DATE 01/30/2008

Columbia County Building Permit

This Permit Must Be Prominently Posted on Premises During Construction

PERMIT
000026689

APPLICANT CHARESE NORTON PHONE 752-3331

ADDRESS 3367 S US HIGHWAY 441 LAKE CITY FL 32025

OWNER CHRIS & CHARNA RAINES PHONE 752-7069

ADDRESS 272 SE MYRTIS DOTCH TERR LAKE CITY FL 32025

CONTRACTOR JAMES NORTON PHONE 752-3331

LOCATION OF PROPERTY 90E, TR ON 100, TR ON PRICE CREEK RD, TL ON WEEKS,
TR ON MYRTIS DORTCH TERR,2ND ON RIGHT

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 151300.00

HEATED FLOOR AREA 1969.00 TOTAL AREA 3026.00 HEIGHT STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 10/12 FLOOR SLAB

LAND USE & ZONING A-3 MAX. HEIGHT 25

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 24-4S-17-08720-101 SUBDIVISION DEER HAMMOCK UNREC

LOT A BLOCK PHASE UNIT TOTAL ACRES 10.00

		RB0031780	Charese J Norton
Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor

EXISTING	08-0062	BK	JH	Y
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: FLOOR ONE FOOT ABOVE THE ROAD

Check # or Cash 24298

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by

Foundation _____ date/app. by

Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by

Slab _____ date/app. by

Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by

Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by

Heat & Air Duct _____ date/app. by

Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by

C.O. Final _____ date/app. by

Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by

Pool _____ date/app. by

Reconnection _____ date/app. by

Pump pole _____ date/app. by

Utility Pole _____ date/app. by

M/H Pole _____ date/app. by

Travel Trailer _____ date/app. by

Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 760.00 CERTIFICATION FEE $ 15.13 SURCHARGE FEE $ 15.13

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ **TOTAL FEE** 865.26

INSPECTORS OFFICE _____ CLERKS OFFICE MSS

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

For Office Use Only Application # 0801-91 Date Received 1/17 By JW Permit # 26689

Zoning Official B2K Date 29.01.08 Flood Zone X FEMA Map # N/A Zoning A-3

Land Use A-3 Elevation N/A MFE 1st floor Rd River N/A Plans Examiner DK JTH Date 1-29-08

Comments

(1) NOC ~~EH~~ ~~Deed or PA~~ ~~Site Plan~~ □ State Road Info □ Parent Parcel #

□ Dev Permit # □ In Floodway □ Letter of Authorization from Contractor

□ Unincorporated area □ Incorporated area □ Town of Fort White □ Town of Fort White Compliance letter

Septic Permit No. 08-0062 Fax 386-752-6427

Name Authorized Person Signing Permit Charese Norton Phone 386-752-3331

Address 3367 S. US Hwy 441, Ste 101, Lake City, FL 32025

Owners Name Chris & Charna Raines Phone 386-752-7069

911 Address 272 SE Myrtis Dortch Terr., Lake City, FL 32025

Contractors Name James H. Norton Phone 386-752-3331

Address 3367 S US Hwy 441, Ste 101, Lake City, FL 32025

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Tim Deibene, 192 SW Sagewood Gln. Lake City, FL 32024 Mark Disaway, P.O. Box 868, Lake City, FL 32056

Mortgage Lenders Name & Address 1st Fed Savings, 2571 W. US Hwy 90, Lake City, FL 32055

Circle the correct power company – FL Power & Light – (Clay Elec.) – Suwannee Valley Elec. – Progress Energy

Property ID Number 24-4S-17-08720-101 Estimated Cost of Construction $200,000.00

Subdivision Name N/A PARCEL "A" DEER HAMMACK UNREC Lot – Block – Unit – Phase –

Driving Directions SR 100 South, TR on Price Creek Rd (CR 245), TL on Weeks Lane, TR on Myrtis Dortch Terrace; 2nd on right Number of Existing Dwellings on Property 0

Construction of SFD, new home const. Total Acreage 10 Lot Size –

Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive Total Building Height 25'

Actual Distance of Structure from Property Lines - Front ~~320'~~ Side ~~150'~~ Side ~~149'~~ Rear ~~863'~~

Number of Stories 2 Heated Floor Area 1969 Total Floor Area 3026 Roof Pitch 10/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

JW called Charese 1.29.08

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment
According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:
YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

_____
Owners Signature

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

James H. Norton
Contractor's Signature (Permitee)

Contractor's License Number RB0031780
Columbia County
Competency Card Number 182

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 16th **day of** Jan **20**08

Personally known ✓ **or Produced Identification** _____

Patricia T Peeler
State of Florida Notary Signature (For the Contractor)

SEAL:
PATRICIA T. PEELER
Notary Public, State of Florida
My comm. exp. Sep. 5, 2010
Comm. No. DD 579471

Columbia County Property Appraiser

DB Last Updated: 1/15/2008

2008 Proposed Values

Tax Record | Property Card | Interactive GIS Map | Print

Parcel: 24-4S-17-08720-101

Owner & Property Info

Search Result: 1 of 1

Owner's Name	RAINES CHRISTOPHER A & CHARNA		
Site Address			
Mailing Address	E 156 SE PEYTON LOOP APT 105 LAKE CITY, FL 32025		
Use Desc. (code)	NO AG ACRE (009900)		
Neighborhood	24417.00	**Tax District**	3
UD Codes	MKTA04	**Market Area**	04
Total Land Area	10.010 ACRES		
Description	COMM AT NE COR OF SEC, RUN S 746.57 FT FOR POB, CONT SOUTH 347.88 FT, W 1253.98 FT, NORTH 347.91 FT, E 1253.16 FT TO POB ORB 1035-2121 AKA PARCEL "A" DEER HAMMOCK UNR		

GIS Aerial

Property & Assessment Values

Mkt Land Value	cnt: (1)	$42,942.00
Ag Land Value	cnt: (0)	$0.00
Building Value	cnt: (0)	$0.00
XFOB Value	cnt: (0)	$0.00
Total Appraised Value		$42,942.00

Just Value	$42,942.00
Class Value	$0.00
Assessed Value	$42,942.00
Exempt Value	$0.00
Total Taxable Value	$42,942.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale Vlmp	Sale Qual	Sale RCode	Sale Price
1/17/2005	1035/2122	WD	V	Q		$38,000.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

Extra Features & Out Buildings

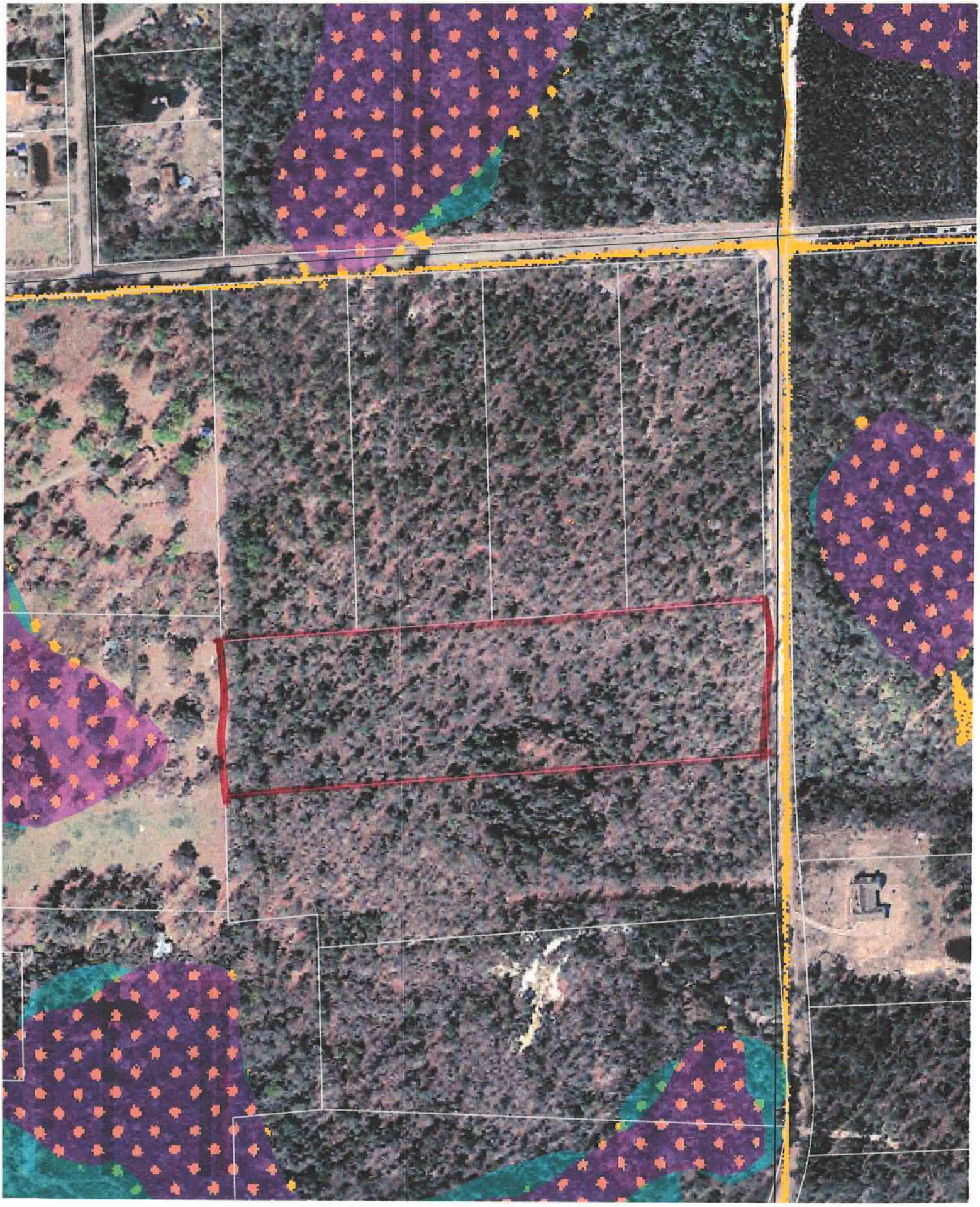
Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
009900	AC NON-AG (MKT)	10.010 AC	1.00/1.00/1.00/1.00	$4,290.00	$42,942.00

Columbia County Property Appraiser DB Last Updated: 1/15/2008

1 of 1



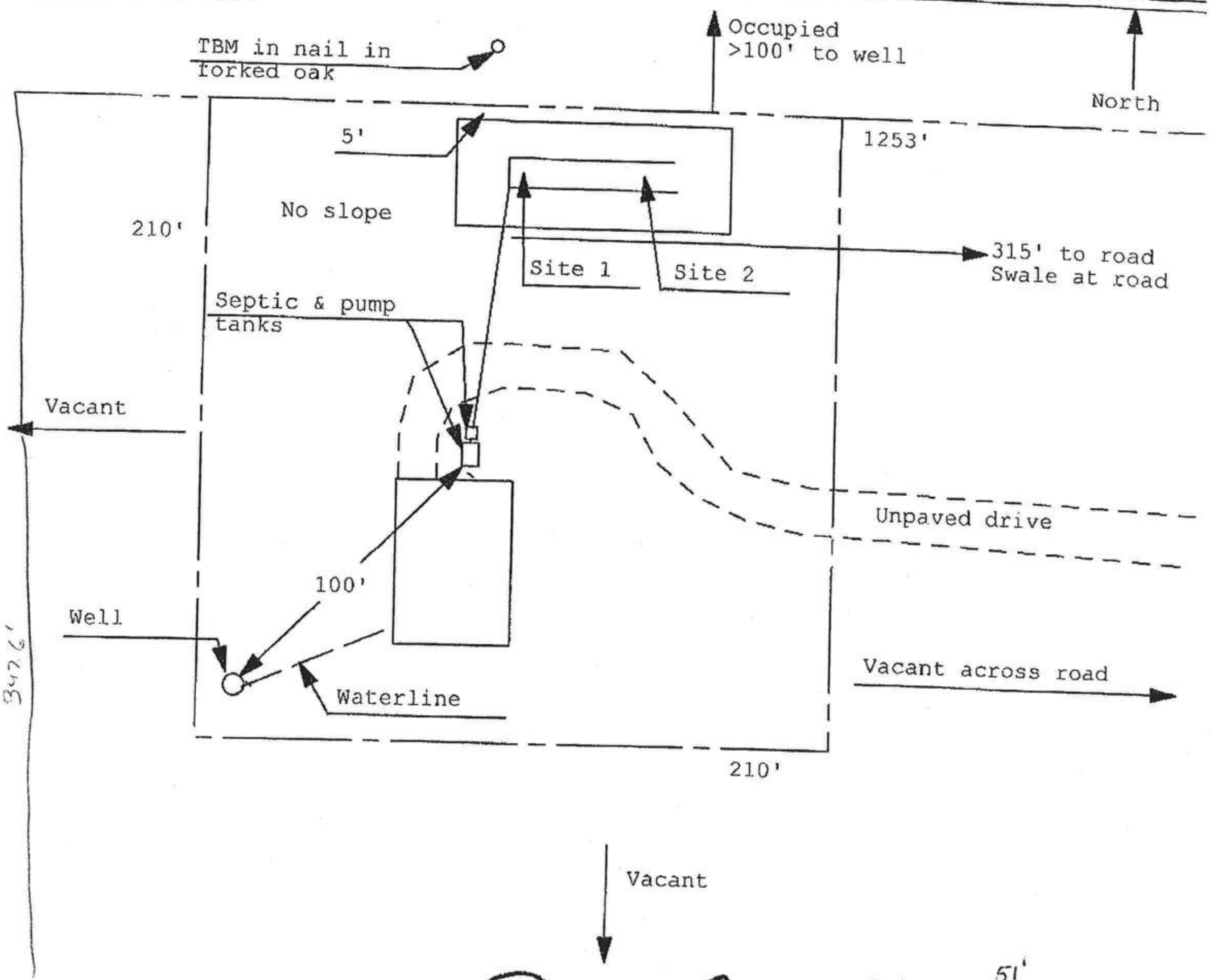
0801-91

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 08-0062

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

RAINES/CR 06-3950



1 inch = ~~50~~ 51' feet

Site Plan Submitted By Paul Lloyd Date 4/16/07

Plan Approved ✓ Not Approved ____ Date 1-15-08

By [signature] Columbia CPHU

Notes:

LYNCH WELL DRILLING, INC.

173 SW Tustenuggee Ave
Lake City, FL. 32025
Phone 386-752-6677
Fax 386-752-1477

Building Permit #_____ Owner's Name Raines

Well Depth_____ Ft. Casing Depth _____Ft. Water Level _____Ft.

Casing Size 4 inch Steel Pump Installation: Deep Well Submersible

Pump Make Schaefer Pump Model TI14Y18X10 HP 1

System Pressure (PSI) _____ On 30 Off 50 Average Pressure 50

Pumping System GPM at average pressure and pumping level 18 (GPM)

Tank Installation: (Bladder)/Galvanized Make Challenger
Model PC244 Size 81

Tank Draw-down per cycle at system pressure 25.1 gallons

I HEREBY VERTIFY THAT THIS WATER WELL SYSTEM HAS BEEN INSTALLED AS PER THE ABOVE INFORMATION.

Linda Newcomb
Signature

Linda Newcomb
Print Name

2609
License Number

1/15/08
Date

FORM 600A-2001

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**Raines Residence**	Builder:	Norton Home Imp.
Address:	Lot: A, Sub: Deer Hammock, Plat: (unrecorded)	Permitting Office:	Columbia Co.
City, State:	Lake City, FL 32055-	Permit Number:	26689
Owner:	C. Raines	Jurisdiction Number:	121000
Climate Zone:	North		

1. New construction or existing		New	
2. Single family or multi-family		Single family	
3. Number of units, if multi-family		1	
4. Number of Bedrooms		3	
5. Is this a worst case?		No	
6. Conditioned floor area (ft²)		1969 ft²	
7. Glass area & type	Single Pane	Double Pane	
a. Clear glass, default U-factor	0.0 ft²	131.0 ft²	
b. Default tint	0.0 ft²	0.0 ft²	
c. Labeled U or SHGC	0.0 ft²	0.0 ft²	
8. Floor types			
a. Slab-On-Grade Edge Insulation		R=0.0, 168.0(p) ft	
b. N/A			
c. N/A			
9. Wall types			
a. Frame, Wood, Exterior		R=13.0, 1339.0 ft²	
b. Frame, Wood, Adjacent		R=13.0, 874.0 ft²	
c. N/A			
d. N/A			
e. N/A			
10. Ceiling types			
a. Under Attic		R=30.0, 1416.0 ft²	
b. Under Attic		R=30.0, 553.0 ft²	
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Garage		Sup. R=6.0, 15.0 ft	
b. N/A			

12. Cooling systems		
a. Central Unit	Cap: 35.0 kBtu/hr	
	SEER: 14.00	
b. N/A		
c. N/A		
13. Heating systems		
a. Electric Heat Pump	Cap: 35.0 kBtu/hr	
	HSPF: 7.90	
b. N/A		
c. N/A		
14. Hot water systems		
a. Electric Resistance	Cap: 30.0 gallons	
	EF: 0.90	
b. N/A		
c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	PT, CF,	

Glass/Floor Area: 0.07

Total as-built points: 22786

Total base points: 30924

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Tim Delbene

DATE: 1/23/08

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: A, Sub: Deer Hammock, Plat: (unrecorded), Lake City, FL, 32055** PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points
.18	1969.0	20.04	7102.6	Double, Clear	N	2.0	5.0	6.0	19.20	0.87	100.3
				Double, Clear	N	2.0	5.0	9.0	19.20	0.87	150.5
				Double, Clear	N	2.0	7.0	15.0	19.20	0.92	265.6
				Double, Clear	S	2.0	5.0	12.0	35.87	0.72	311.4
				Double, Clear	E	10.0	7.0	60.0	42.06	0.44	1114.4
				Double, Clear	W	2.0	5.0	9.0	38.52	0.80	277.1
				Double, Clear	W	10.0	9.0	20.0	38.52	0.50	388.1
				As-Built Total:				**131.0**			**2607.5**

WALL TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM	= Points
Adjacent	874.0	0.70	611.8	Frame, Wood, Exterior	13.0	1339.0	1.50	2008.5
Exterior	1339.0	1.70	2276.3	Frame, Wood, Adjacent	13.0	874.0	0.60	524.4
Base Total:	**2213.0**		**2888.1**	**As-Built Total:**		**2213.0**		**2532.9**

DOOR TYPES	Area X	BSPM	= Points	Type	Area X	SPM	= Points
Adjacent	21.0	2.40	50.4	Exterior Insulated	21.0	4.10	86.1
Exterior	21.0	6.10	128.1	Adjacent Insulated	21.0	1.60	33.6
Base Total:	**42.0**		**178.5**	**As-Built Total:**	**42.0**		**119.7**

CEILING TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM X SCM	= Points
Under Attic	1969.0	1.73	3406.4	Under Attic	30.0	1416.0	1.73 X 1.00	2449.7
				Under Attic	30.0	553.0	1.73 X 1.00	956.7
Base Total:	**1969.0**		**3406.4**	**As-Built Total:**		**1969.0**		**3406.4**

FLOOR TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM	= Points
Slab	168.0(p)	-37.0	-6216.0	Slab-On-Grade Edge Insulation	0.0	168.0(p	-41.20	-6921.6
Raised	0.0	0.00	0.0					
Base Total:			**-6216.0**	**As-Built Total:**		**168.0**		**-6921.6**

INFILTRATION	Area X	BSPM	= Points	Area X	SPM	= Points
	1969.0	10.21	20103.5	1969.0	10.21	20103.5

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: A, Sub: Deer Hammock, Plat: (unrecorded), Lake City, FL, 32055** PERMIT #:

BASE			AS-BUILT						
Summer Base Points:		**27463.0**	**Summer As-Built Points:**						**21848.4**
Total Summer Points	X System Multiplier	= Cooling Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points
27463.0	**0.4266**	**11715.7**	21848.4 **21848.4**	1.000 **1.00**	(1.090 x 1.147 x 1.00) **1.250**	0.244 **0.244**	0.902 **0.902**	6009.9 **6009.9**

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: A, Sub: Deer Hammock, Plat: (unrecorded), Lake City, FL, 32055** PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points
.18	1969.0	12.74	4515.3	Double, Clear	N	2.0	5.0	6.0	24.58	1.01	148.4
				Double, Clear	N	2.0	5.0	9.0	24.58	1.01	222.7
				Double, Clear	N	2.0	7.0	15.0	24.58	1.00	369.9
				Double, Clear	S	2.0	5.0	12.0	13.30	1.40	223.4
				Double, Clear	E	10.0	7.0	60.0	18.79	1.38	1553.6
				Double, Clear	W	2.0	5.0	9.0	20.73	1.06	197.6
				Double, Clear	W	10.0	9.0	20.0	20.73	1.18	488.7
				As-Built Total:				**131.0**			**3204.4**
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points
Adjacent	874.0	3.60	3146.4	Frame, Wood, Exterior			13.0	1339.0	3.40		4552.6
Exterior	1339.0	3.70	4954.3	Frame, Wood, Adjacent			13.0	874.0	3.30		2884.2
Base Total:	**2213.0**		**8100.7**	**As-Built Total:**				**2213.0**			**7436.8**
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =		Points
Adjacent	21.0	11.50	241.5	Exterior Insulated				21.0	8.40		176.4
Exterior	21.0	12.30	258.3	Adjacent Insulated				21.0	8.00		168.0
Base Total:	**42.0**		**499.8**	**As-Built Total:**				**42.0**			**344.4**
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X WCM =		Points
Under Attic	1969.0	2.05	4036.5	Under Attic			30.0	1416.0	2.05 X 1.00		2902.8
				Under Attic			30.0	553.0	2.05 X 1.00		1133.7
Base Total:	**1969.0**		**4036.5**	**As-Built Total:**				**1969.0**			**4036.5**
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points
Slab	168.0(p)	8.9	1495.2	Slab-On-Grade Edge Insulation			0.0	168.0(p	18.80		3158.4
Raised	0.0	0.00	0.0								
Base Total:			**1495.2**	**As-Built Total:**				**168.0**			**3158.4**
INFILTRATION	Area X	BWPM =	Points					Area X	WPM =		Points
	1969.0	-0.59	-1161.7					1969.0	-0.59		-1161.7

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: A, Sub: Deer Hammock, Plat: (unrecorded), Lake City, FL, 32055** PERMIT #:

BASE			AS-BUILT					
Winter Base Points:		**17485.8**	**Winter As-Built Points:**					**17018.7**
Total Winter Points	X System Multiplier	= Heating Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
17485.8	**0.6274**	**10970.6**	17018.7 **17018.7**	1.000 **1.00**	(1.069 x 1.169 x 1.00) **1.250**	0.432 **0.432**	0.950 **0.950**	8721.1 **8721.1**

FORM 600A-2001

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: A, Sub: Deer Hammock, Plat: (unrecorded), Lake City, FL, 32055** PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X	Tank Ratio	X Multiplier	X Credit Multiplier	= Total
3		2746.00	8238.0	30.0	0.90	3		1.00	2684.98	1.00	8054.9
				As-Built Total:							**8054.9**

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
11716	**10971**	**8238**	**30924**	**6010**	**8721**	**8055**	**22786**

PASS

FORM 600A-2001

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: A, Sub: Deer Hammock, Plat: (unrecorded), Lake City, FL, 32055** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	✓
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	✓
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	✓
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	✓
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	✓
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	✓
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	✓

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	✓
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	N/A
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	✓
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	✓
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	✓
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	✓

permit # 26689

doc. 18.50
cert. copy 3.50

2)

THIS INSTRUMENT WAS PREPARED BY:
FIRST FEDERAL SAVINGS BANK OF FLORIDA
4705 WEST U.S. HIGHWAY 90
P.O. BOX 2029
LAKE CITY, FLORIDA 32056

PERMIT NO._____ TAX FOLIO NO. 24-4S-17-08720-101

NOTICE OF COMMENCEMENT

STATE OF FLORIDA
COUNTY OF COLUMBIA

Inst:200812002037 Date:2/1/2008 Time 9 53 AM
DC,P DeWitt Cason,Columbia County Page 1 of 2

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

1. Description of property: AS DESCRIBED ON EXHIBIT "A" ATTACHED HERETO
2. General description of improvement: **Construction of Dwelling**
3. Owner information:
   a. Name and address: CHRISTIPHER A. RAINES and his wife, CHARNA E. RAINES, 156 SE Peyton Loop, Apt. 105, Lake City, FL 32025
   b. Interest in property: **Fee Simple**
   c. Name and address of fee simple title holder (if other than Owner): **NONE**
4. Contractor (name and address): NORTON HOME IMPROVEMENT COMPANY, INC., 3367 South US Highway 441, Suite 101, Lake City, FL 32025
5. Surety:
   a. Name and address: _____
   b. Amount of bond: _____
6. Lender: **FIRST FEDERAL SAVINGS BANK OF FLORIDA**
   4705 WEST U.S. HIGHWAY 90
   P. O. BOX 2029
   LAKE CITY, FLORIDA 32056
7. Persons within the State of Florida designated by Owner upon whom notices or other document may be served as provided by Section 713.13 (1) (a) 7., Florida Statutes: NONE
8. In addition to himself, Owner designates **PAULA HACKER of FIRST FEDERAL SAVINGS BANK OF FLORIDA, 4705 West U.S. Highway 90 / P. O. Box 2029, Lake City, Florida 32056** to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) (b), Florida Statutes.
9. Expiration date of notice of commencement (the expiration date is 1 year from the date of recording unless a different date is specified).

Christopher A. Raines
Borrower Name Christopher A. Raines

Charna E. Raines
Co-Borrower Name Charna E. Raines

The foregoing instrument was acknowledged before me this 31st day of January 2008, by CHRISTOPHER A. RAINES & his wife, CHARNA E. RAINES, who is personally known to me or who has produced driver's license for identification.

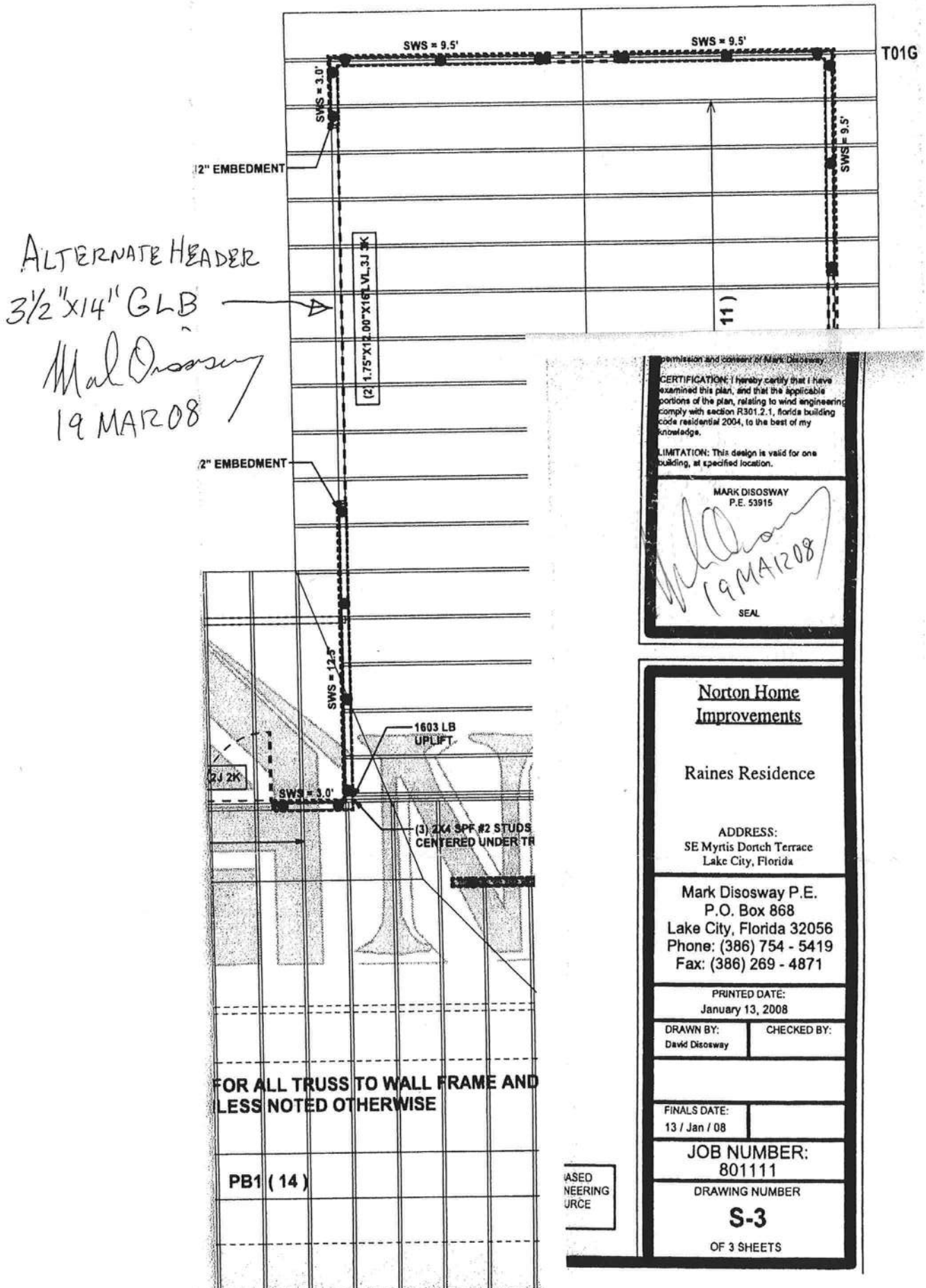
Notary Public
My Commission Expires:

TERRY MCDAVID
MY COMMISSION # DD 600788
EXPIRES: January 16, 2010
Bonded Thru Notary Public Underwriters

STATE OF FLORIDA, COUNTY OF COLUMBIA
I HEREBY CERTIFY, that the above and foregoing is a true copy of the original filed in this office.
P. DeWITT CASON, CLERK OF COURTS
By Dawn Tragle
Deputy Clerk
Date 02-01-2008

CLERK CIRCUIT COURT
COLUMBIA COUNTY, FLORIDA

Permit # 26689



26689

CAL-TECH TESTING, INC.
ENGINEERING & TESTING
LABORATORY
P.O. Box 1625 • Lake City, FL 32056 • (386) 755-3633 • Fax (386) 752-5456

2230 Greensboro Hwy.
Quincy, FL 32351
(850) 442-3495 • Fax (850) 442-4008
4784 Rosselle St.
Jacksonville, FL 32254
(904) 381-8901 • Fax (904) 381-8902

REPORT OF DAILY CONSTRUCTION TESTING AND MONITORING

Client Norton Home Improvement
Project Raines Res.
Contractor SAC

Date 2-20-08
Job. No 08-125
Technician P. Grijo

WORK ORDER:

☑ DENSITY
Spec's: 95%
Test No.: 1-4
Inches: 12"

☐ CONCRETE
☐ Cylinders
☐ Beams
☐ Prisms
☐ Pick-Up
Set No. _____

☑ Pick-Up Proctor 1 M/P 10778

☐ Pick-Up LBR _____

DESCRIPTION OF DAYS ACTIVITIES:

Conducted density test in footer area of project. Collected soil sample for proctr. Transported soil sample to Lake City lab for processing. Results of density test are pending proctr outcome.

