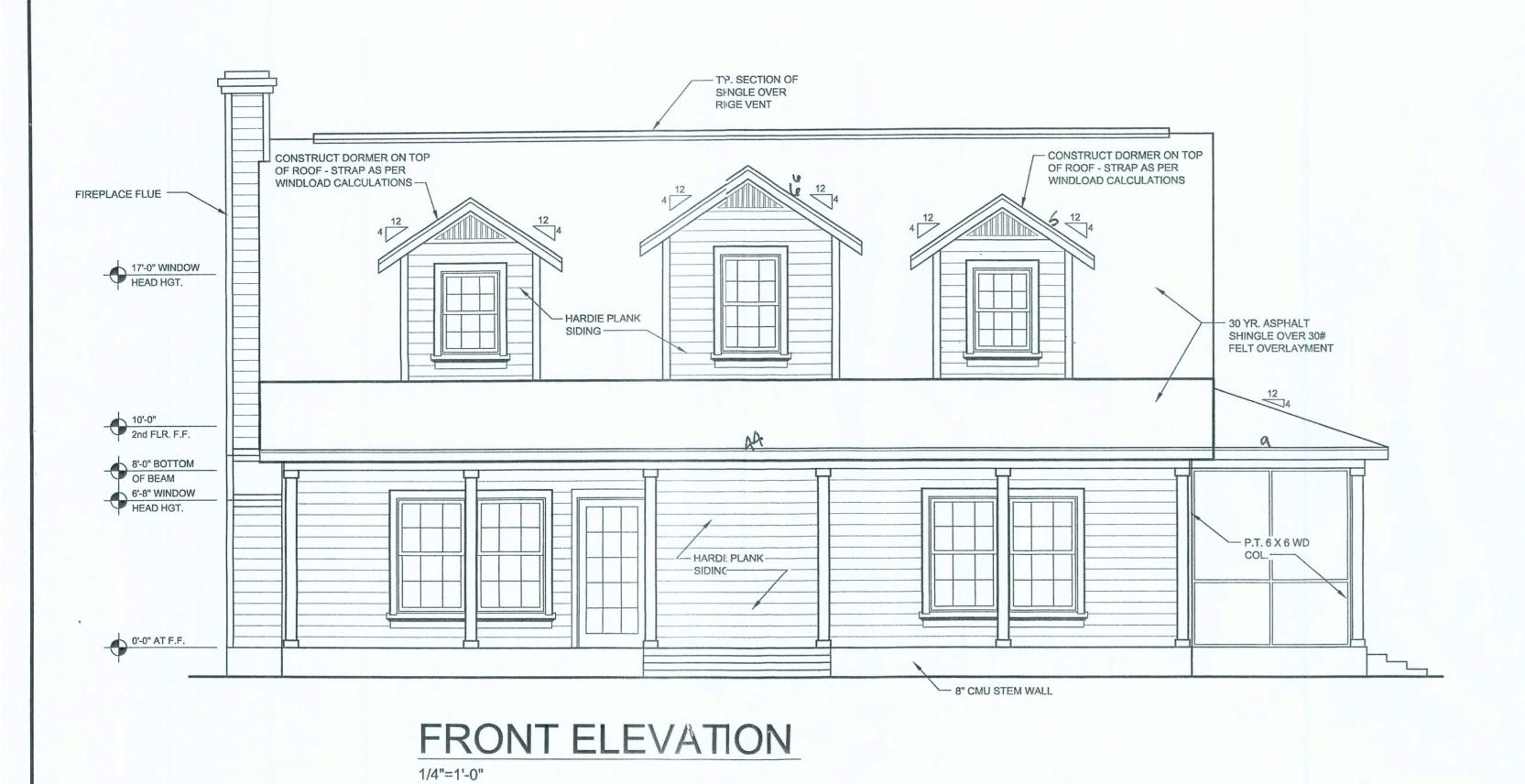
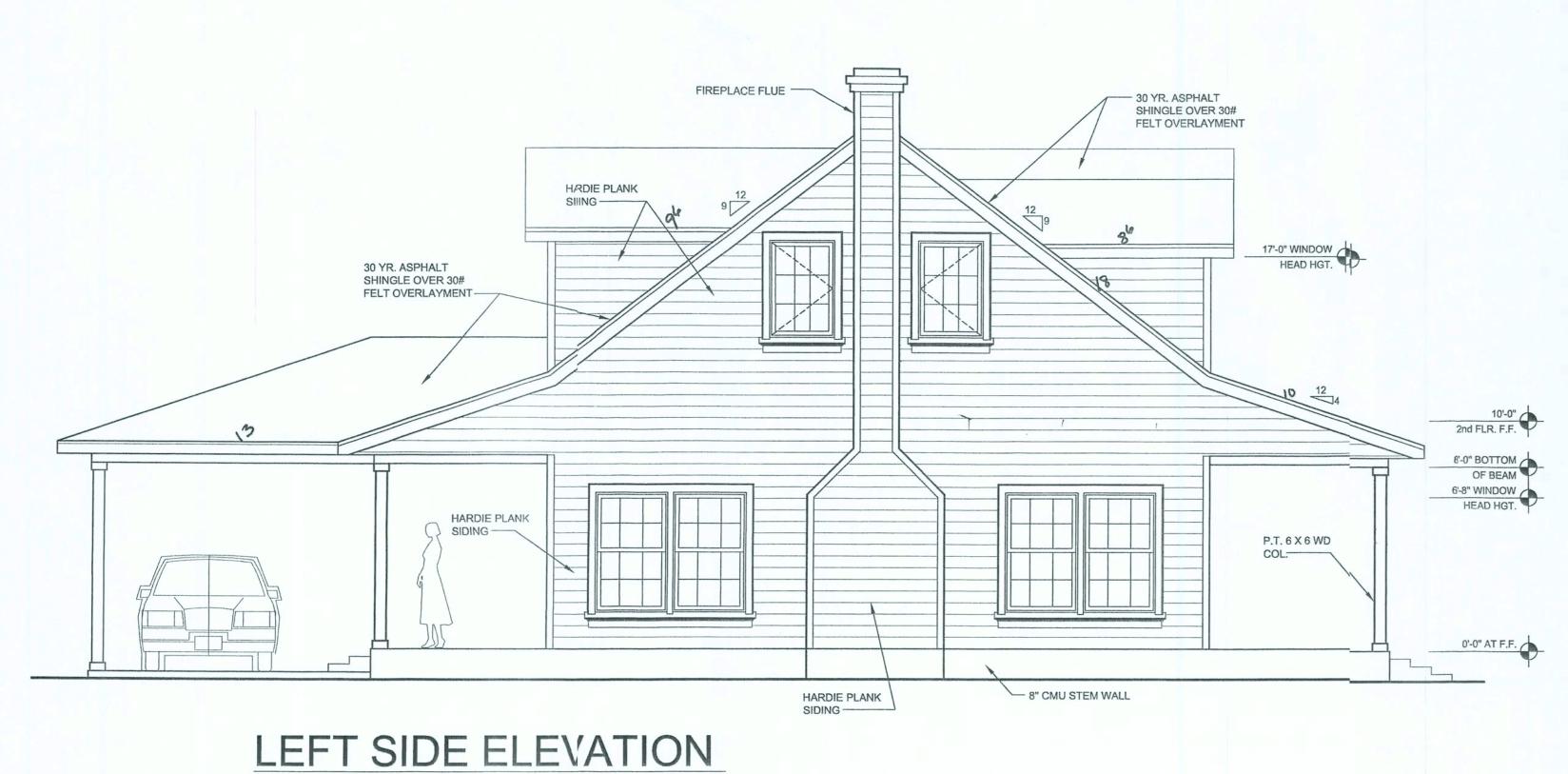
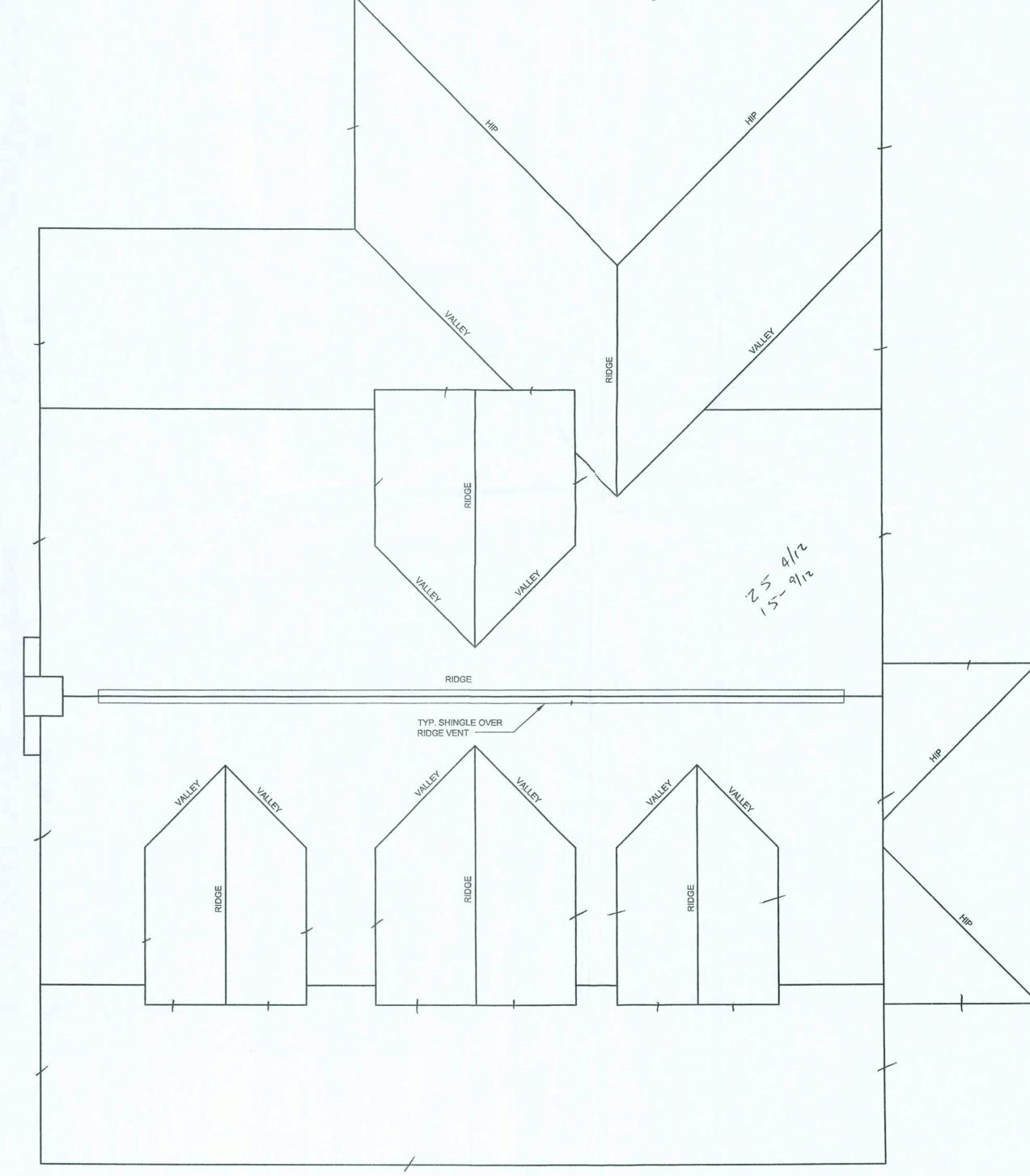


