Project Information for:	L106588		
Builder:	Aaron Simque	Date:	3/8/2005
Lot:	Lot 5	Start Number:	252
Subdivision:	Hunters Ridge		
County or City:	Columbia County		
Truss Page Count:	40		

Truss Design Load Information (UNO) Design Program: MiTek 5.2

Gravity		Wind		Building Code:	FBC2001
Roof (psf):	42	Wind Standard:	ASCE 7-98		
Floor (psf):	55	Wind Speed (mph):	110		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

SIMQUE, AARON D. RB29003130
Address: 320 SW AINSLEY GLN.
LAKE CITY, FL. 32024 Designer: 62

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:
1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI 1-1995 Section 2.2
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	030805252	3/8/2005				
2	CJ3	030805253	3/8/2005				
3	CJ5	030805254	3/8/2005				
4	EJ7	030805255	3/8/2005				
5	HJ7	030805256	3/8/2005				
6	PB01	030805257	3/8/2005				
7	PB02	030805258	3/8/2005				
8	PB04	030805259	3/8/2005				
9	PB05	030805260	3/8/2005				
10	PB06	030805261	3/8/2005				
11	PB07	030805262	3/8/2005				
12	T01G	030805263	3/8/2005				
13	T02	030805264	3/8/2005				
14	T02G	030805265	3/8/2005				
15	T03	030805266	3/8/2005				
16	T04	030805267	3/8/2005				
17	T04G	030805268	3/8/2005				
18	T05	030805269	3/8/2005				
19	T07	030805270	3/8/2005				
20	T08	030805271	3/8/2005				
21	T09	030805272	3/8/2005				
22	T10	030805273	3/8/2005				
23	T11	030805274	3/8/2005				
24	T12	030805275	3/8/2005				
25	T13	030805276	3/8/2005				
26	T13A	030805277	3/8/2005				
27	T13B	030805278	3/8/2005				
28	T14	030805279	3/8/2005				
29	T16	030805280	3/8/2005				
30	T17	030805281	3/8/2005				
31	T18	030805282	3/8/2005				
32	T19	030805283	3/8/2005				
33	T20	030805284	3/8/2005				
34	T21	030805285	3/8/2005				
35	T22	030805286	3/8/2005				
36	T23	030805287	3/8/2005				
37	T25	030805288	3/8/2005				
38	T26	030805289	3/8/2005				
39	T27	030805290	3/8/2005				
40	T28	030805291	3/8/2005				

MAR 08 2005

Thomas E. Miller, FL PE 56877, EB #9196

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Licensee Details

Licensee Information

Name:	**SIMQUE, AARON DAVID (Primary Name)**
	AARON SIMQUE HOMES INC (DBA Name)
Main Address:	**320 SW AINSLEY GLN.** **LAKE CITY, Florida 32024**

License Information

License Type:	**Registered Building Contractor**
Rank:	**Reg Building**
License Number:	**RB29003130**
Status:	**Current, Active**
Licensure Date:	**10/23/2002**
Expires:	**08/31/2005**

Special Qualifications	Effective Date
Bldg Code Core Course Credit	
Qualified Business License Required	02/20/2004

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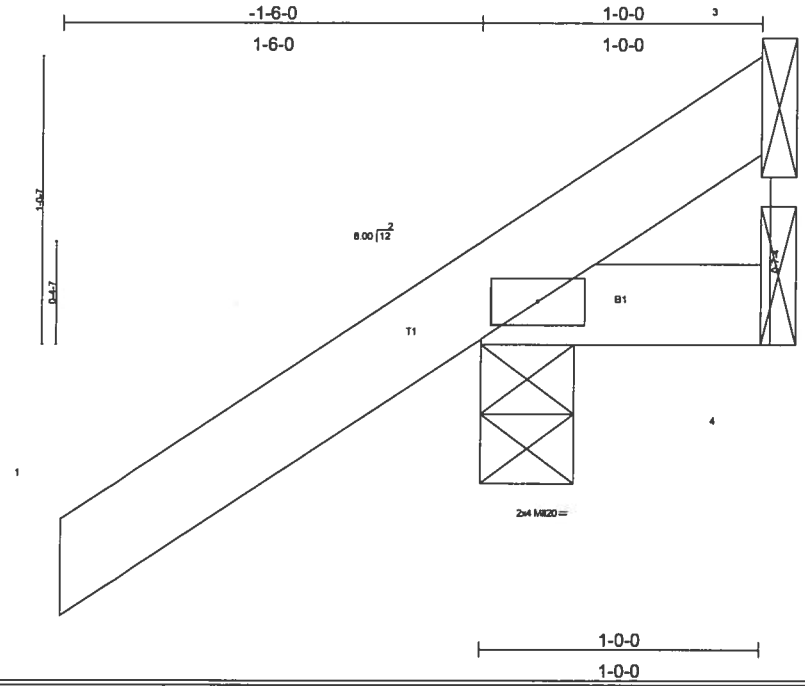
New Search E

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR
L106588	CJ1	MONO TRUSS	8	1	Job Reference (optional)

Dwg.#030805252

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Dec 15 2004 MiTek Industries, Inc. Mon Mar 07 16:55:59 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.14	Vert(LL)	-0.00	2	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 6 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=189/0-4-0, 4=14/Mechanical, 3=-41/Mechanical
Max Horz 2=94(load case 5)
Max Uplift2=-189(load case 5), 3=-41(load case 1)
Max Grav 2=189(load case 1), 4=14(load case 1), 3=70(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-55/48
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 189 lb uplift at joint 2 and 41 lb uplift at joint 3.

LOAD CASE(S) Standard

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR
L106588	CJ3	MONO TRUSS	8	1	Job Reference (optional)

Dwg.#030805253

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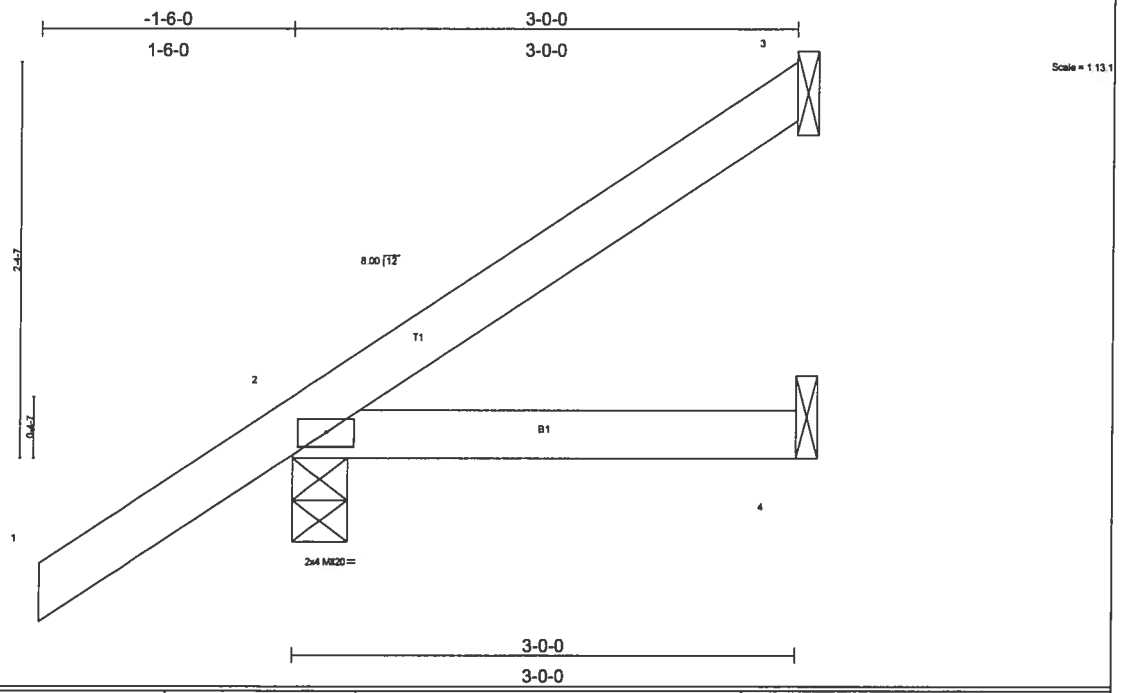

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.16	Vert(LL)	-0.00	2-4	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.05	Vert(TL)	-0.00	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=48/Mechanical, 2=233/0-4-0, 4=42/Mechanical
Max Horz 2=154(load case 5)
Max Uplift3=-47(load case 5), 2=-143(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-60/19
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 47 lb uplift at joint 3 and 143 lb uplift at joint 2.

LOAD CASE(S) Standard

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR
L106588	CJ5	MONO TRUSS	8	1	Job Reference (optional)

Dwg.#030805254

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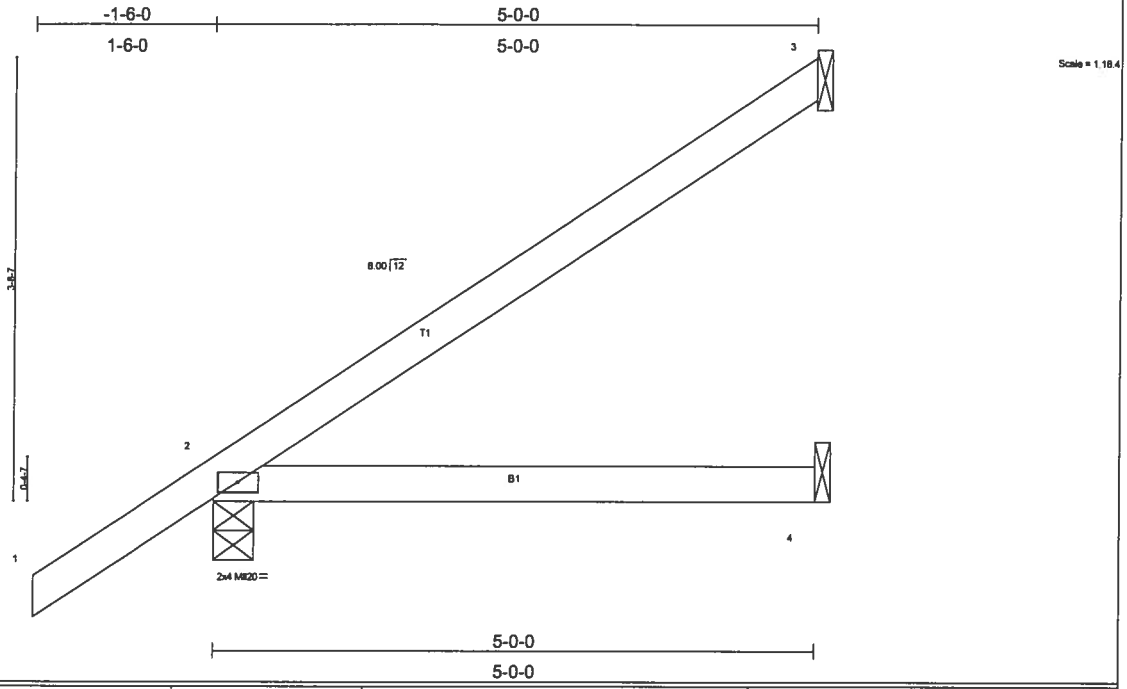

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.19	Vert(LL)	-0.03	2-4	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.14	Vert(TL)	-0.04	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=113/Mechanical, 2=306/0-4-0, 4=72/Mechanical
Max Horz 2=215(load case 5)
Max Uplift3=-121(load case 5), 2=-138(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-100/50
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 121 lb uplift at joint 3 and 138 lb uplift at joint 2.

LOAD CASE(S) Standard

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR
L106588	EJ7	MONO TRUSS	20	1	Job Reference (optional)

Dwg.#030805255

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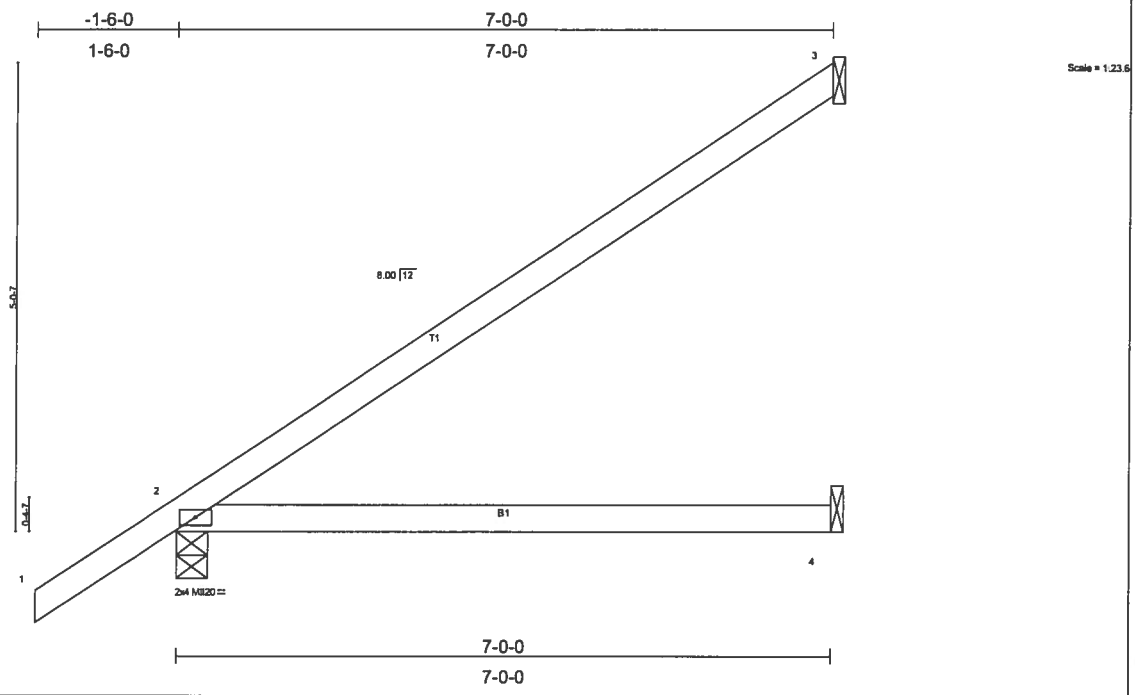

Plate Offsets (X,Y): [2:0-0-0,0-0-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.10	2-4	>783	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.28	Vert(TL)	-0.16	2-4	>522	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=172/Mechanical, 2=385/0-4-0, 4=102/Mechanical
Max Horz 2=277(load case 5)
Max Uplift3=-186(load case 5), 2=-142(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-146/77
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 186 lb uplift at joint 3 and 142 lb uplift at joint 2.

LOAD CASE(S) Standard

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR
L106588	HJ7	MONO TRUSS	4	1	Job Reference (optional)

Dwg.#030805256

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Dec 15 2004 MiTek Industries, Inc. Mon Mar 07 16:56:01 2005 Page 1

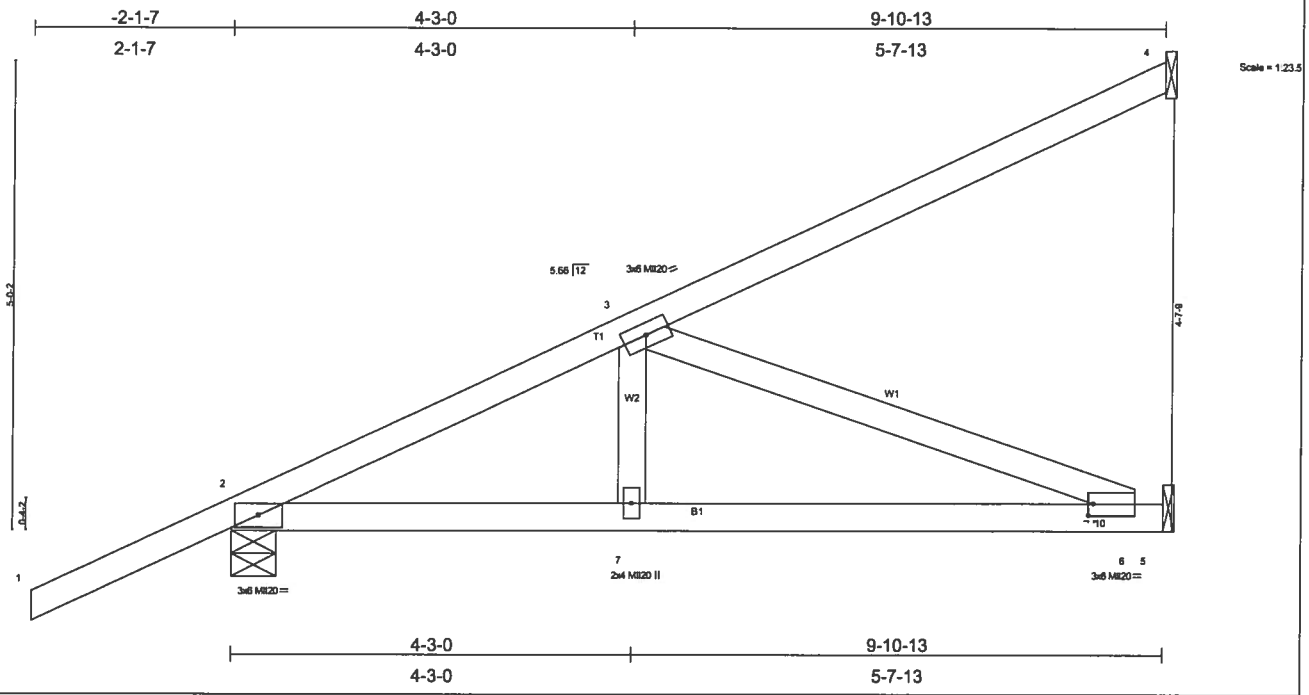

Plate Offsets (X,Y): [6:0-0-10,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.09	6-7	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.12	6-7	>922	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.43	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 46 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=266/Mechanical, 2=486/0-5-11, 5=389/Mechanical
Max Horz 2=342(load case 4)
Max Uplift4=-266(load case 4), 2=-184(load case 4), 5=-97(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/48, 2-3=-743/56, 3-4=-149/82
BOT CHORD 2-7=-330/660, 6-7=-330/660, 5-6=0/0
WEBS 3-7=0/201, 3-6=-706/353

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 266 lb uplift at joint 4, 184 lb uplift at joint 2 and 97 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=0(F=15, B=15)-to-5=-74(F=-22, B=-22)

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR
L106588	PB01	HIP PIGGYBACK	1	1	Job Reference (optional)

Dwg.#030805257

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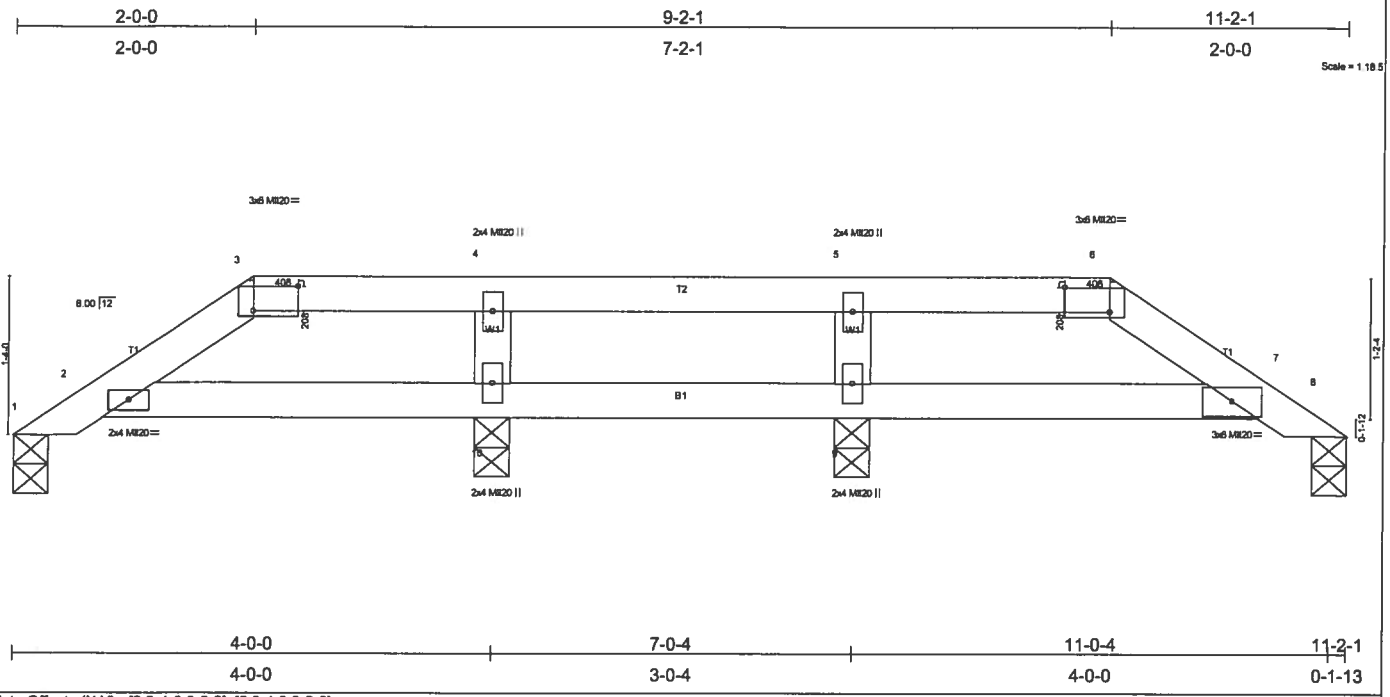

Plate Offsets (X,Y): [3:0-4-8,0-2-8], [6:0-4-8,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.14	Vert(LL)	0.01	7-9	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.01	7-9	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.01	8	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 34 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=159/0-3-8, 10=277/0-3-8, 9=292/0-3-8, 8=163/0-3-8
Max Horz 1=-44(load case 3)
Max Uplift1=-51(load case 5), 10=-142(load case 4), 9=-142(load case 3), 8=-55(load case 6)
Max Grav 1=159(load case 1), 10=291(load case 7), 9=305(load case 8), 8=163(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-82/47, 2-3=-161/106, 3-4=-131/108, 4-5=-131/108, 5-6=-131/108, 6-7=-161/107, 7-8=-84/48
BOT CHORD 2-10=-57/131, 9-10=-57/131, 7-9=-57/131
WEBS 4-10=-172/150, 5-9=-181/157

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 51 lb uplift at joint 1, 142 lb uplift at joint 10, 142 lb uplift at joint 9 and 55 lb uplift at joint 8.