Time Out: 10:15 Am
Time In: 11:30 Am

FDT's Performed 4
Cyls Cast/Cal-Tech _____
Cyls Cast/Client _____
Beams Cast/Cal-Tech: _____

Weather: _____
Hours Worked: .50
Other Tests: _____

Hours Travel: .75
Miles Travel: _____
Hours Standby: _____
Hours O.T.: _____

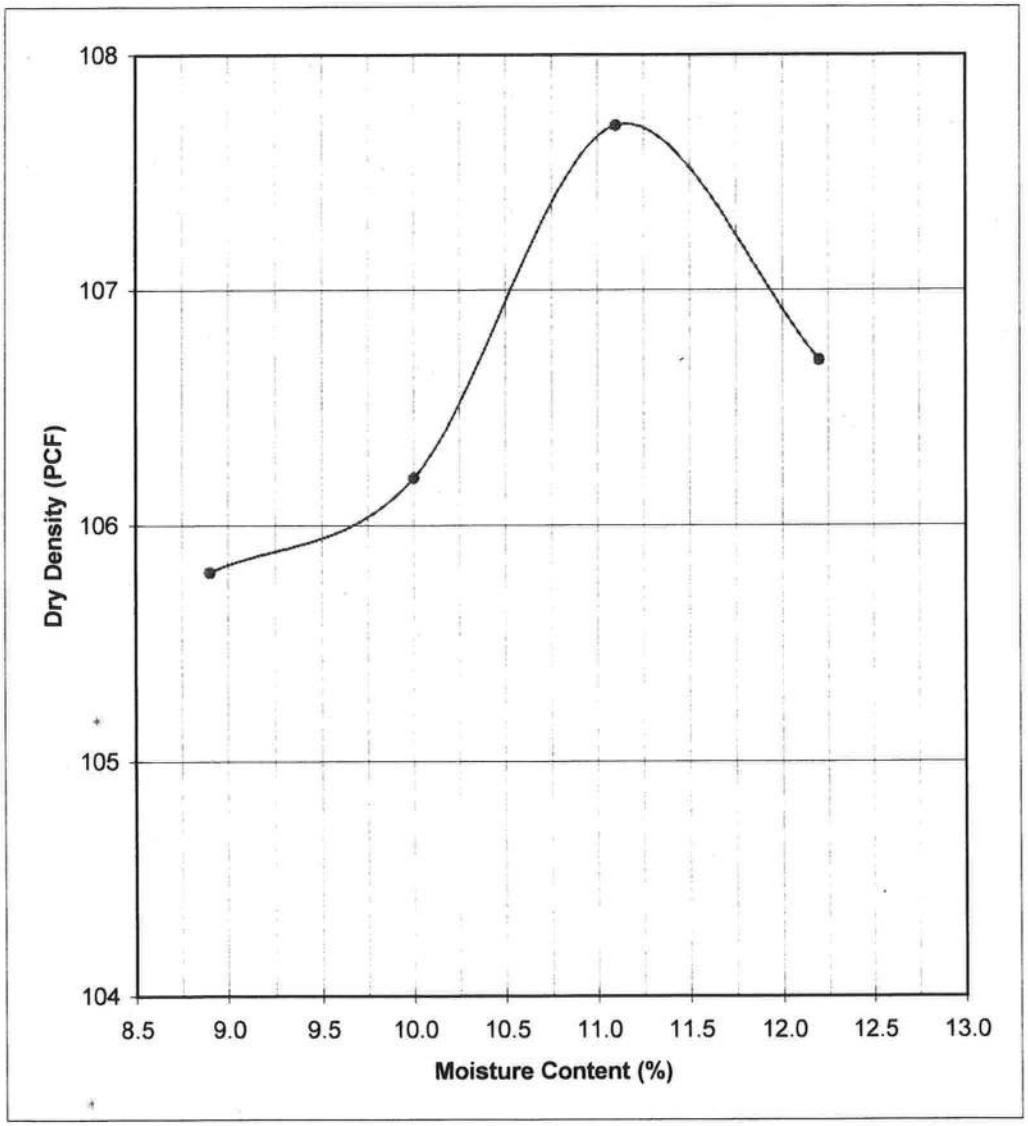
Cal-Tech Testing, Inc.

• **Engineering** P.O. Box 1625 • Lake City, FL 32056-1625 • Tel(386)755-3633 • Fax(386)752-5456
• **Geotechnical** 4784 Rosselle St., Jacksonville, FL 32254 • Tel(904)381-8901 • Fax(904)381-8902
• **Environmental** 2230 Greensboro Hwy • Quincy, FL 32351 • Tel(850)442-3495 • Fax(850)442-4008
Laboratories

REPORT OF LABORATORY COMPACTION TEST

Client: Norton's Home Improvements, 3367 US Hwy 441, Suite 101, Lake City, FL 32025
Project Name: Raines Residence
Project Location: Lake City, FL
Contractor: Norton's Home Improvements

File No: 08-00125-01
Date: 2/21/2008
Lab No: 10778



PROCTOR DATA

Proctor No.: 1

Modified Proctor (ASTM D-1557) ☑

Standard Proctor (ASTM D-698) ☐

Maximum Dry Dens. Pcf: 107.7

Optimum Moisture Percent: 11.1

The test results presented in this report are specific only to the samples tested at the time of testing. The tests were performed in accordance with generally accepted methods and standards. Since material conditions can vary between test locations and change with time, sound judgement should be exercised with regard to the use and interpretation of the data.

Sample Description: Light Brown Sand
Sample Location: Existing Material
Proposed Use: Footing
Sampled By: Pam Geiger **Date:** 2/20/2008
Tested By: Tim Cassidy **Date:** 2/21/2008
Remarks: 1cc: Client
1cc: File

Linda Creamer, CEO, DBE
Linda M. Creamer
President - CEO
Reviewed By:
Date: 2/21/08
Licensed, Florida No.: 57842

EE

Cal-Tech Testing, Inc.

• Engineering
• Geotechnical
• Environmental
Laboratories

P.O. Box 1625 • Lake City, FL 32056-1625 • Tel(386)755-3633 • Fax(386)752-5456
4784 Rosselle St., Jacksonville, FL 32254 • Tel(904)381-8901 • Fax(904)381-8902
2230 Greensboro Hwy • Quincy, FL 32351 • Tel(850)442-3495 • Fax(850)442-4008

REPORT OF IN-PLACE DENSITY TEST

JOB NO.: 08-00125-01
DATE TESTED: 2/20/08
DATE REPORTED: 2/21/08

PROJECT:	Raines Residence, Lake City, FL
CLIENT:	Norton's Home Improvements, 3367 US Hwy 441, Suite 101, Lake City, FL 32025
GENERAL CONTRACTOR:	Norton's Home Improvements
EARTHWORK CONTRACTOR:	Norton's Home Improvements
INSPECTOR:	Pam Geiger

ASTM METHOD	**SOIL USE**
(D-2922) Nuclear	OTHER

SPECIFICATION REQUIREMENTS: 95% Footing

TEST NO.		TEST LOCATION	TEST DEPTH	WET DENSITY (lb/ft^3)	MOISTURE PERCENT	DRY DENSITY (lb/ft^3)	PROCTOR TEST NO.	PROCTOR VALUE	% MAXIMUM DENSITY
Footing									
1		20' North of SE Corner	12"	116.4	11.0	104.9	1	107.7	97%
2		25' West of NE Corner	12"	115.8	12.4	103.0	1	107.7	96%
3		15' South of NW Corner	12"	112.8	10.6	102.0	1	107.7	95%
4		25' East of SW Corner	12"	114.2	12.1	101.9	1	107.7	95%

REMARKS: The Above Tests Meet Specification Requirements.

PROCTORS

PROCTOR NO.		SOIL DESCRIPTION	MAXIMUM DRY UNIT WEIGHT (lb/ft^3)	OPT. MOIST.	TYPE
1		Light Brown Sand	107.7	11.1	MODIFIED (ASTM D-1557)

Respectfully Submitted,
CAL-TECH TESTING, INC.

Linda Creamer, CEO, DBE

Linda M. Creamer
President - CEO

Reviewed By:

Date: 2/21/08
Licensed, Florida No: 57842

ee

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 24-4S-17-08720-101 Building permit No. 000026689

Use Classification SFD,UTILITY Fire: 24.42

Permit Holder JAMES NORTON Waste: 33.50

Owner of Building CHRIS & CHARNA RAINES Total: 57.92

Location: 272 SE MYRTIS DORTCH TERR.,LAKE CITY, FL

Date: 08/01/2008

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

PERFORMANCE TESTING INC.
4076 - 148th Ave. N.E. • Redmond, Washington 98052-5165 • (425) 883-9788 (425) 869-5266

SPECIFICATION CONFORMANCE TEST REPORT

PERFORMANCE FILE: 00-426 **DATE OF REPORT:** 4/26/00

DATE TEST STARTED: 3/31/00 **TEST COMPLETED:** 4/26/00

CLIENT: Jordan Company
4661 Burbank Rd.
Memphis, TN 38118

SERIES: 88 Series Sliding Glass Door

TEST STANDARDS: AAMA/NWWDA 101/I.S.2-97 Voluntary Specification for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.

CMBSO/CAWM 301-90, Forced Entry Resistance Test for Windows

SPECIFICATION: SGD-R40 71 1/2" wide by 79 1/2" high
Type I Door

DESCRIPTION OF SAMPLE TESTED

CONFIGURATION & SIZE: XO measuring 71 1/2" wide by 79 1/2" high.

PANEL/FIXED LITE: One interior sliding panel measuring 36 3/4" wide by 77 1/4" high. The fixed lite measured 33 1/4" wide by 73 13/16" high.

GLASS & GLAZING: All glazing measured 1" overall using two panes of double strength tempered glass. The glass was set on blocks, placed against double sided adhesive foam glazing tape and retained on the exterior with vinyl snap-in glazing beads.

WEATHER-STRIPPING: Wool pile with center fin measuring 0.230" was applied to the exterior perimeter of the sliding panel and to the interior face of the fixed interlock.

DESCRIPTION

Continued

HARDWARE: A handle assembly was double screw connected using #8 x 2 1/8" screws to the sliding panel lead stile. The handle engaged a cam lock double screw connected to the lead stile with #6 x 1/2" screws. The cam lock engaged a keeper screw connected to the frame jamb using two #10 x 1" screws and two #10 x 1/2" screws. The panel bottom rail contained tandem adjustable rollers fitted into milled slots and single screw connected using a #14 x 3/4" screw at each end. The rollers operated on a stainless steel insert located on the snap-in sill track.

WATER DRAINAGE: The snap-in sill track was cut short 1/4" at each end. The sliding panel sill pocket drained through a 1/2" x 3/16" slot approximately 2" from each corner. Water then drained to the exterior through a 1/8" x 3/16" slot approximately 1 1/2" from the exterior corners. The filler bar below the fixed lite allowed water to pass through a 3/16" x 1/8" slot at each end. The sliding panel bottom rail glazing pocket drained through a 3/8" x 1/8" slot approximately 3" from the interior corners, then through the tooling for the rollers.

FRAME/PANEL: Vinyl extrusions with welded corners. The fixed interlock was fitted and double screw connected with #8 x 2 1/2" screws to the head and sill. Vinyl snap-in filler bars were fitted in the head and sill, above and below the fixed lite, and sealed to the frame jamb and fixed interlock.

REINFORCEMENT: Aluminum reinforcement was placed in the fixed and sliding interlocks. The aluminum reinforcement was held in the fixed interlock using two #6 x 5/8" screws and in the sliding interlock using three #6 x 5/8" screws. Steel reinforcement was used in the lead stile.

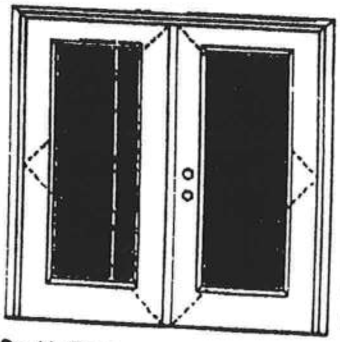
Detailed assembly drawings showing the wall thickness of all members, corner construction, and hardware have been compared to the test sample and are attached to this report. Cross sections of the sample will be retained by Performance Testing for a period of 4 years.

AAMA TEST RESULTS

Paragraph Number	Title of Test	Measured	Allowed
2.1.2	**Air Infiltration** per ASTM E283-91 CFM per square foot of door area @ 1.57 PSF	0.1	0.3
	Reported to the second decimal	0.10	
	Total CFM	3.8	
2.1.3	**Water Resistance** per ASTM E547-96 @ 2.86 PSF Leakage after 4 Cycles of 5 minutes With and without screens	None	None

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Double Door
Maximum unit size = 6'0" x 6'8"

Design Pressure
+40.5/-40.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0002-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0002-02.

APPROVED DOOR STYLES:

1/4 GLASS:


100 Series


133, 135 Series


136 Series


680 Series


822 Series

1/2 GLASS:


105 Series*


106, 160 Series*


120 Series*


200 Series*


12 R/L, 23 R/L, 24 R/L Series*


107 Series*


108 Series

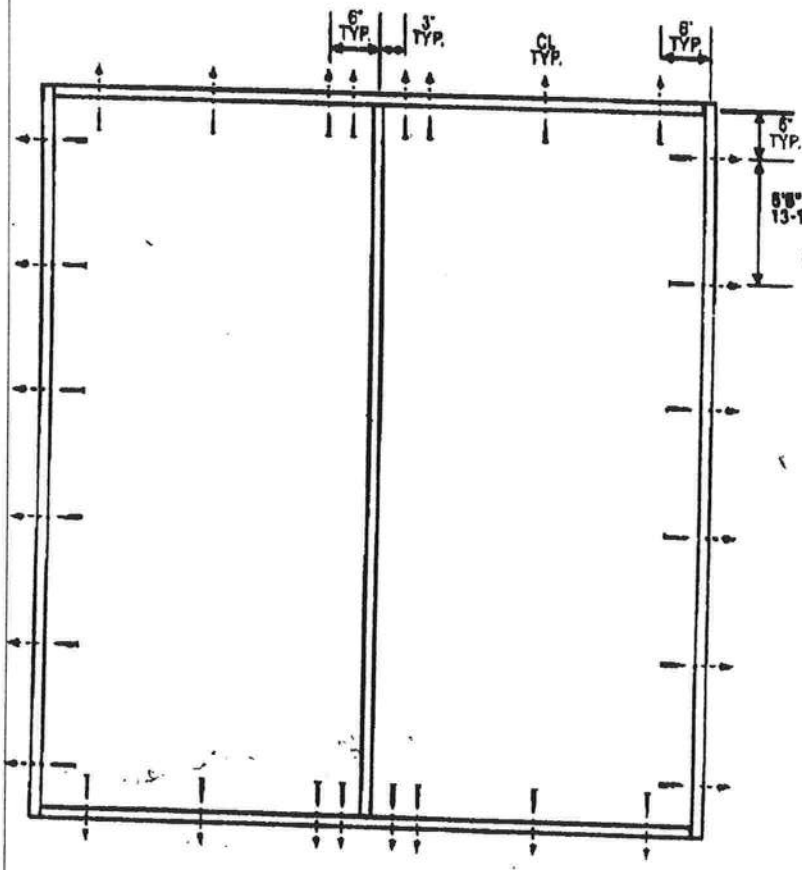

304 Series

*This glass kit may also be used in the following door styles: 6-panel; 6-panel with scroll; Eyebrow 6-panel; Eyebrow 6-panel with scroll.

XX
Unit

MID-WL-MA0002-02

DOUBLE DOOR



Minimum Fastener Count

- 6 per vertical framing member
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Latching Hardware:

- Compliance requires that GRADE 2 or better (ANSI/BHMA A156.2) cylinderical and deadlock hardware be installed.

Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons.
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dad's Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

XX
Glazed Inswing Unit

COP-WI-JH4142-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:


404 Series


410 Series


450 Series

FULL GLASS:


100 Series


114, 120, 122 Series


152 Series


160 Series


300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthy

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Johnson Entry Systems

March 29, 2002

PREMDOR Collection Premium Quality Doors

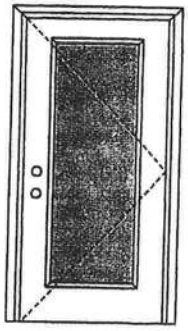
Exclusively from Masonite
Masonite International Corporation

X
Glazed Inswing Unit

COP-WL-JH4141-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+40.5/-40.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02 and MAD-WL-MA0041-02.

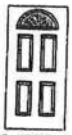
MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:

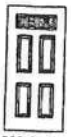
1/4 GLASS:


160 Series


133, 135 Series


136 Series


680 Series


822 Series

1/2 GLASS:


105 Series*


106, 160 Series*


129 Series*


200 Series*


12 R/L, 23 R/L, 24 R/L Series*


107 Series*


108 Series


304 Series

*This glass kit may also be used in the following door styles: 5-panel; 5-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.

Johnson™ EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors
Exclusively from Masonite®
Masonite International Corporation

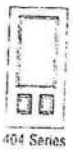
X
Glazed Inswing Unit

COP-WL-JH4141-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:


404 Series


410 Series


450 Series

FULL GLASS:


109 Series


114, 120, 122 Series


152 Series


149 Series


300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey WH

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

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June 17, 2002
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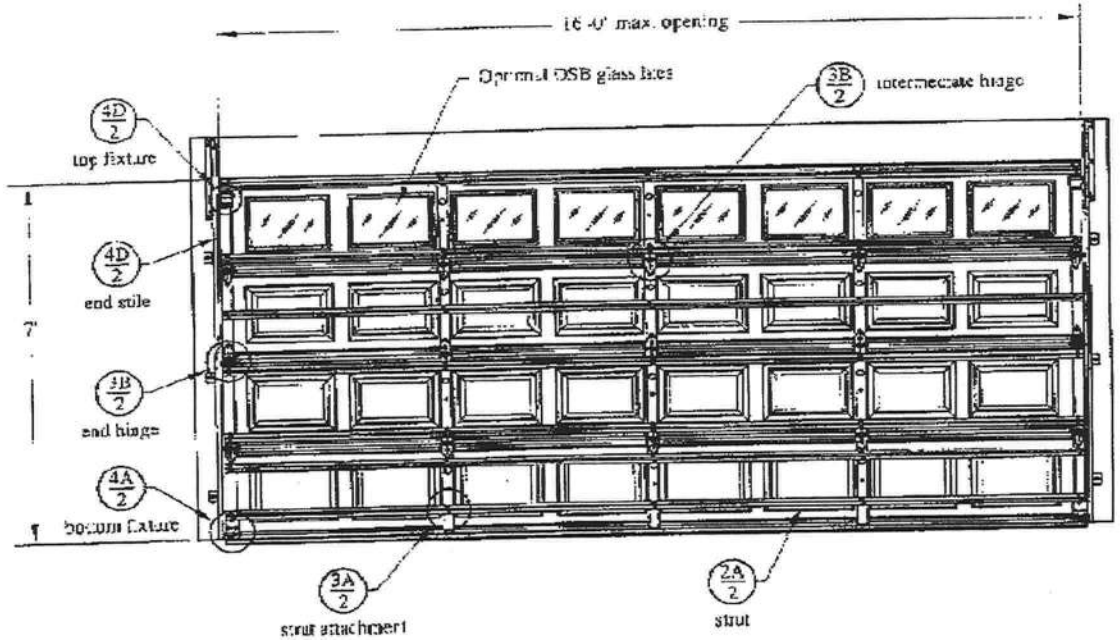
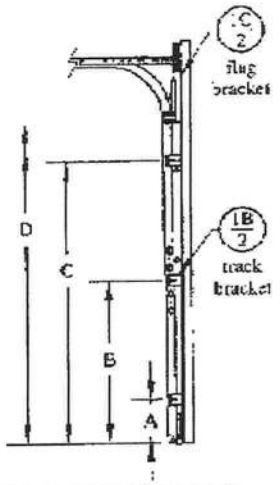
Masonite International Corporation



Door Model	Gauge	Deflection
2250/2251	25	.0185
4250/4251	25	.0185
2240/2241	24	.0225
4240/4241	24	.0225
5240/5241	24	.0225

door height	section quantity	strut quantity	trk brkt per side
5'-6" to 7'-0"	4	7	3
7'-6" to 8'-0"	5	8	4
8'-3" to 8'-6"	5	9	4
8'-9" to 10'-6"	6	11	5
10'-9" to 12'-0"	7	13	6
12'-6" to 14'-0"	8	15	7

Refer to Supplemental Instructions for strut placement on doors over 7'-0" high



Track Bracket Chart		door height								
		6'-6"	6'-9"	7'-0"	7'-6"	7'-9"	8'-0"	8'-3"	8'-6"	8'-9"
track brackets	D	n/a	n/a	n/a	22"	68"	72"	31"	84"	87"
	C	60"	63"	65"	38"	55"	58"	60"	63"	65"
	B	35"	35"	38"	34"	34"	34"	32"	35"	38"
	A	10"	7"	10"	10"	7"	10"	4"	7"	10"

Track bracket locations shown above are for doors up to five sections high. Additional door sections may be added for a maximum door height of 14'-0". One track bracket (per track) must be added for each section and spaced at a distance not greater than the corresponding section height.

This door has been tested in accordance with ANSI/DASMA 108-2002
Design Pressure (DP): 18.4 psf / 20.7 neg
Test Pressure (TP): 27.8 psf / 31.1 neg

For 2004 FBC Table 1609.6E, DP meets or exceeds basic wind speed of:
V = 130 MPH for Exposure B and mean roof height of 30' or less
V = 93 MPH for Exposure C and mean roof height of 30' or less

Maximum door size: 16'-0" wide by 14'-0" tall

Glazing and door have not been tested for windborne debris.

Wood buck and supporting structural elements shall be designed by a registered professional engineer for wind loads shown on this drawing.

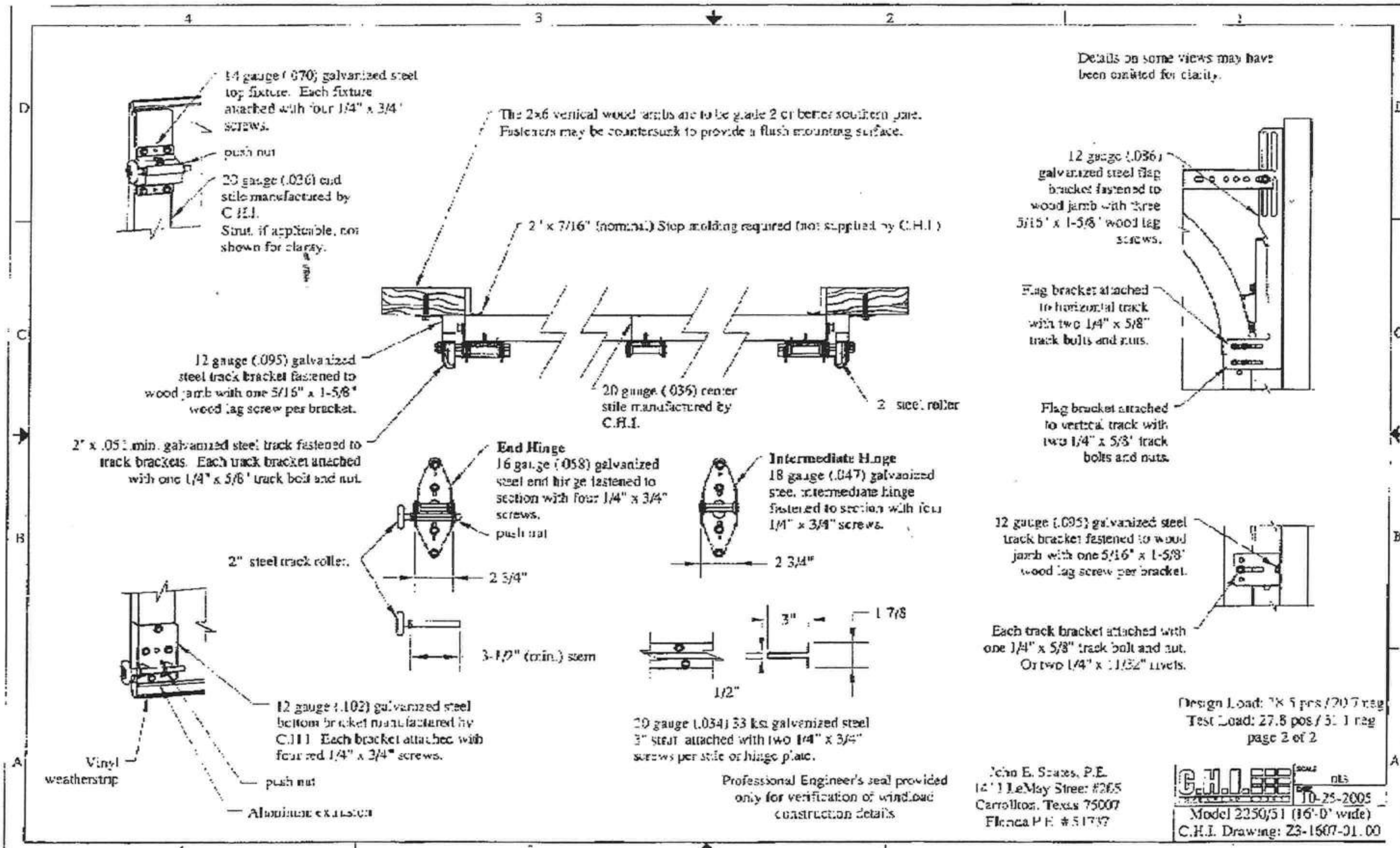
If door is not electrically operated, a lock must be installed

Professional Engineer's seal provided only for verification of wind load construction details

John E. Seates, P.E.
141 LeMay Street #205
Carrollton, Texas 75007
Florida P.E. # 51737

FL 5514

nts
10-25-2005
Model 2250/51 (16'-0" wide)
C.H.I. Drawing: Z3-1607-05100


Details on some views may have been omitted for clarity.
14 gauge (.070) galvanized steel top fixture. Each fixture attached with four 1/4" x 3/4" screws.
push nut
20 gauge (.036) end stile manufactured by C.H.I.
Strut, if applicable, not shown for clarity.
The 2x6 vertical wood jambs are to be grade 2 or better southern pine. Fasteners may be countersunk to provide a flush mounting surface.
2" x 7/16" (nominal) Stop molding required (not supplied by C.H.I.)
12 gauge (.036) galvanized steel flag bracket fastened to wood jamb with three 5/16" x 1-5/8" wood lag screws.
Flag bracket attached to horizontal track with two 1/4" x 5/8" track bolts and nuts.
12 gauge (.095) galvanized steel track bracket fastened to wood jamb with one 5/16" x 1-5/8" wood lag screw per bracket.
20 gauge (.036) center stile manufactured by C.H.I.
2" steel roller
Flag bracket attached to vertical track with two 1/4" x 5/8" track bolts and nuts.
2" x .051 min. galvanized steel track fastened to track brackets. Each track bracket attached with one 1/4" x 5/8" track bolt and nut.
End Hinge
16 gauge (.058) galvanized steel end hinge fastened to section with four 1/4" x 3/4" screws.
push nut
Intermediate Hinge
18 gauge (.047) galvanized steel intermediate hinge fastened to section with four 1/4" x 3/4" screws.
12 gauge (.095) galvanized steel track bracket fastened to wood jamb with one 5/16" x 1-5/8" wood lag screw per bracket.
2" steel track roller.
2 3/4"
2 3/4"
3-1/2" (min.) stem
3"
1 7/8
1/2"
Each track bracket attached with one 1/4" x 5/8" track bolt and nut. Or two 1/4" x 11/32" rivets.
12 gauge (.102) galvanized steel bottom bracket manufactured by C.H.I. Each bracket attached with four red 1/4" x 3/4" screws.
20 gauge (.034) 33 ksi galvanized steel 3" strut attached with two 1/4" x 3/4" screws per stile or hinge plate.
Design Load: 28.5 pos / 20.7 neg
Test Load: 27.8 pos / 31.1 neg
page 2 of 2
Vinyl weatherstrip
push nut
Aluminum extrusion
Professional Engineer's seal provided only for verification of windload construction details.
John E. Scates, P.E.
1411 LeMay Street #205
Carrollton, Texas 75007
Florida P.E. #51737
C.H.I.
nts
10-25-2005
Model 2250/51 (16'-0" wide)
C.H.I. Drawing: Z3-1607-01.00

AAMA/WDMA 101/I.S. 2-97
TEST REPORT

Rendered to:

JORDAN COMPANIES

SERIES/MODEL: Series 8900
TYPE: PVC Fixed Window

Title of Test	Results
AAMA Rating	F-C50 60 x 78
Uniform Load Deflection Test Pressure	±50.0 psf
Air Infiltration	<0.01 cfm/ft^2
Water Resistance Test Pressure	7.5 psf
Uniform Load Structural Test Pressure	±75.0 psf
Corner Weld Test	Pass
Forced Entry Resistance	Grade 40

Reference should be made to full report for test specimen description and data.

