

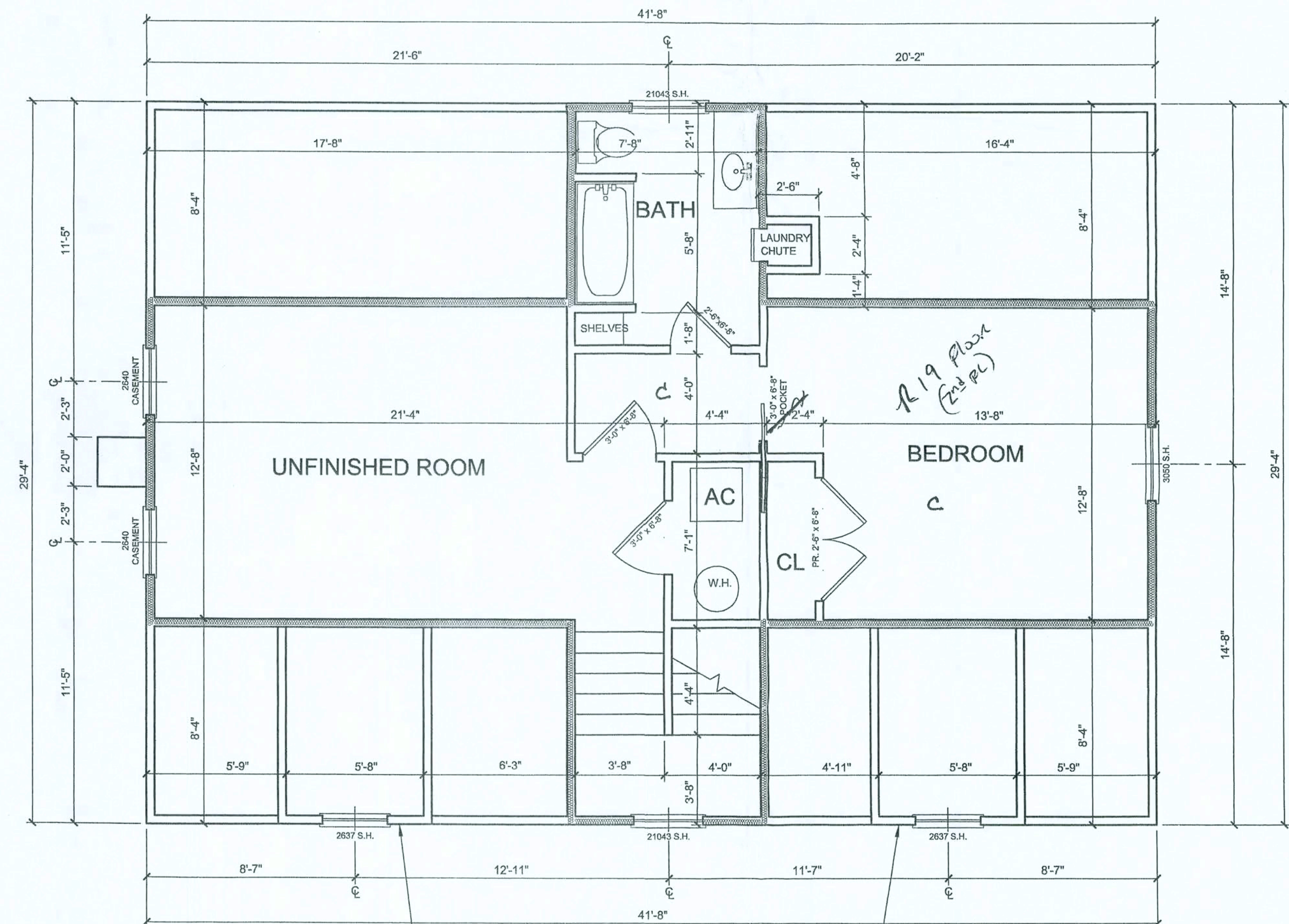
NOTE:  
PROVIDE MIN. 6" DEEP WINDOW  
SILL INSIDE (TYP.)

## FIRST FLOOR PLAN

1/4"=1'-0"



FIRST FLOOR COND.	1,222 S.F.
SECOND FLOOR COND.	672 S.F.
PORCHES	639 S.F.
SCREENED-IN PORCH	123 S.F.
CARPORT	312 S.F.
TOTAL	2,968 S.F.



CONSTRUCT DORMER ON TOP  
OF ROOF - STRAP AS PER  
WINDLOAD CALCULATIONS

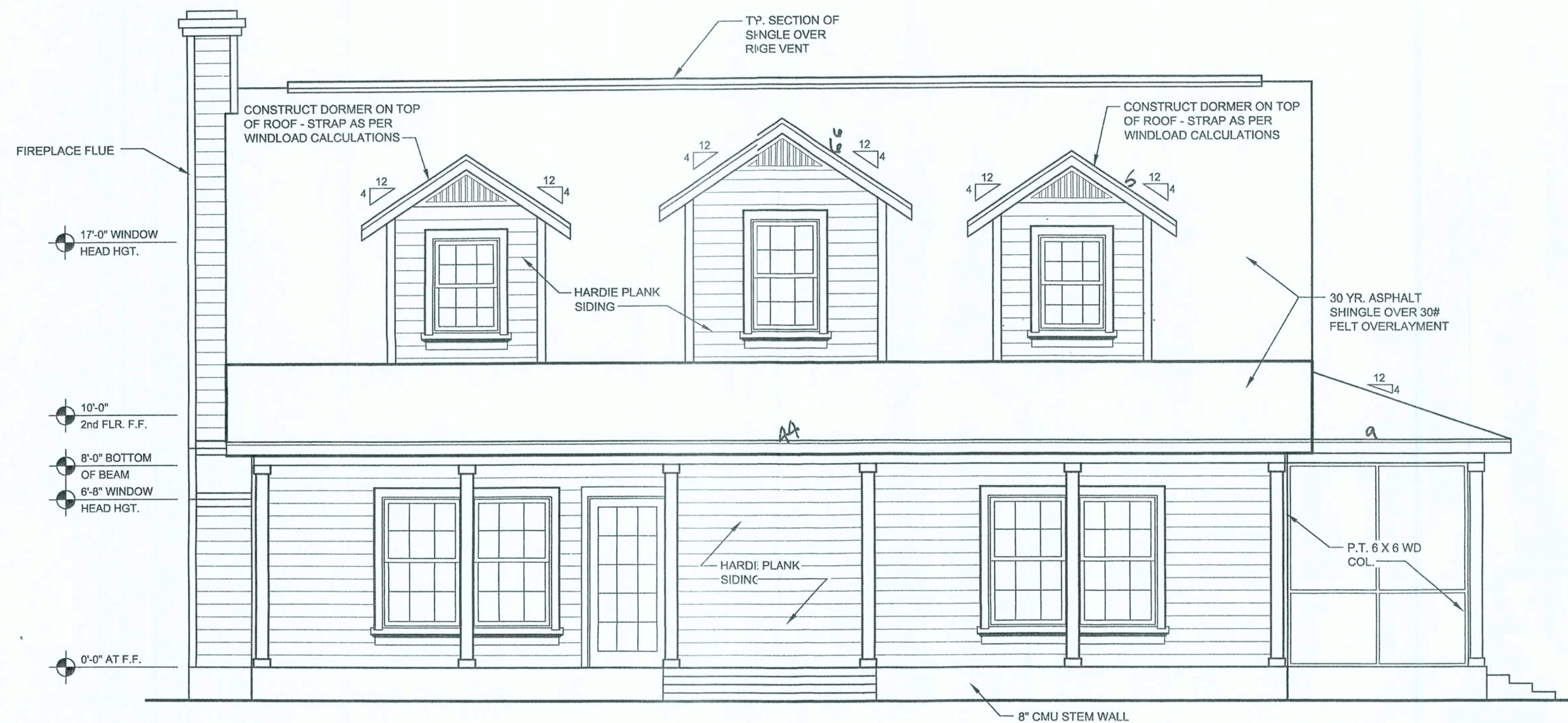
NOTE:  
PROVIDE MIN. 6" DEEP WINDOW  
SILL INSIDE (TYP.)

## SECOND FLOOR PLAN

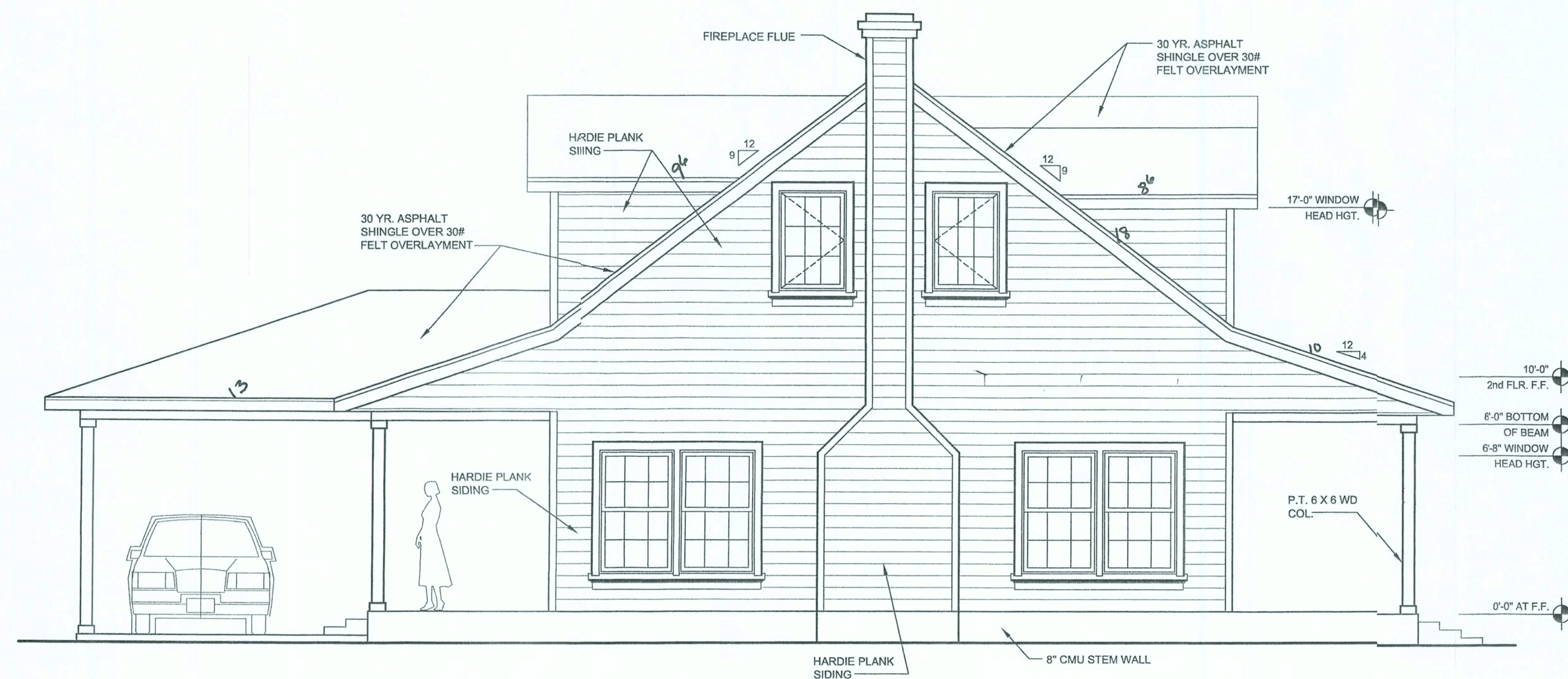
1/4"=1'-0"