LOAD CASE(S) Standard

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR
L106588	PB02	HIP PIGGYBACK	1	1	Job Reference (optional)

Dwg.#030805258

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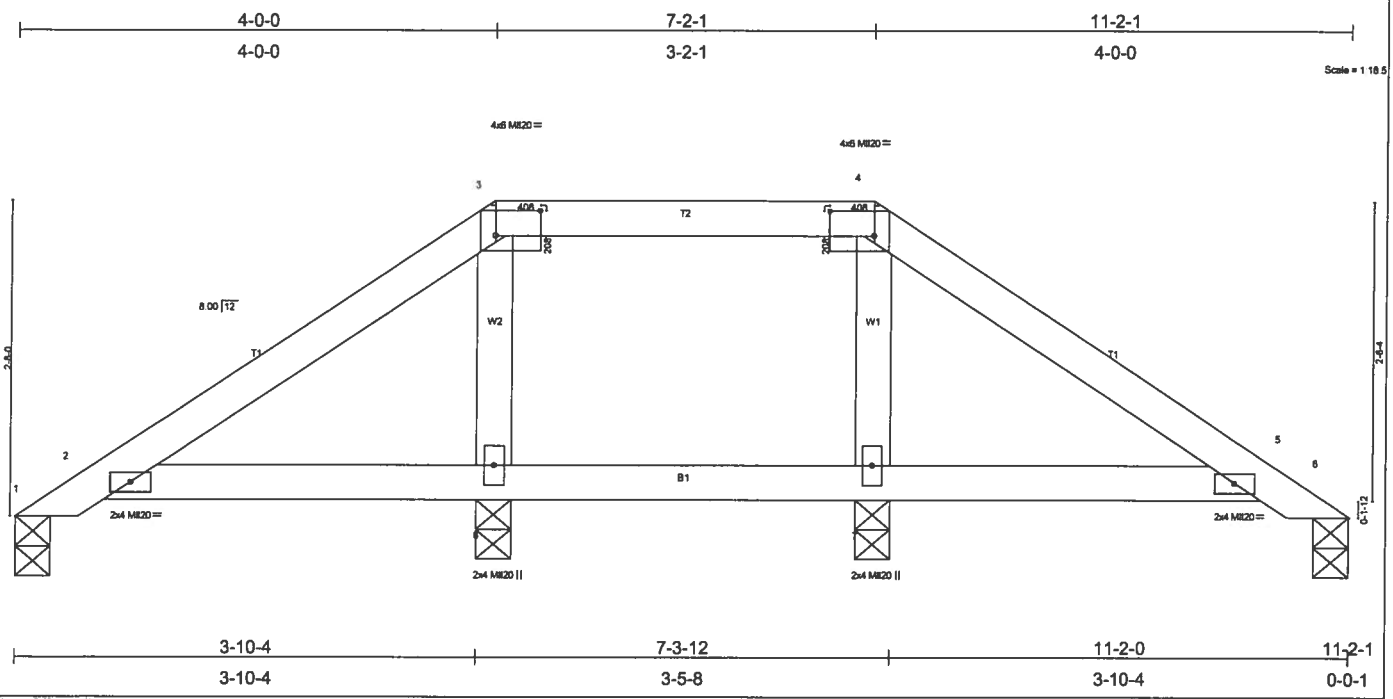


Plate Offsets (X,Y): [3:0-4-8,0-2-8], [4:0-4-8,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.11	Vert(LL)	0.01	2-8	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	0.01	2-8	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 39 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=40/0-3-8, 8=406/0-3-8, 7=406/0-3-8, 6=40/0-3-8
Max Horz 1=90(load case 4)
Max Uplift1=-15(load case 3), 8=-154(load case 4), 7=-129(load case 6), 6=-34(load case 3)
Max Grav 1=62(load case 7), 8=406(load case 1), 7=406(load case 1), 6=63(load case 8)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-79/89, 2-3=-101/241, 3-4=-25/152, 4-5=-83/241, 5-6=-31/23
BOT CHORD 2-8=-152/129, 7-8=-152/129, 5-7=-151/129
WEBS 3-8=-291/192, 4-7=-291/192

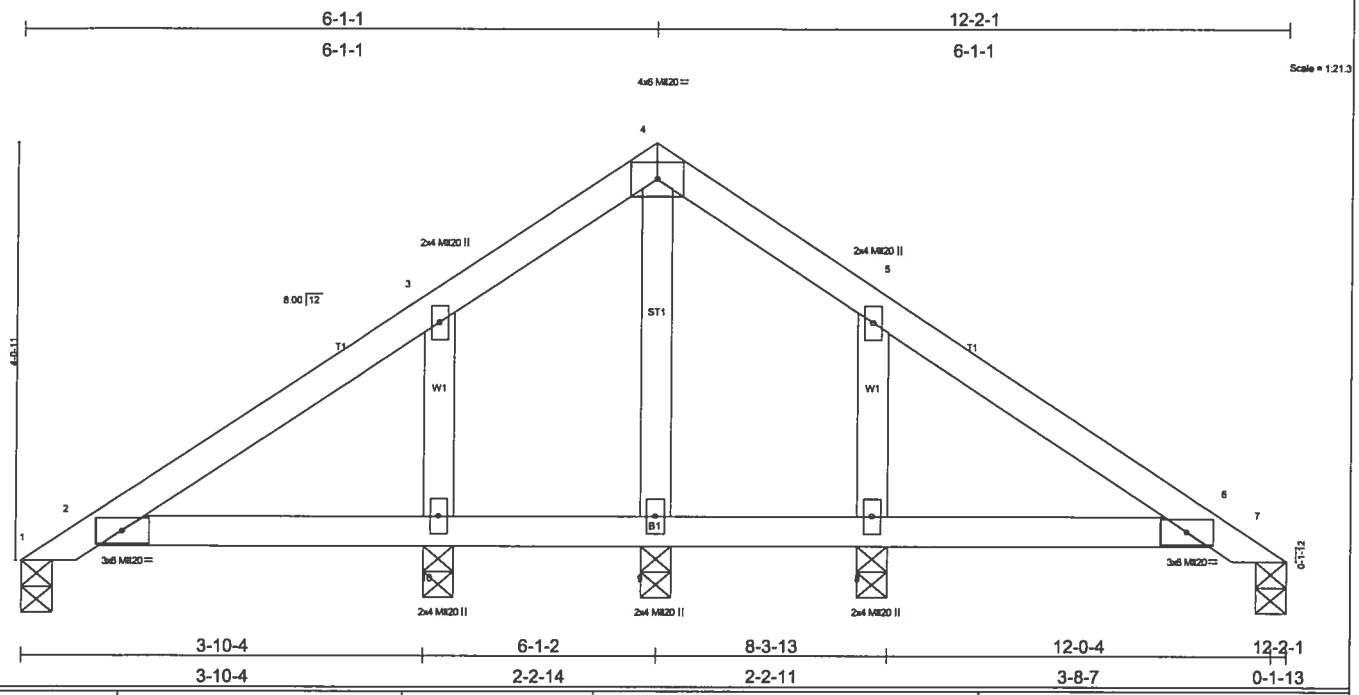
NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 15 lb uplift at joint 1, 154 lb uplift at joint 8, 129 lb uplift at joint 7 and 34 lb uplift at joint 6.

LOAD CASE(S) Standard

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR	Dwg.#030805259
L106588	PB04	PIGGYBACK	8	1	Job Reference (optional)	

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Dec 15 2004 MiTek Industries, Inc. Mon Mar 07 16:56:03 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.11	Vert(LL)	0.01	2-10	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	2-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.00	7	n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 48 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=51/0-3-8, 9=243/0-3-8, 10=315/0-3-8, 8=315/0-3-8, 7=51/0-3-8
Max Horz 1=138(load case 4)
Max Uplift1=-29(load case 3), 9=-12(load case 5), 10=-192(load case 5), 8=-183(load case 6), 7=-16(load case 6)
Max Grav 1=68(load case 7), 9=243(load case 1), 10=320(load case 7), 8=320(load case 8), 7=68(load case 8)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-137/137, 2-3=-124/214, 3-4=-28/165, 4-5=-13/165, 5-6=-94/214, 6-7=-34/13
BOT CHORD 2-10=-130/138, 9-10=-130/138, 8-9=-130/138, 6-8=-130/138
WEBS 4-9=-229/35, 3-10=-192/184, 5-8=-192/180

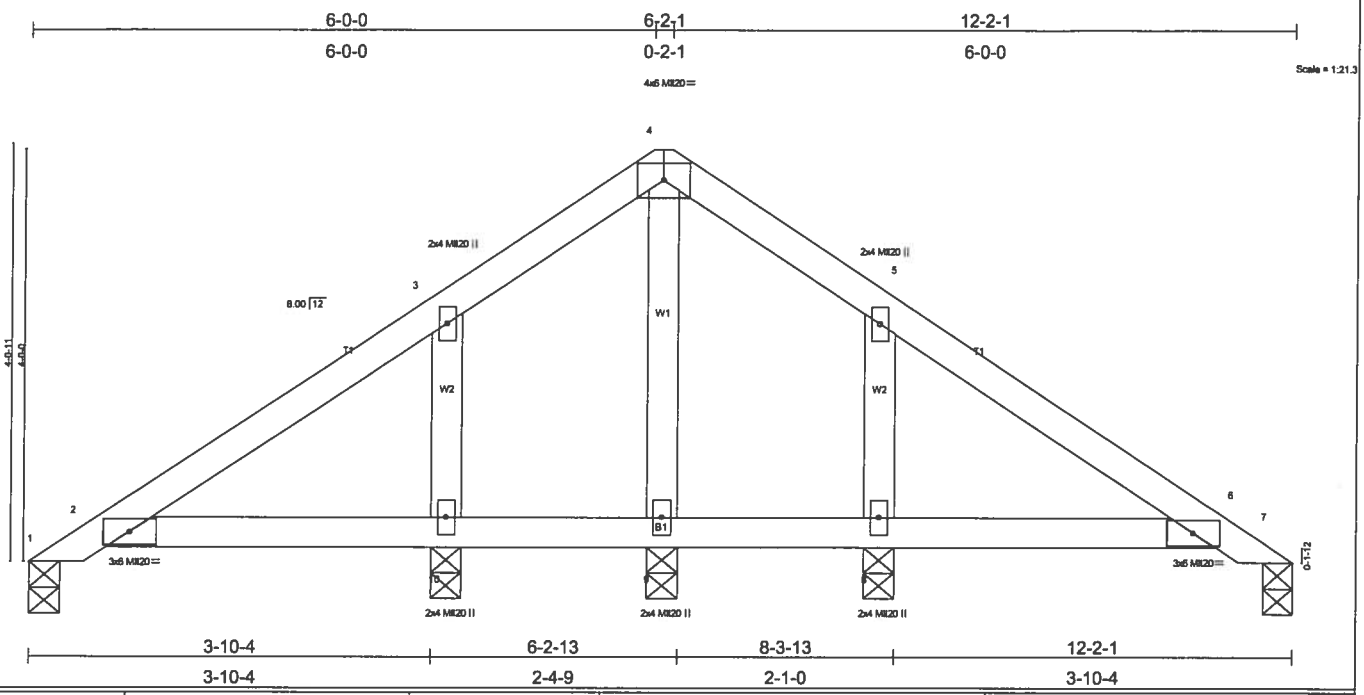
NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 29 lb uplift at joint 1, 12 lb uplift at joint 9, 192 lb uplift at joint 10, 183 lb uplift at joint 8 and 16 lb uplift at joint 7.

LOAD CASE(S) Standard

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR	Dwg.#030805260
L106588	PB05	HIP PIGGYBACK	1	1	Job Reference (optional)	

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Dec 15 2004 MiTek Industries, Inc. Mon Mar 07 16:56:04 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.11	Vert(LL)	0.01	2-10	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	2-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.00	7	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 48 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=51/0-3-8, 7=51/0-3-8, 9=243/0-3-8, 10=315/0-3-8, 8=315/0-3-8
Max Horz 1=-138(load case 3)
Max Uplift1=-29(load case 3), 7=-16(load case 6), 9=-12(load case 5), 10=-192(load case 5), 8=-183(load case 6)
Max Grav 1=68(load case 7), 7=68(load case 8), 9=243(load case 1), 10=320(load case 7), 8=320(load case 8)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-137/137, 2-3=-124/214, 3-4=-28/165, 4-5=-13/165, 5-6=-94/214, 6-7=-34/13
BOT CHORD 2-10=-130/138, 9-10=-130/138, 8-9=-130/138, 6-8=-130/138
WEBS 4-9=-229/35, 3-10=-192/184, 5-8=-192/180

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 29 lb uplift at joint 1, 16 lb uplift at joint 7, 12 lb uplift at joint 9 , 192 lb uplift at joint 10 and 183 lb uplift at joint 8.

LOAD CASE(S) Standard

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR	Dwg.#030805261
L106588	PB06	HIP PIGGYBACK	1	1	Job Reference (optional)	

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Dec 15 2004 MiTek Industries, Inc. Mon Mar 07 16:56.05 2005 Page 1

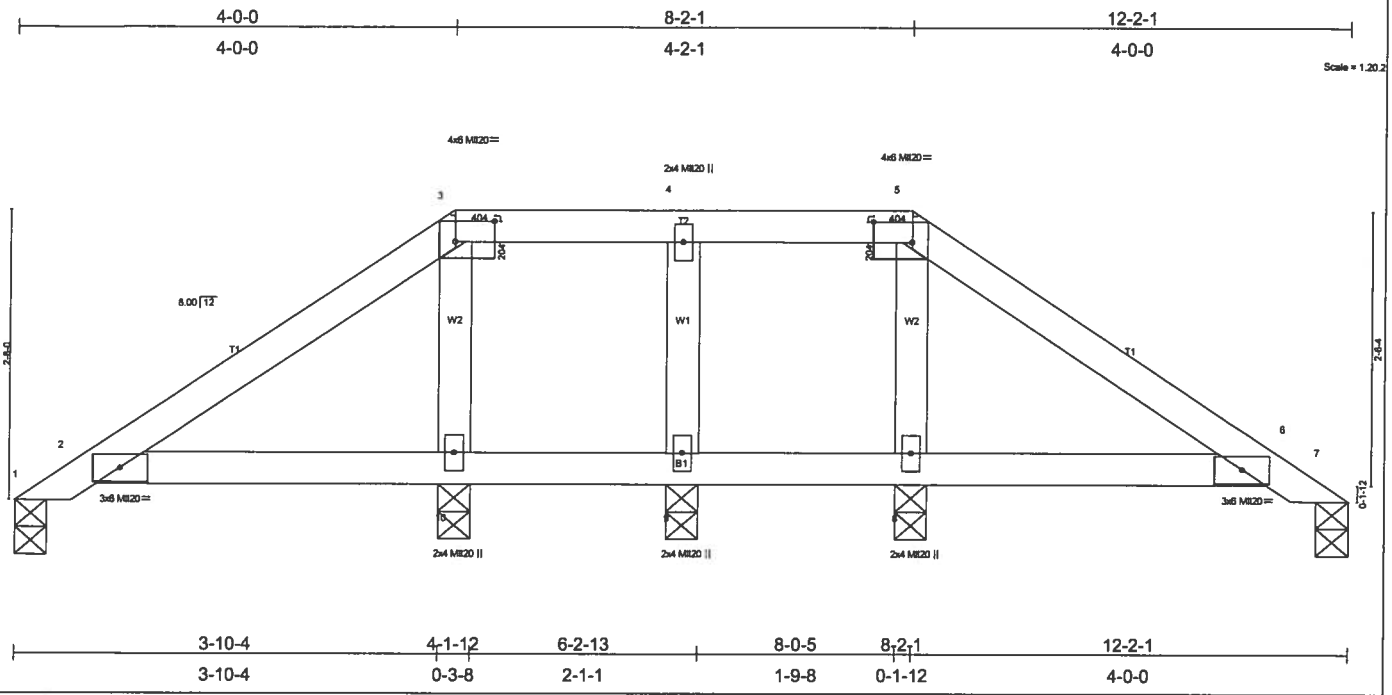


Plate Offsets (X,Y): [3:0-4-4,0-2-4], [5:0-4-4,0-2-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.11	Vert(LL)	0.01	2-10	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	0.01	2-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.00	7	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=43/0-3-8, 7=43/0-3-8, 9=83/0-3-8, 10=404/0-3-8, 8=403/0-3-8
Max Horz 1=90(load case 4)
Max Uplift1=-15(load case 6), 7=-31(load case 3), 9=-91(load case 3), 10=-164(load case 5), 8=-140(load case 6)
Max Grav 1=63(load case 7), 7=63(load case 8), 9=112(load case 7), 10=404(load case 1), 8=403(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-81/87, 2-3=-89/232, 3-4=-14/145, 4-5=-14/145, 5-6=-75/232, 6-7=-31/21
BOT CHORD 2-10=-145/123, 9-10=-145/123, 8-9=-145/123, 6-8=-145/123
WEBS 4-9=-88/100, 3-10=-279/177, 5-8=-279/177

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 15 lb uplift at joint 1, 31 lb uplift at joint 7, 91 lb uplift at joint 9 , 164 lb uplift at joint 10 and 140 lb uplift at joint 8.

LOAD CASE(S) Standard

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR
L106588	PB07	HIP PIGGYBACK	1	1	Job Reference (optional)

Dwg.#030805262

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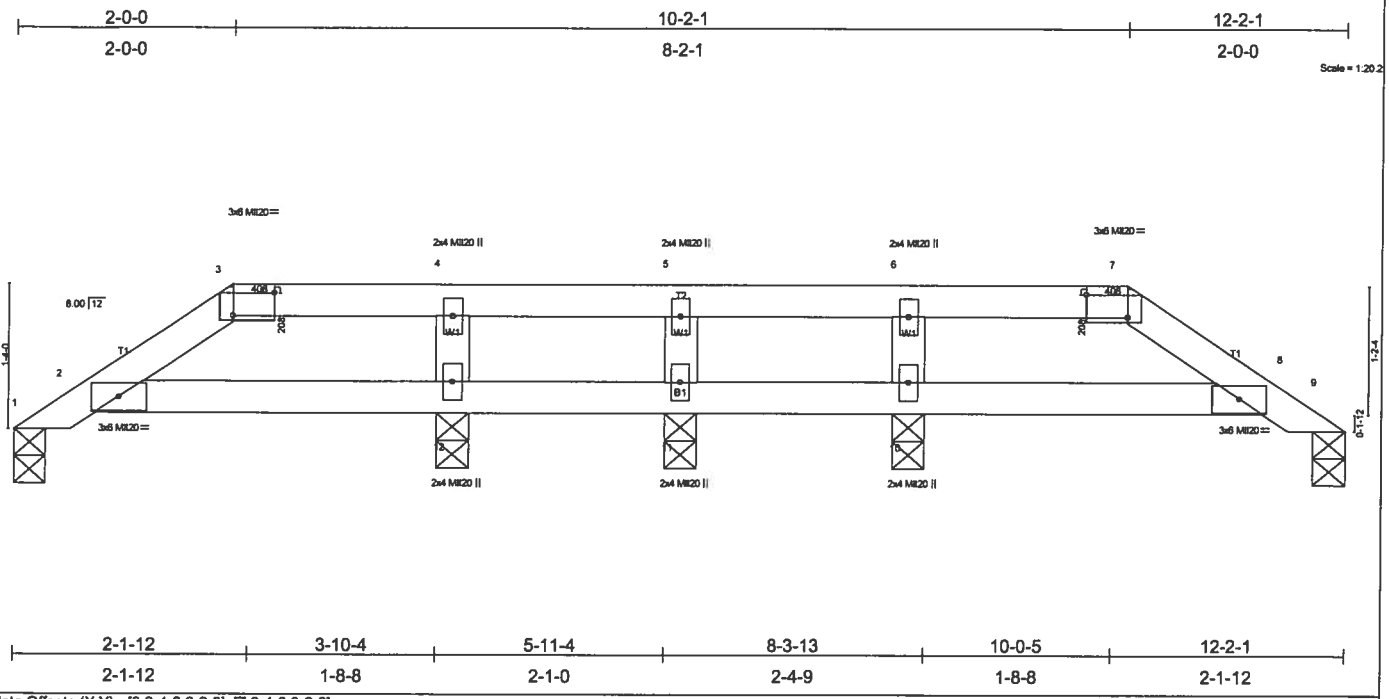

Plate Offsets (X,Y): [3:0-4-8,0-2-8], [7:0-4-8,0-2-8]

LOADING (psf)		SPACING 2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.13	Vert(LL)	0.01	2-12	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.01	2-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.03	Horz(TL)	0.01	9	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 38 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=154/0-3-8, 9=154/0-3-8, 12=294/0-3-8, 10=294/0-3-8, 11=80/0-3-8
Max Horz 1=44(load case 4)
Max Uplift1=-50(load case 5), 9=-54(load case 6), 12=-128(load case 4), 10=-116(load case 3), 11=-74(load case 3)
Max Grav 1=154(load case 1), 9=154(load case 1), 12=297(load case 7), 10=297(load case 8), 11=92(load case 8)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-79/48, 2-3=-145/101, 3-4=-117/103, 4-5=-117/103, 5-6=-117/103, 6-7=-117/103, 7-8=-145/101, 8-9=-79/45
BOT CHORD 2-12=-59/117, 11-12=-59/117, 10-11=-59/117, 8-10=-59/117
WEBS 4-12=-164/129, 6-10=-164/129, 5-11=-78/87

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 50 lb uplift at joint 1, 54 lb uplift at joint 9, 128 lb uplift at joint 12, 116 lb uplift at joint 10 and 74 lb uplift at joint 11.

LOAD CASE(S) Standard

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR
L106588	T01G	COMMON	1	1	Job Reference (optional)

Dwg.#030805263

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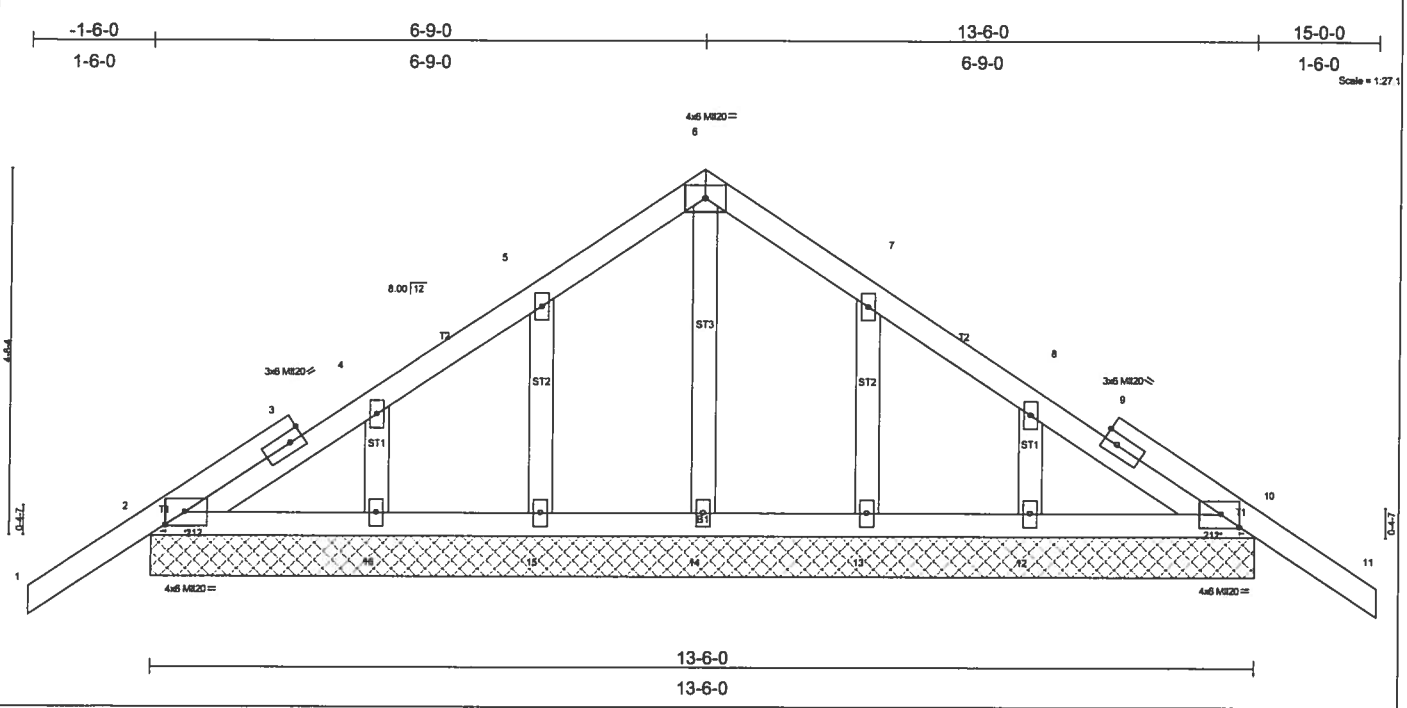

Plate Offsets (X,Y): [2:0-2-12,0-2-0], [10:0-2-12,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.25	Vert(LL)	-0.01	11	n/r	90	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.03	Vert(TL)	-0.02	11	n/r	80		
BCLL	10.0	Rep Stress Incr	NO	WB	0.06	Horz(TL)	0.00	10	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 72 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=411/13-6-0, 10=411/13-6-0, 14=254/13-6-0, 15=281/13-6-0, 16=325/13-6-0, 13=281/13-6-0, 12=325/13-6-0
Max Horz 2=151(load case 4)
Max Uplift2=-194(load case 5), 10=-216(load case 6), 14=-6(load case 4), 15=-157(load case 5), 16=-123(load case 5), 13=-154(load case 6), 12=-129(load case 6)
Max Grav 2=411(load case 1), 10=411(load case 1), 14=254(load case 1), 15=286(load case 7), 16=325(load case 1), 13=286(load case 8), 12=325(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-11/91, 2-3=-114/101, 3-4=-99/99, 4-5=-98/108, 5-6=-97/146, 6-7=-97/146, 7-8=-98/75, 8-9=-44/44, 9-10=-114/45, 10-11=-11/91
BOT CHORD 2-16=-19/121, 15-16=-19/121, 14-15=-19/121, 13-14=-19/121, 12-13=-19/121, 10-12=-19/121
WEBS 6-14=-192/20, 5-15=-230/165, 4-16=-253/148, 7-13=-230/162, 8-12=-253/152

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MII20 unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 194 lb uplift at joint 2, 216 lb uplift at joint 10, 6 lb uplift at joint 14, 157 lb uplift at joint 15, 123 lb uplift at joint 16, 154 lb uplift at joint 13 and 129 lb uplift at joint 12.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-6=-114(F=-60), 6-11=-114(F=-60), 2-10=-30

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR
L106588	T02	SPECIAL	7	1	Job Reference (optional)

Dwg.#030805264

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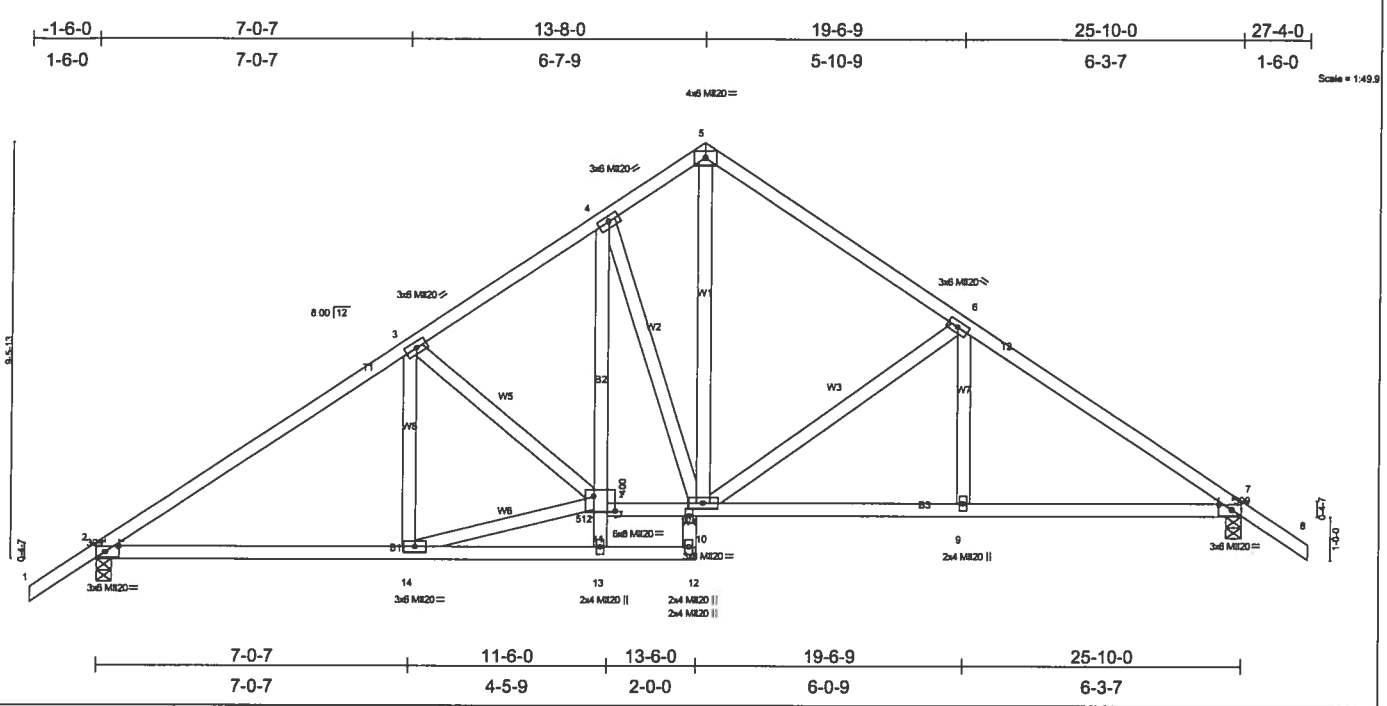

Plate Offsets (X,Y): [2:0-3-9,0-1-8], [7:0-3-9,0-1-8], [11:0-5-12,0-4-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.11	12	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.16	12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.51	Horz(TL)	0.05	7	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 164 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D *Except*
B2 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 4-10-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
JOINTS 1 Brace at Jt(s): 11

REACTIONS (lb/size) 2=1194/0-4-0, 7=1192/0-4-0
Max Horz 2=319(load case 4)
Max Uplift2=-406(load case 5), 7=-390(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-1597/504, 3-4=-1401/515, 4-5=-1073/482, 5-6=-1169/464, 6-7=-1627/508, 7-8=0/45
BOT CHORD 2-14=-394/1234, 13-14=-27/50, 12-13=0/0, 11-13=0/117, 4-11=-211/609, 10-11=-230/1086, 9-10=-252/1267, 7-9=-252/1267
WEBS 3-14=-120/119, 11-14=-380/1228, 3-11=-226/212, 6-10=-468/294, 6-9=0/205, 5-10=-339/911, 4-10=-590/288

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 406 lb uplift at joint 2 and 390 lb uplift at joint 7.

