

4'-0" MAX SPACING

* ATTACHMENT CAN BE MADE DIRECTLY OR THROUGH PLYWOOD SHEATHING

NOT REQUIRED IF SLEEPERS ARE USED)

BY CUTTING A 2" x 4" NOTCH IN SHEATHING

BEVEIL RAFTER CUT AS REQ'D FOR PITCH

VALLEY ROOF PLAN

RETROFIT ROOF OVER FRAMING & BRACING DETAIL

CONNECTION REQUIREMENT NOTES

RIDGE BOARD TO ROOF BLOCK

PURLIN TO TRUSS (IF CRIPPLE IS ATTACHED TO PURLIN)

6'-0" FOR 2X4, 9'-0" FOR 2X6 SPF #2 OR SYP #2.

IIMUM OF 8 - 8d COMMON WIRE NAILS.

MAXIMUM VALLEY HEIGHT: 14'-0" OR LESS

EXPOSURE CATEGORY "B", I = 1.0, Kzt = 1.0

CRIPPLE, BRACING, & BLOCKING NOTES

MAXIMUM TOTAL LOADING: 40 psf MEETS FBC 2001/ASCE 7-98 WIND REQUIREMENTS

NARROW EDGE OF CRIPPLE CAN FACE RIDGE OR RAFTER

INSTALL BLOCKING UNDER RAFTER IF SLEEPERS ARE NOT USED. INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FALL BETWEEN

AS LONG AS THE PROPER NUMBER OF NAILS ARE INSTALLED INTO RIDGE BOARD

MAXIMUM MEAN ROOF HEIGHT: 30 FEET

-MAXIMUM WIND SPEED: 120 MPH

VAILS UNLESS NOTED OTHERWISE

16ft2 IN ZONES 2 & 3 , 24ft2 IN ZONE 1. (EXAMPLE: 4'-0" O.C. X 4'-0" SPAN

PURLINS REQUIRED 2-0" SPAN = 16ft2)

PURLINS REQUIRED 2-0" O.C. IF EXISTING SHEATHING IS REMOVED.

PURLINS SHOULD OVERLAP SHEATHING ONE TRUSS SPACING MINIMUM.

IN CASES THAT THIS IS IMPRACTICAL, OVERLAP SHEATHING A MINIMUM.

OF 6", AND NAIL UPWARDS THROUGH SHEATHING INTO PURLIN WITH A

THIS DRAWING APPLIES TO VALLEYS WITH THE FOLLOWING CONDITIONS -SPANS (DISTANCS BETWEEN HEELS) 40'-0" OR LESS

2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 5'-0" TO 10'-0" LONG

NAILED w/ 2 - 10d NAILS OR 2X4 "T" OR SCAB BRACE NAILD TO FLAT EDGE OF CRIPPLE WITH 8d NAILS @ 8" O.C. "T" OR SCAB MUST BE 90% OF CRIPPLE LENGTH. CRIPPLES

OVER 10'-0" LONG REQURE TWO CLB's OR BOTH FACES w/ "T" OR SCAB. USE STRESS

LOWER TRUSS TOP CHORDS AND LATERAL BRACING IS NOT USED.

APPLY ALL NAILING IN ACCORDANCE TO NDS-1997 SECTION 12. NAILS ARE COMMON WIRE

ONE STORY WOOD FRAME

CRIPPLE TO RIDGE

SLEEPER TO TRUSS

RIDGE BOARD TO TRUSS

PURLIN TO TRUSS (TYP.