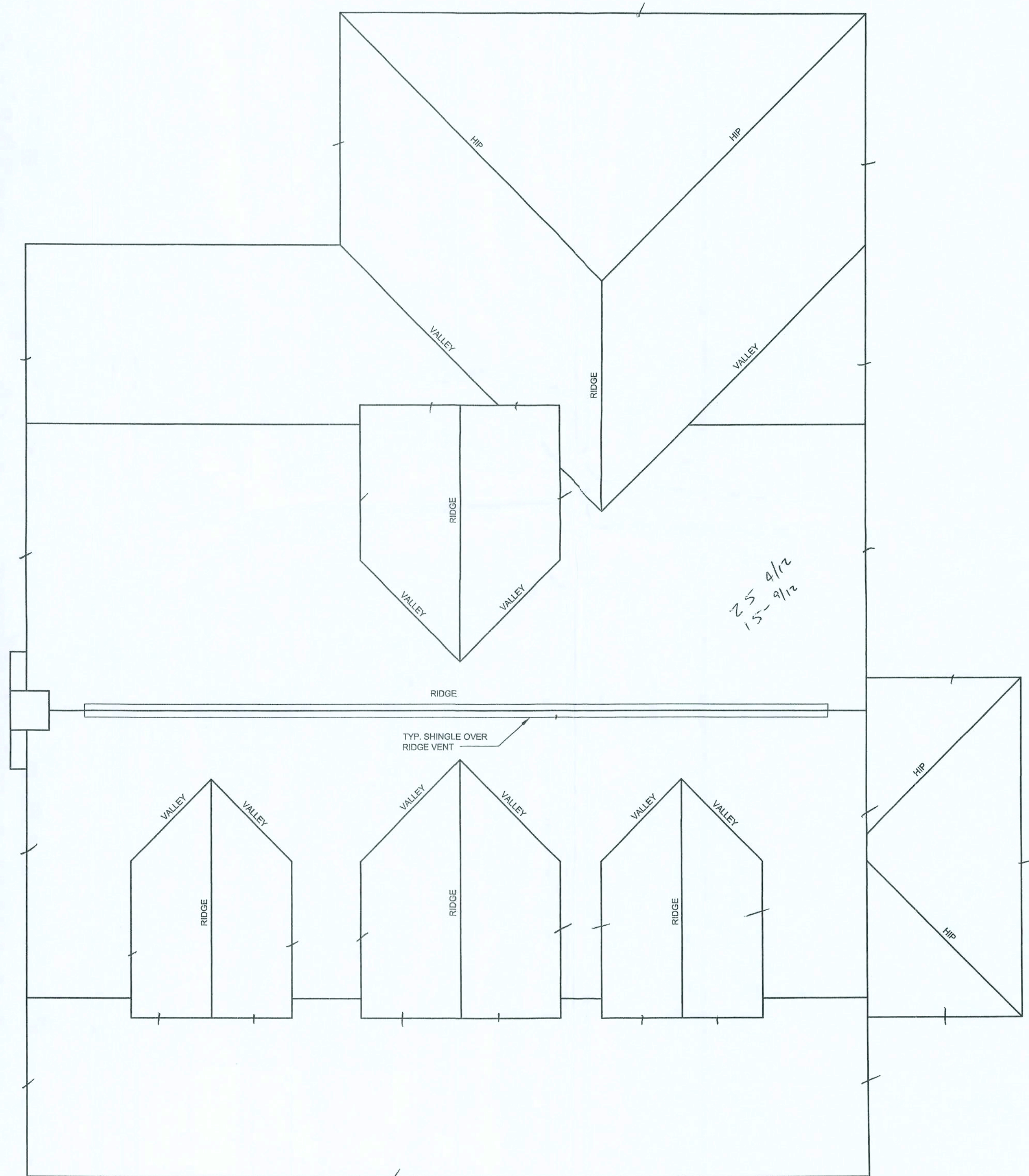




**FRONT ELEVATION**  
1/4"=1'-0"

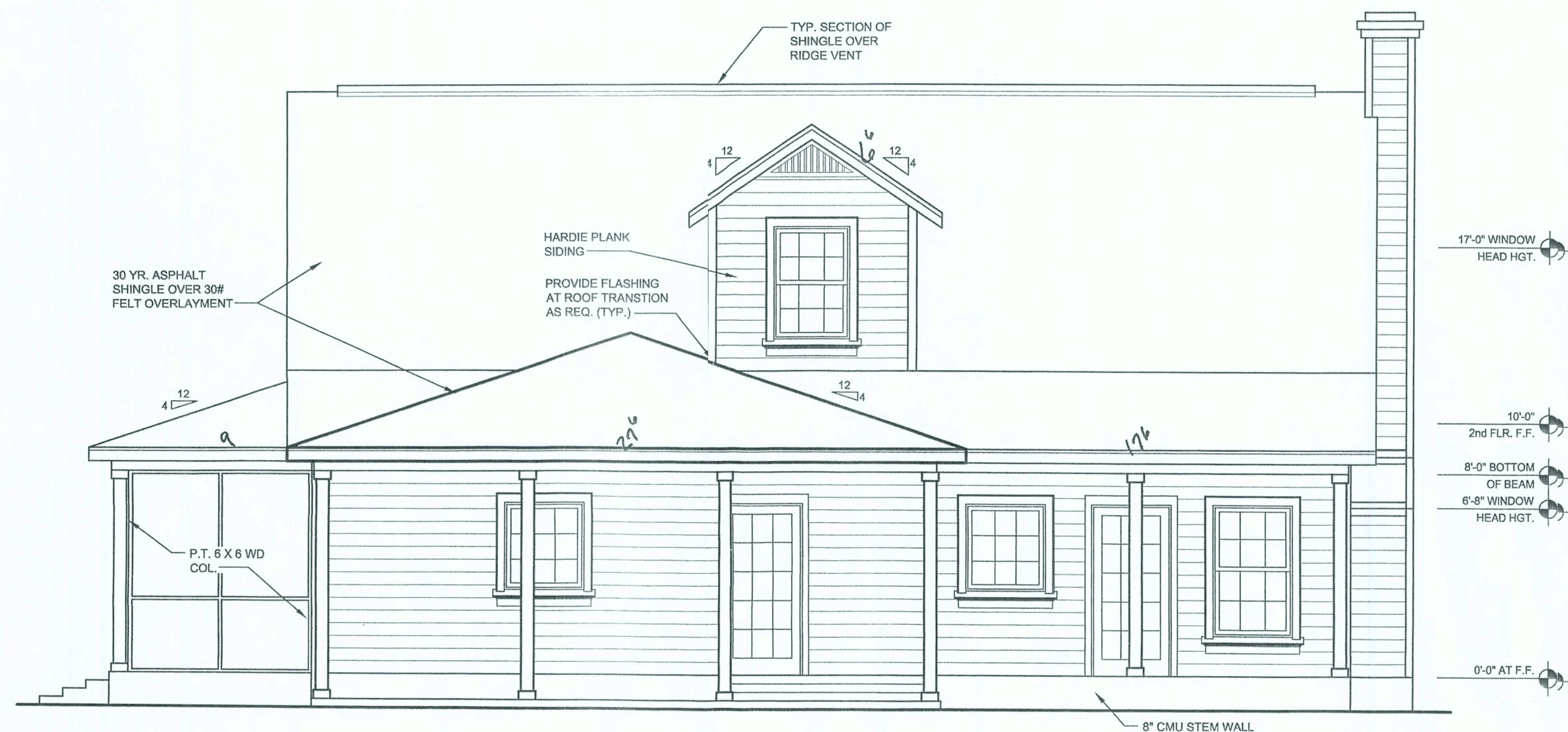


**LEFT SIDE ELEVATION**  
1/4"=1'-0"



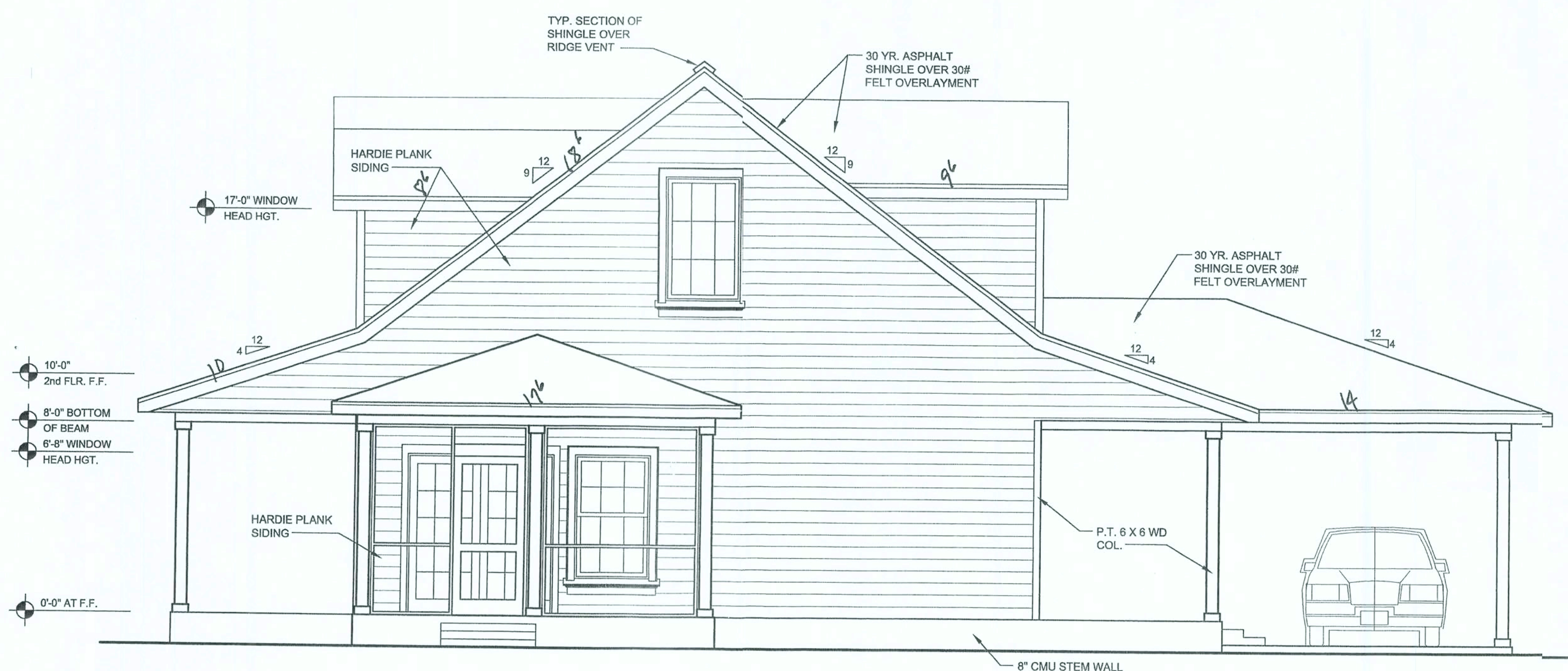
**ROOF PLAN PLAN**  
1/4"=1'-0"





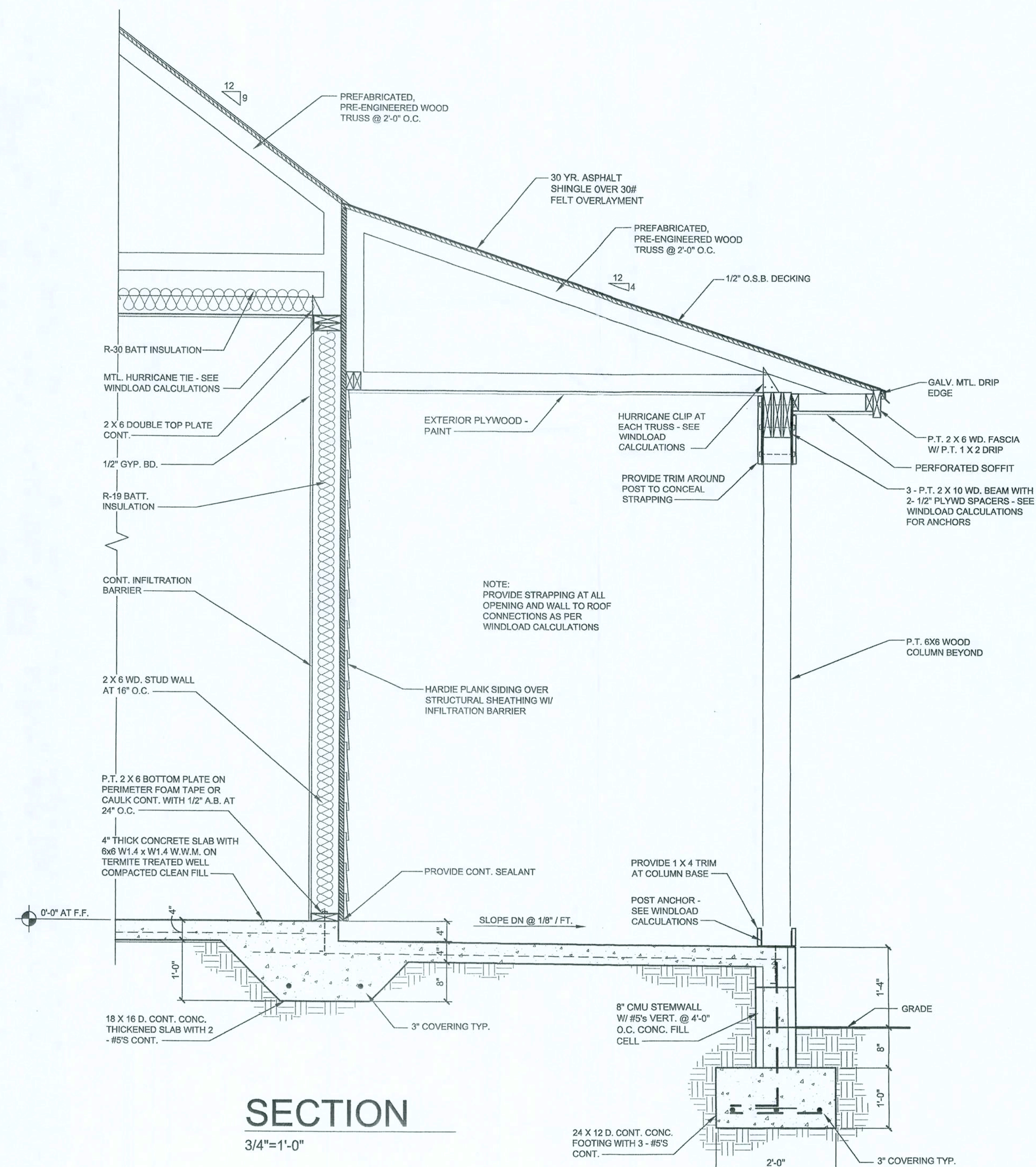