RESIDENCE

PROJECT: RCA0510 DRAWN BY: R.L.A. CHECKED: R.C. DATE: 11-09-2005







ROOF PLAN PLAN

1/4"=1'-0"

DATE 11-09-2005

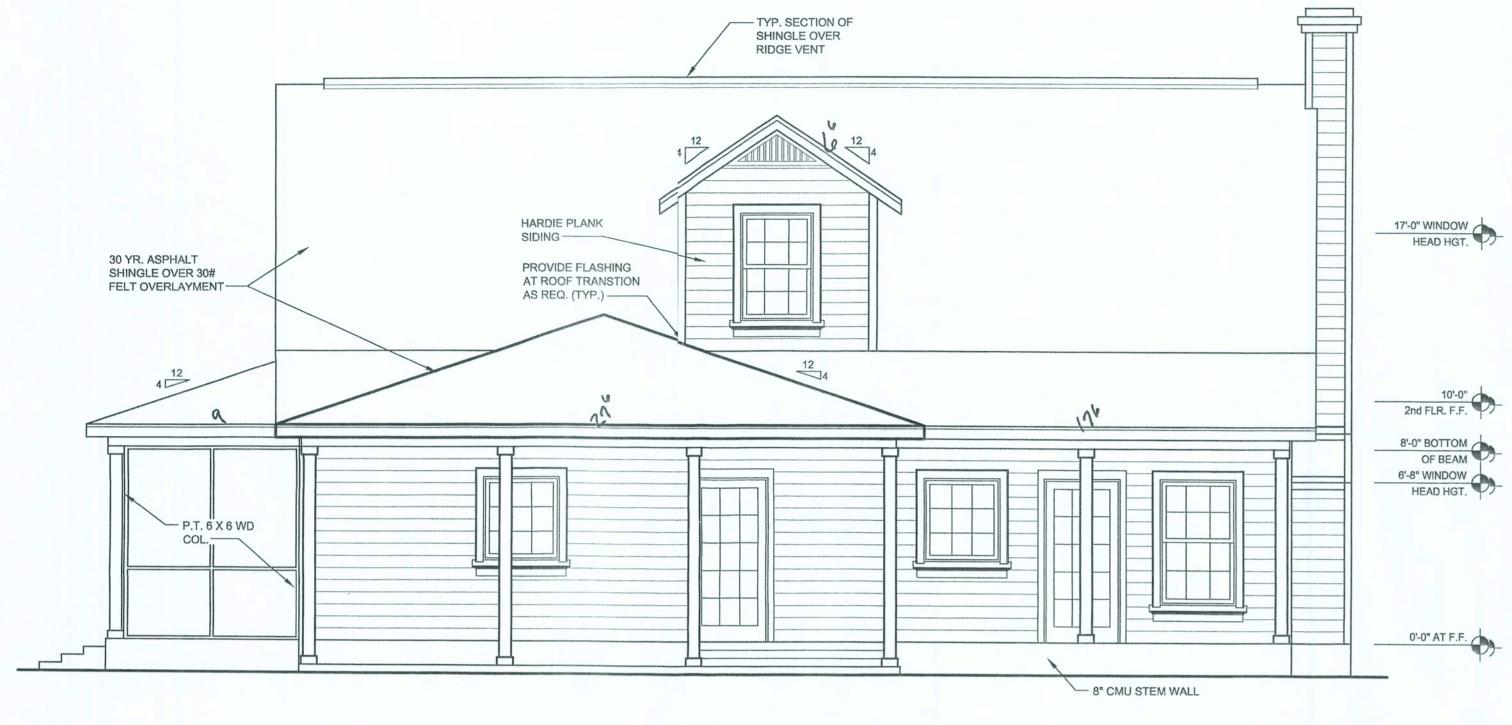
RESIDENCE

PROECT: RCA0510 DRAVN BY: R.L.A.

CHECKED:

