

W1 - SINGLE STORY EXT. WALL SECTION

16" X 10" AB ATTACHED TO 16"

THREADED ROD WITH 1/2" COUPLE THRU COLUMN & HEADER WITH 2

WASHER & NUT TOP

%" X 10" AB ATTACHED TO %"

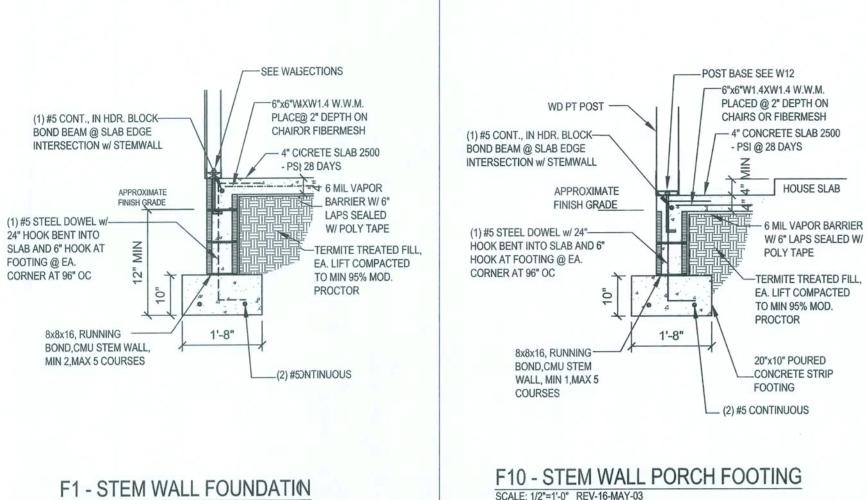
THREADED ROD WITH %" COUPLER THRU COLUMN & HEADER WITH 3"

W12 - PORCH HEADER ANCHORS

1500 LB

2300 LB

SCALE: N.T.S. REV-18-JUL-03



ONNECTION w/ (4) 12dS

12dS = 12d SINKER

OR .135"X3 1/8"

OR .131X 3 1/4"

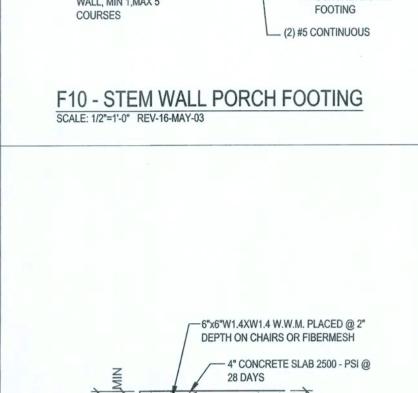
2X4 SPF #2 BLOCKING

SOE RAT RUN & DIAGONAL BRACE 6'-0" O.(

W83 - GABLE BRACING DE'AIL

FILE COFFOR GABLE HEIGHT UP TO 25'-0" 110 MPH, EX. C, ENCLOSED

SCALE: 1/2"=1'-0" REV-27-MAY-03



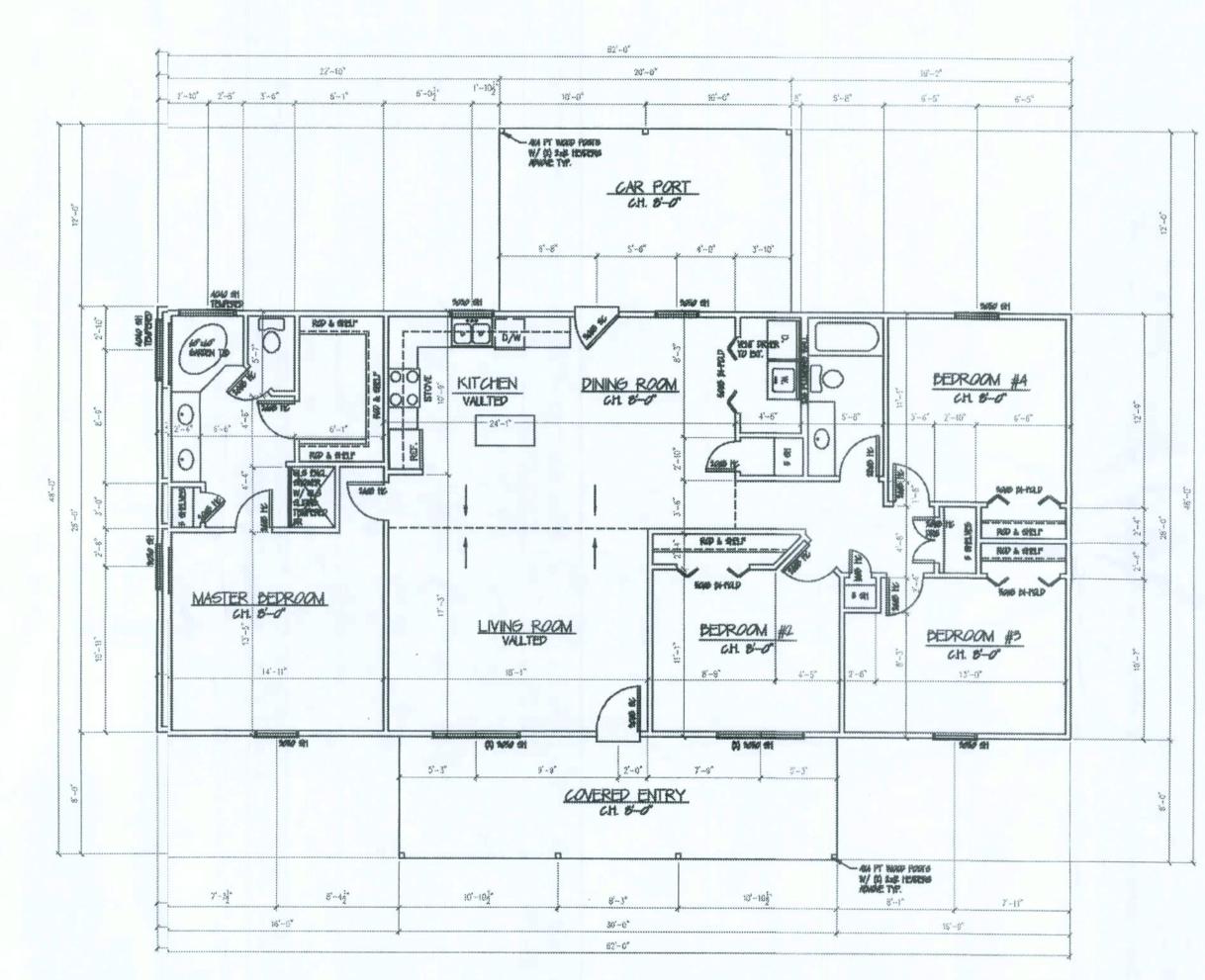
(2) #5 CONTINUOUS-

6 MIL VAPOR BARRIER W/ 6" LAPS SEALED W/

POLY TAPE

EA. LIFT COMPACTED TO

MIN 95% MOD. PROCTOR



BUILDER MUST SELECT HEADER BASED

BY SUPPLIER OR ENGINEER.