TRUSS TO BLOCKING

CRIPPLE TO TRUSS

CRIPPLE TO PURLIN

GENERAL NOTES

CRIPPLE TO RAFTERS

3 -16d OR 6 - .131 x 3" TOE NAILS

3 - 16d OR 6 - .131 x 3" FACE NAILS

3 - 16d OR 6 - .131 x 3" FACE NAILS

3 -16d OR 6 - .131 x 3" TOE NAILS

3 -16d OR 6 - .131 x 3" NAILS

4-16d OR 8 - .131 x 3" NAILS

3 -16d OR 6 - .131 x 3" TOE NAILS

3 -16d OR 6 - .131 x 3" END NAILS

3 -16d OR 6 - .131 x 3" FACE NAILS

3 -16d OR 6 - .131 x 3" FACE NAILS

4 - 16d OR 8 - .131 x 3" FACE NAILS FACH TRUS

RAFTER TO SLEEPER OR BLOCKING 6-16d OR 12 - .131 x 3" TOE NAILS

TRUSS BELOW

BOARD

SEE TRUSS DESIGN - SOUTHEREN PINE MATERIAL

- CS20 RIDGE TENSION STRAP w/ 8 - 8d OR 2X4 COLLAR TIE 3 - 16d OR 4 - .131 x3"

4'-0" (TYP.)

SECTION CUT PARALLEL TO VALLEY RAFTER

WHERE NO SHEATHING IS APPLIED)

SCALE: N.T.S

-2X4 VALLEY RAFTER

- SEE CRIPPLE, BRACING

& BLOCKING NOTES

4'-0" MAX SPACING

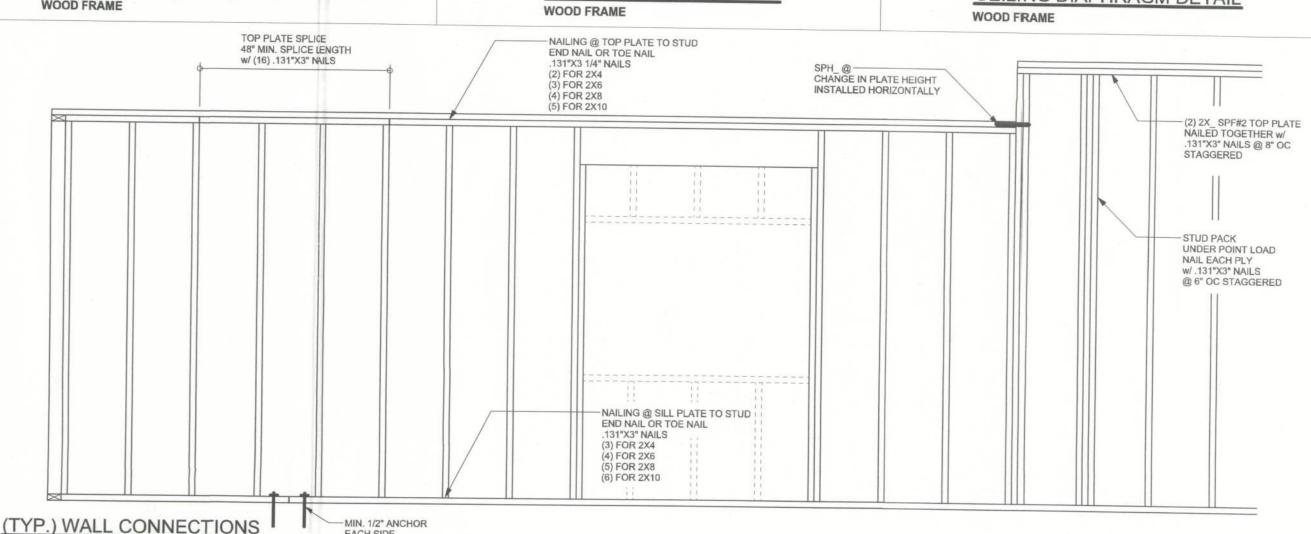
EXISTING ROOF -

FRAMING 2x SYP @ 24" O.C.