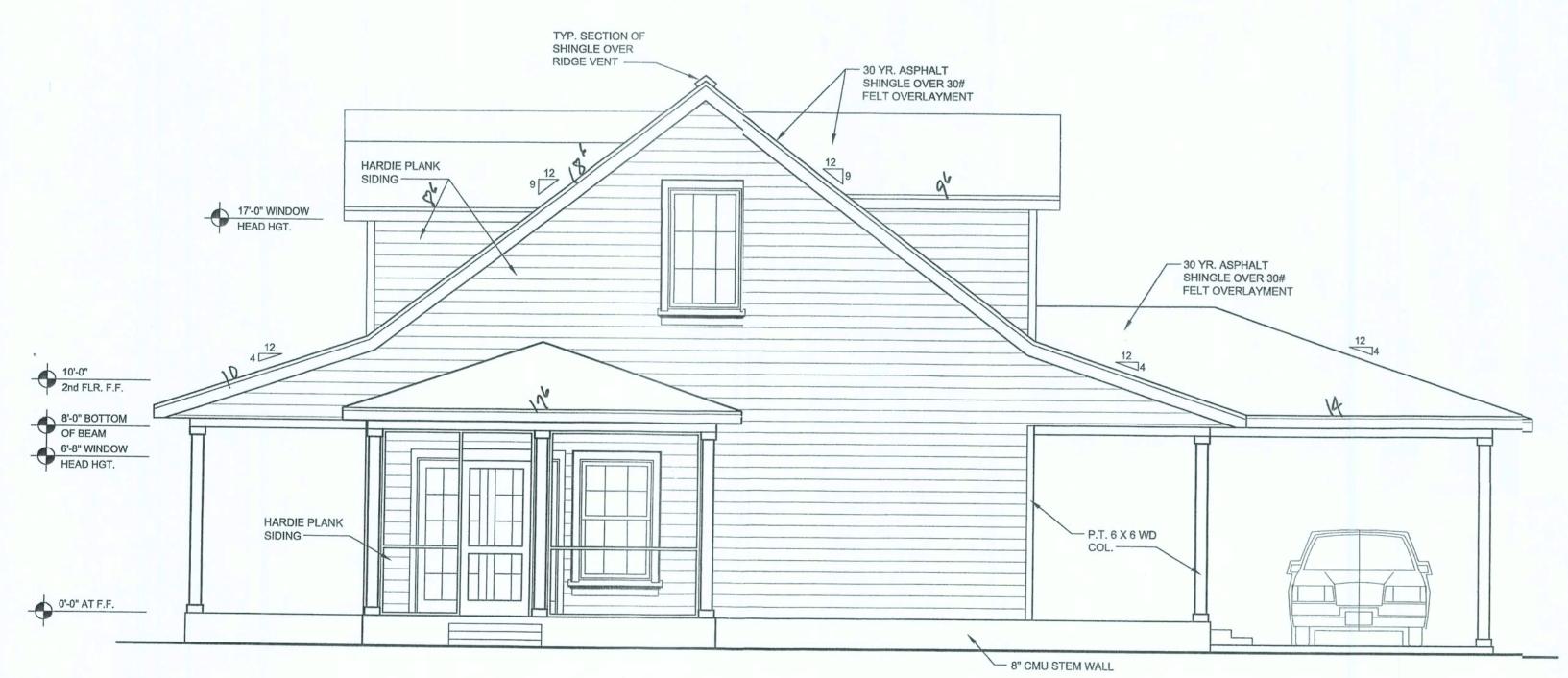
PROJECT: RCA0510
DRAWN B': R.L.A.
CHECKED: R.C.
DATE: 11-09-2005

A4



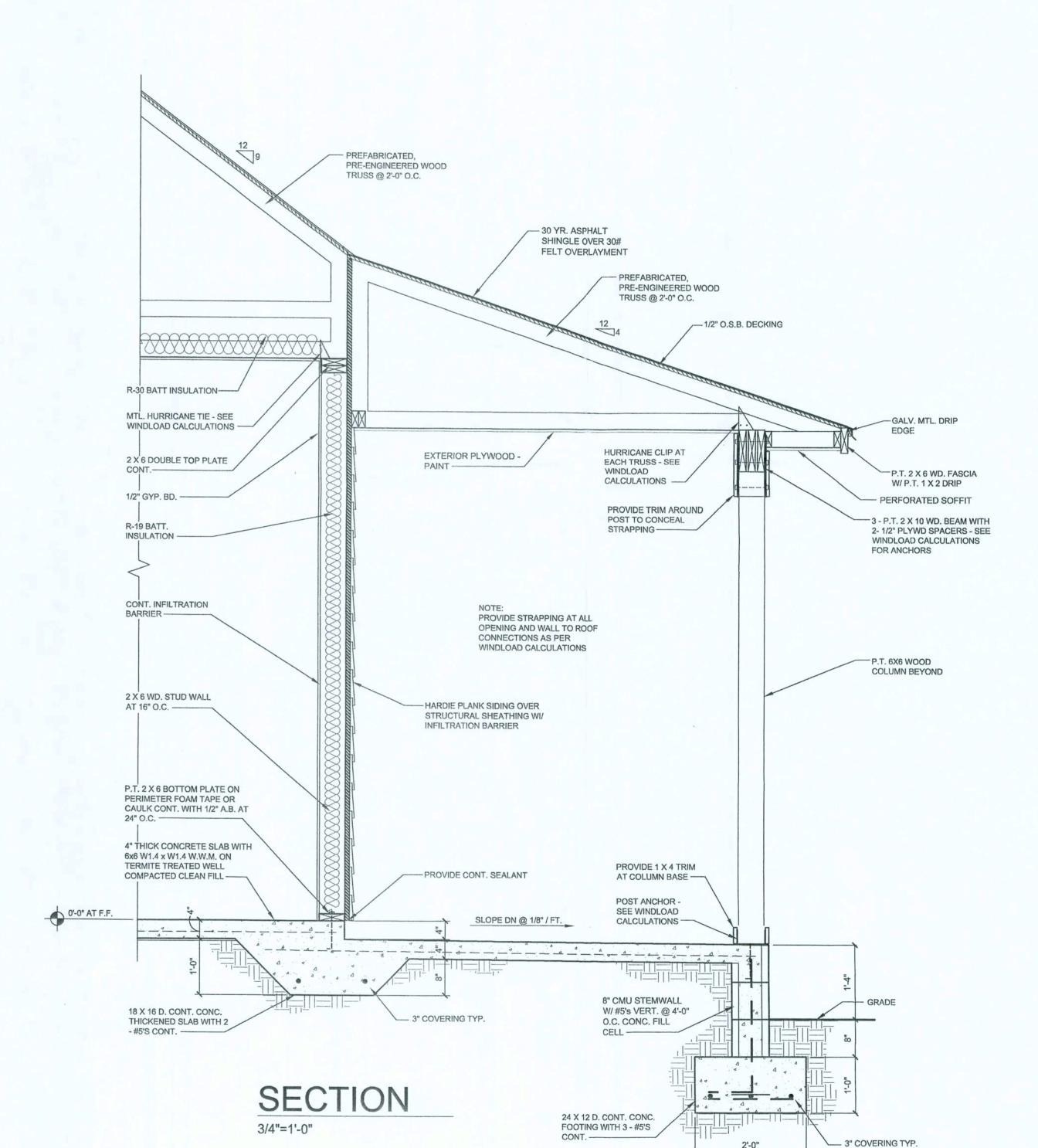
REAR ELEVATION

1/4"=1'-0"