- ENDNAIL OR TOE NAIL

WITH (6) .131 X 31/4

HEADER TO HEADER STUD

TYPICAL STRAPPING

(SEE TABLE FOR SPECIFIC

EXAMPLES BASED ON

— CONNECT HEADER SUD PACK TO

HEADER CONNECTION TABLE)

SCALE: N.T.S. REV 01-JUN-06

FOUNDATION PER TRUSS UPLIFT (SEE

W13-TYPICAL HEADER SIZING & STRAPING DETAIL

TRUSS UPLIFT)

ON FBCR 2004 TABLES OR HAVE IT SIZED

-CONTINUE SPACING OF

SP4/6 STUD STRAPS OVER

CONNECT TOP OF

HEADER STUDS /

JACK STUDS TO

HEADER PER

TRUSS UPLIFT

ANCHOR BOLTS MAY

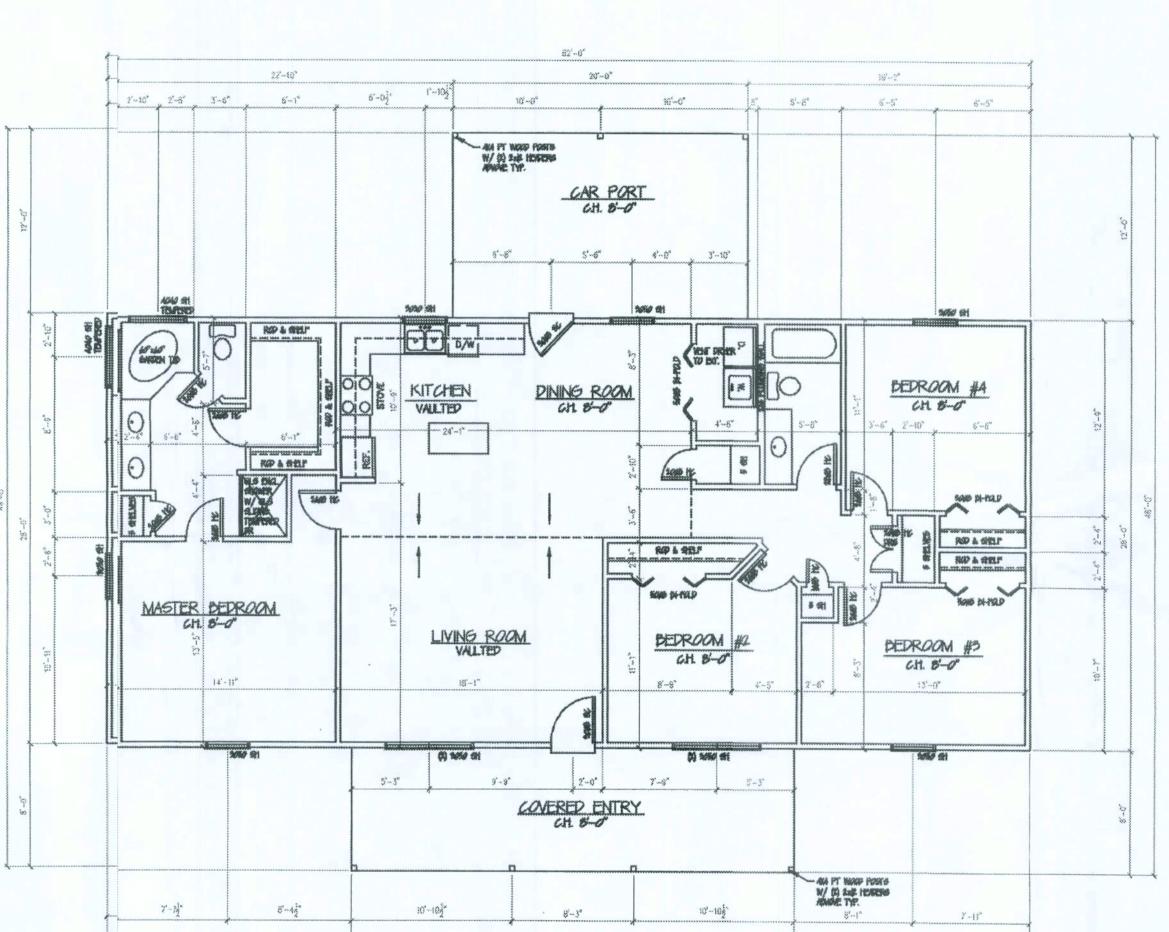
EITHER SIDE OF KING

STUDS. PLATE MUST

BE CONT. BETWEEN

**BOLT AND KING STUDS** 

BE LOCATED AT





FOUNDATION: FOR POINT LOADS GRATER THAN 5000 Ib OR REPETITIVE TRUSS LOADS GRATER THAN 2000 Ib PER TRUSS PROVIDE A THICKENED SLAB OR PAD FOOTING 1'-0"D X 1 sq ft. FOR EVERY 1000 Ib OF BEARING REINFORCE WITH #5 @ 8" O.C. EACH WAY

oad Bearing Header Sizing Methods (BY BUILDER)

Jack Studs and King Studs (BY BUILDER)

Header Uplift Connections (BY BUILDER)

dividing by the length of the header.

Use one jack stud for every 3000 lb vertical load.

connection) and stud to foundation (bottom connection).

< 800 End nail or toe nail

< 1750 LSTA18, 14-10d

< 2500 (2) LSTA18, 14-10d 2

w/6-.131"x3.25"

LSTA12, 10-10d 755

#5 < 3885 (3) LSTA18, 14-10d 3480 HTT16, 18-16c% "x10" AB 4175

FBCR 2004, TABLE R502.5(1) Header Spans Building With / Truss Span (ft)

Option # Uplift, lb. Top Connector

Uplift greater than 3885 lb requires engineering design

Header Spans For Exterior Bearing Walls

Supporting Roof+Ceiling (20psf+20psf)

NOTES: NJ = Number of jack studs

width is measured perpendicular to

the ridge. For widths between those

shown, spans may be interpolated.

Spans are based on uniform loads on

required to support each end. Building

Use supplier published data or Southern pine span tables.