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING TRUSS CONNECTOR UPLIFT SYP UPLIFT SPF F1 SYP F2 SYP F1 SPF F2 SPF TO RAFTER/TRUSS TO PLATES 265 115 200 100 170 4-8d x 1 1/2" 4-8d x 1 1/2" 4-8d x 1 1/2" H2.5 415 365 150 150 130 130 5-8d x 1 1/2" 5-8d x 1 1/2 H2.5A 480 | 110 | 110 | 110 | 110 5-8d x 1 1/2" 5-8d x 1 1/2" 8-8d 8-8d 745 5-10d x 1 1/2" 5-10d x 1 1/2 H14-1 1050 515 265 480 245 12-8d x 1 1/2" 13-8d H14-2 1050 515 265 480 245 12-8d x 1 1/2" 15-8d H10 850 585 525 505 450 8-8d x 1 1/2" 8-8d x 1 1/2 H10-2 655 455 395 390 340 6-10d 6-10d 1470 2-10d x 1 1/2" 10-10d x 1 1/ H16-2 1470 1265 2-10d x 1 1/2" 10-10d x 1 1/2 LTS12 - LTS20 1000 620 6-10d x 1 1/2" 6-10d x 1 1/2 MTS12 - MTS30 1000 860 7-10d x 1 1/2" 7-10d x 1 1/2 HTS16 - HTS30 1450 1245 12-10d x 1 1/2" | 12-10d x 1 1/2" HEAVY GIRDER TIEDOW TO FOUNDATION 1785 700 170 700 170 14-16d LGT3-SDS2.5 2655 795 410 795 410 12-SDS 1/4" x 2 1/2" 3685 26-16dS LGT4-SDS3 4060 3860 12-SDS 1/4" x 3" 36-16dS 3330 22 -10d 5/8" ANCHOR HGT-2 10980 6485 16 -10d 2-5/8" ANCHOR HGT-3 10530 9035 16 -10d 2-5/8" ANCHOR HGT-4 9250 9250 16 -10d 2-5/8" ANCHOR STUD STRAP CONNECTO TO STUDS SSP DOUBLE TOP PLATE 435 435 3-10d 4 -10d SSP SINGLE SILL PLATE 455 420 1-10d 4-10d DSP DOUBLE TOP PLATE 825 825 6 -10d 8 -10d DSP SINGLE SILL PLATE 825 600 2 -10d 8 -10d 585 535 4-10d 6 -10d 1065 605 6 -10d 6 -10d 760 6-10d x 1 1/2 SPH4 1240 1065 10-10d x 1 1/2 760 6-10d x 1 1/2" SPH6 1240 1065 10-10d x 1 1/2 LSTA18 1235 1110 14-10d LSTA21 1235 1235 16-10d CS20 1030 1030 14-10d CS16 1705 1705 22-10d STUD ANCHORS TO STUDS TO FOUNDATION LTT19 1350 1305 8-16d 1/2" ANCHOR LTTI31 2310 2310 18-10d x 1 1/2 5/8" ANCHOR 2775 2570 2-5/8" BOLTS 5/8" ANCHOR 4175 3695 18-16d 5/8" ANCHOR 5260 5250 32-16d 5/8" ANCHOR ABU44 2200 2200 12-16d 5/8" ANCHOR 2300 2300 12-16d 5/8" ANCHOR ABU88 2320 2320 18-16d 2-5/8" ANCHOR

(2) FOR SYP GIRDER & SPF STUDS 1/2" GWB UNBLOCKED .131"X3" NAILS 12" OC ---5d COOLER NAILS 7" OC EDGE 10" OC FIELD -8d 6" OC @ PANEL EDGES 8d 12" OC NOT @ PANEL EDGES 2X_FULL HEIGHT STUDS (TYP.) -- EXTERIOR WALL 8d 6" OC @ PANEL EDGES -8d 6" OC @ PANEL EDGES -8d 12" OC NOT @ PANEL EDGES -.131"X3" NAILS 6" OC ----8d 6" OCTHIS STUD FOR SHEAR TRANSFER **OUTSIDE CORNER** 8d 6" OC@ PANEL EDGES 8d 12" OC NOT @ PANEL EDGES - 1/2" GWB UNBLOCKED 5d COOLER NAILS - 2X_ FULL HEIGHT STUDS (TYP.) 7" OC EDGE 10" OC FIELD INTERIOR SHEARWALL -.131"X3" NAILS 12" OC ---1/2" GWB UNBLOCKED -2X_ FULL HEIGHT STUDS (TYP.) -7" OC EDGE 10" OC FIELD - 8d 6" OC@ PANEL EDGES 8d 12" OC NOT @ PANEL EDGES .131"X3" NAILS 12" OC --8d 6" OC @ PANEL EDGES 8d 12" OC NOT @ PANEL EDGES **INSIDE CORNER** (TYP.) INTERSECTING WALL FRAMING (TYP.) CORNER FRAMING