Report No: 02-46046.01
Report Date: 07/23/03
Expiration Date: 07/17/07

Architectural Testing

AAMA/WDMA 101/I.S. 2-97 TEST REPORT

Rendered to:

JORDAN COMPANIES
4661 Burbank Road, P.O. Box 18377
Memphis, Tennessee 38118

Report No:	02-46046.01
Test Date:	07/17/03
Report Date:	07/23/03
Expiration Date:	07/17/07

Project Summary: Architectural Testing, Inc. (ATI) was contracted by Jordan Companies, to perform testing on Series 8900 PVC Fixed window. The sample tested successfully met the performance requirements for a F-C50 60 x 78 rating. Test specimen description and results are reported herein.

Test Procedure: The test specimens were evaluated in accordance with AAMA/WDMA 101/I.S. 2-97, *"Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors."*

Test Specimen Description:

Series/Model: Series 8900

Type: PVC Fixed Window

Overall Size: 4' 11-3/4" wide by 6' 5-3/4" high

Area: 32.3 ft²

Finish: All vinyl was white.

Glazing Details: The window utilized a nominal 3/4" thick insulating glass unit fabricated from two nominal double strength sheets of annealed glass separated by a desiccant filled metal spacer system. The glass was set from the interior against a silicone sealant backbedding. PVC glazing stops were utilized on the interior.

Frame Construction: The frame corners were miter cut and welded.

Installation: The window was installed within a nominal 2" by 8" SPF wood test buck. The window was anchored to the buck with #8 by 1-5/8" wood screws spaced 6" from each corner and 8" to 10" on center. Silicone sealant was used to seal the window to the test buck.

849 Western Avenue North
Saint Paul, MN 55117-5245
phone: 651.636.3835
fax: 651.636.3843
www.archtest.com

Test Results: The results are tabulated as follows:

Paragraph	Title of Test – Test Method	Results	Allowed
2.1.2	Air Infiltration per ASTM E 283-91 (See Note #1)		
	@ 1.57 psf (25 mph)	<0.01 cfm/ft^2	0.30 cfm/ft^2 max.
	@ 6.24 psf (50 mph)	<0.01 cfm/ft^2	--

Note #1: The tested specimen meets (or exceeds) the performance levels specified in AAMA/WDMA 101/I.S. 2-97 for air infiltration.

2.1.3	Water Resistance per ASTM E 547-00 (See Note #2)		
2.1.4.1	Uniform Load Deflection per ASTM E 330-97 (See Note #2)		
2.1.4.2	Uniform Load Structural per ASTM E 330-97 (See Note #2)		

Note #2: The client opted to start at a pressure higher than the minimum required. Those results are listed under "Optional Performance."

2.1.7	Welded Corner Test	Pass	<100% break on weld
2.1.8	Forced Entry Resistance per ASTM F 588-97		
	Type D		
	Grade 40		
	Lock Manipulation Test	No entry	No entry

Optional Performance:

4.3	Water Resistance per ASTM E 547-00 and 331-00		
	WTP = 7.5 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330-97 (See Note #3)		
	(Measurements reported were taken in between the anchor points)		
	(Loads were held for 60 seconds)		
	@ 50.0 psf (positive)	0.04"	No Damage
	@ 50.0 psf (negative)	0.03"	No Damage
4.4.2	Uniform Load Structural per ASTM E 330-97		
	(Measurements reported were taken in between the anchor points)		
	(Loads were held for 10 seconds)		
	@ 75.0 psf (positive)	<0.01"	0.16" max.
	@ 75.0 psf (negative)	<0.01"	0.16" max.

Note #3: The Uniform Load Deflection test is not an AAMA/WDMA 101/I.S. 2-97 requirement for this product designation. The data is recorded in this report for information only.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product which may only be granted by the certification program administrator. This report may not be reproduced, except in full, without the approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC.

Eric J. Schoenthaler
Technician

Daniel A. Johnson
Regional Manager

EJS/mb
02-46046.01

Architectural Testing

AAMA/WDMA 101/I.S. 2-97
TEST REPORT

Rendered to:

JORDAN COMPANIES

SERIES/MODEL: 8540
TYPE: PVC Casement Window

Title of Test	Results
AAMA/WDMA Rating	C-R40 (36 x 72)
Uniform Load Deflection Test Pressure	± 40.0 psf
Air Infiltration	0.08 cfm/ft^2
Water Resistance Test Pressure	7.5 psf
Uniform Load Structural Test Pressure	± 60.0 psf
Forced Entry Resistance	Pass Grade 10

Reference should be made to full report for test specimen description and data.

Report No: 02-48974.01
Report Date: 02/06/04
Expiration Date: 02/06/08

849 Western Avenue North
Saint Paul, Minnesota 55117
phone: 651.636.3835
fax: 651.636.3843
www.archtest.com

Architectural Testing

AAMA/WDMA 101/I.S.2-97 TEST REPORT

Rendered to:

JORDAN COMPANIES
P.O. Box 18377
Memphis, Tennessee 38118

Report No:	02-48974.01
Test Dates:	01/13/04
Thru:	02/06/04
Report Date:	02/12/04
Expiration Date:	02/06/08

Project Summary: Architectural Testing, Inc. (ATI) was contracted by Jordan Companies to perform tests on a Jordan Companies Series 8540 Casement Window. The sample tested successfully met the performance requirements for a C-R40 36 x 72 rating. Test specimen description and results are reported herein.

Test Procedure: The test specimen was evaluated in accordance with AAMA/NWDMA 101/I.S. 2-97, *"Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors."*

Test Specimen Description:

Series/Model: 8540

Type: PVC Casement Window

Overall Size: 3' 0" wide by 6' 0" high

Sash Size: 2' 10-1/4" wide by 5' 10-1/4" high

Finish: All PVC was white.

Glazing Type: The window utilized nominal 3/4" insulating glass comprised of two double-strength annealed sheets and a desiccant-filled metal spacer system. The glass was set from the exterior against a bed of silicone with PVC stops used on the exterior.

849 Western Avenue North
Saint Paul, Minnesota 55117
phone: 651.636.3635
fax: 651.636.3843
www.archtest.com

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.460" high pile with center fin	1 Row	Perimeter of sash exterior
Foam-filled vinyl bulb gasket	1 Row	Perimeter of sash interior
1/4" EPDM rubber bulb	1 Row	Perimeter of frame

Frame Construction: Frame corners were miter-cut and welded.

Sash Construction: Sash corners were miter-cut and welded.

Hardware:

Dual arm roto-operator	1	Sill
4-point lock with keepers on the sash	1	Locking jamb
Casement hinges	2	Top and bottom corner of sash on hinge side
Metal snubbers	2	24" from top and bottom on hinge side

Installation: The unit was installed into a grade 2 SPF 2" by 8" wood test buck and secured with 1-5/8" screws through the nail fin spaced 4" from corners and 8" on center. The nail fin was sealed to the buck with silicone.

Test Results:

The results are tabulated as follows.

Paragraph	Title of Test	Results	Allowed
2.1.2	Air Infiltration per ASTM E 283-01 (See Note #1)		
	@ 1.57 psf (25 mph)	0.08 cfm/ft^2	0.3 cfm/ft^2 max.
	@ 6.24 psf (50 mph)	0.13 cfm/ft^2	

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA101/I.S.2-97 for air infiltration.

Test Results: (Continued)

Paragraph	Title of Test	Results	Allowed
2.1.3	Water Resistance per ASTM 547-97 (See Note #2)		
2.1.4.1	Uniform Load Deflection per ASTM E 330-97 (See Note #2)		
2.1.4.2	Uniform Load Structural per ASTM E 330-97 (See Note #2)		

Note #2: The client opted to start at a pressure higher than the minimum required. Those results are listed under "Optional Performance."

2.2.5.6.1	Vertical Deflection Test @ 45lbs	0.09"	0.71"
2.2.5.6.2	Hardware Load Test @ 5lbs/ft²	No damage	No damage
2.1.7	Corner Weld Test	Meets as stated	Meets as stated
2.1.8	Forced Entry Resistance per ASTM F 588-97 Type B Grade 10 Lock Manipulation Test Tests B1 through B3 Lock Manipulation Test	 No entry No entry No entry	 No entry No entry No entry

Optional Performance:

4.3	Water Resistance per ASTM E 547-00 WTP = 7.5 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330-97 (See Note #3) (Measurements reported were taken on the top rail) (Loads were held for 60 seconds) @ 40.0 psf (positive) @ 40.0 psf (negative)	 0.10" 0.30"	 (See Note #3) (See Note #3)
4.4.2	Uniform Load Structural per ASTM E 330-97 (Measurements reported were taken on the top rail) (Loads were held for 10 seconds) @ 60.0 psf (positive) @ 60.0 psf (negative)	 0.01" 0.01"	 0.136" max. 0.136" max.

Note #3: The Uniform Load Deflection test is not a AAMA/NWWDA 101/I.S. 2-97 requirement for this product designation. The data is recorded in this report for information only.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced except in full without the approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC.

Digitally Signed by: Paul L. Spiess

Paul L. Spiess
Project Manager

Digitally Signed by: Daniel A. Johnson

Daniel A. Johnson
Regional Manager

PLS/jb
02-48974.01

DOCUMENT CONTROL ADDENDUM 02-48974.00

Current Issue Date: 02/12/04

Report No. 02-48974.01

Requested by: Darrel Booth, Jordan Companies
Purpose: AAMA/WDMA 101/I.S. 2-97 testing on a Jordan 8540 Casement
Issue Date: 02/12/04
Comments: Reports and drawings forwarded to ALI for AAMA certification.

TAMKO®
ROOFING PRODUCTS

January 31, 2002

TO: OUR FLORIDA CUSTOMERS:

Effective February 1, 2002, the following TAMKO shingles, as manufactured at TAMKO's Tuscaloosa, Alabama, facility, comply with ASTM D-3161, Type I modified to 110 mph. Testing was conducted using four nails per shingle. These shingles also comply with Florida Building Code TAS 100 for wind driven rain.

- Glass-Seal AR
- Elite Glass-Seal AR
- ASTM Heritage 30 AR (formerly ASTM Heritage 25 AR)
- Heritage 40 AR (formerly Heritage 30 AR)
- Heritage 50 AR (formerly Heritage 40 AR)

All testing was performed by Florida State certified independent labs.

Please direct all questions to TAMKO's Technical Services Department at 1-800-641-4691.

TAMKO Roofing Products, Inc.

2300 35th STREET P.O. BOX 2149 TUSCALOOSA

** LAMAR BOOZER **	PROJECT:	CUSTOM
900 EAST PUTNAM STREET	CLIENT:	NORTON
LAKE CITY, FL 32055	DATE:	1 12 08
RESIDENTIAL/LIGHT COMMERCIAL HVAC LOADS	DESIGNER:	LAMAR BOOZER

CLIENT INFORMATION:

NAME: NORTON
ADDRESS:
CITY, STATE: LAKE CITY, FLORIDA

TOTAL BUILDING LOADS:

BLDG. LOAD DESCRIPTIONS	AREA QUAN	SEN. LOSS	LAT. + GAIN	SEN. = GAIN	TOTAL GAIN
3-C WINDOW DBL PANE CLR GLS METL FR	183	5,975	0	12,057	12,057
9-I FRENCH DOOR DBL CLR GLS METL FR	40	1,357	0	2,896	2,896
12-D WALL R-11 +1/2"ASPHLT BRD(R-1.3)	1,117	4,023	0	2,197	2,197
11-C DOOR METAL POLYSTYRENE CORE	20	423	0	231	231
16-G CEILING R-30 INSULATION	1,325	1,967	0	1,967	1,967
22-A SLAB ON GRADE NO EDGE INSUL	159	5,796	0	0	0
SUBTOTALS FOR STRUCTURE:	2,844	19,541	0	19,348	19,348
PEOPLE	19	0	0	5,700	5,700
APPLIANCES	0	0	1,800	1,500	3,300
DUCTWORK	0	977	0	2,655	2,655
INFILTRATION W.CFM: 0.0 S.CFM: 0.0	0	0	0	0	0
VENTILATION W.CFM: 0.0 S.CFM: 0.0	0	0	0	0	0
SENSIBLE GAIN TOTAL				29,203	
TEMP. SWING MULTIPLIER				X 1.00	
BUILDING LOAD TOTALS		20,518	1,800	29,203	31,003

SUPPLY CFM AT 20 DEG DT:	1,327	CFM PER SQUARE FOOT:	1.002
SQUARE FT. OF ROOM AREA:	1969	SQUARE FOOT PER TON:	512.854

TOTAL HEATING REQUIRED WITH OUTSIDE AIR: 20.518 MBH
TOTAL COOLING REQUIRED WITH OUTSIDE AIR:

CALCULATIONS ARE BASED ON 7TH EDITION OF ACCA MANUAL J.
ALL COMPUTED RESULTS ARE ESTIMATES AS BUILDING USE AND WEATHER MAY VARY.
BE SURE TO SELECT A UNIT THAT MEETS BOTH SENSIBLE AND LATENT LOADS.

COLUMBIA COUNTY BUILDING DEPARTMENT
RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST
FOR THE FLORIDA RESIDENTIAL BUILDING CODE 2004 with 2005 & 2006
Supplements and One (1) and Two (2) Family Dwellings

ALL REQUIREMENTS ARE SUBJECT TO CHANGE

ALL BUILDING PLANS MUST INDICATE COMPLIANCE with the Current FLORIDA BUILDING CODES and the Current FLORIDA RESIDENTIAL CODE. ALL PLANS OR DRAWING SHALL PROVIDED CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS.

FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEEDS ARE PER FIGURE R301.2(4) of the Residential Code (Florida Wind speed map) **SHALL BE USED.**

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE -------- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE --------110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

GENERAL REQUIREMENTS;

- Two (2) complete sets of plans containing the following:
- All drawings must be clear, concise and drawn to scale, details that are not used shall be marked void
- Condition space (Sq. Ft.) and total (Sq. Ft.) under roof shall be shown on the plans.
- Designers name and signature shall be on all documents and a licensed architect or engineer, signature and official embossed seal shall be affixed to the plans and documents per FBC 106.1.

Site Plan information including:

- Dimensions of lot or parcel of land
- Dimensions of all building set backs
- Location of all other structures (include square footage of structures) on parcel, existing or proposed well and septic tank and all utility easements.
- Provide a full legal description of property.

Wind-load Engineering Summary, calculations and any details required:

- Plans or specifications must meet state compliance with FRC Chapter 3
- The following information must be shown as per section FRC
- Basic wind speed (3-second gust), miles per hour
- Wind importance factor and nature of occupancy
- Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated
- The applicable internal pressure coefficient, Components and Cladding The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional.

Elevations Drawing including:

- All side views of the structure
- Roof pitch
- Overhang dimensions and detail with attic ventilation
- Location, size and height above roof of chimneys
- NA Location and size of skylights with Florida Product Approval
- 2 Number of stories
- e) Building height from the established grade to the roofs highest peak

Floor Plan including:

- ✓ Dimensioned area plan showing rooms, attached garage, breeze ways, covered porches, deck, balconies and raised floor surfaces located more than 30 inches above the floor or grade
- ✓ All exterior and interior shear walls indicated
- ✓ Shear wall opening shown (Windows, Doors and Garage doors
- ✓ Emergency escape and rescue opening in each bedroom (net clear opening shown)
- ✓ Safety glazing of glass where needed
- ✓ Fireplaces types (gas appliance) (vented or non-vented) or wood burning with Hearth (see chapter 10 of FRC)
- ✓ Stairs with dimensions (width, tread and riser and total run) details of guardrails, Handrails (see FRC 311)
- ✓ Plans must show and identify accessibility of bathroom (see FRC 322)

All materials placed within opening or onto/into exterior shear walls, soffits or roofs shall have Florida product approval number and mfg. installation information submitted with the plans (see Florida product approval form)

Foundation Plans Per FRC 403:

- ✓ a) Location of all load-bearing walls footings indicated as standard, monolithic, dimensions, size and type of reinforcing.
- ✓ b) All posts and/or column footing including size and reinforcing
- ✓ c) Any special support required by soil analysis such as piling.
- ✓ d) Assumed load-bearing valve of soil_____ (psf)
- ✓ e) Location of horizontal and vertical steel, for foundation or walls (include # size and type)

CONCRETE SLAB ON GRADE Per FRC R506

- ✓ Show Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
- ✓ Show control joints, synthetic fiber reinforcement or welded fire fabric reinforcement and Supports

PROTECTION AGAINST TERMITES Per FRC 320: Subterranean

- ✓ Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or submit other approved termite protection methods. Protection shall be provided by registered termiticides

Masonry Walls and Stem walls (load bearing & shear Walls) FRC Section R606

NA
- Show all materials making up walls, wall height, and Block size, mortar type
- Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement

Metal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof. Engineer or Architect

Floor Framing System: First and/or second story

- ✓ Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer
- ✓ Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, stem walls and/or priers
- ✓ Girder type, size and spacing to load bearing walls, stem wall and/or priers
- ✓ Attachment of joist to girder
- ✓ Wind load requirements where applicable
- ✓ Show required under-floor crawl space
- ✓ Show required amount of ventilation opening for under-floor spaces
- ✓ Show required covering of ventilation opening.
- ✓ Show the required access opening to access to under-floor spaces
- ✓ Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges & intermediate of the areas structural panel sheathing
- ✓ Show Draft stopping, Fire caulking and Fire blocking
- ✓ Show fireproofing requirements for garages attached to living spaces, per FRC section R309
- ✓ Provide live and dead load rating of floor framing systems (psf).

WOOD WALL FRAMING CONSTRUCTION FRC CHAPTER 6

- Stud type, grade, size, wall height and oc spacing for all load bearing or shear walls.
- Fastener schedule for structural members per table R602.3 (1) are to be shown.
- Show wood structural panel's sheathing attachment to studs, joist, trusses, rafters and structural members, showing fastener schedule attachment on the edges & intermediate of the areas structural panel sheathing
- Show all required connectors with a max uplift rating and required number of connectors and oc spacing for continuous connection of structural walls to foundation and roof trusses or rafter systems.
- Show sizes, type, span lengths and required number of support jack studs, king studs for shear wall opening and girder or header per FRC Table R502.5 (1)
- Indicate where pressure treated wood will be placed.
- Show all wall structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing edges & intermediate areas
- A detail showing gable truss bracing, wall balloon framing details or/ and wall hinge bracing detail

ROOF SYSTEMS:

- Truss design drawing shall meet section FRC R802.10 Wood trusses. Include a layout and truss details and be signed and sealed by Fl. Pro. Eng.
- Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters
- Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details
- Provide dead load rating of trusses

Conventional Roof Framing Layout Per FRC 802:

- Rafter and ridge beams sizes, span, species and spacing
- Connectors to wall assemblies' include assemblies' resistance to uplift rating.
- Valley framing and support details
- Provide dead load rating of rafter system.

ROOF SHEATHING FRC Table R602,3(2) FRC 803

- Include all materials which will make up the roof decking, identification of structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing on the edges & intermediate areas

ROOF ASSEMBLIES FRC Chapter 9

- Include all materials which will make up the roof assembles covering; with Florida Product Approval numbers for each component of the roof assembles covering.

FCB Chapter 13 Florida Energy Efficiency Code for Building Construction

- Residential construction shall comply with this code by using the following compliance methods in the FBC Subchapter 13-6, Residential buildings compliance methods. Two of the required forms are to be submitted, showing dimensions condition area equal to the total condition living space area
- Show the insulation R value for the following areas of the structure: Attic space, Exterior wall cavity and Crawl space (if applicable)

HVAC information shown

- Manual J sizing equipment or equivalent computation
- Exhaust fans locations in bathrooms

Plumbing Fixture layout shown

- All fixtures waste water lines shall be shown on the foundation plan

Electrical layout shown including:

- Switches, outlets/receptacles, lighting and all required GFCI outlets identified
- Ceiling fans
- Smoke detectors
- Service panel, sub-panel, location(s) and total ampere ratings

- ✓ On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Indicate if the utility company service entrance cable will be of the overhead or underground type.
- ✓ Appliances and HVAC equipment and disconnects
- ✓ Arc Fault Circuits (AFCI) in bedrooms

✓ NA Notarized Disclosure Statement for Owner Builders

- Notice of Commencement Recorded (in the Columbia County Clerk Office) Notice Of Commencement is required to be filed with the building department Before Any Inspections Will Be Done.

Private Potable Water

- ✓ Size of pump motor
- ✓ Size of pressure tank
- ✓ Cycle stop valve if used

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

- Building Permit Application: A current Building Permit Application form is to be completed and submitted for all residential projects.

- Parcel Number: The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.

- Environmental Health Permit or Sewer Tap Approval: A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued. (386) 758-1058 (Toilet facilities shall be provided for construction workers)

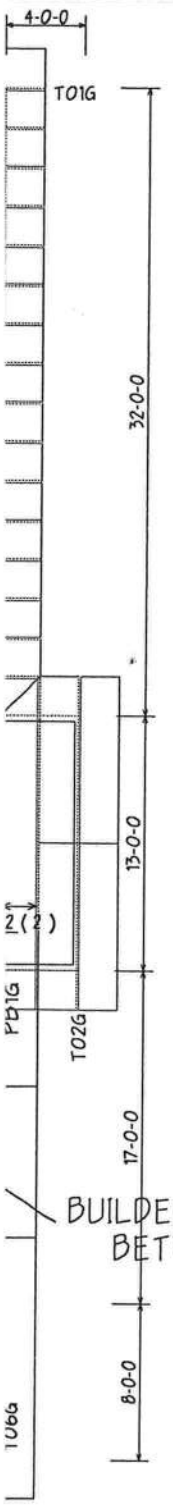
- City Approval: If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321

- Flood Information: All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED. A development permit will also be required. The permit cost is $50.00.

- Driveway Connection: If the property does not have an existing access to a public road, then an application for a culvert permit ($25.00) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver ($50.00). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial.

- 911 Address: If the project is located in an area where the 911 address has been issued, then the proper Paper work from the 911 Addressing Departments must be submitted. (386) 758-1125

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. NOTIFICATION WILL BE GIVEN WHEN THE APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT.

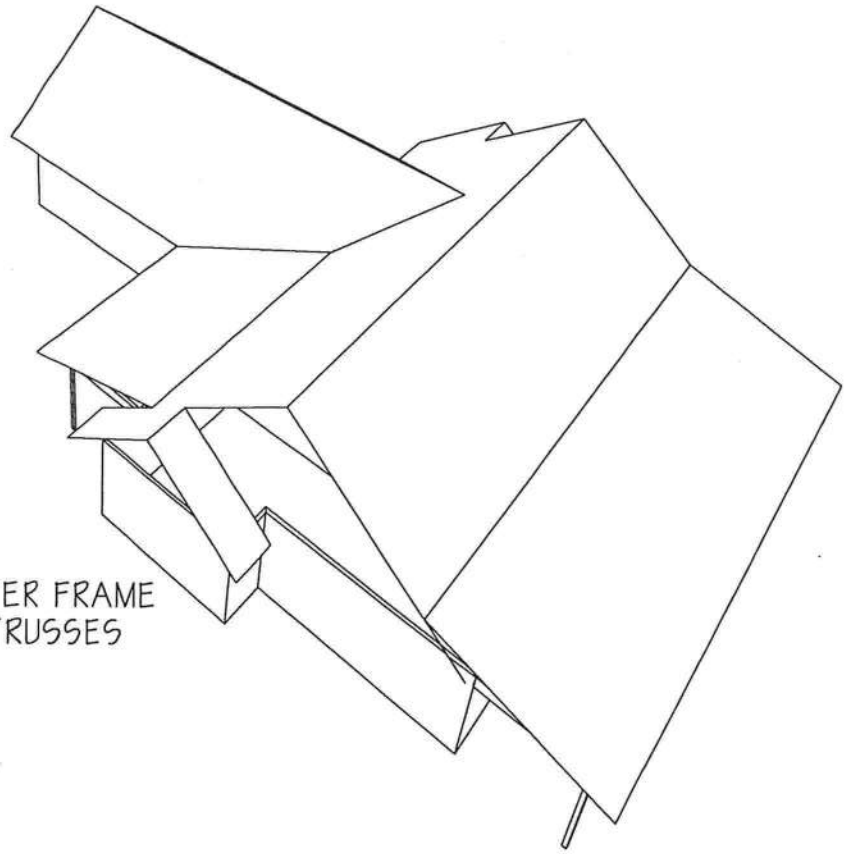


10/12 PITCH UNLESS NOTED

ALL 24" O.H.

