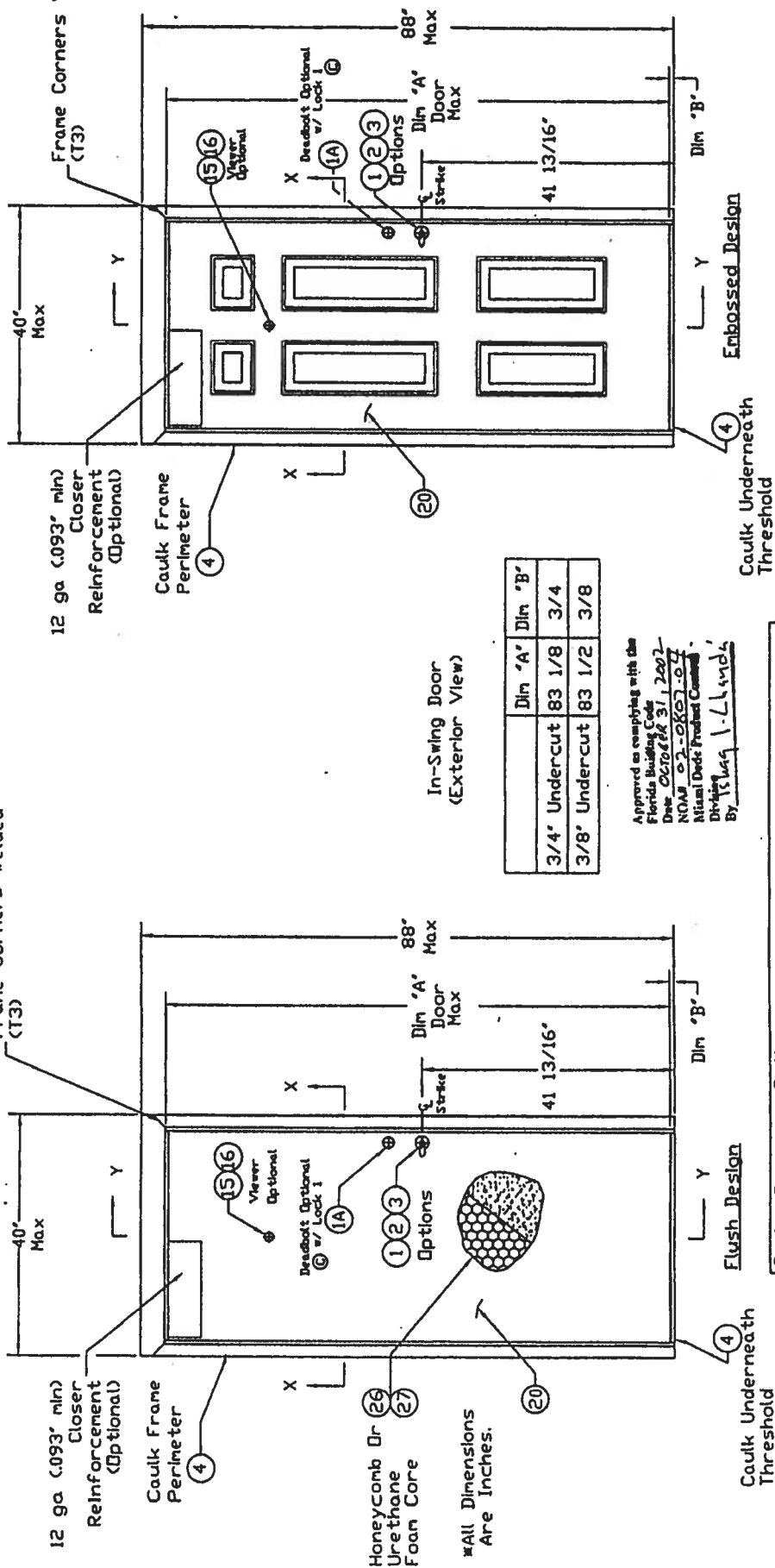
INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.

This NOA consists of this page 1 as well as approval document mentioned above.

The submitted documentation was reviewed by Ishaq I. Chanda, P.E.



NOA No 02-0807.04
Expiration Date: October 31, 2007
Approval Date: October 31, 2002
Page 1



	Dim "A"	Dim "B"
3/4" Undercut	83 1/8	3/4
3/8" Undercut	83 1/2	3/8

Approved as complying with the
Florida Building Code
Date OCTOBER 31, 2002
NONAD 02-0807-04
Miami Dade Product Control
Division
By Isaac L. Lundy

Design Pressure Rating		
	Where Water Infiltration Requirement Is Needed	Where Water Infiltration Requirement Is Not Needed
Positive	Not Approved	+70 PSF
Negative	Not Approved	-70 PSF

Sheet 2	Frame Anchor Installation
Sheet 3	Threshold Installation
Sheet 3	Weatherstrip Installation
Sheet 4	Door Latch Reinforcement
Sheet 5-8	Cross Section View
Sheet 9	Bill Of Material

Notes:

- 1) In-swing Not Approved For Water Infiltration
- 2) This Door Does Not Need A Hurricane Protection System
- 3) Hinge Spacing Is 33" OC, 13" From Top Of Frame & 9" From The Bottom.

MATERIAL SPECIFICATIONS:

Ferri: Rust Inhibitive Primer

3-0 x 7-0 Series

**Regent, Omega, Imperial, & Versadoor
In-Swing Elevation Drawing**

CECO DOOR PRODUCTS
MILAN, Tennessee 38358

B 10/10/02 LT	Revised Per Marked-Up Drawings From Isthaq Chanda.
A 22/02/02 LT	Revised Per Marked-Up Drawings From Isthaq Chanda.

DATE	REVISIONS
11/11/11	1.0

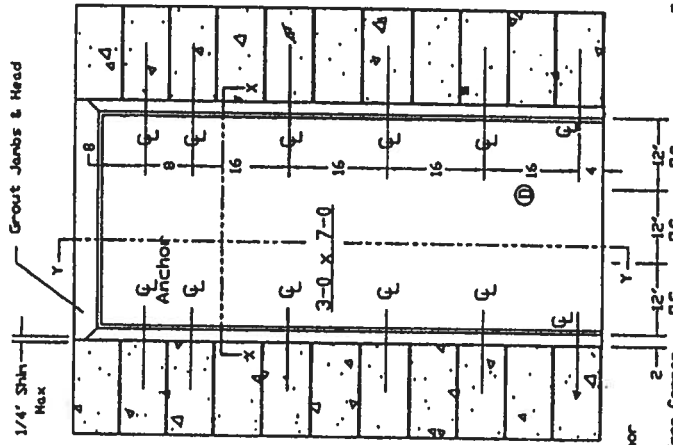
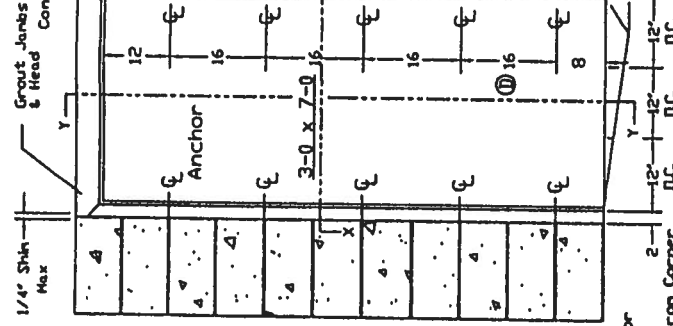
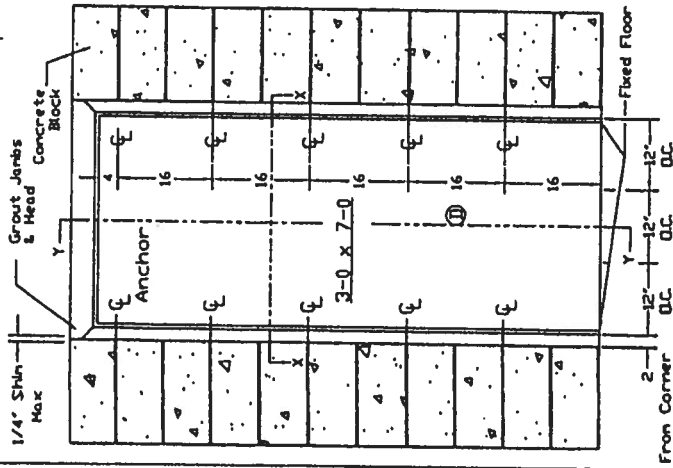
RAWN BY:	DATE:
LT	5/22/02

TRAINING NUMBERS:

RD0728
Sheet 1 of 9

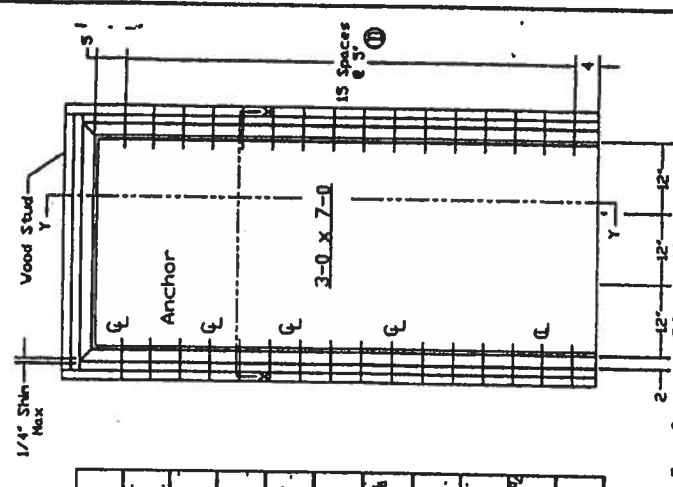
Masonry 'T' Anchor

Min. 3500 PSI



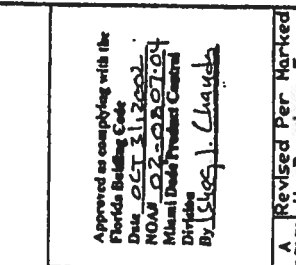
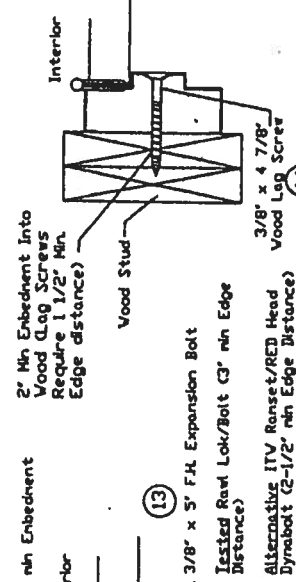
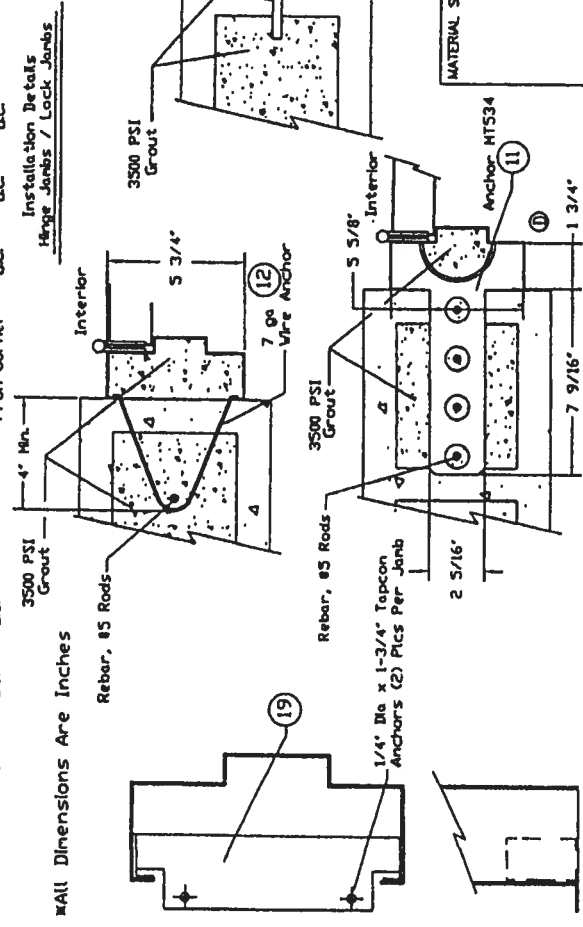
Existing Opening V/Lockbolt or Sleeve Anchor Into Block

Min. 3500 PSI



Existing Opening V/Lockbolt or Sleeve Anchor Into Wood Stud

ALL Dimensions Are Inches



Approved as complying with the Florida Building Code
 Date OCT 31, 2003
 NOAH OCT 31, 2003
 Miami Dade Permit Control
 Division
 By: Susan J. Chiquito

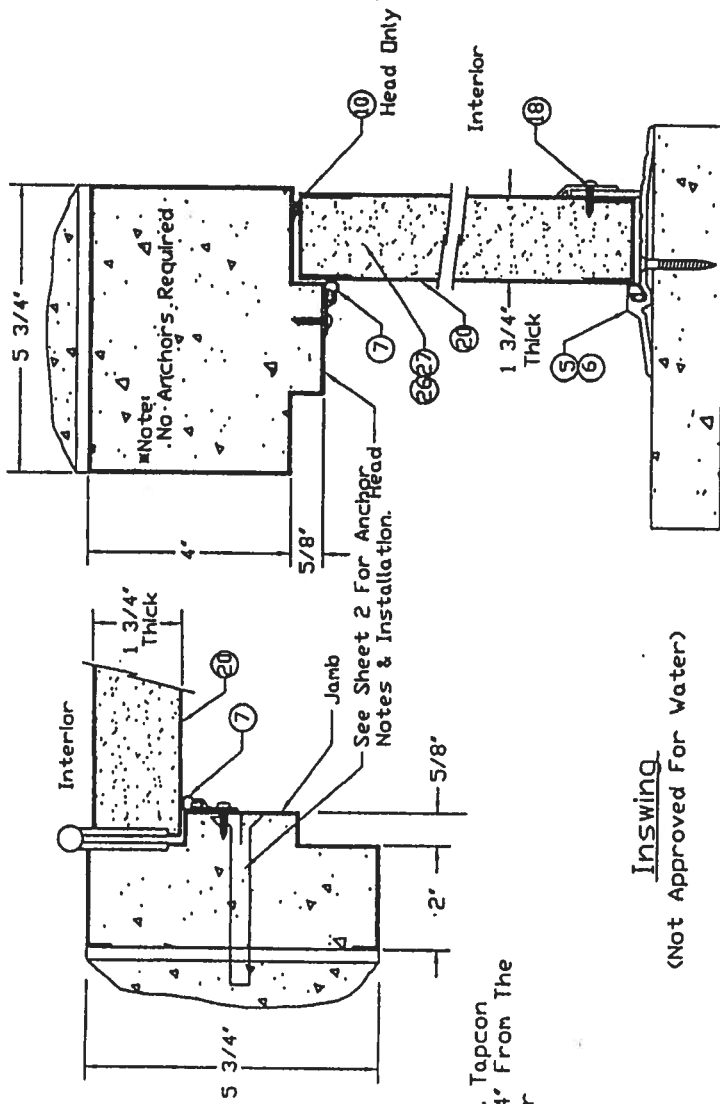
Revised Per Marked
 1/2\"/>

ISSUE
 DRAWN BY: LT
 DATE: 5/22/02

DRAWING NUMBER: RD0728
 Sheet 2 of 9

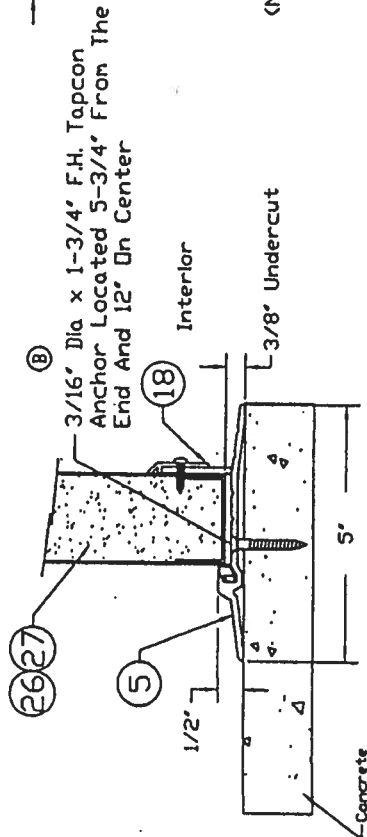
MATERIAL SPECIFICATIONS:
 Frame Anchor (Inswing Doors)
 Regent, Omega, Imperial & Versadoor
 Installation Details
 CECCO DOOR PRODUCTS
 Milan, Tennessee 38358

*Note: Structural Member At Header Must Be Designed To Carry 58.3#/ft. load Imposed And Must Be Reviewed By Building Official.

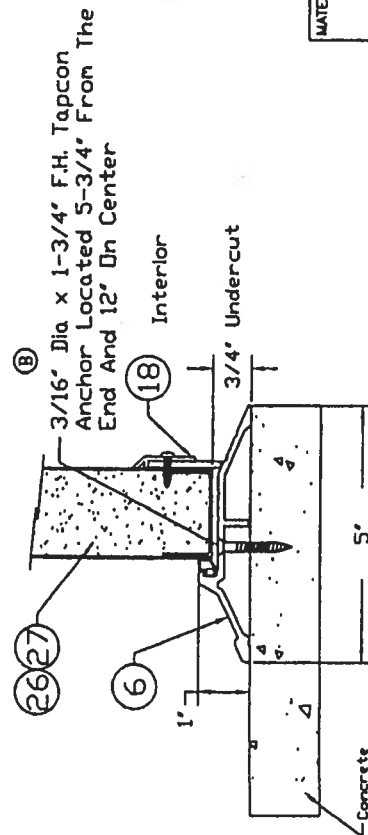


Inswing
(Not Approved For Water)

Note: Thresholds Not Approved For Water.



Threshold: Penko 2005AV

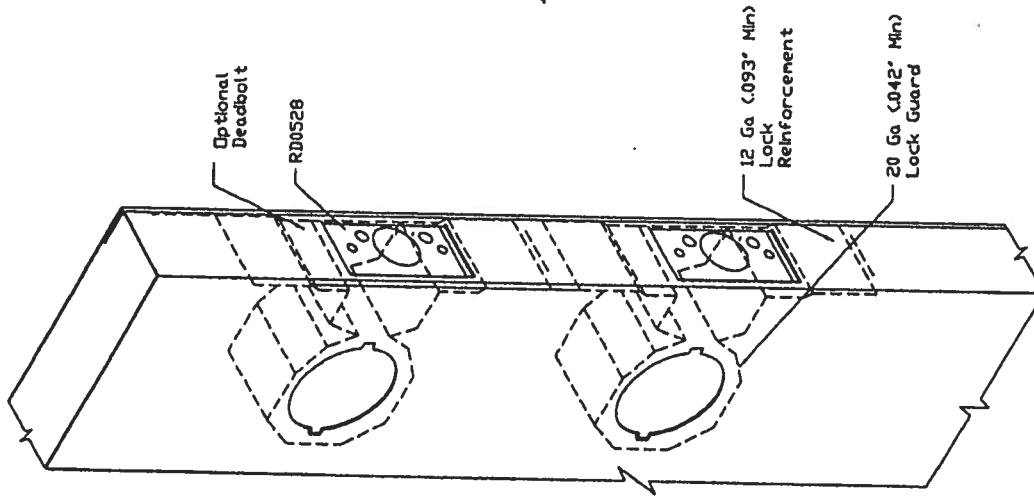


Threshold: Penko 181AV

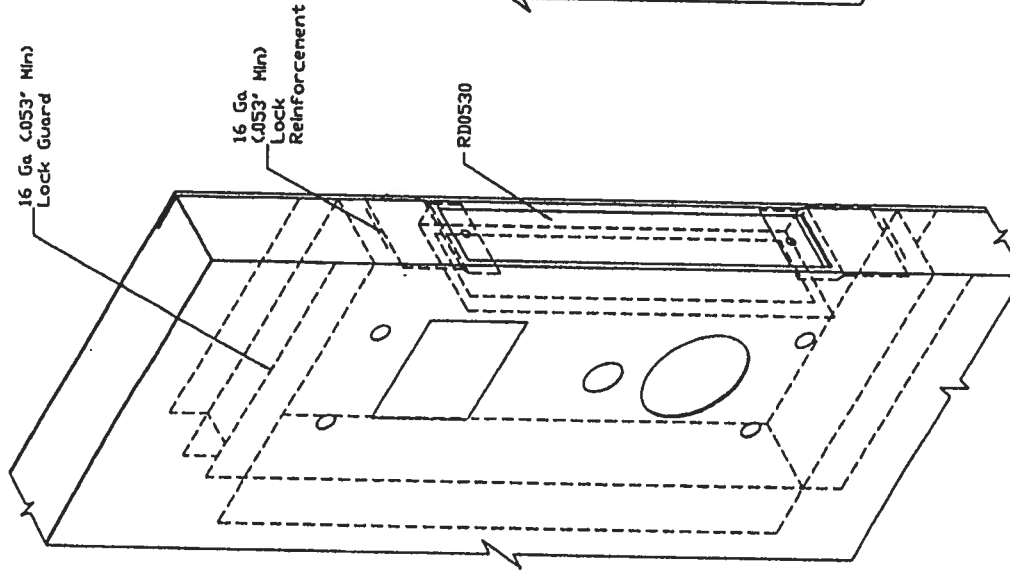
Approved as complying with the
Florida Building Code
Date: OCT 31, 2002
NOA# 02-080703
Missal Desk Product Control
Division
By: [Signature]

B	Revised Per Marked-Up Drawings From Ishag	LT	Change
C	Revised Per Marked-Up Drawings From Ishag	LT	Change
ISSUE	REVISIONS	DATE	
DRAWN BY:	LT	5/22/02	
RD0728			Sheet 3 of 9