LOAD CASE(S) Standard

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR	Dwg.#030805265
L106588	T02G	SPECIAL	1	1	Job Reference (optional)	

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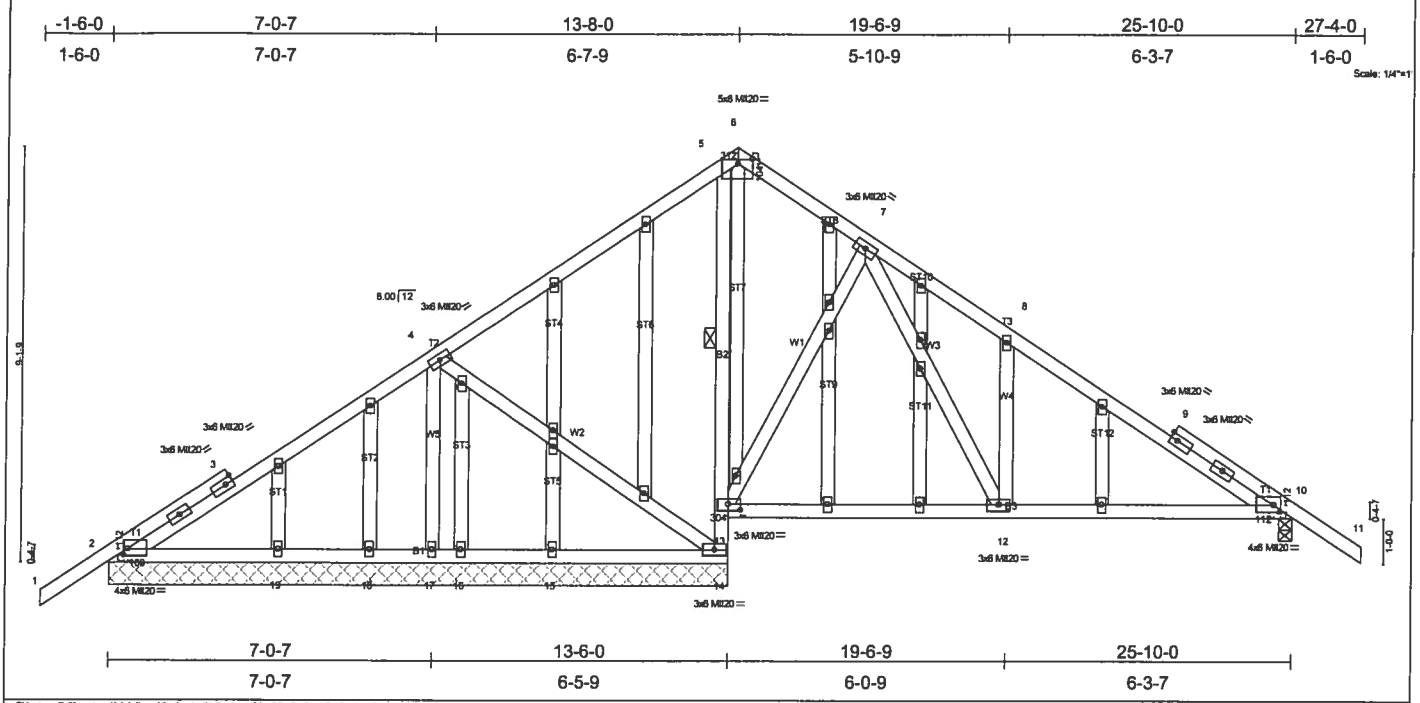

Plate Offsets (X,Y): [2:0-1-0,0-1-12], [6:0-3-12,0-1-4], [10:0-1-12,0-1-12], [13:0-3-4,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.68	Vert(LL)	-0.08	10-12	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.65	Vert(TL)	-0.11	10-12	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.53	Horz(TL)	0.03	10	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 213 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D *Except* B2 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 3-11-2 oc bracing. Except:
1 Row at midpt 5-13

REACTIONS (lb/size) 2=355/13-6-0, 10=914/0-3-8, 17=949/13-6-0, 14=1529/13-6-0, 15=154/13-6-0, 16=-44/13-6-0, 18=-54/13-6-0, 19=255/13-6-0
Max Horz 2=307(load case 4)
Max Uplift2=-142(load case 6), 10=-471(load case 6), 17=-573(load case 5), 14=-413(load case 5), 15=-43(load case 6), 16=-44(load case 1), 18=-54(load case 1), 19=-71(load case 5)
Max Grav 2=364(load case 7), 10=941(load case 8), 17=967(load case 7), 14=1529(load case 1), 15=154(load case 1), 16=63(load case 6), 18=59(load case 5), 19=255(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-11/91, 2-3=-267/272, 3-4=-290/539, 4-5=-109/453, 5-6=-15/173, 6-7=-44/381, 7-8=-814/550, 8-9=-671/335, 9-10=-833/356, 10-11=-12/94
BOT CHORD 2-19=-263/232, 18-19=-263/232, 17-18=-263/232, 16-17=-263/232, 15-16=-263/232, 14-15=-263/232, 13-14=-1537/442, 5-13=-862/285, 12-13=0/114, 10-12=-132/558
WEBS 4-17=-862/536, 4-14=-22/69, 7-13=-673/338, 8-12=-640/430, 7-12=-447/965

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MII20 unless otherwise indicated.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 142 lb uplift at joint 2, 471 lb uplift at joint 10, 573 lb uplift at joint 17, 413 lb uplift at joint 14, 43 lb uplift at joint 15, 44 lb uplift at joint 16, 54 lb uplift at joint 18 and 71 lb uplift at joint 19.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-6=-114(F=-60), 6-11=-114(F=-60), 2-14=-30, 10-13=-30

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR
L106588	T03	SPECIAL	1	3	Job Reference (optional)

Dwg.#030805266

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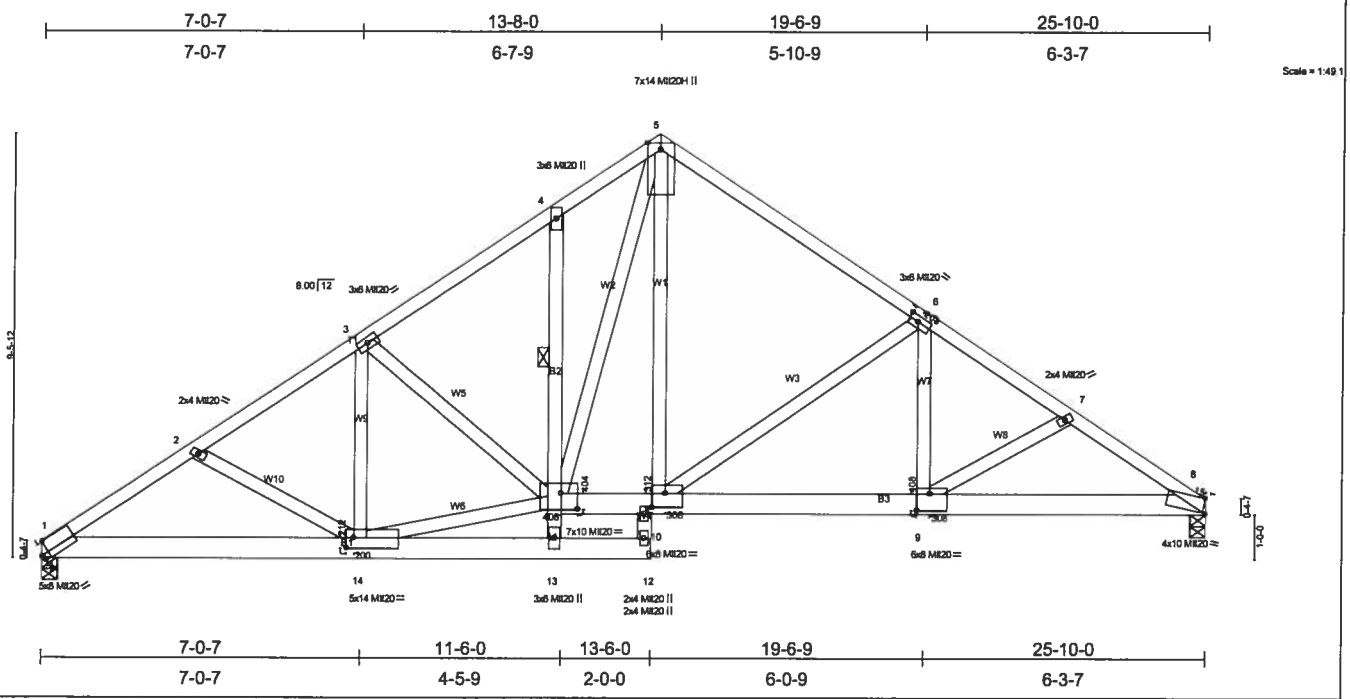

Plate Offsets (X,Y): [1:Edge,0-3-4], [6:0-2-8,0-1-8], [8:0-0-15,Edge], [9:0-3-8,0-4-8], [10:0-3-8,0-3-12], [11:0-4-8,0-4-4], [14:0-2-0,0-2-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.18	9-10	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.82	Vert(TL)	-0.26	9-10	>999	180	MII20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.73	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 581 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 6 SYP No.1D *Except*
B2 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. Except:
1 Row at midpt 4-11
JOINTS 1 Brace at Jt(s): 11

REACTIONS (lb/size) 1=6390/0-4-0, 8=9522/0-4-0
Max Horz 1=1499(load case 3)
Max Uplift1=-2203(load case 4), 8=-3768(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-9538/3240, 2-3=-9392/3197, 3-4=-9016/3269, 4-5=-8882/3974, 5-6=-8092/3441, 6-7=-12997/5199, 7-8=-13134/5231
BOT CHORD 1-14=-4022/7869, 13-14=-464/1134, 12-13=0/0, 11-13=-487/1393, 4-11=-65/527, 10-11=-2695/6693, 9-10=-4231/10767, 8-9=-4288/10873
WEBS 3-14=0/304, 11-14=-3529/6811, 3-11=-462/135, 6-10=-5001/2052, 6-9=-2018/5411, 5-10=-2726/6438, 5-11=-1445/2715, 7-9=-128/116, 2-14=-122/139

NOTES
1) 3-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-7-0 oc, 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) All plates are MT20 plates unless otherwise indicated.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2203 lb uplift at joint 1 and 3768 lb uplift at joint 8.
7) Girder carries tie-in span(s): 25-4-0 from 0-0-0 to 13-6-0; 39-4-0 from 13-6-0 to 25-10-0; 15-8-0 from 0-0-0 to 13-6-0

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-54, 5-8=-54, 1-13=-310(F=-280), 12-13=-310(F=-280), 10-11=-310(F=-280), 8-10=-807(F=-777)

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR
L106588	T04	COMMON	2	1	Job Reference (optional)

Dwg.#030805267

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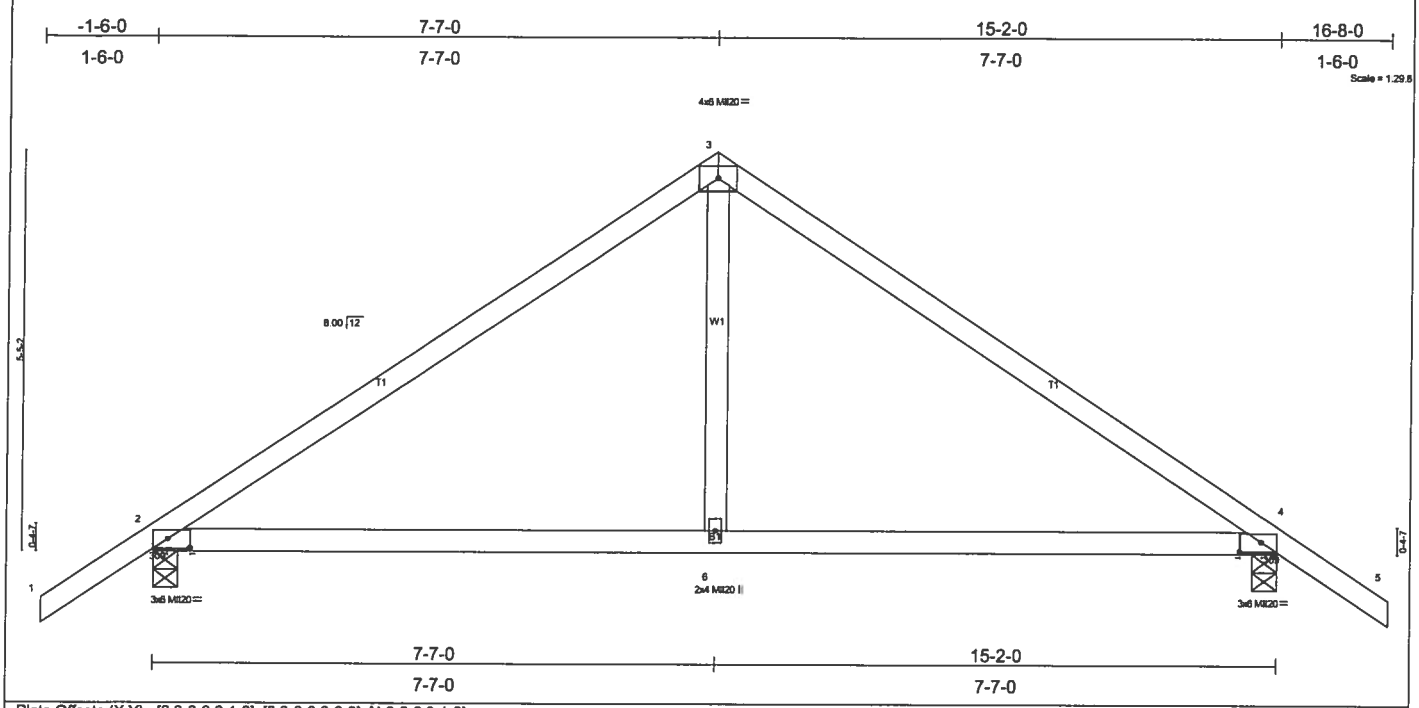

Plate Offsets (X,Y): [2:0-3-9,0-1-8], [3:0-0-0,0-0-0], [4:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.11	4-6	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.14	4-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.09	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 63 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=713/0-4-0, 4=713/0-4-0
Max Horz 2=182(load case 4)
Max Uplift2=-280(load case 5), 4=-280(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-755/250, 3-4=-755/250, 4-5=0/45
BOT CHORD 2-6=-90/542, 4-6=-90/542
WEBS 3-6=0/289

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 280 lb uplift at joint 2 and 280 lb uplift at joint 4.

LOAD CASE(S) Standard

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR	Dwg.#030805268
L106588	T04G	COMMON	1	1	Job Reference (optional)	

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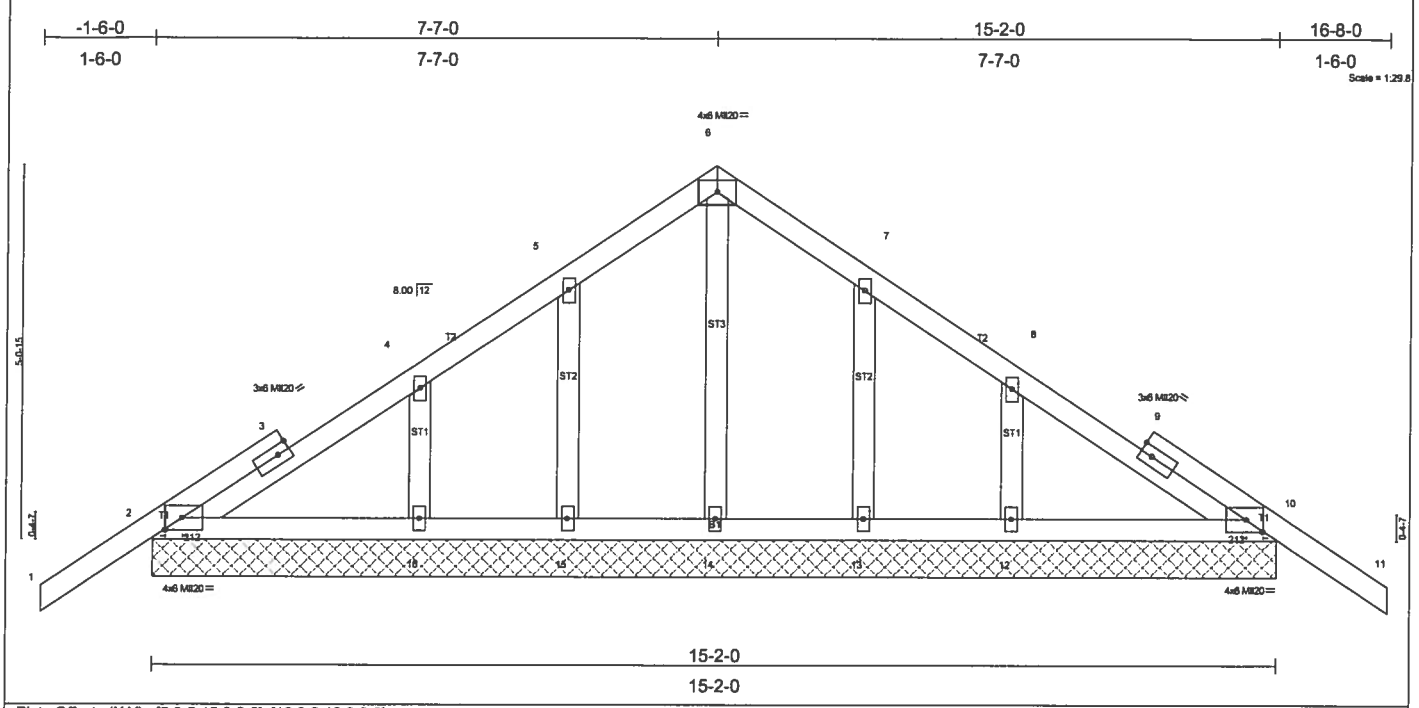

Plate Offsets (X,Y): [2:0-2-12,0-2-0], [10:0-2-12,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.25	Vert(LL)	-0.01	11	n/r	90	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	11	n/r	80		
BCLL	10.0	Rep Stress Incr	NO	WB	0.11	Horz(TL)	0.00	10	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 82 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=423/15-2-0, 10=423/15-2-0, 14=331/15-2-0, 15=236/15-2-0, 16=438/15-2-0, 13=236/15-2-0, 12=438/15-2-0
Max Horz 2=170(load case 4)
Max Uplift2=-187(load case 5), 10=-211(load case 6), 14=-18(load case 5), 15=-138(load case 5), 16=-184(load case 5), 13=-134(load case 6), 12=-191(load case 6)
Max Grav 2=424(load case 7), 10=424(load case 8), 14=331(load case 1), 15=241(load case 7), 16=438(load case 1), 13=241(load case 8), 12=438(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-11/91, 2-3=-111/104, 3-4=-116/126, 4-5=-60/106, 5-6=-45/134, 6-7=-45/134, 7-8=-60/74, 8-9=-58/126, 9-10=-81/44, 10-11=-11/91
BOT CHORD 2-16=-19/153, 15-16=-19/153, 14-15=-19/153, 13-14=-19/153, 12-13=-19/153, 10-12=-19/153
WEBS 6-14=-261/31, 5-15=-200/148, 4-16=-333/202, 7-13=-200/144, 8-12=-333/207

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MII20 unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 187 lb uplift at joint 2, 211 lb uplift at joint 10, 18 lb uplift at joint 14, 138 lb uplift at joint 15, 184 lb uplift at joint 16, 134 lb uplift at joint 13 and 191 lb uplift at joint 12.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-6=-114(F=-60), 6-11=-114(F=-60), 2-10=-30

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR
L106588	T05	SPECIAL	3	1	Job Reference (optional)

Dwg.#030805269

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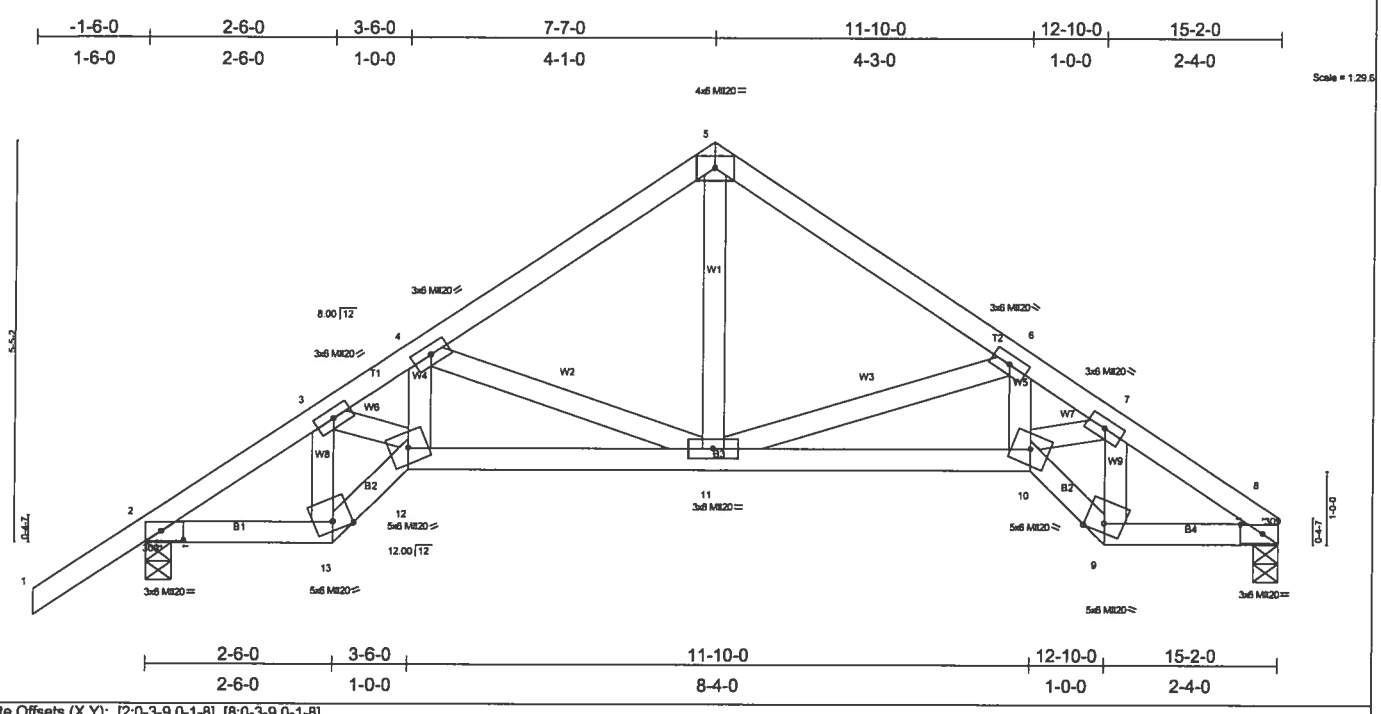

Plate Offsets (X,Y): [2:0-3-9,0-1-8], [8:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.14	Vert(LL)	-0.05	10-11	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.32	Vert(TL)	-0.07	10-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.21	Horz(TL)	0.05	8	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 83 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 5-4-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 8=618/0-4-0, 2=718/0-4-0
Max Horz 2=199(load case 4)
Max Uplift8=-179(load case 6), 2=-282(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-895/260, 3-4=-1373/424, 4-5=-763/286, 5-6=-767/284, 6-7=-1481/501, 7-8=-940/326
BOT CHORD 2-13=-197/677, 12-13=-250/881, 11-12=-346/1102, 10-11=-335/1182, 9-10=-270/929, 8-9=-220/721
WEBS 3-13=-561/199, 3-12=-195/582, 4-12=-66/405, 4-11=-541/266, 6-11=-619/282, 6-10=-98/466, 7-10=-154/616, 7-9=-569/172, 5-11=-145/538

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 179 lb uplift at joint 8 and 282 lb uplift at joint 2.