TRUSS HEELS RAISED TO ALLOW FOR
8'-1"1/8 FLAT CLG AS PLANS. 4/12 HEELS
RAISED TO MATCH 10/12 SOFFIT ACCORDINGLY

BEARING ADDED SIDE OF STAIRWAY TO TRUSS ROOF



BEARING HEIGHT SCHEDULE	
	9'-1"-1/8

NOTES:

1) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.
2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL Y10S FOR ALTERNATE BRACING REQUIREMENTS.
3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.
4) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.
5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.
6) ST42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.
7) ALL ROOF TRUSS HANGERS TO BE SIMPSON H1U26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.
8) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

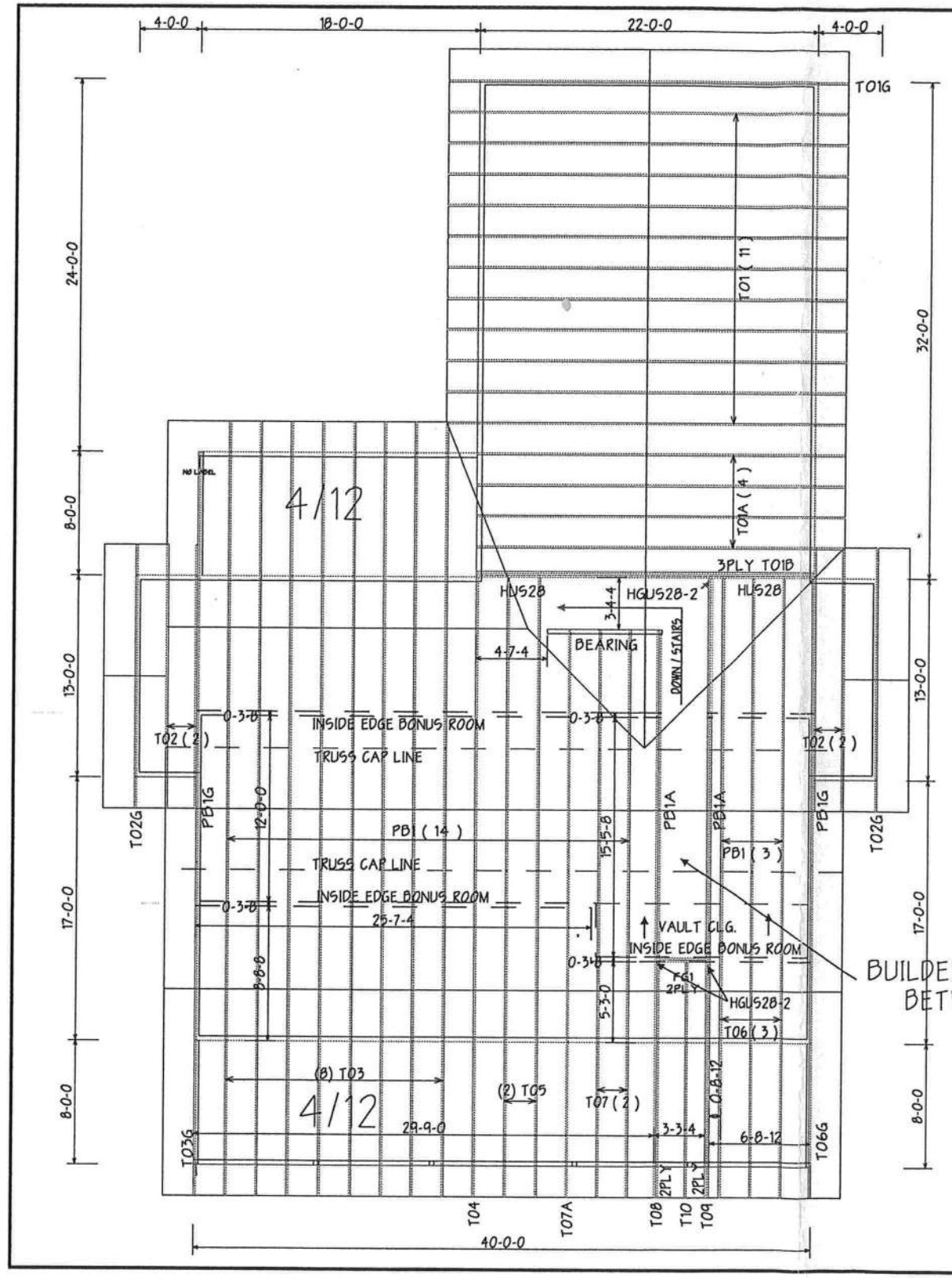
THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date: _____

Approved By: _____ Date: _____

Builders FirstSource
Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994
Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973
Lake City
PHONE: 386-755-6894 FAX: 386-755-7973
Sanford
PHONE: 407-322-0059 FAX: 407-322-5993

BUILDER: NORTON		
LEGAL ADDRESS: DEER HAMMOCK / RAINES RES.		
MODEL: CUSTOM		REVISION: SCALE: NTS
DATE: 1-5-08	DRAWN BY: B.CANNADY	JOB #: L264794

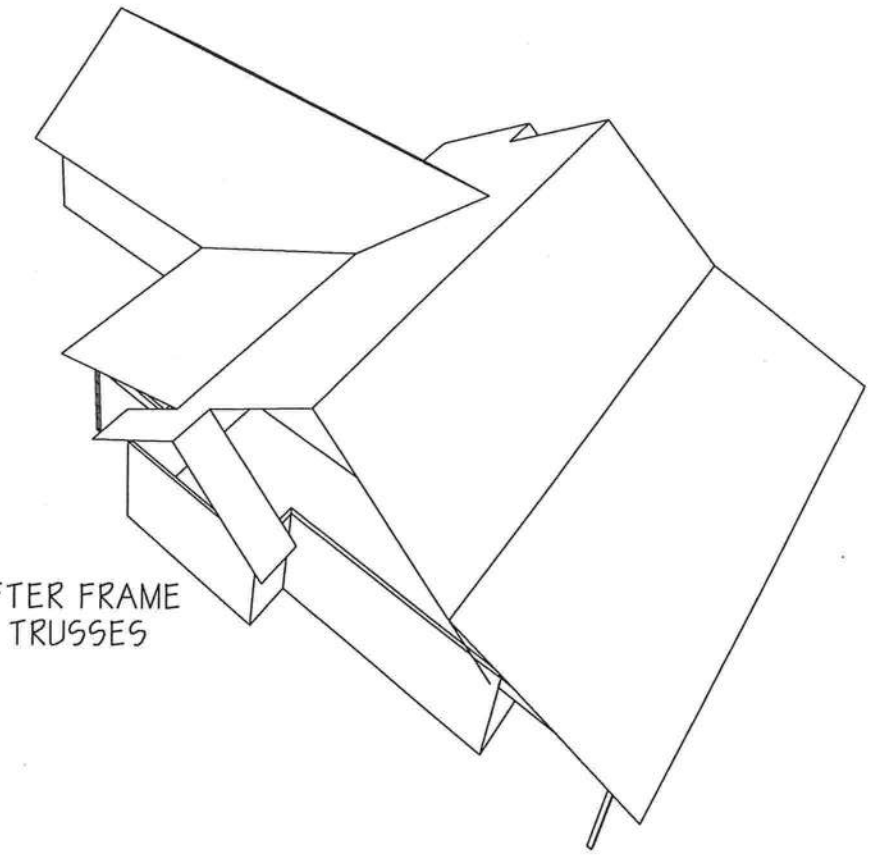


10/12 PITCH UNLESS NOTED

ALL 24" O.H.

TRUSS HEELS RAISED TO ALLOW FOR 8'-1"1/8 FLAT CLG AS PLANS. 4/12 HEELS RAISED TO MATCH 10/12 SOFFIT ACCORDINGLY

BEARING ADDED SIDE OF STAIRWAY TO TRUSS ROOF



BEARING HEIGHT SCHEDULE

	9'-1"-1/8

NOTES:

1) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.
2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V105 FOR ALTERNATE BRACING REQUIREMENTS.
3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.
4) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.
5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.
6) ST42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.
7) ALL ROOF TRUSS HANGERS TO BE SIMPSON H1026 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.
8) BEAM/HEADER, INTEL (MBR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date: _____

Approved By: _____ Date: _____

Builders FirstSource
Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994
Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1975
Lake City
PHONE: 386-755-6894 FAX: 386-755-7975
Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

BUILDER: NORTON		
LEGAL ADDRESS: DEER HAMMOCK / RAINES RES.		
MODEL: CUSTOM		ELEVATION: SCALE: NTS
DATE: 1-5-08	DESIGNER: D.CANNADY	JOB #: L264794

Builders FirstSource

Project Information for: L264794

Builder: Norton Home Improvement Company, Inc.
Address: 272 Southeast Myrtise Dortch Terrace
... Lake City, Florida
County: Columbia
Truss Count: 21
Design Program: MiTek 20/20 6.3
Building Code: FBC2004/TPI2002

Truss Design Load Information:

Gravity:
Roof (psf): 42.0
Floor (psf): 55.0

Wind:
Wind Standard: ASCE 7-02
Wind Speed (mph): 110
Wind Exposure: B

Note: See the individual truss drawings for special loading conditions.

Contractor of Record, responsible for structural engineering:
James H. Norton Florida License No. RB0031780
Address: 3367 South US Highway 441 Suite 101 Lake City, Florida 32025

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869
Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2
2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg. #	Truss ID	Date
1	J1923045	FG1	1/9/08
2	J1923046	PB1	1/9/08
3	J1923047	PB1A	1/9/08
4	J1923048	PB1G	1/9/08
5	J1923049	T01	1/9/08
6	J1923050	T01A	1/9/08
7	J1923051	T01B	1/9/08
8	J1923052	T01G	1/9/08
9	J1923053	T02	1/9/08
10	J1923054	T02G	1/9/08
11	J1923055	T03	1/9/08
12	J1923056	T03G	1/9/08
13	J1923057	T04	1/9/08
14	J1923058	T05	1/9/08
15	J1923059	T06	1/9/08
16	J1923060	T06G	1/9/08
17	J1923061	T07	1/9/08
18	J1923062	T07A	1/9/08
19	J1923063	T08	1/9/08
20	J1923064	T09	1/9/08
21	J1923065	T10	1/9/08

Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES.
L264794	FG1	SPECIAL	1	2	J1923045
					Job Reference (optional)

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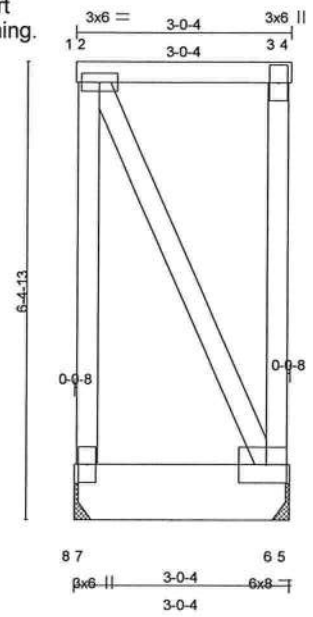
Warning: This truss has not been designed to support any additional load from conventional framing.



Scale = 1:30.4

Simpson HGUS28-2

Simpson HGUS28-2

Plate Offsets (X,Y): [6:0-3-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.00	TC	0.06	Vert(LL)	-0.00	7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.00	BC	0.01	Vert(TL)	-0.00	7	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 82 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 10 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 7=161/Mechanical, 6=161/Mechanical
Max Uplift 7=-45(load case 3), 6=-45(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 2-7=-74/35, 1-2=0/0, 2-3=0/0, 3-4=0/0, 3-6=-74/35
BOT CHORD 7-8=0/0, 6-7=-0/0, 5-6=0/0
WEBS 2-6=-0/0

JOINT STRESS INDEX
2 = 0.01, 3 = 0.01, 6 = 0.01 and 7 = 0.01

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 10 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34836
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

January 9,2008

Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES.
L264794	FG1	SPECIAL	1	2	J1923045
					Job Reference (optional)

NOTES

4) Provide adequate drainage to prevent water ponding.
5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 45 lb uplift at joint 7 and 45 lb uplift at joint 6.
8) Girder carries tie-in span(s): 5-3-0 from 0-0-0 to 3-0-4

LOAD CASE(S) Standard

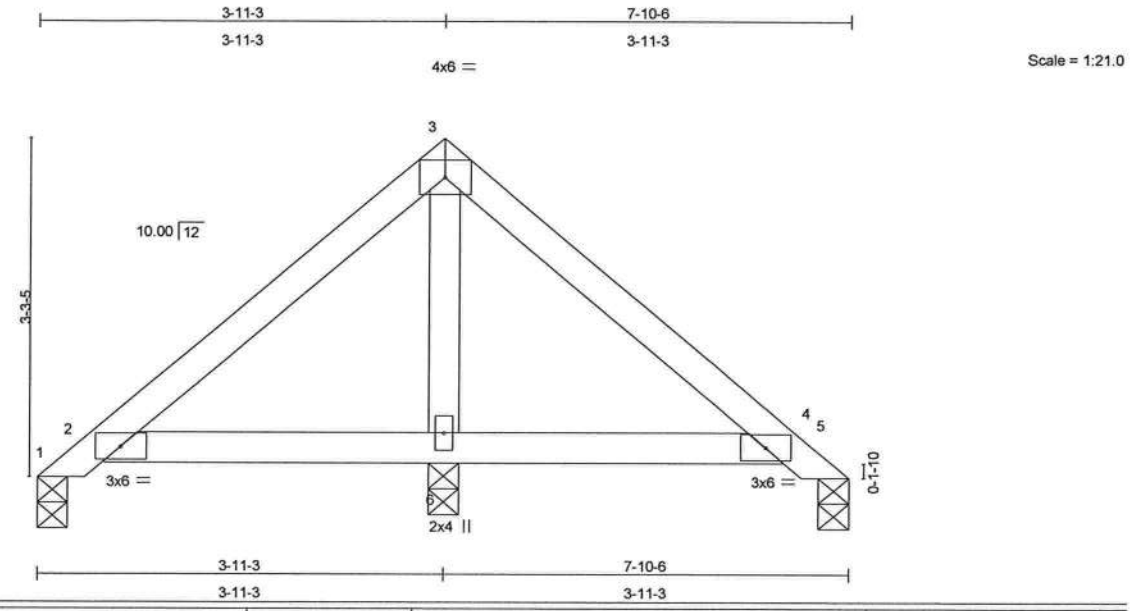
1) Regular: Lumber Increase=1.00, Plate Increase=1.00
   Uniform Loads (plf)
   Vert: 1-2=-14, 2-3=-54, 3-4=-14, 5-8=-57(F=-47)

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1326 Coastal Bay Blvd
Boynton Beach, FL 33435

January 9,2008

Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES.
L264794	PB1	PIGGYBACK	17	1	J1923046
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Sat Jan 05 19:28:14 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.11	Vert(LL)	-0.01	2-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	4-6	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.07	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 29 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=39/0-3-8, 5=39/0-3-8, 6=409/0-3-8
Max Horz 1=-88(load case 4)
Max Uplift 1=-11(load case 11), 5=-24(load case 4), 6=-104(load case 6)
Max Grav 1=64(load case 10), 5=64(load case 11), 6=409(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-80/81, 2-3=-75/168, 3-4=-75/168, 4-5=-35/18
BOT CHORD 2-6=-77/131, 4-6=-77/131
WEBS 3-6=-336/235

JOINT STRESS INDEX
2 = 0.29, 3 = 0.24, 4 = 0.29 and 6 = 0.13

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

January 9,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES. J1923046
L264794	PB1	PIGGYBACK	17	1	Job Reference (optional)

NOTES

5) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 11 lb uplift at joint 1, 24 lb uplift at joint 5 and 104 lb uplift at joint 6.
7) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

January 9,2008

Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES. J1923047
L264794	PB1A	PIGGYBACK	2	1	Job Reference (optional)

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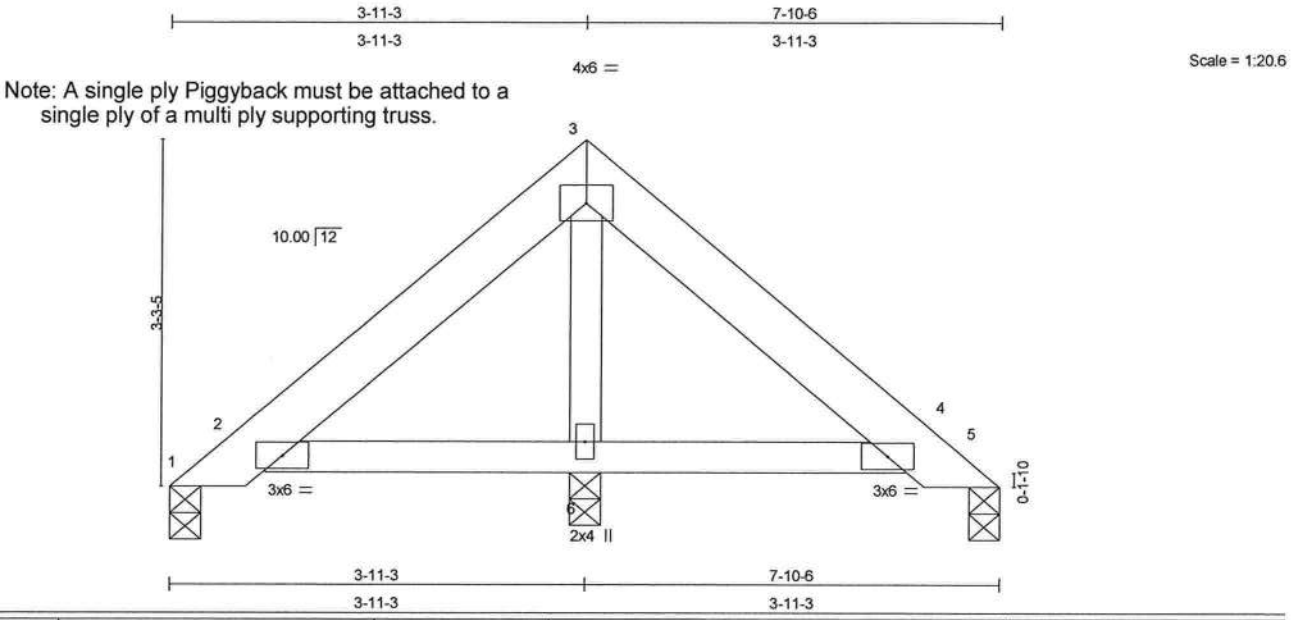


Note: A single ply Piggyback must be attached to a single ply of a multi ply supporting truss.

LOADING (psf)		SPACING	2-6-2	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.08	Vert(LL)	-0.00	2-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	2-6	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.07	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 36 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD 2-0-0 oc purlins (6-0-0 max.)
(Switched from sheeted: Spacing > 2-0-0).
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
JOINTS 1 Brace at Jt(s): 3

REACTIONS (lb/size) 1=76/0-3-8, 5=76/0-3-8, 6=460/0-3-8
Max Horz 1=-110(load case 4)
Max Uplift 1=-19(load case 7), 5=-32(load case 4), 6=-106(load case 6)
Max Grav 1=99(load case 10), 5=99(load case 11), 6=460(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-102/108, 2-3=-67/180, 3-4=-58/180, 4-5=-48/27
BOT CHORD 2-6=-66/132, 4-6=-66/132
WEBS 3-6=-392/254

JOINT STRESS INDEX
2 = 0.16, 3 = 0.36, 4 = 0.16 and 6 = 0.14

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

January 9,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES. J1923047
L264794	PB1A	PIGGYBACK	2	1	Job Reference (optional)

NOTES

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 19 lb uplift at joint 1, 32 lb uplift at joint 5 and 106 lb uplift at joint 6.
7) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

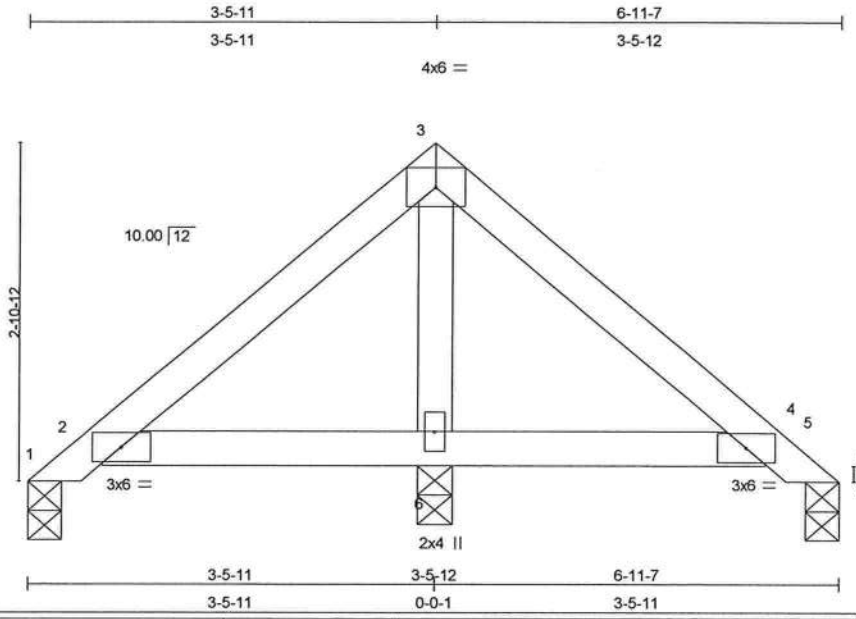
LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd.
Boynton Beach, FL 33435

January 9,2008

Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES.
L264794	PB1G	GABLE	2	1	J1923048
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 11:48:04 2008 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	-0.01	2-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.17	Vert(TL)	-0.01	2-6	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.13	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 25 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=98/0-3-8, 5=98/0-3-8, 6=813/0-3-8
Max Horz 1=-96(load case 4)
Max Uplift 1=-43(load case 7), 5=-50(load case 7), 6=-393(load case 6)
Max Grav 1=118(load case 10), 5=118(load case 11), 6=813(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-81/93, 2-3=-149/331, 3-4=-149/331, 4-5=-63/28
BOT CHORD 2-6=-139/154, 4-6=-139/154
WEBS 3-6=-693/427

JOINT STRESS INDEX
2 = 0.57, 3 = 0.47, 4 = 0.57 and 6 = 0.25

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

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Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES. J1923048
L264794	PB1G	GABLE	2	1	Job Reference (optional)

NOTES

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 43 lb uplift at joint 1, 50 lb uplift at joint 5 and 393 lb uplift at joint 6.
7) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
9) Truss designed for wind loads in plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail".

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-2=-153(F=-87), 2-3=-141(F=-87), 3-4=-141(F=-87), 4-5=-153(F=-87), 2-4=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34183
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

January 9,2008

Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES. J1923049
L264794	T01	COMMON	11	1	Job Reference (optional)

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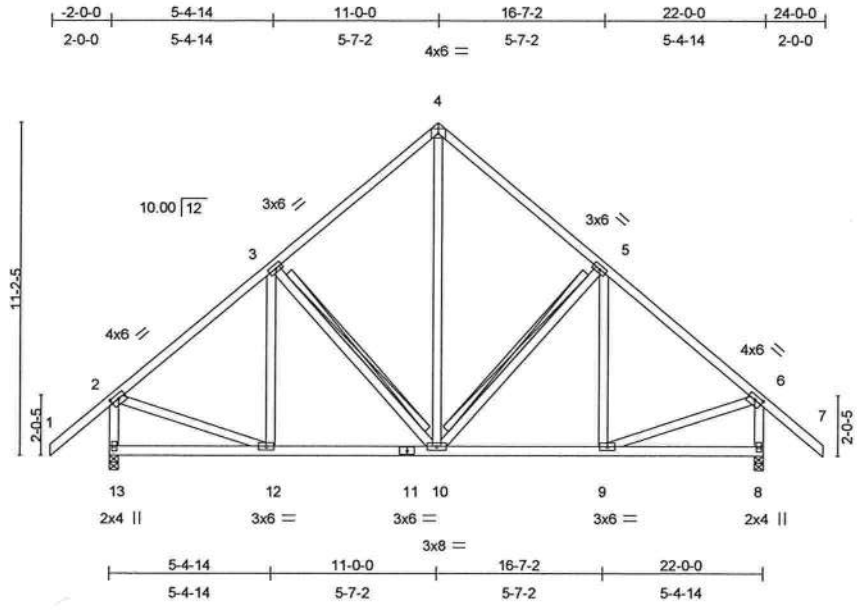

Plate Offsets (X,Y): [2:0-3-0,0-1-12], [6:0-3-0,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.02	10-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.04	9-10	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.63	Horz(TL)	0.01	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 161 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 3-10, 5-10
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 13=811/0-3-8, 8=811/0-3-8
Max Horz 13=321(load case 5)
Max Uplift 13=-220(load case 6), 8=-220(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/74, 2-3=-678/355, 3-4=-552/409, 4-5=-552/409, 5-6=-678/355, 6-7=0/74, 2-13=-783/458, 6-8=-783/458
BOT CHORD 12-13=-298/344, 11-12=-182/450, 10-11=-182/450, 9-10=-10/450, 8-9=0/84
WEBS 3-12=-97/68, 3-10=-182/226, 4-10=-300/328, 5-10=-182/226, 5-9=-97/68, 2-12=-61/459, 6-9=-61/459

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

JOINT STRESS INDEX
2 = 0.69, 3 = 0.43, 4 = 0.51, 5 = 0.43, 6 = 0.69, 8 = 0.54, 9 = 0.34, 10 = 0.56, 11 = 0.15, 12 = 0.34 and 13 = 0.54

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Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES. J1923049
L264794	T01	COMMON	11	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 220 lb uplift at joint 13 and 220 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

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Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES. J1923050
L264794	T01A	COMMON	4	1	Job Reference (optional)

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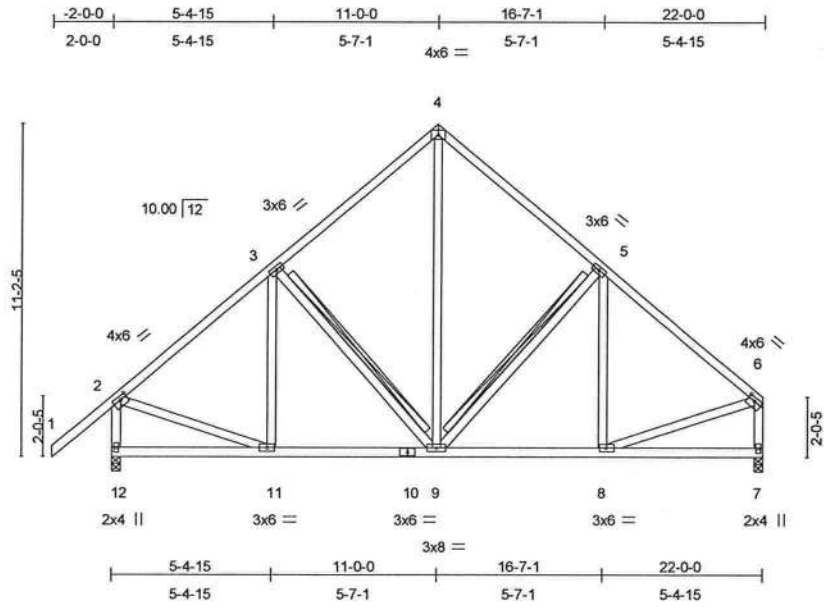

Plate Offsets (X,Y): [2:0-2-12,0-1-8], [6:0-2-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.02	9-11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.04	8-9	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.63	Horz(TL)	0.01	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 157 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 3-9, 5-9
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 12=816/0-3-8, 7=689/0-3-8
Max Horz 12=328(load case 5)
Max Uplift 12=-218(load case 6), 7=-131(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/74, 2-3=-684/353, 3-4=-560/407, 4-5=-561/408, 5-6=-694/341, 2-12=-789/456, 6-7=-659/328
BOT CHORD 11-12=-305/269, 10-11=-189/455, 9-10=-189/455, 8-9=-160/466, 7-8=-67/75
WEBS 3-11=-99/68, 3-9=-182/227, 4-9=-302/331, 5-9=-197/223, 5-8=-86/72, 2-11=-62/464, 6-8=-99/413

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

JOINT STRESS INDEX
2 = 0.80, 3 = 0.43, 4 = 0.50, 5 = 0.43, 6 = 0.80, 7 = 0.57, 8 = 0.34, 9 = 0.56, 10 = 0.15, 11 = 0.34 and 12 = 0.57

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Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES. J1923050
L264794	T01A	COMMON	4	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 218 lb uplift at joint 12 and 131 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd.
Boynton Beach, FL 33435

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Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES.
L264794	T01B	COMMON	1	3	J1923051
					Job Reference (optional)

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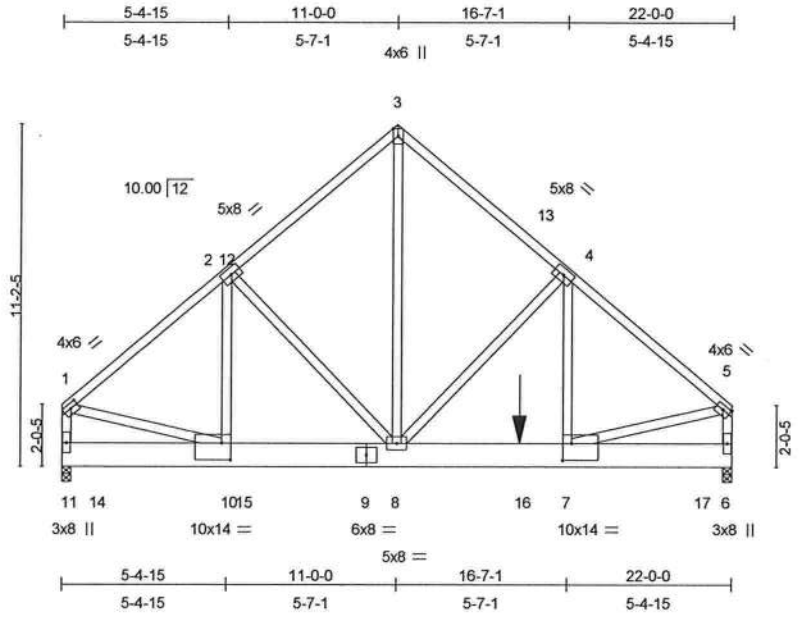

Plate Offsets (X,Y): [1:0-3-0,0-1-12], [5:0-3-0,0-1-12], [7:0-3-8,0-6-12], [10:0-3-8,0-6-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.00	TC	0.66	Vert(LL)	-0.06	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.00	BC	0.55	Vert(TL)	-0.12	7-8	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.42	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 607 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 10 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 11=6580/0-3-8, 6=8253/0-3-8
Max Horz 11=-315(load case 3)
Max Uplift 11=-1758(load case 5), 6=-2220(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-12=-5558/1506, 2-12=-5365/1505, 2-3=-4418/1256, 3-13=-4085/1239, 4-13=-4370/1251, 4-5=-7720/2105, 1-11=-4737/1265, 5-6=-6617/1784
BOT CHORD 11-14=-457/549, 14-15=-457/549, 10-15=-457/549, 9-10=-1239/4195, 8-9=-1239/4195, 8-16=-1522/5850, 7-16=-1522/5850, 7-17=-175/562, 6-17=-175/562
WEBS 2-10=-397/1586, 2-8=-1548/504, 3-8=-1388/4792, 4-8=-3859/1174, 4-7=-1263/4697, 1-10=-958/3805, 5-7=-1452/5518

JOINT STRESS INDEX
1 = 0.79, 2 = 0.69, 3 = 0.51, 4 = 0.69, 5 = 0.79, 6 = 0.66, 7 = 0.59, 8 = 0.60, 9 = 0.34, 10 = 0.59 and 11 = 0.66

NOTES
1) 3-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 10 - 4 rows at 0-4-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

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Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES.
L264794	T01B	COMMON	1	3	J1923051

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NOTES

3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1758 lb uplift at joint 11 and 2220 lb uplift at joint 6.