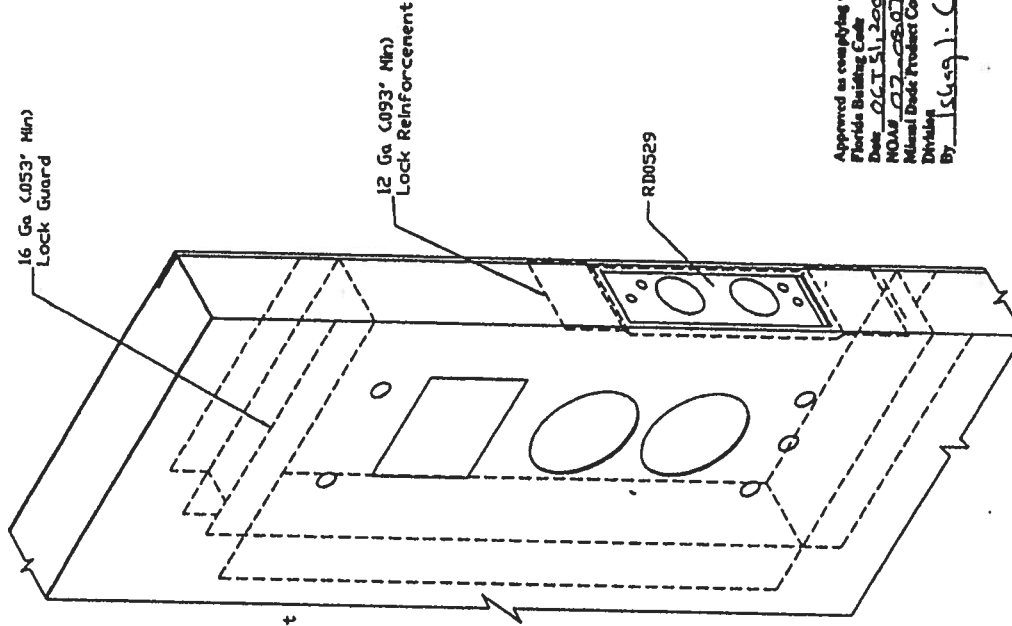
MATERIAL SPECIFICATIONS:	Threshold & Weatherstrip (Inswing Doors) Regent, Omega, Imperial, Versadoor Installation Details
	CECO DOOR PRODUCTS Milan, Tennessee 38358



Schlage AL53PD



Saflok MI



Saflok Premier SL2500

Approved as complying with the
Florida Building Code
Date: 06/15/2002
RD00728-2500-01
Minimal Dead Product Control
Division
By: [Signature]

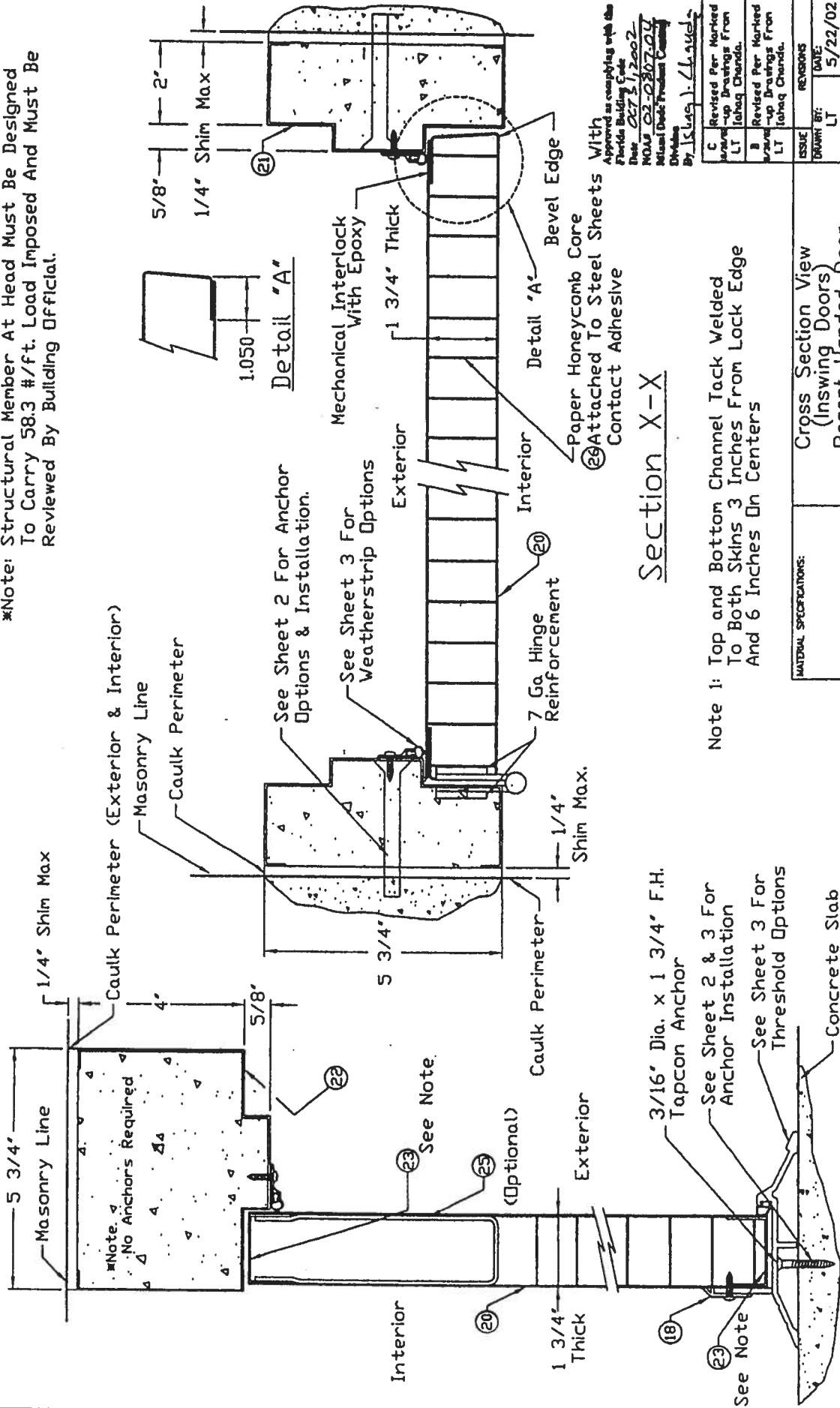
MATERIAL SPECIFICATIONS:

Lock Reinforcement (Inswing Doors)
Regent, Omega, Imperial, Versadoor
Reinforcement Details

CECO DOOR PRODUCTS
Milan, Tennessee 38358

A	Added RD0528, RD0529 & RD0530.	
	LT	REVISIONS
ISSUE	DATE	DATE
DRAWN BY: LT	5/28/02	
DRAWING NUMBER: RD0728		
Sheet 4 of 9		

*Note: Structural Member At Head Must Be Designed To Carry 58.3 #/ft. Load Imposed And Must Be Reviewed By Building Official.



Section X-X

Note 1: Top and Bottom Channel Tack Welded To Both Skins 3 Inches From Lock Edge And 6 Inches On Centers

Section Y-Y

MATERIAL SPECIFICATIONS:

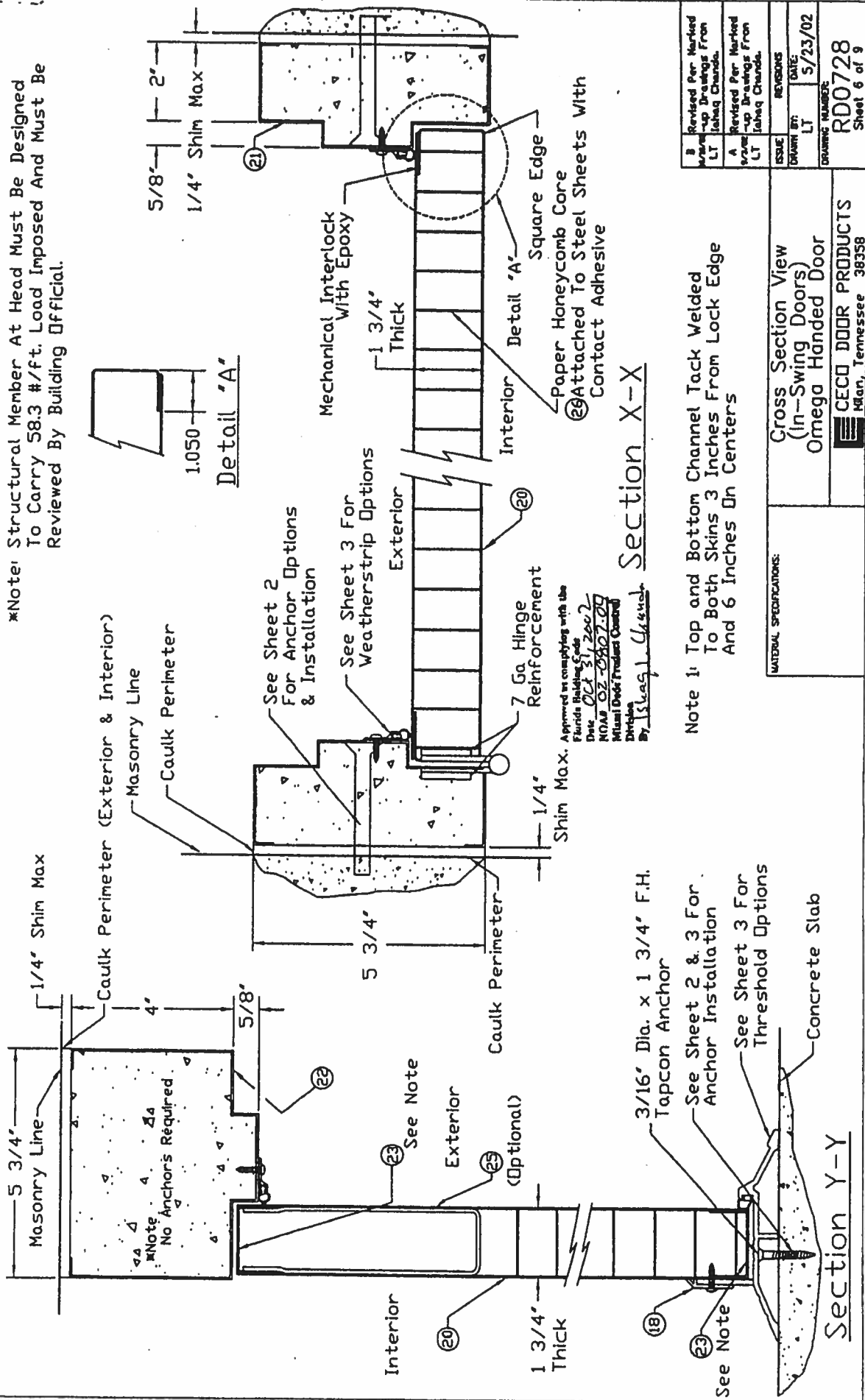
Cross Section View
(Inswing Doors)
Regent Handed Door

CECO DOOR PRODUCTS
Milan, Tennessee 38358

Approved as complying with the
Florida Building Code
Date 02/21/2002
NOAA 03-0207-01
Initial Date Product Change
Division
By 15691-1/1/2002

C	Revised Per Marked Drawings From LT
B	Revised Per Marked Drawings From LT
LT	Initial Date Product Change
ISSUE	REVISIONS
DATE	5/22/02
DRAWING NUMBER	RD0728
SHEET	5 of 9

*Note: Structural Member At Head Must Be Designed To Carry 58.3 #/ft. Load Imposed And Must Be Reviewed By Building Official.



Section X-X

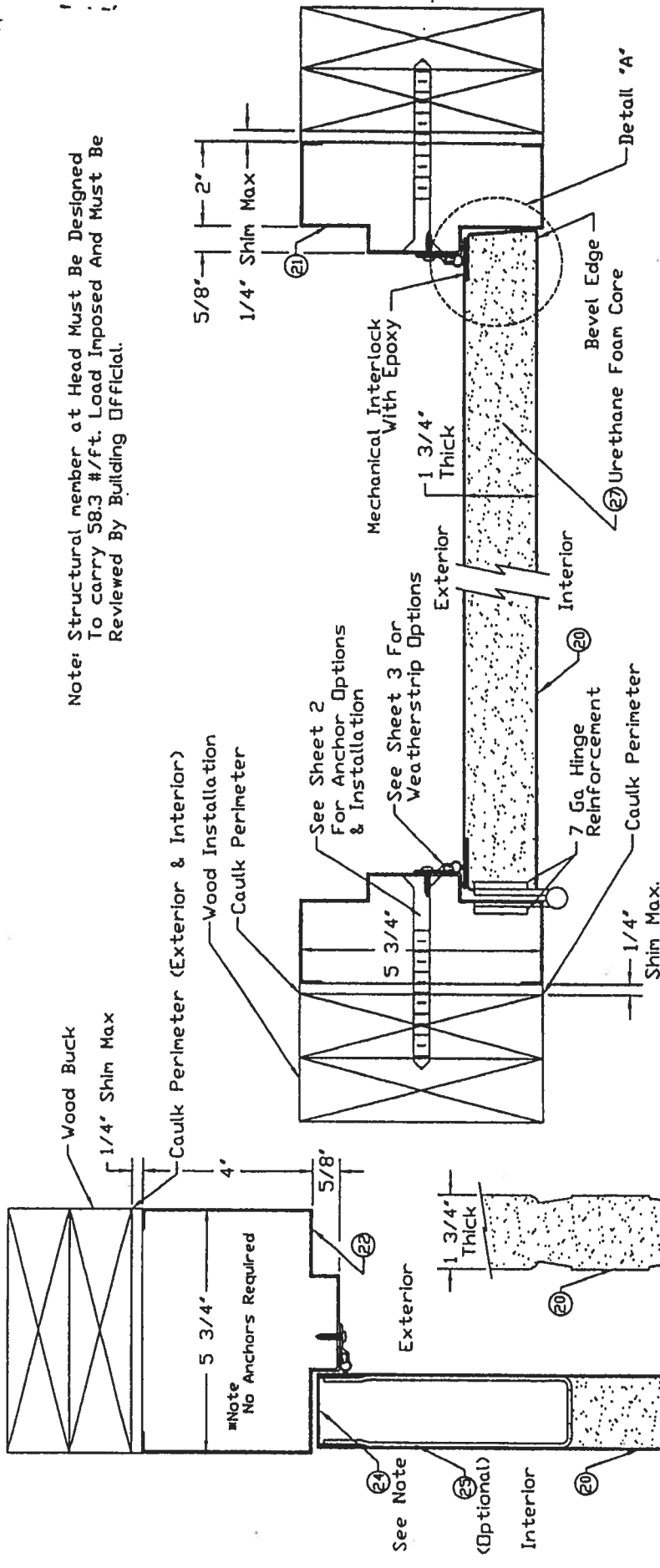
Section Y-Y

Note 1: Top and Bottom Channel Tack Welded To Both Skins 3 Inches From Lock Edge And 6 Inches On Centers

3	Revised Per Marked Drawings From LT Inaug Change.
4	Revised Per Marked Drawings From LT Inaug Change.
DATE	5/23/02
DESIGNED BY	LT
DRAWING NUMBER	RD0728
SHEET	6 of 9

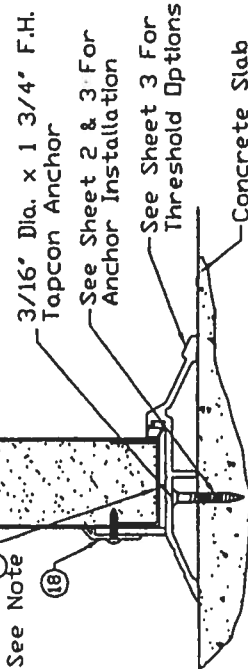
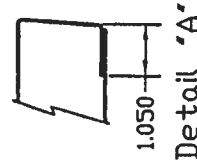
MATERIAL SPECIFICATIONS:	Gross Section View (In-Swing Doors) Omega Handed Door
	CECO DOOR PRODUCTS Kean, Tennessee 38358

Note: Structural member at Head Must Be Designed To Carry 58.3 #/ft. Load Imposed And Must Be Reviewed By Building Official.



Section X-X

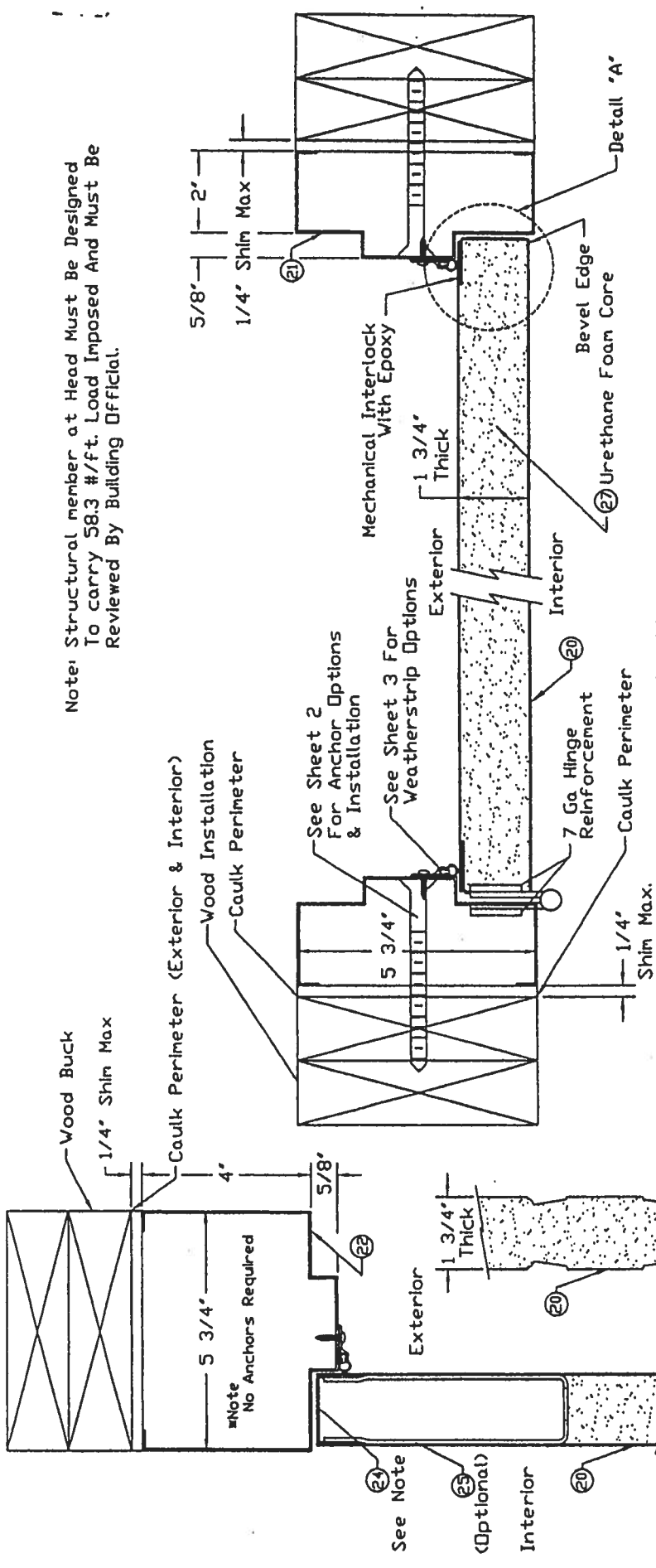
Note: Top & Bottom Channels Assembled Std Method To Skins With Spot Welds & Tape. Channels Are Then Tack Welded To Both Skins 3" From Lock Edge And 6 Inches On Center.



Section Y-Y

GENERAL SPECIFICATIONS	Cross Section View (In-Swing Doors) Imperial Handed Door	
	CECO DOOR PRODUCTS Milan, Tennessee 38358	

Approved as existing with Florida Building Code Date: OCT 31, 2002 FOAB 22-0807-01 Actual Date Product Center By: J. Slag 1. Change		REVISIONS	
8	Revised Per Market- place Up Drawings From LT Ismael Orlando.	DATE:	5/23/02
ISSUE	LT	DATE:	5/23/02
DRAWING BY:		DESIGNED BY:	
RD0728		Sheet 7 of 9	



Note: Structural member at Head Must Be Designed To Carry 58.3 #/ft. Load Imposed And Must Be Reviewed By Building Official.

Note: Top & Bottom Channels Assembled Std Method To Skins With Spot Welds & Tape. Channels Are Then Tack Welded To Both Skins 3" From Lock Edge And 6 Inches On Center.

Section X-X

Section Y-Y

Detail "A"

Cross Section View
(In-Swing Doors)
Versador Handed Door

MATERIAL SPECIFICATIONS:

Approved as complying with the Florida Building Code	
Date: OCT 21, 2002	NOAH 02-0807-00
By: [Signature]	Michael Deane Product Control
Division: [Signature]	By: [Signature]
3 Reduced Per Worked- ness (to Be revised From LT) [Signature]	ISSUE
DATE	REVISIONS
5/23/02	LT
DRAWING NUMBER: RD0728	
Sheet 6 of 9	

CECO DOOR PRODUCTS
Milan, Tennessee 38358

1	Cylindrical Lock & Lock Reinforcement (RD0528)	Schlage	AL53PD
1A	Deadbolt (Optional) ①	Schlage	B100
2	Dr Cylindrical Lock & Lock Reinforcement	Saflok	Premier SL2500
3	Dr Mortise Lock	Saflok	MT
4	Caulk	Dow Corning	899 Silicone Glazing Sealant
5	Threshold	Penko	2005AV36
6	Dr	Penko	18IAV36
7	Weatherstrip	Penko	303AV3684
8	Hinge (Ball Bearing)	Hager or Equal (Attached w/ (8) #12-24 x 1/2 MS Per Hinge)	4-1/2 x 4-1/2 x .134 (Std Weight)
9	Dr (Spring)	Hager or Equal (Attached w/ (8) #12-24 x 1/2 MS Per Hinge)	4-1/2 x 4-1/2 x .134 (Std Weight)
10	Weatherstrip	Penko	S88
11	Frame Anchor	Masonry Tee (RD0057)	16 ga (.053' min) Galv Steel Fynln = 30ksi
12	Dr	Wire, Relaxed Dimension 9' x 8'	#7 (.167' min) Galv Steel Wire (70,000 - 90,000 psi Tensile Strength)
13	Dr	Expansion Bolt	3/8" x 5" F.H. Rawl Lok/Bolt
14	Dr	Wood Lag Screw	3/8" x 4-5/8"
15	Viewer	Hager	1755
16	Dr	MAG Security	8724-C
17	Drip Cap/ Top	Penko	346
18	Sweep	Penko	315 N
19	Floor Anchor	Fixed Floor Anchor	16 ga (.053' min) galvanized Steel
20	Face Sheet A60 Galv Conforming To ASTM A653	Commercial Steel Type B (Minimum Yield Strength 30,000psi)	16 Ga (.053' min)
21	Series SF, Frame Jamb, Double Rabbit Profile, A60 Galv Conforming To ASTM A653	Commercial Steel Type B (Minimum Yield Strength 30,000psi)	2' Face, 5-3/4' Depth Min. (RD0033)
22	Series SF, Frame Head, Double Rabbit, Profile A60 Galv Conforming To ASTM A653	Commercial Steel Type B (Minimum Yield Strength 30,000psi)	4' Face, 5-3/4' Depth Min. (RD0033)
23	Door Channels/ Spot Welded To Bottom Skin	16 Ga (.053' min) A60 Galv Conforming To ASTM A653	16 ga (.053' min) x 1' x 1-3/4' x 1'
24	Door Channels/ Spot Welded To Both	Commercial Steel Type B (Minimum Yield Strength 30,000psi)	16 ga (.053' min) x 1' x 1-3/4' x 1'
25	Taped To Top Skin/ Jack Welded To Both	Commercial Steel Type B (Minimum Yield Strength 30,000psi)	12 ga (.093' min) x 5-3/8' x 16'
26	Closer Reinforcement (Optional)	12 Ga (.093' min) CS Type B	1.2' Nominal Cell Size
27	Honeycomb Core	Non-impregnated Kraft Paper (E)	2 lb/ft ³ Density
	Urethane Core	Foam Enterprises	

Approved as complying with the
Florida Building Code
Date Oct 31, 2002
NOAH 02-0802-007
Milan Door Product Control
Division
By SL/ea J.C. Gaud

B	Revised Per Marked- 10/10/02 Up Drawings From LT	Revisions
A	Revised Per Marked- 9/4/02 Up Drawings From LT	Revisions
ISSUE	DRAWN BY: LT	DATE: 5/28/02
	DRAWING NUMBER:	
		RD0728
		Sheet 9 of 9

MATERIAL SPECIFICATIONS:

3-0 x 7-0 Series

In-Swing Bill Of Materials

 **CECO DOOR PRODUCTS**
Milan, Tennessee 38358



Architectural Testing

**ANSI/AAMA/NWWDA 101/I.S.2-97
TEST REPORT**

Rendered to:

MI HOME PRODUCTS, INC.