## REAR ELEVATION

1/4"=1'-0"



## RIGHT SIDE ELEVATION

1/4"=1'-0"



## SECTION

3/4"=1'-0"

RCA

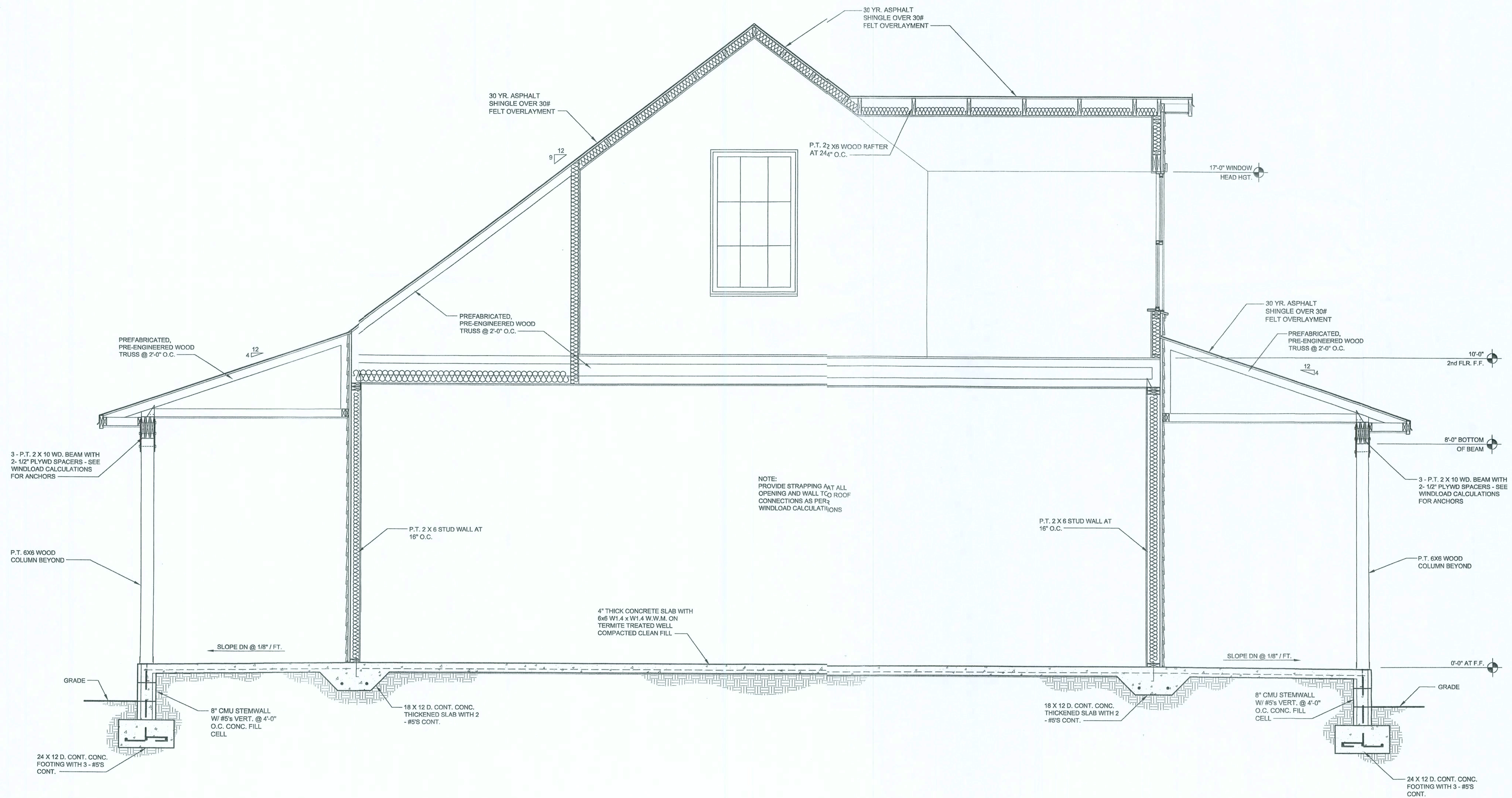
RICARDO CAVALLINO  
AND ASSOCIATES, INC.  
ARCHITECTURE-PLANNING  
AA 002865  
352-377-1751 (O)  
352-377-1765 (F)  
622 SE SECOND ST.  
GAINESVILLE, FL 32601