LOAD CASE(S) Standard

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR	Dwg.#030805270
L106588	T07	MONO HIP	1	1	Job Reference (optional)	

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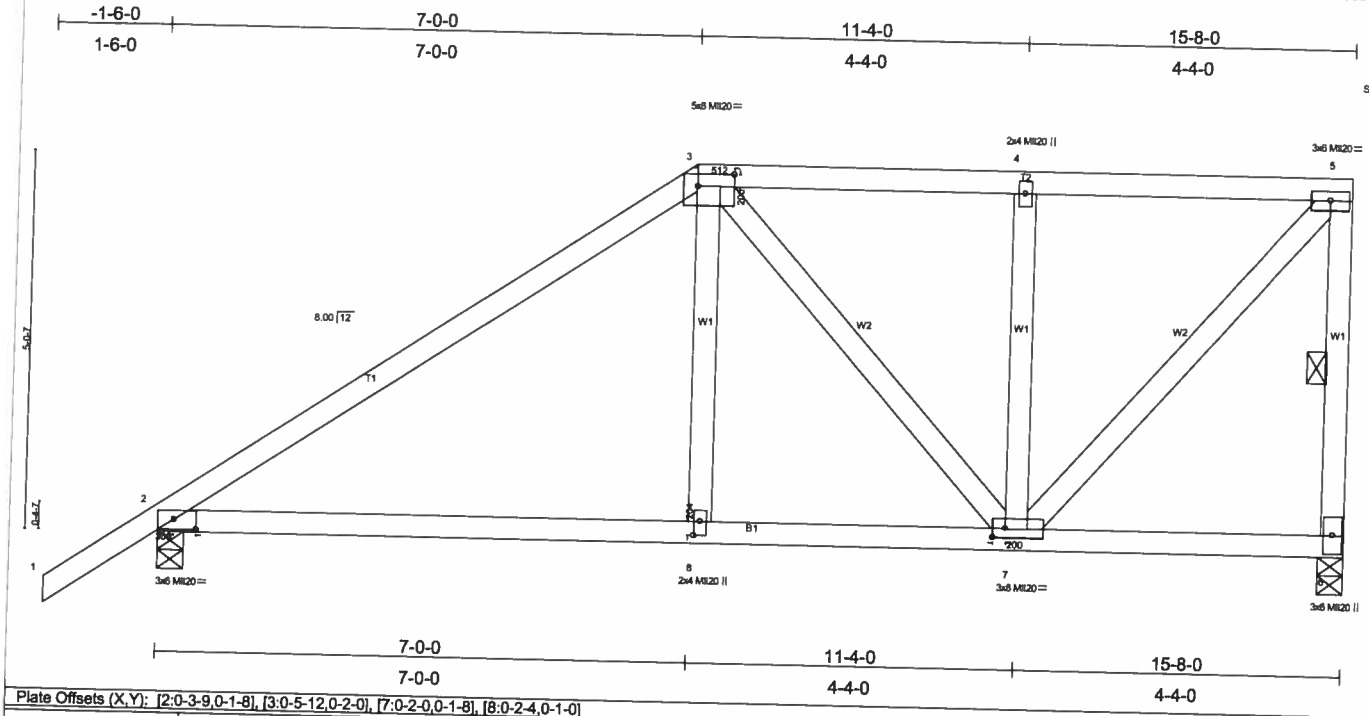

Plate Offsets (X,Y): [2:0-3-9,0-1-8], [3:0-5-12,0-2-0], [7:0-2-0,0-1-8], [8:0-2-4,0-1-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.09	2-8	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.49	Vert(TL)	-0.13	2-8	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.61	Horz(TL)	0.02	6	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 89 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 4-7-11 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-1-12 oc bracing.
WEBS 1 Row at midpt 5-6

REACTIONS (lb/size) 6=1486/0-4-0, 2=1263/0-4-0
Max Horz 2=281(load case 4)
Max Uplift6=-878(load case 3), 2=-664(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-1721/881, 3-4=-1026/612, 4-5=-1025/613, 5-6=-1347/889
BOT CHORD 2-8=-799/1338, 7-8=-811/1363, 6-7=-23/41
WEBS 3-8=-381/818, 3-7=-501/366, 4-7=-493/640, 5-7=-892/1488

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 878 lb uplift at joint 6 and 664 lb uplift at joint 2.
4) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 410 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-117(F=-63), 2-8=-30, 6-8=-65(F=-35)
Concentrated Loads (lb)
Vert: 8=-539(F)

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR	Dwg.#030805271
L106588	T08	MONO HIP	1	1	Job Reference (optional)	

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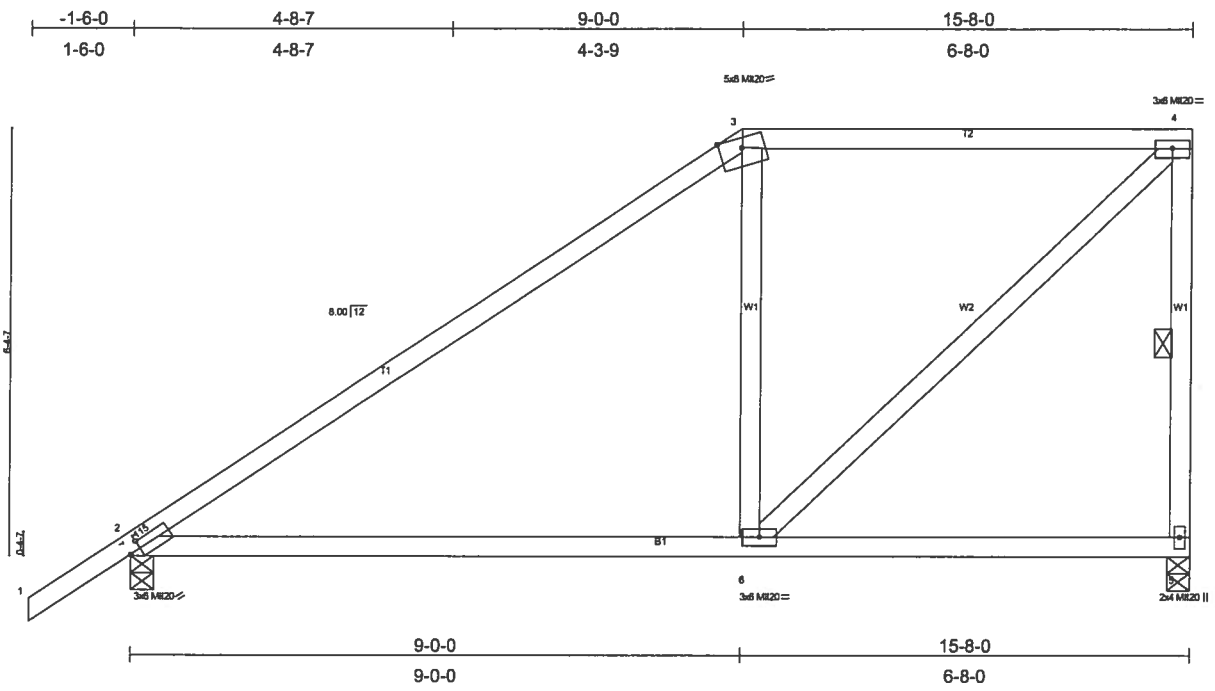


Plate Offsets (X,Y): [2:0-1-15,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.22	2-6	>824	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.32	2-6	>573	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.46	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 82 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 4-5

REACTIONS (lb/size) 5=640/0-4-0, 2=740/0-4-0
Max Horz 2=341(load case 5)
Max Uplift5=-223(load case 4), 2=-261(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-717/166, 3-4=-489/250, 4-5=-578/322
BOT CHORD 2-6=-256/492, 5-6=-27/15
WEBS 3-6=-154/260, 4-6=-341/655

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 223 lb uplift at joint 5 and 261 lb uplift at joint 2.

LOAD CASE(S) Standard

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR
L1065E...	T09	MONO HIP	1	1	Job Reference (optional)

Dwg.#030805272

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Dec 15 2004 MiTek Industries, Inc. Mon Mar 07 16:56:12 2005 Page 1

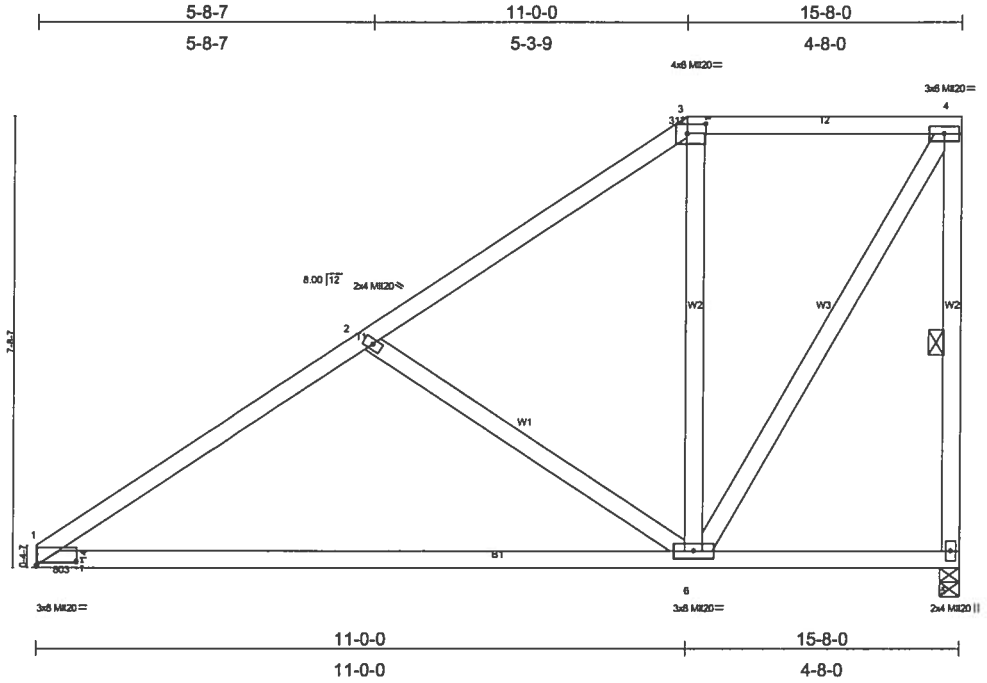


Plate Offsets (X,Y): [1:0-8-3,0-0-14], [3:0-3-12,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	-0.33	1-6	>570	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	-0.48	1-6	>386	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.38	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 93 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 4-5

REACTIONS (lb/size) 1=649/Mechanical, 5=649/0-4-0
Max Horz 1=342(load case 5)
Max Uplift1=-150(load case 5), 5=-250(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-761/194, 2-3=-485/112, 3-4=-326/157, 4-5=-651/330
BOT CHORD 1-6=-398/604, 5-6=-8/3
WEBS 2-6=-335/309, 3-6=-48/150, 4-6=-300/635

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 150 lb uplift at joint 1 and 250 lb uplift at joint 5.

LOAD CASE(S) Standard

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR	Dwg.#030805273
L1065[illegible]	T10	MONO HIP	1	1	Job Reference (optional)	

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Dec 15 2004 MiTek Industries, Inc. Mon Mar 07 16:56:12 2005 Page 1

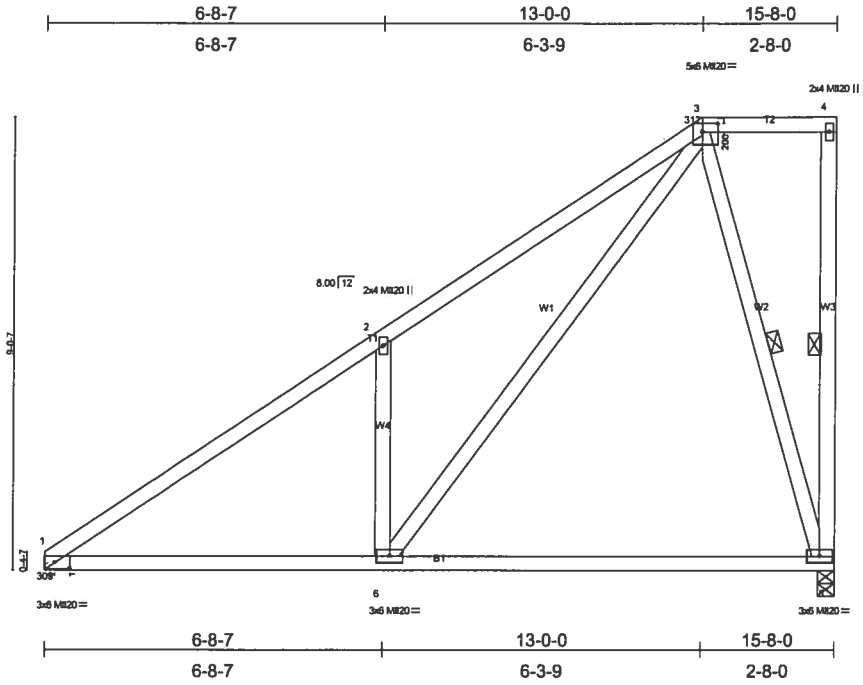


Plate Offsets (X,Y): [1:0-3-9,0-1-8], [3:0-3-12,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	-0.12	5-6	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	-0.18	5-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.96	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 98 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 4-5, 3-5

REACTIONS (lb/size) 1=649/Mechanical, 5=649/0-4-0
Max Horz 1=403(load case 5)
Max Uplift1=-127(load case 5), 5=-313(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-863/103, 2-3=-874/363, 3-4=-16/0, 4-5=-37/67
BOT CHORD 1-6=-379/641, 5-6=-107/159
WEBS 2-6=-379/411, 3-6=-470/824, 3-5=-517/400

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 127 lb uplift at joint 1 and 313 lb uplift at joint 5.

LOAD CASE(S) Standard

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR	Dwg.#030805274
L106588	T11	COMMON	4	1	Job Reference (optional)	

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Dec 15 2004 MiTek Industries, Inc. Mon Mar 07 16:56:13 2005 Page 1

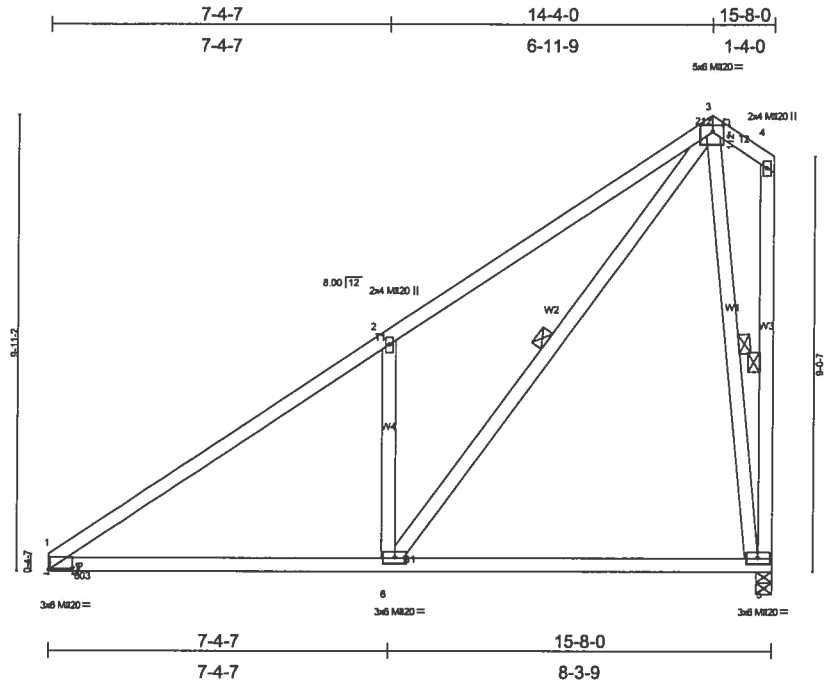


Plate Offsets (X,Y): [1:0-6-3,0-0-6], [3:0-2-12,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	-0.11	1-6	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.15	1-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.51	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 102 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3 *Except*
W3 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 3-6, 4-5, 3-5

REACTIONS (lb/size) 1=649/Mechanical, 5=649/0-4-0
Max Horz 1=422(load case 5)
Max Uplift1=-119(load case 5), 5=-348(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-823/90, 2-3=-838/364, 3-4=-76/53, 4-5=-133/76
BOT CHORD 1-6=-368/602, 5-6=-60/88
WEBS 2-6=-423/451, 3-6=-525/877, 3-5=-599/511

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 119 lb uplift at joint 1 and 348 lb uplift at joint 5.

LOAD CASE(S) Standard

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR
L1065/38	T12	SPECIAL	1	1	Job Reference (optional)

Dwg.#030805275

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Dec 15 2004 MiTek Industries, Inc. Mon Mar 07 16:56:13 2005 Page 1

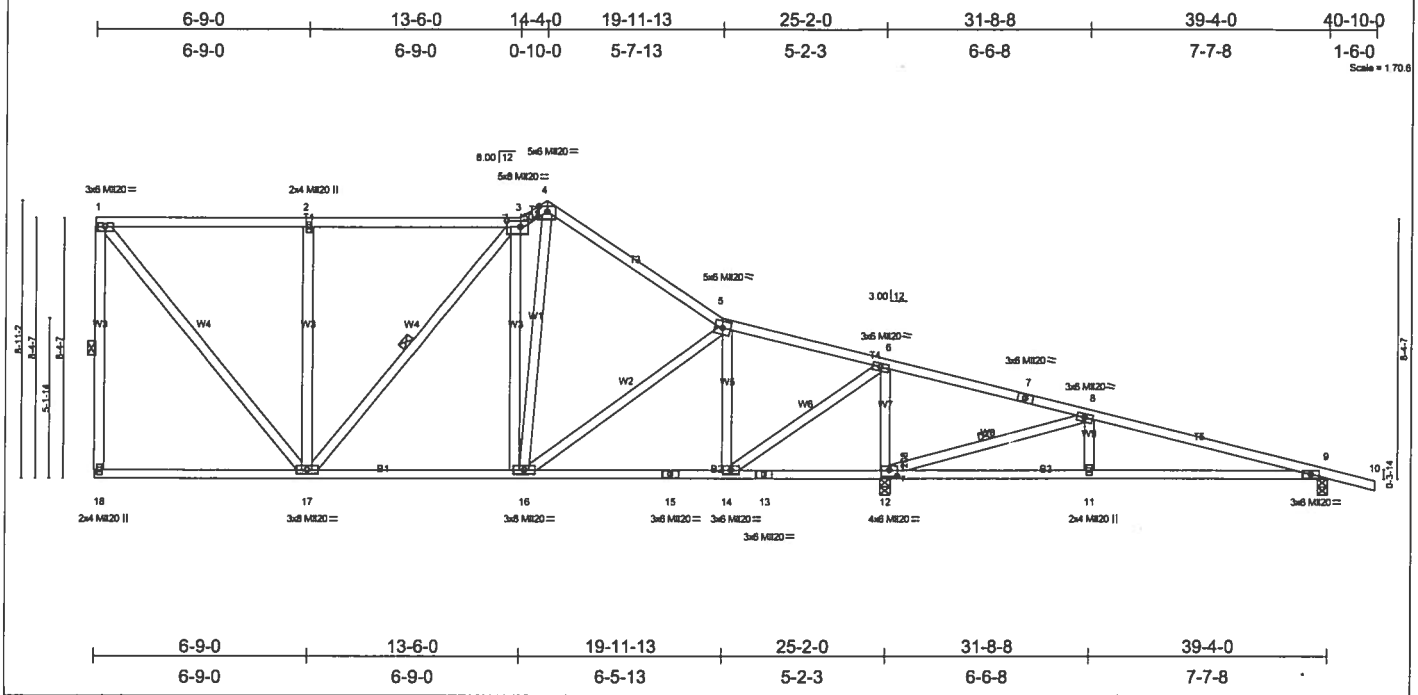


Plate Offsets (X,Y): [3:0-5-4,0-2-8], [12:0-3-0,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	0.20	9-11	>828	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.33	Vert(TL)	0.17	9-11	>973	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.71	Horz(TL)	0.01	12	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 244 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 1-18, 3-17, 8-12

REACTIONS (lb/size) 18=926/Mechanical, 12=1980/0-4-0, 9=461/0-4-0
Max Horz 18=-406(load case 6)
Max Uplift18=-408(load case 3), 12=-849(load case 4), 9=-473(load case 4)
Max Grav 18=926(load case 1), 12=1980(load case 1), 9=481(load case 8)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-18=-833/419, 1-2=-575/247, 2-3=-575/246, 3-4=-720/296, 4-5=-776/241, 5-6=-443/29, 6-7=-595/865, 7-8=-602/788, 8-9=-535/600, 9-10=0/18
BOT CHORD 17-18=0/443, 16-17=-0/632, 15-16=0/415, 14-15=0/415, 13-14=-802/770, 12-13=-802/770, 11-12=-493/474, 9-11=-493/474
WEBS 1-17=-374/874, 2-17=-401/341, 3-17=-109/94, 3-16=-562/304, 4-16=-214/651, 5-16=-133/208, 5-14=-687/403, 6-14=-595/1466, 6-12=-1457/769, 8-12=-1250/1317, 8-11=-283/243

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 408 lb uplift at joint 18, 849 lb uplift at joint 12 and 473 lb uplift at joint 9.

LOAD CASE(S) Standard

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

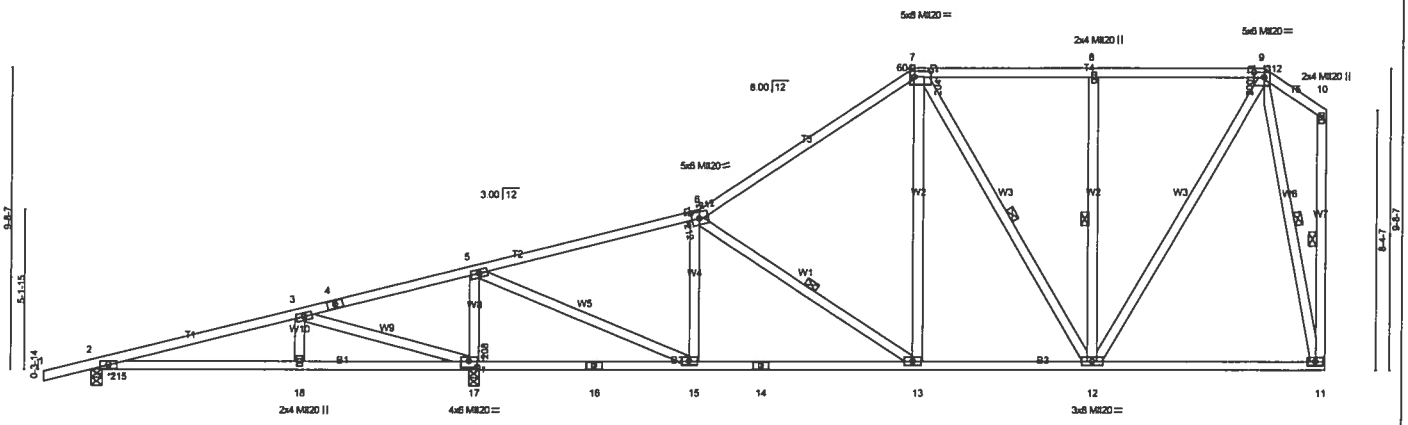
Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR
L106588	T13	SPECIAL	2	1	Job Reference (optional)

Dwg.#030805276

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Dec 15 2004 MiTek Industries, Inc. Mon Mar 07 16:56:14 2005 Page 1

1-6-0	6-7-8	12-2-0	19-4-3	26-2-0	31-10-12	37-4-0	39-4-0
1-6-0	6-7-8	5-6-8	7-2-3	6-9-13	5-8-12	5-5-4	2-0-0

Scale = 1:70.5



6-7-8	12-2-0	19-4-3	26-2-0	31-10-12	37-4-0	39-4-0
6-7-8	5-6-8	7-2-3	6-9-13	5-8-12	5-5-4	2-0-0

Plate Offsets (X,Y): [2:0-0-1,0-0-0], [3:0-0-0,0-0-0], [4:0-0-0,0-0-0], [5:0-0-0,0-0-0], [6:0-2-12,0-2-12], [7:0-6-4,0-2-4], [8:0-0-0,0-0-0], [9:0-3-12,0-2-0], [10:0-0-0,0-0-0], [17:0-3-0,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	0.12	2-18	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.31	Vert(TL)	-0.13	13-15	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.84	Horz(TL)	0.02	11	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 252 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3 *Except*
W7 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 6-13, 7-12, 8-12, 10-11, 9-11

REACTIONS (lb/size) 2=413/0-4-0, 17=1903/0-4-0, 11=1052/Mechanical
Max Horz 2=423(load case 5)
Max Uplift2=-396(load case 3), 17=-901(load case 3), 11=-346(load case 4)
Max Grav 2=413(load case 7), 17=1903(load case 1), 11=1052(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/18, 2-3=-364/319, 3-4=-722/619, 4-5=-711/684, 5-6=-1022/162, 6-7=-976/311, 7-8=-605/283, 8-9=-605/283, 9-10=-35/25, 10-11=-56/31
BOT CHORD 2-18=-616/313, 17-18=-616/313, 16-17=-627/405, 15-16=-627/405, 14-15=-358/961, 13-14=-358/961, 12-13=-285/730, 11-12=-81/195
WEBS 3-18=-243/193, 3-17=-981/1068, 5-17=-1426/857, 5-15=-821/1718, 6-15=-470/387, 6-13=-289/120, 7-13=-36/347, 7-12=-244/156, 8-12=-325/306, 9-12=-315/822, 9-11=-927/430

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are 3x6 MII20 unless otherwise indicated.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 396 lb uplift at joint 2, 901 lb uplift at joint 17 and 346 lb uplift at joint 11.