**SERIES/MODEL: 480/680/880 Drop-in
PRODUCT TYPE: Aluminum Horizontal
Sliding Window (XO-Fin)**

Title	Results	
	Test Specimen #1	Test Specimen #2
Rating	HS-C30 71 x 71	HS-C40 71 x 59
Operating Force	11 lbf max.	14 lbf max.
Air Infiltration	0.11 cfm/ft ²	0.09 cfm/ft ²
Water Resistance Test Pressure	5.3 psf	6.0 psf
Uniform Load Deflection Test Pressure	± 30.0 psf	+ 45.0 psf -47.2 psf
Uniform Structural Load Test Pressure	± 45.0 psf	+ 67.5 psf -70.8 psf
Forced Entry Resistance	Grade 10	Grade 10

Reference should be made to ATI Report Identification No. 01-47320.03 for complete test specimen description and data.

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com



Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
P.O. Box 370
650 West Market Street
Gratz, Pennsylvania 17030-0370

ATI Report Identification No.: 01-47320.03

Test Dates: 10/07/03

Through: 10/08/03

And: 12/01/03

And: 12/15/03

And: 03/17/04

Report Date: 04/16/04

Expiration Date: 10/07/07

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to witness testing on two Series/Model 480/680/880 Drop-in, aluminum horizontal sliding windows at MI Home Products, Inc. test facility in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for the following ratings: Test Specimen #1: HS-C30 71 x 71; Test Specimen #2: HS-C40 71 x 59. Test specimen description and results are reported herein.

Test Specification: The test specimens were evaluated in accordance with ANSI/AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors*.

Test Specimen Description:

Series/Model: 480/680/880 Drop-in

Product Type: Aluminum Horizontal Sliding Window (XO Fin)

Test Specimen #1: HS-C30 71 x 71

Overall Size: 5' 11-7/16" wide by 5' 11" high

Active Sash Size: 2' 11-5/8" wide by 5' 8-3/8" high

Fixed Daylight Opening Size: 2' 8-3/16" wide by 5' 5-5/8" high

Screen Size: 2' 10" wide by 5' 6-1/2" high



Architectural Testing

Test Specimen Description: (Continued)

Weatherstripping:

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
0.250" high by 0.187" backed polypile with center fin	1 Row	Active sash top and bottom rails and fixed meeting rail interlock
0.250" high by 0.187" backed polypile with center fin	2 Rows	Jamb stile

Test Specimen #2: HS-C40 71 x 59

Overall Size: 5' 11-3/8" wide by 4' 11-1/8" high

Active Sash Size: 2' 11-5/8" wide by 4' 8-1/4" high

Fixed Daylight Opening Size: 2' 8-1/4" wide by 4' 5-7/8" high

Screen Size: 2' 10-1/4" wide by 4' 7-1/8" high

Weatherstripping:

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
0.310" high by 0.187" backed polypile with center fin	1 Row	Active sash top and bottom rails
0.250" high by 0.187" backed polypile with center fin	1 Rows	Fixed meeting rail interlock
0.310" high by 0.187" backed polypile with center fin	2 Rows	Jamb stile
0.550" high by 1" by 1" backed polypile pad	1 Pad	Corner of bottom rail and locking stile



Architectural Testing

Test Specimen Description: (Continued)

The following descriptions apply to all specimens.

Finish: All aluminum was white.

Glazing Details: The window utilized 5/8" thick sealed insulating glass constructed from two sheets of 1/8" thick clear annealed glass and a Swiggle spacer system. The lites were interior glazed onto double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

Frame Construction: The frame was constructed of thermally broken extruded aluminum. The corners were secured utilizing three #8 x 1" screws per corner through the jambs into the head and sill screw bosses. End caps were utilized on the ends of the fixed meeting rails and secured with two #8 x 3/4" screws per cap. The meeting rails were then secured to the frame with two #8 x 3/4" screws.

Sash Construction: The sash was constructed of thermally broken extruded aluminum. The corners were secured utilizing one #8 x 1" screw per corner through the head and sill into the jambs screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible vinyl spline.

Hardware:

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
Cam lock	1	One midspan of active panel with integral lock keeper on fixed meeting stile
Roller assembly	2	One each end of bottom rail
Screen constant force spring	2	5" from rails on screen stiles
Screen lift handles	2	5" from rails on screen stiles

Drainage:

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
1-1/4" long by 1/4" wide weepslot with cover	2	3-1/2" from jambs on sill face
1/2" long by 1/8" wide weepslot	2	2" from jambs on sill track

Reinforcement: No reinforcement was utilized.

Installation: The window was installed into a #2 Spruce-Pine-Fir wood buck. The window was secured utilizing #8 x 1-5/8" drywall screws located in corners and 12" on center around nail-fin perimeter. Silicone was utilized around the exterior perimeter.



Architectural Testing

Test Results:

The results are tabulated as follows:

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
<u>Test Specimen #1:</u> HS-C30 71 x 71			
2.2.2.5.1	Operating Force	11 lbf	25 lbf max.
2.1.2	Air Infiltration per ASTM E 283 1.57 psf (25 mph)	0.11 cfm/ft ²	0.3 cfm/ft ² max.
<i>Note #1: The tested specimen meets the performance levels specified in ANSI/AAMA/NWDA 101/I.S. 2-97 for air infiltration.</i>			
2.1.3	Water Resistance per ASTM E 547-00 (with and without screen) 4.50 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 52 seconds) 30.0 psf (positive) 30.0 psf (negative)	0.75" 0.71"	See Note #2 See Note #2
<i>Note #2: The Uniform Load Deflection test is not requirement of ANSI/AAMA/NWDA 101/I.S.2-97 for this product designation. The deflection data is recorded in this report for special code compliance and information only.</i>			
2.1.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds) 45.0 psf (positive) 45.0 psf (negative)	0.13" <0.01"	0.26" max. 0.26" max.
2.2.2.5.2	Deglazing Test per ASTM E 987 In operating direction - 70 lbs		
	Handle stile	0.13"/25%	0.50"/100%
	Lock stile	0.19"/38%	0.50"/100%
	In remaining direction - 50 lbs		
	Top rail	0.09"/19%	0.50"/100%
	Bottom rail	0.06"/13%	0.50"/100%



Architectural Testing

Test Results: (Continued)

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
<u>Test Specimen #1:</u> HS-C30 71 x 71 (Continued)			
2.1.8	Forced Entry Resistance per ASTM F 588		
Type: A	Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 thru A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
<u>Optional Performance</u>			
4.3	Water Resistance per ASTM E 547-00 (with and without screen) 5.3 psf	No leakage	No leakage
<u>Test Specimen #2:</u> HS-C40 71 x 59			
2.2.2.5.1	Operating Force	14 lbf	25 lbf max.
2.1.2	Air Infiltration per ASTM E 283 1.57 psf (25 mph)	0.09 cfm/ft ²	0.3 cfm/ft ² max.
<i>Note #1: The tested specimen meets the performance levels specified in ANSI/AAMA/NWDA 101/I.S. 2-97 for air infiltration.</i>			
2.1.3	Water Resistance per ASTM E 547-00 (with and without screen) 4.50 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 52 seconds) 30.0 psf (positive) 30.0 psf (negative)	0.62" 0.51"	See Note #2 See Note #2
2.1.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds) 45.0 psf (positive) 45.0 psf (negative)	0.03" 0.04"	0.21" max. 0.21" max.



Architectural Testing

Test Results: (Continued)

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
<u>Test Specimen #2:</u> HS-C40 71 x 59 (Continued)			
2.2.2.5.2	Deglazing Test per ASTM E 987 In operating direction - 70 lbs		
	Handle stile	0.13"/25%	0.50"/100%
	Lock stile	0.13"/25%	0.50"/100%
	In remaining direction - 50 lbs		
	Top rail	0.03"/6%	0.50"/100%
	Bottom rail	0.03"/6%	0.50"/100%
2.1.8	Forced Entry Resistance per ASTM F 588		
	Type: A	Grade: 10	
	Lock Manipulation Test	No entry	No entry
	Test A1 thru A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
<u>Optional Performance</u>			
4.3	Water Resistance per ASTM E 547-00 (with and without screen) 6.0 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 52 seconds)		
	45.0 psf (positive)	0.62"	See Note #2
	47.2 psf (negative)	0.54"	See Note #2
4.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	67.5 psf (positive)	0.04"	0.21" max.
	70.8 psf (negative)	0.08"	0.21" max.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years from the original test date. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced except in full without approval of Architectural Testing.

For ARCHITECTURAL TESTING, INC.



Digitally Signed by: Eric Westphal

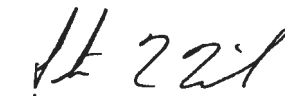
Eric Westphal
Technician

EW:dme
01-47320.03



Digitally Signed by: Steven M. Urich

Steven M. Urich, P. E.
Senior Project Engineer


APRIL 20, 2004

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11:34:27 AM

Licensee Details**Licensee Information**

Name: **JENKINS, MICHAEL CHRISTIAN (Primary Na
JENKINS CONTRACTING LLC (DBA Name)**

Main Address: **PO BOX 1734
LAKE CITY Florida 32056**

County: **COLUMBIA**

License Mailing:

LicenseLocation: **9016 SW CR 240
LAKE CITY FL 32024**

County: **COLUMBIA**

License Information

License Type: **Certified General Contractor**

Rank: **Cert General**

License Number: **CGC1507486**

Status: **Current,Active**

Licensure Date: **07/09/2004**

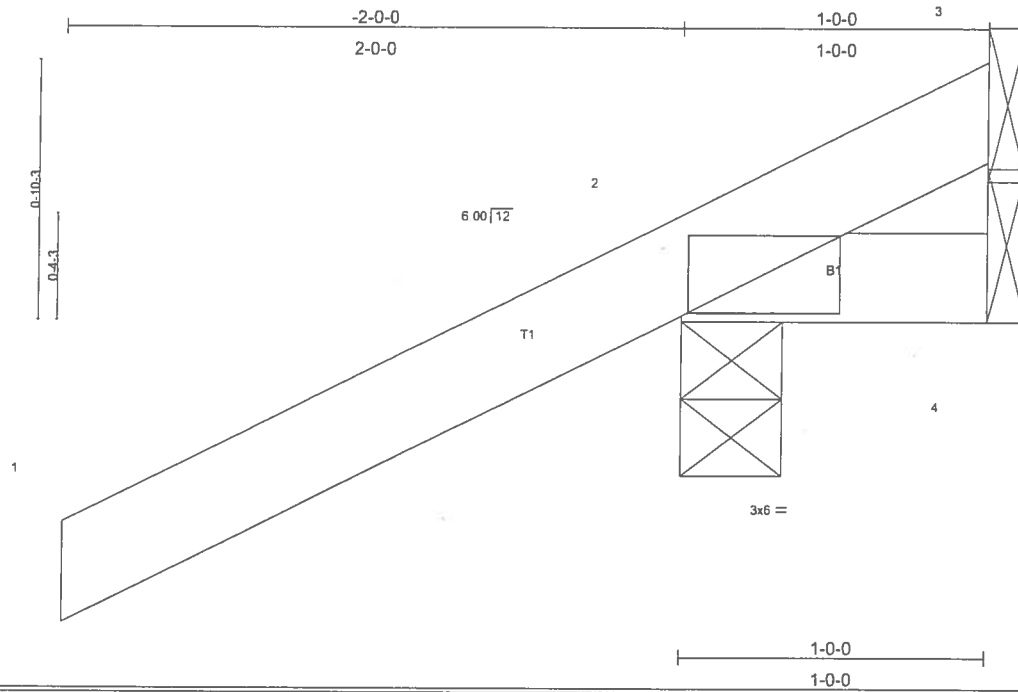
Expires: **08/31/2006**

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Qualifications****Qualification Effective**[View Related License Information](#)[View License Complaint](#)[Terms of Use](#) | [Privacy Statement](#)

Job L161719	Truss CJ1	Truss Type JACK	Qty 5	Ply 1	Job Reference (optional)
-----------------------	---------------------	---------------------------	-----------------	-----------------	--------------------------

Builders FirstSource, Lake City, Fl 32055

6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 24 13:42:10 2006 Page 1



Scale = 1/2" = 1'-0"

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.28	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.01	Vert(TL)	-0.00	2	>999	180		
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)							
								Weight: 7 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2

BRACING
 TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=267/0-4-0, 4=14/Mechanical, 3=91/Mechanical
 Max Horz 2=87(load case 5)
 Max Uplift 2=275(load case 5), 3=91(load case 1)
 Max Grav 2=267(load case 1), 4=14(load case 1), 3=128(load case 5)

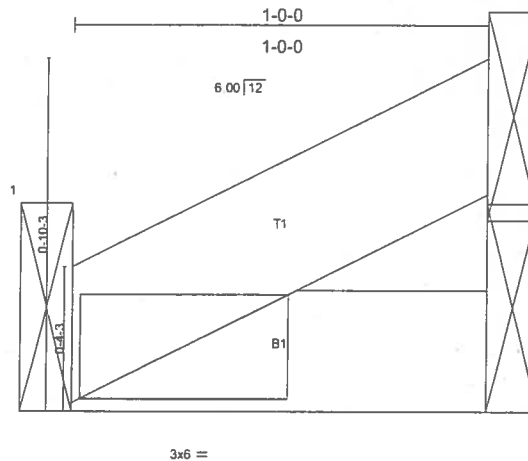
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-69/76
 BOT CHORD 2-4=0/0

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Refer to girder(s) for truss to truss connections.
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 275 lb uplift at joint 2 and 91 lb uplift at joint 3.

LOAD CASE(S) Standard

Job L161719	Truss CJ1A	Truss Type JACK	Qty 1	Ply 1	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 24 13:42:32 2006 Page 1		



Scale = 1/5.3

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.01	Vert(LL)	-0.00	1	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.01	Vert(TL)	-0.00	1	>999	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 3 lb

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2

BRACING
 TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=41/Mechanical, 3=15/Mechanical, 2=26/Mechanical
 Max Horz 1=25(load case 5)
 Max Uplift 1=9(load case 5), 2=25(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-23/10
 BOT CHORD 1-3=0/0

NOTES

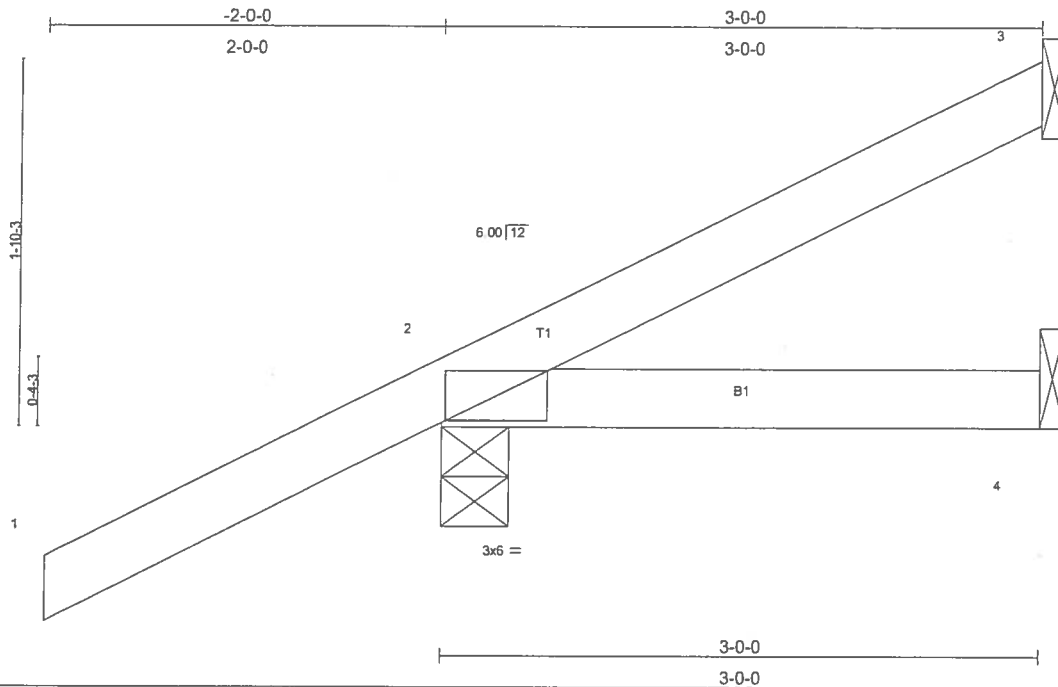
- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Refer to girder(s) for truss to truss connections.
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 9 lb uplift at joint 1 and 25 lb uplift at joint 2.

LOAD CASE(S) Standard

Job L161719	Truss CJ3	Truss Type JACK	Qty 5	Ply 1	Job Reference (optional)
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Builders FirstSource, Lake City, FL 32055

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Scale = 1:11.1

LOADING (psf)	SPACING 2-0-0	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.30	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.05	Vert(LL) -0.00 2-4 >999 240		
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Vert(TL) -0.01 2-4 >999 180		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)	Horz(TL) -0.00 3 n/a n/a		
				Weight: 13 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2

BRACING
 TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=29/Mechanical, 2=279/0-4-0, 4=42/Mechanical
 Max Horz 2=132(load case 5)
 Max Uplift 3=-27(load case 6), 2=-205(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-58/7
 BOT CHORD 2-4=0/0

NOTES

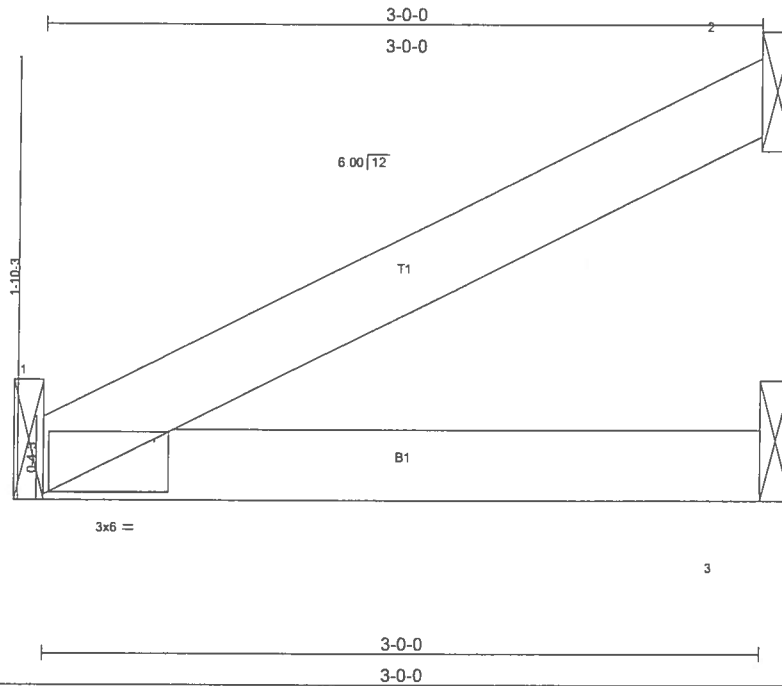
- 1) Wind: ASCE 7-02: 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Refer to girder(s) for truss to truss connections.
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 3 and 205 lb uplift at joint 2.

LOAD CASE(S) Standard

Job L161719	Truss CJ3A	Truss Type JACK	Qty 1	Ply 1	Job Reference (optional)
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Builders FirstSource, Lake City, FL 32055

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LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.12	Vert(LL)	-0.00	1-3	>999	240	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.06	Vert(TL)	-0.01	1-3	>999	180		
BCLL 10.0	Lumber Increase 1.25	WB 0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2004/TP12002								
								Weight: 10 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=121/Mechanical, 2=78/Mechanical, 3=43/Mechanical
Max Horz 1=69(load case 5)
Max Uplift1=-27(load case 5), 2=-72(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

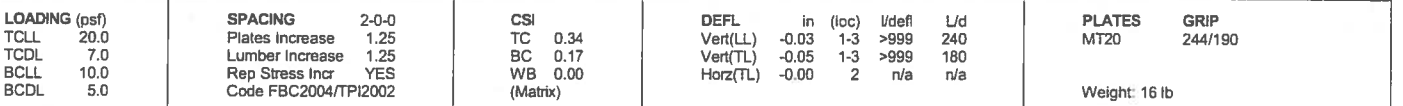
TOP CHORD 1-2=-63/29
BOT CHORD 1-3=0/0

NOTES

- 1) Wind: ASCE 7-02: 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Refer to girder(s) for truss to truss connections.
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 1 and 72 lb uplift at joint 2.