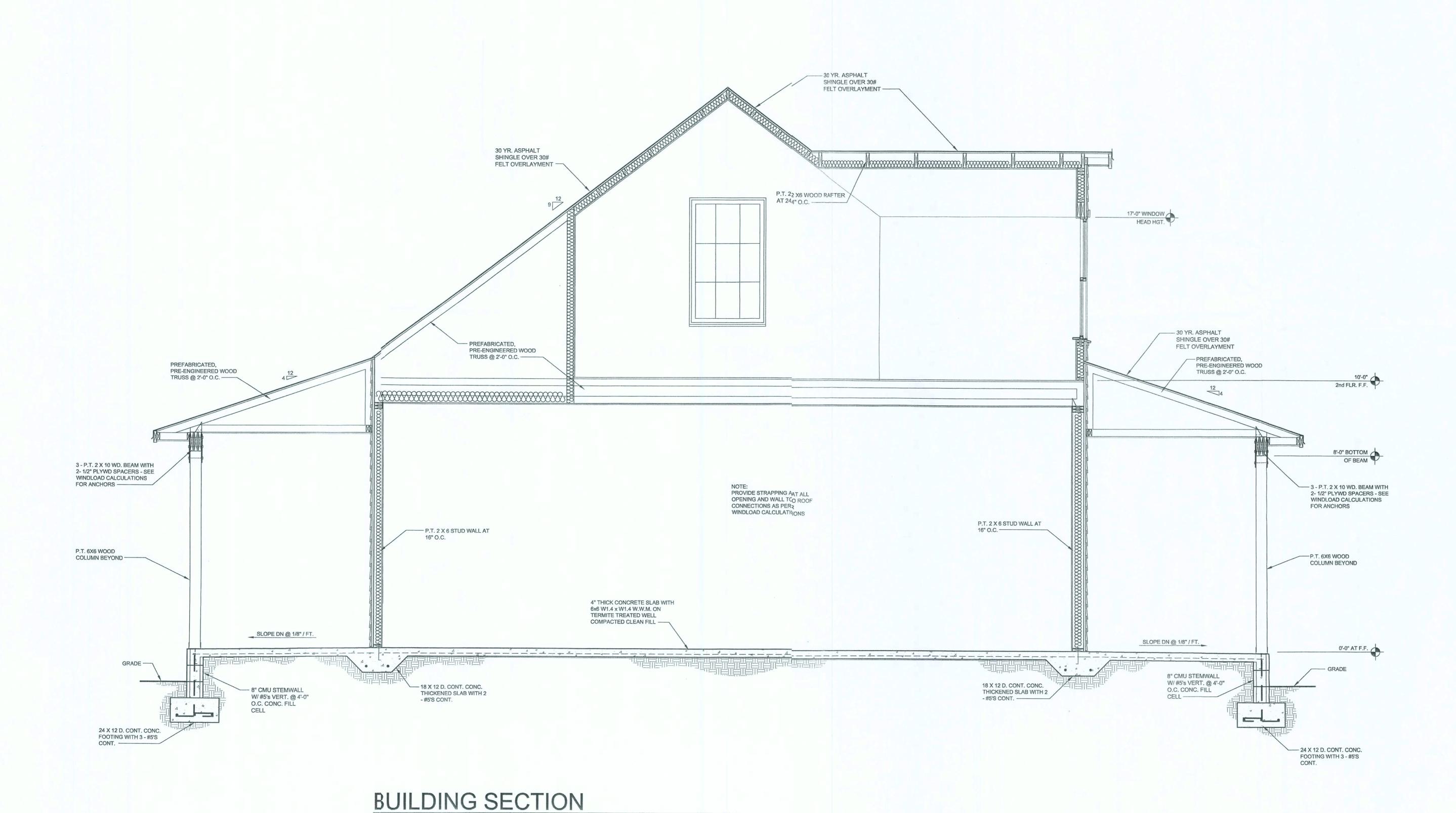


RIGHT SIDE ELEVATION

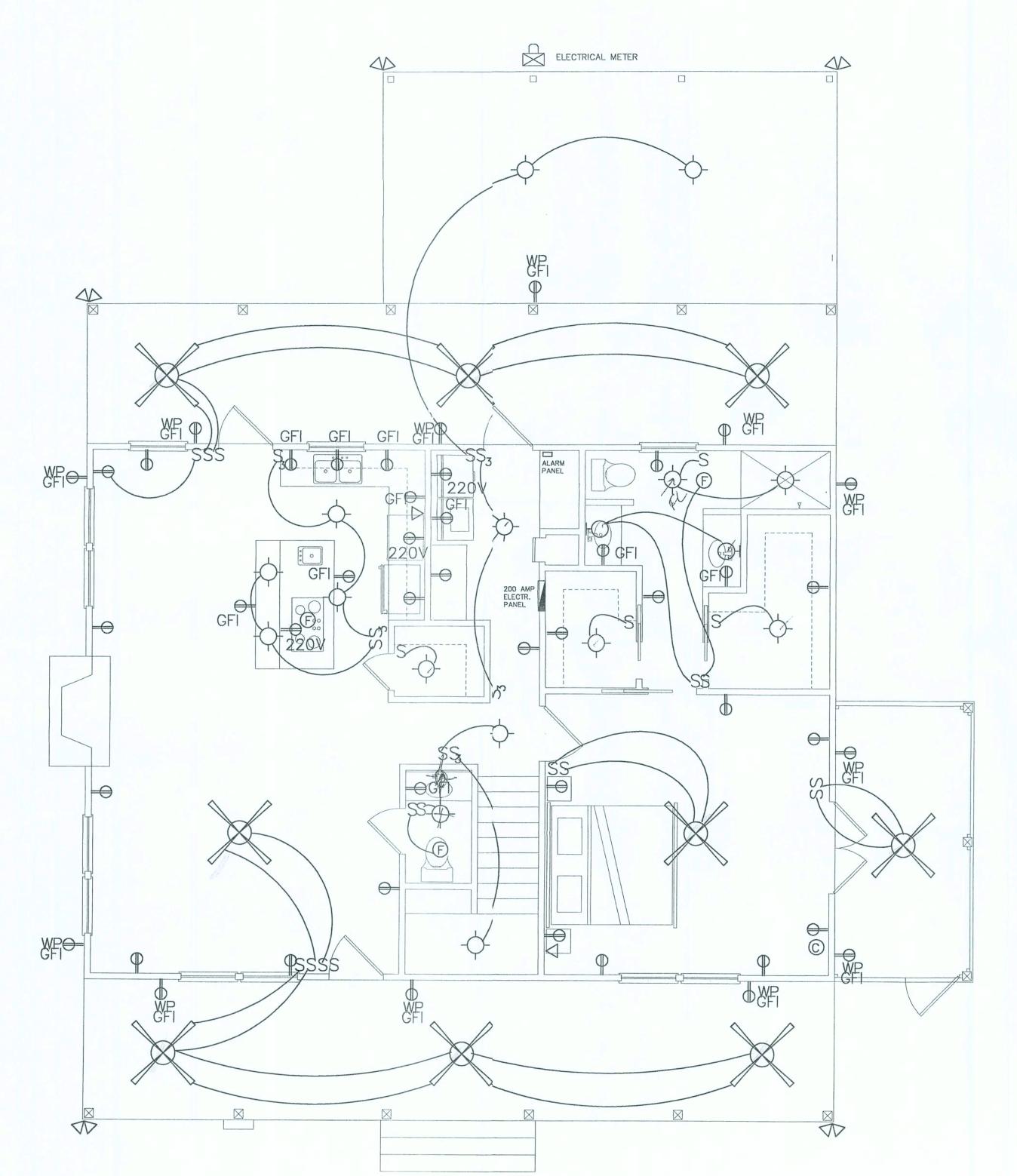
1/4"=1'-0



PROJECT: RCA0510 DRAWN BY: R.L.A. CHECKED: DATE: 1'-09-2005



1/:"=1'-0"



FIRST FLOOR ELECTRICAL PLAN

ELECTRICAL LEGEND

DUPLEX RECEPTACLE OUTLET

PHONE OUTLET

- LIGHT FIXTURE

F EXHAUST FAN

C CABLE TV

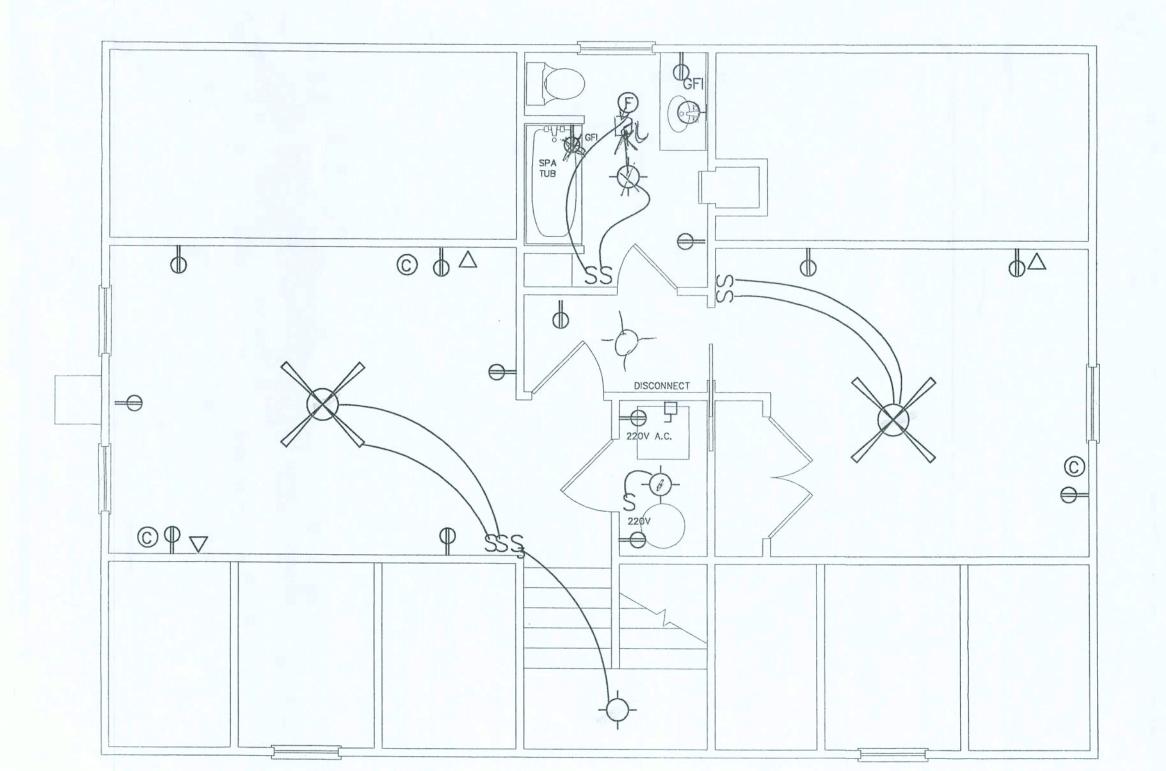
EXTERIOR FLOOD LIGHT WITH MOTION DETECTOR