NEW RESIDENCE FOR  
**MR. AND MRS. ROWE**  
COLUMBIA COUNTY, FLORIDA

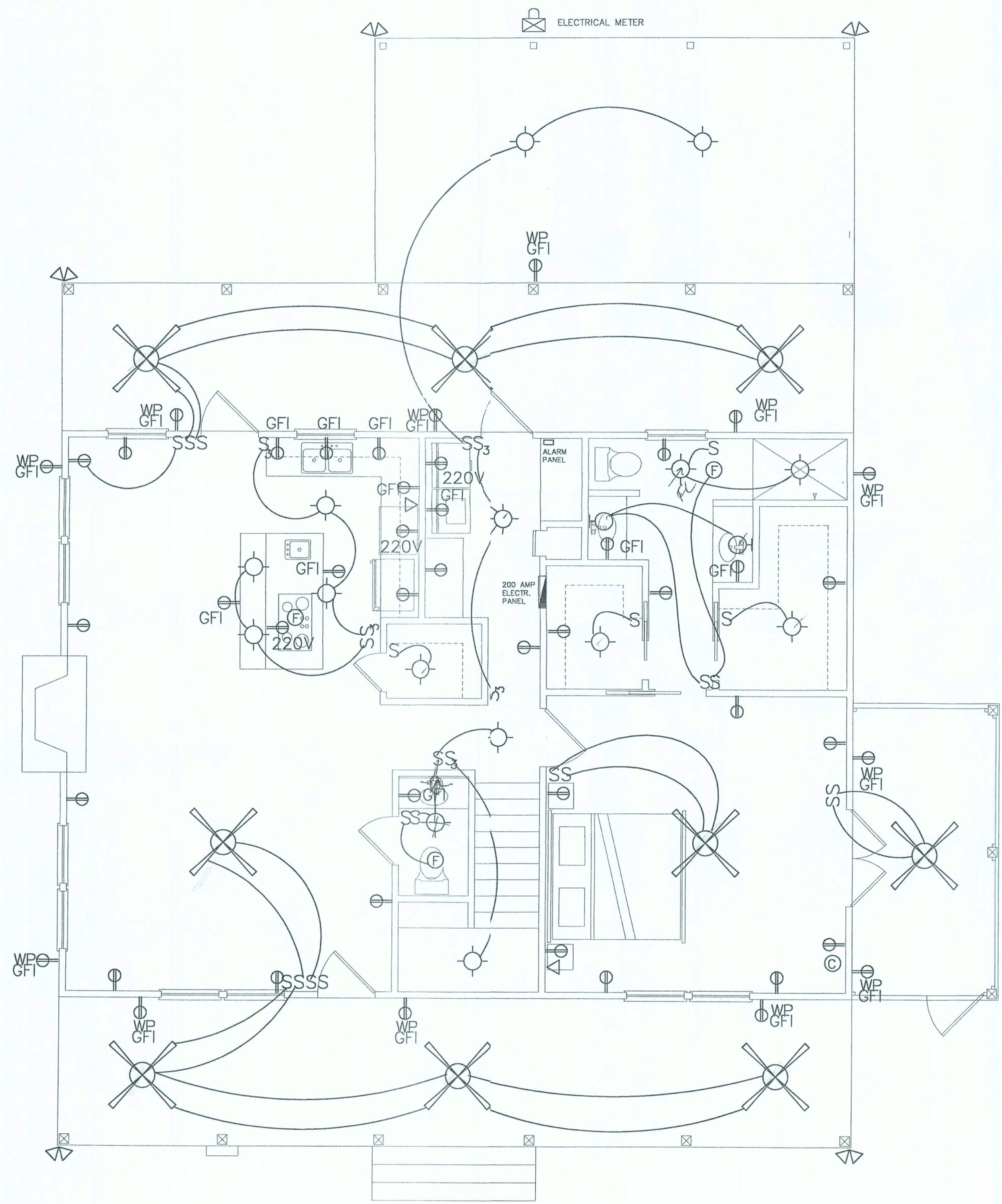
PROJECT: RCA0510  
DRAWN BY: R.L.A.  
CHECKED: R.C.  
DATE: 11-09-2005

A4









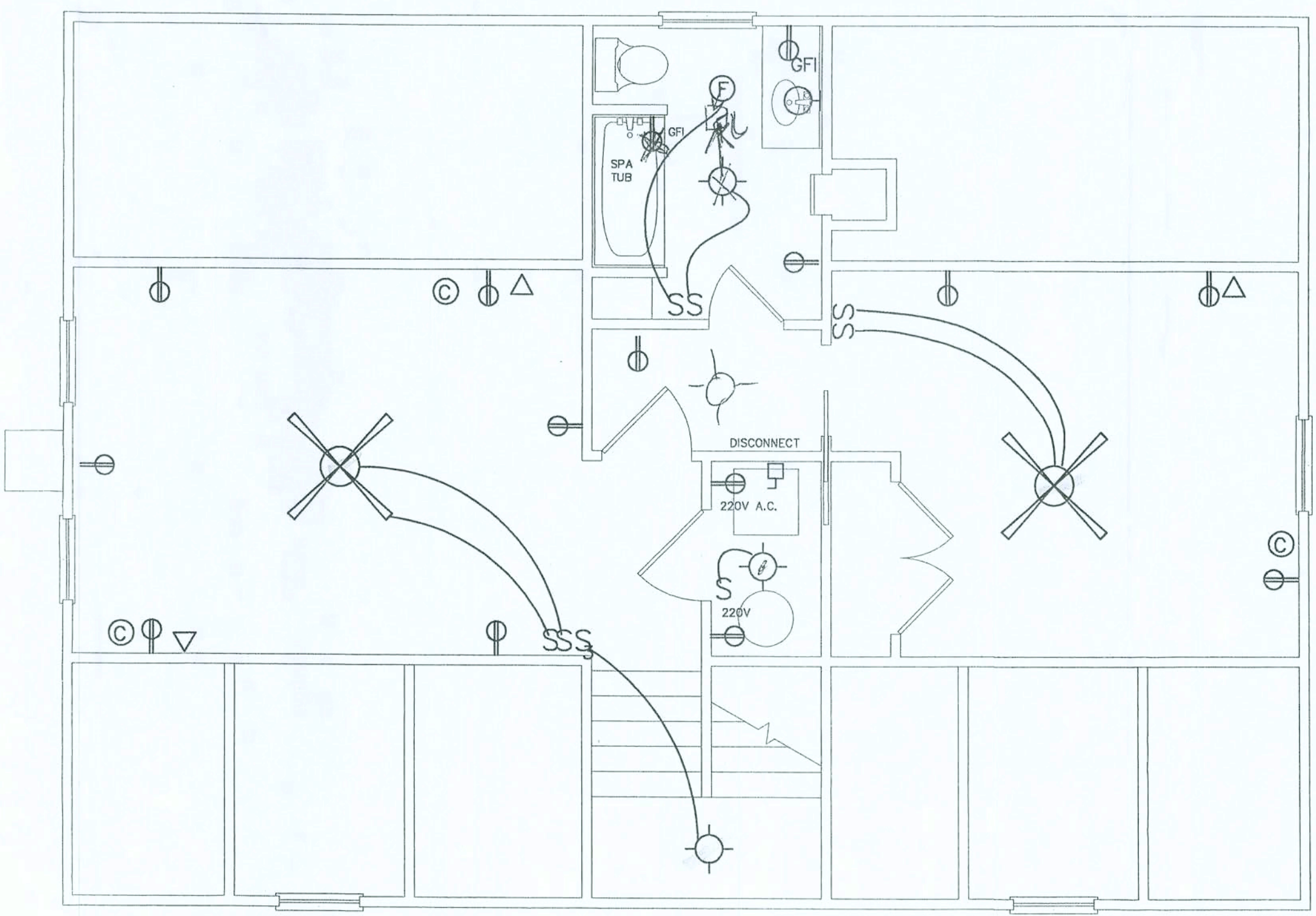
**FIRST FLOOR ELECTRICAL PLAN**

1/4"=1'-0"



**ELECTRICAL LEGEND**

- ⊕ DUPLEX RECEPTACLE OUTLET
- △ PHONE OUTLET
- LIGHT FIXTURE
- WALL MOUNTED LIGHT FIXTURE
- ⊖ EXHAUST FAN
- Ⓢ CABLE TV
- ⚡ EXTERIOR FLOOD LIGHT WITH MOTION DETECTOR
- ⚡ ELECTRICAL PANEL 200 AMP OVERHEAD
- ⊖ ELECTRICAL METER
- Ⓢ SMOKE DETECTOR
- ⊗ CEILING FAN WITH LIGHT FIXTURE



**SECOND FLOOR ELECTRICAL PLAN**

1/4"=1'-0"