LOAD CASE(S) Standard

APRIL 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549



BRACING	
TOP CHORD	Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-105/48
BOT CHORD 1-3=0/0

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDF=4.2psf; BCDF=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

LOAD CASE(S) Standard

Job L161719	Truss EJ7	Truss Type MONO TRUSS	Qty 29	Ply 1	Job Reference (optional)
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Builders FirstSource, Lake City, FL 32055

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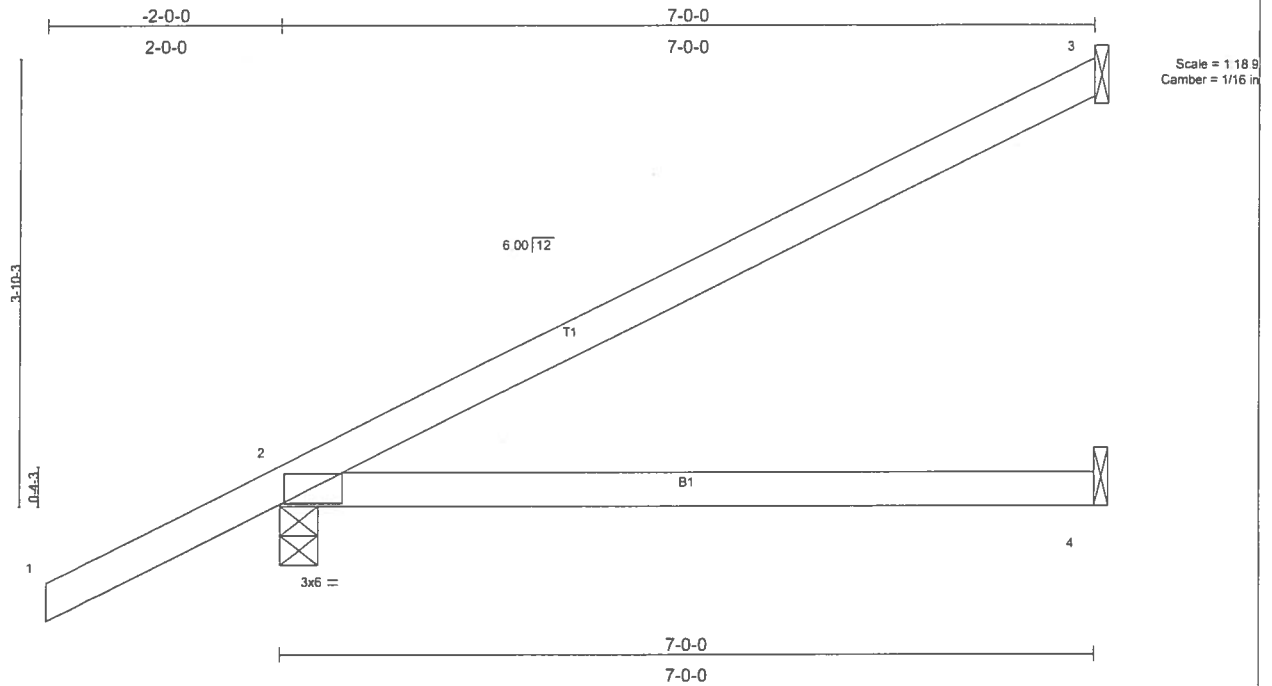


Plate Offsets (X,Y): [2:0-2-12:0-1-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.48	Vert(LL)	-0.12	2-4	>674	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.34	Vert(TL)	-0.20	2-4	>403	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2004/TP12002		(Matrix)						Weight: 26 lb	

LUMBERTOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2**BRACING**TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.**REACTIONS** (lb/size) 3=162/Mechanical, 2=420/0-4-0, 4=104/Mechanical

Max Horz 2=224(load case 5)

Max Uplift 3=133(load case 5), 2=211(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

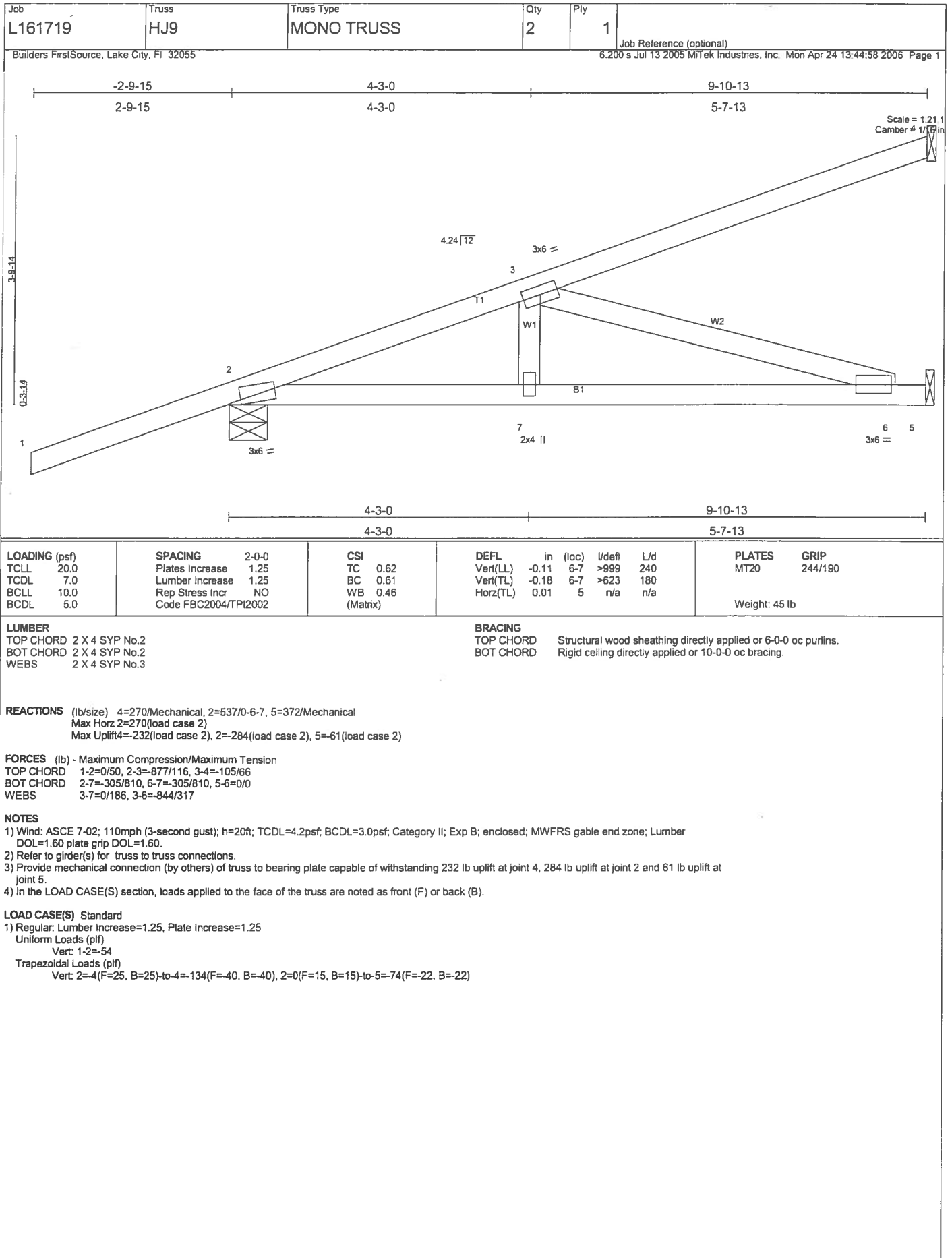
TOP CHORD 1-2=0/47, 2-3=119/58

BOT CHORD 2-4=0/0

NOTES

- 1) Wind: ASCE 7-02: 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Refer to girder(s) for truss to truss connections.
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 133 lb uplift at joint 3 and 211 lb uplift at joint 2.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	Job Reference (optional)
L161719	HJ9A	MONO TRUSS	1	1	
Builders FirstSource, Lake City, FL 32055			6/2005 s Jul 13 2005 Mitek Industries, Inc. Mon Apr 24 13:46:52 2006 Page 1		

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 5-9-6 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 9-5-1 oc bracing.

REACTIONS (lb/size) 1=332/0-6-7, 3=265/Mechanical, 4=398/Mechanical
Max Horz 1=200(load case 2)
Max Uplift 1=70(load case 2), 3=-226(load case 2), 4=-93(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-985/252, 2-3=-103/64
BOT CHORD 1-6=-442/926, 5-6=-442/926, 4-5=0/0
WEBS 2-6=0/234, 2-5=-964/461

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II, Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) Refer to girder(s) for truss to truss connections.
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 70 lb uplift at joint 1, 226 lb uplift at joint 3 and 93 lb uplift at joint 4.
- 4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Trapezoidal Loads (plf)
Vert: 1=0(F=27, B=27)-to-3=-134(F=40, B=40), 1=0(F=15, B=15)-to-4=-74(F=22, B=22)

**APRIL 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

Job L161719	Truss T02	Truss Type HIP	Qty 1	Ply 1	Job Reference (optional) 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 24 13:49:42 2006 Page 1
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Builders FirstSource, Lake City, FL 32055

Scale = 1:67.0
Camber = 3/16 in

Plate Offsets (X,Y): [2-0-1-11,Edge], [5-0-4-0-0-3-0], [9-0-1-11,Edge]					
LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.39	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.83	Ver(LL) -0.32 12-13 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.98	Ver(TL) -0.51 12-13 >867 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.16 9 n/a n/a		
	Code FBC2004/TPI2002			Weight: 189 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-3-10 oc purlins.

BOT CHORD Rigid ceiling directly applied or 6-0-3 oc bracing.

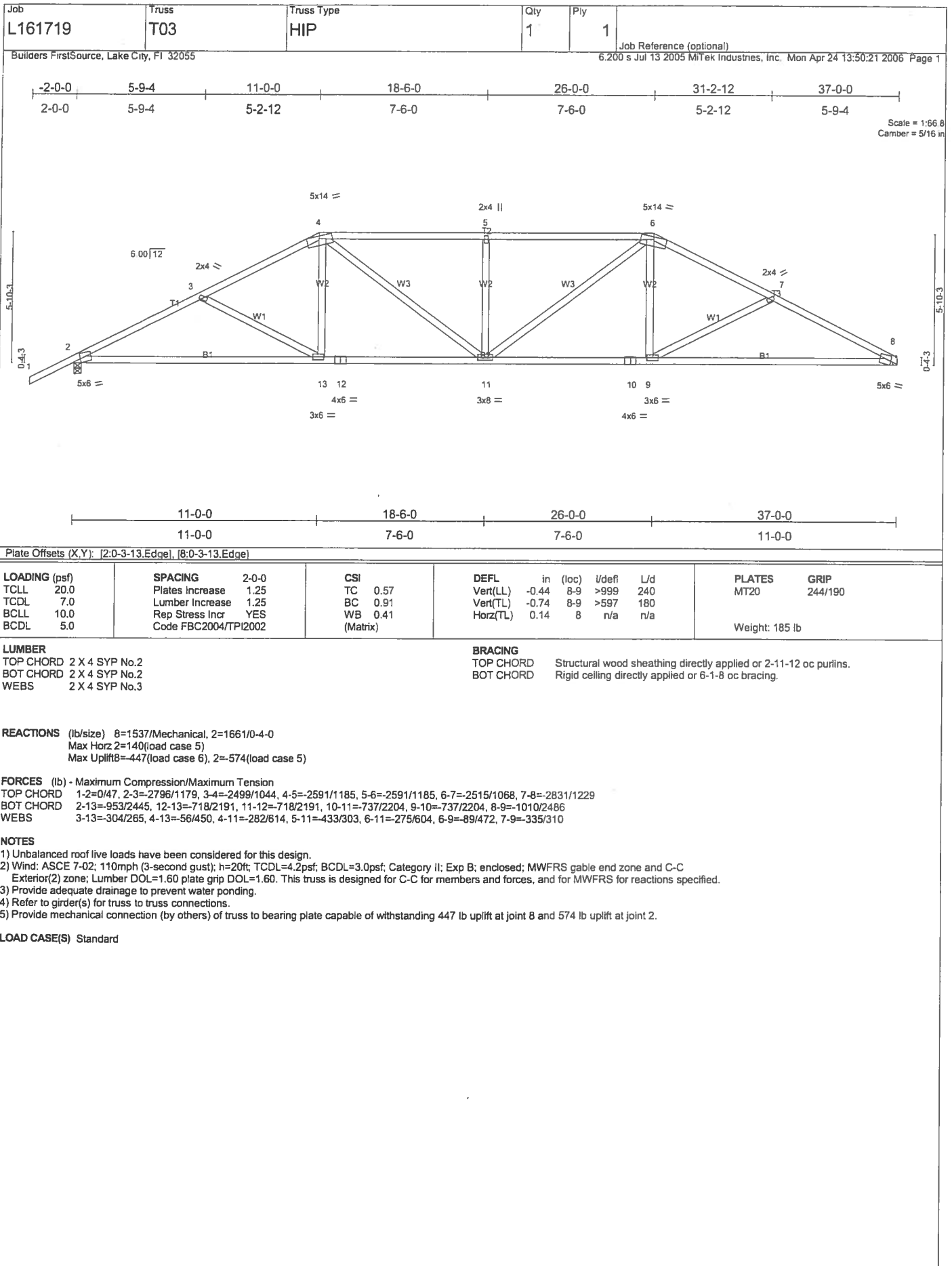
REACTIONS (lb/size) 9=1537/Mechanical, 2=1661/0-4-0
 Max Horz 2=126(load case 5)
 Max Uplift 9=428(load case 6), 2=556(load case 5)

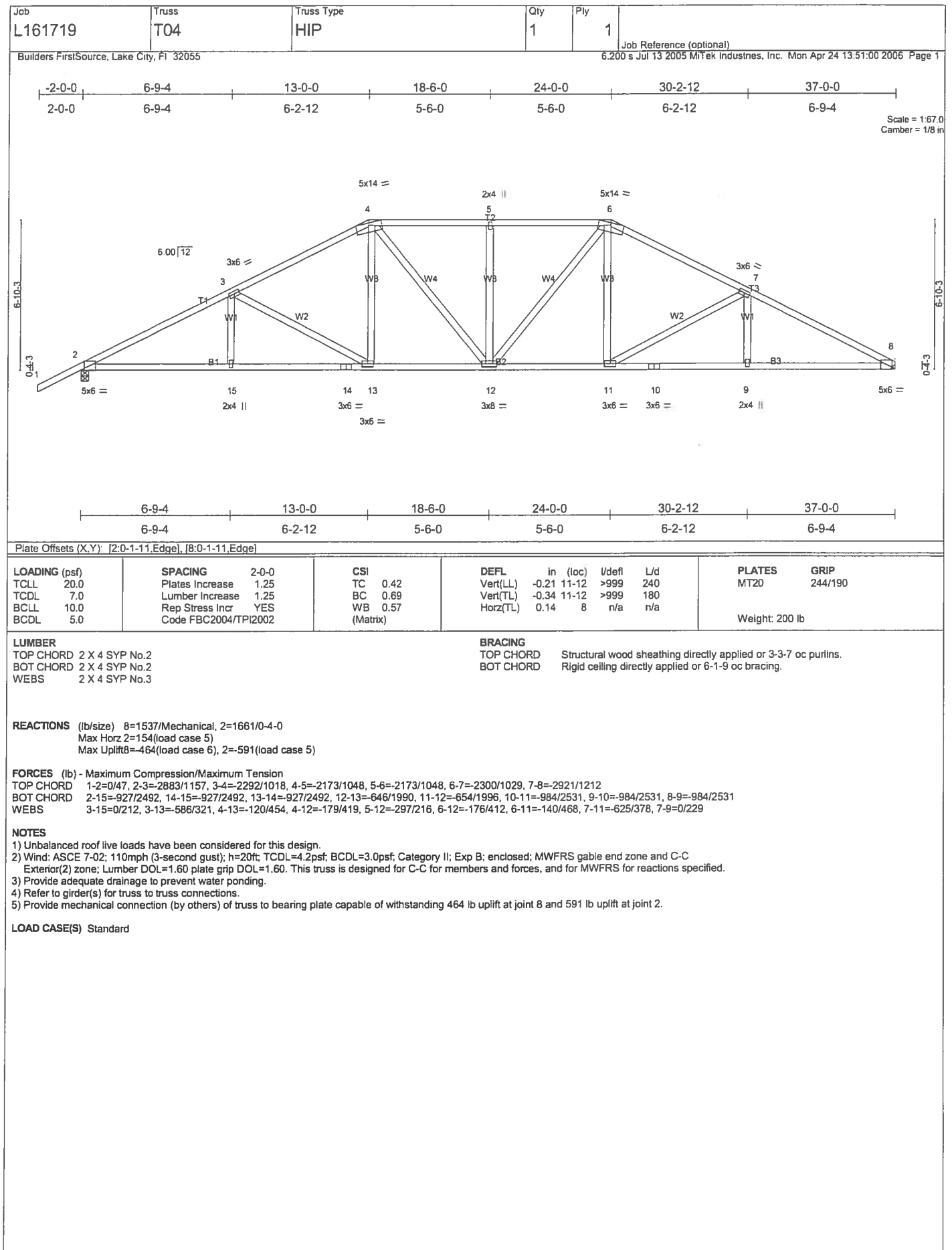
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=2844/1161, 3-4=2633/1072, 4-5=2341/1023, 5-6=3054/1328, 6-7=3054/1328, 7-8=2657/1108, 8-9=2891/1227
 BOT CHORD 2-15=-945/2482, 14-15=-1090/3049, 13-14=-1090/3049, 12-13=-1090/3049, 11-12=-813/2344, 10-11=-813/2344, 9-10=-1020/2536
 WEBS 3-15=-193/184, 4-15=-243/851, 5-15=-954/394, 5-13=0/172, 5-12=63/72, 6-12=349/244, 7-12=-390/957, 7-10=-66/363, 8-10=-232/234

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 428 lb uplift at joint 9 and 556 lb uplift at joint 2.

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	
L161719	T05	HIP	1	1	Job Reference (optional)

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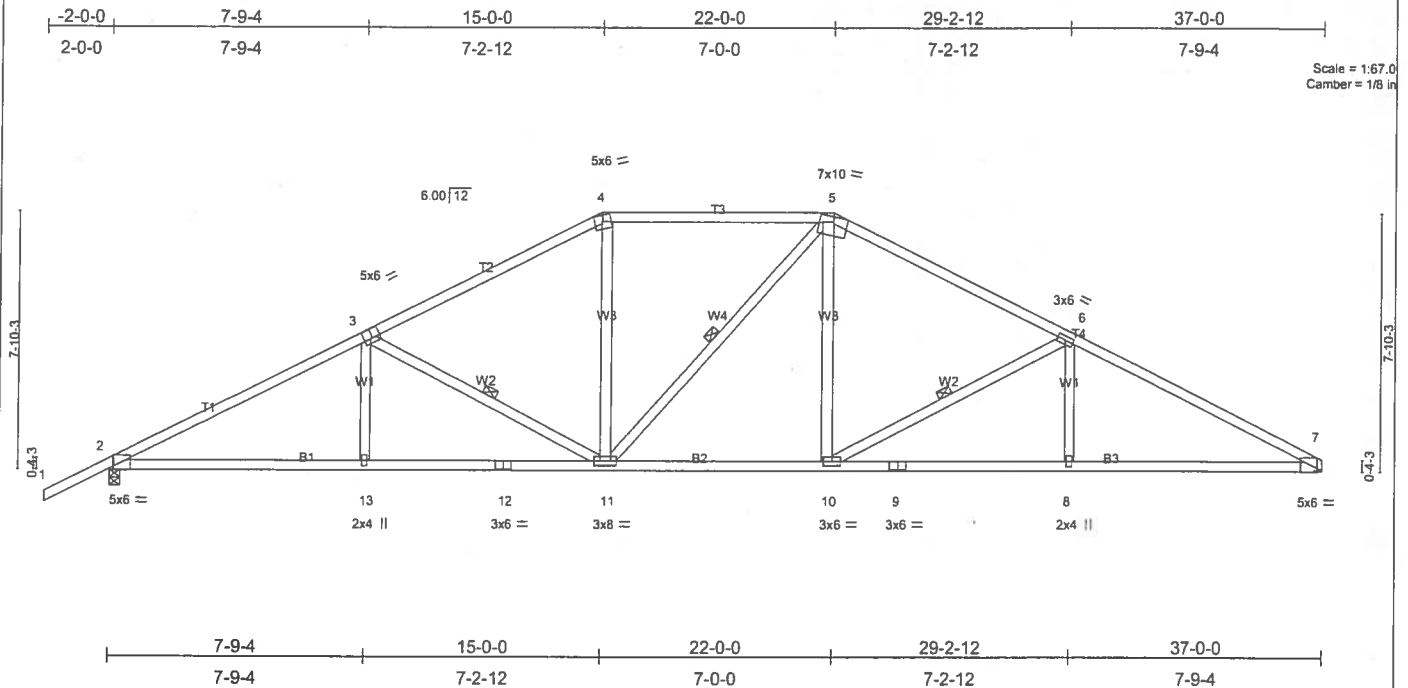


Plate Offsets (X,Y): [2:0-1-11,Edge], [3:0-3-0-3-4], [7:0-1-11,Edge]											
LOADING (psf)		SPACING 2-0-0		CSI		DEFL in (loc) l/defl L/d				PLATES GRIP	
TCLL	20.0	Plates Increase 1.25		TC 0.59		Vert(LL) -0.22 7-8 >999 240				MT20 244/190	
BCDL	7.0	Lumber Increase 1.25		BC 0.76		Vert(TL) -0.36 7-8 >999 180					
BCLL	10.0	Rep Stress Incr YES		WB 0.25		Horz(TL) 0.14 7 n/a n/a					
BCDL	5.0	Code FBC2004/TP12002		(Matrix)						Weight: 190 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 3-0-9 oc purtins.
BOT CHORD 2 X 4 SYP No.2	Rigid ceiling directly applied or 6-0-12 oc bracing.
WEBS 2 X 4 SYP No.3	1 Row at midpt 3-11, 5-11, 6-10

REACTIONS (lb/size) 7=1537/Mechanical, 2=1661/0-4-0
Max Horz 2=168(load case 5)
Max Uplift 7=478(load case 6), 2=606(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=2841/1162, 3-4=2138/981, 4-5=1841/956, 5-6=2141/987, 6-7=2854/1207
 BOT CHORD 2-3=2138/981, 12-13=920/2448, 11-12=920/2448, 10-11=575/1843, 9-10=968/2482, 8-9=968/2482, 7-8=968/2482
 WEBS 3-13=0/253, 4-11=701/397, 4-11=151/538, 5-11=181/178, 5-10=163/546, 6-10=736/448, 6-8=0/268

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); $h=20$ ft; $TCDL=4.2$ psf; $BCDL=3.0$ psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber $DOL=1.60$ plate grip $DOL=1.60$. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 478 lb uplift at joint 7 and 606 lb uplift at joint 2.