LOAD CASE(S) Standard

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR
L106588	T13A	SPECIAL	1	1	Job Reference (optional)

Dwg.#030805277

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Dec 15 2004 MiTek Industries, Inc. Mon Mar 07 16:56:15 2005 Page 1

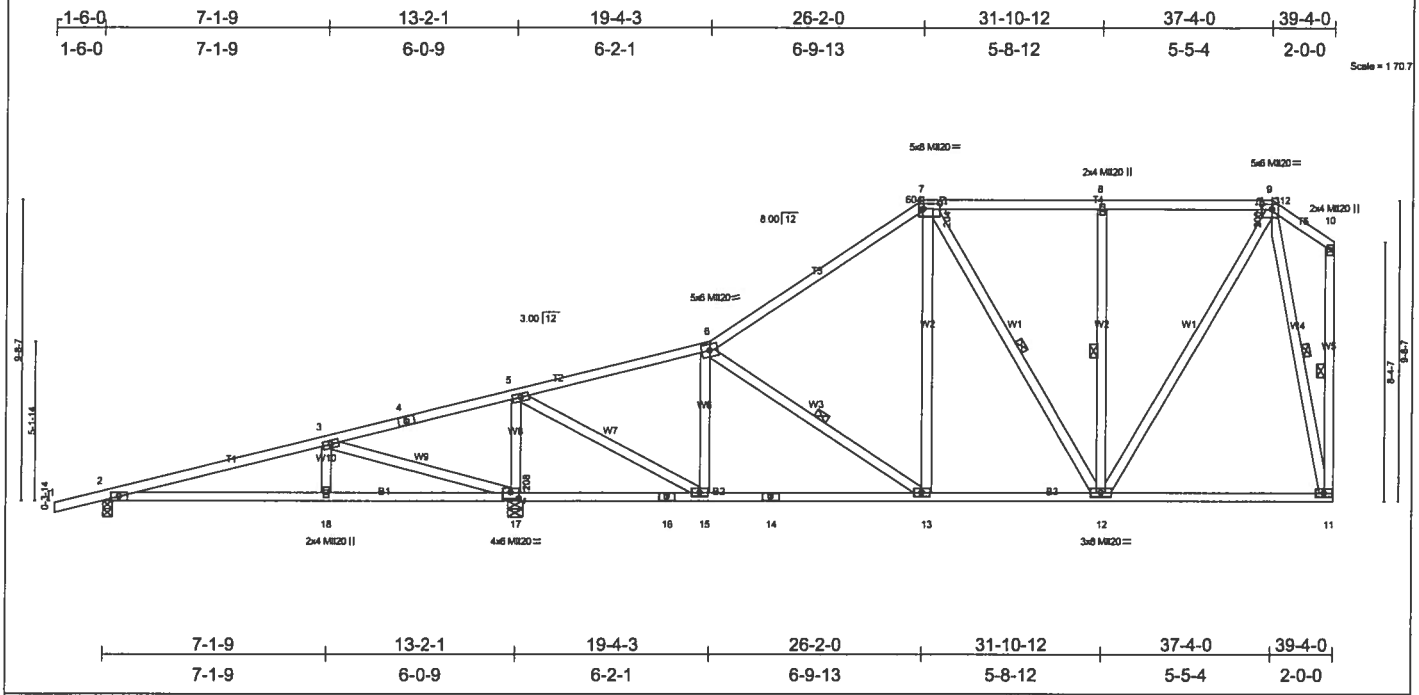


Plate Offsets (X,Y): [7:0-6-4,0-2-4], [9:0-3-12,0-2-0], [17:0-3-0,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	0.16	2-18	>987	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.30	Vert(TL)	0.13	2-18	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.79	Horz(TL)	-0.01	11	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 252 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3 *Except*
W5 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 6-13, 7-12, 8-12, 10-11, 9-11

REACTIONS (lb/size) 2=441/0-4-0, 17=1935/0-5-11, 11=992/Mechanical
Max Horz 2=423(load case 5)
Max Uplift2=-420(load case 3), 17=-930(load case 3), 11=-324(load case 4)
Max Grav 2=441(load case 7), 17=1935(load case 1), 11=992(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/18, 2-3=-429/373, 3-4=-784/696, 4-5=-773/768, 5-6=-750/14, 6-7=-878/252, 7-8=-558/256, 8-9=-558/256, 9-10=-34/24, 10-11=-57/30
BOT CHORD 2-18=-672/374, 17-18=-672/374, 16-17=-708/479, 15-16=-708/479, 14-15=-214/707, 13-14=-214/707, 12-13=-242/649, 11-12=-77/183
WEBS 3-18=-263/220, 3-17=-1128/1201, 5-17=-1437/854, 5-15=-773/1596, 6-15=-570/433, 6-13=-145/33, 7-13=0/230, 7-12=-178/129, 8-12=-325/302, 9-12=-289/752, 9-11=-868/409

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are 3x6 MII20 unless otherwise indicated.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 420 lb uplift at joint 2, 930 lb uplift at joint 17 and 324 lb uplift at joint 11.

LOAD CASE(S) Standard

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR
L106588	T13B	SPECIAL	1	1	Job Reference (optional)

Dwg.#030805278

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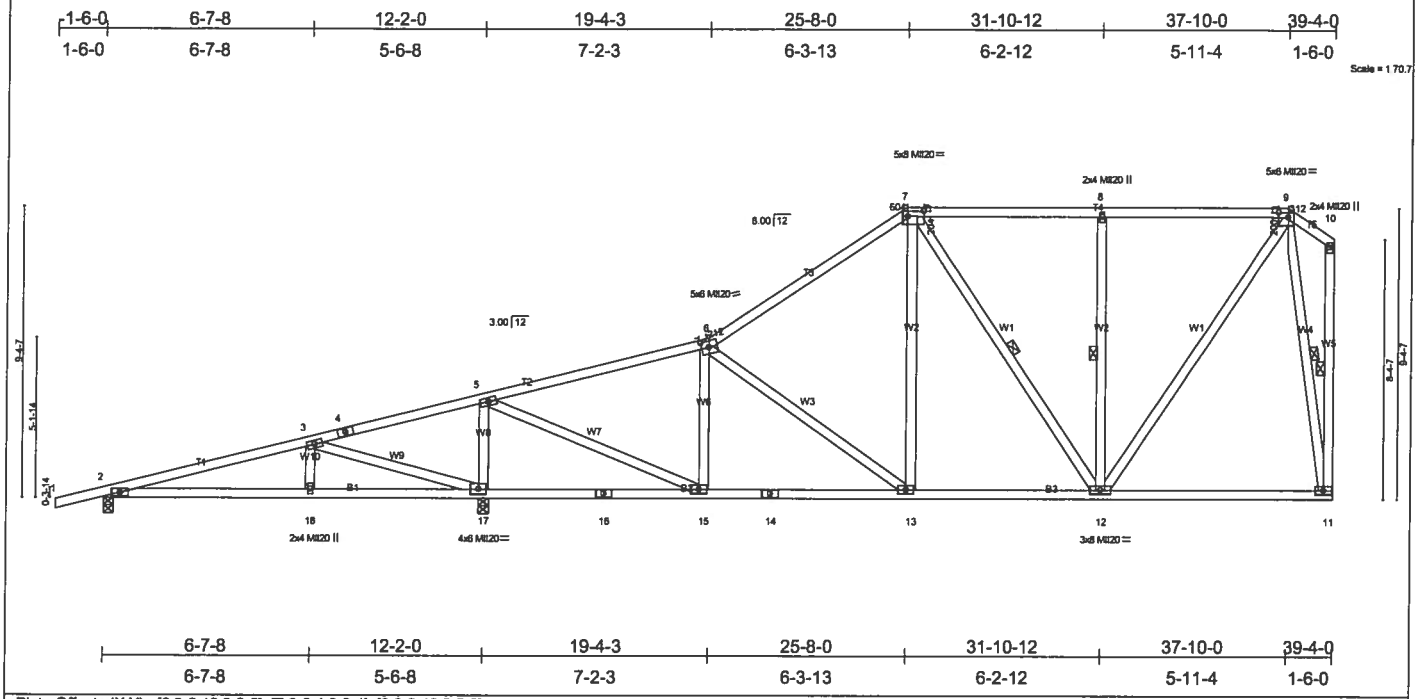


Plate Offsets (X,Y): [6:0-2-12,0-2-8], [7:0-6-4,0-2-4], [9:0-3-12,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	0.12	2-18	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.29	Vert(TL)	-0.11	13-15	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.83	Horz(TL)	0.02	11	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 249 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3 *Except*
W5 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 7-12, 8-12, 10-11, 9-11

REACTIONS (lb/size) 2=413/0-4-0, 17=1902/0-4-0, 11=1052/Mechanical
Max Horz 2=417(load case 5)
Max Uplift2=-393(load case 3), 17=-916(load case 3), 11=-376(load case 4)
Max Grav 2=414(load case 7), 17=1902(load case 1), 11=1052(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/18, 2-3=-364/307, 3-4=-721/615, 4-5=-710/680, 5-6=-1017/160, 6-7=-995/314, 7-8=-632/282, 8-9=-632/282, 9-10=-44/24, 10-11=-77/20
BOT CHORD 2-18=-616/313, 17-18=-616/313, 16-17=-623/404, 15-16=-623/404, 14-15=-356/956, 13-14=-356/956, 12-13=-296/752, 11-12=-69/156
WEBS 3-18=-244/192, 3-17=-976/1066, 5-17=-1424/859, 5-15=-817/1706, 6-15=-476/384, 6-13=-262/108, 7-13=-33/338, 7-12=-214/139, 8-12=-356/326, 9-12=-341/870, 9-11=-951/477

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are 3x6 MII20 unless otherwise indicated.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 393 lb uplift at joint 2, 916 lb uplift at joint 17 and 376 lb uplift at joint 11.

LOAD CASE(S) Standard

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR
L106588	T14	SPECIAL	1	1	Job Reference (optional)

Dwg.#030805279

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Dec 15 2004 MiTek Industries, Inc. Mon Mar 07 16:56:16 2005 Page 1

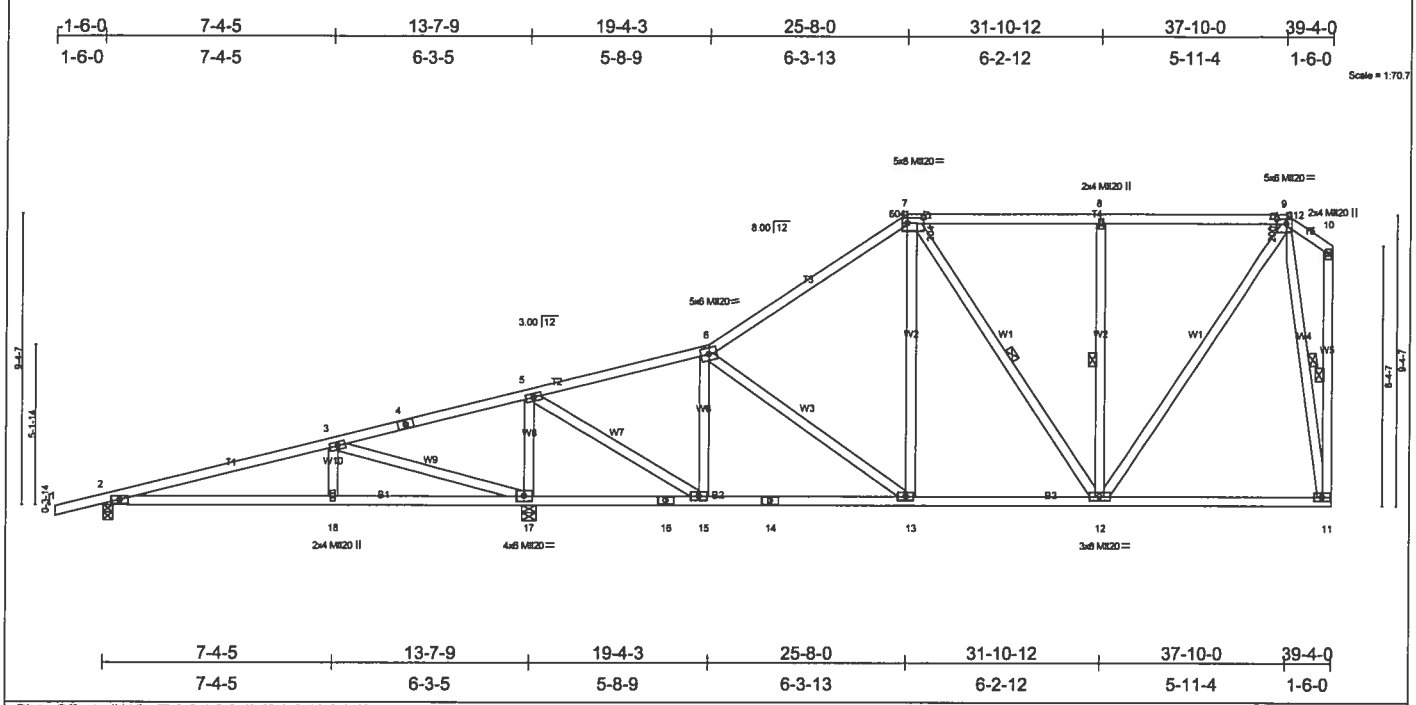


Plate Offsets (X,Y): [7:0-6-4,0-2-4], [9:0-3-12,0-2-0]

LOADING (psf)		SPACING 2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	0.18	2-18	>903	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.30	Vert(TL)	-0.15	2-18	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.89	Horz(TL)	-0.01	11	n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 249 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3 *Except*
W5 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 7-12, 8-12, 10-11, 9-11

REACTIONS (lb/size) 2=455/0-4-0, 17=1947/0-5-11, 11=965/Mechanical
Max Horz 2=417(load case 5)
Max Uplift2=-428(load case 3), 17=-960(load case 3), 11=-343(load case 4)
Max Grav 2=456(load case 7), 17=1947(load case 1), 11=965(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/18, 2-3=-462/385, 3-4=-807/718, 4-5=-800/792, 5-6=-621/0, 6-7=-840/221, 7-8=-562/241, 8-9=-562/241, 9-10=-45/26, 10-11=-79/24
BOT CHORD 2-18=-697/405, 17-18=-697/405, 16-17=-732/507, 15-16=-732/507, 14-15=-155/585, 13-14=-155/585, 12-13=-236/625, 11-12=-64/144
WEBS 3-18=-272/231, 3-17=-1185/1256, 5-17=-1437/855, 5-15=-743/1524, 6-15=-621/449, 6-13=-189/89, 7-13=0/160, 7-12=-156/98, 8-12=-356/321, 9-12=-302/765, 9-11=-867/446

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are 3x6 MII20 unless otherwise indicated.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 428 lb uplift at joint 2, 960 lb uplift at joint 17 and 343 lb uplift at joint 11.

LOAD CASE(S) Standard

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR	Dwg.#030805280
L106588	T16	SPECIAL	1	1	Job Reference (optional)	

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Dec 15 2004 MiTek Industries, Inc. Mon Mar 07 16:56:17 2005 Page 1

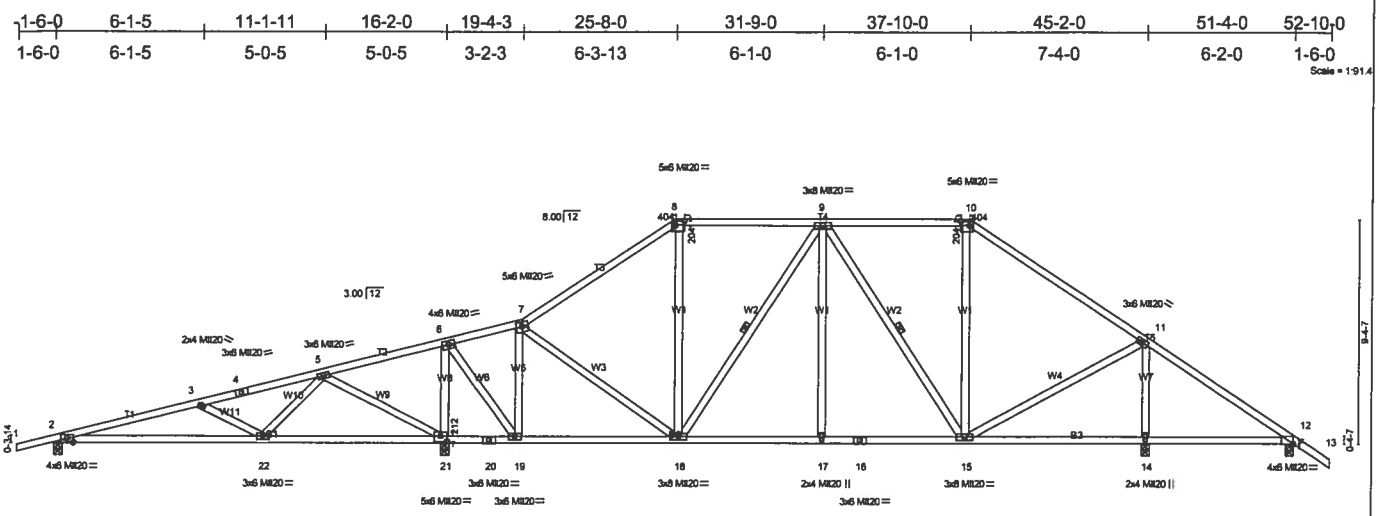


Plate Offsets (X,Y): [8:0-4-4,0-2-4], [10:0-4-4,0-2-4], [12:Edge,0-0-4], [21:0-3-0,0-2-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.23	2-22	>822	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.35	Vert(TL)	0.20	2-22	>965	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.55	Horz(TL)	0.02	14	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 302 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 9-18, 9-15

REACTIONS (lb/size) 2=529/0-4-0, 21=2237/0-4-0, 14=1430/0-4-0, 12=269/0-4-0
Max Horz 2=328(load case 4)
Max Uplift2=-517(load case 3), 21=-1081(load case 3), 14=-408(load case 6), 12=-267(load case 6)
Max Grav 2=531(load case 7), 21=2237(load case 1), 14=1430(load case 1), 12=285(load case 8)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/18, 2-3=-813/879, 3-4=-410/552, 4-5=-348/562, 5-6=-719/946, 6-7=-272/36, 7-8=-805/262, 8-9=-597/299, 9-10=-630/387, 10-11=-868/359, 11-12=-68/145, 12-13=0/45
BOT CHORD 2-22=-816/763, 21-22=-121/146, 20-21=-887/886, 19-20=-887/886, 18-19=0/517, 17-18=-264/766, 16-17=-264/766, 15-16=-264/766, 14-15=-74/128, 12-14=-74/128
WEBS 3-22=-459/452, 5-22=-701/653, 5-21=-922/961, 6-21=-1624/842, 6-19=-678/1488, 7-19=-1072/626, 7-18=-407/702, 8-18=0/162, 9-18=-343/318, 9-17=0/164, 9-15=-323/205, 10-15=-14/140, 11-15=-216/754, 11-14=-1197/473

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 517 lb uplift at joint 2, 1081 lb uplift at joint 21, 408 lb uplift at joint 14 and 267 lb uplift at joint 12.

LOAD CASE(S) Standard

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR	Dwg.#030805281
L106588	T17	SPECIAL	4	1	Job Reference (optional)	

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Dec 15 2004 MiTek Industries, Inc. Mon Mar 07 16:56:18 2005 Page 1

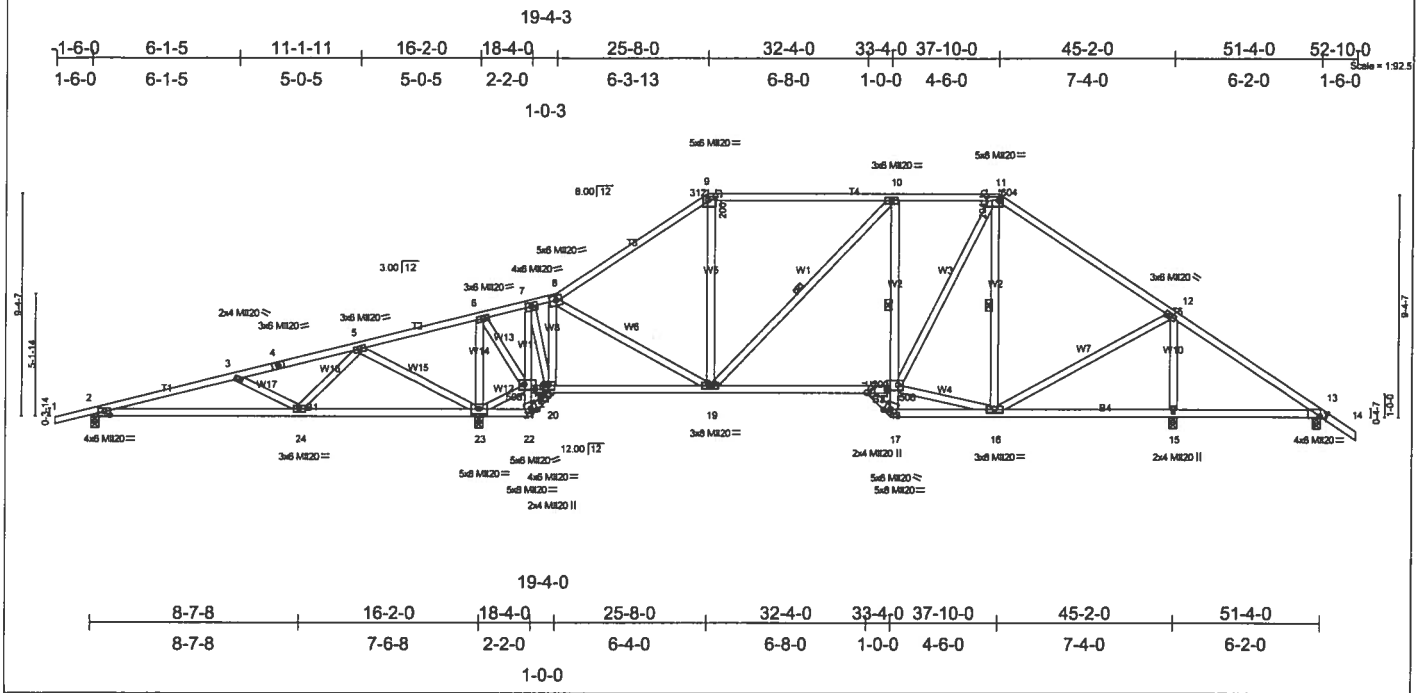


Plate Offsets (X,Y): [9:0-3-12,0-2-0], [11:0-6-4,0-2-4], [13:Edge,0-0-4], [18:0-5-8,0-2-8], [20:0-1-8,0-0-7], [21:0-5-8,0-2-8], [25:0-2-0,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	0.24	2-24	>811	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	0.20	2-24	>951	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.55	Horz(TL)	0.02	15	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 319 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 10-19, 10-17, 11-16

REACTIONS (lb/size) 2=519/0-4-0, 13=265/0-4-0, 23=2252/0-4-0, 15=1429/0-4-0
Max Horz 2=328(load case 4)
Max Uplift2=-534(load case 3), 13=-277(load case 6), 23=-1057(load case 3), 15=-402(load case 6)
Max Grav 2=523(load case 7), 13=279(load case 8), 23=2252(load case 1), 15=1429(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/18, 2-3=-780/945, 3-4=-374/616, 4-5=-337/626, 5-6=-646/982, 6-7=-498/429, 7-8=-310/87, 8-9=-888/270, 9-10=-674/308, 10-11=-842/426, 11-12=-857/375, 12-13=-60/149, 13-14=0/45
BOT CHORD 2-24=-879/731, 23-24=-171/96, 22-23=0/7, 20-21=-423/740, 19-20=-18/593, 18-19=-296/844, 16-17=-4/11, 15-16=-71/105, 13-15=-71/105
WEBS 3-24=-462/453, 5-24=-700/653, 5-23=-914/951, 6-23=-1114/375, 6-21=-240/903, 21-22=-27/65, 7-21=-1339/666, 8-20=-1117/677, 8-19=-481/787, 9-19=0/174, 10-19=-281/301, 17-18=0/38, 10-18=-212/260, 16-18=-146/619, 11-16=-316/183, 12-16=-198/743, 12-15=-1195/466, 21-23=-1051/918, 7-20=-730/1340, 11-18=-313/537

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 534 lb uplift at joint 2, 277 lb uplift at joint 13, 1057 lb uplift at joint 23 and 402 lb uplift at joint 15.