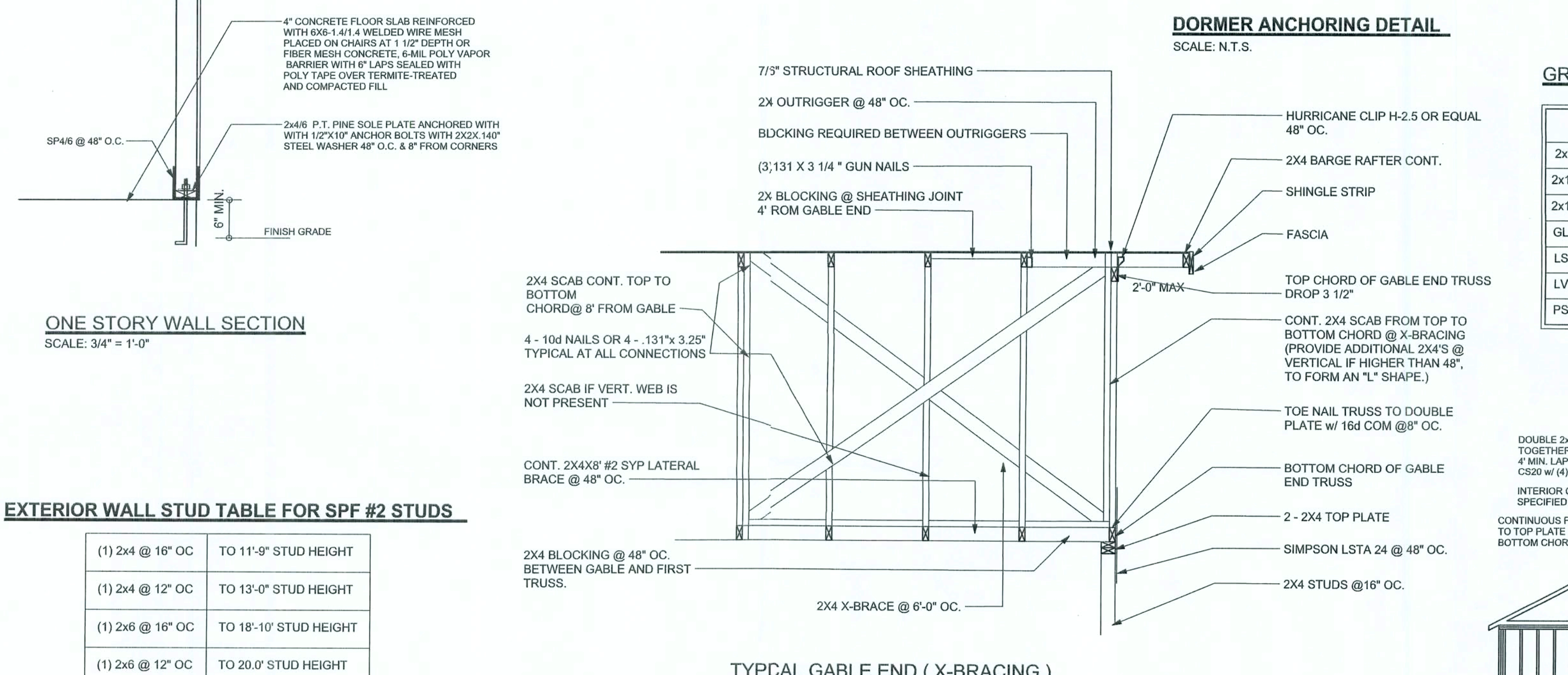
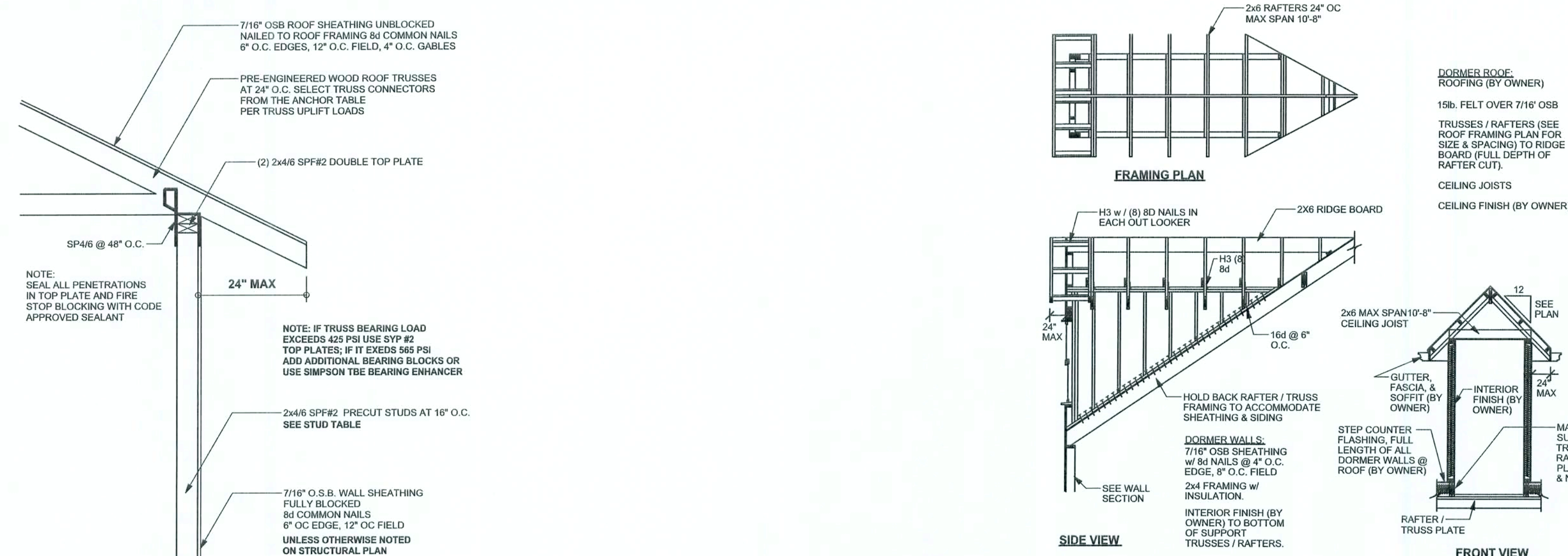
**RICARDO CAVALLINO  
AND ASSOCIATES, INC.**  
ARCHITECTURE-PLANNING  
AA 002805  
352-377-1751 (O)  
352-377-1755 (F)  
622 SE SECOND ST.  
GAINESVILLE, FL 32601

NEW RESIDENCE FOR  
**MR. AND MRS. ROWE**  
COLUMBIA COUNTY, FLORIDA

REVISED:  
PROJECT: RGA0510  
DRAWN BY: R.L.A.  
CHECKED: R.C.  
DATE: 11-09-2005

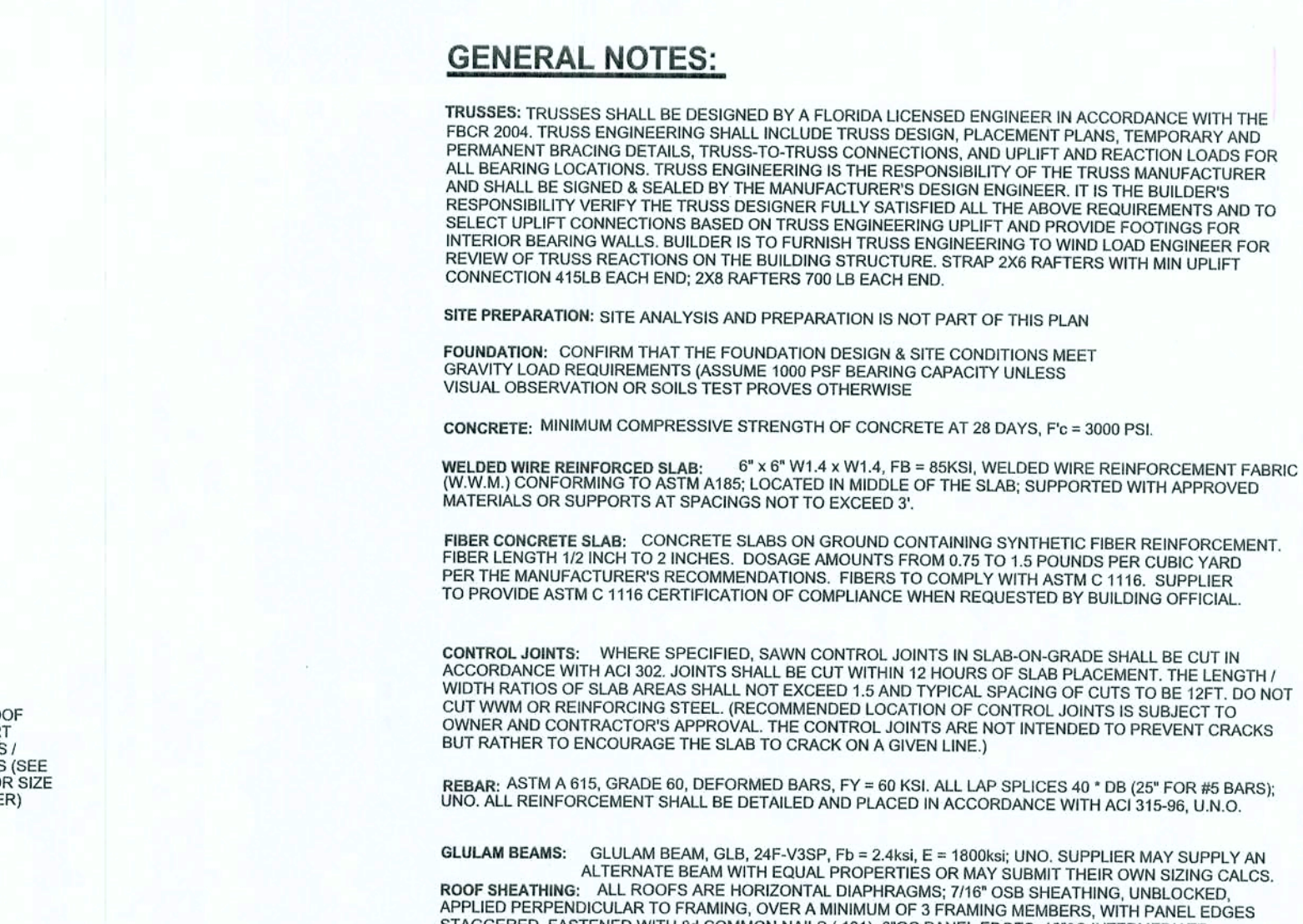
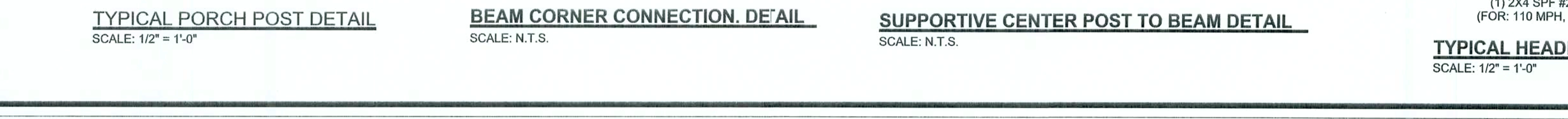
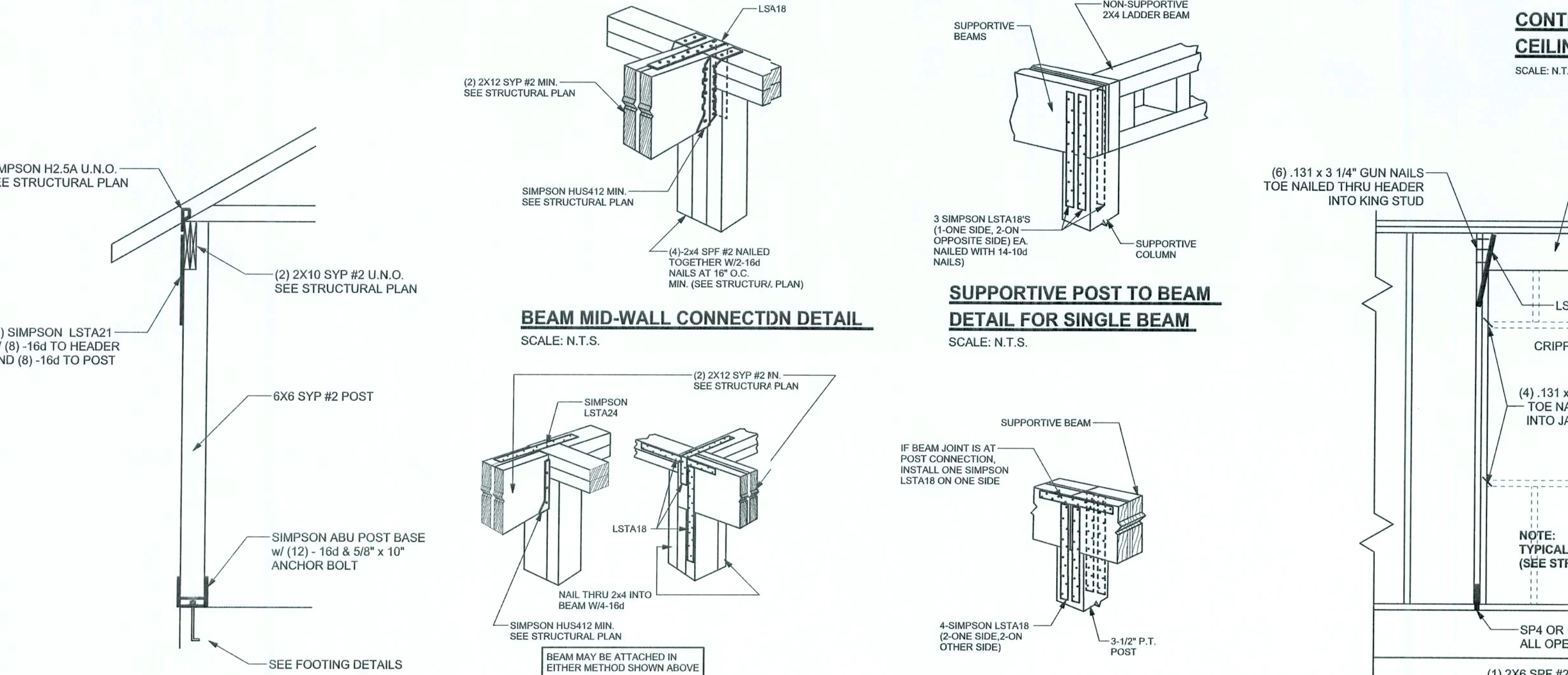
**E1**