Determine header size from FBCR 2004, Tables R502.5(1) or R5025(2)

6. Total king plus jack studs = studs needed to be there if no opening \as there.

Calculate the uplift at each end of the header by summing the momnts of all truss uplifts and

Bottom Coinector

(2) SP4, 6-10d1½",½" AB 1380

5 LTT20B, 10-16 ½" AB 1750

LTTI31, 18-101/2"x10" AB 2185

SP4, 6-10dx1;"

8. Select header connections from table below or mfg. catalog to connct header to stud (top

For engineered lumber beams have suppliers engineer size beam.

Lookup jack studs from FBCR 2004, Tables R502.5(1) or R502.5(2)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS SHALIBE F'C = 3000 PSI. WHERE EXCESS WATER IS ADDED TO THE CONCRETE SO THAT ITS SERVICABILITY DEGRADED, THE ATTAINMENT OF REQUIRED STRENGTH SHALL NOT RELEASE THE CONTRACTOR FROM PROVIDING SUCH MODIFICATIONS AS MAY BE REQUIRED BY THE ENGINEER TO PROVIDE A SERVICE.BLE MEMBER OR SURFACE. ALL CONCRETE SHALL BE VIBRATED. NO REPAIR OR RUBBING OF CONCRETE SURFACES SHALL BE MADE PRIOR TO INSPECTION BY AND APPROVAL OF THE ENGINEER, OWIER OR HIS REPRESENTATIVE.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE FEINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; JUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTHS SHALL BE 1/2 INCH TO 2 INCHES IN LENGTH. D(SAGE AMOUNTS SHALL BE FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD IN ACCORDANCE WITH THEMANUFACTURER'S RECOMMENDATIONS. SYNTHETIC FIBERS SHALL COMPLY WITH ASTM C 1116. THEMANUFACTURER OR SUPPLIER SHALL PROVIDE CERTIFICATION OF COMPLIANCE WITH ASTM C 1116 WIEN REQUESTED BY THE BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADESHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLCEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING O'CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROLJOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTEIDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, FY = 40 KSI. ALL LAPS SPLICESIO \* DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDAICE WITH ACI 315-95 INCHES.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTE) IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTUREFS INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIELIN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMJ.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTSTO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; NO.

WINDLOAD ENGINEERING

"EVERYTHING YOU NEED FOR YOUR BUILDING PERMIT"

Mark Disosway P.E.

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Fax: (386) 269-4871 Email: windloadengineer@belsouth.net

Destiny Lee Residence

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

SCALE: 1/2"=1'-0" REV-01-M/		AI. VVALL	SECTION	ON						
NOTE: ALL POSTS & SELECT UPLIFT  4x4/6X6  SELECT UPLIFT	SEE FOOTING DESIZE AND REINFO	SST SELECT STRAPING PER UPLIFT FROM TABLE STAILS FOR		SELECT STRAPING PER UPLIFT FROM TABLE FLOOR SYSTEM TO BE DESIGNED BY OTHERS	TRUSSES (TRUSS CO) ANCHOR T UPLIFT LO:  1/2" X 6 SPACIN  4" CONCRI PSI AT 28 I	S" WEDGE ANCHO NG PER TABLE ETE SLAB 2500		STRAF SP4/S NOTE: ALTER		BOTTOM W/ TABLE OTTOM  V.M. H ON ESH OR BARRIER PS SEALED / TAPE  ATED FILL, MPACTED
						TYPICAL TRUSS UPLIFT 400 LB	WEDGE ANCHOR SPACING 48" O.C.	SP4 / SP6 SPACING 48" O.C.	CONN'OR H2.	
							48* O.C.	32" O.C.	H	
	2" / 3" WASHER & N	NUT				600 LB		16* O.C.	HTS	
						1000 LB	32* O.C.		(2) HTS20ILED TO	
	LOAD BEARING HOLLOW COLUM	M				2200 LB	WEDGE ANCHOR	N/A	STUECK	
	SYP #2 HEADER	<b>V</b>					ERIOR B 0" REV-22-AUG-		FOUING	2
- ½" / - ½" /	%" THREADED RI /%" COUPLER /%" X 10" AB  SEE FOOTING DE SIZE AND REINFO	TAILS FOR			7/16" OSB 8d 6" ( EDGE & 12" O.C.			EDGE, 12" O.C.	F SHEATING 8d 6 FIELD, & O.C. G QUIRED TWEEN — (4) 12d	ABLES OUTRIGGERS
							F #2 DIAGONAL I LOCKING AT TOP			3) 12dS
	SYP #2 PT WD POSTS						D AND RAT RUN			
TYPICAL POST UPLIFT	POST BASE ANCHOR	BETWEEN FLOOR STRAPING	HEADER STRAPING		ATTACH RAT R	UN TO		/X		E MUST BE NAILE
555 LB	ABA44 W/ (6)-10d & ½" AB	(2) LSTA21 W/ (6)-10d EA.	(2) LSTA21 W/ (6)-10d EA.		BLOCKING w/ (4	1) 12dS	1740		OVER' IT MAY	S FOR LENGTH BE "T" BRACED U
720 LB	ABA66 W/ (8)-16d & <b>5⁄8"</b> AB	(2) LSTA21 W/ (8)-10d EA.	(2) LSTA21 W/ (8)-10d EA.		TOE NAIL TRUS		12dS -(4) 12dS	30 45	TO 12ND UNBI	RACED UP TO 7'
2200 LB	ABU44 W/ (12)-16d, (2)½ BOLTS & % "AB	(2) LSTA21 W/ (16)-10d EA.	(2) LSTA21 W/ (16)-10d EA.		12d @ 6" O.C.			1		
2300 LB	ABU66 W/ (12)-16d, (2)½ BOLTS &5/6*AB	(2) LSTA21 W/ (16)-10d EA.	(2) LSTA21 W/ (16)-10d EA.				(4) 120	dS		AT RUN NAIL EACH