LOAD CASE(S) Standard

Job L161719	Truss T06	Truss Type HIP	Qty 1	Ply 1	Job Reference (optional)
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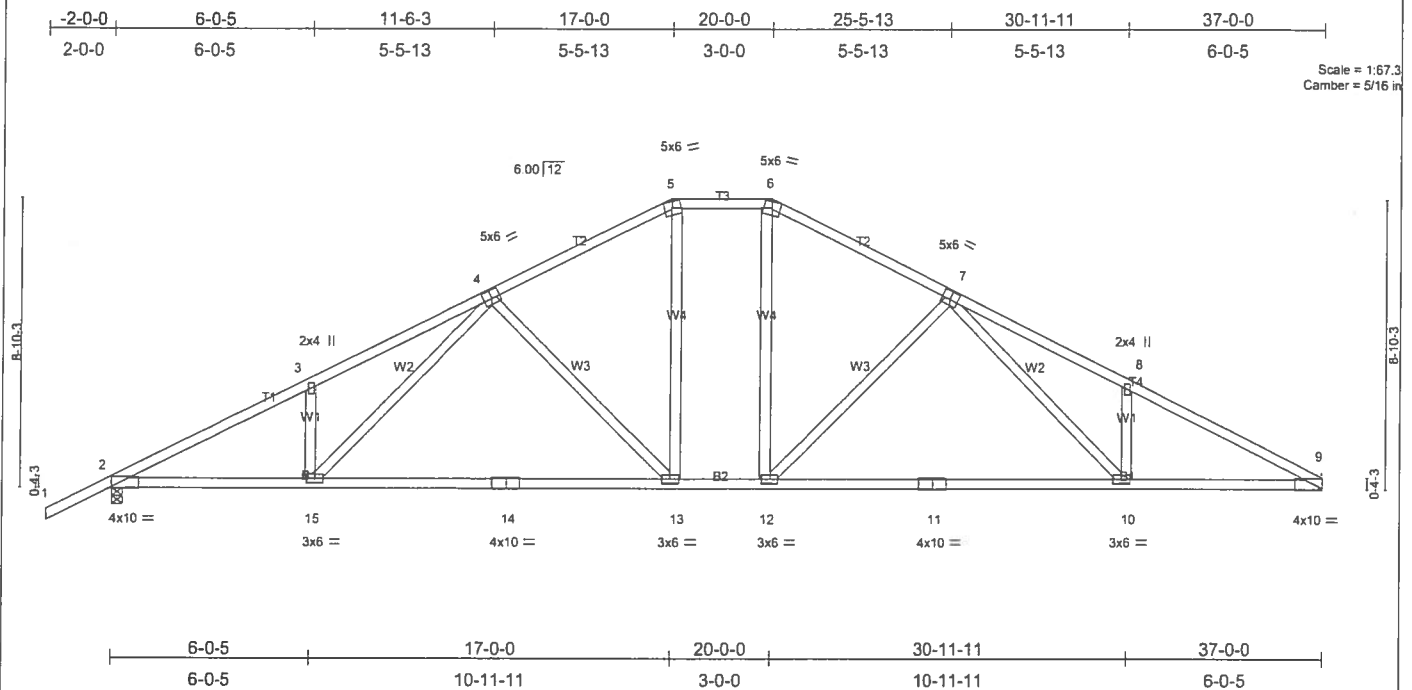


Plate Offsets (X,Y) [2 Edge 0-0-4], [4-0-3-0-0-3-0], [7-0-3-0-0-3-0], [9-0-0-0-0-4]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.35	Vert(LL)	-0.61	10-12	>727	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.96	Vert(TL)	-0.93	10-12	>474	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.73	Horz(TL)	0.13	9	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 199 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 3-4-3 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

REACTIONS (lb/size) 9=1541/Mechanical, 2=1665/0-4-0
 Max Horz 2=182(load case 5)
 Max Uplift 9=493(load case 6), 2=619(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-2975/1180, 3-4=-2937/1326, 4-5=-1943/956, 5-6=-1684/917, 6-7=-1943/956, 7-8=-3023/1417, 8-9=-3043/1255
 BOT CHORD 2-15=-955/2571, 14-15=-761/2068, 13-14=-761/2068, 12-13=-482/1684, 11-12=-779/2087, 10-11=-779/2087, 9-10=-1033/2647
 WEBS 3-15=-237/276, 4-15=-283/756, 4-13=-608/403, 5-13=-254/614, 6-12=-257/620, 7-12=-633/429, 7-10=-371/834, 8-10=-280/314

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 493 lb uplift at joint 9 and 619 lb uplift at joint 2.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	
L161719	T07	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

Job Reference (optional)

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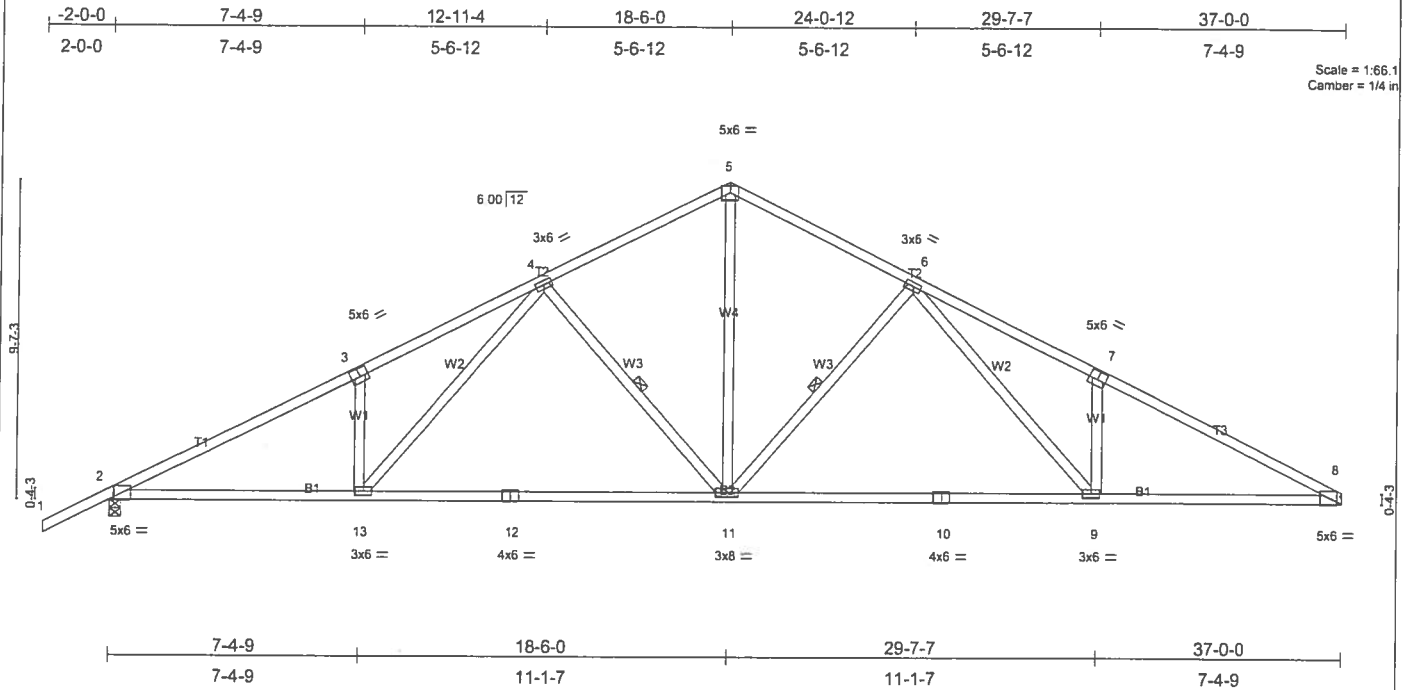


Plate Offsets (X,Y) [2:0-1-11,Edge], [3:0-3-0-0-3-0], [7:0-3-0-0-3-0], [8:0-1-11,Edge]									
LOADING (psf)	SPACING 2-0-0		CSI	DEFL in (loc) l/def L/d			PLATES	GRIP	
TCLL 20.0	Plates Increase 1.25		TC 0.50	Vert(LL) -0.40	11-13	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25		BC 0.90	Vert(TL) -0.67	11-13	>658	180		
BCLL 10.0	Rep Stress Incr YES		WB 0.88	Horz(TL) 0.13	8	n/a	n/a		
BCDL 5.0	Code FBC2004/TP12002		(Matrix)						
							Weight: 193 lb		

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 3-1-15 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-5 oc bracing.
WEBS	1 Row at midpt 4-11, 6-11

REACTIONS (lb/size) 2=1661/0-4-0, 8=1537/Mechanical
Max Horz 2=192(load case 5)
Max Uplift 2=-627(load case 5), 8=-500(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/47, 2-3=-2883/182, 3-4=-2845/1350, 4-5=-1817/930, 5-6=-1817/930, 6-7=-2885/1409, 7-8=-2916/1229
BOT CHORD	1-2=-940/2485, 12-13=-694/1949, 11-12=-694/1949, 10-11=-705/1957, 9-10=-705/1957, 8-9=-990/2519
WEBS	3-13=-320/337, 4-13=-369/819, 4-11=-603/348, 5-11=-576/1252, 6-11=-615/435, 6-9=-429/860, 7-9=-339/363

NOTES

- 1) Unsanctioned roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); $h=20ft$; $TCDL=4.2psf$; $BCDL=3.0psf$; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber $DOL=1.80$ plate grip $DOL=1.60$. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 627 lb uplift at joint 2 and 500 lb uplift at joint 8.

LOAD CASE(S) Standard

Job L161719	Truss T08	Truss Type SPECIAL	Qty 2	Ply 1	Job Reference (optional)
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Builders FirstSource, Lake City, FL 32055

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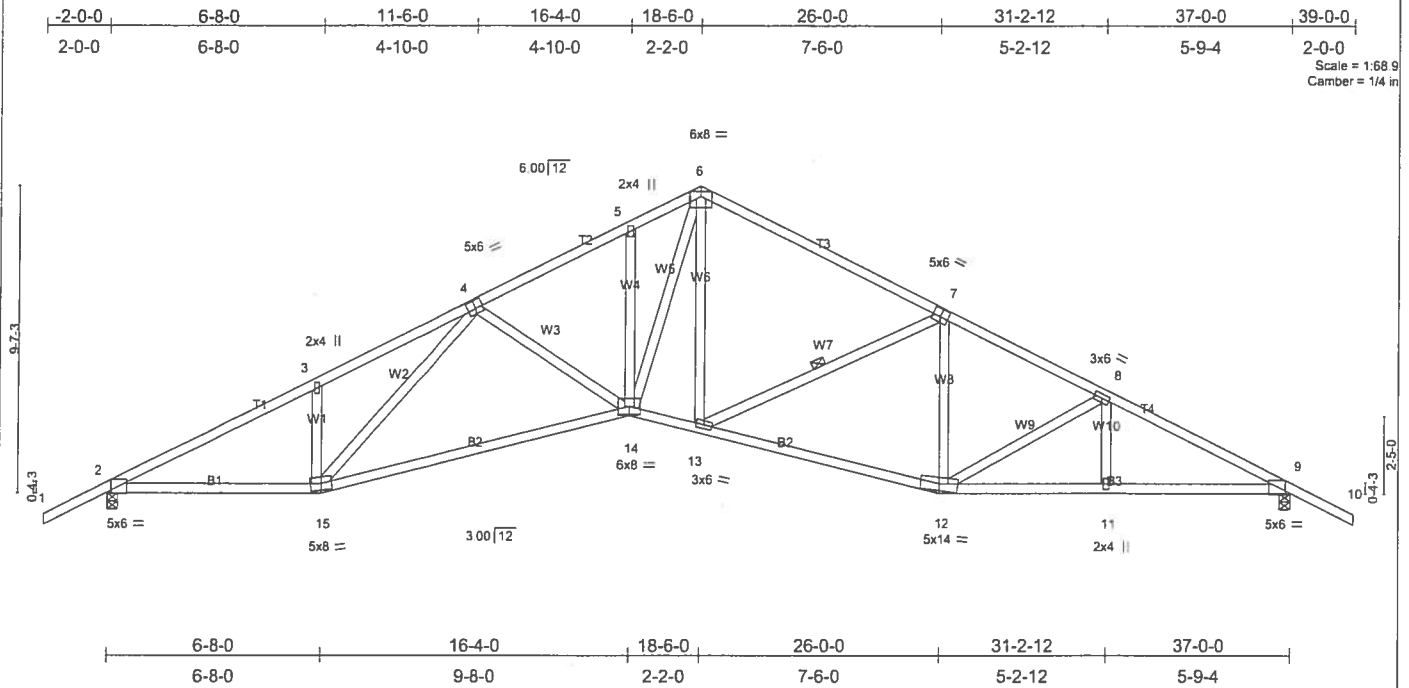


Plate Offsets (X,Y): [2:0-1-11,Edge], [4:0-3-0,0-3-0], [7:0-3-0,0-3-0], [9:0-1-11,Edge]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.45	Vert(LL)	-0.36	14-15	>999	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.64	Vert(TL)	-0.60	14-15	>732		
BCLL 10.0	Lumber Increase 1.25	WB 0.51	Horz(TL)	0.19	9	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)						
	Code FBC2004/TP12002						Weight: 209 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.1D *Except*
 T1 2 X 4 SYP No.2, T4 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.1D *Except*
 B3 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-5-1 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-8-11 oc bracing.
 WEBS 1 Row at midpt 7-13

REACTIONS

(lb/size) 2=1657/0-4-0, 9=1657/0-4-0
 Max Horz 2=168(load case 5)
 Max Uplift 2=625(load case 5), 9=625(load case 6)

FORCES

(lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-2909/1179, 3-4=-2858/1314, 4-5=-2769/1175, 5-6=-2674/1219, 6-7=-2253/1008, 7-8=-2467/1111, 8-9=-2894/1187,
 9-10=0/47
 BOT CHORD 2-15=-868/2512, 14-15=-879/2710, 13-14=-447/2010, 12-13=-728/2251, 11-12=-883/2504, 9-11=-883/2504
 WEBS 3-15=-265/280, 4-15=-168/9, 4-14=-282/298, 5-14=-158/115, 6-13=-185/340, 7-13=-354/308, 7-12=-140/111, 8-12=-405/207, 8-11=0/151,
 6-14=-552/1596

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 625 lb uplift at joint 2 and 625 lb uplift at joint 9.

LOAD CASE(S) Standard

Job L161719	Truss T09	Truss Type SPECIAL	Qty 3	Ply 1	00
Builders FirstSource, Lake City, FL 32055					Job Reference (optional)
					6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 24 14:26:34 2006 Page 1

-2-0-0	6-8-0	11-6-0	16-4-0	18-6-0	26-0-0	31-2-12	37-0-0	39-0-0
2-0-0	6-8-0	4-10-0	4-10-0	2-2-0	7-6-0	5-2-12	5-9-4	2-0-0
Scale = 1/8" = 1'-0"								
Camber = 1/8" in								

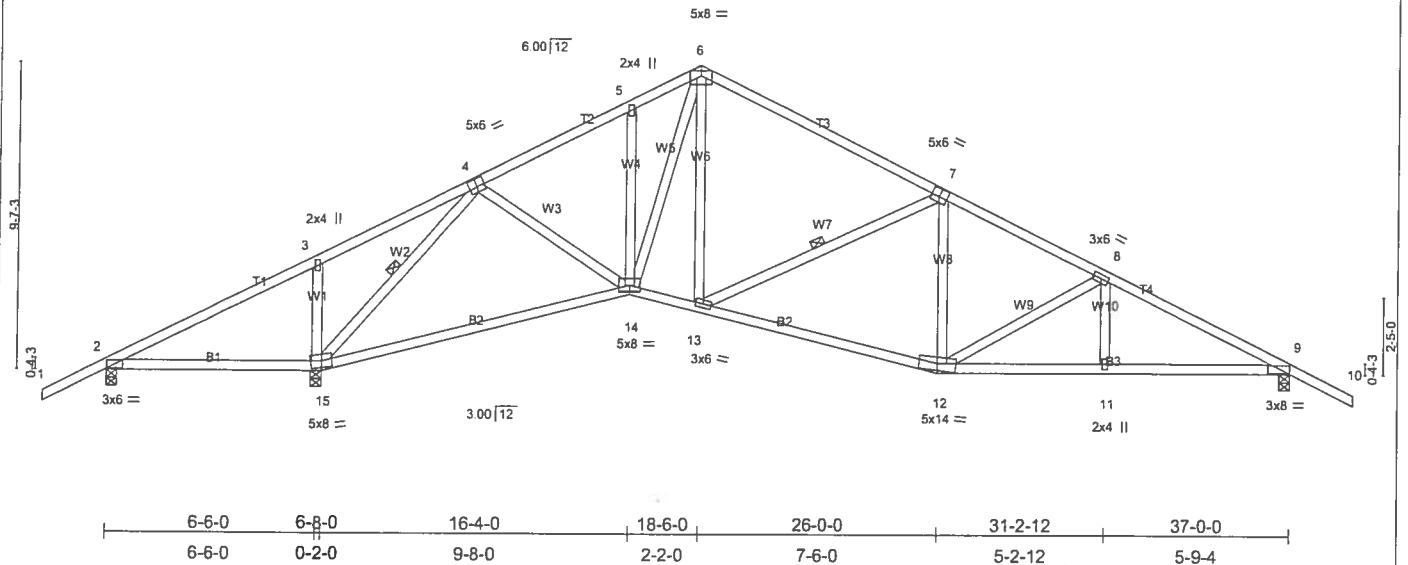


Plate Offsets (X,Y): [4:0-3-0,0-3-0], [7:0-3-0,0-3-0], [9:0-4-12,0-1-8]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.45	Vert(LL)	0.10	2-15	>767	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.54	Vert(TL)	-0.33	14-15	>999		
BCLL 10.0	Lumber Increase 1.25	WB 0.60	Horz(TL)	0.09	9	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)						
	Code FBC2004/TPI2002							
							Weight: 209 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-1-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-15, 7-13

REACTIONS

(lb/size) 2=17/0-4-0, 15=1993/0-4-0, 9=1304/0-4-0
Max Horz 2=168(load case 5)
Max Uplift 2=188(load case 5), 15=712(load case 5), 9=539(load case 6)
Max Grav 2=118(load case 9), 15=1993(load case 1), 9=1304(load case 1)

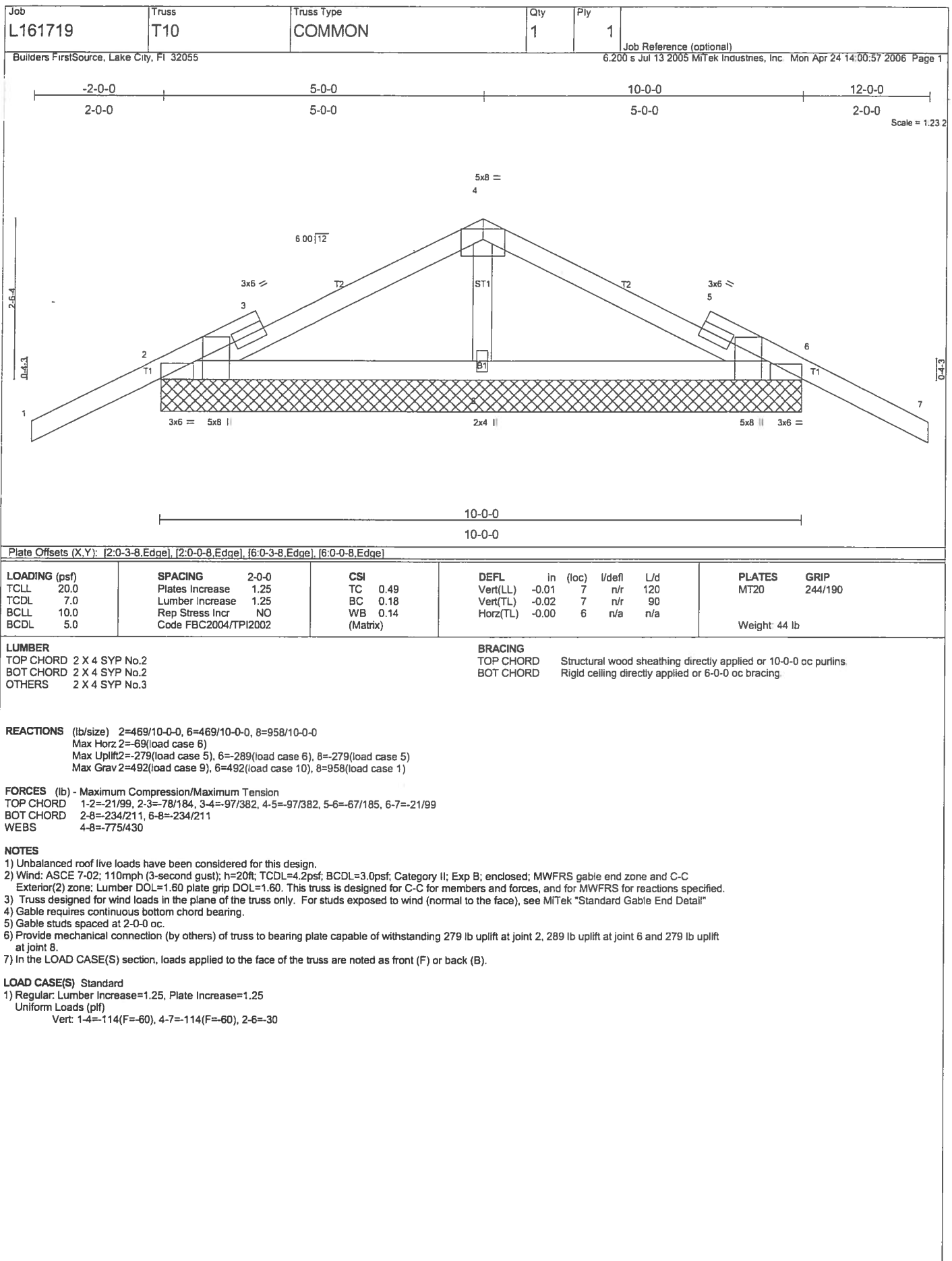
FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-263/785, 3-4=-115/765, 4-5=-1370/601, 5-6=-1289/653, 6-7=-1273/611, 7-8=-1697/799, 8-9=-2135/879, 9-10=0/47
BOT CHORD 2-15=-623/371, 14-15=-151/693, 13-14=-107/1106, 12-13=-441/1543, 11-12=-610/1831, 9-11=-610/1830
WEBS 3-15=-338/337, 4-15=-1993/689, 4-14=-42/645, 5-14=-183/127, 6-14=-127/480, 6-13=-249/433, 7-13=-504/381, 7-12=0/85, 8-12=-417/214, 8-11=0/152

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 188 lb uplift at joint 2, 712 lb uplift at joint 15 and 539 lb uplift at joint 9.