ONE STORY WALL SECTION	
SCALE: 3/4" = 1'-0"	

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS	
(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT



DORMER ANCHORING DETAIL	
SCALE: N.T.S.	

GRADE & SPECIES TABLE

	Fb (psi)	E (10 <sup>6</sup> psi)
SYP #2	1200	1.6
SYP #2	1050	1.6
SYP #2	975	1.6
24F-V3 SP	2400	1.8
TIMBERSTRAND	1700	1.7
MICROLAM	1600	1.9
PARALAM	2900	2.0

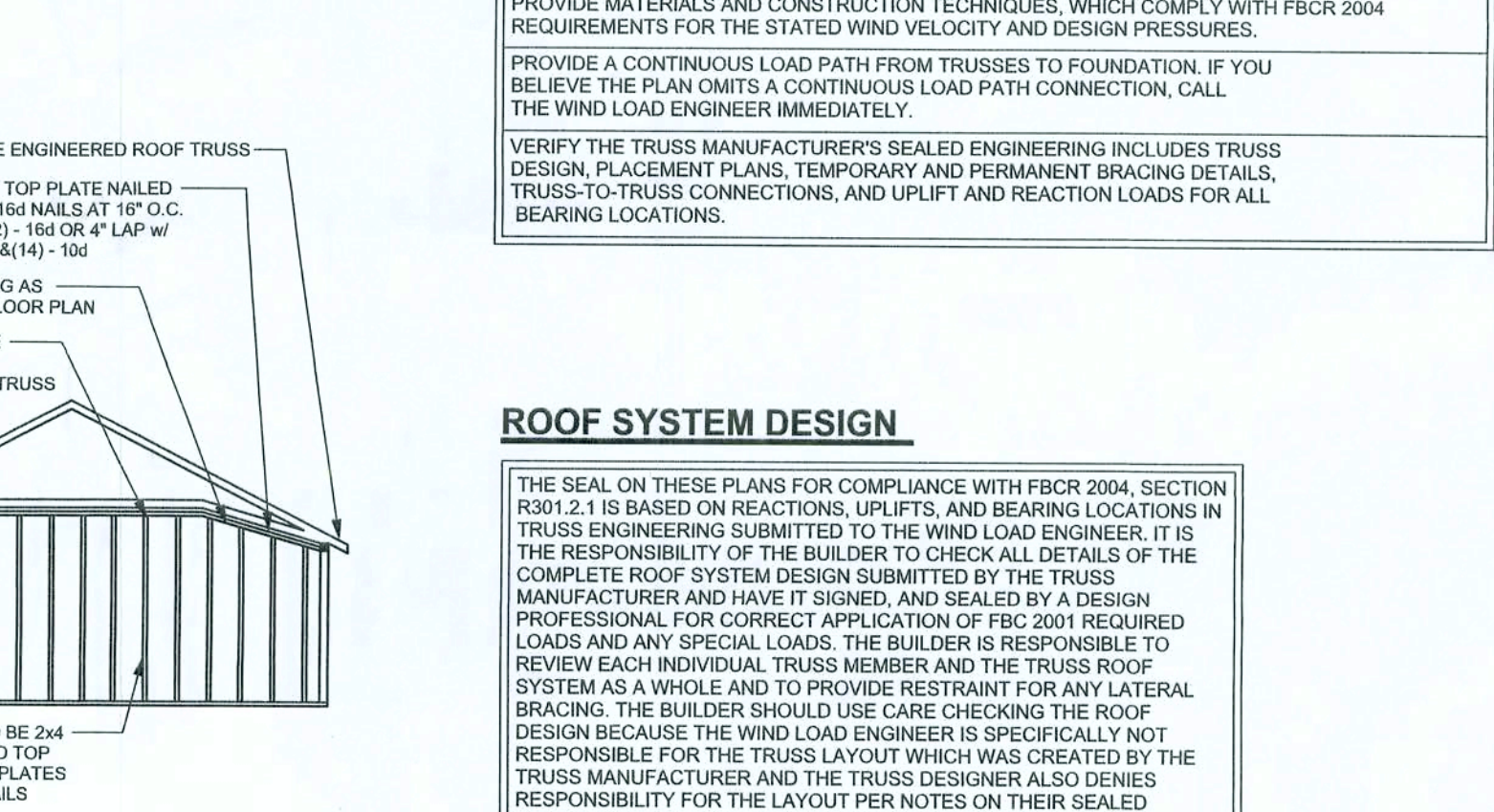
WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64", WITH 5/8" BOLTS TO BE 3" x 3" x 9/64", WITH 3/4" BOLTS TO BE 2" x 3" x 9/64", WITH 7/8" BOLTS TO BE 3" x 3" x 5/16" DIA.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

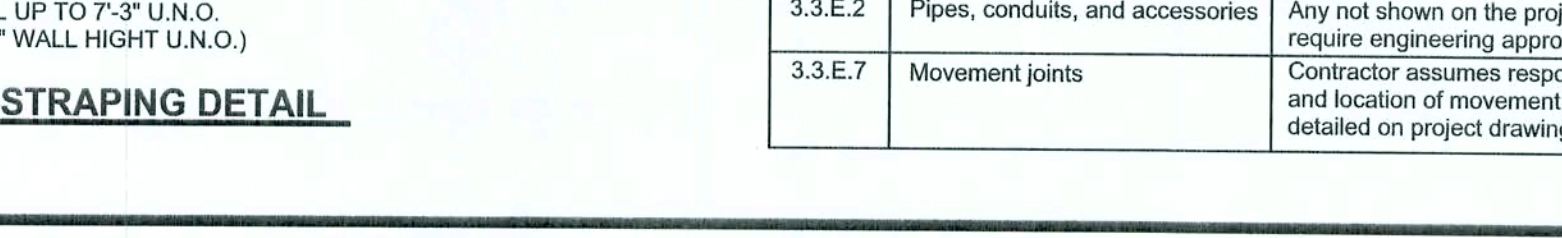
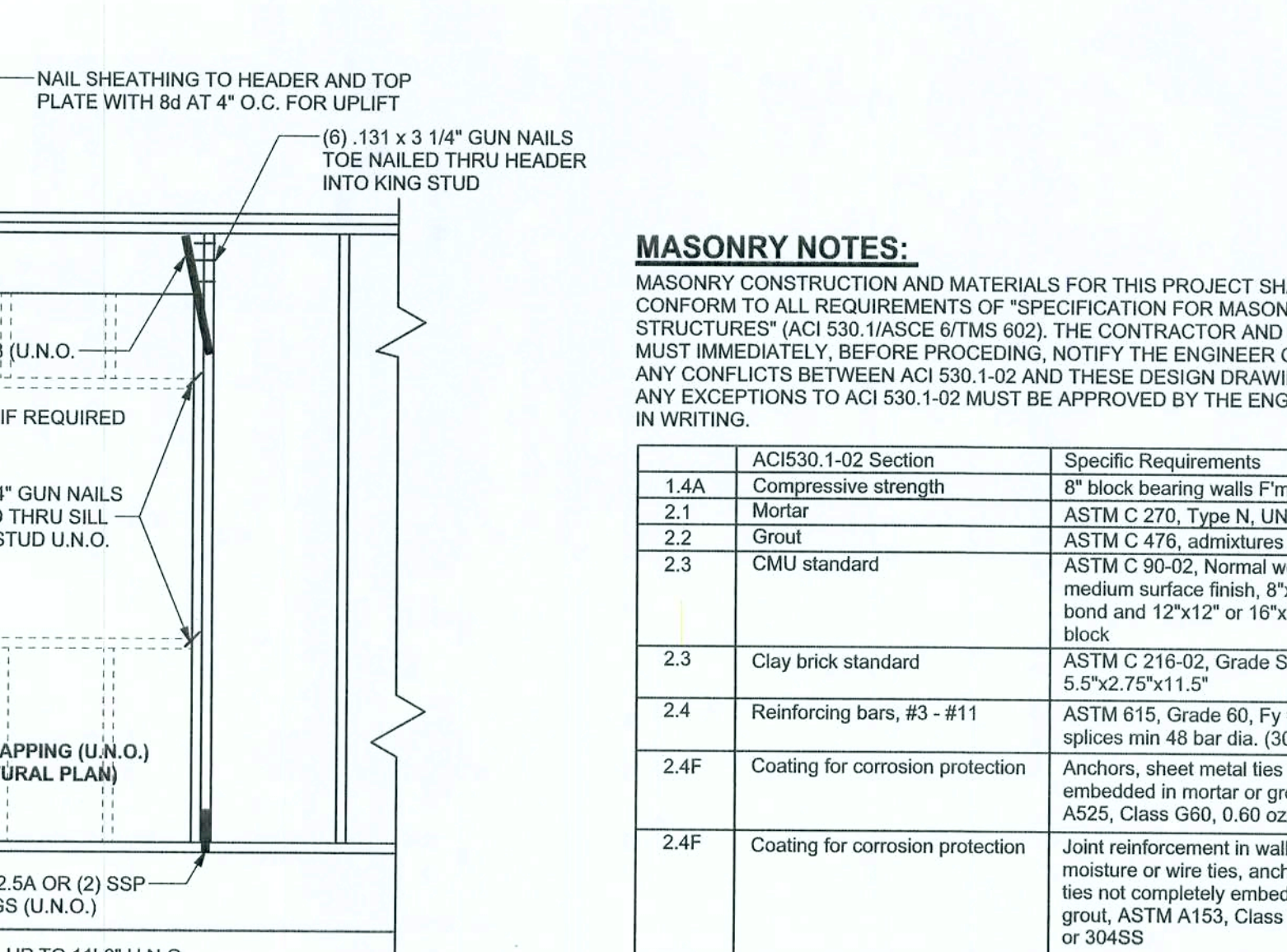
BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.



CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL	
SCALE: N.T.S.	



# GENERAL NOTES:

**TRUSSES:** TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X6 RAFTERS 700 LB EACH END.

**SITE PREPARATION:** SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

**FOUNDATION:** CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

**CONCRETE:** MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F<sub>c</sub> = 3000 PSI.

**WELDED WIRE REINFORCED SLAB:** 6" x 6" W14 x W14, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A188; LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 2'.

**FIBER CONCRETE SLAB:** CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 1.5 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

**CONTROL JOINTS:** WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB EDGES SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12 FT. DO NOT CUT W/M OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE).

**REBAR:** ASTM A 615, GRADE 60, DEFORMED BARS, F<sub>y</sub> = 60 KSI, ALL LAP SPLICES 40" DB (25' FOR #5 BARS); UNO, ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-08, UNO.

**GLULAM BEAMS:** GLULAM BEAM, GLB, 24F-V3SP, F<sub>b</sub> = 2400, E = 1800ksi, UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCULATIONS.

**ROOF SHEATHING:** ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4"OC, UNO.

**STRUCTURAL CONNECTIONS:** MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

**ANCHOR BOLTS:** A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

**WASHERS:** WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

**NAILS:** ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

# BUILDER'S RESPONSIBILITY

**THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.**

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

# ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

# MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 8/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.	
1.4A	Compressive strength
2.1	Mortar
2.2	Grout
2.3	CMU standard
2.3	Clay brick standard
2.4	Reinforcing bars, #3 - #11
2.4F	Coating for corrosion protection
2.4F	Coating for corrosion protection
3.3.E.2	Pipes, conduits, and accessories
3.3.E.7	Movement joints

# ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING					
UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 380	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5A	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 950	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2-HTS24			
< 2050	< 1785	LGT2	14-16d	14-16d	
HEAVY GIRDER TIEDOWNS*					
< 3965	< 3330	MG1		22-10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
		STUD STRAP CONNECTOR*	TO STUDS		
< 435	< 435	SSP DOUBLE TOP PLATE	3-10d		4-10d
< 455	< 420	SSP SINGLE SILL PLATE	1-10d		4-10d
< 825	< 825	DSP DOUBLE TOP PLATE	6-10d		8-10d
< 825	< 600	DSP SINGLE SILL PLATE	2-10d		8-10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SP4M			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SP6M			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1039	< 1039	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS	TO FOUNDATION	
< 1359	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTT131	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18-16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18-16d		2-5/8" AB

# DESIGN DATA

**WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1**

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT, NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSERVED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

1.) BASIC WIND SPEED = 110 MPH

2.) WIND EXPOSURE = B

3.) WIND IMPORTANCE FACTOR = 1.0

4.) BUILDING CATEGORY = II

5.) ROOF ANGLE = 10-45 DEGREES

6.) MEAN ROOF HEIGHT = <30 FT

7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone	Effective Wind Area (ft <sup>2</sup> )	10	100
1	19.9	-21.8	-18.1
2	19.9	-25.5	-21.8
2 Other		-40.8	-40.8
3	19.9	-25.5	-18.1
3 Other		-68.3	-42.4
4	21.8	-23.6	-20.4
5	21.8	-29.1	-22.6

Doors & Windows	21.8	-29.1
Worst Case (Zone 5, 10 ft <sup>2</sup> )		
8x7 Garage Door	19.5	-22.9
16x7 Garage Door	18.5	-21.0

DESIGN LOADS	
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
SOIL BEARING CAPACITY 1000PSF	
NOT IN FLOOD ZONE (BUILDER TO VERIFY)	

REVISIONS	



WINDLOAD ENGINEER: Mark Discoway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Discoway, P.E. for resolution. Do not proceed without clarification.

COPYRIGHTS AND PROPERTY RIGHTS: Mark Discoway, P.E. hereby expressly reserves its common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first obtaining written permission and consent of Mark Discoway.

CERTIFICATION: I hereby certify that I have examined this plan, and the applicable portions of the plan, relating to wind engineering comply with section R301.21, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISCOWAY  
P.E. 53915

03JAN06

SEAL

**B&B Homes, Inc**

Rowe Residence

ADDRESS:  
626 SW Uth St.  
Pt. White, Florida

Mark Discoway P.E.  
P.O. Box 868  
Lake City, Florida 32056  
Phone: (386) 754 - 5419  
Fax: (386) 269 - 4871

PRINTED DATE:  
January 02, 2006

DRAWN BY:  
David Discoway

CHECKED BY:

FINALS DATE:  
02 / Jan / 05

JOB NUMBER:  
512231

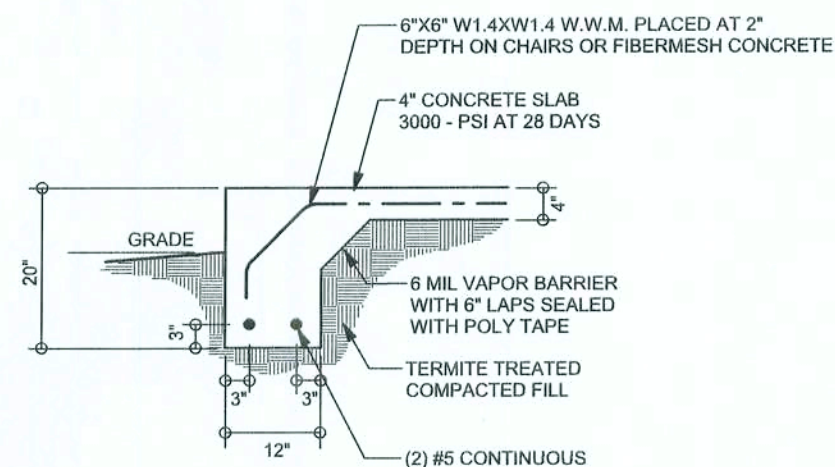
DRAWING NUMBER

**S-1**  
OF 3 SHEETS

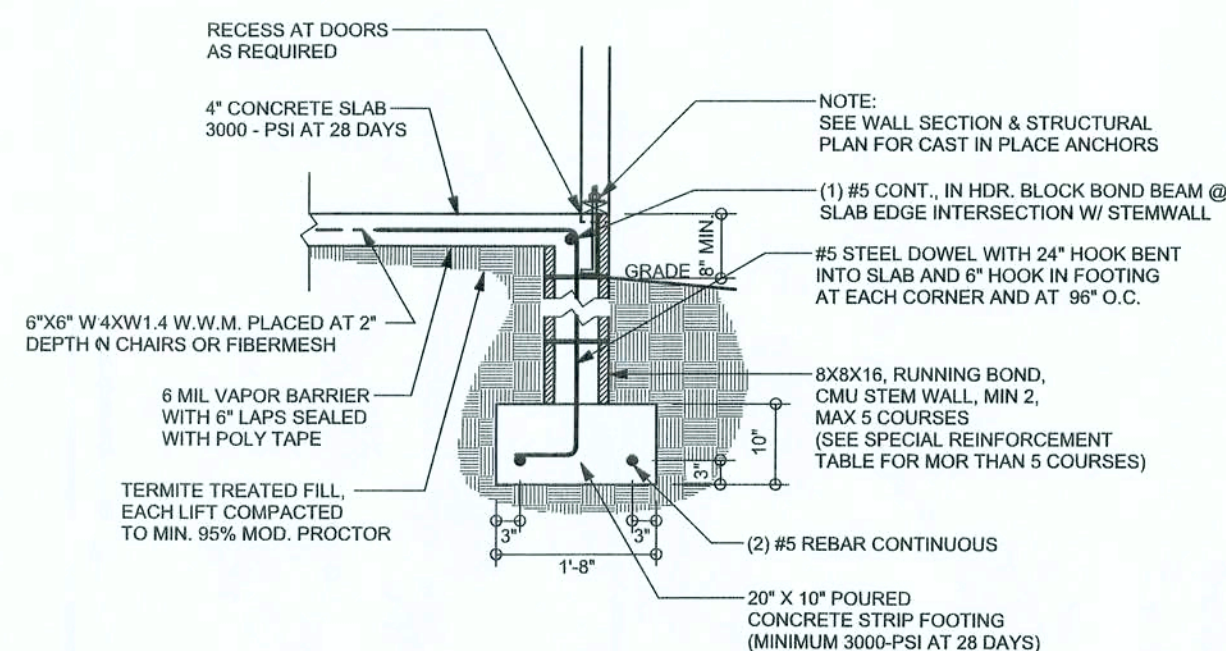


REVISIONS	

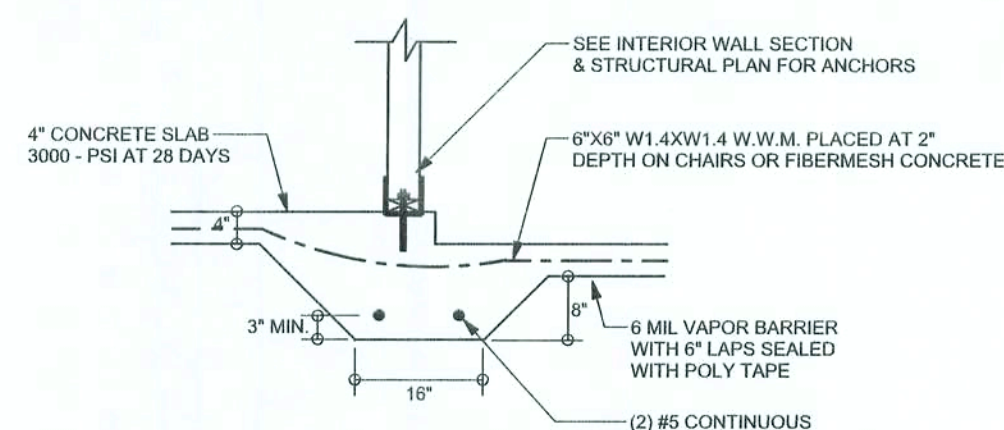
SOFTPLAN  
ARCHITECTURAL DESIGN SOFTWARE



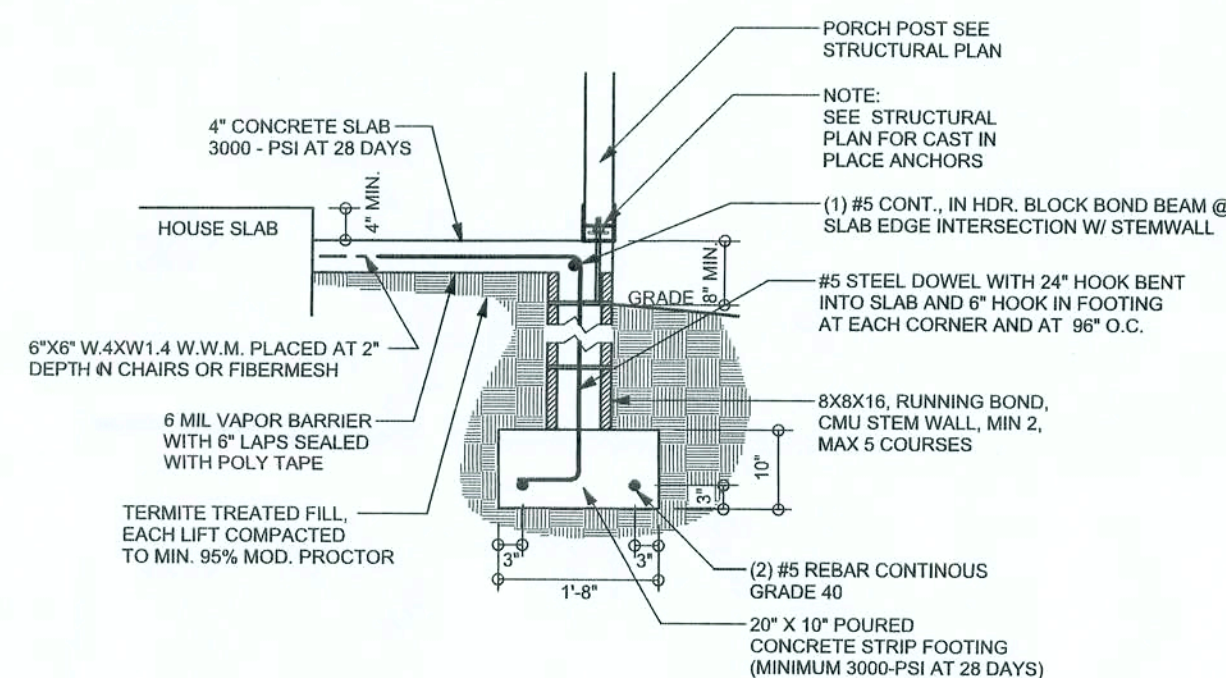
**F1 S-2** MONOLITHIC FOOTING  
SCALE: 1/2" = 1'-0"



**F9 S-2** STEM WALL FOOTING  
SCALE: 1/2" = 1'-0"



**F3 S-2** INTERIOR BEARING STEP FOOTING  
SCALE: 1/2" = 1'-0"

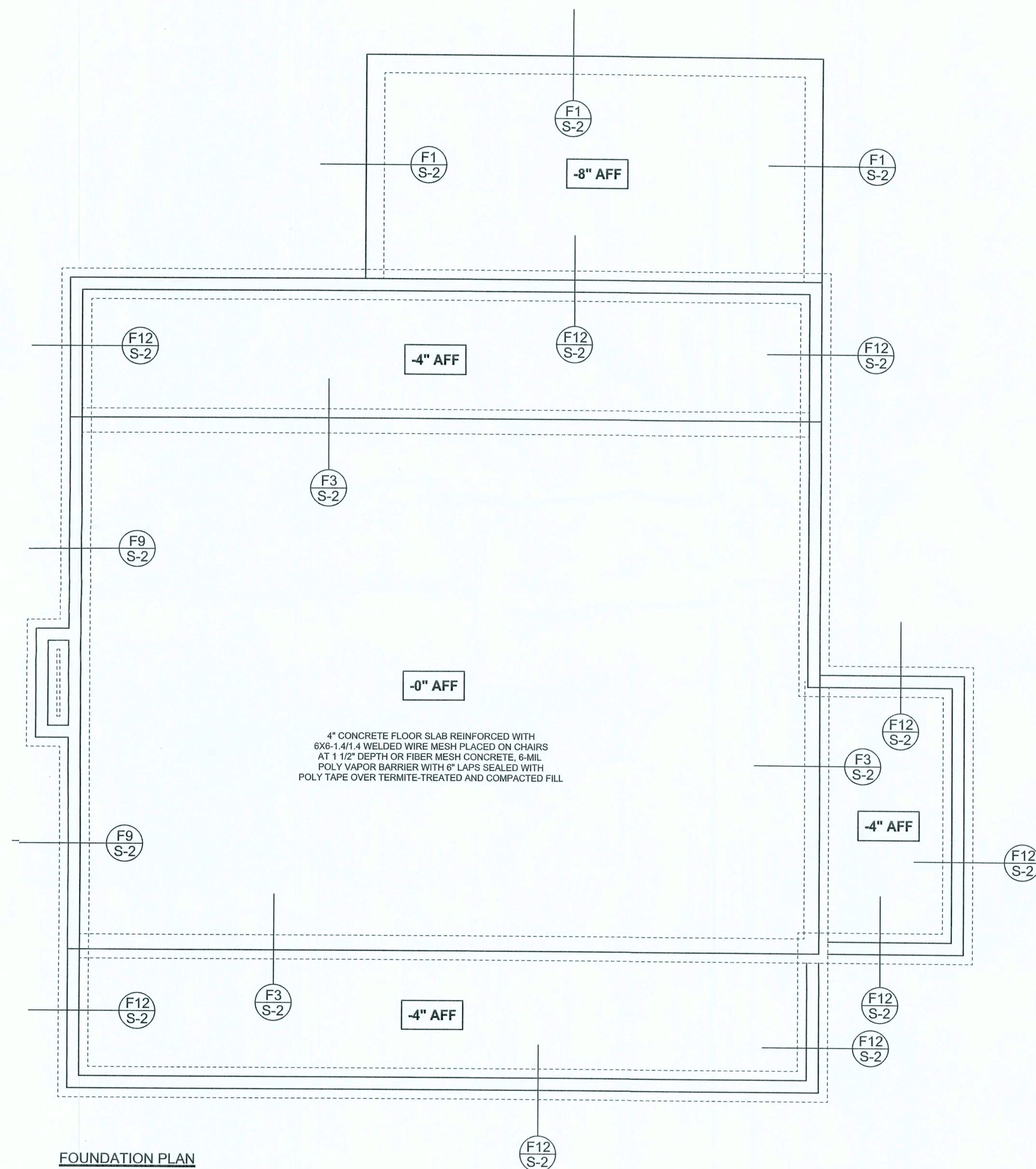


**F12 S-2** ALT. STEM WALL PORCH FOOTING  
SCALE: 1/2" = 1'-0"

#### TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16" O.C. vertically or a horizontal bond beam with 185 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



#### FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

DIMENSIONS ON STRUCTURAL SHEETS  
ARE NOT EXACT. REFER TO ARCHITECTURAL  
FLOOR PLAN FOR ACTUAL DIMENSIONS

WINDLOAD ENGINEER: Mark Disoway,  
PE No. 53815, POB 888, Lake City, FL  
32056, 386-754-5419

DIMENSIONS:  
Stated dimensions supersede scaled  
dimensions. Refer all questions to  
Mark Disoway, P.E. for resolution.  
Do not proceed without clarification.

COPYRIGHTS AND PROPERTY RIGHTS:  
Mark Disoway, P.E. hereby expressly reserves  
its common law copyrights and property right in  
these instruments of service. This document is  
not to be reproduced, altered or copied in any  
form or manner without first he express written  
permission and consent of Mark Disoway.

CERTIFICATION: I hereby certify that I have  
examined this plan, and that he applicable  
portions of the plan, relating to wind engineering  
comply with section R301.2.2, Florida building  
code residential 2004, to the best of my  
knowledge.

LIMITATION: This design is valid for one  
building, at specified location

MARK DISOWAY  
P.E. 53815

*Mark Disoway*  
02-24-06  
SEAL

B&B Homes, Inc

Rowe Residence

ADDRESS:  
626 SW Utan St.  
Ft. White, Florida

Mark Disoway P.E.  
P.O. Box 368  
Lake City, Florida 32056  
Phone: (386) 734 - 5419  
Fax: (386) 263 - 4871

PRINTED DATE:  
January 02, 2006

DRAWN BY: David Disoway  
CHECKED BY:

FINALS DATE:  
02 / Jan / 05

JOB NUMBER:  
512231

DRAWING NUMBER

**S-2**

OF 3 SHEETS