LOAD CASE(S) Standard

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

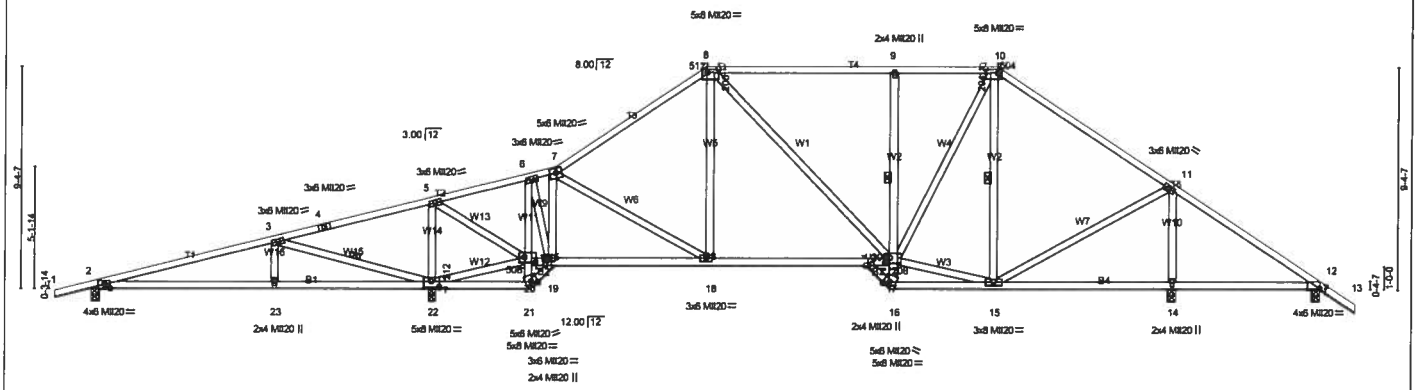
Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR	Dwg.#030805282
L106588	T18	SPECIAL	3	1	Job Reference (optional)	

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Dec 15 2004 MiTek Industries, Inc. Mon Mar 07 16:56:19 2005 Page 1

1-6-0 | 7-7-8 | 14-2-0 | 18-4-0 | 19-4-3 | 25-8-0 | 32-4-0 | 33-4-0 | 37-10-0 | 44-4-9 | 51-4-0 | 52-10-0

1-6-0 | 7-7-8 | 6-6-8 | 4-2-0 | 1-0-3 | 6-3-13 | 6-8-0 | 1-0-0 | 4-6-0 | 6-6-9 | 6-11-7 | 1-6-0

Scale = 1:92.5



7-7-8 | 14-2-0 | 18-4-0 | 19-4-0 | 25-8-0 | 32-4-0 | 33-4-0 | 37-10-0 | 45-2-0 | 51-4-0

7-7-8 | 6-6-8 | 4-2-0 | 1-0-0 | 6-4-0 | 6-8-0 | 1-0-0 | 4-6-0 | 7-4-0 | 6-2-0

Plate Offsets (X,Y): [8:0-5-12,0-2-0], [10:0-6-4,0-2-4], [12:Edge,0-0-4], [17:0-2-8,0-2-8], [19:0-2-0,0-0-15], [20:0-5-8,0-2-8], [22:0-4-0,0-2-12], [24:0-2-0,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	0.20	2-23	>834	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	0.17	2-23	>975	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.52	Horz(TL)	0.03	14	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 317 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 5-9-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 3-22, 9-16, 10-15

REACTIONS (lb/size) 2=411/0-4-0, 12=228/0-4-0, 22=2254/0-4-0, 14=1567/0-4-0
Max Horz 2=328(load case 4)
Max Uplift2=-476(load case 3), 12=-265(load case 6), 22=-1013(load case 3), 14=-450(load case 3)
Max Grav 2=416(load case 7), 12=251(load case 8), 22=2254(load case 1), 14=1567(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/18, 2-3=-281/564, 3-4=-640/992, 4-5=-632/1069, 5-6=-375/50, 6-7=-610/109, 7-8=-1148/422, 8-9=-957/495, 9-10=-961/499, 10-11=-927/417, 11-12=-62/213, 12-13=0/45
BOT CHORD 2-23=-496/228, 22-23=-496/228, 21-22=-2/8, 19-20=-46/360, 18-19=-150/632, 17-18=-312/891, 15-16=-5/19, 14-15=-124/137, 12-14=-124/137
WEBS 3-23=-283/243, 3-22=-1260/1319, 5-22=-1462/709, 5-20=-556/1569, 20-21=0/38, 6-20=-1066/520, 6-19=-490/990, 7-19=-834/500, 7-18=-280/299, 8-18=0/193, 8-17=-240/176, 16-17=0/40, 9-17=-354/333, 10-17=-365/653, 10-15=-378/212, 11-15=-252/869, 11-14=-1332/548, 20-22=-1032/787, 15-17=-169/666

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 476 lb uplift at joint 2, 265 lb uplift at joint 12, 1013 lb uplift at joint 22 and 450 lb uplift at joint 14.

LOAD CASE(S) Standard

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR
L106588	T19	CAL. PORCH TRUSS	1	1	Job Reference (optional)

Dwg.#030805283

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Dec 15 2004 MiTek Industries, Inc. Wed Mar 09 11:40:53 2005 Page 1

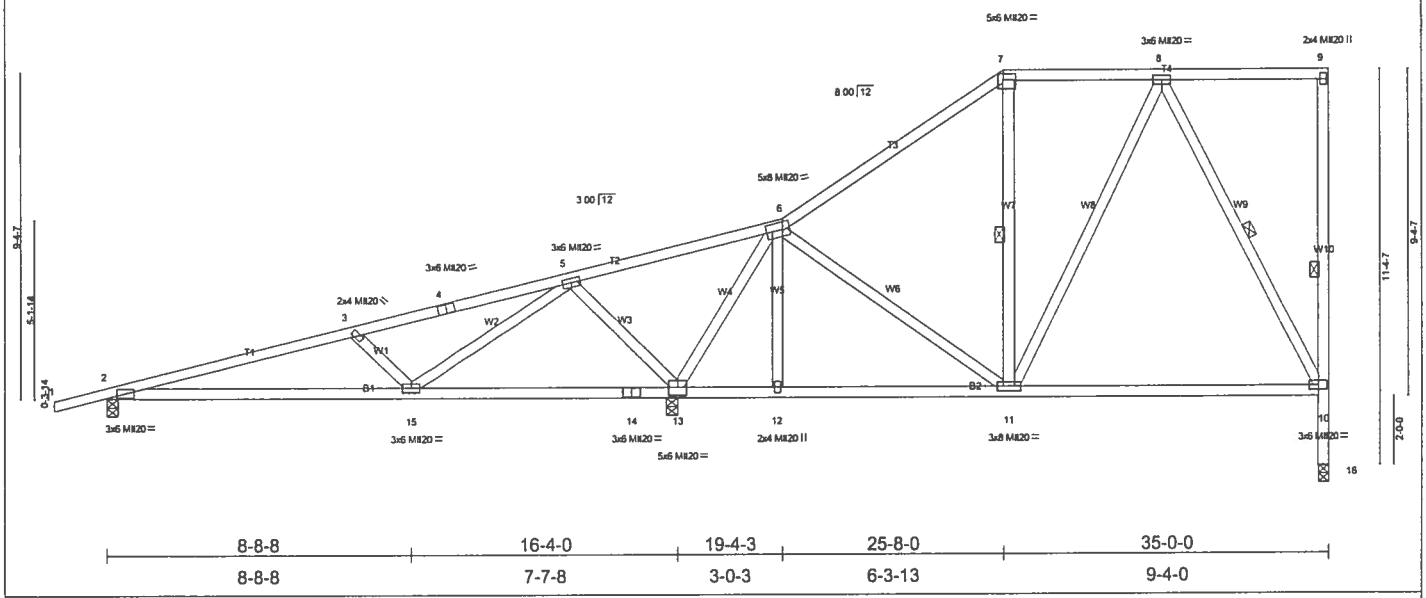


Plate Offsets (X,Y): [6:0-5-8,0-2-8], [7:0-4-4,0-2-4], [10:0-3-4,0-1-8], [13:0-3-0,0-2-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	0.29	2-15	>679	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.36	Vert(TL)	0.24	2-15	>793	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.73	Horz(TL)	0.12	16	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 212 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 9-16, 7-11, 8-10

REACTIONS (lb/size) 16=586/0-3-8, 2=554/0-4-0, 13=1864/0-4-0
Max Horz 2=441(load case 5)
Max Uplift16=-191(load case 4), 2=-486(load case 3), 13=-1013(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/18, 2-3=-810/613, 3-4=-523/413, 4-5=-450/424, 5-6=-981/926, 6-7=-392/12, 7-8=-242/50, 8-9=-20/0, 10-16=-586/191, 9-10=-122/83
BOT CHORD 2-15=-1007/754, 14-15=-333/164, 13-14=-333/164, 12-13=-164/400, 11-12=-166/399, 10-11=-72/194
WEBS 3-15=-425/426, 5-15=-1093/990, 5-13=-778/816, 6-13=-1319/764, 6-12=0/89, 6-11=-427/513, 7-11=-135/245, 8-11=-19/247, 8-10=-380/157

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Bearing at joint(s) 16 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 191 lb uplift at joint 16, 486 lb uplift at joint 2 and 1013 lb uplift at joint 13.

LOAD CASE(S) Standard

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 66877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR	Dwg.#030805284
L106588	T20	MONO HIP	1	1	Job Reference (optional)	

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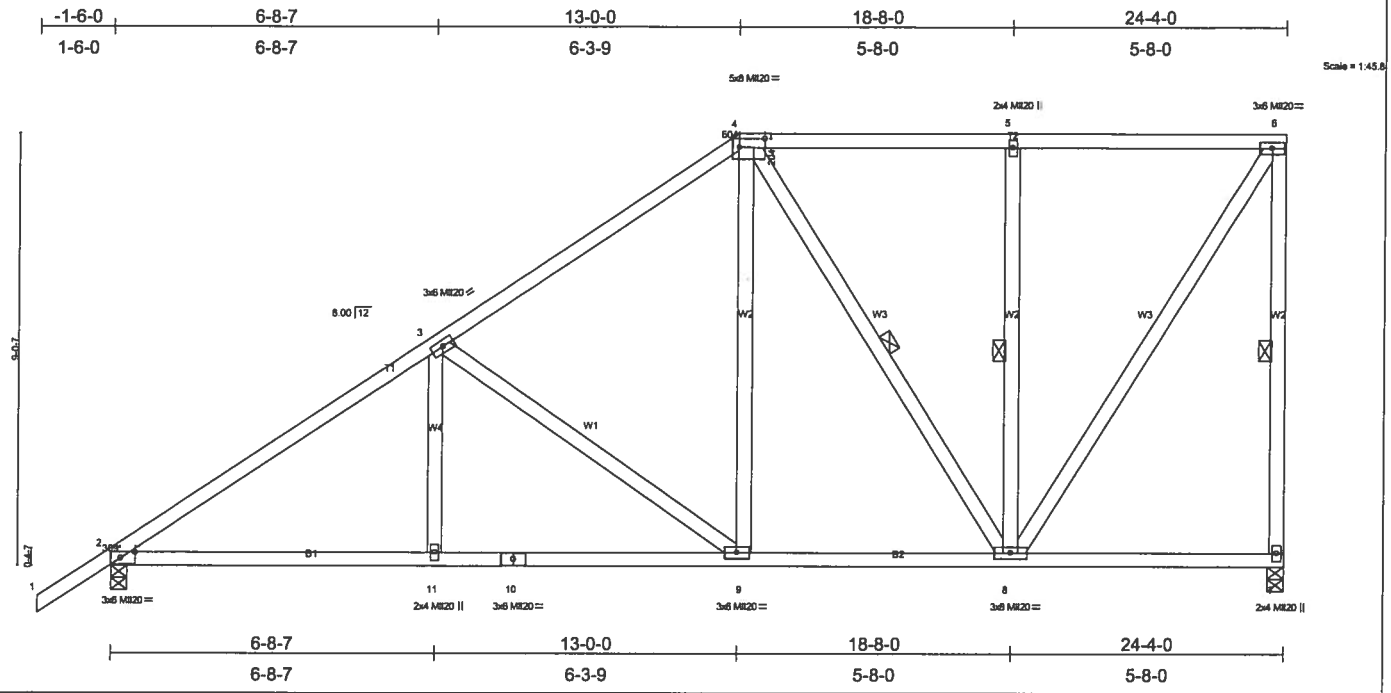

Plate Offsets (X,Y): [2:0-3-9,0-1-8], [4:0-6-4,0-2-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	-0.08	2-11	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.11	2-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.79	Horz(TL)	0.03	7	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 164 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 5-1-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-5-0 oc bracing.
WEBS 1 Row at midpt 6-7, 4-8, 5-8

REACTIONS (lb/size) 7=1006/0-4-0, 2=1102/0-4-0
Max Horz 2=464(load case 5)
Max Uplift7=-353(load case 4), 2=-354(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-1457/384, 3-4=-950/328, 4-5=-515/233, 5-6=-514/234, 6-7=-927/456
BOT CHORD 2-11=-588/1124, 10-11=-588/1124, 9-10=-588/1124, 8-9=-350/715, 7-8=-6/14
WEBS 3-11=0/225, 3-9=-510/325, 4-9=-150/465, 4-8=-369/234, 5-8=-315/301, 6-8=-427/937

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 353 lb uplift at joint 7 and 354 lb uplift at joint 2.

LOAD CASE(S) Standard

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR
L106588	T21	MONO HIP	1	1	Job Reference (optional)

Dwg.#030805285

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Dec 15 2004 MiTek Industries, Inc. Mon Mar 07 16:56:21 2005 Page 1

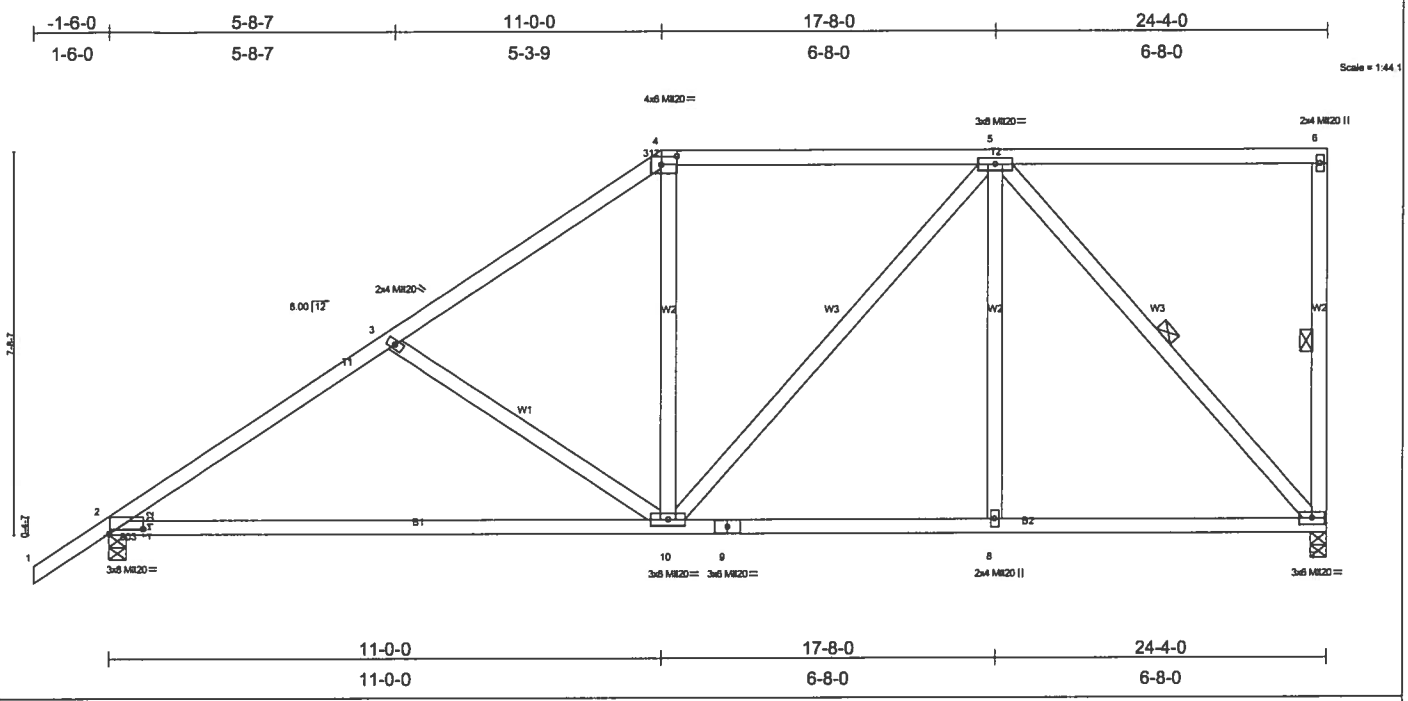

Plate Offsets (X,Y): [2:0-8-3,0-1-2], [4:0-3-12,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	-0.31	2-10	>926	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.55	Vert(TL)	-0.46	2-10	>624	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.69	Horz(TL)	0.03	7	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 148 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 5-0-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-4-12 oc bracing.
WEBS 1 Row at midpt 6-7, 5-7

REACTIONS (lb/size) 7=1006/0-4-0, 2=1102/0-4-0
Max Horz 2=403(load case 5)
Max Uplift7=-361(load case 4), 2=-354(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-1369/452, 3-4=-1113/386, 4-5=-860/390, 5-6=-25/10, 6-7=-157/126
BOT CHORD 2-10=-598/1094, 9-10=-303/683, 8-9=-303/683, 7-8=-303/683
WEBS 3-10=-288/283, 4-10=0/261, 5-10=-165/268, 5-8=0/162, 5-7=-996/445

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 361 lb uplift at joint 7 and 354 lb uplift at joint 2.

LOAD CASE(S) Standard

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR
L106588	T22	MONO HIP	1	1	Job Reference (optional)

Dwg.#030805286

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Dec 15 2004 MiTek Industries, Inc. Mon Mar 07 16:56:21 2005 Page 1

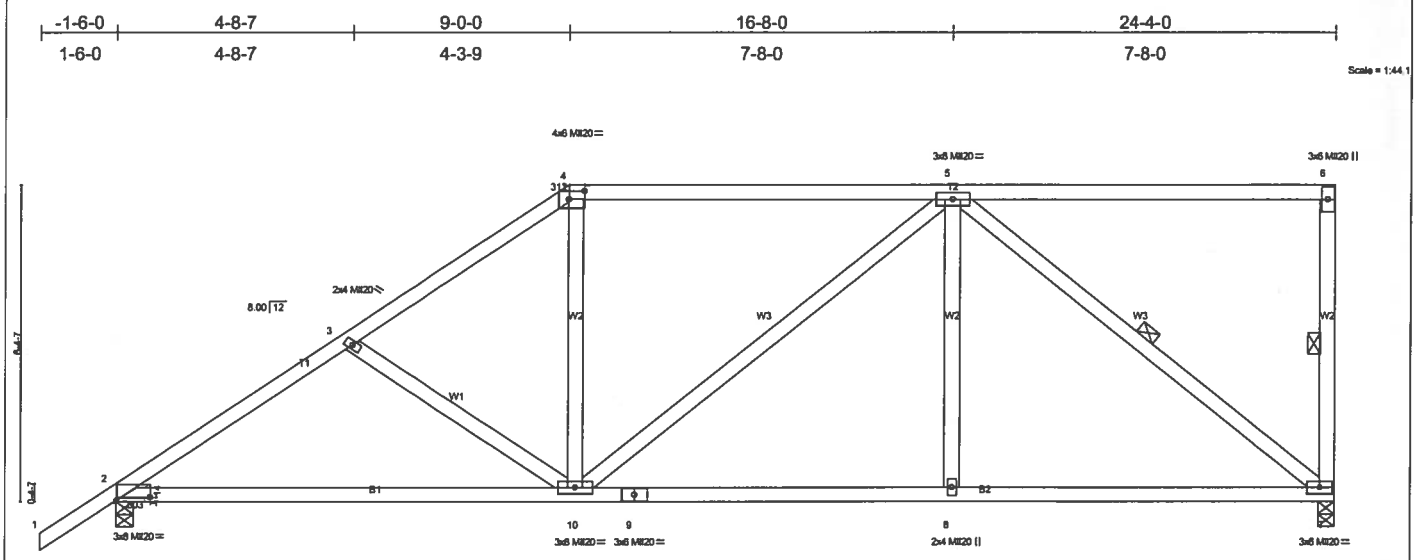

Plate Offsets (X,Y): [2:0-8-3,0-0-14], [4:0-3-12,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.15	2-10	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.42	Vert(TL)	-0.21	2-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.75	Horz(TL)	0.03	7	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 139 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 5-2-6 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-6-10 oc bracing.
WEBS 1 Row at midpt 6-7, 5-7

REACTIONS (lb/size) 7=1006/0-4-0, 2=1102/0-4-0
Max Horz 2=341(load case 5)
Max Uplift7=-381(load case 3), 2=-348(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-1421/489, 3-4=-1230/446, 4-5=-985/436, 5-6=-44/19, 6-7=-183/149
BOT CHORD 2-10=-580/1131, 9-10=-393/924, 8-9=-393/924, 7-8=-393/924
WEBS 3-10=-187/214, 4-10=-11/320, 5-10=-111/146, 5-8=0/219, 5-7=-1132/484

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 381 lb uplift at joint 7 and 348 lb uplift at joint 2.

LOAD CASE(S) Standard

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR
L106588	T23	MONO HIP	1	1	Job Reference (optional)

Dwg.#030805287

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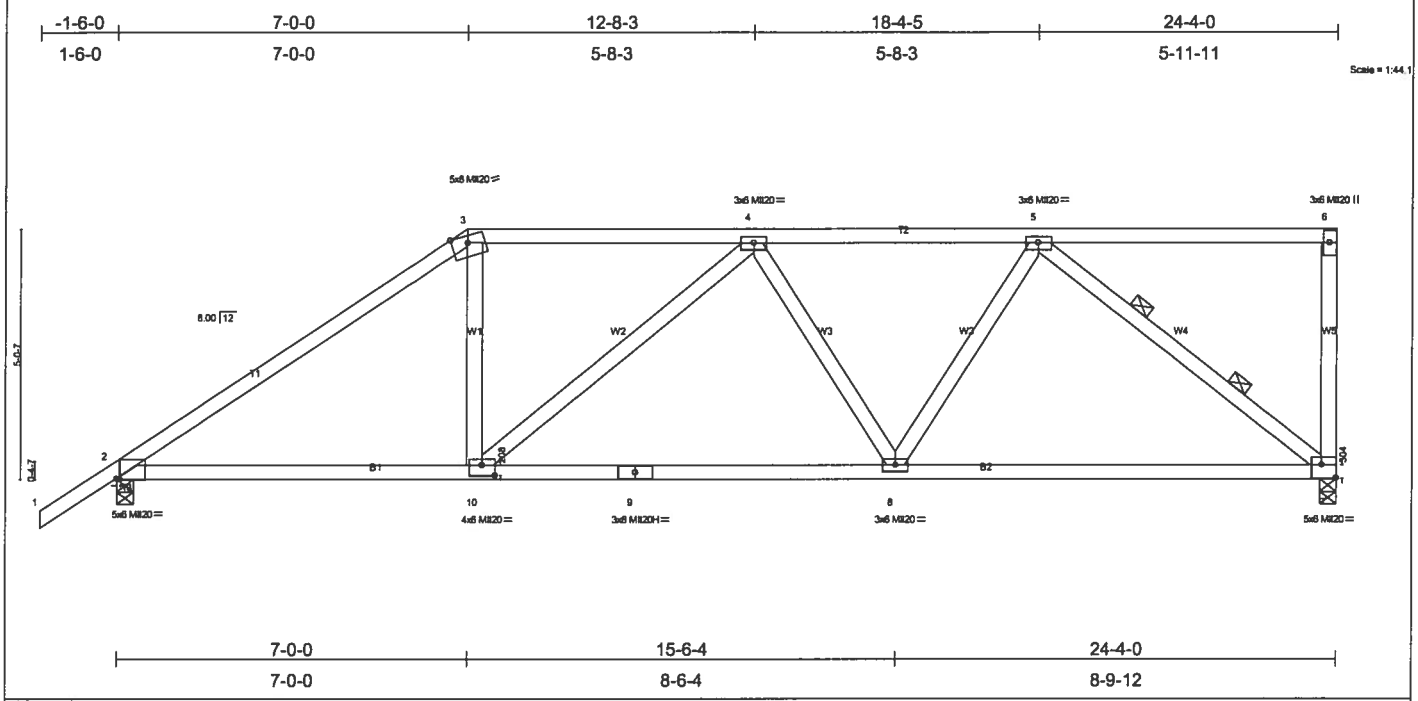

Plate Offsets (X,Y): [2:0-0-13,Edge], [7:Edge,0-3-4], [10:0-3-0,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.60	Vert(LL)	-0.25	8-10	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.92	Vert(TL)	-0.37	8-10	>785	180	MII20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.63	Horz(TL)	0.08	7	n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 128 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D *Except*
B2 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W5 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 3-3-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-10-1 oc bracing.
WEBS 2 Rows at 1/3 pts 5-7

REACTIONS (lb/size) 7=2243/0-4-0, 2=2083/0-4-0
Max Horz 2=281(load case 4)
Max Uplift7=-1363(load case 2), 2=-1085(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-3200/1730, 3-4=-2595/1501, 4-5=-2712/1531, 5-6=-120/43, 6-7=-330/343
BOT CHORD 2-10=-1484/2556, 9-10=-1790/2933, 8-9=-1790/2933, 7-8=-1349/2140
WEBS 3-10=-658/1227, 4-10=-446/526, 4-8=-429/527, 5-8=-374/1115, 5-7=-2605/1685

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) All plates are MT20 plates unless otherwise indicated.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1363 lb uplift at joint 7 and 1085 lb uplift at joint 2.
5) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 410 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-6=-117(F=-63), 2-10=-30, 7-10=-65(F=-35)
Concentrated Loads (lb)
Vert: 10=-539(F)

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR	Dwg.#030805288
L106588	T25	COMMON	21	1	Job Reference (optional)	

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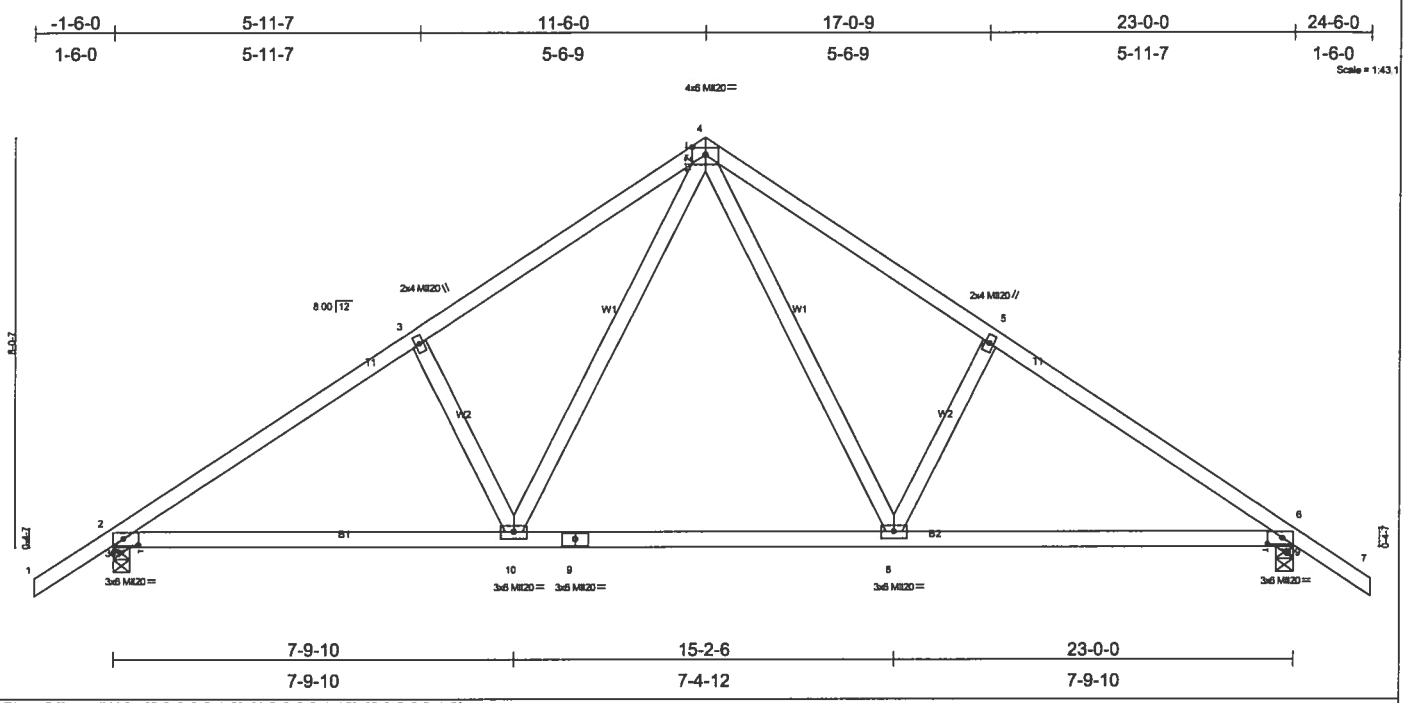

Plate Offsets (X,Y): [2:0-3-9,0-1-8], [4:0-3-0,0-1-12], [6:0-3-9,0-1-8]

LOADING (psf)		SPACING 2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	-0.21	8-10	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.78	Vert(TL)	-0.29	8-10	>929	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.41	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 118 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 4-9-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-8-9 oc bracing.