ELECTRICAL PANEL 200 AMP OVERHEAD

D ELECTRICAL METER

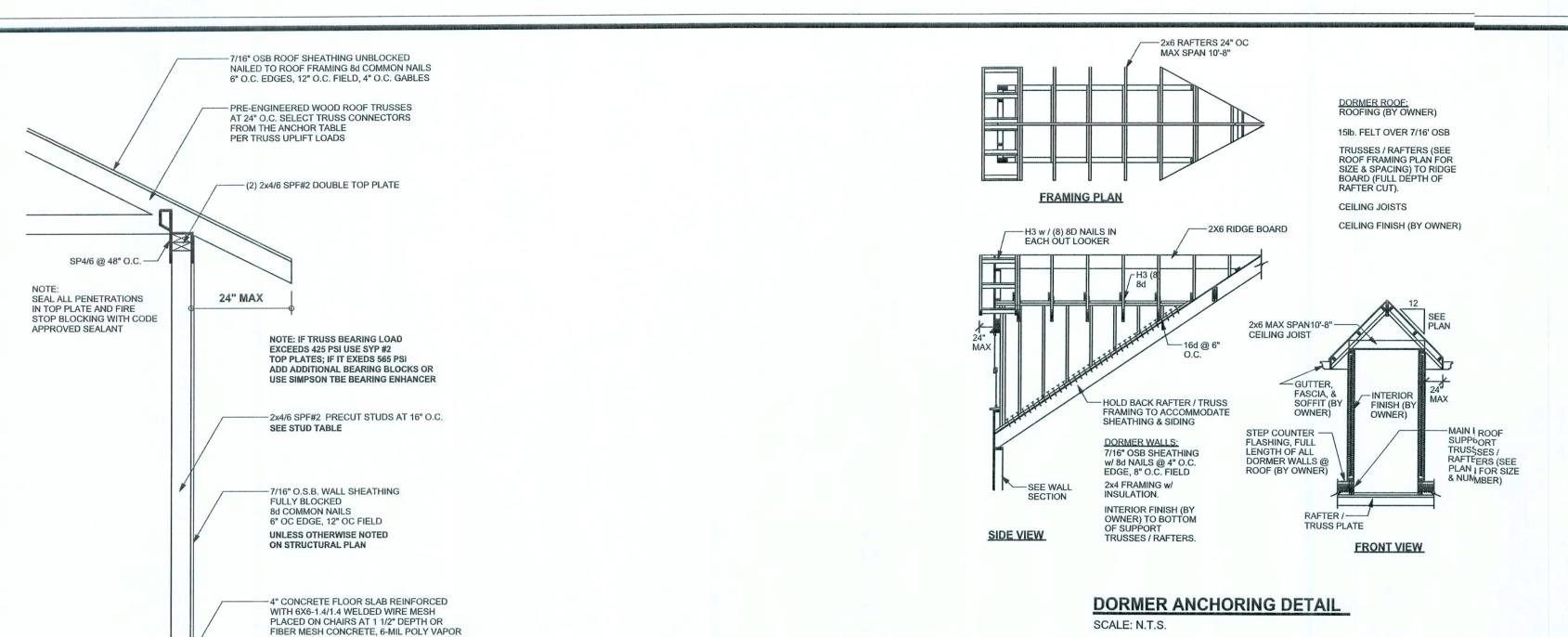
S SMOKE DETECTOR





SECOND FLOOR ELECTRICAL PLAN

1/4"=1'-0"



7/5" STRUCTURAL ROOF SHEATHING -

BDCKING REQUIRED BETWEEN OUTRIGGERS —

2X4 X-BRACE @ 6'-0" OC. -

TYPCAL GABLE END (X-BRACING)

ALL MEMBERS SHALL BE SYP

2X OUTRIGGER @ 48" OC. —

(3)131 X 3 1/4 " GUN NAILS -

4' ROM GABLE END —

2X BLOCKING @ SHEATHING JOINT

ONE STORY WALL SECTION SCALE: 3/4" = 1'-0"

SP4/6 @ 48" O.C. -

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

BARRIER WITH 6" LAPS SEALED WITH

POLY TAPE OVER TERMITE-TREATED

-2x4/6 P.T. PINE SOLE PLATE ANCHORED WITH

WITH 1/2"X10" ANCHOR BOLTS WITH 2X2X.140"

STEEL WASHER 48" O.C. & 8" FROM CORNERS

AND COMPACTED FILL

FINISH GRADE

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10' STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20.0' STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B. EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING.

EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

GRANDE & SPECIES TABLE

HURRICANE CLIP H-2.5 OR EQUAL

TOP CHORD OF GABLE END TRUSS

CONT. 2X4 SCAB FROM TOP TO

BOTTOM CHORD @ X-BRACING

(PROVIDE ADDITIONAL 2X4'S @.

VERTICAL IF HIGHER THAN 48", TO FORM AN "L" SHAPE.)

TOE NAIL TRUSS TO DOUBLE

PLATE w/ 16d COM @8" OC.

BOTTOM CHORD OF GABLE

SIMPSON LSTA 24 @ 48" OC.

END TRUSS

- 2 - 2X4 TOP PLATE

- 2X4 STUDS @16" OC.

- 2X4 BARGE RAFTER CONT

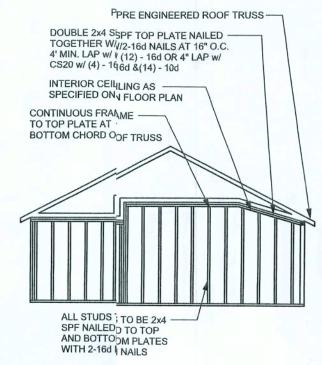
48" OC.

- FASCIA

2'-0" MAX

- SHINGLE STRIP

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0



CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL

SCALE: N.T.S.

NAIL SHEATHING TO HEADER AND TOP PLATE WITH 8d AT 4" O.C. FOR UPLIFT (6) .131 x 3 1/4" GUN NAILS ---(6) .131 x 3 1/4" GUN NAILS TOE NAILED THRU HEADER TOE NAILED THRU HEADER INTO KING STUD INTO KING STUD -LSTA418 (U.N.O.-CRIPPLES IF REQUIRED (4) .131 x 3 1/4" GUN NAILS - TOE NAIL! ED THRU SILL -INTO JACKK STUD U.N.O. TYPICAL STRAPPING (U.N.O.) (SEE STRU(CTURAL PLAN) -SP4 OR (2) H2.5A OR (2) SSP----ALL OPENINGS (U.N.O.)

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" × 6" W 1.4 × W 1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLABS: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT.

FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL. CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN

ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH /

WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT

CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO DWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.) REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS);

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" \times 2" \times 9/64"; WITH 5/8" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 3/4" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 7/8" BOLTS TO BE 3" \times 3" \times 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNE SPECIFICALLY NOT PART	ER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE T OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
CONFIRM SITE CONDITIONS, F	OUNDATION BEARING CAPACITY, GRADE AND
BACKFILL HEIGHT, WIND SPEE	D AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIALS AND CON	NSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004
REQUIREMENTS FOR THE STA	TED WIND VELOCITY AND DESIGN PRESSURES.
PROVIDE A CONTINUOUS LOAI	D PATH FROM TRUSSES TO FOUNDATION. IF YOU
BELIEVE THE PLAN OMITS A CO	ONTINUOUS LOAD PATH CONNECTION, CALL
THE WIND LOAD ENGINEER IMI	MEDIATELY.
DESIGN, PLACEMENT PLANS, T	TURER'S SEALED ENGINEERING INCLUDES TRUSS FEMPORARY AND PERMANENT BRACING DETAILS, DNS, AND UPLIFT AND REACTION LOADS FOR ALL

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

MASONRY NOTES:

Grout

3.3.E.7 | Movement joints

CMU standard

Clay brick standard

Reinforcing bars, #3 - #11

ACI530.1-02 Section

IN WRITING.

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL

CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY

MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF

ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS.

ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER

Specific Requirements

5.5"x2.75"x11.5"

Coating for corrosion protection Anchors, sheet metal ties completely

3.3.E.2 Pipes, conduits, and accessories Any not shown on the project drawings

or 304SS

ASTM C 270, Type N, UNO

8" block bearing walls F'm = 1500 psi

ASTM C 476, admixtures require approva

medium surface finish, 8"x8"x16" running

ASTM C 90-02, Normal weight, Hollow,

bond and 12"x12" or 16"x16" column

ASTM C 216-02, Grade SW, Type FBS,

ASTM 615, Grade 60, Fy = 60 ksi, Lap

splices min 48 bar dia. (30" for #5)

embedded in mortar or grout, ASTM

A525, Class G60, 0.60 oz/ft2 or 304SS

moisture or wire ties, anchors, sheet metal

ties not completely embedded in mortar or

Contractor assumes responsibility for type

and location of movement joints if not

grout, ASTM A153, Class B2, 1.50 oz/ft2

require engineering approval.

detailed on project drawings.

STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON

DESIGN DATA

ANCHOR TABLE

MANUFACTURER'S ENGINEERING

< 420

< 455

< 360

< 455

< 415

< 600

< 950

< 745

< 1465

< 1465

< 990

< 760

< 1470

< 1470

< 1000

< 1450

< 2900

< 2050

< 3965

< 10980

< 10530

< 9250

< 435

< 455

< 825

< 825

< 885

< 1240

< 885

< 1240

< 1235

< 1235

< 1030

< 1705

< 1350

< 2310

< 2775

< 4175

< 1400

< 3335

< 2200

< 2300

< 2320

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS

UPLIFT LBS. SYP UPLIFT LBS. SPF TRUSS CONNECTOR*

< 265

< 235

< 320

< 365

< 535

< 820

< 565

< 1050

< 1050

< 850

< 655

< 1265

< 1265

< 860

< 1245

< 2490

< 1785

< 3330

< 6485

< 9035

< 9250

< 435

< 420

< 825

< 600

< 760

< 1065

< 760

< 1065

< 1165

< 1235

< 1030

< 1705

< 1305

< 2310

< 2570

< 3695

< 1400

< 3335

< 2200

< 2300

< 2320

TO PLATES TO RAFTER/TRUSS

4-8d

4-8d

4-8d

5-8d

5-8d

8-8d

5-10d, 1 1/2"

12-8d, 1 1/2"

12-8d, 1 1/2"

8-8d, 1 1/2"

6-10d

2-10d, 1 1/2"

2-10d, 1 1/2"

12-10d 1 1/2" 12-10d 1 1/2"

7-10d 1 1/2"

14 -16d

22 -10d

16 -10d

16 -10d

16 -10d

3-8d

4-8d

4-8d

4-8d

5-8d

5-8d

8-8d

5-10d, 1 1/2"

13-8d

15-8d

8-8d, 1 1/2"

6-10d

10-10d, 1 1/2"

10-10d, 1 1/2"

7-10d 1 1/2"

14 -16d

3 -10d

6 -10d

14-10d

16-10d

28-8d

TO STUDS

8-16d

18-10d, 1 1/2"

2-5/8" BOLTS

18 - 16d

16-16d

16-16d

12-16d

12-16d

18 - 16d

18-8d

H2.5

H2.5A

H14-1

H14-2

H10-1

H10-2

H16-1

H16-2

MTS24C

HTS24

2 - HTS24

LGT2

EAVY GIRDER TIEDOWNS

HGT-2

HGT-3

HGT-4

STUD STRAP CONNECTORS

SSP DOUBLE TOP PLATE

SSP SINGLE SILL PLATE

DSP DOUBLE TOP PLATE

DSP SINGLE SILL PLATE

SP4

SPH4

SPH6

LSTA18

LSTA21

CS20

CS16

STUD ANCHORS*

LTT19

LTTI31

HD2A

PAHD42

HPAHD22

TO STUDS

TO FOUNDATION

5/8" THREADED ROI

12" EMBEDMENT

12" EMBEDMENT

5/8" THREADED RO

12" EMBEDMENT

-5/8" THREADED ROL

12" EMBEDMENT

TO STUDS

4 -10d

4 -10d

8 -10d

8 -10d

6-10d, 1 1/2"

10-10d, 1 1/2"

10-10d, 1 1/2"

6-10d, 1 1/2"

TO FOUNDATION

1/2" AB

1/2" AB

5/8" AB

5/8" AB

1/2" AB

1/2" AB

2-5/8" AB

5/8" THREADED ROD

_	
WIN	ID LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1
ON	CLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; AN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% OPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)
BUI	LDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE
BUI	LDING IS NOT IN THE WIND-BORNE DEBRIS REGION
1.)	BASIC WIND SPEED = 110 MPH
2.)	WIND EXPOSURE = B
3.)	WIND IMPORTANCE FACTOR = 1.0
4.)	BUILDING CATEGORY = II
5.)	ROOF ANGLE = 10-45 DEGREES
6.)	MEAN ROOF HEIGHT = <30 FT
7.)	INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
8.)	COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone Effective Wind Area (ft2) 10 100 19.9 -21.8 18.1 -18.1 19.9 -25.5 18.1 -21.8 2 O'hg -40.6 19.9 -25.5 | 18.1 | -21.8 3 O'hg -68.3 -42.4 4 21.8 -23.6 18.5 -20.4 5 21.8 -29.1 18.5 -22.6 Doors & Windows 21.8 -29.1 Worst Case (Zone 5, 10 ft2) 8x7 Garage Door 19.5 -22.9 16x7 Garage Door | 18.5 | -21.0

SOIL BEARING CAPACITY 1000PSF

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

	55 22			
DESIGN	LOADS			
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)			
	30 PSF (SLEEPING ROOMS)		 	
	30 PSF (ATTICS WITH STORAGE)		 	
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)		 	
ROOF	20 PSF (FLAT OR <4:12)			
	16 PSF (4:12 TO <12:12)			
	12 PSF (12:12 AND GREATER)			
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)		 	

REVISIONS

not to be reproduced, altered or copied in any form or manner without firstthe express writte ermission and consent of lark Disosway. CERTIFICATION: I hereby :ertify that I have examined this plan, and tha the applicable portions of the plan, relatingto wind enginee comply with section R301.21, florida building code residential 2004, to the best of my LIMITATION: This design isvalid for one building, at specified location. P.E. 539'5

PE No.53915, POB 868, Lace City, FL

Stated dimensions supercele scaled

dimensions. Refer all questons to Mark Disosway, P.E. for resolution.

Do not proceed without clarfication.

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32056, 386-754-5419

IMENSIONS

B&B Homes, Inc

ADDRESS: 626 SW Utsh St. Ft. White, Florida

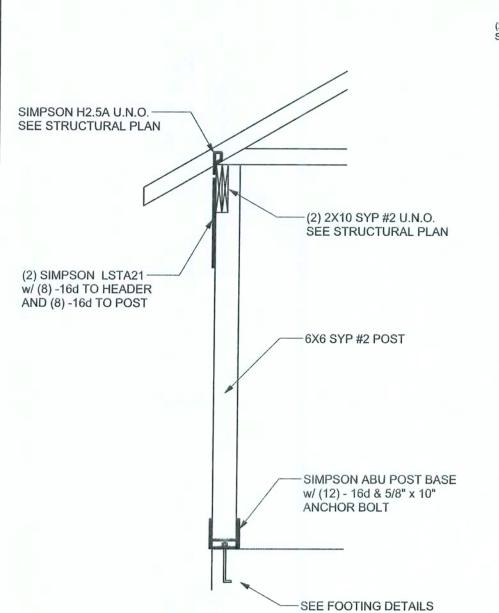
Rowe Residence

Mark Disosway P.E. P.O. Box868 Lake City, Florda 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

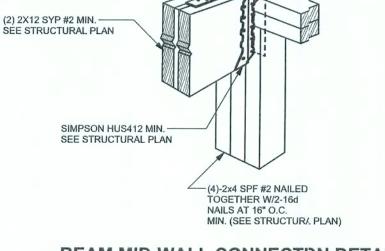
PRINTED DATE: January 02, 1006 DRAWN BY: CHECKED BY: David Disosway

FINALS DATE: 02 / Jan / 05 JOB NUMBER: 512231

> DRAWING NUMBER **S-1** OF 3 SHEETS



TYPICAL PORCH POST DETAIL



2X4 SCAB CONT. TOP TO

CHORD@ 8' FROM GABLE -

2X4 SCAB IF VERT. WEB IS

CONT. 2X4X8' #2 SYP LATERAL

2X4 BLOCKING @ 48" OC.