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.00, Plate Increase=1.00
   Uniform Loads (plf)
   Vert: 1-12=-54, 3-12=-104(F=-50), 3-13=-104(F=-50), 5-13=-54, 11-14=-10, 14-15=-1025(F=-1015), 15-16=-60(F=-50), 16-17=-1015(F=-1005), 6-17=-10
   Concentrated Loads (lb)
   Vert: 16=-2447(F)

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

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Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES. J1923052
L264794	T01G	GABLE	1	1	Job Reference (optional)

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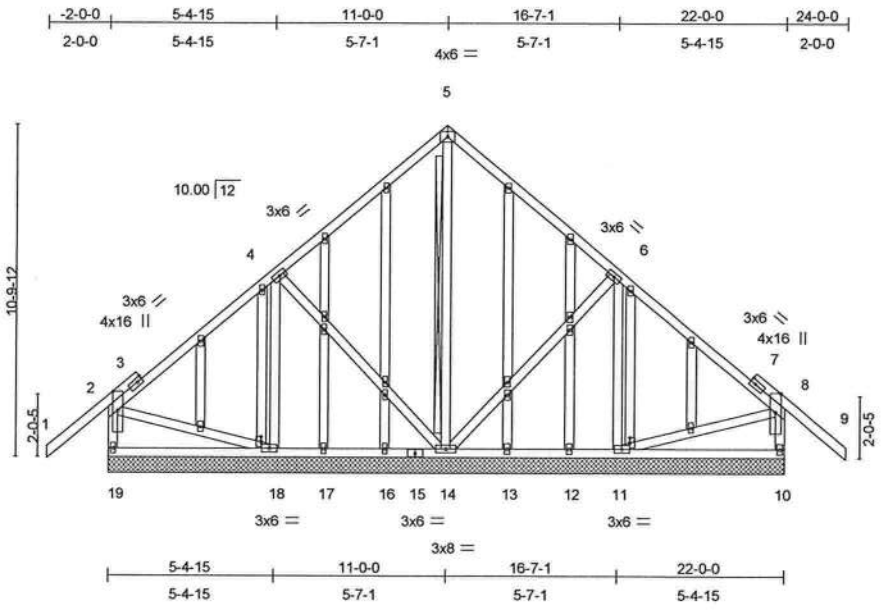

Plate Offsets (X,Y): [11:0-2-0,0-0-0], [18:0-2-0,0-0-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.02	9	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.12	Vert(TL)	-0.03	9	n/r	90		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.17	Horz(TL)	0.01	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 231 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	T-Brace: 2 X 4 SYP No.3 - 5-14 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance. Brace must cover 90% of web length.

REACTIONS (lb/size) 19=336/22-0-0, 18=276/22-0-0, 14=343/22-0-0, 11=276/22-0-0, 10=336/22-0-0, 16=24/22-0-0, 17=3/22-0-0, 13=24/22-0-0, 12=3/22-0-0
Max Horz 19=397(load case 5)
Max Uplift 19=-135(load case 6), 18=-164(load case 6), 14=-245(load case 6), 11=-136(load case 7), 10=-164(load case 7), 17=-2(load case 7), 12=-1(load case 6)
Max Grav 19=336(load case 1), 18=277(load case 10), 14=343(load case 1), 11=277(load case 11), 10=336(load case 1), 16=73(load case 2), 17=3(load case 6), 13=73(load case 2), 12=3(load case 1)

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd.
Boynton Beach, FL 33435

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Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES. J1923052
L264794	T01G	GABLE	1	1	Job Reference (optional)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/68, 2-3=-146/103, 3-4=-95/126, 4-5=-125/154, 5-6=-125/154, 6-7=-23/69, 7-8=-146/33, 8-9=0/68, 2-19=-307/207, 8-10=-307/207

BOT CHORD 18-19=-375/409, 17-18=-174/301, 16-17=-174/301, 15-16=-174/301, 14-15=-174/301, 13-14=-41/220, 12-13=-41/220, 11-12=-41/220, 10-11=-8/50

WEBS 4-18=-237/126, 4-14=-50/189, 5-14=-270/48, 6-14=-64/204, 6-11=-237/107, 2-18=-112/216, 8-11=-67/195

JOINT STRESS INDEX

2 = 0.61, 3 = 0.00, 3 = 0.22, 4 = 0.43, 5 = 0.54, 6 = 0.43, 7 = 0.00, 7 = 0.22, 8 = 0.61, 10 = 0.55, 11 = 0.34, 11 = 0.47, 12 = 0.33, 13 = 0.33, 14 = 0.56, 15 = 0.15, 16 = 0.33, 17 = 0.33, 18 = 0.34, 18 = 0.47, 19 = 0.55, 20 = 0.33, 20 = 0.33, 21 = 0.33, 22 = 0.33, 22 = 0.33, 23 = 0.33, 24 = 0.33, 25 = 0.33, 26 = 0.33, 27 = 0.33, 28 = 0.33, 28 = 0.33, 29 = 0.33, 30 = 0.33, 30 = 0.33, 31 = 0.33, 32 = 0.33 and 33 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 2-0-0 oc.
8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 135 lb uplift at joint 19, 164 lb uplift at joint 18, 245 lb uplift at joint 14, 136 lb uplift at joint 11, 164 lb uplift at joint 10, 2 lb uplift at joint 17 and 1 lb uplift at joint 12.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

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Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES.
L264794	T02	COMMON	4	1	J1923053
					Job Reference (optional)

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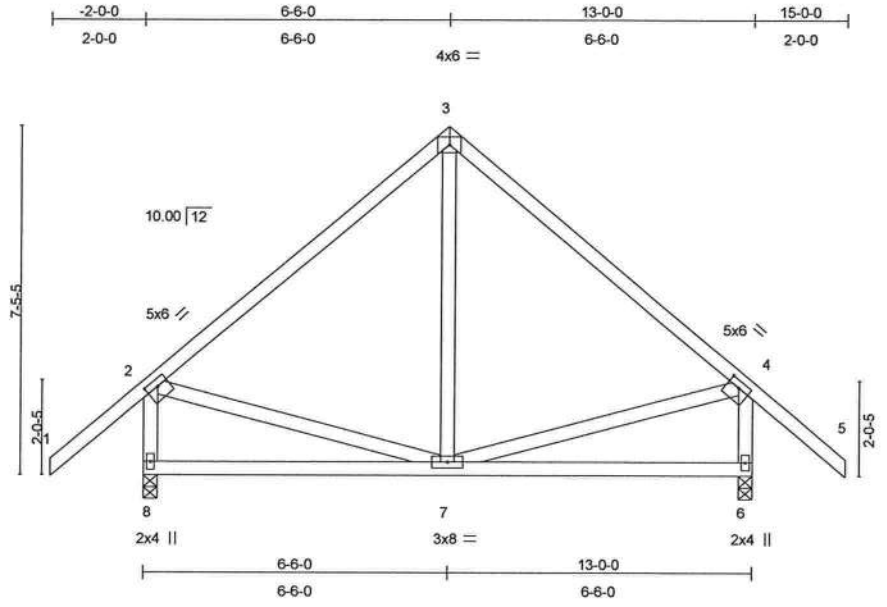

Plate Offsets (X,Y): [2:0-3-0,0-1-12], [4:0-3-0,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	-0.02	6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.04	6-7	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 86 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 8=523/0-3-8, 6=523/0-3-8
Max Horz 8=217(load case 5)
Max Uplift 8=-164(load case 6), 6=-164(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/74, 2-3=-362/218, 3-4=-362/218, 4-5=0/74, 2-8=-490/344, 4-6=-490/344
BOT CHORD 7-8=-236/275, 6-7=-21/61
WEBS 3-7=-24/151, 2-7=-103/202, 4-7=-106/205

JOINT STRESS INDEX
2 = 0.83, 3 = 0.65, 4 = 0.83, 6 = 0.74, 7 = 0.14 and 8 = 0.74

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd.
Boynton Beach, FL 33435

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Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES. J1923053
L264794	T02	COMMON	4	1	Job Reference (optional)

NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 164 lb uplift at joint 8 and 164 lb uplift at joint 6.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd.
Boynton Beach, FL 33435

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Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES. J1923054
L264794	T02G	GABLE	2	1	Job Reference (optional)

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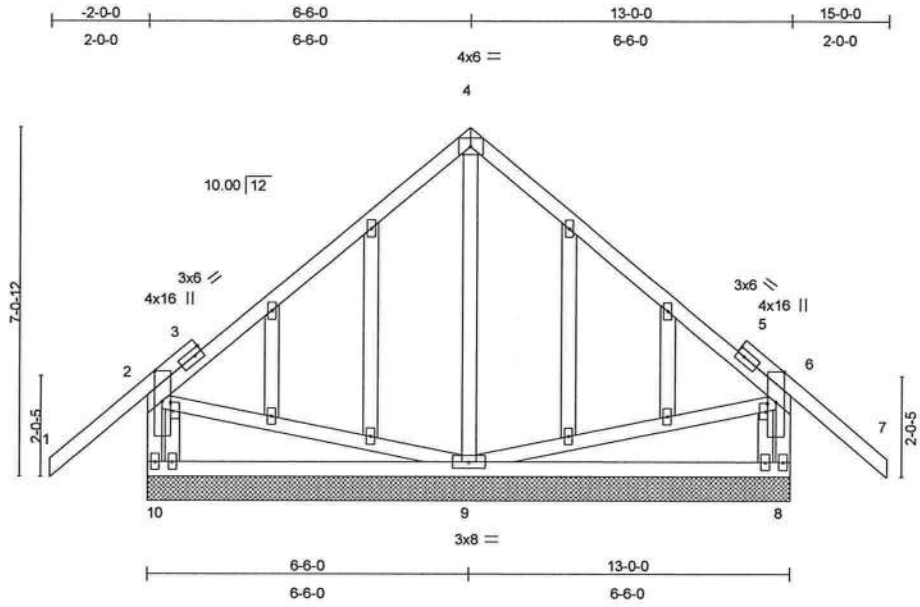

Plate Offsets (X,Y): [2:0-2-0,0-0-8], [6:0-2-0,0-0-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	0.01	6-7	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.17	Vert(TL)	0.01	6-7	n/r	90		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.25	Horz(TL)	0.00	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 112 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 10=313/13-0-0, 8=313/13-0-0, 9=420/13-0-0
Max Horz 10=-267(load case 4)
Max Uplift 10=-169(load case 6), 8=-171(load case 7), 9=-217(load case 6)
Max Grav 10=323(load case 10), 8=323(load case 11), 9=420(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/68, 2-3=-115/46, 3-4=-49/98, 4-5=-49/70, 5-6=-115/41, 6-7=0/68, 2-10=-288/223, 6-8=-288/223
BOT CHORD	9-10=-324/341, 8-9=-63/105
WEBS	4-9=-319/153, 2-9=-148/293, 6-9=-150/295

JOINT STRESS INDEX

2 = 0.58, 2 = 0.00, 3 = 0.00, 3 = 0.28, 4 = 0.62, 5 = 0.00, 5 = 0.28, 6 = 0.58, 6 = 0.00, 8 = 0.74, 8 = 0.00, 9 = 0.20, 10 = 0.74, 10 = 0.00, 11 = 0.00, 12 = 0.00, 13 = 0.00, 14 = 0.00, 15 = 0.00, 16 = 0.00, 17 = 0.00 and 18 = 0.00

NOTES

1) Unbalanced roof live loads have been considered for this design.

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

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Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES. J1923054
L264794	T02G	GABLE	2	1	Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 2-0-0 oc.
8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 169 lb uplift at joint 10, 171 lb uplift at joint 8 and 217 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

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Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES. J1923055
L264794	T03	ROOF TRUSS	8	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Sat Jan 05 19:28:24 2008 Page 1

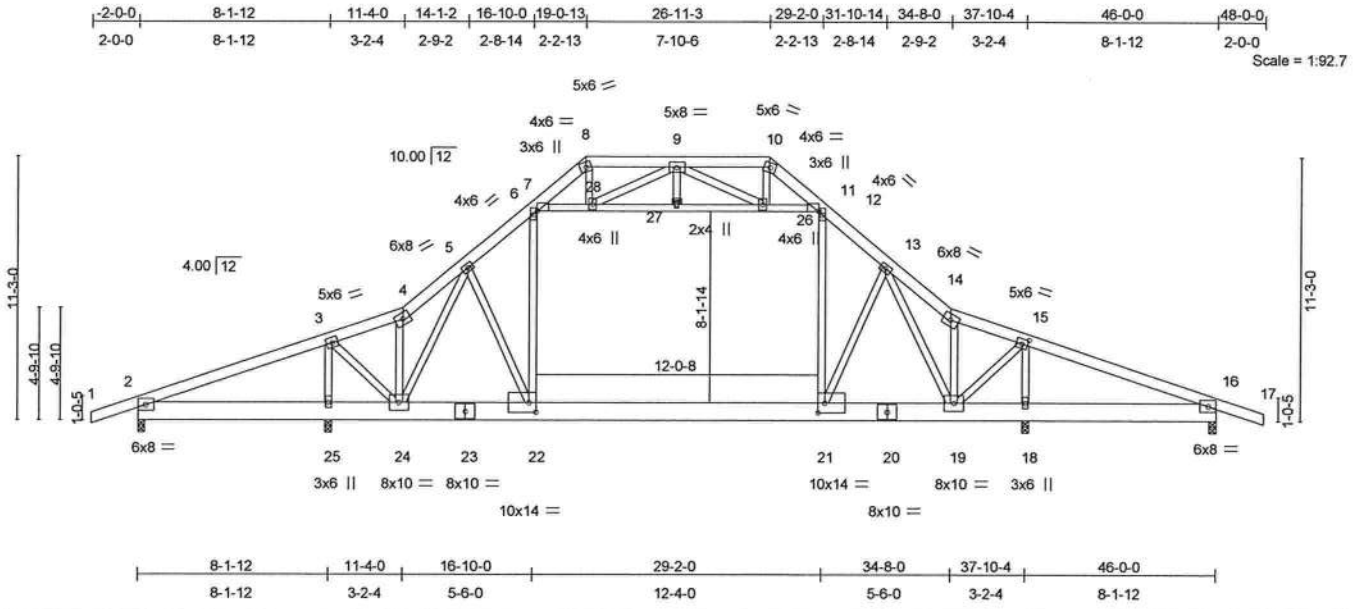

Plate Offsets (X,Y): [7:0-0-2,0-0-0], [11:0-0-2,0-0-0], [15:0-2-15,0-2-11], [21:0-3-8,0-5-0], [22:0-3-8,0-5-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.20	Vert(LL)	-0.15	21-22	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.70	Vert(TL)	-0.24	21-22	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.93	Horz(TL)	0.02	18	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 442 lb	

LUMBER

TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 10 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
7-11 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-8-1 oc purlins, except
2-0-0 oc purlins (6-0-0 max.): 8-10.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 7-11

REACTIONS (lb/size) 2=206/0-3-8, 25=2483/0-3-8, 18=2483/0-3-8, 16=206/0-3-8
Max Horz 2=-136(load case 7)
Max Uplift 2=-391(load case 4), 25=-610(load case 5), 18=-596(load case 4), 16=-376(load case 5)
Max Grav 2=206(load case 1), 25=2519(load case 11), 18=2519(load case 12), 16=206(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/14, 2-3=-351/504, 3-4=-1108/340, 4-5=-1358/474, 5-6=-2164/563, 6-7=-1505/489, 7-8=-562/258, 8-9=-420/237, 9-10=-420/237, 10-11=-562/258, 11-12=-1505/489, 12-13=-2165/563, 13-14=-1358/474, 14-15=-1108/340, 15-16=-312/504, 16-17=0/14

BOT CHORD 2-25=-398/252, 24-25=-398/252, 23-24=-107/1483, 22-23=-107/1483, 21-22=-39/1624, 20-21=-107/1483, 19-20=-107/1483, 18-19=-398/328, 16-18=-398/328

WEBS 3-25=-2107/824, 3-24=-485/1801, 4-24=-581/211, 5-24=-1179/52, 5-22=0/381, 6-22=-196/905, 12-21=-196/905, 13-21=0/381, 13-19=-1179/31, 14-19=-581/211, 15-19=-485/1801, 15-18=-2107/824, 10-26=-50/203, 9-27=0/42, 9-26=-332/143, 7-28=-1230/325, 27-28=-953/195, 26-27=-953/195, 11-26=-1230/325, 8-28=-50/203

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

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Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES. J1923055
L264794	T03	ROOF TRUSS	8	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.21, 3 = 0.62, 4 = 0.36, 5 = 0.33, 6 = 0.29, 7 = 0.28, 8 = 0.20, 9 = 0.27, 10 = 0.20, 11 = 0.28, 12 = 0.29, 13 = 0.33, 14 = 0.36, 15 = 0.68, 16 = 0.21, 18 = 0.33, 19 = 0.37, 20 = 0.21, 21 = 0.20, 22 = 0.20, 23 = 0.21, 24 = 0.37, 25 = 0.33, 26 = 0.32, 27 = 0.33 and 28 = 0.32

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Ceiling dead load (5.0 psf) on member(s). 6-7, 11-12, 7-28, 27-28, 26-27, 11-26; Wall dead load (5.0psf) on member(s).6-22, 12-21
6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 21-22
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 391 lb uplift at joint 2, 610 lb uplift at joint 25, 596 lb uplift at joint 18 and 376 lb uplift at joint 16.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54, 4-6=-54, 6-7=-64, 7-8=-54, 8-10=-54, 10-11=-54, 11-12=-64, 12-14=-54, 14-17=-54, 2-24=-10, 22-24=-70(F=-60), 21-22=-110, 19-21=-70(F=-60), 16-19=-10, 7-11=-10
   Drag: 6-22=-10, 12-21=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

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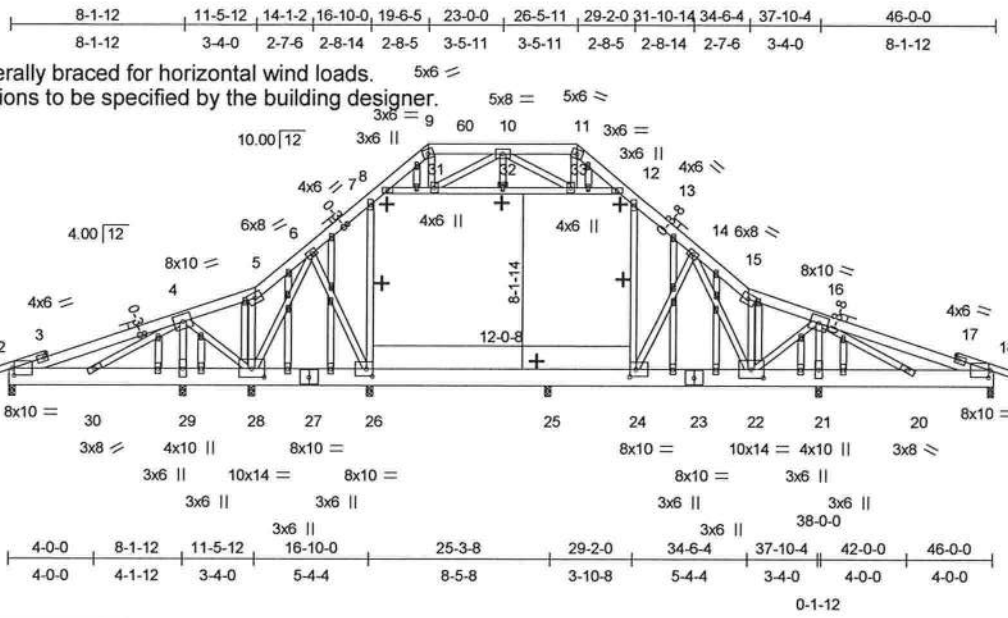
Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES. J1923056
L264794	T03G	GABLE	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jan 09 11:49:23 2008 Page 1



+ Member must be laterally braced for horizontal wind loads.
Bracing and connections to be specified by the building designer.

Plate Offsets (X,Y): [2:0-1-0,0-4-0], [18:0-1-0,0-4-0], [22:0-7-0,0-4-12], [24:0-3-8,0-4-0], [26:0-3-8,0-4-0], [28:0-7-0,0-4-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	0.06	22-24	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.85	Vert(TL)	-0.08	22-24	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.89	Horz(TL)	0.02	21	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 502 lb	

LUMBER

TOP CHORD	2 X 6 SYP No.1D *Except* 1-3 2 X 4 SYP No.2, 17-19 2 X 4 SYP No.2
BOT CHORD	2 X 10 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 9-11.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 21-22,20-21,18-20.
WEBS	1 Row at midpt 8-12

REACTIONS (lb/size) 2=891/0-3-8, 18=540/0-3-8, 29=1049/0-3-8, 28=1107/0-3-8, 26=821/0-3-8, 21=2431/0-3-8, 25=1189/0-3-8
Max Horz 2=158(load case 5)
Max Uplift 2=-936(load case 4), 18=-769(load case 5), 29=-914(load case 4), 28=-931(load case 4), 26=-380(load case 5), 21=-1674(load case 4), 25=-67(load case 4)
Max Grav 2=891(load case 1), 18=540(load case 1), 29=1049(load case 1), 28=1107(load case 1), 26=971(load case 11), 21=2431(load case 1), 25=1193(load case 12)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-11/20, 2-3=-889/793, 3-4=-699/688, 4-5=-749/573, 5-6=-924/722, 6-7=-1341/1050, 7-8=-1282/1097, 8-9=-1083/922, 9-60=-773/707, 10-60=-771/705, 10-11=-761/675, 11-12=-1006/837, 12-13=-1219/1026, 13-14=-1359/1051, 14-15=-1038/780, 15-16=-903/617, 16-17=-110/351, 17-18=0/356, 18-19=-11/20

BOT CHORD 2-30=-524/663, 29-30=-273/538, 28-29=-273/538, 27-28=-441/935, 26-27=-441/935, 25-26=-389/944, 24-25=-389/944, 23-24=-482/988, 22-23=-482/988, 21-22=-353/367, 20-21=-353/367, 18-20=-206/139

WEBS 4-29=-881/820, 4-28=0/162, 5-28=-607/544, 6-28=-729/568, 6-26=-46/153, 7-26=-464/479, 13-24=-174/163, 14-24=-109/226, 14-22=-513/341, 15-22=-506/440, 16-22=-921/1462, 16-21=-2140/1718, 8-31=-227/238, 31-32=-379/277, 32-33=-379/277, 12-33=-203/236, 9-31=-173/255, 10-32=0/33, 11-33=-188/258, 10-31=-430/369, 10-33=-437/402,

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Crandal Bay Blvd
Boynton Beach, FL 33435

January 9,2008

Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES. J1923056
L264794	T03G	GABLE	1	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.61, 3 = 0.00, 3 = 0.00, 3 = 0.64, 4 = 0.30, 5 = 0.30, 6 = 0.34, 7 = 0.16, 8 = 0.15, 9 = 0.43, 10 = 0.28, 11 = 0.43, 12 = 0.15, 13 = 0.16, 14 = 0.34, 15 = 0.30, 16 = 0.30, 17 = 0.00, 17 = 0.64, 17 = 0.00, 18 = 0.61, 20 = 0.12, 21 = 0.42, 22 = 0.47, 22 = 0.00, 23 = 0.24, 24 = 0.12, 26 = 0.12, 27 = 0.24, 28 = 0.47, 28 = 0.00, 29 = 0.42, 30 = 0.12, 31 = 0.32, 32 = 0.34, 33 = 0.32, 34 = 0.34, 35 = 0.34, 36 = 0.34, 36 = 0.34, 37 = 0.34, 38 = 0.16, 39 = 0.34, 39 = 0.34, 40 = 0.34, 41 = 0.16, 42 = 0.34, 43 = 0.16, 44 = 0.34, 45 = 0.16, 46 = 0.34, 47 = 0.34, 48 = 0.34, 49 = 0.34, 50 = 0.16, 51 = 0.34, 51 = 0.34, 52 = 0.34, 53 = 0.16, 54 = 0.34, 54 = 0.34, 55 = 0.34, 56 = 0.16, 57 = 0.34, 58 = 0.16 and 59 = 0.34

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Provide adequate drainage to prevent water ponding.
5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) All plates are 2x4 MT20 unless otherwise indicated.
7) Gable studs spaced at 2-0-0 oc.
8) Ceiling dead load (5.0 psf) on member(s). 7-8, 12-13, 8-31, 31-32, 32-33, 12-33; Wall dead load (5.0psf) on member(s).7-26, 13-24
9) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 25-26, 24-25
10) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 936 lb uplift at joint 2, 769 lb uplift at joint 18 , 914 lb uplift at joint 29, 931 lb uplift at joint 28, 380 lb uplift at joint 26, 1674 lb uplift at joint 21 and 67 lb uplift at joint 25.
12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-114(F=-60), 4-5=-141(F=-87), 5-7=-141(F=-87), 7-8=-151(F=-87), 8-9=-141(F=-87), 9-60=-141(F=-87), 11-60=-114(F=-60), 11-12=-114(F=-60), 12-13=-124(F=-60), 13-15=-114(F=-60), 15-19=-114(F=-60), 2-26=-10, 24-26=-110, 18-24=-10, 8-12=-10
   Drag: 7-26=-10, 13-24=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Ray Blvd.
Boynton Beach, FL 33435

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Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES. J1923057
L264794	T04	ROOF TRUSS	1	1	Job Reference (optional)

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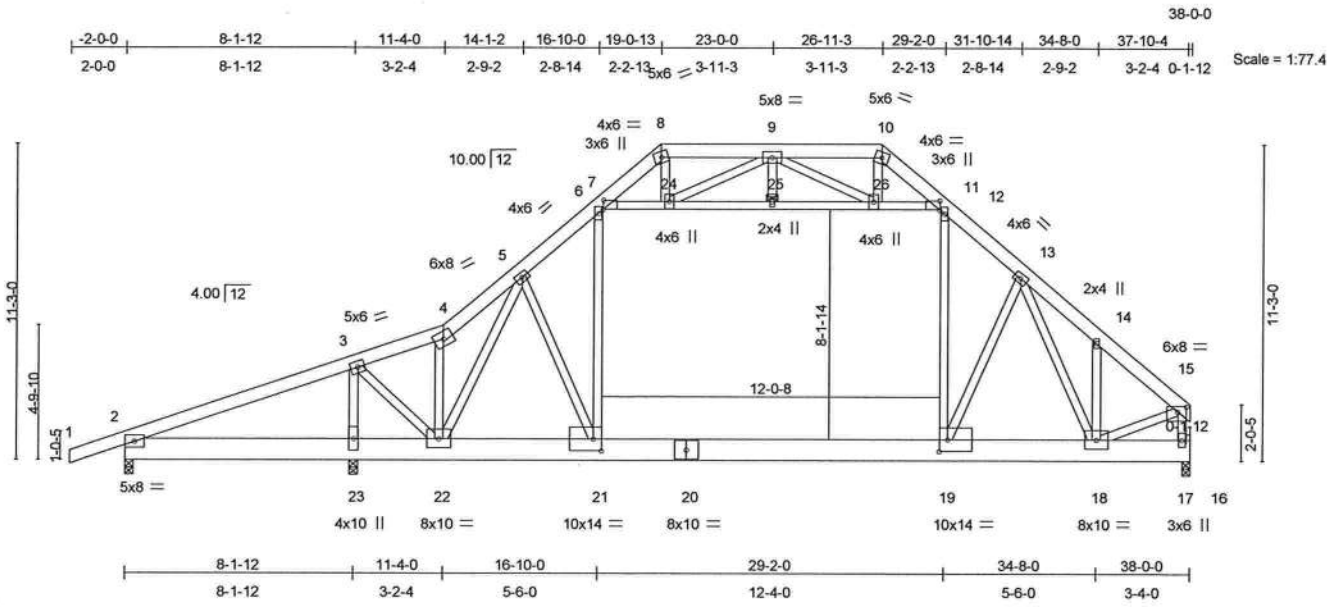

Plate Offsets (X,Y): [7:0-0-2,0-4-0], [11:0-0-2,0-4-0], [15:0-3-8,0-2-0], [19:0-3-8,0-5-0], [21:0-3-8,0-5-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.14	19-21	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.73	Vert(TL)	-0.24	19-21	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.91	Horz(TL)	0.03	17	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 384 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 10 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
7-11 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-5-12 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 8-10.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 2-23,22-23.
WEBS 1 Row at midpt 7-11

REACTIONS (lb/size) 2=278/0-3-8, 23=2430/0-3-8, 17=2025/0-3-8
Max Horz 2=307(load case 5)
Max Uplift 2=-451(load case 4), 23=-660(load case 5), 17=-240(load case 4)
Max Grav 2=278(load case 1), 23=2502(load case 11), 17=2025(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/14, 2-3=-519/450, 3-4=-1257/422, 4-5=-1566/577, 5-6=-2284/638, 6-7=-1583/539, 7-8=-558/252, 8-9=-417/225, 9-10=-435/244, 10-11=-566/262, 11-12=-1577/534, 12-13=-2297/648, 13-14=-1826/755, 14-15=-1833/635, 15-17=-1882/657
BOT CHORD 2-23=-348/221, 22-23=-348/221, 21-22=-293/1593, 20-21=-222/1722, 19-20=-222/1722, 18-19=-332/1641, 17-18=-47/110, 16-17=0/0
WEBS 3-23=-2090/801, 3-22=-466/1785, 4-22=-649/251, 5-22=-1153/127, 5-21=0/364, 6-21=-234/1000, 12-19=-260/984, 13-19=-17/339, 13-18=-728/0, 14-18=-153/162, 15-18=-422/1409, 7-24=-1342/399, 24-25=-1050/256, 25-26=-1050/256, 11-26=-1318/379, 8-24=-57/217, 9-25=0/43, 10-26=-50/203, 9-24=-358/161, 9-26=-331/150