OF PLATE JOINT



EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC TO 10'-1" STUD HEIGHT (1) 2x4 @ 12" OC TO 11'-2" STUD HEIGHT (1) 2x6 @ 16" OC TO 15'-7" STUD HEIGHT (1) 2x6 @ 12" OC TO 17'-3" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B4, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C. TUD DEFLECTION LIMIT H/240 (NOT OK FOR SOME BRITTLE FINISH?) STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

PRE ENGINEERED ROOF TRUSS -

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE 2010 FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, USS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

REVISIONS

SOFTPLAN

WINDLOAD ENGINEER: Mark Disosway,

PE No.53915, POB 868, Lake City, FL

Stated dimensions supercede scaled

dimensions. Refer all questions to

Mark Disosway, P.E. for resolution.

Do not proceed without clarification.

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permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have

examined this plan, and that the applicable

portions of the plan, relating to wind enginee

comply with section R301.2.1, 2010 Florida

LIMITATION: This design is valid for one

539/15

TATE OF

Erkinger

Construction Group

Collier Addition

ADDRESS:

1994 SW CR240

Columbia County, FL

Mark Disosway P.E.

P.O. Box 868

PRINTED DATE:

February 21, 2013

STRUCTURAL BY

Evan Beamsley

Building Code Residential

to the best of my knowledge.

building, at specified location.

32056, 386-754-5419

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI. WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDING. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY: 4"OC. UNC

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT.
AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES, MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNC

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH 2010 FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH 2010 FBCR, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF 2010 FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN LOADS

ROOF 20 PSF (FLAT OR <4:12)

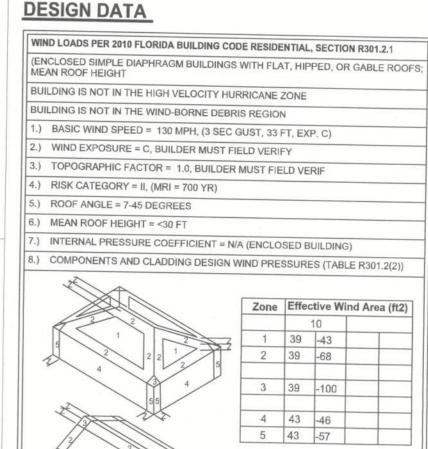
SOIL BEARING CAPACITY 1000PSF

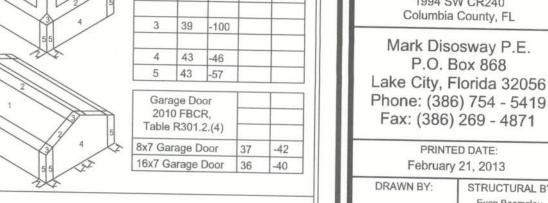
16 PSF (4:12 TO <12:12)