BETWEEN GABLE AND FIRST

NOT PRESENT -

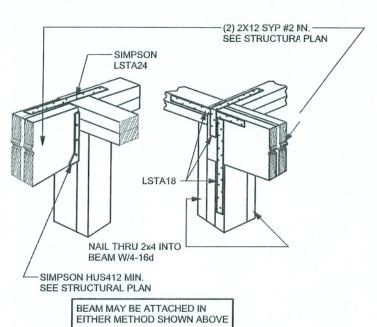
BRACE @ 48" OC. -

TRUSS.

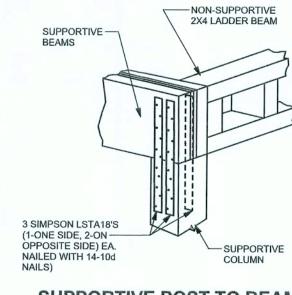
4 - 10d NAILS OR 4 - .131"x 3.25"

TYPICAL AT ALL CONNECTIONS

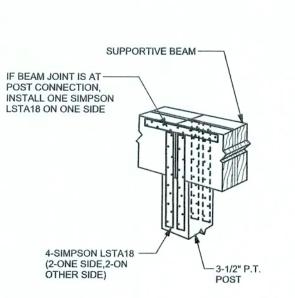
BEAM MID-WALL CONNECTION DETAIL SCALE: N.T.S.



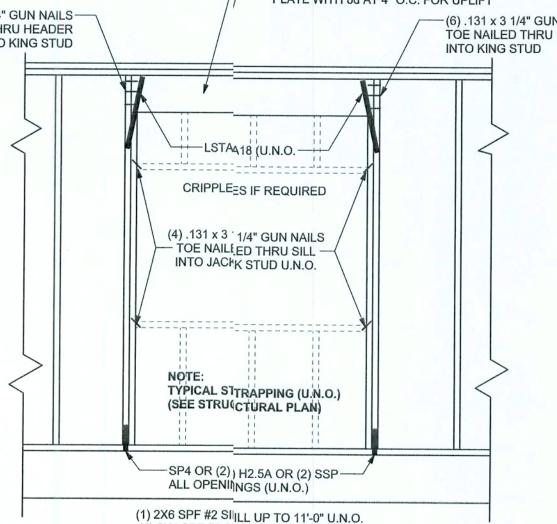
BEAM CORNER CONNECTION, DETAIL SCALE: N.T.S.



SUPPORTIVE POST TO BEAM DETAIL FOR SINGLE BEAM SCALE: N.T.S.

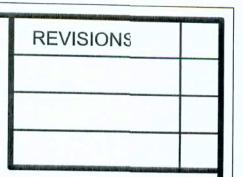


SUPPORTIVE CENTER POST TO BEAM DETAIL SCALE: N.T.S.

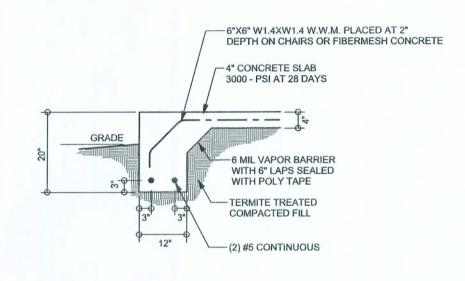


TYPICAL HEADEIR STRAPING DETAIL

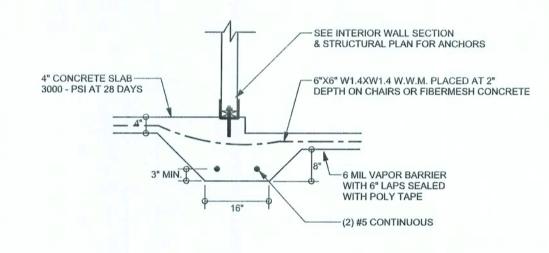
(1) 2X4 SPF #2 SILL UP TO 7'-3" U.N.O. (FOR: 110 MPH, 10')'-0" WALL HIGHT U.N.O.)



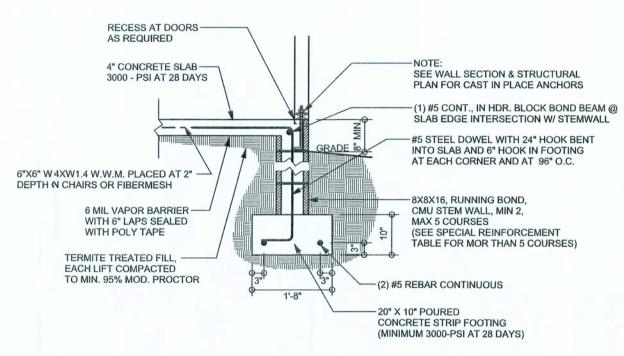
SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

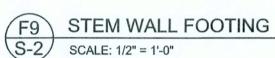


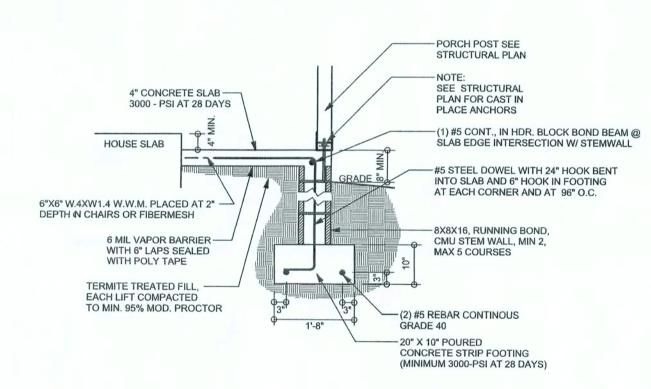




F3 INTERIOR BEARING STEP FOOTING
S-2 SCALE: 1/2" = 1'-0"