LOAD CASE(S) Standard



Job L161719	Truss T10G	Truss Type COMMON	Qty 1	Ply 1	0 0
Builders FirstSource, Lake City, FL 32055			Job Reference (optional)		

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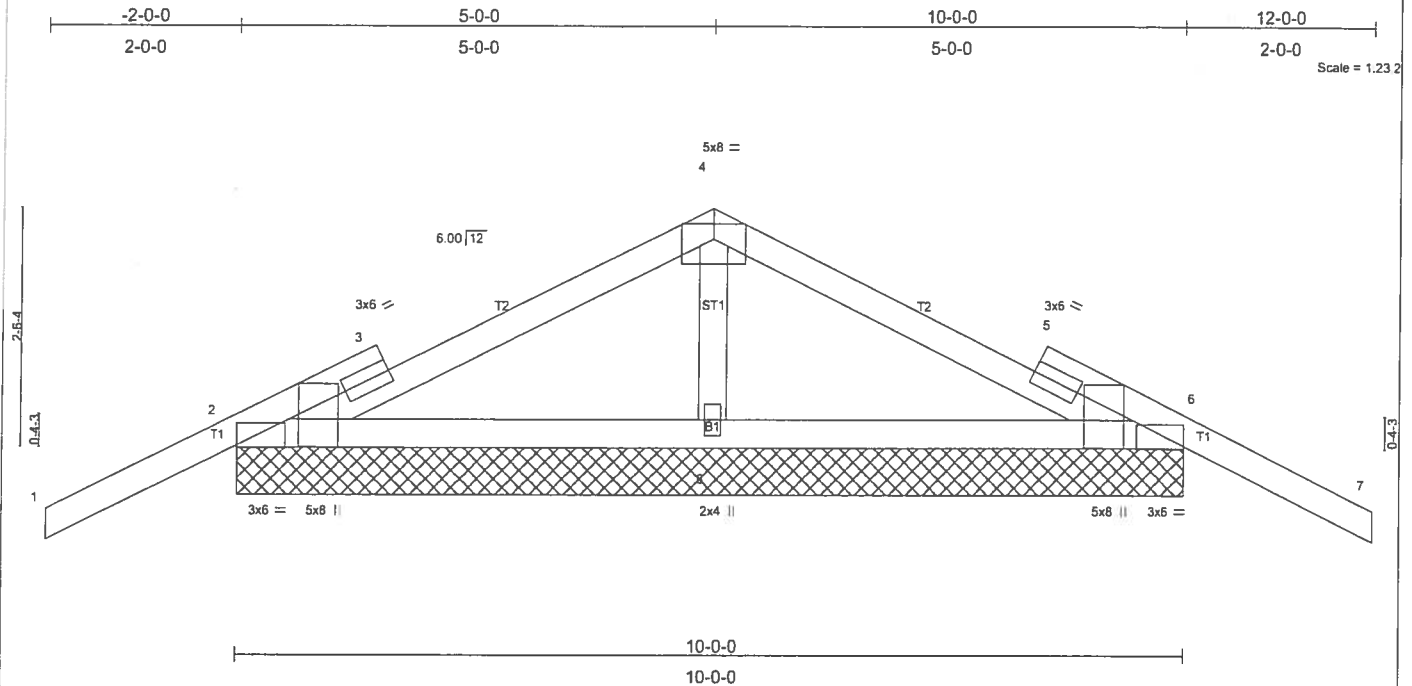


Plate Offsets (X,Y): [2-0-3-8,Edge], [2-0-0-8,Edge], [6-0-3-8,Edge], [6-0-0-8,Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.49	Vert(LL)	-0.01	7	n/r	120	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.18	Vert(TL)	-0.02	7	n/r	90		
BCLL 10.0	Rep Stress Incr	NO	WB 0.14	Horz(TL)	-0.00	6	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						Weight: 44 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=469/10-0-0, 6=469/10-0-0, 8=958/10-0-0

Max Horz 2=-69(load case 6)
 Max Uplift 2=-279(load case 5), 6=-289(load case 6), 8=-279(load case 5)
 Max Grav 2=492(load case 9), 6=492(load case 10), 8=958(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

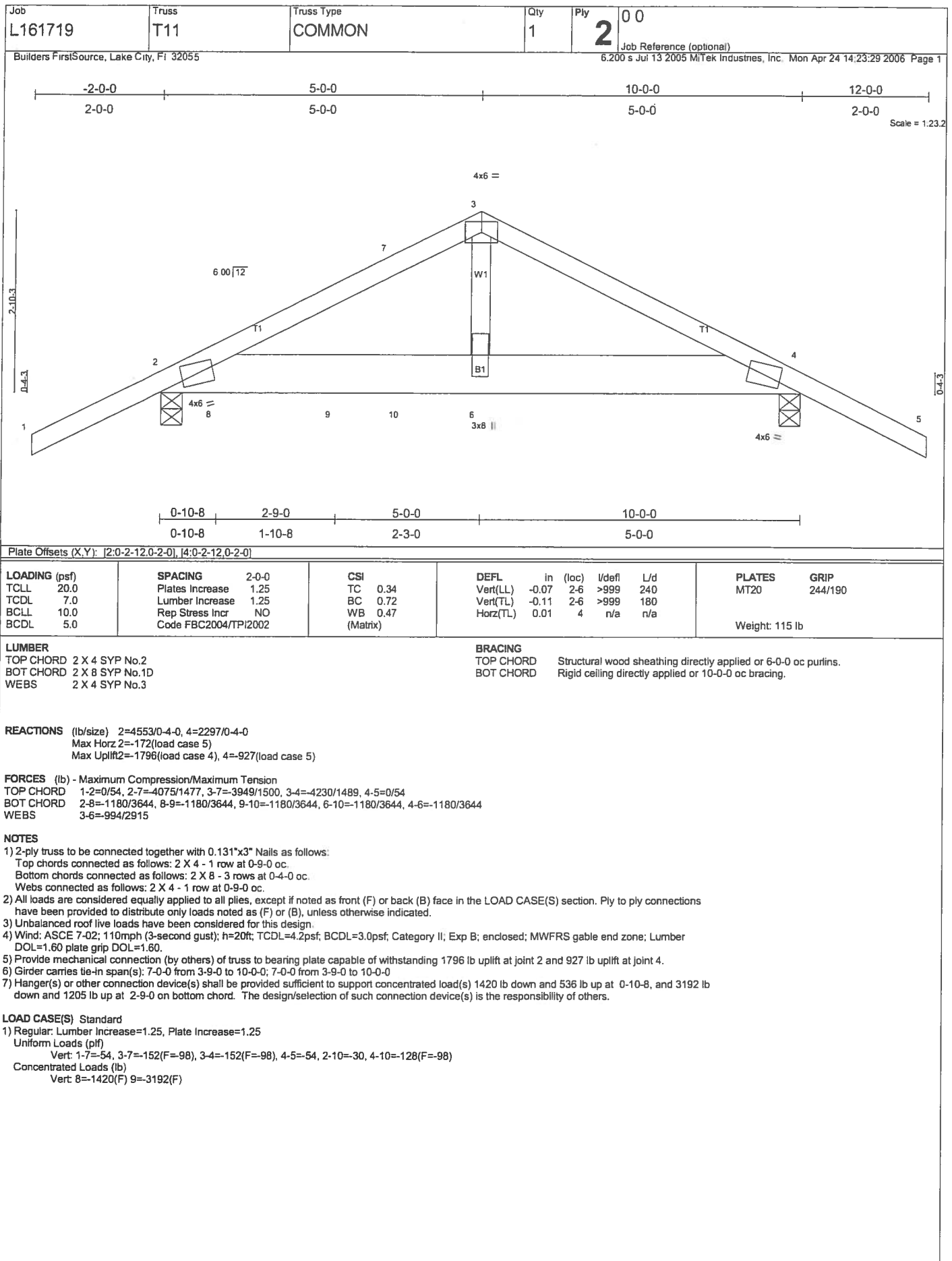
TOP CHORD 1-2=-21/99, 2-3=-78/184, 3-4=-97/382, 4-5=-97/382, 5-6=-67/185, 6-7=-21/99
 BOT CHORD 2-8=-234/211, 6-8=-234/211
 WEBS 4-8=-775/430

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 279 lb uplift at joint 2, 289 lb uplift at joint 6 and 279 lb uplift at joint 8.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-4=-114(F=-60), 4-7=-114(F=-60), 2-6=-30



Job L161719	Truss T12	Truss Type SPECIAL	Qty 1	Ply 1	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 24 14:01:56 2006 Page 1		

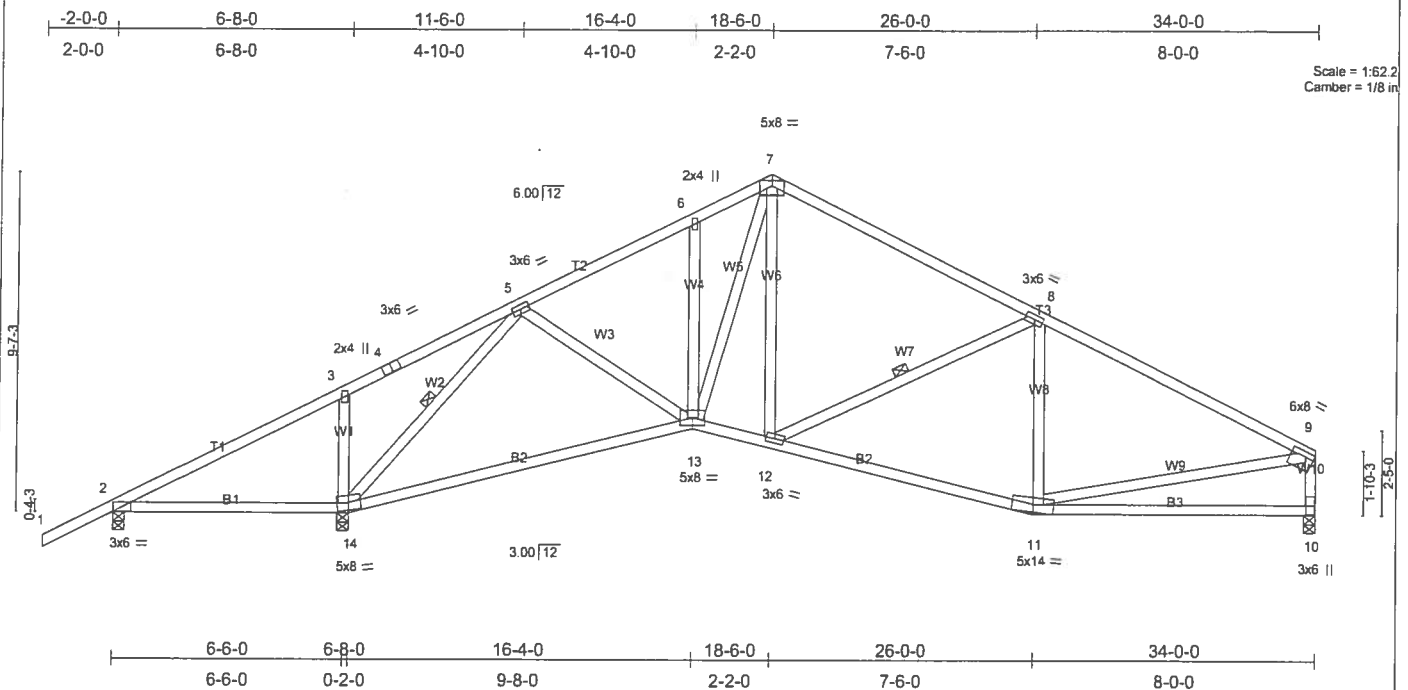


Plate Offsets (X,Y): [9 Edge,0-1-12]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.82	Vert(LL)	0.11	2-14	>729	240	MT20	244/190
TCCL 7.0	Plates Increase 1.25	BC 0.50	Vert(TL)	-0.30	13-14	>999	180		
BCLL 10.0	Lumber Increase 1.25	WB 0.50	Horz(TL)	0.06	10	n/a	n/a		
BCCL 5.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2004/TP1/2002							Weight: 198 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-0-5 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
 WEBS 1 Row at midpt 5-14, 8-12

REACTIONS (lb/size) 2=148/0-4-0, 14=1719/0-4-0, 10=1079/0-4-0

Max Horz 2=244(load case 5)
 Max Uplift 2=198(load case 5), 14=672(load case 5), 10=360(load case 6)
 Max Grav 2=199(load case 9), 14=1719(load case 1), 10=1079(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-243/503, 3-4=-94/378, 4-5=-77/486, 5-6=-1269/591, 6-7=-1192/649, 7-8=-1148/578, 8-9=-1373/607, 9-10=-961/482
 BOT CHORD 2-14=-373/202, 13-14=-230/730, 12-13=-205/987, 11-12=-450/1201, 10-11=-138/245
 WEBS 3-14=-331/338, 5-14=-1658/672, 5-13=-35/491, 6-13=-185/135, 7-13=-173/534, 7-12=-170/277, 8-12=-288/254, 8-11=-233/227,
 9-11=-304/920

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 198 lb uplift at joint 2, 672 lb uplift at joint 14 and 360 lb uplift at joint 10.

LOAD CASE(S) Standard

Job L161719	Truss T13	Truss Type HIP	Qty 1	Ply 2	00
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Apr 25 08:08:14 2006 Page 1		

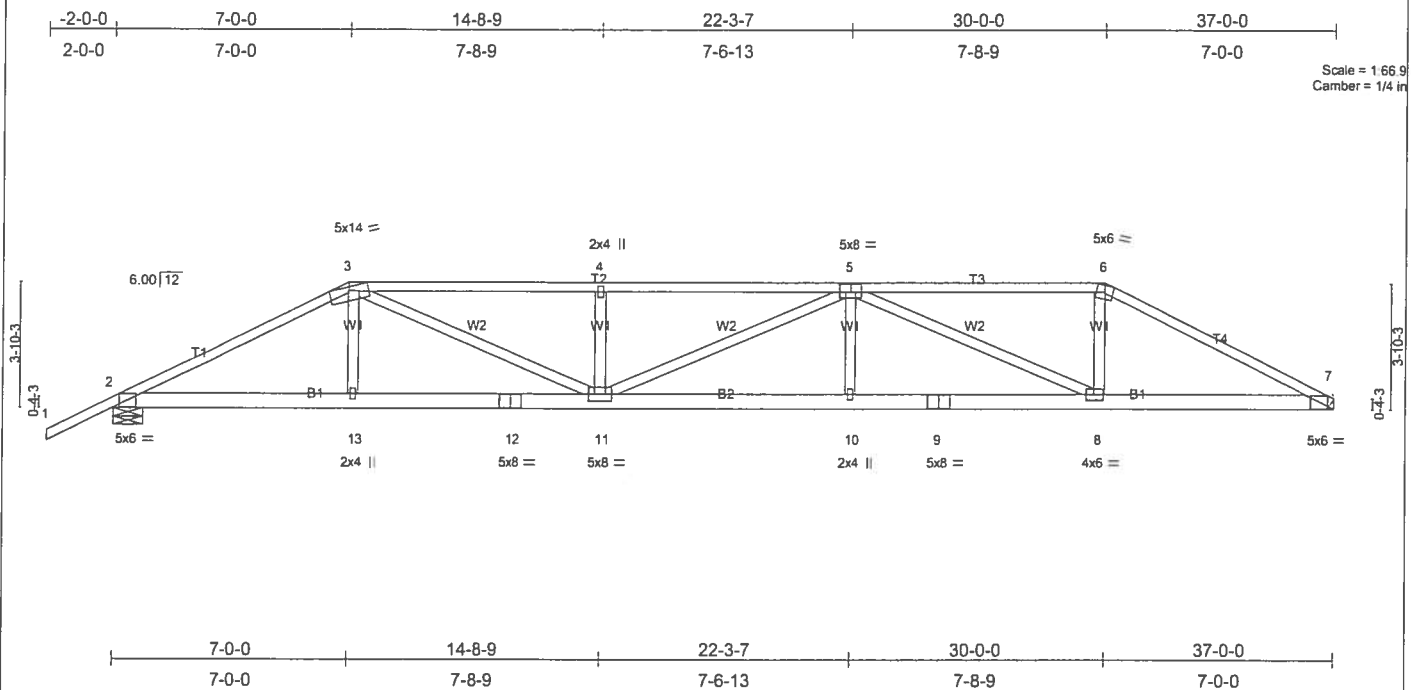


Plate Offsets (X,Y): [2-0-3-0,0-2-9], [5-0-4-0-0-3-0], [7-0-3-0-0-2-9]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.97	Vert(LL) 0.43	10-11	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.58	Vert(TL) -0.67	10-11	>659	180		
BCCL 10.0	Rep Stress Incr NO	WB 0.91	Horz(TL) 0.12	7	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)						
Weight: 403 lb								

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 8-1-1 oc bracing.

REACTIONS (lb/size) 7=3192/Mechanical, 2=3345/0-10-11
Max Horz 2=116(load case 4)
Max Uplift 7=1761(load case 2), 2=1869(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/52, 2-3=6572/3574, 3-4=8956/5117, 4-5=8956/5118, 5-6=6008/3417, 6-7=6679/3705
BOT CHORD 2-13=3108/5778, 12-13=3124/5817, 11-12=3124/5817, 10-11=5097/9056, 9-10=5097/9056, 8-9=5097/9056, 7-8=3267/5910
WEBS 3-13=398/917, 3-11=2077/3525, 4-11=916/749, 5-11=152/86, 5-10=101/532, 5-8=3460/1958, 6-8=1206/2317

NOTES

- 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02: 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- Provide adequate drainage to prevent water ponding.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1761 lb uplift at joint 7 and 1869 lb uplift at joint 2.
- Girder carries hip end with 7-0-0 end setback.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 30-0-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (psf)
Vert: 1-3=54, 3-6=118(F=64), 6-7=54, 2-13=30, 8-13=65(F=35), 7-8=30
Concentrated Loads (lb)
Vert: 13=539(F) 8=539(F)

Job L161719	Truss T14	Truss Type HIP	Qty 1	Ply 1	Job Reference (optional)
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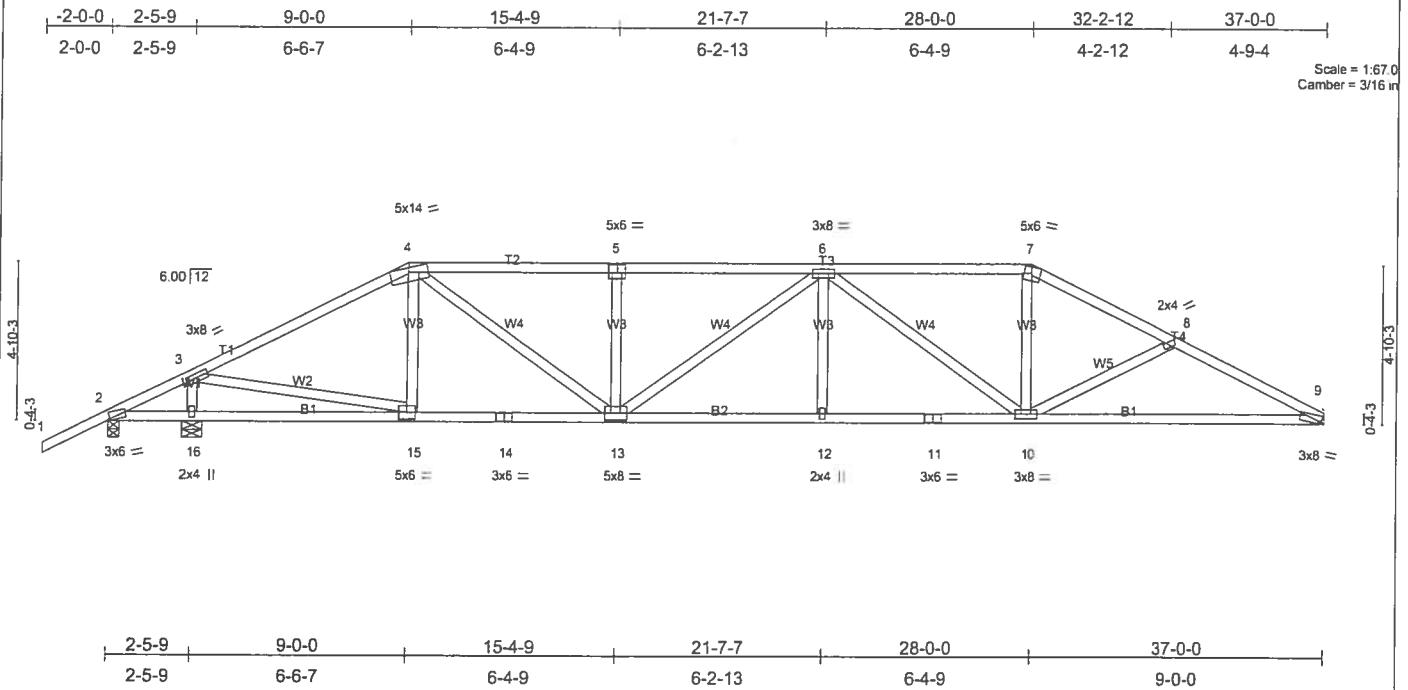
Scale = 1:67.0
Camber = 3/16 in

Plate Offsets (X,Y): [2-0-0-10-Edge], [5-0-3-0-0-3-0], [9-0-1-1-0-0-7]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.41	Vert(LL)	-0.25	9-10	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.79	Vert(TL)	-0.42	9-10	>986	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.78	Horz(TL)	0.10	9	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 193 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 3-5-10 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 9=1420/Mechanical, 2=-89/0-4-0, 16=1867/0-7-3
 Max Horz 2=126(load case 5)
 Max Uplift 9=403(load case 6), 2=-217(load case 10), 16=-552(load case 4)
 Max Grav 9=1420(load case 1), 2=28(load case 4), 16=1867(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-214/517, 3-4=-1875/775, 4-5=-2500/1106, 5-6=-2500/1106, 6-7=-2129/963, 7-8=-2403/1010, 8-9=-2641/1132
 BOT CHORD 2-16=-420/197, 15-16=-420/197, 14-15=-529/1600, 13-14=-529/1600, 12-13=-949/2663, 11-12=-949/2663, 10-11=-949/2663,
 9-10=-935/2315
 WEBS 3-16=-1700/804, 3-15=-744/2046, 4-15=-195/181, 4-13=-436/1172, 5-13=-351/246, 6-13=-231/93, 6-12=0/164, 6-10=-753/331,
 7-10=-227/757, 8-10=-237/238

NOTES

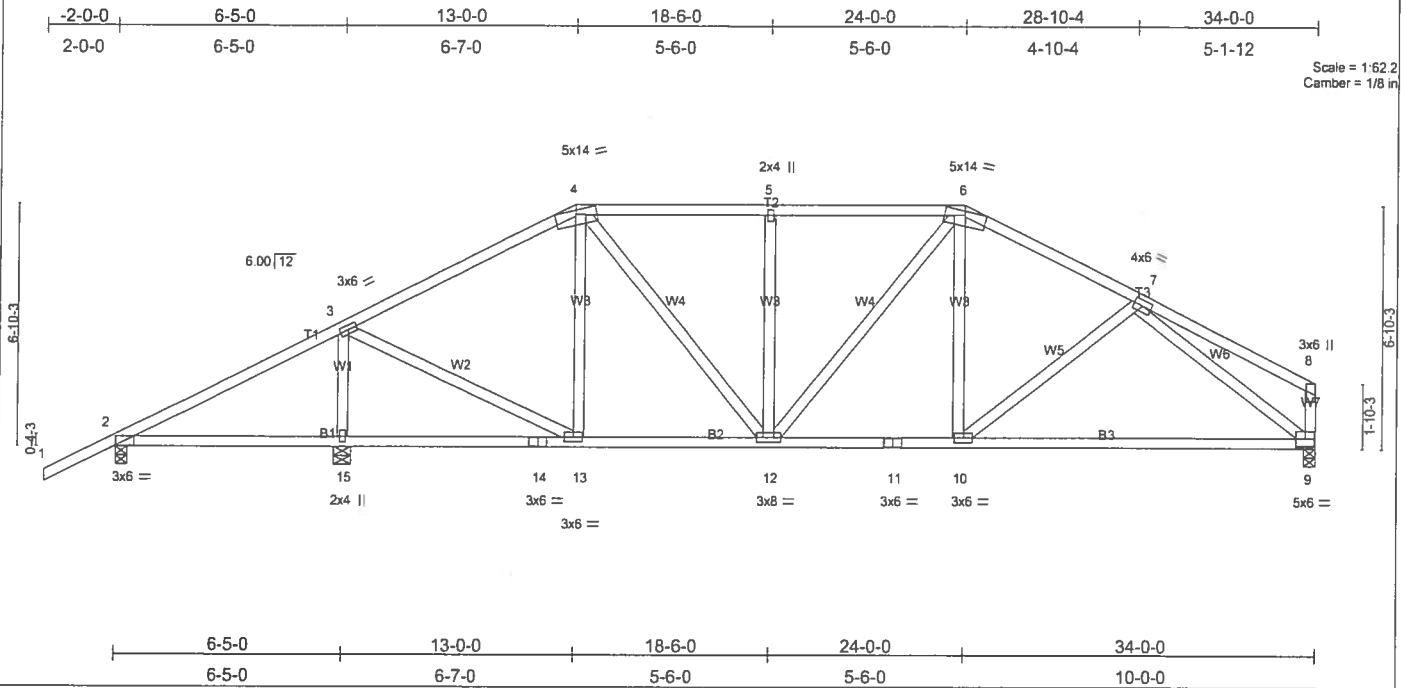
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 403 lb uplift at joint 9, 217 lb uplift at joint 2 and 552 lb uplift at joint 16.