NOT IN FLOOD ZONE (BUILDER TO VERIFY

12 PSF (12:12 AND GREATER)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)





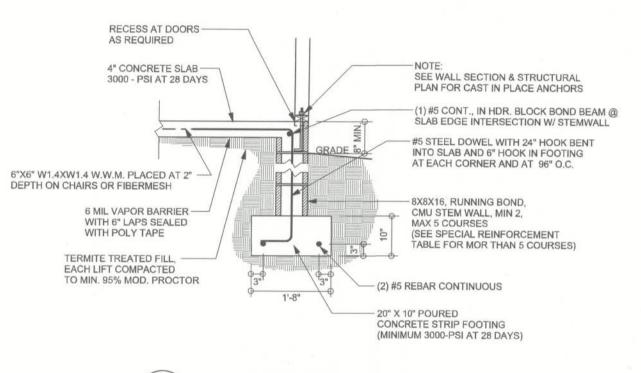
FLOOR 40 PSF (ALL OTHER DWELLING ROOMS) 30 PSF (SLEEPING ROOMS 30 PSF (ATTICS WITH STORAGE FINALS DATE: 10 PSF (ATTICS WITHOUT STORAGE, <3:12)

2013-02-20 JOB NUMBER 1302026

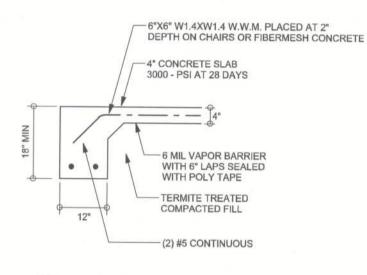
DRAWING NUMBER

5-1 OF 2 SHEETS

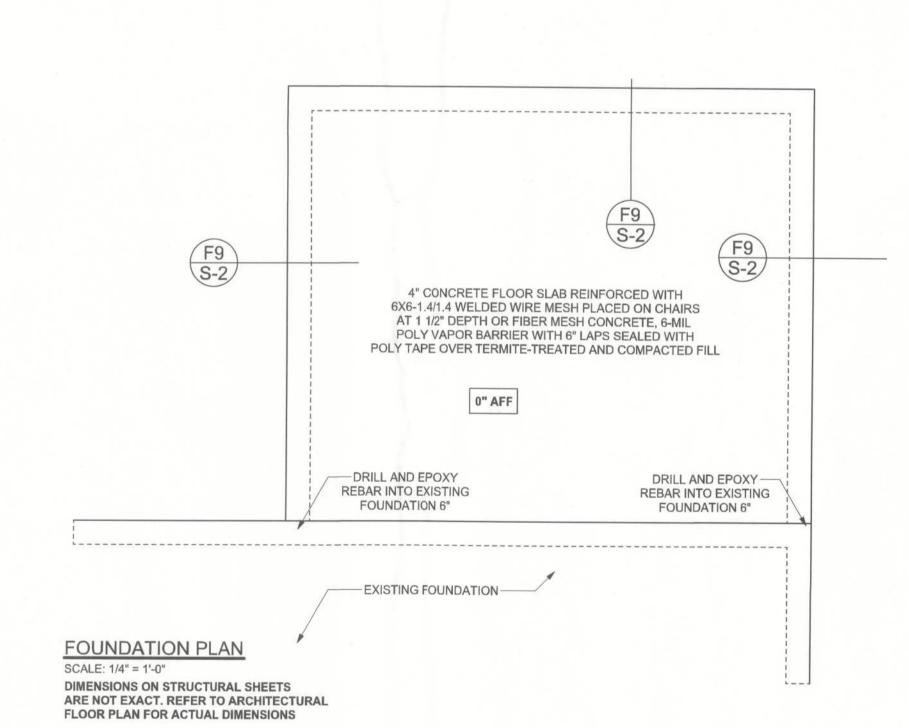
SINGLE 2X SPF TOP PLATE -INTERIOR CEILING AS -SPECIFIEID ON FLOOR PLAN CONTINUIOUS FRAME -O TOP PLATE AT BOTTOM ICHORD OF TRUSS ALL STUDS TO BE 2X -SPF NAILED TO TO AND BOTTOM PLATES CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL

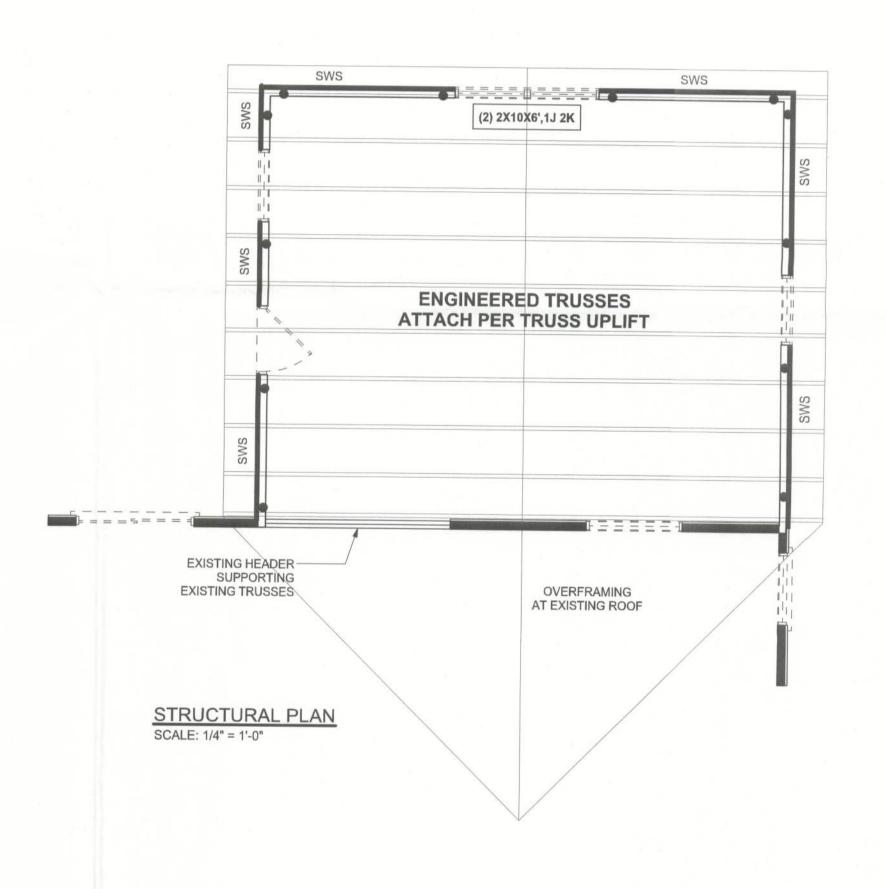


ALT. STEM WALL FOOTING SCALE: 1/2" = 1'-0"



MONO FOOTING S-2 | SCALE: 1/2" = 1'-0"





STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP#2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

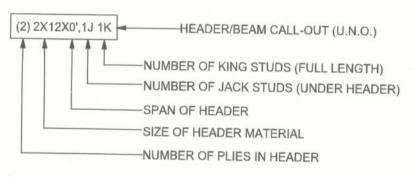
SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. SN-4 LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

		EXTERIOR WALL
		INTERIOR NON-LOAD BEARING WALL
		INTERIOR LOAD BEARING WALL w/ NO UPLIFT
		INTERIOR LOAD BEARING WALL w/ UPLIFT

HEADER LEGEND



THREADED ROD LEGEND

- INDICATES LOCATION OF: 1ST FLOOR 3/8" A307 ALL THREADED ROD

TOTAL SHEAR WALL SEGMENTS INDICATES SHEAR WALL SEGMENTS

REQUIRED ACTUAL TRANSVERSE 9.3' 16.0 LONGITUDINAL N/A 27.0

REVISIONS

SOFTPIAN

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

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not to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disosway. CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineerin comply with section R301.2.1, 2010 Florida Building Code Residential to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location. MARK DISOSWAY

P.E. 53915 Nd 53915 STATE OF February 21, 2013

Erkinger Construction Group

Collier Addition

ADDRESS: 1994 SW CR240 Columbia County, FL

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: February 21, 2013 STRUCTURAL BY: DRAWN BY: Evan Beamsley

FINALS DATE: 2013-02-20

> JOB NUMBER: 1302026 DRAWING NUMBER

> > **S-2** OF 2 SHEETS