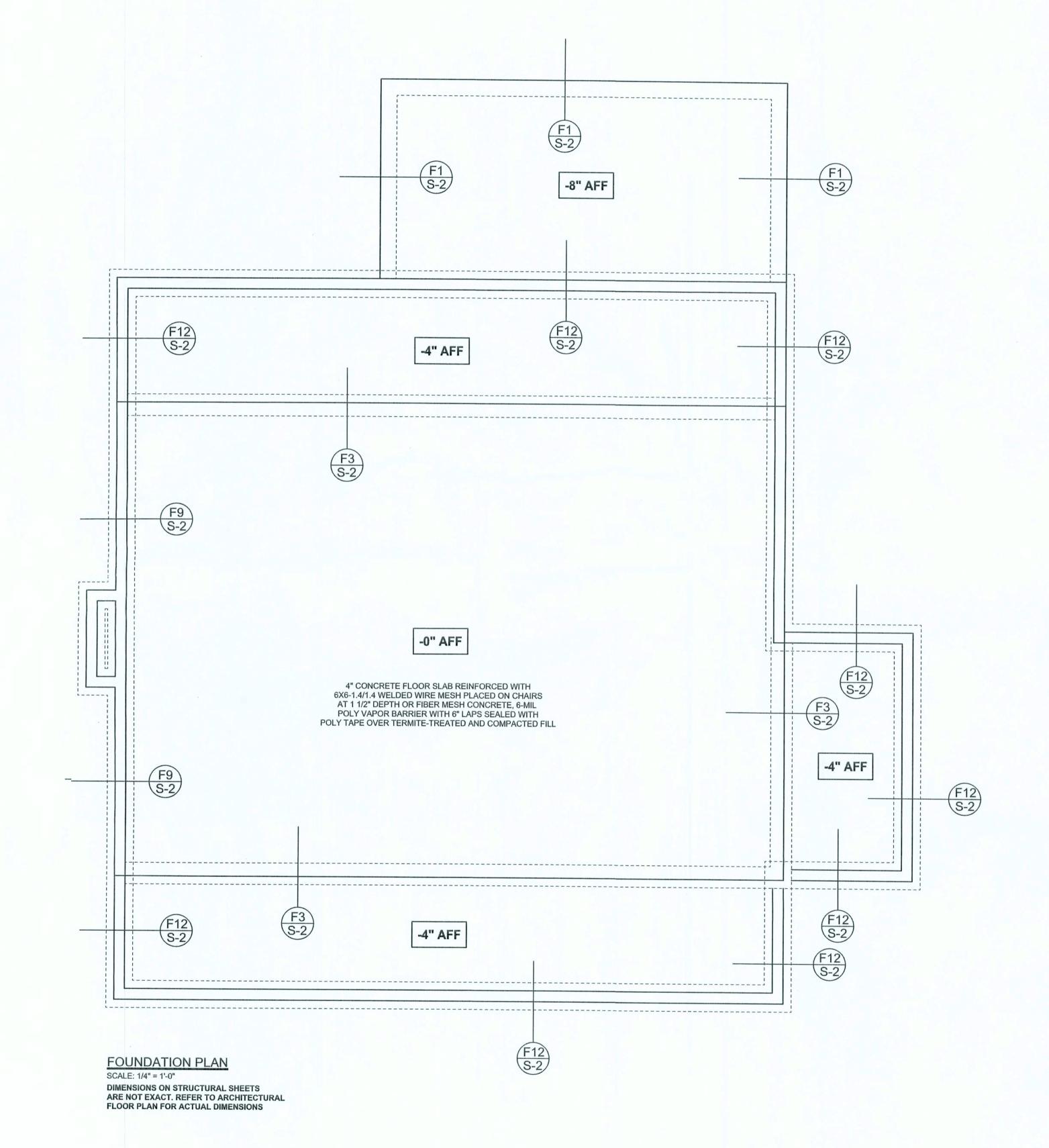


F12 ALT. STEM WALL PORCH FOOTING
S-2 SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)		VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)			
	#5	#7	#8	#5	#7	#8	
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



dimensions. Refer all questions to Mark Disosway, P.E. for resdution. Do not proceed without clarifcation.

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CERTIFICATION: I hereby critify that I have examined this plan, and that he applicable portions of the plan, relating b wind engineering comply with section R301.2.*, florida building code residential 2004, to the sest of my

WINDLOAD ENGINEER: Mak Disosway, PE No.53915, POB 868, Lale City, FL

Stated dimensions superced scaled

32056, 386-754-5419

knowledge.

LIMITATION: This design is valid for one building, at specified location

MARK DISOSVAY
P.E. 53915

MARK DISOSVAY
P.E. 53916

SEAL

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Rowe Residence

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Mark Disosway P.E. P.O. Box 368 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

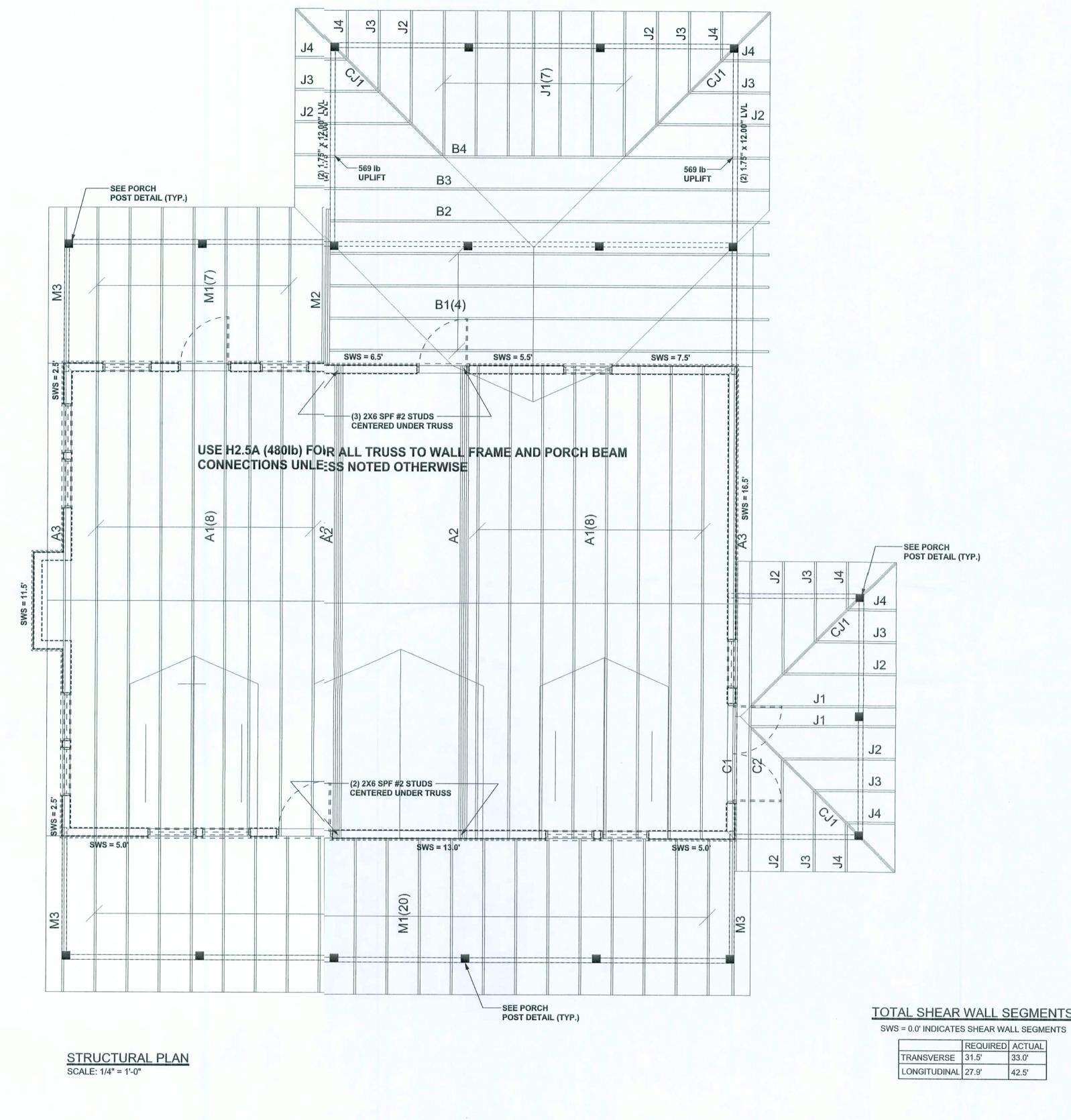
PRINTED DATE:
January 02, 2)06

DRAWN BY: (HECKED BY:
David Disosway

FINALS DATE: 02 / Jan / 05

> JOB NUMBER: 512231 DRAWING NUMBER

> > **S-2**OF 3 SHEETS



RISERS AT 7 3/4" MAX TREADS AT 9" MINIMUM (2) RISERS + (1) TREAD SHALL NOT BE LESS THAN 24" OR MORE THAN 25" TREADS 10" OR LESS MUST HAVE 1" NOSE 3/4" PLYWOOD -----2ND. FIN. FLR. NOTE: SEAL ALL PENITRATIONS IN FIRE STOP BLOCKINGWITH CODE APPROVED SEALANT -(3) 2x12 STRINGERS -5/8" TYPE "X" (FIRECODE) GYP. BOARD UNDER ALL INT. SAIRS 2x4 PLATE 1ST FIN. FLR.

> STAIR DETAIL SCALE: NTS

ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP #2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

STRUCTURAL PLAN NOTES

SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-033. BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SSEALED TRUSS PACKAGE

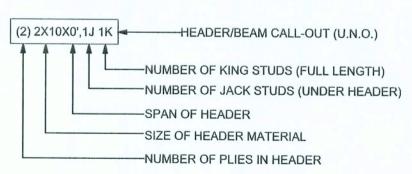
WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
SWS = 0.0'	2ND FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

TOTAL SHEAR WALL SEGMENTS

- 0.0 INDICATES	S SHEAR WA	ILL SEGIVIE
	REQUIRED	ACTUAL
TRANSVERSE	31.5'	33.0'
LONGITUDINAL	27.9'	42.5'

HEADER LEGEND



CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. MAYO TRUSS CO. INC. JOB #BB- ROWE

REVISIONS

WINDLOAD ENGINER: Mark Disosway, PE No.53915, POB 863, Lake City, FL 32056, 386-754-5419

Stated dimensions suprcede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed withou clarification.

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examined this plan, and that the applicable portions of the plan, relating to wind engineerin comply with section R31.2.1, florida building code residential 2004, o the best of my knowledge.

LIMITATION: This design is valid for one building, at specified loation.

MARK DSOSWAY P.E.53915

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PRINTED DATE: January 12, 2006 DRAWN BY: CHECKED BY: David Disosway

FINALS DATE: 02 / Jan / 05

JOB NUMBER: 512231 DRAWINGNUMBER

> **S-3** OF 3 SHEETS