REACTIONS (lb/size) 2=1227/0-4-0, 6=1227/0-4-0
Max Horz 2=-273(load case 3)
Max Uplift2=-444(load case 5), 6=-444(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-1692/581, 3-4=-1551/643, 4-5=-1551/643, 5-6=-1692/581, 6-7=0/45
BOT CHORD 2-10=-440/1324, 9-10=-184/903, 8-9=-184/903, 6-8=-314/1324
WEBS 3-10=-254/276, 4-10=-314/752, 4-8=-314/752, 5-8=-254/276

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 444 lb uplift at joint 2 and 444 lb uplift at joint 6.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-10=-30, 8-10=-80(F=-50), 6-8=-30

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR
L106588	T26	HIP	1	1	Job Reference (optional)

Dwg.#030805289

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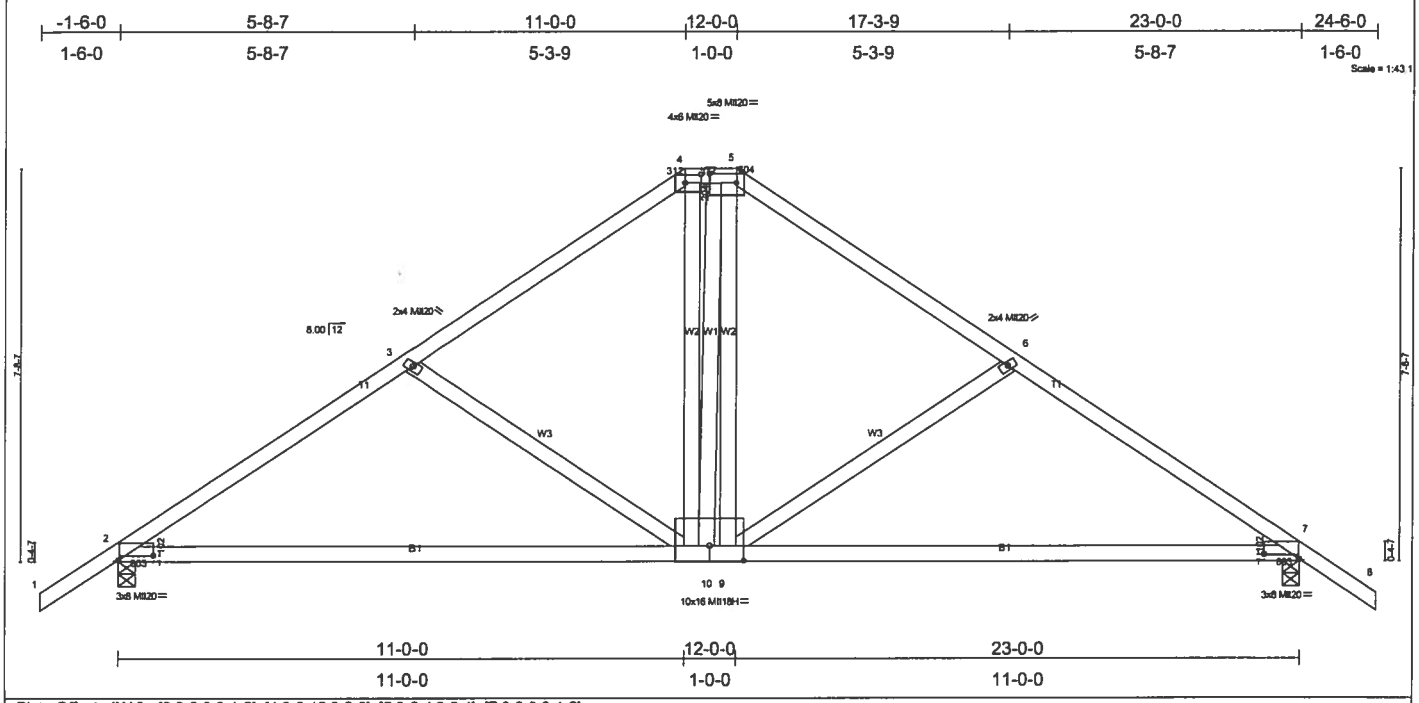

Plate Offsets (X,Y): [2:0-8-3,0-1-2], [4:0-3-12,0-2-0], [5:0-6-4,0-2-4], [7:0-8-3,0-1-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.28	2-10	>959	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.74	Vert(TL)	-0.42	2-10	>648	180	MII18H	195/188
BCLL	10.0	Rep Stress Incr	YES	WB	0.25	Horz(TL)	0.03	7	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 132 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 5-3-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=1042/0-4-0, 7=1042/0-4-0
Max Horz 2=263(load case 4)
Max Uplift2=-371(load case 5), 7=-371(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-1274/478, 3-4=-1003/401, 4-5=-781/407, 5-6=-1016/406, 6-7=-1278/476, 7-8=0/45
BOT CHORD 2-10=-336/1015, 9-10=-89/771, 7-9=-233/1018
WEBS 3-10=-311/283, 4-10=-160/399, 5-10=-187/259, 5-9=-97/252, 6-9=-308/283

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 371 lb uplift at joint 2 and 371 lb uplift at joint 7.

LOAD CASE(S) Standard

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR
L106588	T27	HIP	1	1	Job Reference (optional)

Dwg.#030805290

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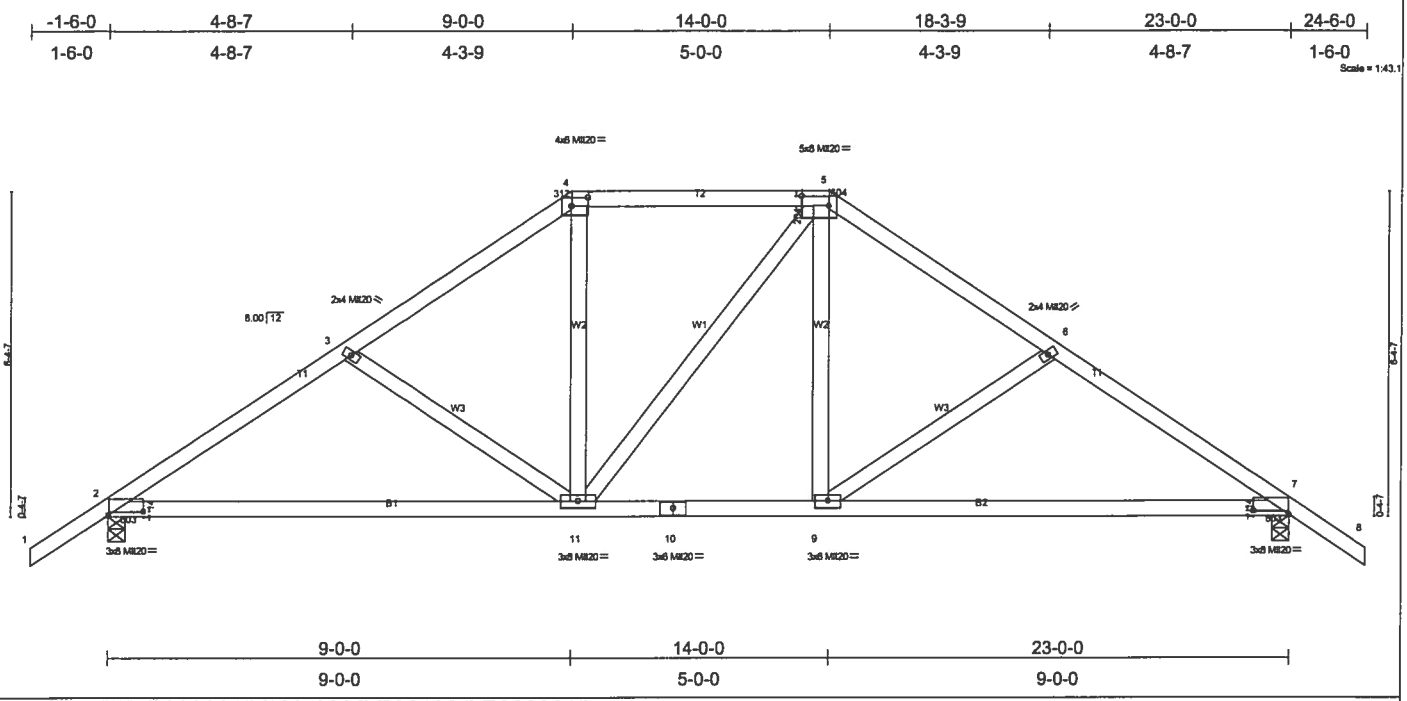

Plate Offsets (X,Y): [2:0-8-3,0-0-14], [4:0-3-12,0-2-0], [5:0-6-4,0-2-4], [7:0-8-3,0-0-14]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	-0.16	7-9	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.24	7-9	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.13	Horz(TL)	0.03	7	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 123 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 5-4-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=1042/0-4-0, 7=1042/0-4-0
Max Horz 2=216(load case 4)
Max Uplift2=-356(load case 5), 7=-356(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-1317/495, 3-4=-1113/446, 4-5=-876/429, 5-6=-1113/446, 6-7=-1317/495, 7-8=0/45
BOT CHORD 2-11=-330/1051, 10-11=-147/876, 9-10=-147/876, 7-9=-260/1051
WEBS 3-11=-221/221, 4-11=-75/329, 5-11=-127/127, 5-9=-81/330, 6-9=-222/221

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 356 lb uplift at joint 2 and 356 lb uplift at joint 7.

LOAD CASE(S) Standard

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	A.SIMQUE - LOT 5 HR
L106588	T28	HIP	1	1	Dwg.#030805291

Job Reference (optional)

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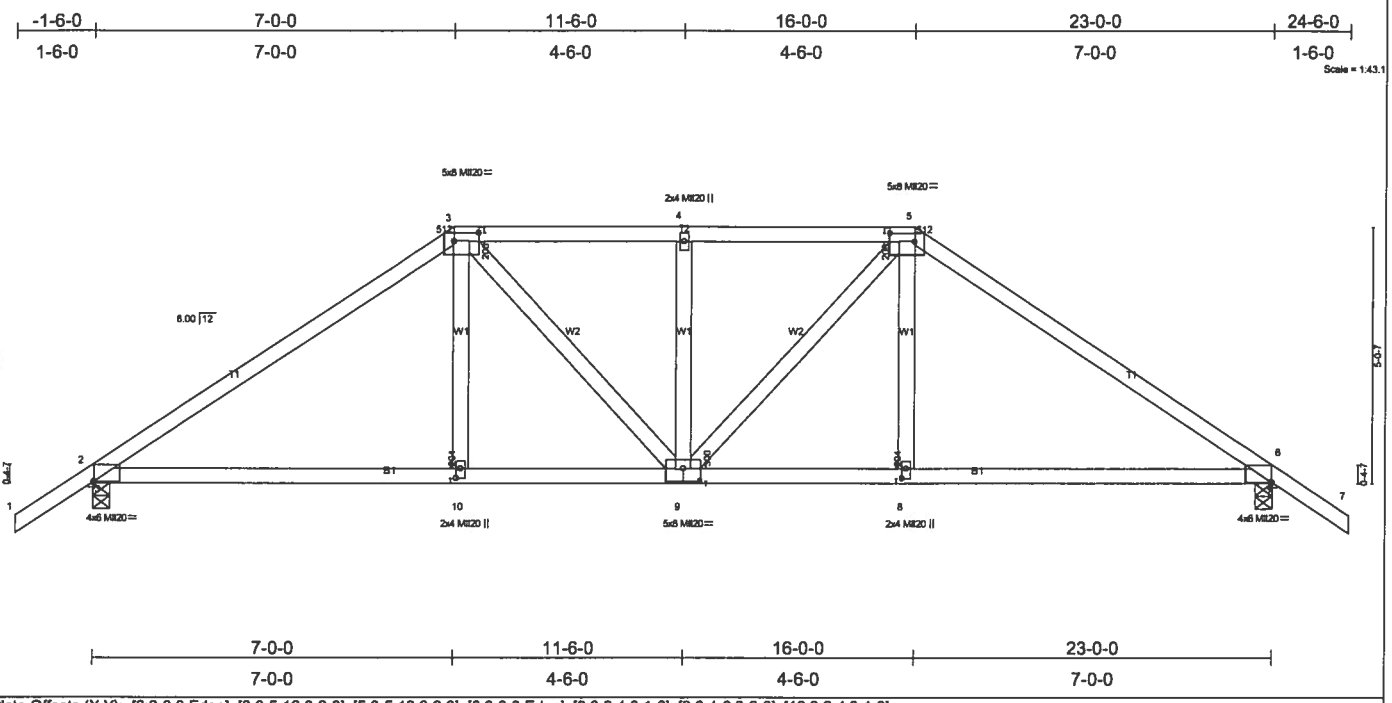


Plate Offsets (X,Y): [2:0-0-3,Edge], [3:0-5-12,0-2-0], [5:0-5-12,0-2-0], [6:0-0-3,Edge], [8:0-2-4,0-1-0], [9:0-4-0,0-3-0], [10:0-2-4,0-1-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	0.12	8-9	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.68	Vert(TL)	-0.18	6-8	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.36	Horz(TL)	0.07	6	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 116 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 3-4-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-4-10 oc bracing.

REACTIONS (lb/size) 2=2017/0-4-0, 6=2017/0-4-0
Max Horz 2=169(load case 3)
Max Uplift2=-1084(load case 4), 6=-1084(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-3046/1675, 3-4=-2799/1664, 4-5=-2799/1664, 5-6=-3046/1675, 6-7=0/45
BOT CHORD 2-10=-1443/2428, 9-10=-1454/2454, 8-9=-1315/2454, 6-8=-1305/2428
WEBS 3-10=-374/822, 3-9=-507/605, 4-9=-511/635, 5-9=-507/605, 5-8=-374/822

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1084 lb uplift at joint 2 and 1084 lb uplift at joint 6.
5) Girder carries hip end with 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 410 lb up at 16-0-0, and 539 lb down and 410 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-117(F=-63), 5-7=-54, 2-10=-30, 8-10=-65(F=-35), 6-8=-30
Concentrated Loads (lb)
Vert: 10=-539(F) 8=-539(F)

MARCH 8, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

COLUMBIA COUNTY BUILDING DEPARTMENT

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2001 ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE MARCH 1, 2002

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 1606 OF THE FLORIDA BUILDING CODE 2001 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1606 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ------ 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE ------110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS; Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☐	☐	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☐	☐	Designers name and signature on document (FBC 104.2.1). If licensed WILLIAM MYERS architect or engineer, official seal shall be affixed. MARK DISOSWAY
☐	☑	**Site Plan including:** See ATTACH SITE PLAN a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property. PROPERTY APPRAISER
☐	☑	**Wind-load Engineering Summary, calculations and any details required** a) Plans or specifications must state compliance with FBC Section 1606 CERTIFICATION b) The following information must be shown as per section 1606.1.7 FBC BY MARK DISOS See Sheet S-1 a. Basic wind speed (MPH) 110 b. Wind importance factor (I) and building category I II c. Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated B d. The applicable internal pressure coefficient e. Components and Cladding. The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional
☐	☑	**Elevations including:**
☐	☑	a) All sides
☐	☑	b) Roof pitch 12/8
☐	☑	c) Overhang dimensions and detail with attic ventilation 24" MAX CONTINUOUS RIDGE VENT
☐	☑	d) Location, size and height above roof of chimneys NONE SHOWN ON ELEVATION PLAN
☐	☑	e) Location and size of skylights NONE SHOWN ON ELEVATION PLAN
☐	☑	f) Building height 21'4" PLUS FOUNDATION
☐	☑	e) Number of stories 1

Floor Plan including:

- ☐ ☑ a) Rooms labeled and dimensioned
- ☐ ☑ b) Shear walls Sheet S-3 TRAN. 138.5 LONG. 61.5 GLASS Blocks SEE ATTACHED PACKAGE
- ☐ ☑ c) Windows and doors (including garage doors) showing size, mfg., approval listing and attachment specs. (FBC 1707) and safety glazing where needed (egress windows in bedrooms to be shown) ALL Four bedroom show egress
- ☐ ☑ d) Fireplaces (gas appliance) (vented or non-vented) or wood burning with hearth
- ☐ ☑ e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails
- ☐ ☑ f) Must show and identify accessibility requirements (accessible bathroom) Both BATHroom

Foundation Plan including:

- ☐ ☑ a) Location of all load-bearing wall with required footings indicated as standard Or monolithic and dimensions and reinforcing see sheet S-2
- ☐ ☑ b) All posts and/or column footing including size and reinforcing See sheet S-2 F-5
- ☐ ☑ c) Any special support required by soil analysis such as piling
- ☐ ☑ d) Location of any vertical steel

Roof System:

- ☐ ☑ a) Truss package including: FirstSource Builders
  1. Truss layout and truss details signed and sealed by Fl. Pro. Eng. Thomas E. Miller
  2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating) See Sheet S-1 Anchor TABLE
- ☐ ☑ b) Conventional Framing Layout including:
  1. Rafter size, species and spacing
  2. Attachment to wall and uplift
  3. Ridge beam sized and valley framing and support details
  4. Roof assembly (FBC 104.2.1 Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

- ☐ ☑ a) Masonry wall
  1. All materials making up wall
  2. Block size and mortar type with size and spacing of reinforcement
  3. Lintel, tie-beam sizes and reinforcement
  4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
  5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation
  6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
  7. Fire resistant construction (if required)
  8. Fireproofing requirements
  9. Shoe type of termite treatment (termiticide or alternative method)
  10. Slab on grade
      a. Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
      b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
  11. Indicate where pressure treated wood will be placed
  12. Provide insulation R value for the following:
      a. Attic space
      b. Exterior wall cavity
      c. Crawl space (if applicable)

☐ ☑ b) Wood frame wall

1. All materials making up wall
2. Size and species of studs 2x4/6 ON 16" OC
3. Sheathing size, type and nailing schedule 7/16 OSB
4. Headers sized See Sheet S-3 Header legend — 10x7 OHd. For Garage Headers
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail Sheet S-1 TYPICAL GABLE END X BRACING
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating) Sheet S-1 ANCHOR TABLE
8. Fire resistant construction (if applicable)
9. Fireproofing requirements See ONE STORY WALL SECTION Sheet S-1
10. Show type of termite treatment (termiticide or alternative method) Sheet S-2
11. Slab on grade Sheet S-2
    a. Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports — Sheet S-2
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space R-30 Sheet A1
    b. Exterior wall cavity R-13
    c. Crawl space (if applicable)

☐ ☑ X c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

☐ ☑ a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer

☐ ☐ b) Floor joist size and spacing

☐ ☐ X c) Girder size and spacing

☐ ☐ d) Attachment of joist to girder

☐ ☐ e) Wind load requirements where applicable

☐ ☑ **Plumbing Fixture layout**

Electrical layout including:

☐ ☑ a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified

☐ ☑ b) Ceiling fans 3

☐ ☑ c) Smoke detectors 5

☐ ☑ d) Service panel and sub-panel size and location(s) IN GARAGE

☐ ☑ e) Meter location with type of service entrance (overhead or underground) UNKNOWN

☐ ☑ f) Appliances and HVAC equipment

☐ ☑ g) Arc Fault Circuits (AFCI) in bedrooms See ELECTRICAL NOTES Sheet A.4

HVAC information

☐ ☑ a) Manual J sizing equipment or equivalent computation

☐ ☑ b) Exhaust fans in bathroom

☐ ☑ **Energy Calculations** (dimensions shall match plans) DO MATCH

☐ ☑ **Gas System** Type (LP or Natural) Location and BTU demand of equipment

X **Disclosure Statement for Owner Builders**

Notice Of Commencement NOT AS OF 3-22-05

Private Potable Water

a) Size of pump motor LYNCH WELL DRILLING

b) Size of pressure tank

c) Cycle stop valve if used

MI HOME PRODUCTS

- PRIME ALUMINUM WINDOWS -

INSTALLATION INSTRUCTIONS FOR "NAIL FIN" PRODUCTS

MI Home Products appreciates your recent purchase of a maintenance free prime window, which will not rust, rot, mildew, or warp. This is a quality product that left our factory in good condition – proper handling and installation are just as important as good design and workmanship. Please follow these recommendations to allow this product to complete its function.

1 Handle units one at a time in the closed and locked position and take care not to scratch frame or glass or to bend the nailing fin.

2. Set unit plumb and square into opening and make sure that there is 3/16" ± 1/16" clearance around the frame. Fasten unit into opening in the closed and locked position, making sure that fasteners are screwed in straight in order to avoid twisting or bowing of the frame. Make sure that sill is straight and level. Check operation of unit before any and all fasteners are set.

3. Use # 8 sheet metal or wood screws with a minimum of 1" penetration into the framing (stud). Place first screws (two at each corner) 3" from end of fin. For positive and negative DPs (design pressures) up to **35**, do not exceed **24"** spacing of additional screws. For DPs from **35.1** to **50**, do not exceed **18"**. Install load bearing shim adjacent to each anchor. Use shim where space exceeds 1/16".

4. Flash over head and caulk outside perimeter in accordance with code requirements and good installation practices.

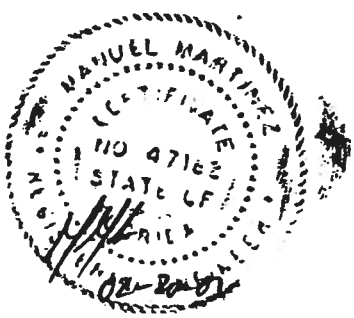
5. Fill voids between frame and construction with loose batten type insulation or non-expanding aerosol foam specifically formulated for windows and doors to eliminate drafts. The use of expanding aerosol type insulating foam, which can bow the frame, waives all stated warranties.

6. Remove plaster, mortar, paint and any other debris that may have collected on the unit and make sure that sash/vent tracks and interlocks are also clear. Do not use abrasives, solvents, ammonia, vinegar, alkaline, or acid solutions for clean-up, especially with insulated glass units as their use could cause chemical breakdown of the glass seal. Take care not to scratch glass; scratches severely weaken glass and it could eventually break from thermal expansion and contraction. Clean units with water and mild detergent as you would you automobile.

CAUTION -

MI Home Products or its representatives are unable to control and cannot assume responsibility for the selection and placement of their products in a building or structure in a manner required by laws, statutes, and/or building codes. The purchaser is solely responsible for knowledge of and adherence to the same. MI Home Products window products are not provided with safety glazing unless specifically ordered with such. Many laws and codes require safety glazing near doors, bathtubs, and shower enclosures. Also be aware of emergency egress code requirements.