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd.
Boynton Beach, FL 33435

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Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES. J1923057
L264794	T04	ROOF TRUSS	1	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.28, 3 = 0.62, 4 = 0.41, 5 = 0.33, 6 = 0.32, 7 = 0.31, 8 = 0.20, 9 = 0.27, 10 = 0.20, 11 = 0.31, 12 = 0.31, 13 = 0.33, 14 = 0.33, 15 = 0.34, 17 = 0.39, 18 = 0.38, 19 = 0.20, 20 = 0.36, 21 = 0.21, 22 = 0.37, 23 = 0.25, 24 = 0.32, 25 = 0.33 and 26 = 0.32

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Ceiling dead load (5.0 psf) on member(s). 6-7, 11-12, 7-24, 24-25, 25-26, 11-26; Wall dead load (5.0psf) on member(s).6-21, 12-19
6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 19-21
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 451 lb uplift at joint 2, 660 lb uplift at joint 23 and 240 lb uplift at joint 17.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54, 4-6=-54, 6-7=-64, 7-8=-54, 8-10=-54, 10-11=-54, 11-12=-64, 12-15=-54, 2-22=-10, 21-22=-70(F=-60), 19-21=-110, 18-19=-70(F=-60), 17-18=-10, 7-11=-10
   Drag: 6-21=-10, 12-19=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

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Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES. J1923058
L264794	T05	ROOF TRUSS	2	1	Job Reference (optional)

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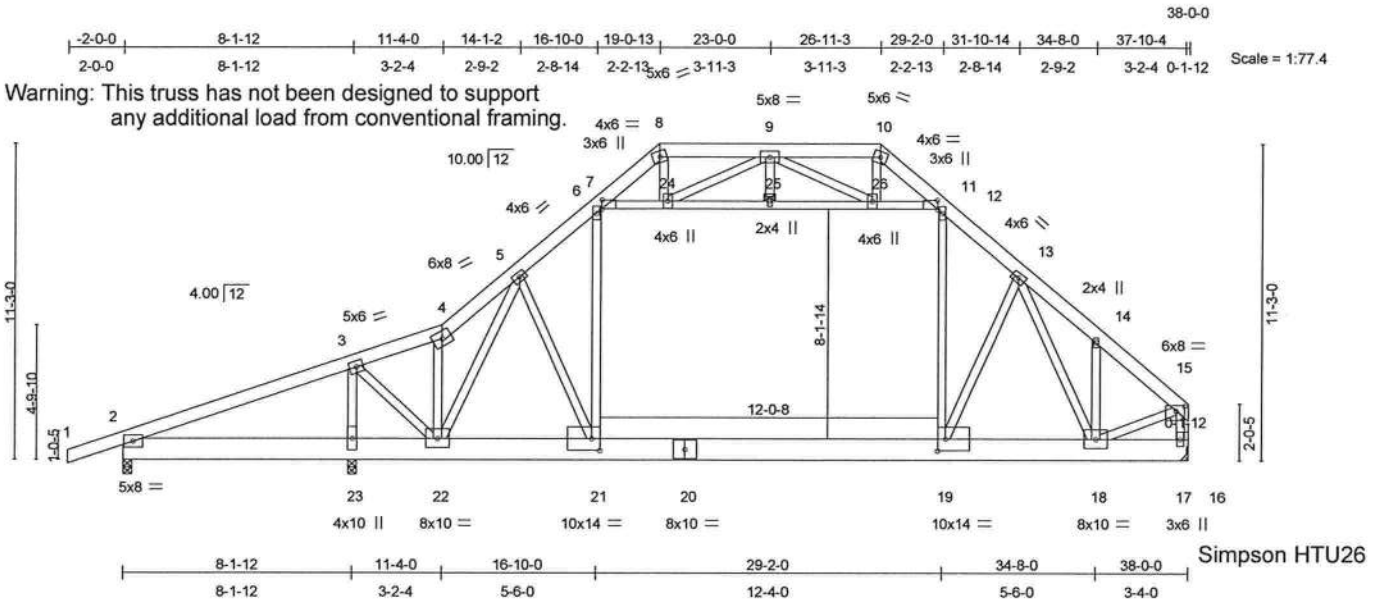

Simpson HTU26

Plate Offsets (X,Y): [7:0-0-2,0-4-0], [11:0-0-6,0-4-0], [15:0-3-8,0-2-0], [19:0-3-8,0-5-0], [21:0-3-8,0-5-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.14	19-21	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.73	Vert(TL)	-0.24	19-21	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.91	Horz(TL)	0.03	17	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 384 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 10 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
7-11 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-5-12 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 8-10.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 2-23,22-23.
WEBS 1 Row at midpt 7-11

REACTIONS (lb/size) 2=278/0-3-8, 23=2430/0-3-8, 17=2025/Mechanical
Max Horz 2=307(load case 5)
Max Uplift 2=-451(load case 4), 23=-660(load case 5), 17=-240(load case 4)
Max Grav 2=278(load case 1), 23=2502(load case 11), 17=2025(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/14, 2-3=-519/450, 3-4=-1257/422, 4-5=-1566/577, 5-6=-2284/638, 6-7=-1583/539, 7-8=-558/252, 8-9=-417/225, 9-10=-435/244, 10-11=-566/262, 11-12=-1577/534, 12-13=-2297/648, 13-14=-1826/755, 14-15=-1833/635, 15-17=-1882/657
BOT CHORD 2-23=-348/221, 22-23=-348/221, 21-22=-293/1593, 20-21=-222/1722, 19-20=-222/1722, 18-19=-332/1641, 17-18=-47/110, 16-17=0/0
WEBS 3-23=-2090/801, 3-22=-466/1785, 4-22=-649/251, 5-22=-1153/127, 5-21=0/364, 6-21=-234/1000, 12-19=-260/984, 13-19=-17/339, 13-18=-728/0, 14-18=-153/162, 15-18=-422/1409, 7-24=-1342/399, 24-25=-1050/256, 25-26=-1050/256, 11-26=-1318/379, 8-24=-57/217, 9-25=0/43, 10-26=-50/203, 9-24=-358/161, 9-26=-331/150

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd.
Boynton Beach, FL 33435

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Continued on page 2

Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES. J1923058
L264794	T05	ROOF TRUSS	2	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.28, 3 = 0.62, 4 = 0.41, 5 = 0.33, 6 = 0.32, 7 = 0.31, 8 = 0.20, 9 = 0.27, 10 = 0.20, 11 = 0.32, 12 = 0.31, 13 = 0.33, 14 = 0.33, 15 = 0.34, 17 = 0.39, 18 = 0.38, 19 = 0.20, 20 = 0.36, 21 = 0.21, 22 = 0.37, 23 = 0.25, 24 = 0.32, 25 = 0.33 and 26 = 0.32

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Ceiling dead load (5.0 psf) on member(s). 6-7, 11-12, 7-24, 24-25, 25-26, 11-26; Wall dead load (5.0psf) on member(s).6-21, 12-19
6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 19-21
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 451 lb uplift at joint 2, 660 lb uplift at joint 23 and 240 lb uplift at joint 17.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54, 4-6=-54, 6-7=-64, 7-8=-54, 8-10=-54, 10-11=-54, 11-12=-64, 12-15=-54, 2-22=-10, 21-22=-70(F=-60), 19-21=-110, 18-19=-70(F=-60), 17-18=-10, 7-11=-10
   Drag: 6-21=-10, 12-19=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

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Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES. J1923059
L264794	T06	ROOF TRUSS	3	1	Job Reference (optional)

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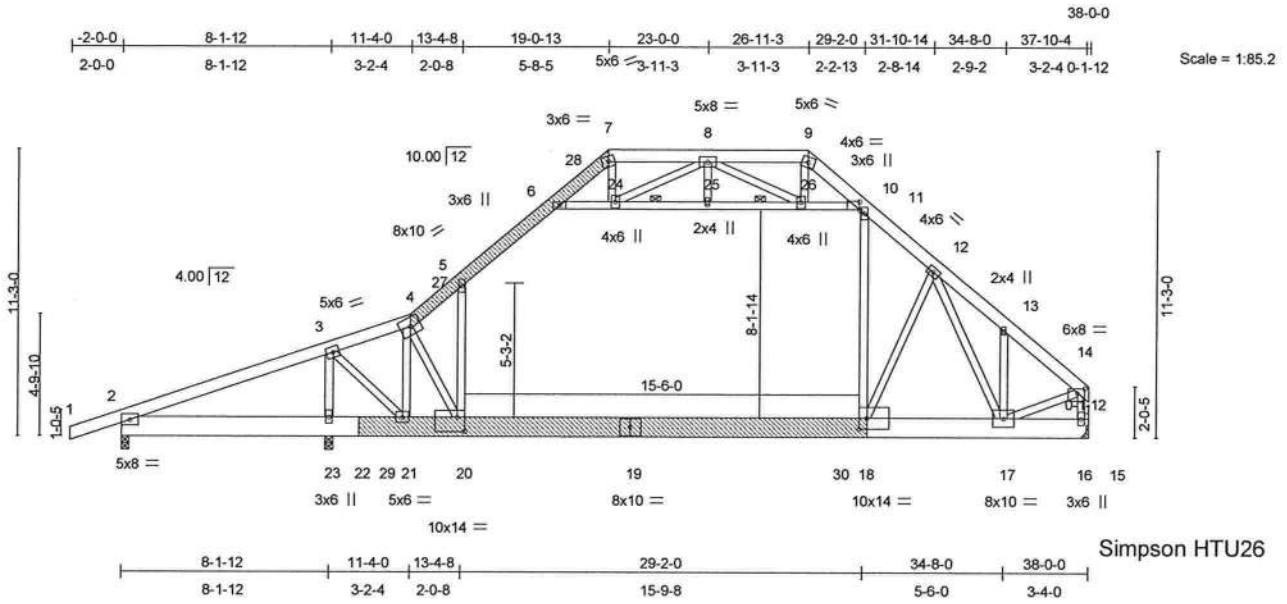

Simpson HTU26

Plate Offsets (X,Y): [10:0-0-6,0-4-0], [14:0-3-8,0-2-0], [18:0-3-8,0-5-0], [20:0-3-8,0-6-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	-0.25	18-20	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.63	Vert(TL)	-0.42	18-20	>853	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.77	Horz(TL)	0.02	16	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 469 lb	

LUMBER

TOP CHORD	2 X 6 SYP No.1D
BOT CHORD	2 X 10 SYP No.2
WEBS	2 X 4 SYP No.3 *Except* 6-10 2 X 4 SYP No.2
LBR SCAB	4-7 2 X 6 SYP No.1D one side 18-22 2 X 10 SYP No.2 one side

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-9-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 7-9.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 2-23,21-23.
WEBS	2 Rows at 1/3 pts 6-10

REACTIONS (lb/size) 2=166/0-3-8, 23=2587/0-3-8, 16=2007/Mechanical
Max Horz 2=305(load case 5)
Max Uplift 2=-537(load case 4), 23=-175(load case 5), 16=-207(load case 4)
Max Grav 2=166(load case 1), 23=2614(load case 11), 16=2007(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/14, 2-3=-86/649, 3-4=-1139/363, 4-27=-2531/411, 5-27=-2465/420, 5-6=-1748/503, 6-28=-239/229, 7-28=-188/240, 7-8=0/363, 8-9=-447/256, 9-10=-503/259, 10-11=-1439/476, 11-12=-2302/584, 12-13=-1797/716, 13-14=-1826/601, 14-16=-1877/624
BOT CHORD	2-23=-531/0, 22-23=-531/0, 22-29=-531/0, 21-29=-531/0, 20-21=-232/1127, 19-20=-161/1650, 19-30=-161/1650, 18-30=-161/1650, 17-18=-289/1626, 16-17=-45/110, 15-16=0/0
WEBS	3-23=-2266/231, 3-21=0/1950, 4-21=-2719/46, 5-20=-209/1356, 11-18=-251/1192, 12-18=-34/326, 12-17=-710/0, 13-17=-118/154, 14-17=-394/1400, 6-24=-1902/409, 24-25=-1217/222, 25-26=-1217/222, 10-26=-1218/305, 7-24=-79/419, 8-25=0/41, 9-26=-66/81, 8-24=-768/203, 8-26=-73/187, 4-20=0/1250

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

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Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES. J1923059
L264794	T06	ROOF TRUSS	3	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.26, 3 = 0.67, 4 = 0.66, 4 = 0.00, 5 = 0.42, 5 = 0.00, 6 = 0.65, 6 = 0.00, 7 = 0.63, 7 = 0.00, 8 = 0.27, 9 = 0.24, 10 = 0.30, 11 = 0.38, 12 = 0.33, 13 = 0.33, 14 = 0.34, 16 = 0.39, 17 = 0.38, 18 = 0.21, 18 = 0.00, 19 = 0.46, 19 = 0.00, 20 = 0.38, 20 = 0.00, 21 = 0.63, 21 = 0.00, 22 = 0.00, 22 = 0.00, 23 = 0.36, 24 = 0.32, 25 = 0.33 and 26 = 0.32

NOTES

1) Attached 10-5-7 scab 4 to 7, front face(s) 2 X 6 SYP No.1D with 2 row(s) of 10d (0.131"x3") nails spaced 9" o.c.except : starting at 0-0-3 from end at joint 4, nail 2 row(s) at 7 o.c. for 3-7-11; starting at 6-2-3 from end at joint 4, nail 2 row(s) at 7 o.c. for 3-10-15; starting at 4-2-8 from end at joint 4, nail 2 row(s) at 7 o.c. for 2-10-8; starting at 4-3-0 from end at joint 4, nail 2 row(s) at 4 o.c. for 2-0-0; starting at 16-4-5 from end at joint 4, nail 2 row(s) at 7 o.c. for 2-0-0.
2) Attached 20-0-0 scab 18 to 22, front face(s) 2 X 10 SYP No.2 with 2 row(s) of 10d (0.131"x3") nails spaced 9" o.c.except : starting at 4-7-4 from end at joint 22, nail 2 row(s) at 7 o.c. for 3-7-11; starting at 10-6-8 from end at joint 22, nail 2 row(s) at 7 o.c. for 3-10-15; starting at 0-0-0 from end at joint 22, nail 2 row(s) at 7 o.c. for 2-10-8; starting at 3-0-12 from end at joint 22, nail 2 row(s) at 4 o.c. for 2-0-0; starting at 17-10-4 from end at joint 22, nail 2 row(s) at 7 o.c. for 2-0-0.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
5) Provide adequate drainage to prevent water ponding.
6) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) Ceiling dead load (5.0 psf) on member(s). 4-5, 5-6, 10-11, 6-24, 24-25, 25-26, 10-26; Wall dead load (5.0psf) on member(s).5-20, 11-18
8) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 18-20
9) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 537 lb uplift at joint 2, 175 lb uplift at joint 23 and 207 lb uplift at joint 16.
11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54, 4-6=-64, 6-7=-54, 7-9=-54, 9-10=-54, 10-11=-64, 11-14=-54, 2-20=-10, 18-20=-110, 17-18=-70(F=-60), 16-17=-10, 6-10=-10
   Drag: 5-20=-10, 11-18=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

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Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES. J1923060
L264794	T06G	GABLE	1	1	Job Reference (optional)

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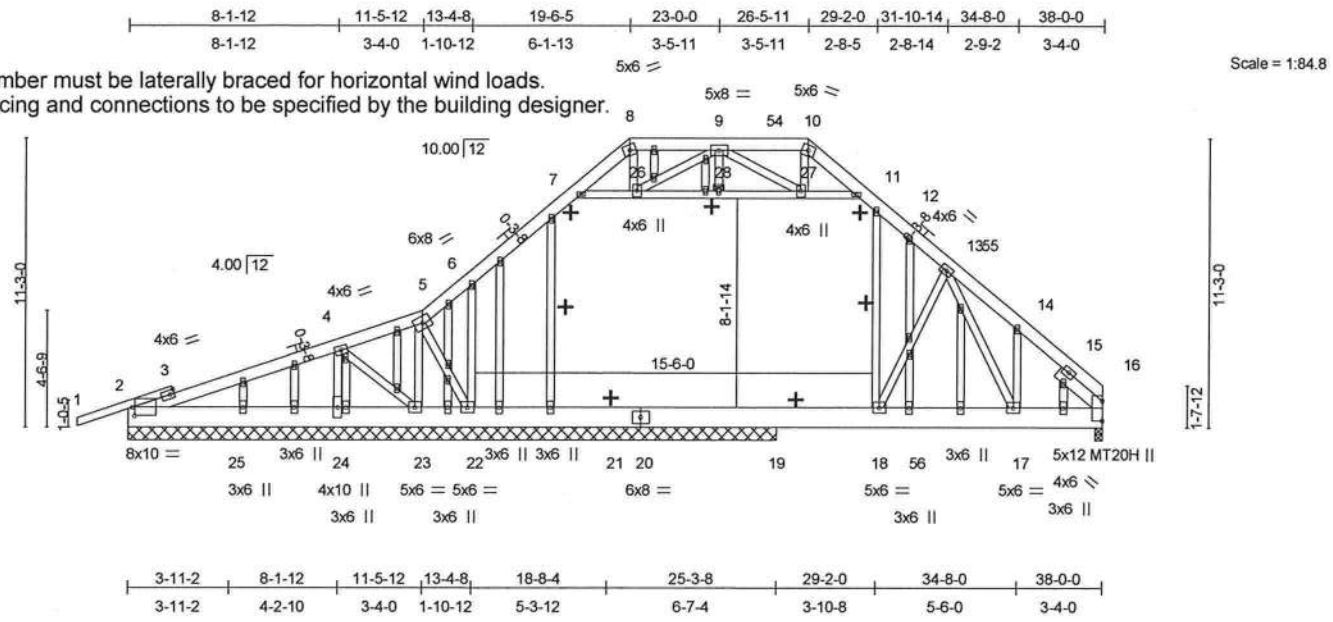

+ Member must be laterally braced for horizontal wind loads.
Bracing and connections to be specified by the building designer.

Plate Offsets (X,Y): [2:0-1-0,0-4-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	0.10	17-18	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.86	Vert(TL)	-0.13	17-18	>999	240	MT20H	187/143
BCLL	10.0	* Rep Stress Incr	NO	WB	0.23	Horz(TL)	0.03	16	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 417 lb	

LUMBER

TOP CHORD	2 X 6 SYP No.1D *Except* 1-3 2 X 4 SYP No.2
BOT CHORD	2 X 10 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3
SLIDER	Right 2 X 6 SYP No.1D 2-3-1

BRACING

TOP CHORD	Structural wood sheathing directly applied or 5-10-0 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 8-10.
BOT CHORD	Rigid ceiling directly applied or 9-9-7 oc bracing.
WEBS	1 Row at midpt 7-11

REACTIONS (lb/size) 2=910/0-3-8, 16=1592/0-3-8, 24=1061/0-3-8, 23=487/0-3-8, 22=835/0-3-8, 19=1393/0-3-8, 21=441/0-3-8, 25=227/0-3-8
Max Horz 2=398(load case 5)
Max Uplift 2=-885(load case 4), 16=-781(load case 6), 24=-702(load case 4), 23=-490(load case 4), 22=-340(load case 6), 19=-531(load case 4), 25=-236(load case 4)
Max Grav 2=910(load case 1), 16=1592(load case 1), 24=1061(load case 1), 23=487(load case 1), 22=878(load case 11), 19=1494(load case 12), 21=557(load case 10), 25=229(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-12/20, 2-3=-1100/789, 3-4=-907/678, 4-5=-1297/923, 5-6=-1607/1154, 6-7=-1732/1382, 7-8=-976/797, 8-9=-692/650, 9-54=-808/719, 10-54=-811/721, 10-11=-1071/925, 11-12=-1483/1298, 12-55=-1601/1345, 13-55=-1757/1426, 13-14=-1673/1301, 14-15=-1890/1339, 15-16=-1945/1322
BOT CHORD	2-25=-729/860, 24-25=-729/860, 23-24=-729/860, 22-23=-840/1171, 21-22=-895/1230, 20-21=-895/1230, 19-20=-895/1230, 18-19=-895/1230, 18-56=-845/1347, 17-56=-845/1347, 16-17=-819/1298
WEBS	4-24=-1053/861, 4-23=-126/402, 5-23=-866/597, 5-22=-126/143, 6-22=-545/538, 12-18=-89/156, 13-18=-333/352, 13-17=-238/398, 14-17=-211/276, 7-26=-598/584, 26-28=-134/201, 27-28=-134/201, 11-27=-449/461, 8-26=-237/326, 10-27=-185/240,

Continued on page 2

julius Lee
Truss Design Engineer
Florida PE No. 34869
4,09 Coastal Bay Blvd
Boynton Beach, FL 33435

January 9,2008

Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES. J1923060
L264794	T06G	GABLE	1	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.60, 3 = 0.00, 3 = 0.00, 3 = 0.60, 4 = 0.35, 5 = 0.44, 6 = 0.34, 7 = 0.35, 8 = 0.42, 9 = 0.28, 10 = 0.47, 11 = 0.34, 12 = 0.34, 13 = 0.34, 14 = 0.34, 15 = 0.00, 16 = 0.86, 16 = 0.71, 17 = 0.24, 18 = 0.24, 20 = 0.21, 22 = 0.23, 23 = 0.19, 24 = 0.21, 24 = 0.16, 26 = 0.32, 27 = 0.32, 28 = 0.34, 29 = 0.34, 30 = 0.34, 31 = 0.34, 32 = 0.34, 33 = 0.16, 34 = 0.34, 35 = 0.16, 36 = 0.34, 37 = 0.34, 37 = 0.34, 38 = 0.34, 39 = 0.16, 40 = 0.34, 41 = 0.34, 42 = 0.34, 43 = 0.16, 44 = 0.34, 45 = 0.16, 46 = 0.34, 47 = 0.34, 48 = 0.16, 49 = 0.34, 49 = 0.34, 50 = 0.16, 51 = 0.34, 52 = 0.16, 53 = 0.00 and 53 = 0.34

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Provide adequate drainage to prevent water ponding.
5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) All plates are MT20 plates unless otherwise indicated.
7) All plates are 2x4 MT20 unless otherwise indicated.
8) Gable studs spaced at 2-0-0 oc.
9) Ceiling dead load (5.0 psf) on member(s). 5-6, 6-7, 11-12, 7-26, 26-28, 27-28, 11-27; Wall dead load (5.0psf) on member(s).6-22, 12-18
10) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 21-22, 19-21, 18-19
11) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 885 lb uplift at joint 2, 781 lb uplift at joint 16 , 702 lb uplift at joint 24, 490 lb uplift at joint 23, 340 lb uplift at joint 22, 531 lb uplift at joint 19 and 236 lb uplift at joint 25.
13) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-5=-114(F=-60), 5-7=-124(F=-60), 7-8=-114(F=-60), 8-54=-114(F=-60), 10-54=-141(F=-87), 10-11=-141(F=-87), 11-12=-151(F=-87), 12-55=-141(F=-87), 16-55=-54, 2-22=-10, 19-22=-110, 18-19=-170(F=-60), 18-56=-70(F=-60), 16-56=-10, 7-11=-10
   Drag: 6-22=-10, 12-18=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1100 Coastal Bay Blvd
Boynton Beach, FL 33435

January 9,2008

Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES. J1923061
L264794	T07	ROOF TRUSS	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Sat Jan 05 19:28:35 2008 Page 1

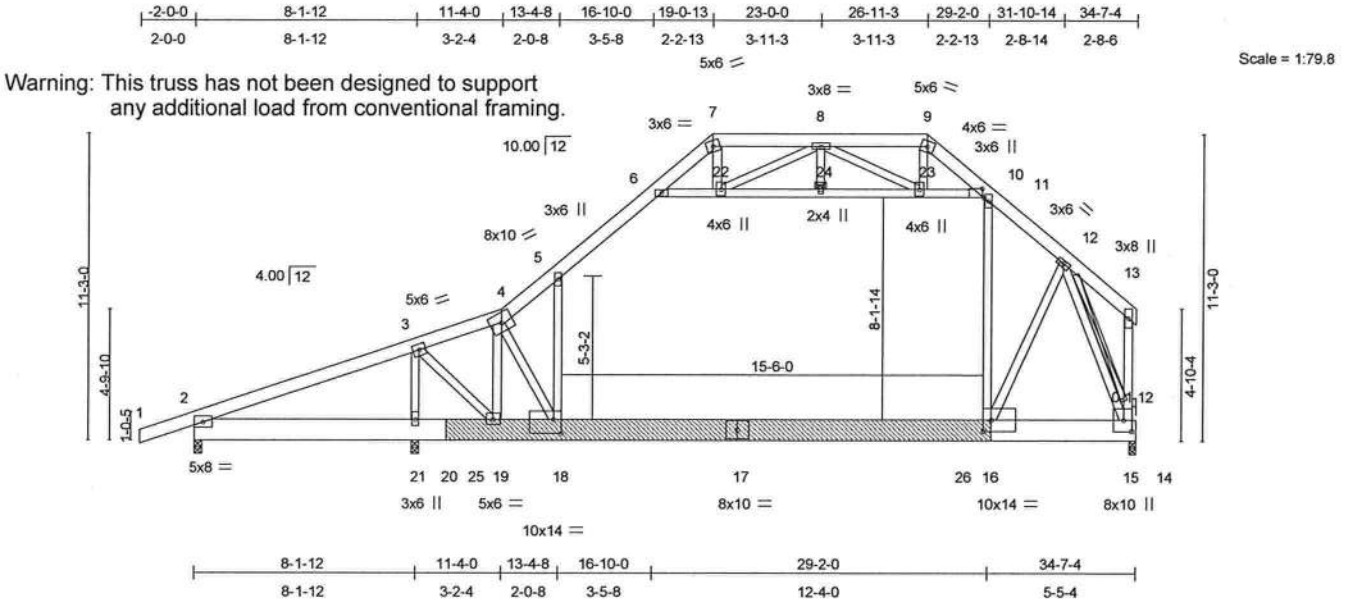

Plate Offsets (X,Y): [10:0-0-10,0-4-0], [15:0-5-0,0-3-8], [16:0-3-8,0-5-0], [18:0-3-8,0-6-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.60	Vert(LL)	-0.23	16-18	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.67	Vert(TL)	-0.37	16-18	>851	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.70	Horz(TL)	0.01	15	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 416 lb	

LUMBER

TOP CHORD	2 X 6 SYP No.1D
BOT CHORD	2 X 10 SYP No.2
WEBS	2 X 4 SYP No.3 *Except* 6-10 2 X 4 SYP No.2
LBR SCAB	16-20 2 X 10 SYP No.2 one side

BRACING

TOP CHORD	Structural wood sheathing directly applied or 5-8-5 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 7-9.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS	1 Row at midpt 6-10 T-Brace: 2 X 4 SYP No.3 - 12-15

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=-164/0-3-8, 21=2774/0-3-8, 15=1929/0-3-0
Max Horz 2=301(load case 5)
Max Uplift 2=-503(load case 4), 21=-161(load case 5), 15=-185(load case 4)
Max Grav 2=20(load case 2), 21=2778(load case 11), 15=1929(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/14, 2-3=-72/1392, 3-4=-381/267, 4-5=-1592/153, 5-6=-1250/340, 6-7=-404/277, 7-8=-179/281, 8-9=-451/231, 9-10=-539/257, 10-11=-1042/354, 11-12=-1463/309, 12-13=-88/76, 13-15=-112/86
BOT CHORD	2-21=-1223/0, 20-21=-1223/0, 20-25=-1223/0, 19-25=-1223/0, 18-19=-134/413, 17-18=-101/1068, 17-26=-101/1068, 16-26=-101/1068, 15-16=-99/717, 14-15=0/0
WEBS	3-21=-2421/267, 3-19=0/2071, 4-19=-2480/0, 5-18=-143/689, 11-16=-21/607, 12-16=-70/893, 12-15=-1983/253, 6-22=-985/108, 22-24=-604/36, 23-24=-604/36,

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

January 9,2008

Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES. J1923061
L264794	T07	ROOF TRUSS	2	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.27, 3 = 0.72, 4 = 0.57, 5 = 0.23, 6 = 0.34, 7 = 0.55, 8 = 0.56, 9 = 0.20, 10 = 0.18, 11 = 0.21, 12 = 0.71, 13 = 0.12, 15 = 0.28, 16 = 0.22, 16 = 0.00, 17 = 0.42, 17 = 0.00, 18 = 0.34, 18 = 0.00, 19 = 0.67, 19 = 0.00, 20 = 0.00, 20 = 0.00, 21 = 0.38, 22 = 0.32, 23 = 0.32 and 24 = 0.33