TRUSSES	INEERED WOOD F @ 24" O.C. SELE	CT	2X4 / 2X6	6 STUDS AT 16" OC. S	SPF #2
ANCHOR UPLIFT LO	TABLE PER TRUS	s _\_		AP STUDS TOP AND SP4 / SP6 SPACING F E: SP2 TOP & SP1 B ERNATE FOR SP4/6	PER TAB
SPACIN	G PER TABLE		/ /	- 6"X6" W1.4XW1.4 W.	
4" CONCI PSI AT 28	RETE SLAB 2500 - DAYS			PLACED AT 2" DEPT CHAIRS OR FIBERM	
			4	6 MIL VAPO WITH 6" LAI	PS SEAL
(2) #5 C	ONTINUOUS ON THE PROPERTY OF T	1'-4"	4 8	WITH 6" LAI WITH POLY TERMITE FILL, EAG COMPAC MIN. 95% PROCTO	PS SEAL TAPE TREAT CH LIFT CTED TO 6 MOD.
(2) #5 C	ONTINUOUS TYPICAL TRUSS UPLIFT	1'-4"  WEDGE ANCHOR SPACING		WITH 6" LAI WITH POLY TERMITE FILL, EAG COMPAC MIN. 95%	PS SEAL TAPE TREAT CH LIFT CTED TO 6 MOD.
(2) #5 C	TYPICAL	WEDGE ANCHOR	SP4/SP6	WITH 6" LAI WITH POLY  TERMITE FILL, EAI COMPAC MIN. 95% PROCTO	PS SEAL TAPE TREAT CH LIFT CTED TO 6 MOD.
(2) #5 C	TYPICAL TRUSS UPLIFT	WEDGE ANCHOR SPACING	SP4 / SP6 SPACING	WITH 6" LAI WITH POLY  TERMITE FILL, EAG MIN. 95% PROCTO  TRUSS CONNECTOR	PS SEAL TAPE TREAT CH LIFT CTED TO 6 MOD.
(2) #5 C	TYPICAL TRUSS UPLIFT 400 LB	WEDGE ANCHOR SPACING 48" O.C.	SP4 / SP6 SPACING 48° O.C.	WITH 6" LAI WITH POLY  TERMITE FILL, EAI COMPAC MIN. 95% PROCTO  TRUSS CONNECTOR  H2.5A	PS SEAL TAPE TREAT CH LIFT CTED TO 6 MOD.

F6 - CAR PORT MONOLITHIC FOOTING

Uplift SPF	Uplift SYP	Truss Connector	To Plate	To Truss / Rafter
320	455	H3	4-8d	4-8d
245	350	H5A	3-8d	3-8d
535	600	H2.5A	5-8d	5-8d
620	720	H10	6-10dx13/"	6-10dx 1½"
850	990	LTS12	8-8dx11/5"	8-8dx 1½"
1245	1450	HTS20	10-10d or 12-10dx11/2"	10-10d or 12-10dx 1½"
1265	1470	H16, H16-2	10-10dx1½"	2-10dx 1½"
1785	2050	LGT2	14-10d Sinker	16-16d Sinker
3655	4200	MGT	%" Thd. Rod	22-10d
SPF	SYP	Strap Connector	To One Member	To Other Member
760	885	SP4	6-10dx1½"	N/A
865	1005	CS20	9-8d or 7-10d	9-8d or 7-10d
1085	1265	LSTA18-24	7-10d	7-10d
1170	1360	SPH4	12-10dx1½"	N/A
1420	1650	CS16	14-8d or 11-10d	14-8d or 11-10d
SPF	SYP	Column Anchor	To Foundation	To Column / Truss
1160	1350	LTT19	5⁄8"x 16" AB	8-16d Sinkers
1985	2310	LTTI31	5⁄8"x 16" AB	18-10dx 1½"
2385	2775	HD2A	%"x 16" AB	2-5/8" Bolts
3590	4175	HTT16	%"x 16" AB	18-16d
1975	2300	ABU66	%"x 16" AB	12-16d

Studs Supporting Trusses: The builder is responsinsible for gravity loads, but you should put an extra 2x4 stud under truss bearing location

Manufacturer and product number are listed for expression to the tribs and top plate (31", 10"-12") and the same or other manufacturer can be substituted for any devices listed in the expression installation instructions must be followed to achievible as long as it meets the required load capacities. Manufacturer's installation instructions must be followed to achievible related loads, All connections exposed directly to the weather shall be hot dipped galvanized after fabrication. Loads are increased and for wind duration. Strap uplift may be reduced proportionally to number of nails.

See spec sheet for alternate nail sizes (10d=.84\*) 41\*(6d, 10dx1½"=.80\*10d, 10d=12d=16d sinker). SPF=.86\*SYP

N5 - TRUSS UPLIFT COONNECTOR TABLE REV-25-AUG-03

He'eader Span vs Load

3 4 5 6 7 8 8 9 10 11 12 13 14 15 16

Header Clear (r Span Between Supports (ft)

Deffection limits: TL=L/240, LL=L/180.

Duration factor, Cd = 1.25, applied to Fb and Fv. (No inc increase to E or Fc for duration of load.)

(For non-uniform loads sum the loads on the header, didude by header span; and multiply by 2.)

2. Duration factor, Cd = 1.25, applied to Fb and Fv. (No incincrease to E or Fc. for duration of load.)
3. 2x headers are SYP#Z with OSB litch spacer and (2) 1/2 12dS 16°OC. (strength increased 5% for OSB)
4. 3.5° x 14° GLB is Anthony Power Header, 2800 Fb. 1.9° 9 E. or GP (2)1°75x14 LV., 2850 Fb. 2.0 E
5. 3.5° x 12° LVL is Blue Linx. GP Lam, (2) 1.75° x 11.78° 78° LVL 2900 Fb. 2.0 E. 285 Fv.
6. Minimum bearing for SYP header, Fc = 565psi, 2500 lb (lb) per jack stud. (For SPF plate, Fc = 425psi, 2200 lb per 7. Shear strength is increased 50% for minimal splits and 5d checks headers cut to size on site.
8. Deflection has not been adjusted for creep due to long te; term loading, Kcr = 1.0
9. The chart is based on NDS2001 bending, horizontal she-hear, and deflection requirements.

Haif of 7" truss span + 24" overhang

Half of 8' truss span + 24" overhang

Half of 20' truss span + 24" overhang Half of 24' truss span + 24" overhang

Half of 32' truss span + 24" overhan Half of 40' truss span + 24" overhang Half of 60' truss span + 24" overhang

Haif of 24' truss span and 14' bonus roo

W71 - HEADER SPAANS FOR ROOF/CEILING LOAD

1100 -

1000

900

800

700

600

500

400

300 -

Garage width, 20 Garage width, 24

Bonus Room, above garage

Truss Design Loading: LL = 20psf, DL = 20psf 7' Jack Truss :