Corporate Headquarters:
650 West Market St.
Gratz, PA 17030-0370
(717) 365-3300



AAMA/NWWDA 101/I.S.2-97
TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650 Fin
TYPE: Aluminum Single Hung Window

Title of Test	Results
Rating	H-R40 52 x 72
Overall Design Pressure	+45.0 psf -47.2 psf
Operating Force	11 lb max.
Air Infiltration	0.13 cfm/ft^2
Water Resistance	6.00 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-41134.01 dated 03/26/02 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess, Technician

MAH:nlb

1 APRIL 2002

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
650 West Market Street
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-41134.01
Test Date: 03/07/02
Report Date: 03/26/02
Expiration Date: 03/07/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on Series/Model 650 Fin, aluminum single hung window at their facility located in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for a H-R40 52 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 650 Fin

Type: Aluminum Single Hung Window

Overall Size: 4' 4-1/4" wide by 6' 0-3/8" high

Active Sash Size: 4' 1-3/4" wide by 3' 0-5/8" high

Daylight Opening Size: 3' 11-3/8" wide by 2' 9-1/2" high

Screen Size: 4' 0-1/4" wide by 2' 11-1/8" high

Finish: All aluminum was white.

Glazing Details: The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap-around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Allen N. Reuss
1 APRIL 2002

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.230" high by 0.270" backed polypile with center fin	1 Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" x 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam-filled vinyl bulb seal	1 Row	Active sash, bottom rail

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. Meeting rail was secured to the frame utilizing two 1-1/4" screws.

Sash Construction: The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each jamb screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

Hardware:

Description	Quantity	Location
Metal cam lock with keeper	1	Midspan, active meeting rail with keeper adjacent on fixed meeting rail
Plastic tilt latch	2	Active sash, meeting rail ends
Metal tilt pin	2	Active sash, bottom rail ends
Balance assembly	2	One in each jamb
Screen plunger	2	4" from rail ends on top rail

Allen N. Reuss
1 APRIL 2002

Test Specimen Description: (Continued)

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood test buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	11 lbs	30 lbs max.
2.1.2	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.13 cfm/ft^2	0.3 cfm/ft^2 max.

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

2.1.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 25.9 psf (positive)	0.42"*	0.26" max.
	@ 34.7 psf (negative)	0.43"*	0.26" max.

**Exceeds L/175 for deflection, but passes all other test requirements.*

2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 38.9 psf (positive)	0.02"	0.18" max.
	@ 52.1 psf (negative)	0.02"	0.18" max.

Allen M. Rees
1 APRIL 2002

Test Specimen Description: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.2	Deglazing Test (ASTM E 987)		
	In operating direction at 70 lbs		
	Meeting rail	0.12"/25%	0.50"/100%
	Bottom rail	0.12"/25%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.06"/12%	0.50"/100%
	Right stile	0.06"/12%	0.50"/100%
2.1.8	Forced Entry Resistance (ASTM F 588-97)		
	Type: A Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Tests A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 6.00 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 45.0 psf (positive)	0.47"*	0.26" max.
	@ 47.2 psf (negative)	0.46"*	0.26" max.
4.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 67.5 psf (positive)	0.05"	0.18" max.
	@ 70.8 psf (negative)	0.05"	0.18" max.

Exceeds L/175 for deflection, but passes all other test requirements.

Allen M. Reese
1 APRIL 2002

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

Allen N. Reeves, P.E.
Director - Engineering Services
1 APRIL 2002

MAH:mlb
01-41134.01

AAMA/NWWDA 101/I.S.2-97
TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 450/650/850 Drop In Glazing
TYPE: Aluminum Single Hung Window

Title	Summary of Results
AAMA Rating	H-LC30 53 x 90
Operating Force	24 lb max.
Air Infiltration	0.11 cfm/ft^2
Water Resistance Test Pressure	6.75 psf
Uniform Load Deflection Test Pressure	+32.8 psf -47.2 psf
Uniform Load Structural Test Pressure	+49.2 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to ATI Report No. 01-42487.01 for complete test specimen description and data.

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
P.O. Box 370
650 West Market Street
Gratz, Pennsylvania 17030-0370

Report No: 01-42487.01
Test Date: 08/14/02
And: 08/15/02
Report Date: 10/02/02
Expiration Date: 08/15/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on a Series/Model 450/650/850 Drop In Glazing, aluminum single hung window at their facility in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-LC30 53 x 90 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 450/650/850 Drop In Glazing

Type: Aluminum Single Hung Window

Overall Size: 4' 5-1/8" wide by 7' 5-5/8" high

Interior Sash Size: 4' 2-3/4" wide by 3' 8-7/8" high

Fixed Daylight Opening Size: 4' 0" wide by 3' 5-3/8" high

Screen Size: 4' 0-3/4" wide by 3' 8-3/4" high

Finish: The unit was white.

Glazing Details: The specimen utilized 5/8" thick, sealed insulating glass constructed from two sheets of 3/32" thick, clear annealed glass and a metal reinforced butyl spacer system. The lites were interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.190" high by 0.187" polypile with center fin	1 Row	Fixed meeting rail interlock
0.190" high by 0.187" polypile with center fin	2 Rows	Interior sash stiles
1/4" vinyl foam-filled bulb seal	1 Row	Interior sash bottom rail
5/8" wide by 7/8" long polypile plug	4 Pieces	Interior sash, all corners

Frame Construction: The frame was constructed of extruded aluminum. Each corner was coped, butted, sealed, and fastened with two #8 x 1" screws per corner through the head and sill into jamb screw boss. End caps were utilized on the ends of the meeting rail and secured with two 1-1/4" screws per cap. Meeting rail was then secured to the frame utilizing two 1-1/4" screws.

Sash Construction: The sash was constructed of extruded aluminum. Each corner was coped, butted, and fastened with one #8 x 1-1/4" screw per corner.

Screen Construction: The screen was constructed of roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

Hardware:

Description	Quantity	Location
Metal cam lock	2	Interior sash, 6-1/2" from top rail ends
Spring-loaded coil balance	2	One per jamb
Plastic tilt latch	2	Interior sash top rail ends
Metal tilt latch pin	2	Interior sash bottom rail ends
Screen spring-loaded retainer pin	2	6-3/4" from rails on stiles

Test Specimen Description: (Continued)

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The specimen was installed into a #2 2 x 8 Spruce-Pine-Fir wood buck. #8 x 1-5/8" drywall screws were placed 3" from corners and 15" on center around nailing fin. Polyurethane was used a s a sealant around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	24 lbs	35 lbs max.
2.1.2	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.11 cfm/ft^2	0.3 cfm/ft^2 max.

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

2.1.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 3.75 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 52 seconds)		
	@ 25.0 psf (positive)	0.64"*	0.29" max.
	@ 25.0 psf (negative)	0.54"*	0.29" max.
2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 37.5 psf (positive)	0.04"	0.20" max.
	@ 37.5 psf (negative)	0.03"	0.20" max.

**Exceeds L/175 for deflection, but meets all other test requirements.*

Test Results:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.2	Deglazing Test (ASTM E 987-88)		
	In operating direction at 70 lbs		
	Interior sash meeting rail	0.12"/25%	0.50"/100%
	Interior sash bottom rail	0.12"/25%	0.50"/100%
	In remaining direction at 50 lbs		
	Interior sash right stile	0.06"/12%	0.50"/100%
	Interior sash left stile	0.06"/12%	0.50"/100%
2.1.8	Forced Entry Resistance (ASTM F 588-97)		
	Type: A Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 6.75 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 32.8 psf (positive)	0.85"*	0.29" max.
	@ 47.2 psf (negative)	0.87"*	0.29" max.
4.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 49.2 psf (positive)	0.09"	0.20" max.
	@ 70.8 psf (negative)	0.12"	0.20" max.

**Exceeds L/175 for deflection, but meets all other test requirements.*

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced except in full without the approval of Architectural Testing.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

Allen. N. Reeves, P.E.
Director - Engineering Services
11 OCTOBER 2002

MAH:nlb
01-42487.01

AAMA/NWWDA 101/I.S.2-97
TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650
TYPE: Aluminum Triple Single Hung Window

Title of Test	Summary of Results
AAMA Rating	H-R35 112 x 72
Uniform Load Deflection Test Pressure	+35.3 psf -47.2 psf
Operating Force	25 lb max.
Air Infiltration	0.16 cfm/ft^2
Water Resistance Test Pressure	5.25 psf
Uniform Load Structural Test Pressure	+53.0 psf -52.5 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to ATI Report No. 01-41641.01 for complete test specimen description and data.

Allen N. Reeves
7 JUNE 2002

ALLEN N. REEVES
NO. 18354
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.
P.O. Box 370
650 West Market Street
Gratz, Pennsylvania 17030-0370

Report No: 01-41641.01
Test Date: 05/13/02
And: 05/16/02
Report Date: 06/05/02
Expiration Date: 05/16/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to witness testing on a Series/Model 650, aluminum triple single hung window at their facility located in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-R35 112 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 650

Type: Aluminum Triple Single Hung Window

Overall Size: 9' 3-1/2" wide by 5' 11-11/16" high

Active Sash Size (3): 3' 0-1/4" wide by 2' 10-3/4" high

Fixed Daylight Opening Size (3): 2' 8-1/4" wide by 2' 9-1/8" high

Screen Size (3): 2' 9-1/8" wide by 2' 11" high

Finish: All aluminum was painted white.

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Allen N. Reeves
7 JUNE 2002

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Test Specimen Description: (Continued)

Glazing Details: The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap-around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

Weatherstripping:

Description	Quantity	Location
0.230" high by 0.270" backed polypile with center fin	Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" by 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam filled vinyl bulb seal	1 Row	Active sash, bottom rail

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. The meeting rail was secured to the frame utilizing two 1-1/4" screws. The mullions were secured utilizing four #8 x 1-1/4" screws through the head and sill into the mullion screw boss.

Sash Construction: The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each stiles' screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

Allen N. Reeves
7 JUNE 2002

Test Specimen Description: (Continued)

Hardware:

Description	Quantity	Location
Metal cam lock with keeper	1	Midspan of each active meeting rail with adjacent keepers
Plastic tilt latch	2	Each active sash meeting rail ends
Metal tilt pin	2	Each active sash bottom rail ends
Balance assembly	2	Each active sash contained one in each jamb
Screen plunger	2	Each screen contained two 4" from rail ends on top rail

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	25 lbs	30 lbs max.
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.16 cfm/ft^2	0.3 cfm/ft^2 max.

***Note #1**: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.*

Water Resistance (ASTM E 547-00) (with and without screen) WTP = 2.86 psf	No leakage	No leakage

Allen N. Reeves
7 JUNE 2002

ALLEN N. REEVES CERTIFICATE NO. 13254 STATE OF FLORIDA PROFESSIONAL ENGINEER

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.1.4.1	Uniform Load Deflection (ASTM E 330-97)		
	(Measurements reported were taken on the mullion)		
	(Loads were held for 52 seconds)		
	@ 15.0 psf (positive)	0.15"	0.41" max.
	@ 15.0 psf (negative)	0.29"	0.41" max.
2.1.4.2	Uniform Load Structural (ASTM E 330-97)		
	(Measurements reported were taken on the mullion)		
	(Loads were held for 10 seconds)		
	@ 22.5 psf (positive)	0.01"	0.29" max.
	@ 22.5 psf (negative)	0.01"	0.29" max.
2.2. .6.2	Deglazing Test (ASTM E 987-88)		
	In operating direction at 70 lbs		
	Right sash, meeting rail	0.12"/25%	0.50"/100%
	Right sash, bottom rail	0.12"/25%	0.50"/100%
	Middle sash, meeting rail	0.12"/25%	0.50"/100%
	Middle sash, bottom rail	0.12"/25%	0.50"/100%
	Left sash, meeting rail	0.12"/25%	0.50"/100%
	Left sash, bottom rail	0.12"/25%	0.50"/100%
	In remaining direction at 50 lbs		
	Right sash, right stile	0.06"/12%	0.50"/100%
	Right sash, left stile	0.06"/12%	0.50"/100%
	Middle sash, right stile	0.06"/12%	0.50"/100%
	Middle sash, left stile	0.06"/12%	0.50"/100%
	Left sash, right stile	0.06"/12%	0.50"/100%
	Left sash, left stile	0.06"/12%	0.50"/100%
2 .8	Forced Entry Resistance (ASTM F 588-97)		
	Type: A		
	Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry

Allen N. Reeves
7 JUNE 2002

ALLEN N. REEVES CERTIFICATE NO. 19354 STATE OF FLORIDA PROFESSIONAL ENGINEER

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
	Optional Performance		
4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 5.25 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 52 seconds)		
	@ 35.3 psf (positive)	0.46"*	0.41" max
	@ 47.2 psf (negative)	0.67"*	0.41" max

**Exceeds L/175 for deflection, but meets all other test requirements.*

Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 10 seconds)		
@ 53.0 psf (positive)	0.03"	0.29" max
@ 52.5 psf (negative)	0.02"	0.29" max

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC

Mark A. Hess
Technician

Allen N. Reeves, P.E.
Director - Engineering Services
7 JUNE 2002

ALLEN N. REEVES
CERTIFICATE
NO. 19364
STATE OF
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PROFESSIONAL ENGINEER

MAH:nlb
01-41641.01

AAMA/NWWDA 101/I.S.2-97
TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650
TYPE: Aluminum Picture Window

Title of Test	Results
Rating	F-R45 60 x 80
Overall Design Pressure	+45.0 psf -47.2 psf
Air Infiltration	0.04 cfm/ft^2
Water Resistance	8.25 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-41135.01 dated 03/26/02 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess, Technician

MAH:nlb

Allen N. Reeves
1 APRIL 2002

ALLEN N. REEVES CERTIFICATE NO. [illegible] STATE OF FLORIDA PROFESSIONAL ENGINEER

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.
650 West Market Street
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-41135.01
Test Date: 03/07/02
Report Date: 03/26/02
Expiration Date: 03/07/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on Series/Model 650, aluminum picture window at their facility located in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for a F-R45 60 x 80 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description

Series/Model: 650

Type: Aluminum Picture Window

Overall Size: 5' 0" wide by 6' 8" high

Daylight Opening Size: 4' 9-1/4" wide by 6' 5-1/4" high

Finish All aluminum was white.

Glazing Details: The test specimen utilized 7/8" thick, sealed insulating glass constructed from two sheets of 3/16" thick, clear annealed glass and a metal reinforced butyl spacer system. The glass was interior glazed against double-sided adhesive foam tape and secured with aluminum snap-in glazing beads.

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PROFESSIONAL ENGINEER

Allen M. Reeves
1 APR 02 2002

Test Specimen Description: (Continued)

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss.

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood test buck. #8 x 2-1/2" installation screws were utilized 18" on center around the interior perimeter. Polyurethane was utilized to seal the exterior.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
	Air Infiltration (ASTM E 283-91)		
	@ 1.57 psf (25 mph)	0.04 cfm/ft^2	0.3 cfm/ft^2 max.

Note #1: *The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.*

	Water Resistance (ASTM E 547-00)		
	WTP = 2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection (ASTM E 330-97)		
	(Measurements reported were taken on the jamb)		
	(Loads were held for 33 seconds)		
	@ 25.9 psf (positive)	0.01"	0.41" max.
	@ 34.7 psf (negative)	0.01"	0.41" max.
2.1.4.2	Uniform Load Structural (ASTM E 330-97)		
	(Measurements reported were taken on the jamb)		
	(Loads were held for 10 seconds)		
	@ 38.9 psf (positive)	0.0"	0.29" max.
	@ 52.1 psf (negative)	0.01"	0.29" max.

Allen H. Reeves
1 APRIL 2002

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
	Forced Entry Resistance (ASTM F 588-97)		
	Type: D Grade: 10		
	Hand and Tool Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance (ASTM E 547-00) WTP = 8.25 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the jamb) (Loads were held for 33 seconds)		
	@ 45.0 psf (positive)	0.02"	0.41" max.
	@ 47.2 psf (negative)	0.02"	0.41" max.
	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the jamb) (Loads were held for 10 seconds)		
	@ 67.5 psf (positive)	0.01"	0.29" max.
	@ 70.8 psf (negative)	0.02"	0.29" max.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

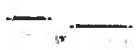
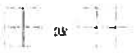
Mark A. Hess
Technician

MAH:nlb
01-41135.01

Allen N. Reeves, P.E.
Director - Engineering Services
1 APRIL 2002

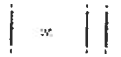
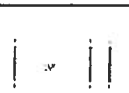
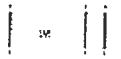
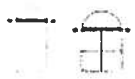
ALLEN N. REEVES
CERTIFICATE
NO. 19354
STATE OF FLORIDA
PROFESSIONAL ENGINEER

650 SH & PW Series - Mile Per Hour (MPH) maximum Size Chart

Series/Type	MPH Zone(s)	Required Mullion		Maximum Sizes Allowed		
				Single Unit	Twin Unit	Triple Unit
650 SH or PW Flange Frame Sng Glz or Ins	up to 120 MPH	Horizontal Mull #5765		N/A	53-1/8" x 72" w/Transom*	32" x 72" w/Transom*
650 SH or PW Flange Frame Sng Glz or Ins	up to 140 MPH	Horizontal Mull #5765		N/A	45" x 72" w/Transom*	30" x 72" w/Transom*
650 SH or PW Fin Frame Sng Glz or Ins	up to 110 MPH	Self-Stacking Sill #CM-45026		53-1/8" x 72" w/Transom*	N/A	N/A
650 SH or PW Fin Frame Sng Glz or Ins	up to 120 MPH	Self-Stacking Sill #CM-45026		48" x 72" w/Transom*	N/A	N/A
650 SH or PW Fin Frame Sng Glz or Ins	up to 140 MPH	Self-Stacking Sill #CM-45026		37" x 72" w/Transom*	N/A	N/A
650 SH or PW Continuous Hdr & Sill Sng Glz or Ins	up to 140 MPH	(No Mullion)		N/A	36" x 72"	36" x 72"

*1 All Transoms (1, 2, & 3-Lites) must be continuous frame.
*2 Transom units must be a minimum of 1/0 tall. The maximum transom height is one half the width of the transom.
Both Single Hung & Picture Windows can be used in combination up to the maximum sizes listed above.

650 SH & PW Series - Mile Per Hour (MPH) maximum Size Chart

Series/Type	MPH Zone(s)	Required Mullion		Maximum Sizes Allowed		
				Single Unit	Twin Unit	Triple Unit
650 SH or PW Fin or Flange Frame Sng Glz or Ins	up to 140 MPH	N/A	N/A	53-1/8" x 72"	N/A	N/A
650 SH or PW Fin Frame Sng Glz or Ins	up to 130 MPH	Vertical Mull #CM-65130		N/A	53-1/8" x 72"	53-1/8" x 72"
650 SH or PW Fin Frame Sng Glz or Ins	up to 140 MPH	Vertical Mull #CM-65130		N/A	53-1/8" x 63" or 42" x 72"	53-1/8" x 63" or 42" x 72"
650 SH or PW Flange Frame Sng Glz or Ins	up to 130 MPH	Vertical Mull #CM-65129		N/A	53-1/8" x 72"	53-1/8" x 72"
650 SH or PW Flange Frame Sng Glz or Ins	up to 140 MPH	Vertical Mull #CM-65129		N/A	53-1/8" x 63" or 42" x 72"	53-1/8" x 63" or 42" x 72"
650 SH or PW Fin Frame Sng Glz or Ins	up to 120 MPH	Horizontal Mull #CM-65131		N/A	53-1/8" x 72" w/Transom*	32" x 72" w/Transom*
650 SH or PW Fin Frame Sng Glz or Ins	up to 140 MPH	Horizontal Mull #CM-65131		N/A	45" x 72" w/Transom*	30" x 72" w/Transom*
650 SH or PW Flange Frame Sng Glz or Ins	up to 120 MPH	Horizontal Mull #CM-65129		N/A	37" x 72" w/Transom*	N/A
650 SH or PW Flange Frame Sng Glz or Ins	up to 140 MPH	Horizontal Mull #CM-65129		N/A	30" x 72" w/Transom*	N/A
650 SH or PW Fin Frame Sng Glz or Ins	up to 120 MPH	Horizontal Mull #5767		N/A	53-1/8" x 72" w/Transom*	32" x 72" w/Transom*
650 SH or PW Fin Frame Sng Glz or Ins	up to 140 MPH	Horizontal Mull #5767		N/A	45" x 72" w/Transom*	30" x 72" w/Transom*

*1 All Transoms (1, 2, & 3-Lites) must be continuous frame.
*2 Transom units must be a minimum of 1/0 tall. The maximum transom height is one half the width of the transom.
Both Single Hung & Picture Windows can be used in combination up to the maximum sizes listed above.

AAMA/NWWDA 101/I.S.2-97
TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650 Fin
TYPE: Aluminum Single Hung Window

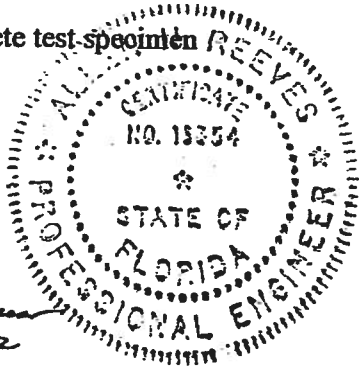
Title of Test	Results
Rating	H-R40 52 x 72
Overall Design Pressure	+45.0 psf -47.2 psf
Operating Force	11 lb max.
Air Infiltration	0.13 cfm/ft^2
Water Resistance	6.00 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-41134.01 dated 03/26/02 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess, Technician

MAH:nlb



1 APRIL 2002

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.
650 West Market Street
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-41134.01
Test Date: 03/07/02
Report Date: 03/26/02
Expiration Date: 03/07/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on Series/Model 650 Fin, aluminum single hung window at their facility located in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for a H-R40 52 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 650 Fin

Type: Aluminum Single Hung Window

Overall Size: 4' 4-1/4" wide by 6' 0-3/8" high

Active Sash Size: 4' 1-3/4" wide by 3' 0-5/8" high

Daylight Opening Size: 3' 11-3/8" wide by 2' 9-1/2" high

Screen Size: 4' 0-1/4" wide by 2' 11-1/8" high

Finish: All aluminum was white.

Glazing Details: The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

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Allen N. Reeves
1 APRIL 2002

REEVES
No. 19354
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.230" high by 0.270" backed polypile with center fin	1 Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" x 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam-filled vinyl bulb seal	1 Row	Active sash, bottom rail

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. Meeting rail was secured to the frame utilizing two 1-1/4" screws.

Sash Construction: The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each jamb screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

Hardware:

Description	Quantity	Location
Metal cam lock with keeper		Midspan, active meeting rail with keeper adjacent on fixed meeting rail
Plastic tilt latch	2	Active sash, meeting rail ends
Metal tilt pin	2	Active sash, bottom rail ends
Balance assembly	2	One in each jamb
Screen plunger	2	4" from rail ends on top rail

ALLEN N. REEVES
CERTIFICATE
NO. 19356
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Allen N. Reeves
1 APRIL 2002

Test Specimen Description: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.2	Deglazing Test (ASTM E 987)		
	In operating direction at 70 lbs		
	Meeting rail	0.12"/25%	0.50"/100%
	Bottom rail	0.12"/25%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.06"/12%	0.50"/100%
	Right stile	0.06"/12%	0.50"/100%
	Forced Entry Resistance (ASTM F 588-97)		
	Type: A		
	Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Tests A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 6.00 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 45.0 psf (positive)	0.47"*	0.26" max.
	@ 47.2 psf (negative)	0.46"*	0.26" max.

**Exceeds L/175 for deflection, but passes all other test requirements.*

	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 67.5 psf (positive)	0.05"	0.18" max.
	@ 70.8 psf (negative)	0.05"	0.18" max.

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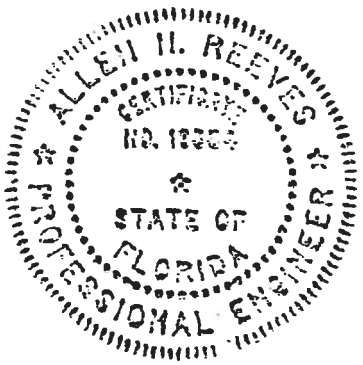
Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

Allen N. Reeves, P.E.
Director - Engineering Services
1 APRIL 2002

MAH:nlb
01-41134.01



AAMA/NWWDA 101/I.S.2-97
TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 4250, 4550, 8500, 8540
TYPE: PVC Fixed Window

	Results	
Title of Test	**Test Specimen #1**	**Test Specimen #2**
Ratings	F-C35 72 x 96	F-C50 65 x 84*
Overall Design Pressure	35 psf	50 psf
Air Infiltration	0.04 cfm/ft^2	N/A
Water Resistance	12.0 psf	N/A
Structural Test Pressure	+58.5, -63.0 psf	±75.0 psf
Deglazing	Passed	Passed
Forced Entry Resistance	Grade 40	N/A

Reference should be made to Report No. 01-39039.03 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Adam Fodor, Technician

AF:tjp/nlb

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
650 West Market Street
Gratz, Pennsylvania 17030-0370

Report No: 01-39039.03
Test Date: 03/09/01
Report Date: 10/19/01
Expiration Date: 03/09/05

Project Summary: Architectural Testing, Inc. (ATI) was contracted to witness tests on two Series/Model 4250, 4550, 8500, 8540, PVC fixed windows at the MI Home Products, Inc. in-plant test facility in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for the following ratings: Test Specimen #1 F-C35 72 x 96; Test Specimen #2F-C50 65 x 84*. Test specimen descriptions and results are reported herein.

General Note: An asterisk (*) next to the performance grade indicates that the size tested for optional performance was smaller than the gateway size for the product type and class.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 4250, 4550, 8500, 8540

Type: PVC Fixed Window

Test Specimen #1: F-C35 72 x 96

Overall Size: 5' 11-1/2" wide by 7' 11-1/2" high

Fixed Daylight Opening Size: 5' 7-3/4" wide by 7' 7-3/4" high

Glazing Type: 7/8" thick sealed insulating glass fabricated from two sheets of 3/16" thick tempered transparent glass.

130 Derry Court
York, PA 17402-9405
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fax: 717.764.4129
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