NOTES

1) Attached 20-0-0 scab 16 to 20, front face(s) 2 X 10 SYP No.2 with 2 row(s) of 10d (0.131"x3") nails spaced 9" o.c.except : starting at 0-0-0 from end at joint 20, nail 2 row(s) at 7 o.c. for 2-10-8; starting at 3-0-12 from end at joint 20, nail 2 row(s) at 4 o.c. for 2-0-0; starting at 17-10-4 from end at joint 20, nail 2 row(s) at 7 o.c. for 2-0-0.
2) Unbalanced roof live loads have been considered for this design.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
4) Provide adequate drainage to prevent water ponding.
5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) Ceiling dead load (5.0 psf) on member(s). 4-5, 5-6, 10-11, 6-22, 22-24, 23-24, 10-23; Wall dead load (5.0psf) on member(s).5-18, 11-16
7) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 16-18
8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 503 lb uplift at joint 2, 161 lb uplift at joint 21 and 185 lb uplift at joint 15.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54, 4-6=-64, 6-7=-54, 7-9=-54, 9-10=-54, 10-11=-64, 11-13=-54, 2-18=-10, 16-18=-110, 15-16=-70(F=-60), 14-15=-60(F), 6-10=-10
   Drag: 5-18=-10, 11-16=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

January 9,2008

Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES. J1923062
L264794	T07A	ROOF TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Sat Jan 05 19:28:36 2008 Page 1

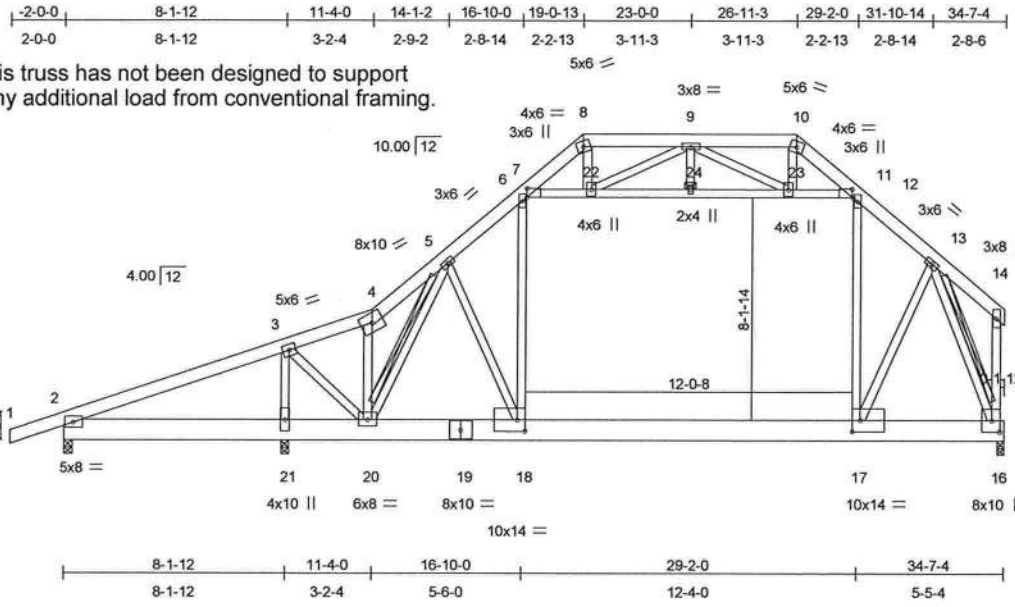

Plate Offsets (X,Y): [7:0-0-2,0-4-0], [11:0-0-6,0-4-0], [16:0-5-0,0-3-8], [17:0-3-8,0-5-0], [18:0-3-8,0-5-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.16	17-18	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.77	Vert(TL)	-0.26	17-18	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.65	Horz(TL)	0.01	16	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 354 lb	

LUMBER

TOP CHORD	2 X 6 SYP No.1D
BOT CHORD	2 X 10 SYP No.2
WEBS	2 X 4 SYP No.3 *Except* 7-11 2 X 4 SYP No.2

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 8-10.	
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.	
WEBS	1 Row at midpt	7-11
	T-Brace:	2 X 4 SYP No.3 - 5-20, 13-16

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=-143/0-3-8, 21=2732/0-3-8, 16=1924/0-3-0
Max Horz 2=304(load case 5)
Max Uplift 2=-396(load case 4), 21=-680(load case 5), 16=-216(load case 4)
Max Grav 2=49(load case 5), 21=2763(load case 11), 16=1924(load case 1)

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

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Continued on page 2

Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES. J1923062
L264794	T07A	ROOF TRUSS	1	1	Job Reference (optional)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/14, 2-3=-578/1397, 3-4=-409/301, 4-5=-517/364, 5-6=-1574/379, 6-7=-1106/369, 7-8=-601/268, 8-9=-494/259, 9-10=-356/216, 10-11=-525/246, 11-12=-1144/380, 12-13=-1459/346, 13-14=-107/78, 14-16=-143/89

BOT CHORD 2-21=-1230/302, 20-21=-1230/302, 19-20=-180/937, 18-19=-180/937, 17-18=-135/1130, 16-17=-118/721, 15-16=0/0

WEBS 3-21=-2374/935, 3-20=-586/2038, 4-20=-245/105, 5-20=-1553/169, 5-18=0/509, 6-18=-114/633, 12-17=-34/423, 13-17=-51/1041, 13-16=-1996/305, 7-22=-666/153, 22-24=-551/60, 23-24=-551/60, 11-23=-808/197, 8-22=-47/154, 10-23=-58/228, 9-24=0/41, 9-22=-241/142, 9-23=-395/162

JOINT STRESS INDEX

2 = 0.29, 3 = 0.70, 4 = 0.11, 5 = 0.55, 6 = 0.20, 7 = 0.16, 8 = 0.21, 9 = 0.56, 10 = 0.20, 11 = 0.20, 12 = 0.15, 13 = 0.76, 14 = 0.12, 16 = 0.29, 17 = 0.22, 18 = 0.17, 19 = 0.17, 20 = 0.74, 21 = 0.28, 22 = 0.32, 23 = 0.32 and 24 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Ceiling dead load (5.0 psf) on member(s). 6-7, 11-12, 7-22, 22-24, 23-24, 11-23; Wall dead load (5.0psf) on member(s).6-18, 12-17
6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 17-18
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 396 lb uplift at joint 2, 680 lb uplift at joint 21 and 216 lb uplift at joint 16.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-54, 4-6=-54, 6-7=-64, 7-8=-54, 8-10=-54, 10-11=-54, 11-12=-64, 12-14=-54, 2-20=-10, 18-20=-70(F=-60), 17-18=-110, 16-17=-70(F=-60), 15-16=-60(F), 7-11=-10
   Drag: 6-18=-10, 12-17=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

January 9,2008

Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES.
L264794	T08	ROOF TRUSS	1	2	J1923063
					Job Reference (optional)

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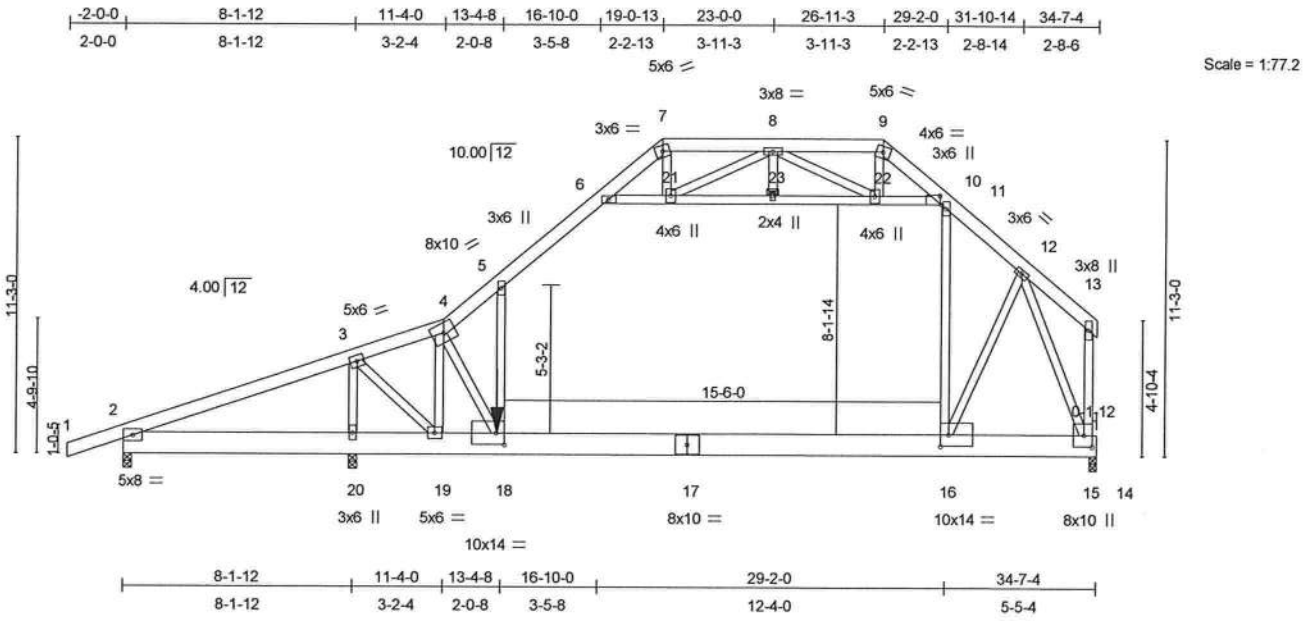

Plate Offsets (X,Y): [10:0-0-10,0-4-0], [15:0-5-0,0-3-8], [16:0-3-8,0-5-0], [18:0-3-8,0-5-0]

LOADING (psf)		SPACING	2-6-2	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	-0.21	16-18	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.68	Vert(TL)	-0.33	16-18	>953	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.45	Horz(TL)	-0.00	20	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 674 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 10 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
13-15 2 X 4 SYP No.2, 6-10 2 X 4 SYP No.2

BRACING
TOP CHORD 2-0-0 oc purlins (6-0-0 max.), except end verticals (Switched from sheeted: Spacing > 2-0-0).
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 6-10
JOINTS 1 Brace at Jt(s): 4, 7, 9, 13

REACTIONS (lb/size) 2=-268/0-3-8, 20=3684/0-3-8, 15=2362/0-3-0
Max Horz 2=378(load case 4)
Max Uplift 2=-566(load case 3), 20=-440(load case 4), 15=-175(load case 3)
Max Grav 20=3701(load case 10), 15=2362(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/18, 2-3=-361/1921, 3-4=-426/309, 4-5=-2099/176, 5-6=-1617/149, 6-7=-489/308, 7-8=-196/369, 8-9=-572/214, 9-10=-676/214, 10-11=-1347/118, 11-12=-1914/41, 12-13=-107/59, 13-15=-140/60
BOT CHORD 2-20=-1698/5, 19-20=-1698/5, 18-19=-65/502, 17-18=0/1397, 16-17=0/1397, 15-16=0/936, 14-15=0/0
WEBS 3-20=-3170/226, 3-19=0/2721, 4-19=-3431/0, 5-18=-170/955, 11-16=-1/845, 12-16=-74/1174, 12-15=-2601/0, 6-21=-1338/65, 21-23=-832/32, 22-23=-832/32, 10-22=-874/59, 7-21=-62/399, 9-22=-100/170, 8-23=0/50, 8-21=-722/186, 8-22=-249/268, 4-18=-61/2193

Julius Lee
Truss Design Engineer
Florida PE# No. 34869
1306 Coastal Bay Blvd
Boynton Beach, FL 33436

JOINT STRESS INDEX
2 = 0.19, 3 = 0.48, 4 = 0.40, 5 = 0.16, 6 = 0.24, 7 = 0.39, 8 = 0.57, 9 = 0.21, 10 = 0.19, 11 = 0.16, 12 = 0.48, 13 = 0.12, 15 = 0.19, 16 = 0.15, 17 = 0.45, 18 = 0.22, 19 = 0.45, 20 = 0.26, 21 = 0.33, 22 = 0.33 and 23 = 0.34

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Continued on page 2

Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES.
L264794	T08	ROOF TRUSS	1	2	J1923063
					Job Reference (optional)

NOTES

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
   Top chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc, 2 X 4 - 1 row at 0-9-0 oc.
   Bottom chords connected as follows: 2 X 10 - 2 rows at 0-9-0 oc.
   Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) Ceiling dead load (5.0 psf) on member(s). 4-5, 5-6, 10-11, 6-21, 21-23, 22-23, 10-22; Wall dead load (5.0psf) on member(s).5-18, 11-16
8) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 16-18
9) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 566 lb uplift at joint 2, 440 lb uplift at joint 20 and 175 lb uplift at joint 15.

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-68, 4-6=-80, 6-7=-68, 7-9=-68, 9-10=-68, 10-11=-80, 11-13=-68, 2-18=-13, 16-18=-138, 14-16=-73(F=-60), 6-10=-13
   Drag: 5-18=-13, 11-16=-13
   Concentrated Loads (lb)
   Vert: 18=-161(F)

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd.
Boynton Beach, FL 33435

January 9,2008

Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES.
L264794	T09	ROOF TRUSS	1	2	J1923064
					Job Reference (optional)

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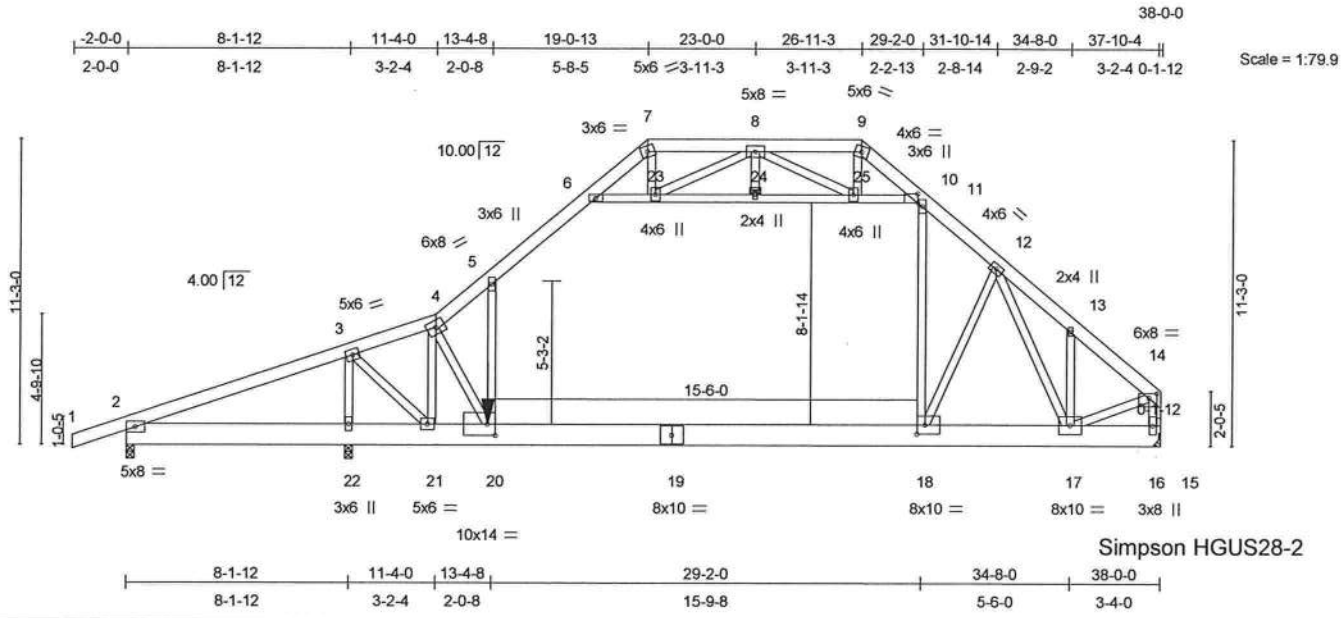

Plate Offsets (X,Y): [10:0-0-2,0-4-0], [14:0-3-8,0-2-0], [18:0-3-8,0-4-0], [20:0-3-8,0-5-0]

LOADING (psf)		SPACING	2-6-2	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.67	Vert(LL)	-0.25	18-20	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.83	Vert(TL)	-0.41	18-20	>862	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.43	Horz(TL)	0.01	16	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 733 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 10 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
14-16 2 X 4 SYP No.2, 6-10 2 X 4 SYP No.2

BRACING
TOP CHORD 2-0-0 oc purlins (6-0-0 max.), except end verticals (Switched from sheeted: Spacing > 2-0-0).
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 2-22,21-22.
WEBS 1 Row at midpt 6-10
JOINTS 1 Brace at Jt(s): 4, 7, 9, 14

REACTIONS (lb/size) 2=74/0-3-8, 22=3533/0-3-8, 16=2447/Mechanical
Max Horz 2=383(load case 4)
Max Uplift 2=-603(load case 3), 22=-423(load case 4), 16=-206(load case 3)
Max Grav 2=141(load case 2), 22=3572(load case 10), 16=2447(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/18, 2-3=-324/1162, 3-4=-1243/400, 4-5=-3158/294, 5-6=-2184/213, 6-7=-304/304, 7-8=0/468, 8-9=-738/234, 9-10=-743/222, 10-11=-1773/166, 11-12=-2934/159, 12-13=-2127/344, 13-14=-2177/265, 14-16=-2231/271
BOT CHORD 2-22=-991/0, 21-22=-991/0, 20-21=-60/1273, 19-20=-10/2043, 18-19=-10/2043, 17-18=-16/2025, 16-17=-26/148, 15-16=0/0
WEBS 3-22=-3038/211, 3-21=0/2661, 4-21=-3748/0, 5-20=-255/1683, 11-18=-100/1629, 12-18=-64/340, 12-17=-1070/0, 13-17=-128/139, 14-17=-142/1646, 6-23=-2351/178, 23-24=-1405/109, 24-25=-1405/109, 10-25=-1344/100, 7-23=-83/578, 8-24=0/51, 9-25=-100/73, 8-23=-1075/228, 8-25=-34/269, 4-20=-26/1887

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd.
Boynton Beach, FL 33435

JOINT STRESS INDEX
2 = 0.17, 3 = 0.47, 4 = 0.60, 5 = 0.27, 6 = 0.41, 7 = 0.66, 8 = 0.28, 9 = 0.22, 10 = 0.16, 11 = 0.26, 12 = 0.34, 13 = 0.34, 14 = 0.22, 16 = 0.19, 17 = 0.23, 18 = 0.16, 19 = 0.44, 20 = 0.26, 21 = 0.44, 22 = 0.25, 23 = 0.33, 24 = 0.34 and 25 = 0.33

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Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES. J1923064
L264794	T09	ROOF TRUSS	1	2	Job Reference (optional)

NOTES

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
   Top chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc, 2 X 4 - 1 row at 0-9-0 oc.
   Bottom chords connected as follows: 2 X 10 - 2 rows at 0-9-0 oc.
   Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) Ceiling dead load (5.0 psf) on member(s). 4-5, 5-6, 10-11, 6-23, 23-24, 24-25, 10-25; Wall dead load (5.0psf) on member(s).5-20, 11-18
8) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 18-20
9) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 603 lb uplift at joint 2, 423 lb uplift at joint 22 and 206 lb uplift at joint 16.

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard

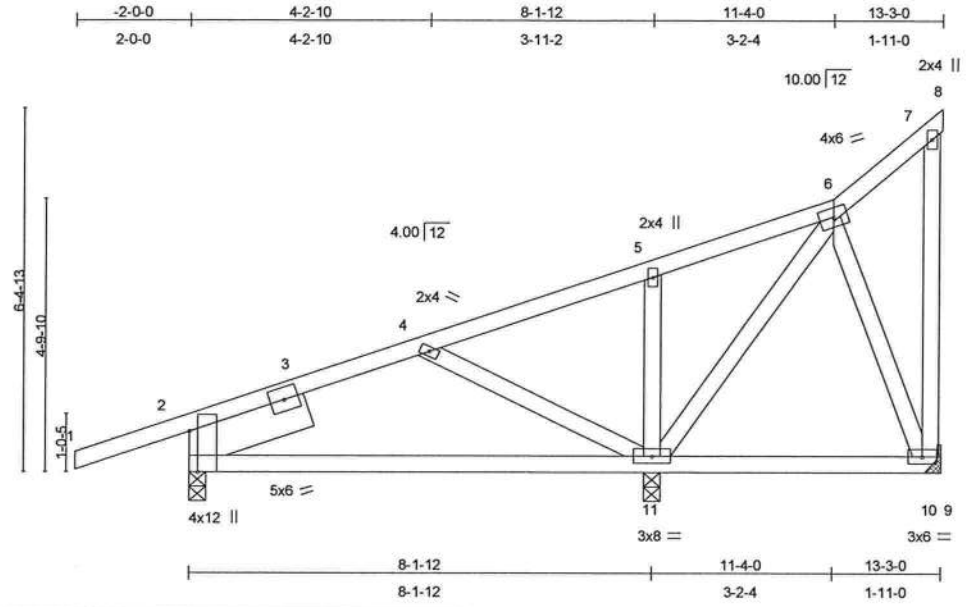
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-68, 4-6=-80, 6-7=-68, 7-9=-68, 9-10=-68, 10-11=-80, 11-14=-68, 2-20=-13, 18-20=-138, 17-18=-73(F=-60), 15-17=-13, 6-10=-13
   Drag: 5-20=-13, 11-18=-13
   Concentrated Loads (lb)
   Vert: 20=-161(F)

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

January 9,2008

Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES. J1923065
L264794	T10	PORCH TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Sat Jan 05 19:28:39 2008 Page 1



Simpson HTU26

Plate Offsets (X,Y): [2:0-8-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	0.21	2-11	>460	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.36	Vert(TL)	-0.11	2-11	>925	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.09	Horz(TL)	-0.01	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 86 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 8 SYP No.1D 2-3-1

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-1-14 oc bracing.

REACTIONS (lb/size) 10=95/Mechanical, 2=342/0-3-8, 11=511/0-3-8
Max Horz 2=191(load case 6)
Max Uplift 10=-75(load case 6), 2=-280(load case 4), 11=-308(load case 4)
Max Grav 10=99(load case 2), 2=342(load case 1), 11=511(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-3/0, 2-3=-244/137, 3-4=-155/170, 4-5=-93/100, 5-6=-34/86, 6-7=-41/20, 7-8=-2/0, 7-10=-35/36
BOT CHORD 2-11=-407/164, 10-11=-44/17, 9-10=0/0
WEBS 4-11=-251/372, 5-11=-217/189, 6-10=-45/153, 6-11=-134/4

JOINT STRESS INDEX
2 = 0.78, 2 = 0.23, 3 = 0.00, 4 = 0.18, 5 = 0.10, 6 = 0.16, 7 = 0.18, 10 = 0.37 and 11 = 0.27

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Julius Lee
Truss Design Engineer
Florida PE No. 34869
105 Crystal Bay Blvd
Boynton Beach, FL 33435

January 9,2008

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	NORTON - RAINES RES. J1923065
L264794	T10	PORCH TRUSS	1	1	Job Reference (optional)

NOTES

2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 75 lb uplift at joint 10, 280 lb uplift at joint 2 and 308 lb uplift at joint 11.

LOAD CASE(S) Standard

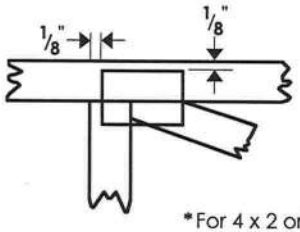
Julius Lee
Truss Design Engineer
Florida PE No. 34869
1300 Congress Way Blvd
Boynton Beach, FL 33435

January 9,2008

Symbols

PLATE LOCATION AND ORIENTATION

*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

*This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

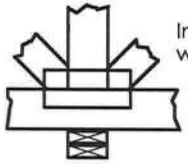
4 x 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

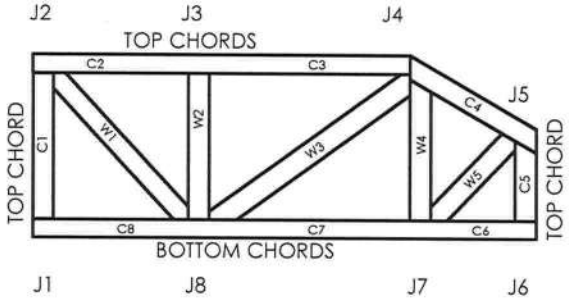
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
NER	561

MiTek®

TEE-LOK®

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"
		#3	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"	12' 11"
		STUD	3' 3"	4' 11"	4' 11"	6' 5"	6' 5"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"	12' 11"
		STANDARD	3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	8' 8"	8' 8"	11' 8"	11' 8"
	SP DFL	#1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#2	3' 7"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#3	3' 6"	5' 0"	5' 0"	6' 8"	6' 8"	8' 3"	8' 8"	10' 4"	10' 4"	12' 11"	13' 7"
		STUD	3' 6"	5' 0"	5' 0"	6' 7"	6' 7"	8' 3"	8' 8"	10' 3"	10' 3"	12' 11"	13' 7"
		STANDARD	3' 4"	4' 3"	4' 3"	5' 8"	5' 8"	7' 8"	7' 8"	8' 10"	8' 10"	12' 0"	12' 0"
16" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"
		#3	3' 9"	6' 0"	8' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	5' 2"	6' 10"	6' 10"	9' 2"	9' 2"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 5"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 3"	7' 4"	7' 7"	8' 9"	8' 11"	10' 5"	10' 8"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STUD	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STANDARD	4' 2"	6' 11"	6' 11"	7' 10"	7' 10"	10' 5"	10' 5"	12' 3"	12' 3"	14' 0"	14' 0"
	SP DFL	#1	4' 8"	7' 4"	7' 11"	8' 9"	8' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#2	4' 7"	7' 4"	7' 11"	8' 9"	9' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 2"	7' 2"	8' 9"	9' 2"	10' 5"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 1"	7' 1"	8' 9"	9' 2"	10' 5"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 3"	6' 1"	6' 1"	8' 0"	8' 0"	10' 5"	10' 8"	12' 6"	12' 6"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

SOUTHERN PINE
#3
STUD
STANDARD

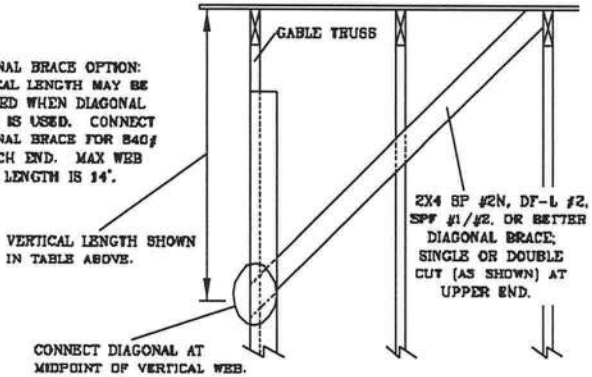
GROUP B:

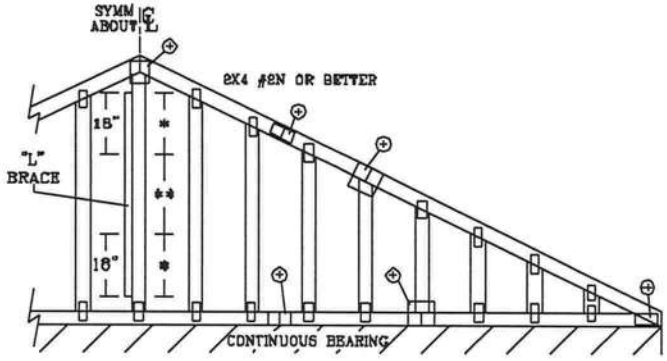
HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#3
#2

DOUGLAS FIR-LARCH
#1
#2

DIAGONAL BRACE OPTION: VERTICAL LENGTH MAY BE DOUBLED WHEN DIAGONAL BRACE IS USED. CONNECT DIAGONAL BRACE FOR 840# AT EACH END. MAX WEB TOTAL LENGTH IS 14'.