Basic Wind Speed	110	MPH				
Wind Exposure	В					
Wind Importance Factor	1.0					
Building Category	II					
Internal pressure Coefficient	N/A	Enclos	ed)			
Building not in the high velocity hurricane zone						
Building not in the wind-borne debris region						
Mean Roof Height	< 30	ft				
Roof Angle	10-4	5 degre	ees			
Components And Cladding Wind Pressures (FBCR	Table R301.2(2)					
A.	Zone	Effect	ive Win	d Area	(ft2)	
		10		100		
	4	21.8	-23.6	18.5	-20.4	
	5	21.8	-29.1	18.5	-22.6	
5	Total S	Shear V	Vall Seg	ments		
2 2 2 5	2*-4*min for 8	3'-0"H wal	2'-10"mir	n for 10'-0'	'H wall	
		Trans	verse	Longi	tudinal	
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1				30.0'		
55	Required	33.5'		30.0		
555	Required Actual	33.5' 47.5'		80.0'		
555	Actual All exterio	47.5' walls		80.0'		
555	Actual All exterio	47.5' walls a	WALL I	80.0' Il shear ength is	the tota	
555	Actual All exterio ACTUAL S of all wall	47.5' walls a SHEAR segmen	WALL I	80.0' Il shear ength is full heigl	the tota	
1 55 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	Actual All exterio ACTUAL 3 of all wall sheathing	47.5' r walls a SHEAR segment and wide	WALL Ints with f	80.0' Il shear ength is full heigl eight rati	the total nt o greate	
3 2 2 2 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	Actual All exterio ACTUAL 5 of all wall sheathing than 1: 3.5	47.5' r walls a SHEAR segment and wide (plus s	WALL leads with for the dispecial s	80.0' Il shear ength is full height right ration	the tota nt o greate	
\$\frac{1}{2}\frac{1}\frac{1}{2}\f	Actual All exterior ACTUAL 5 of all wall sheathing than 1: 3.5 segments	47.5' r walls a SHEAR segment and wide (plus siff noted	WALL Ints with for the with to he special so I.) REQU	80.0' Il shear ength is full height eight ratifichear was signed to the shear was signed to the shear was signed to the shear was signed to the shear	the total of greate all SHEAR	
2 2 2 5 5 5	Actual All exterio ACTUAL 5 of all wall sheathing than 1: 3.5	47.5' r walls a SHEAR segment and wind for (plus siff noted gith is from	WALL Into with for the special some WFC	80.0' Il shear ength is full height ration thear was JIRED St.	the total of greate all SHEAR	
3 3 2 2 3 3 2 4 3 3 4 5 5 5 5 5 5 5 5 5 7	Actual All exterior ACTUAL Sof all wall sheathing than 1: 3.5 segments WALL length	47.5' r walls a SHEAR segment and wide (plus segment) for the first the firs	WALL lets with footh to he special so I.) REQUOM WFC	80.0' Il shear ength is full height ight ration hear watth JIRED S CM-2001 3.17E	the total nt o greate ill SHEAR I, table	

equivalent calculation)

N4-WIND LOAD DESIGN DATA

(Wind loads are per FBCR 2004, Section R301.2.1 for enclosed simple diaphragm buildings with mean roof

N3-WINDLOAD ENGINEER'S SCOPE OF WORK: The wind load engineer is engineer of record for compliance of the structure to wind load requirements of FBCR 2004, Section R301.2.1. If trusses are used, the wind load engineer is

not engineer of record for the trusses and did not design the trusses or delegate to the truss designer. BUILDER'S RESPONSIBILITY: The builder and owner are responsible for the following, which are specifically not par

of the wind load engineer's scope of work. \* Confirm that the foundation design & site conditions meet gravity load requirements (assume 1000 PSF bearing capacity unless visual observation or soils test proves otherwise \* Provide materials and construction techniques, which comply with FBCR 2004 requirements for the stated wind

velocity and design pressures. \* Provide a continuous load path from roof to foundation. If you believe the plan omits a continuous load path connection, call the wind load engineer immediately. \* Verify the truss engineering includes truss design, placement plans, temporary and permanent bracing details,

truss-to-truss connections, and load reactions for all bearing locations. \* Select uplift connections, walls, columns, and footings based on truss engineering bearing locations and reactions; including interior bearing walls. \* Size headers for gravity loads; headers sized by the builder for gravity loads will also satisfy wind loads.

DOCUMENT CONTROL and PRIORITY: Structural requirements on S-1 control unless the building code or architectural sheets have more stringent requirements. Non-structural requirements on architectural sheets control. Specific requirements take precedence over general requirements. Revision control is by the latest signature date and

is the responsibility of the builder. COPYRIGHTS AND PROPERTY RIGHTS: Mark Disosway, P.E. hereby expressly reserves its common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express written permission and conservations. of Mark Disosway.

Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

WINDLOAD ENGINEER: Mark Disosway, PE No.53915

CERTIFICATION: The attached plans and "Windload Engineering", sheet S-1, comply with FBCR 2004, Section

LIMITATION: This design is valid for one building, at specified location. This drawing is not valid for construction unless raised seal is affixed.

R301.2.1 wind loads & lateral loads, to the best of my knowledge.

w/6-.131"x3.25"