LOAD CASE(S) Standard

Job L161719	Truss T16	Truss Type HIP	Qty 1	Ply 1	0 0	Job Reference (optional)
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LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.85	in (loc) l/def L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.54	Vert(LL) 0.08 2-15 >928 240		
BCCL 10.0	Rep Stress Incr YES	WB 0.79	Vert(TL) -0.32 9-10 >999 180		
BCDL 5.0	Code FBC2004/TP12002	(Matrix)	Horz(TL) 0.03 9 n/a n/a		
				Weight: 196 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-5-2 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS

(lb/size) 2=280/0-4-0, 15=1541/0-6-0, 9=1125/0-4-0
 Max Horz 2=206(load case 5)
 Max Uplift 2=248(load case 5), 15=558(load case 5), 9=340(load case 6)
 Max Grav 2=297(load case 9), 15=1541(load case 1), 9=1125(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=107/222, 3-4=1025/465, 4-5=1149/613, 5-6=1149/613, 6-7=1306/606, 7-8=332/110, 8-9=262/144
 BOT CHORD 2-15=140/82, 14-15=140/82, 13-14=140/82, 12-13=228/835, 11-12=346/1122, 10-11=346/1122, 9-10=454/1091
 WEBS 3-15=1321/686, 3-13=339/1068, 4-13=279/209, 4-12=209/547, 5-12=308/221, 6-12=119/150, 6-10=42/241, 7-10=37/163, 7-9=1106/550

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 248 lb uplift at joint 2, 558 lb uplift at joint 15 and 340 lb uplift at joint 9.

LOAD CASE(S) Standard

Job L161719	Truss T17	Truss Type SPECIAL	Qty 1	Ply 1	Job Reference (optional)
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Builders FirstSource, Lake City, Fl 32055

6,200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 24 14:07:24 2006 Page 1

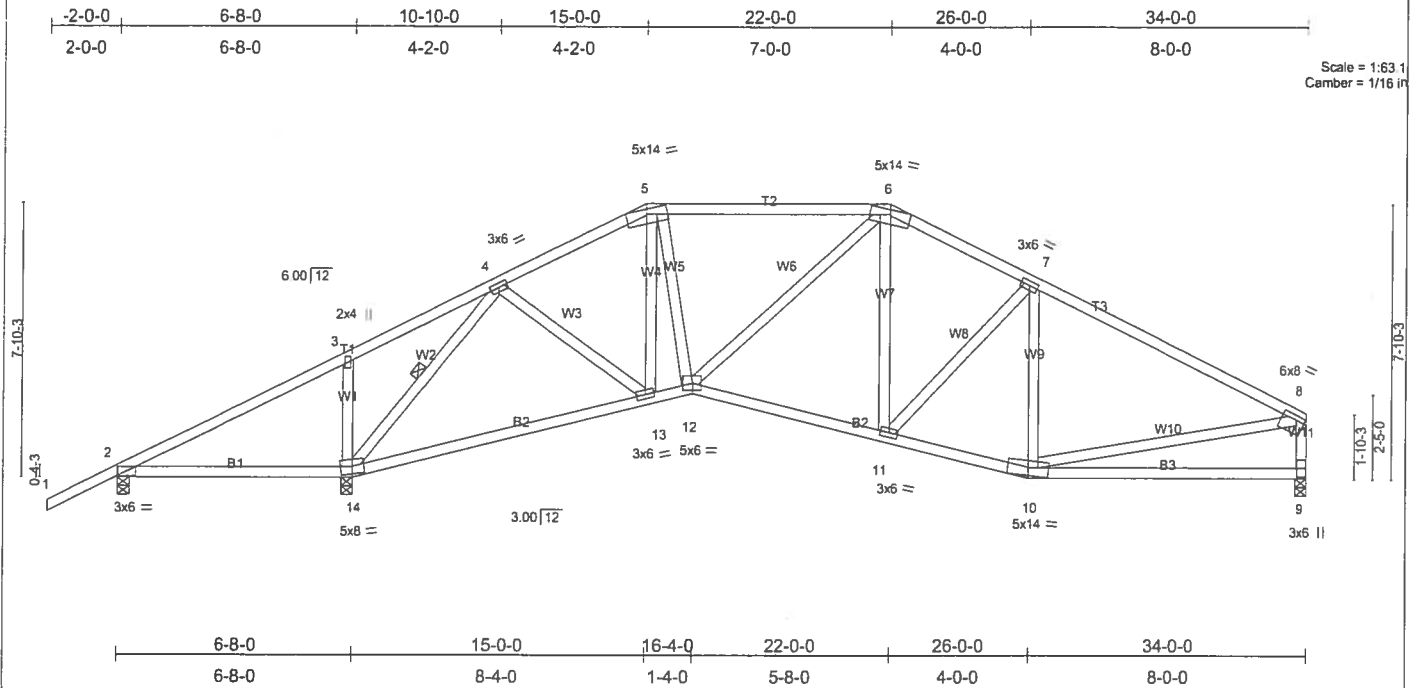


Plate Offsets (X,Y): [8-0-3-0-0-1-8]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.97	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.47	Vert(LL) 0.11 2-14 >742 240		
BCCL 10.0	Lumber Increase 1.25	WB 0.42	Vert(TL) 0.09 2-14 >828 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.06 9 n/a n/a		
	Code FBC2004/TPI2002			Weight: 198 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 4-11-10 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
 WEBS 1 Row at midpt 4-14

REACTIONS (lb/size) 2=139/0-4-0, 14=1730/0-4-0, 9=1077/0-4-0
 Max Horz 2=220(load case 5)
 Max Uplift 2=210(load case 5), 14=632(load case 5), 9=381(load case 6)
 Max Grav 2=218(load case 9), 14=1730(load case 1), 9=1077(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=241/537, 3-4=77/501, 4-5=1216/550, 5-6=1157/586, 6-7=1261/638, 7-8=1345/579, 8-9=948/472
 BOT CHORD 2-14=402/201, 13-14=158/605, 12-13=261/1061, 11-12=318/1134, 10-11=416/1154, 9-10=161/288
 WEBS 3-14=338/336, 4-14=1605/604, 4-13=126/634, 5-13=268/188, 5-12=186/471, 6-12=57/153, 6-11=104/206, 7-11=66/136,
 7-10=269/198, 8-10=248/843

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 210 lb uplift at joint 2, 632 lb uplift at joint 14 and 381 lb uplift at joint 9.

LOAD CASE(S) Standard

Job L161719	Truss T18	Truss Type SPECIAL	Qty 1	Ply 1	Job Reference (optional)
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Builders FirstSource, Lake City, FL 32055

6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 24 14:08:53 2006 Page 1

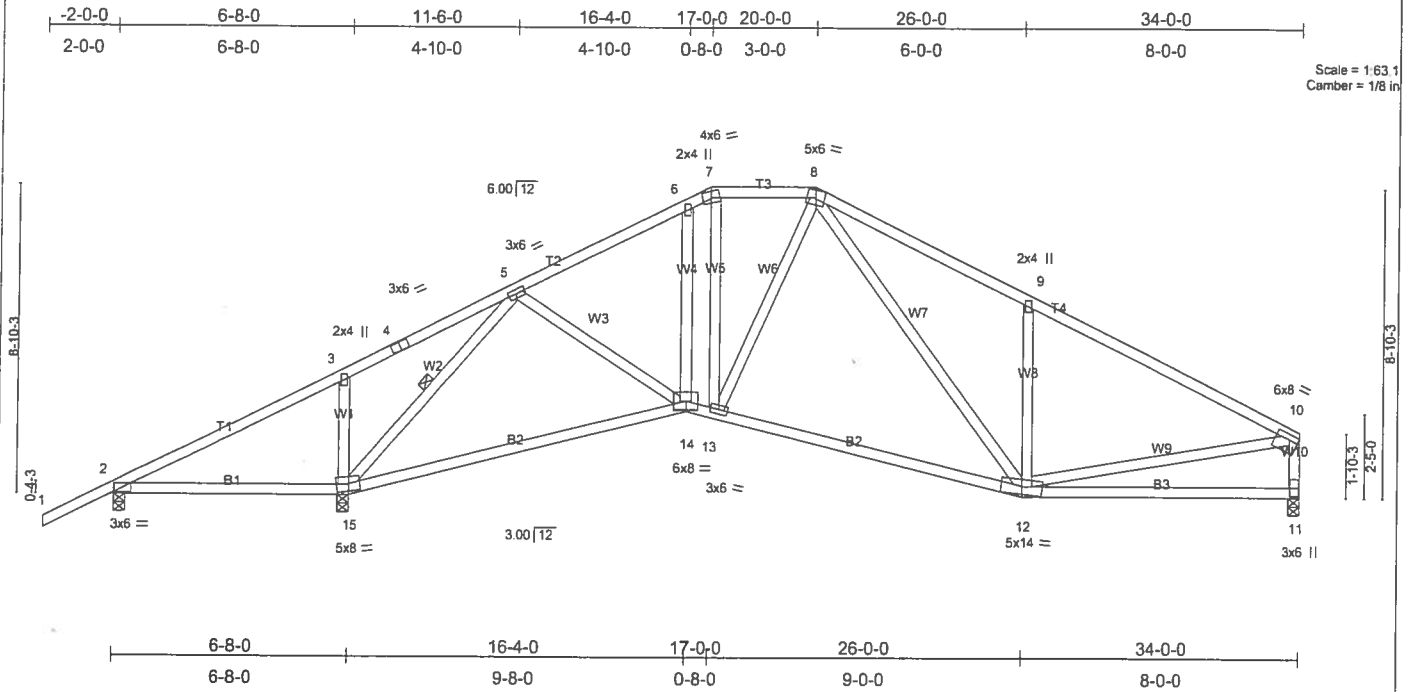


Plate Offsets (X,Y): [10:Edge,0-1-12]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.88	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.61	Vert(LL) 0.10 2-15 >764 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.56	Vert(TL) -0.34 14-15 >950 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.06 11 n/a n/a		
	Code FBC2004/TPI2002			Weight: 198 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 5-0-10 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. Except:
 6-0-0 oc bracing: 2-15.
 WEBS 1 Row at midpt 5-15

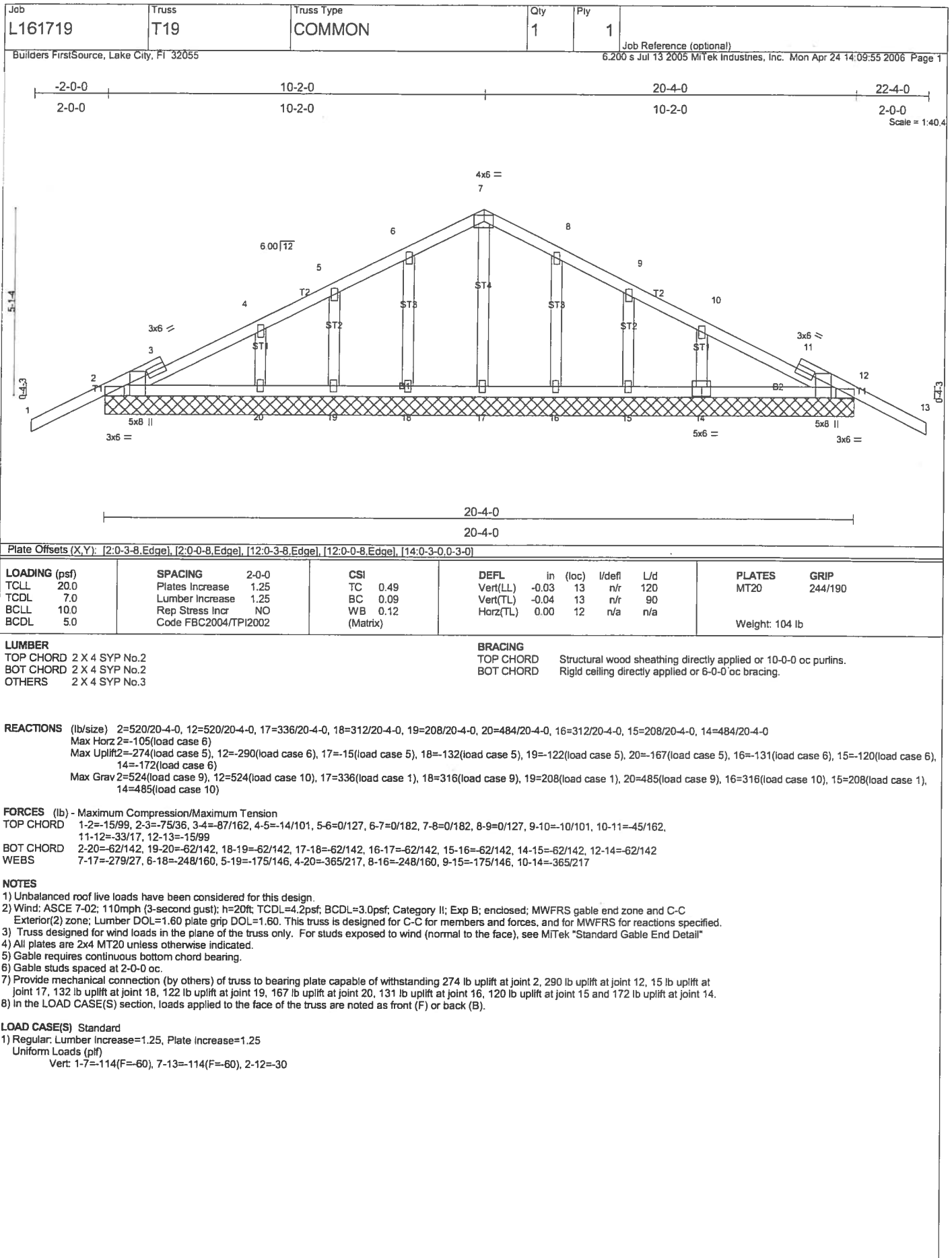
REACTIONS (lb/size) 2=62/0-4-0, 15=1826/0-4-0, 11=1059/0-4-0
 Max Horz 2=234/(load case 5)
 Max Uplift 2=168/(load case 5), 15=700/(load case 5), 11=352/(load case 6)
 Max Grav 2=106/(load case 9), 15=1826/(load case 1), 11=1059/(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-303/687, 3-4=-156/562, 4-5=-139/671, 5-6=-1188/549, 6-7=-1007/600, 7-8=-1002/561, 8-9=-1315/775, 9-10=-1335/580, 10-11=-941/468
 BOT CHORD 2-15=-537/255, 14-15=-178/626, 13-14=-179/1006, 12-13=-253/1008, 11-12=-154/255
 WEBS 3-15=-330/338, 5-15=-1758/688, 5-14=-40/517, 6-14=0/149, 7-13=-187/165, 8-13=-39/266, 8-12=-290/326, 9-12=-392/405, 10-12=-260/873

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 168 lb uplift at joint 2, 700 lb uplift at joint 15 and 352 lb uplift at joint 11.

LOAD CASE(S) Standard



Job L161719	Truss T19G	Truss Type COMMON	Qty 1	Ply 1	0 0
Job Reference (optional)					

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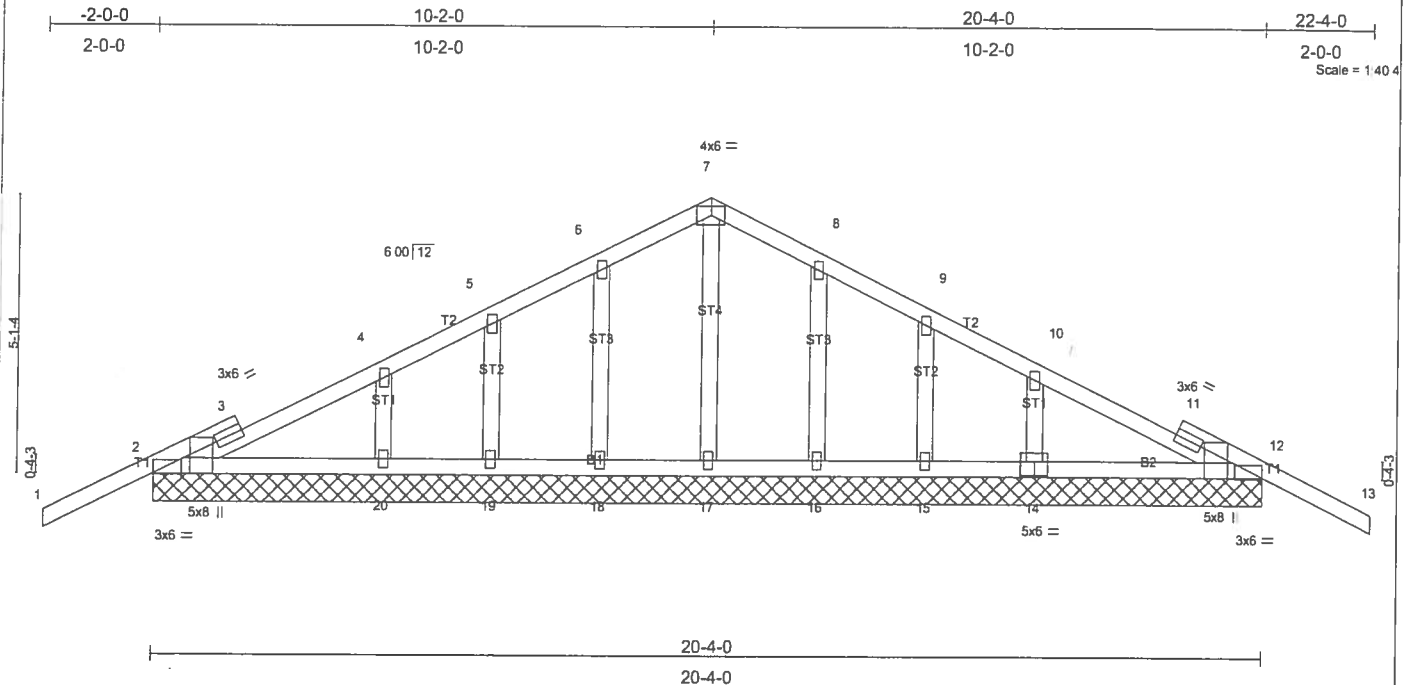


Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-8,Edge], [12:0-3-8,Edge], [12:0-0-8,Edge], [14:0-3-0,0-3-0]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.49	Vert(LL) -0.03	13	n/r	120		MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.09	Vert(TL) -0.04	13	n/r	90			
BCLL 10.0	Rep Stress Incr NO	WB 0.12	Horz(TL) 0.00	12	n/a	n/a			
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)							

Weight: 104 lb

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 OTHERS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=520/20-4-0, 12=520/20-4-0, 17=336/20-4-0, 18=312/20-4-0, 19=208/20-4-0, 20=484/20-4-0, 16=312/20-4-0, 15=208/20-4-0, 14=484/20-4-0
 Max Horz 2=-105(load case 6)
 Max Uplift 2=-274(load case 5), 12=-290(load case 6), 17=-15(load case 5), 18=-132(load case 5), 19=-122(load case 5), 20=-167(load case 5), 16=-131(load case 6), 15=-120(load case 6), 14=-172(load case 6)
 Max Grav 2=524(load case 9), 12=524(load case 10), 17=336(load case 1), 18=316(load case 9), 19=208(load case 1), 20=485(load case 9), 16=316(load case 10), 15=208(load case 1), 14=485(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-15/99, 2-3=-75/36, 3-4=-87/162, 4-5=-14/101, 5-6=0/127, 6-7=0/182, 7-8=0/182, 8-9=0/127, 9-10=-10/101, 10-11=-45/162, 11-12=-33/17, 12-13=-15/99
 BOT CHORD 2-20=-62/142, 19-20=-62/142, 18-19=-62/142, 17-18=-62/142, 16-17=-62/142, 15-16=-62/142, 14-15=-62/142, 12-14=-62/142
 WEBS 7-17=-279/27, 6-18=-248/160, 5-19=-175/146, 4-20=-365/217, 8-16=-248/160, 9-15=-175/146, 10-14=-365/217

NOTES

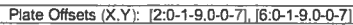
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 274 lb uplift at joint 2, 290 lb uplift at joint 12, 15 lb uplift at joint 17, 132 lb uplift at joint 18, 122 lb uplift at joint 19, 167 lb uplift at joint 20, 131 lb uplift at joint 16, 120 lb uplift at joint 15 and 172 lb uplift at joint 14.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-7=-114(F=-60), 7-13=-114(F=-60), 2-12=-30

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LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 4-7-10 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 9-0-8 oc bracing.