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 136 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 8" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 8"	2X4
GREATER THAN 11' 8"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34859
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF ASCE7-02-GAB13015
DATE 11/26/03
DRWG MITEK STD GABLE 15 E HT
-ENG

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 2"	5' 6"	5' 6"	6' 6"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"
		#3	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STUD	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STANDARD	2' 11"	3' 9"	3' 9"	5' 0"	5' 0"	6' 9"	6' 9"	7' 10"	7' 10"	10' 7"	10' 7"
	SP DFL	#1	3' 6"	5' 6"	5' 11"	6' 6"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#2	3' 5"	5' 6"	5' 11"	6' 6"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#3	3' 3"	4' 6"	4' 6"	6' 0"	6' 0"	7' 10"	8' 1"	9' 4"	9' 4"	12' 3"	12' 6"
		STUD	3' 3"	4' 6"	4' 6"	5' 11"	5' 11"	7' 10"	8' 0"	9' 3"	9' 3"	12' 3"	12' 6"
		STANDARD	3' 0"	3' 10"	3' 10"	5' 1"	5' 1"	6' 11"	6' 11"	8' 0"	8' 0"	10' 10"	10' 10"
16" O.C.	SPF HF	#1 / #2	3' 8"	6' 4"	6' 6"	7' 6"	7' 8"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"
		#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"
		STUD	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"
		STANDARD	3' 7"	4' 8"	4' 8"	6' 2"	6' 2"	8' 3"	8' 3"	9' 7"	9' 7"	12' 11"	12' 11"
	SP DFL	#1	4' 0"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#2	3' 11"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#3	3' 9"	5' 7"	5' 7"	7' 4"	7' 4"	8' 11"	9' 5"	11' 5"	11' 6"	14' 0"	14' 0"
		STUD	3' 9"	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	9' 5"	11' 4"	11' 4"	14' 0"	14' 0"
		STANDARD	3' 8"	4' 9"	4' 9"	6' 3"	6' 3"	8' 5"	8' 5"	9' 9"	9' 9"	13' 3"	13' 3"
12" O.C.	SPF HF	#1 / #2	4' 0"	6' 11"	7' 2"	8' 3"	8' 6"	9' 10"	10' 1"	12' 11"	13' 4"	14' 0"	14' 0"
		#3	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	12' 11"	14' 0"	14' 0"
		STUD	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 10"	12' 10"	14' 0"	14' 0"
		STANDARD	3' 11"	5' 4"	5' 4"	7' 1"	7' 1"	9' 6"	9' 6"	11' 1"	11' 1"	14' 0"	14' 0"
	SP DFL	#1	4' 5"	6' 11"	7' 6"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#2	4' 4"	6' 11"	7' 6"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#3	4' 2"	6' 5"	6' 5"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 3"	14' 0"	14' 0"
		STUD	4' 2"	6' 4"	6' 4"	8' 3"	8' 5"	9' 10"	10' 4"	12' 11"	13' 1"	14' 0"	14' 0"
		STANDARD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

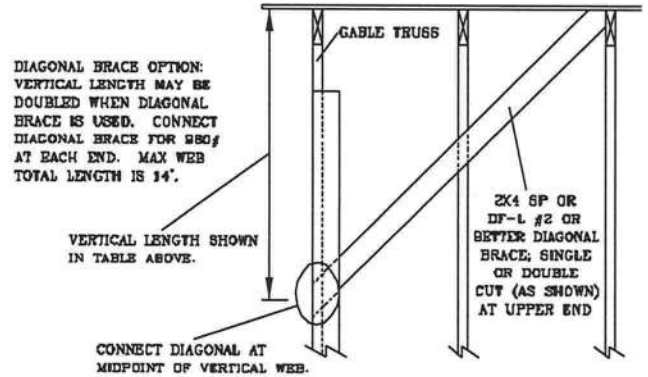
SOUTHERN PINE
#3
STUD
STANDARD

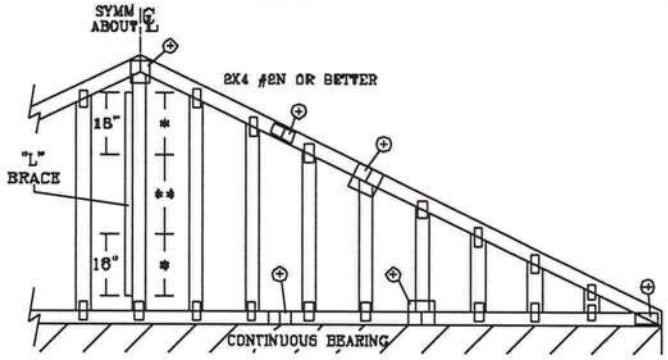
GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#1
#2

DOUGLAS FIR-LARCH
#1
#2





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 180 PLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

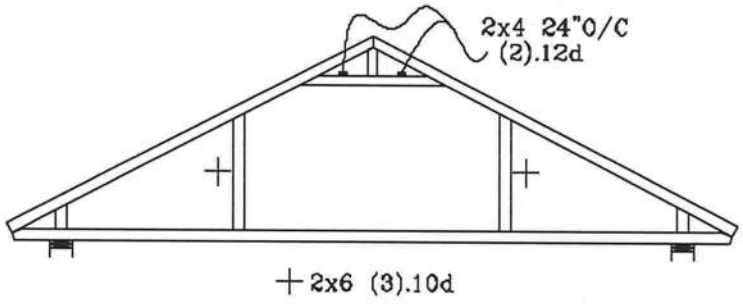
JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161
No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

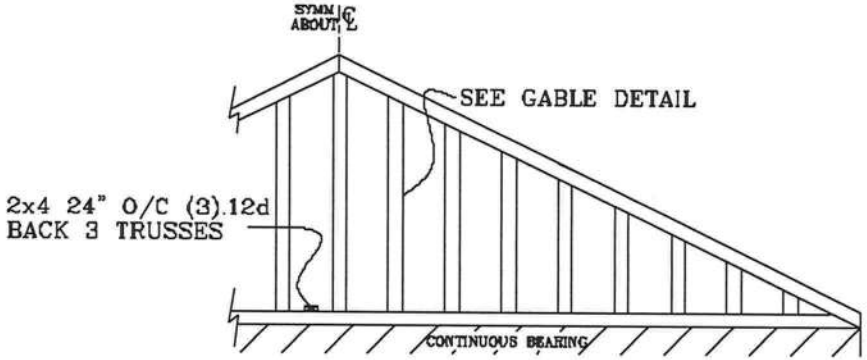
MAX. SPACING 24.0"

REF ASCE7-02-GAB13030
DATE 11/26/03
DWG MITEK STD GABLE 30' E H7
-ENG

TYPICAL ATTIC TRUSS BRACING

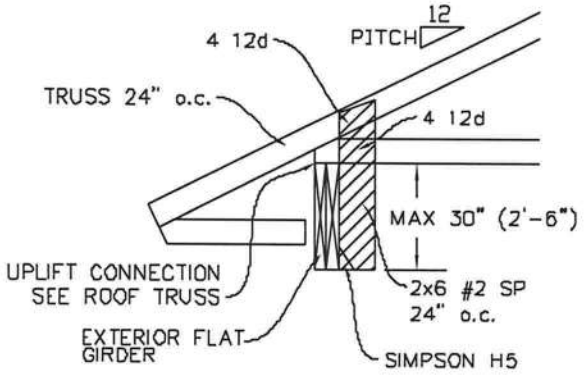


GABLE END TRUSS DETAIL

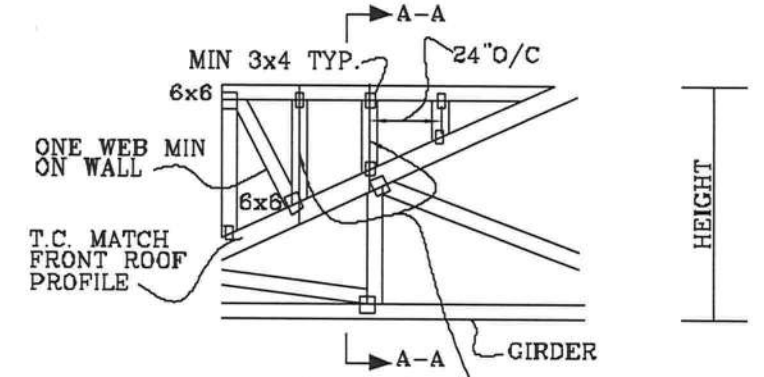


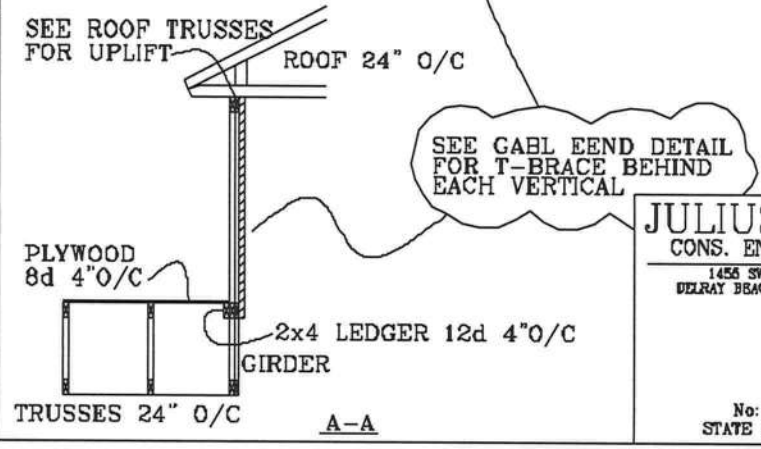
MINIMUM BC BRACING ON GABLE TRUSS. OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR EOR

TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS



TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL





JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

PIGGYBACK DETAIL

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

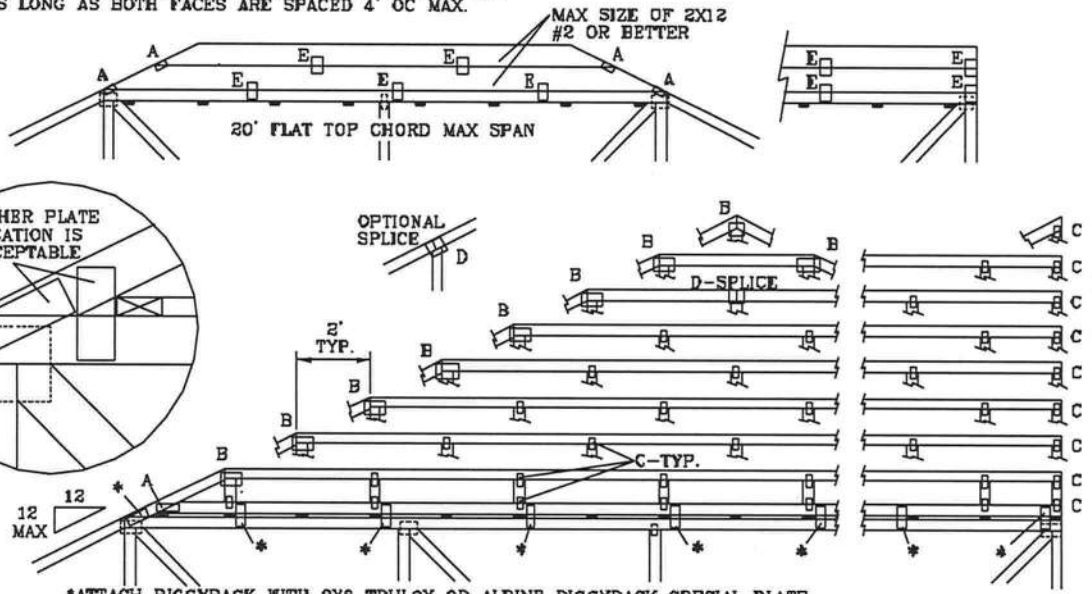
ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

- 110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF
- 110 MPH WIND, 30' MEAN HGT, FBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5 PSF, WIND BC DL=5 PSF
- 130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.



*ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.

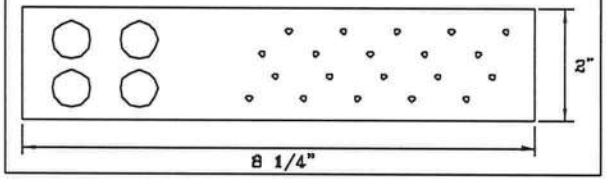
JOINT TYPE	SPANS UP TO			
	30'	34'	38'	52'
A	2X4	2.5X4	2.5X4	3X5
B	4X6	5X6	5X6	5X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X5	5X5	5X6
E	4X6 OR 3X6 TRULOX AT 4' OC, ROTATED VERTICALLY			

ATTACH TRULOX PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

WEB BRACING CHART	
WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4" OC.
10' TO 14'	2x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4" OC.

* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045

xxWARNINGxx TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

No: 34869
STATE OF FLORIDA

MAX LOADING

55 PSF AT 1.33 DUR. FAC.

50 PSF AT 1.25 DUR. FAC.

47 PSF AT 1.15 DUR. FAC.

SPACING 24.0"

REF PIGGYBACK
DATE 09/12/07
DRWG MITEK STD PIGGY
-ENG JL

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

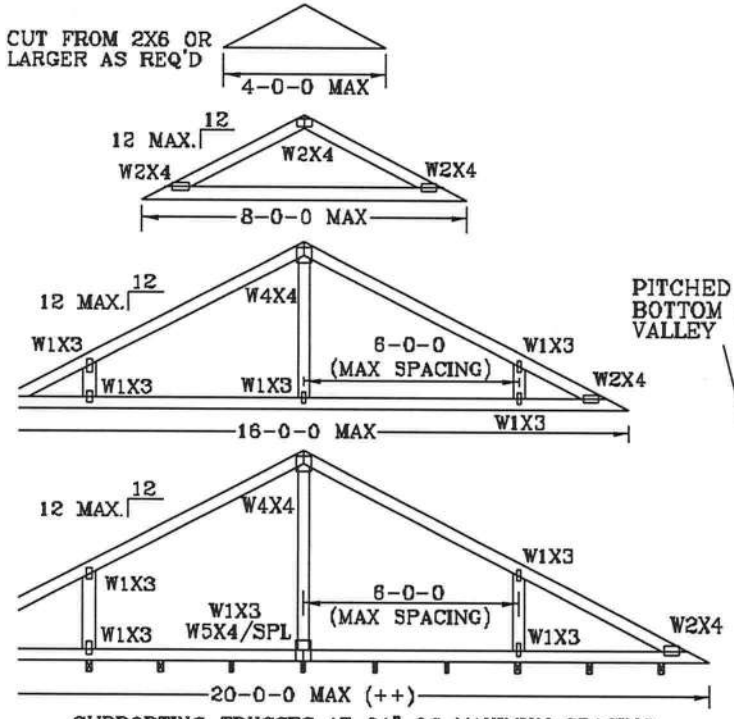
MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION
OR
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN
OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

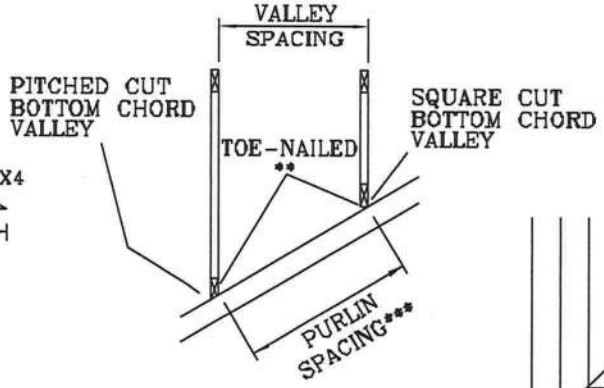
*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

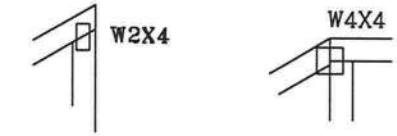
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

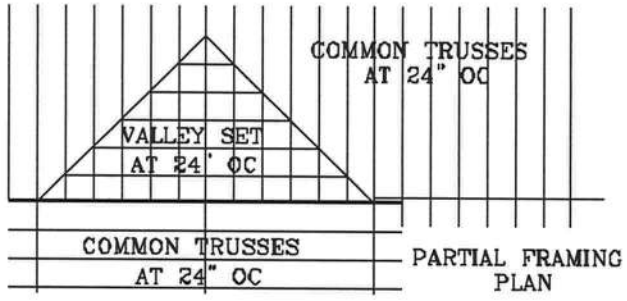
BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.



SUPPORTING TRUSSES AT 24" OC MAXIMUM SPACING.







THIS DRAWING REPLACES DRAWING A105

xxWARNINGxx TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

No: 34669
STATE OF FLORIDA

TC LL	20	20	PSF	REF VALLEY DETAIL
TC DL	7	15	PSF	DATE 11/26/03
BC DL	5	5	PSF	DRWG VALTRUSS1103
BC LL	0	0	PSF	-ENG JL
TOT. LD.	32	40	PSF	
DUR.FAC.	1.25	1.25		
SPACING		24"		

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

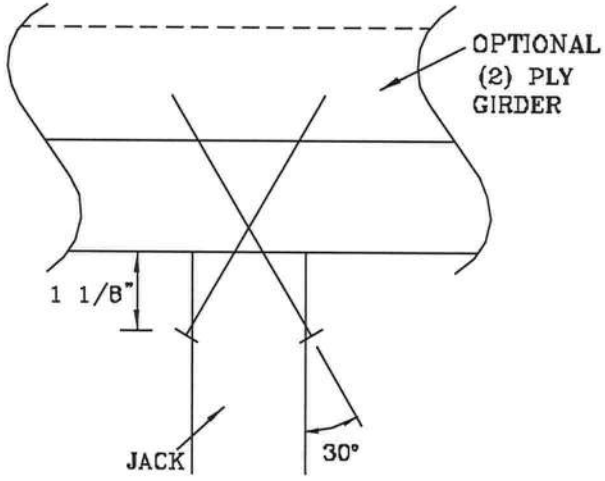
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

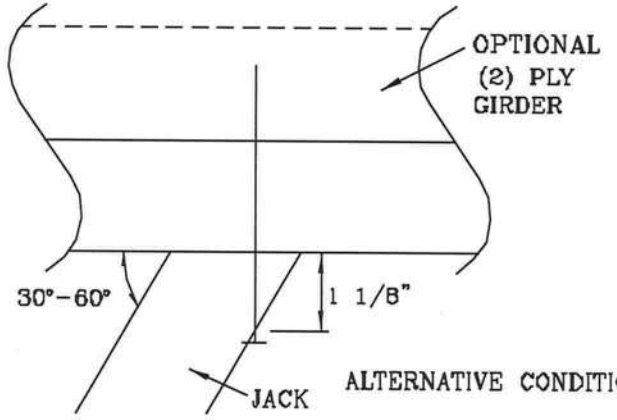
THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES
2	197#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.





ALTERNATIVE CONDITION

THIS DRAWING REPLACES DRAWING 784040

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34669
STATE OF FLORIDA

TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONAIL1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

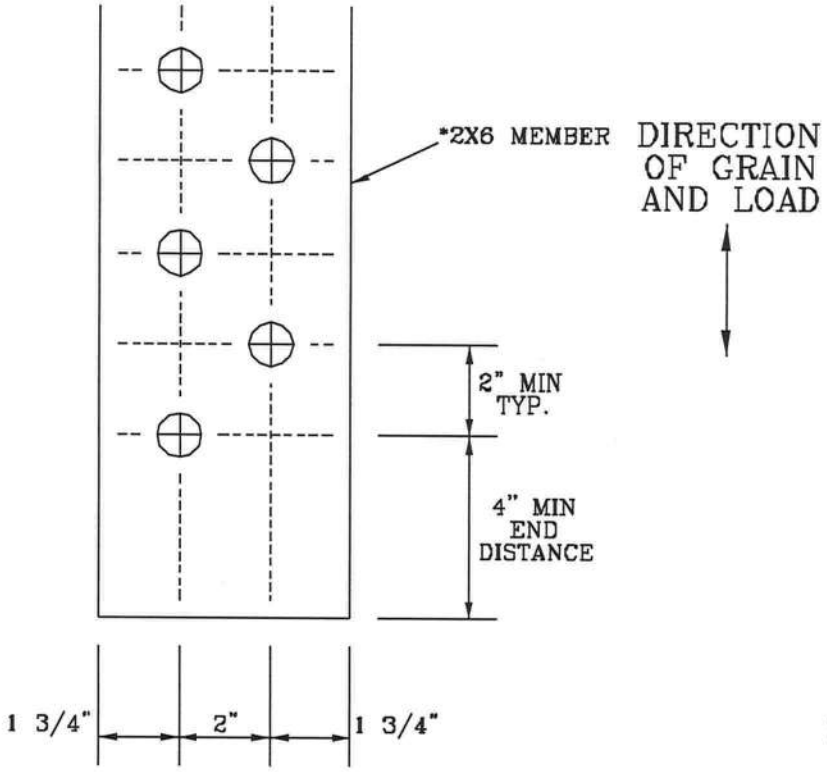
1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.

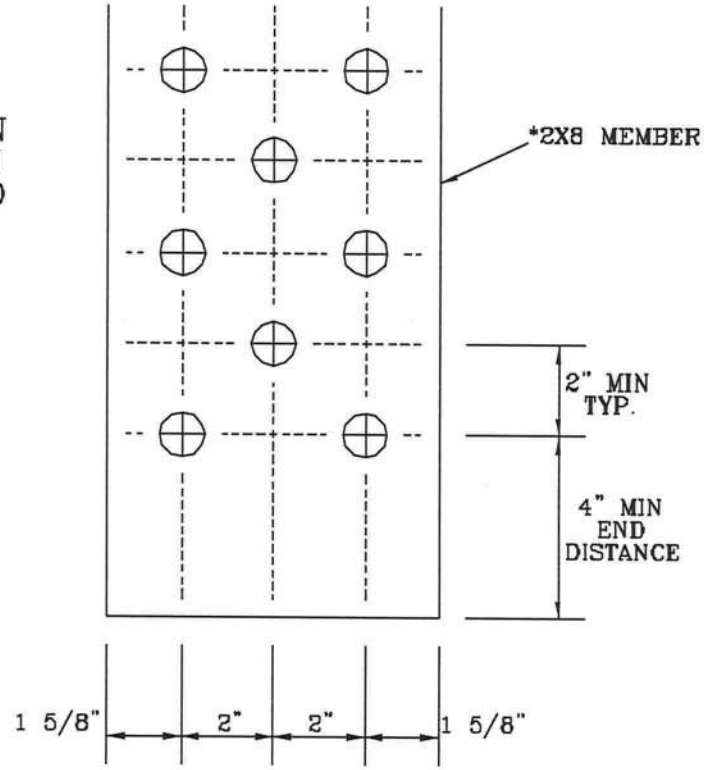
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL



2X8 DETAIL

THIS DRAWING REPLACES DRAWING A628.016

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
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1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	BOLT SPACING
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNBOLTSP1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

TRULOX CONNECTION DETAIL

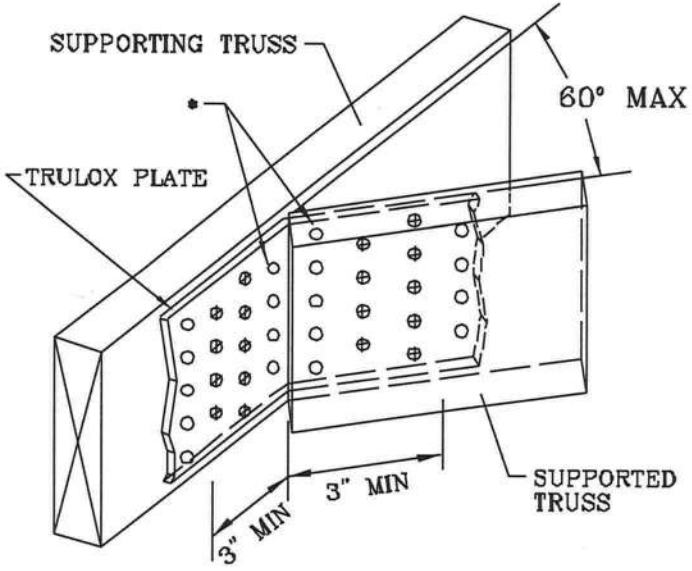
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (⊕).

* NAILS MAY BE OMITTED FROM THESE ROWS.

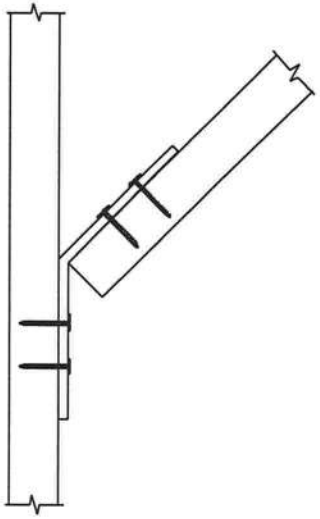
THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

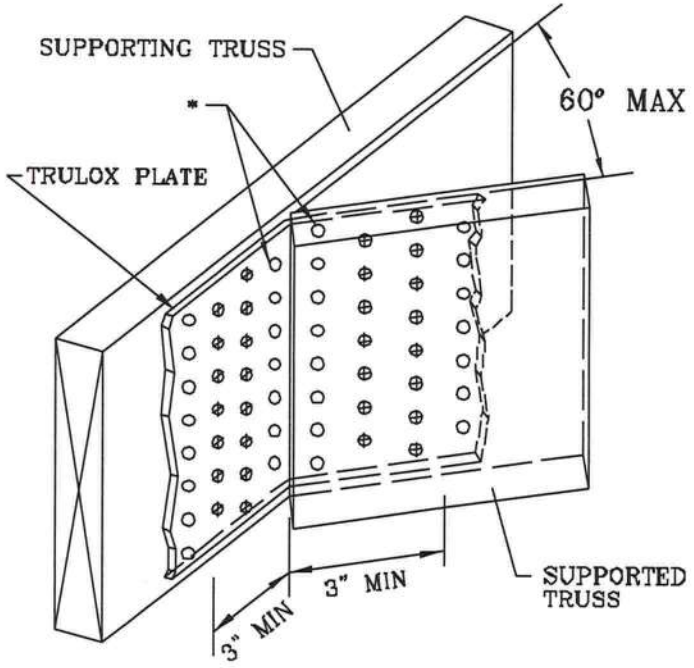
TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



MINIMUM 3X6 TRULOX PLATE





MINIMUM 5X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
5X6	15	990#

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R 1,154,944 1,152,217 1,152,017 1,159,154 & 1,151,524

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
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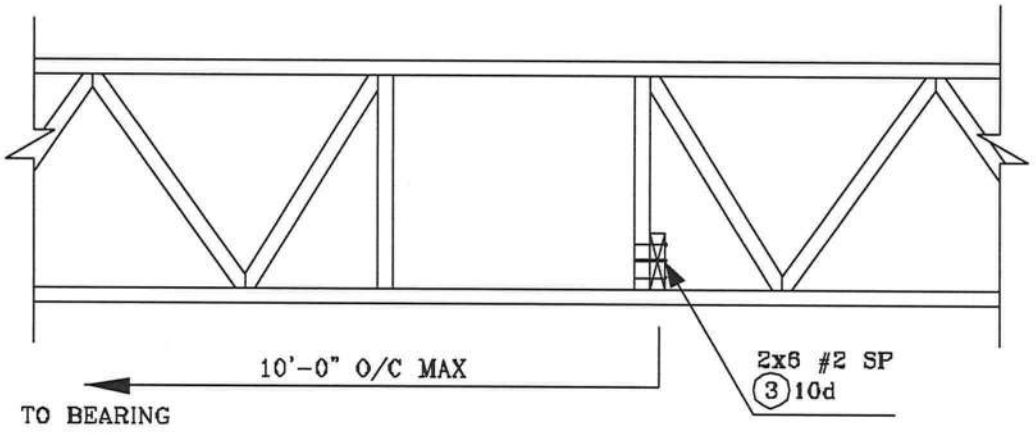
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

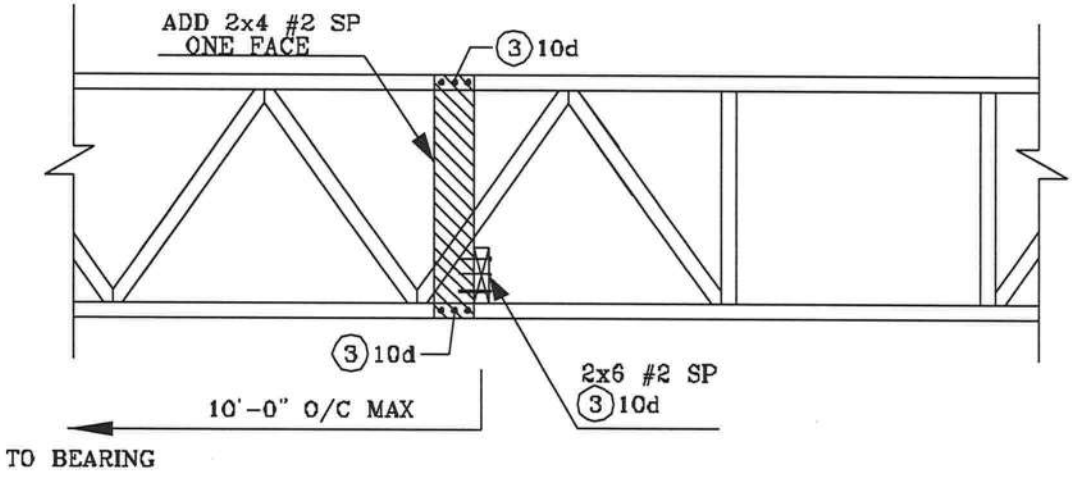
No: 34869
STATE OF FLORIDA

REF	TRULOX
DATE	11/26/03
DRWG	CNTRULOX1103
-ENG	JL

STRONG BACK DETAIL
SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR
STRONG BACK WITH VERTICAL
NOT LINING UP



JULIUS LEE'S
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1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

Notice of Prevention for Subterranean Termites

(As required by Florida Building Code (FBC) 104.2.6)

Live Oak PEST CONTROL, INC.
A locally owned company serving you since 1972

17856 U.S. 129 • McALPIN, FLORIDA 32062
(386) 362-3887 • 1-800-771-3887 • Fax: (386) 364-3529

0000026659

Raines / 220 SE Myrtle Dotch Terr. Lic. FL.
Address of Treatment or Lot/Block of Treatment

3/10/08
Date

8:51 AM
Time

Darrel B.
Applicator

Prowler
Product Used

Imidacloprid
Chemical used *(active ingredient)*

422
Number of gallons applied

.05%
Percent Concentration

2382
Area treated *(square feet)*

317
Linear feet treated

Horizontal - Vertical
Stage of treatment *(Horizontal, Vertical, Adjoining Slab, retreat of disturbed area)*

As per 104.2.6 - If soil chemical barrier method for Subterranean termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial and date this line. _____