W/ 2x2x.140\* STEEL

WALL SECTIONS

Uplift, lb. < 800

WASHER SPACING PER

OPTION #4

Uplift, lb. < 2500

1/2"x10" ANCHOR BOLTS

**OPTION #5** 

Uplift, lb. < 3885

OPTION #3

Uplift, lb. < 1760

W/ 2x2x.140" STEEL

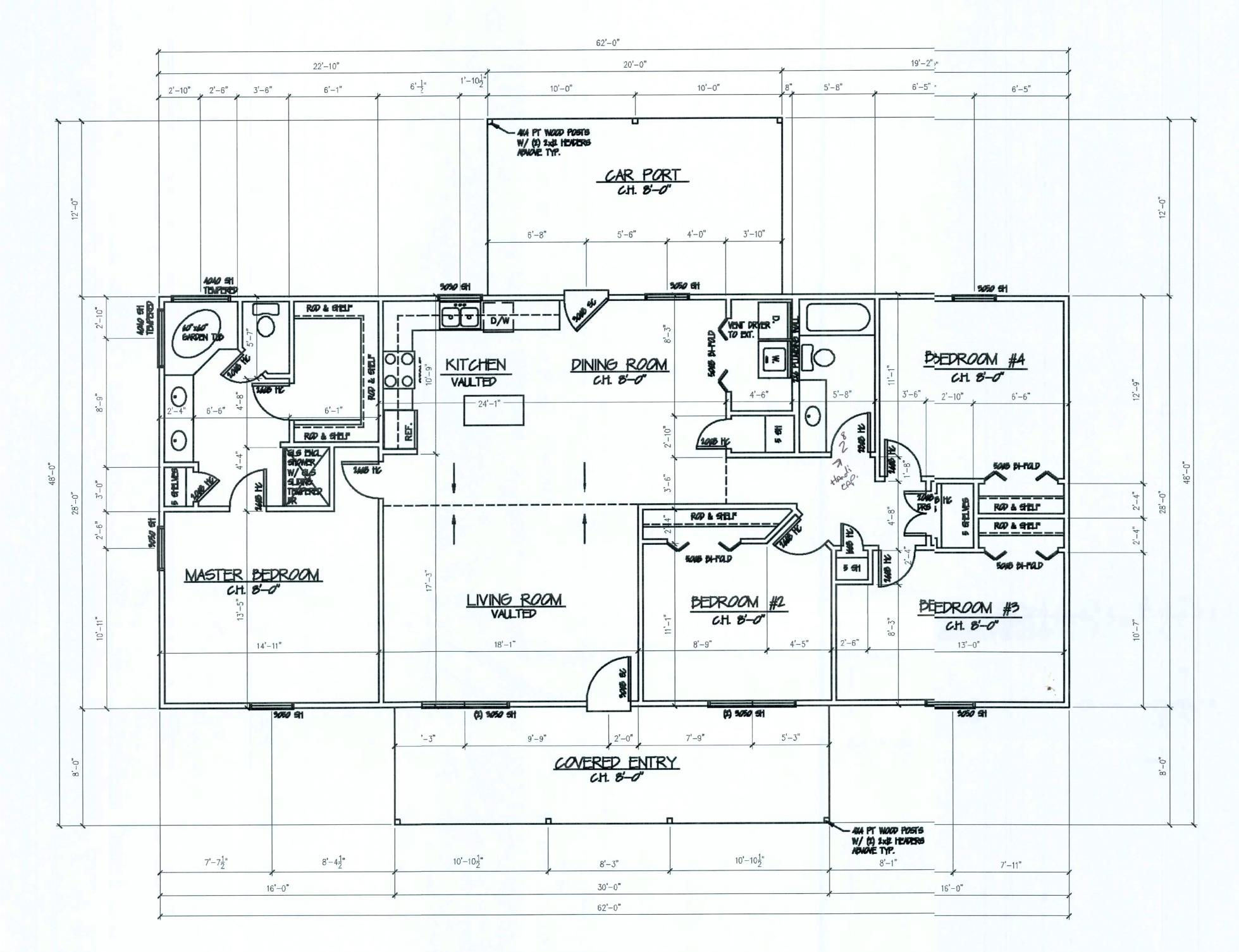
OPTION #2

Uplift, lb. < 1500

Builder: Plumb Level Construction Designer: Sheet S-1 of 1 Sheet

Windload Engineering Job # 812165 REV-01-JUN-06

ocation: Wendy Road Columbia County, Florida



FLOOR PLAN 1/4"= 1-0"

## GENERAL NOTES

I.) THIS PLAN IS TO PROVIDE GENERAL DESIGN DATA ONLY, AND TO BE A SOURCE OF INFORMATION FOR ESTIMATING, PLANNING AND THE PRODUCTION OF OTHER TECHNICAL INPUT BY THE STRUCTURAL ENGINEER NEEDED), THE CONTRACTOR AND SUBCONTRACTORS AND MATERIALS SUPPLIES. IN AND OF ITSELF IT IS ONLY A PORTION OF THE INFORMATION REQUIRED FOR PERMITTING AND SHALL BE ACCOMPANIED BY ENGINEERING NEEDED)/ TECHNICAL DATA PRODUCED BY OTHERS IN ACCORDANCE WITH THE FLORIDA BUILDING CODE 2004.

- 2.) COMPLIANCE WITH SECTION 1600 OF THE FLORIDA BUILDING CODE 2004 (WIND ENGINEERING I" NEEDED) SHALL BE THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER.
- 5) ROOF AND/OR TRUSS ENGINEERING SHALL BE PROVIDED BY THE TRUSS SUPPLIER. THE TRUSS SUPPLIER SHALL PREPARE ENGINEERED DRAWINGS INDICATING TRUSS PLACEMENT, GROER LOCATIONS, TRUSS TO TREYS CONNECTIONS, BEARING REQUIREMENTS AND ANY POINT LOADS.
- 4.) SITE ANALYSIS OR PREPARATION IS NOT A PART OF THIS PLAN AND IS THE RESPONSIBILITY OF THE OWNER/ CONTRACTOR.
- 5.) WINDOWS TO BE DOUBLE GLAZED. SIZES ARE NOMINAL AND MAY VARY WITH THE MANUFACTURER. IT SHALL BE THE CONTRACTORS RESPONSIBILITY TO PROVIDE MANUFACTURE'S DATA FOR DOORS AND WINDOWS THAT COMPLY WITH WIND PRESSURE REQUIREMENTS SET BY THE STRUCTURAL ENGINEER.
- (a) HVAC UNIT AND DUCT DESIGN, AND MANUAL J REPORT TO DE PROVIDED BY THE HVAC CONTRACTOR.
- 7.) CAPINET AND MILLWORK DETAIL IS NOT PART OF THIS PLAN. THE PLAN IS A GENERAL DESIGN AND THESE DETAILS SHALL BE THE RESPONSIBILITY OF THE OWNER AND/OR CONTRACTOR.
- 8.) THE BUILDER/ CONTRACTOR MUST VERIFY ALL DIMENSIONS, SIZES AND DETAILS PRIOR TO CONSTRUCTION. THE DESIGNER DOES NOT ASSUME LIMBILITY FOR ANY ERRORS AND OR OMISSIONS, ALL SOVERNING CODES AND RESULATIONS SHALL SUPERSEDE THESE DRAWINGS
- I. FOOTING DESIGN IS BASED ON MIN. SOIL BEARING CAPACITY OF 2000 PSF @ 95% DENSITY.
- 1. ALL CONCRETE SHALL HAVE A MIN I'C = 3000 PSI @ 18 DAYS
- 9. WELDED WIRE FABRIC TO ASTM A-185.
- 4. ALL REINFORCING STEEL SHALL CONFORM TO ASTM AGS, GRADE 40.
- 5. MINIMUN COVER FOR REINFORCING SHULL BE 5" FOR FOOTINGS, AND CENTER IN SLADS.
- 6. REINFORCING DAR SPLICES FOR \$5 DAR SHALL DE 30° MIN. AND HOOKS SHALL MEET ACI STANDARDS.
- 7. ALL REINFORCING SHALL BE HELD SECURLEY IN POSITION WITH STANDARD ACCESSORIES DURING PLACING OF CONCRETE
- 8. HOLLOW CONCRETE MAGONARY BLOCKS (CMU) SHULL HAVE LLIMATE COMPRESSIVE STRENTIGH (fim) NOT LESS THAN 1950 PSI (ASTM-C-90 STANDARD CONCRETE MAGONARY UNIT)
- 9. HORIZONTAL JOINT REINFORCING SHALL BE HIGH TENSILE STEEL (ASTM AGA-72). ALL WIRE TO BE 9 GA. SIDE ROD AND 9 GA. CROSS ROD. HOT DIPPED GALVANIZED AFTER FARRICATION.
- 10. ALL WOOD SHALL BE FISHED PSI MIN. ALL WOOD IN CONTACT WITH CONCRETE SHALL BE PRESSURE TREATED (P.T.)