Weight: 97 lb

REACTIONS (lb/size) 2=1117/0-4-0, 6=1117/0-4-0
Max Horz 2=109(load case 6)
Max Uplift 2=461(load case 5), 6=461(load case 6)

FORCES (lb)- Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-1761/720, 3-4=-1615/719, 4-5=-1615/719, 5-6=-1761/720, 6-7=0/47
 BOT CHORD 2-10=-473/1503, 9-10=-237/1042, 8-9=-237/1042, 6-8=-473/1503
 WEBS 3-10=-224/209, 4-10=-245/676, 4-8=-244/676, 5-8=-224/209

NOTES

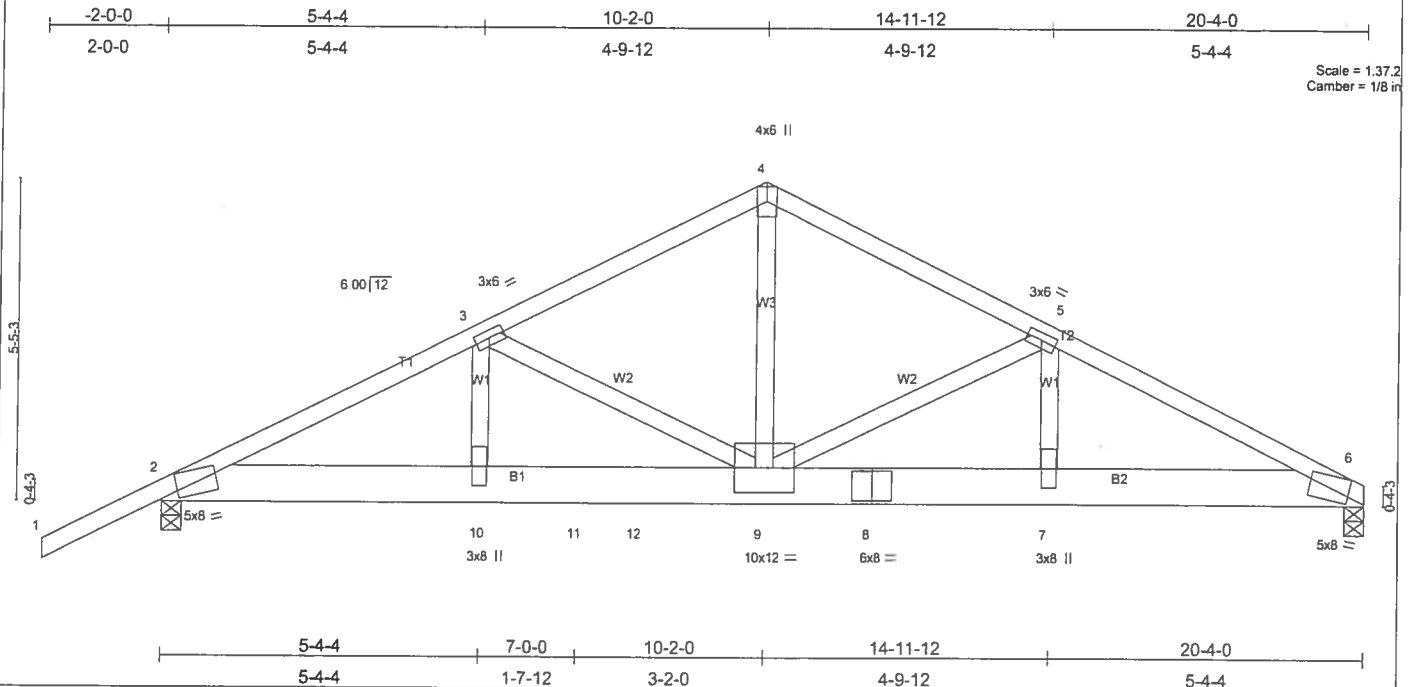
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); $h=20$ ft; $TCDF=4.2$ psf; $BCDF=3.0$ psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber $DOL=1.60$ plate grip $DOL=1.60$. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 461 lb uplift at joint 2 and 461 lb uplift at joint 6.
- 4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-10=-30, 8-10=-80(F=-50), 6-8=-30

Job TEMP	Truss T21	Truss Type COMMON	Qty 1	Ply 3	0 0
Builders FirstSource, Lake City, FL 32055			Job Reference (optional)		

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Scale = 1/37.2
Camber = 1/8 in

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.41	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.50	Vert(LL) -0.16 9-10 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.52	Vert(TL) -0.25 9-10 >942 180		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.05 6 n/a n/a		
	Code FBC2004/TP12002			Weight: 385 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 8 SYP 2400F 2.0E
 WEBS 2 X 4 SYP No.3 "Except"
 W3 2 X 4 SYP No.2

BRACING
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=8155/0-4-0, 2=5889/0-4-0
 Max Horz 2=142(load case 4)
 Max Uplift 6=3035(load case 5), 2=-2264(load case 4)

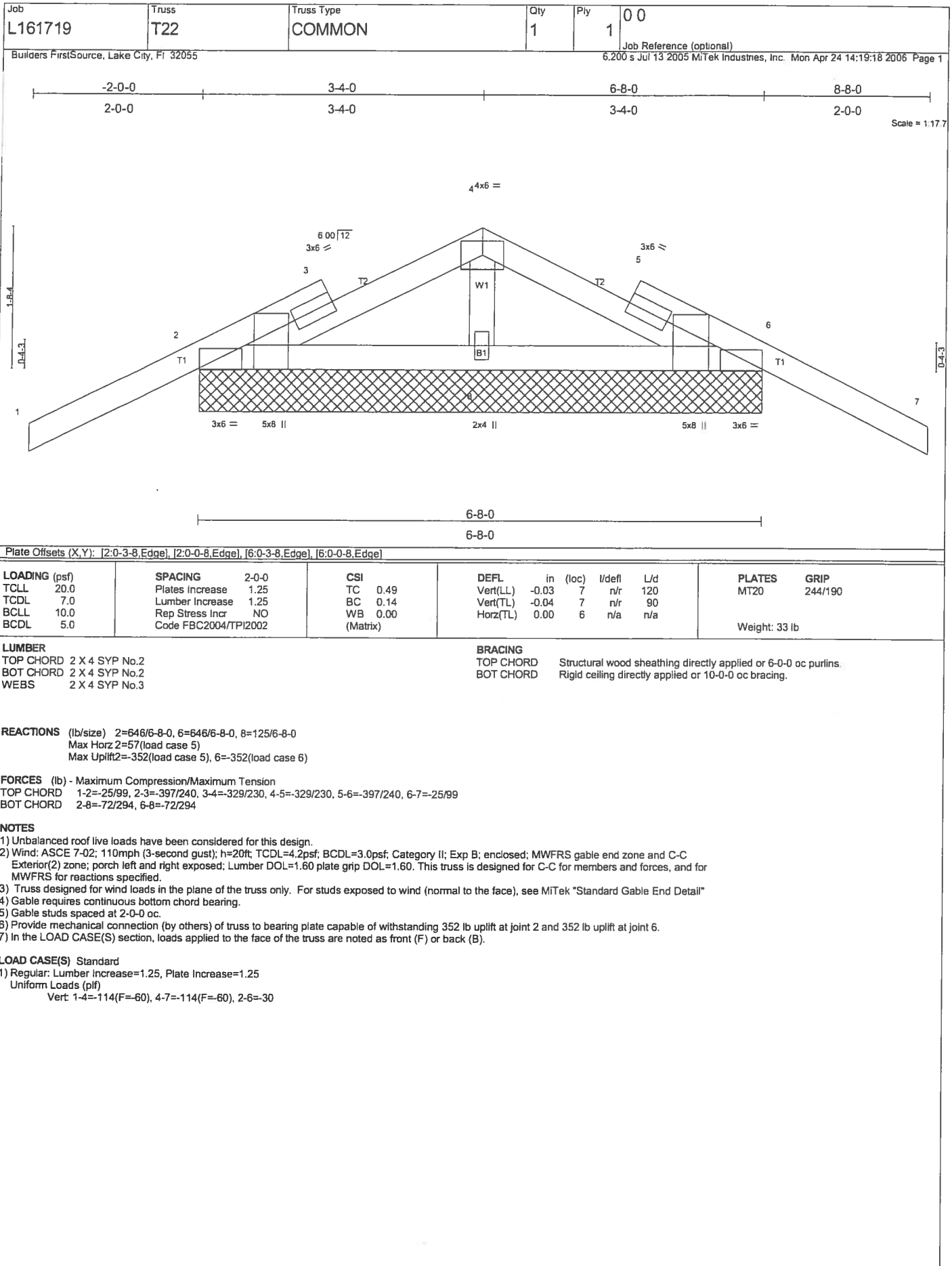
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/54, 2-3=-12548/4593, 3-4=-9819/3683, 4-5=-9822/3672, 5-6=-13791/5117
 BOT CHORD 2-10=-4114/11181, 10-11=-4114/11181, 11-12=-4114/11181, 9-12=-4114/11181, 8-9=-4522/12326, 7-8=-4522/12326, 6-7=-4522/12326
 WEBS 3-10=-764/2266, 3-9=-2767/1078, 4-9=-3110/8420, 5-9=-4064/1607, 5-7=-1288/3367

NOTES

- 3-ply truss to be connected together with 0.131"x3" Nails as follows:
 Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
 Bottom chords connected as follows: 2 X 8 - 3 rows at 0-4-0 oc.
 Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 3035 lb uplift at joint 6 and 2264 lb uplift at joint 2.
- Girder carries tie-in span(s): 37-0-0 from 8-0-0 to 20-4-0
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 3390 lb down and 1280 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-4=-54, 4-6=-54, 2-12=-30, 6-12=-758(F=728)
 Concentrated Loads (lb)
 Vert: 11=-3390(F)



Job L161719	Truss T22G	Truss Type COMMON	Qty 1	Ply 1	0 0
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Job Reference (optional)

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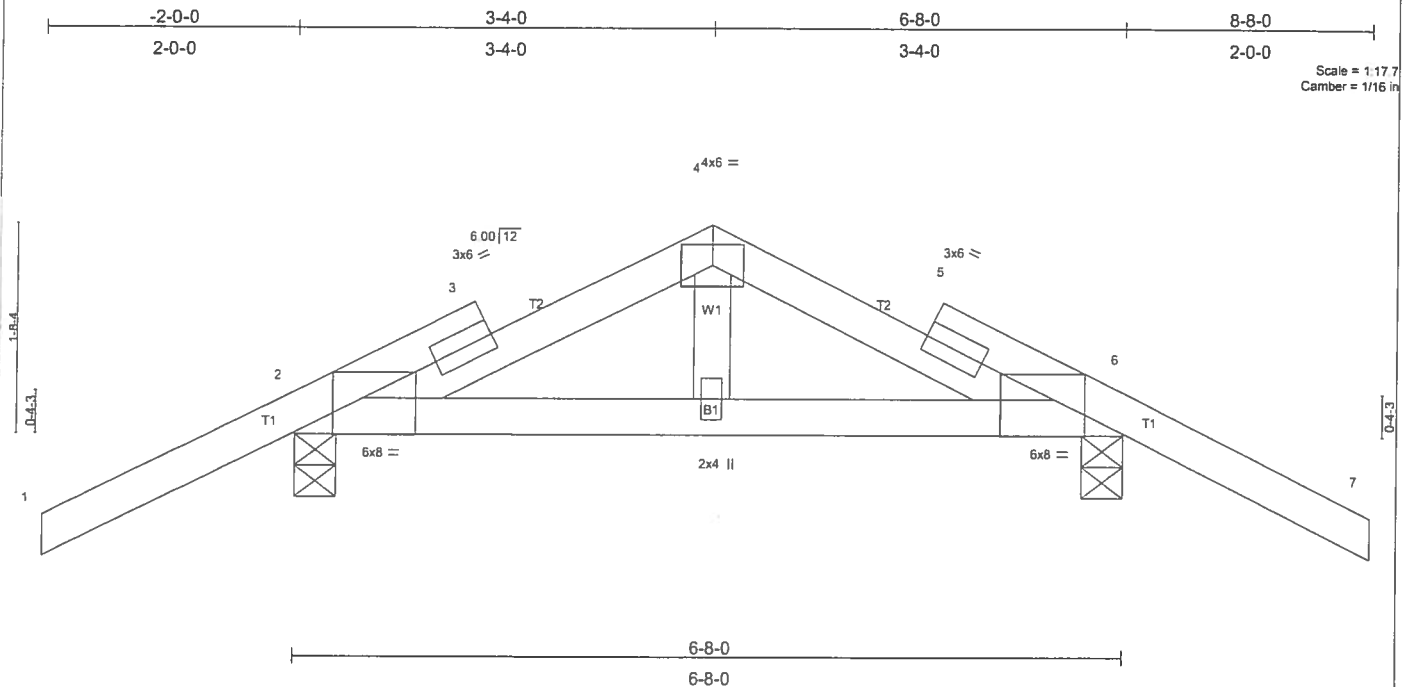


Plate Offsets (X,Y): [2-0-2-13,Edge], [6-0-2-13,Edge]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.58	Vert(LL)	0.26	2-6	>290	240	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.46	Vert(TL)	0.22	2-6	>339	180		
BCLL 10.0	Lumber Increase 1.25	WB 0.00	Horz(TL)	0.00	6	n/a	n/a		
BCDL 5.0	Rep Stress Incr NO	(Matrix)							
	Code FBC2004/TPI2002								
								Weight: 33 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=703/0-4-0, 6=703/0-4-0

Max Horz 2=57(load case 5)

Max Uplift 2=427(load case 5), 6=427(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

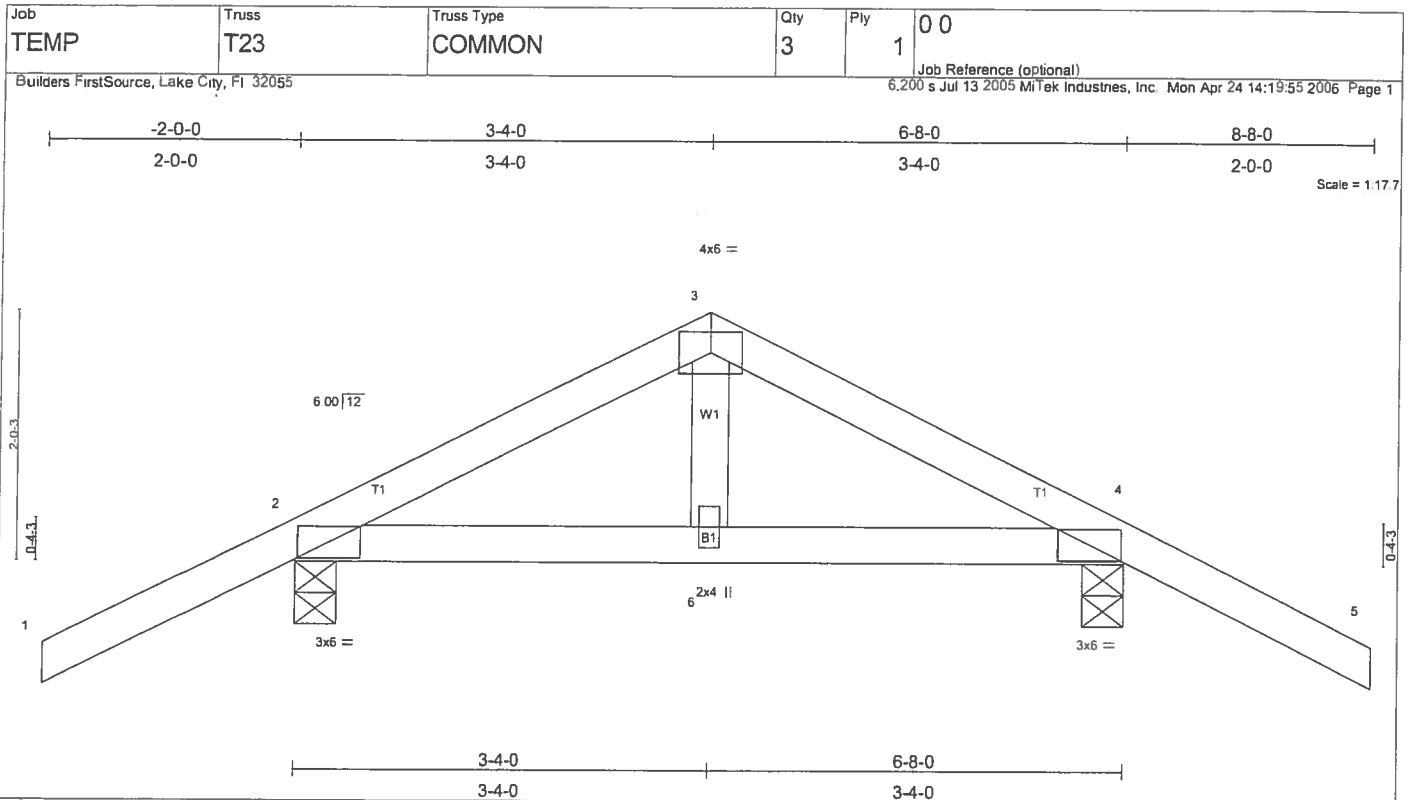
TOP CHORD 1-2=-25/100, 2-3=-330/182, 3-4=-247/168, 4-5=-247/168, 5-6=-330/182, 6-7=-25/100
 BOT CHORD 2-6=-36/221

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) Gable studs spaced at 2-0-0 oc.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 427 lb uplift at joint 2 and 427 lb uplift at joint 6.
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-4=-114(F=-60), 4-7=-114(F=-60), 2-6=-30



LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.30	Vert(LL)	0.01	4-6	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.09	Vert(TL)	0.01	4-6	>999	180		
BCLL 10.0	Rep Stress Incr YES	WB 0.04	Horz(TL)	0.00	4	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)							

Weight: 30 lb

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=383/0-4-0, 4=383/0-4-0
 Max Horz 2=62(load case 5)
 Max Uplift 2=305(load case 5), 4=305(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=276/320, 3-4=276/320, 4-5=0/47
 BOT CHORD 2-6=-115/191, 4-6=-115/191
 WEBS 3-6=-217/115

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 305 lb uplift at joint 2 and 305 lb uplift at joint 4.

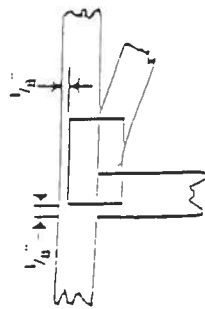
LOAD CASE(S) Standard

Symbols

PLATE LOCATION AND ORIENTATION



- Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



- For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

- This symbol indicates the required direction of slats in connector plates.

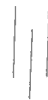


PLATE SIZE

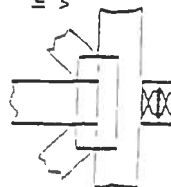
- The first dimension is the width perpendicular to slats. Second dimension is the length parallel to slats.

LATERAL BRACING



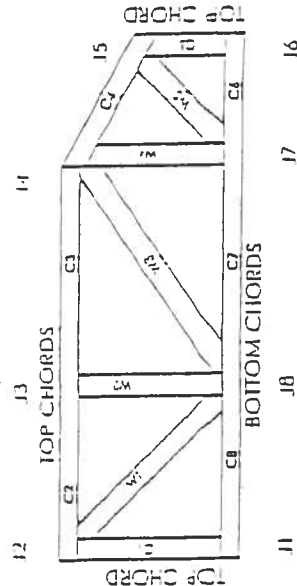
- Indicates location of required continuous lateral bracing.

BEARINGS



- Indicates location of joints at which bearings (supports) occur.

Numbering System

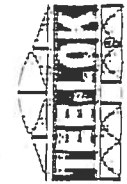


JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SWCCI	9667, 9432A
WISC/DIIR	960022 W, 970036-11
IIR	561



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
- Unless otherwise noted, locate chord splices at 1/4 panel length (1.5' from adjacent joint).
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size and location dimensions shown indicate minimum plating requirements.
- Lumber shall be of the species and size and in all respects, equal to or better than the grade specified.
- Top chords must be sheathed or purlins provided at spacing shown on design.
- Bottom chords require lateral bracing at 10 ft spacing, or less, if no ceiling is installed, unless otherwise noted.
- Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
- Do not overload roof or floor trusses with stacks of construction materials.
- Do not cut or alter truss member or plate without prior approval of a professional engineer.
- Care should be exercised in handling, erection and installation of trusses.

Notice of Treatment

11647

Applicator: **Florida Pest Control & Chemical Co. (www.flapest.com)**

Address: BAYA AVE

City L.I.C. Phone 752-1203

Site Location: Subdivision Wise Estates

Lot # 3 Block# Permit # 24511

Address 172 SW Wise DR

<u>Product used</u>	<u>Active Ingredient</u>	<u>% Concentration</u>
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☐ Premise	Imidacloprid	0.1%
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☒ Termidor	Fipronil	0.12%
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☐ Bora-Care	Disodium Octaborate Tetrahydrate	23.0%
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Type treatment:

☒ Soil

☐ Wood

<u>Area Treated</u>	<u>Square feet</u>	<u>Linear feet</u>	<u>Gallons Applied</u>
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Dwelling

2234

201

200

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____.

6/14/06

Date

1200

Time

F254

Print Technician's Name

Remarks: _____

Applicator - White

Permit File - Canary

Permit Holder - Pink

10/05



COLUMBIA COUNTY OFFICE OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 23-4S-16-03113-103

Building permit No. 000024511

Use Classification SFD, UTILITY

Fire: 67.00

Permit Holder MICHAEL JENKINS

Waste: 201.00

Owner of Building JAMES & SUSAN SPARKS

Total: 268.00

Location: 172 SW WISE DRIVE

Date: 10/11/2006



Harry Bieker

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)