## SQUARE FOOTAGE LEGEND

LIVING AREA	1730
COVERED ENTRY	24
CARPORT	24
TOTAL UNDER ROOF	2210





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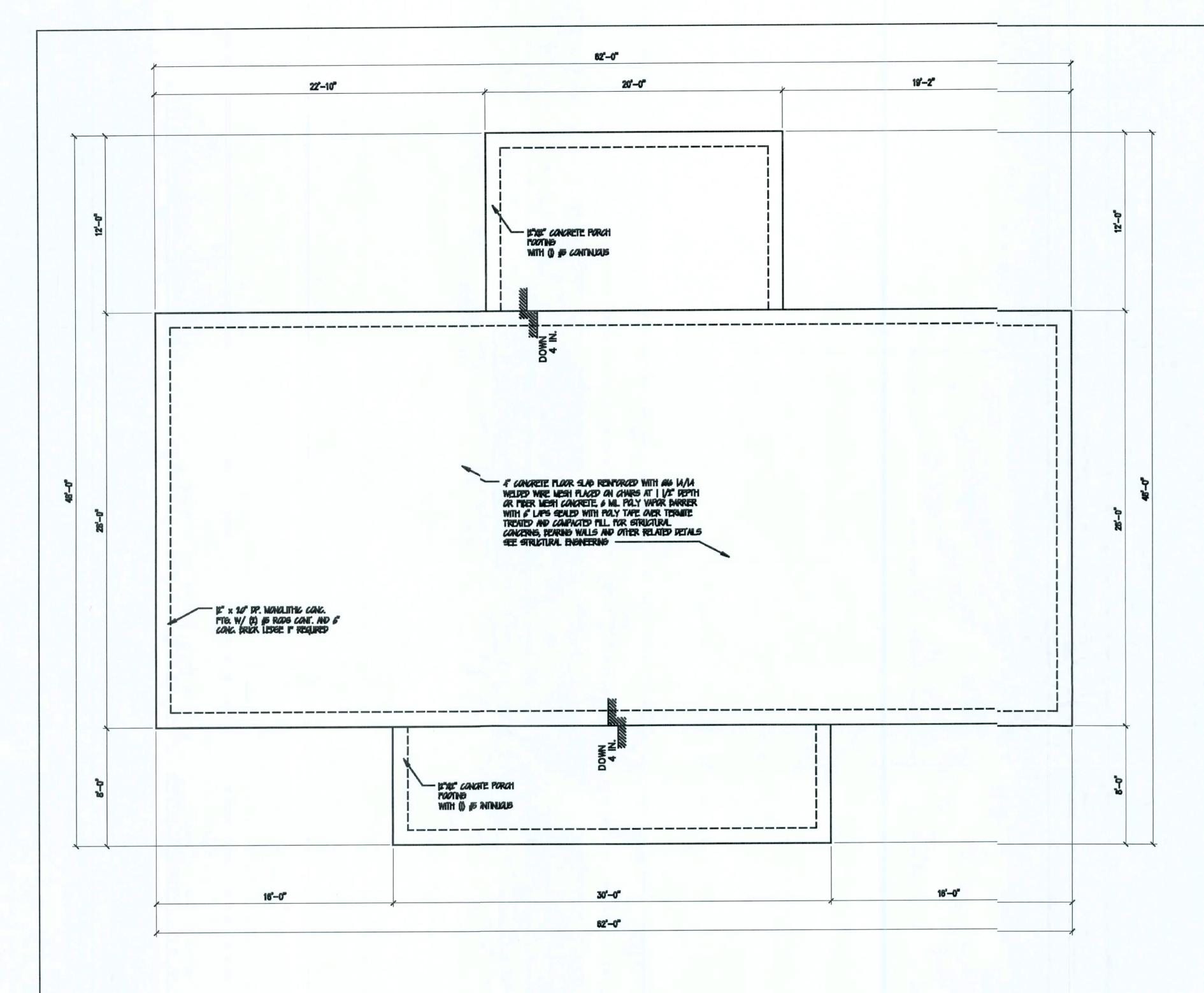
P.O. BOX 123

10/14/08

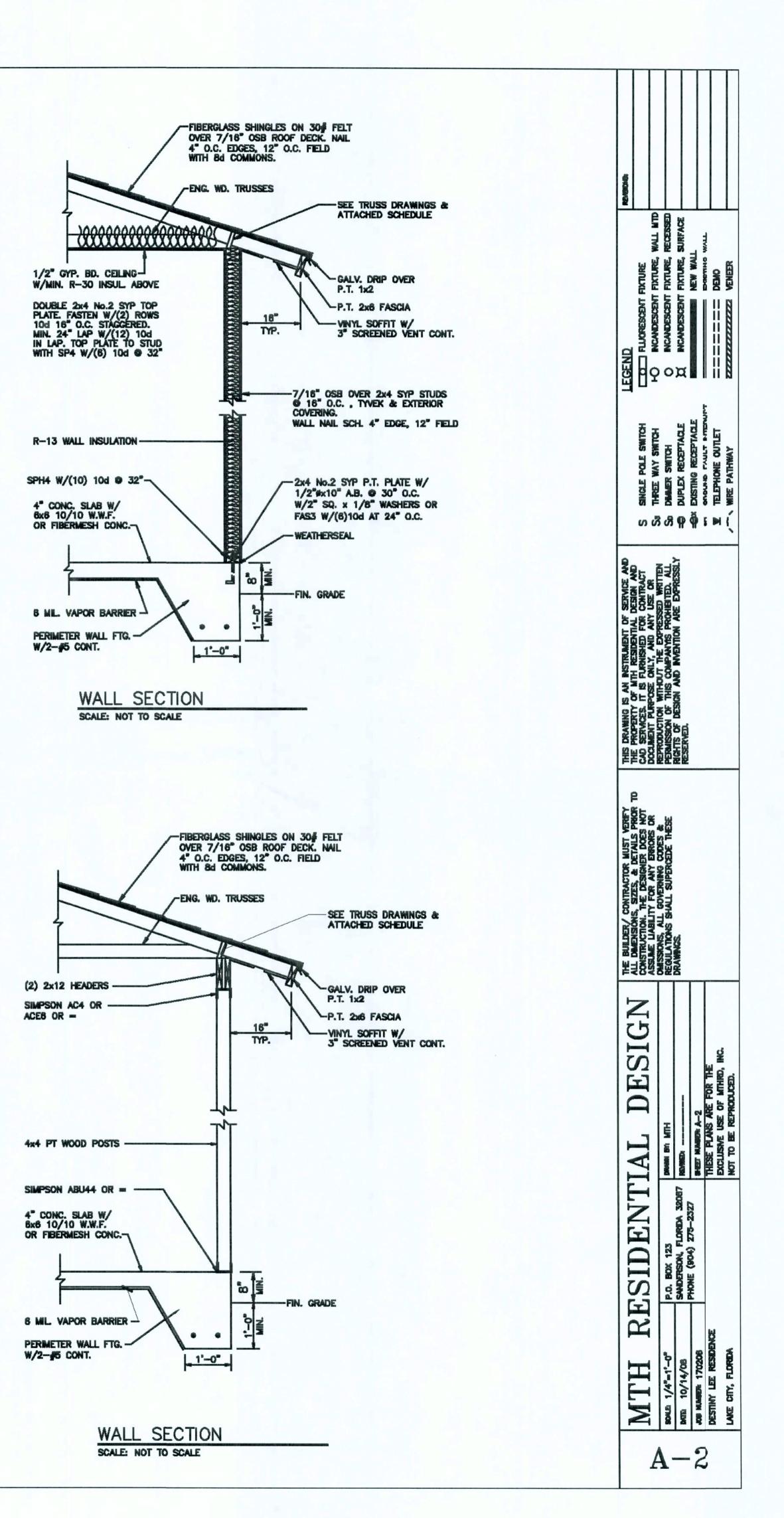
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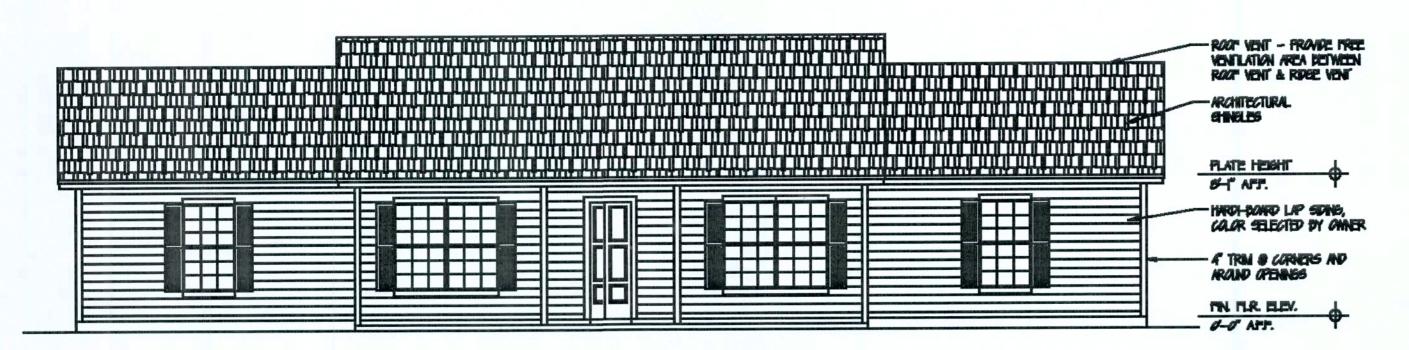
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A-1



FOUNDATION PLAN



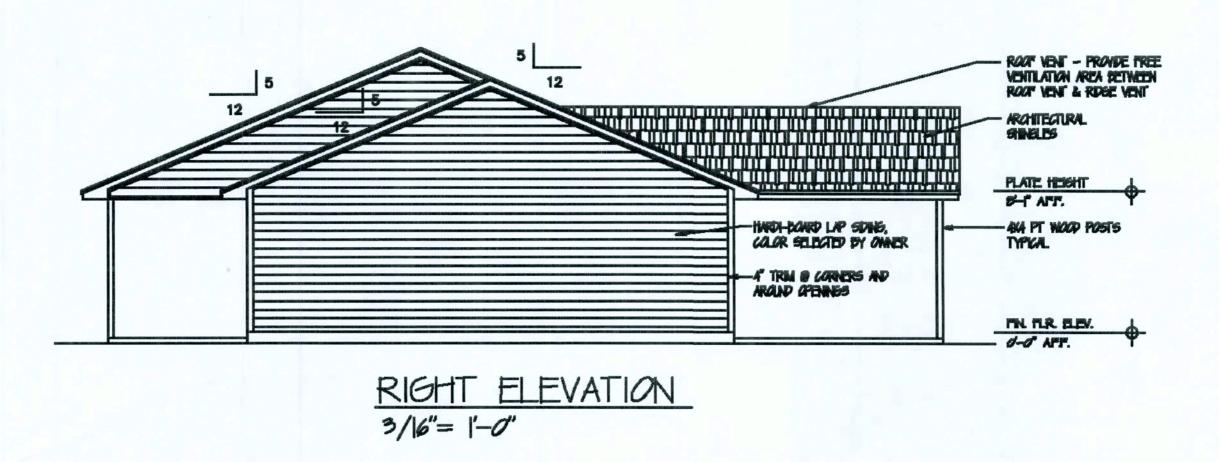


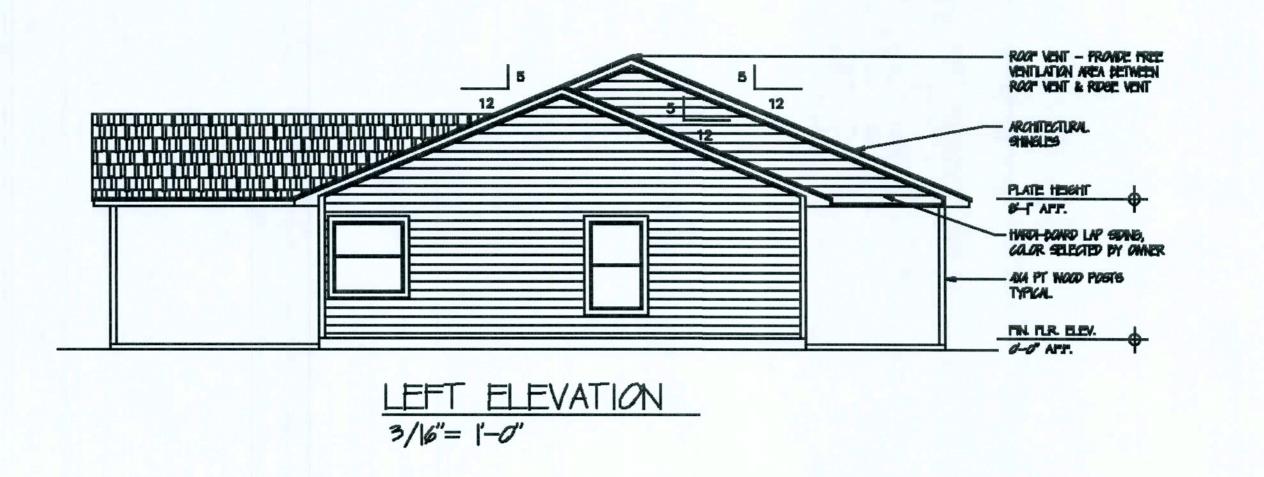
FRONT ELEVATION
3/16"= 1'-6



REAR ELEVATION

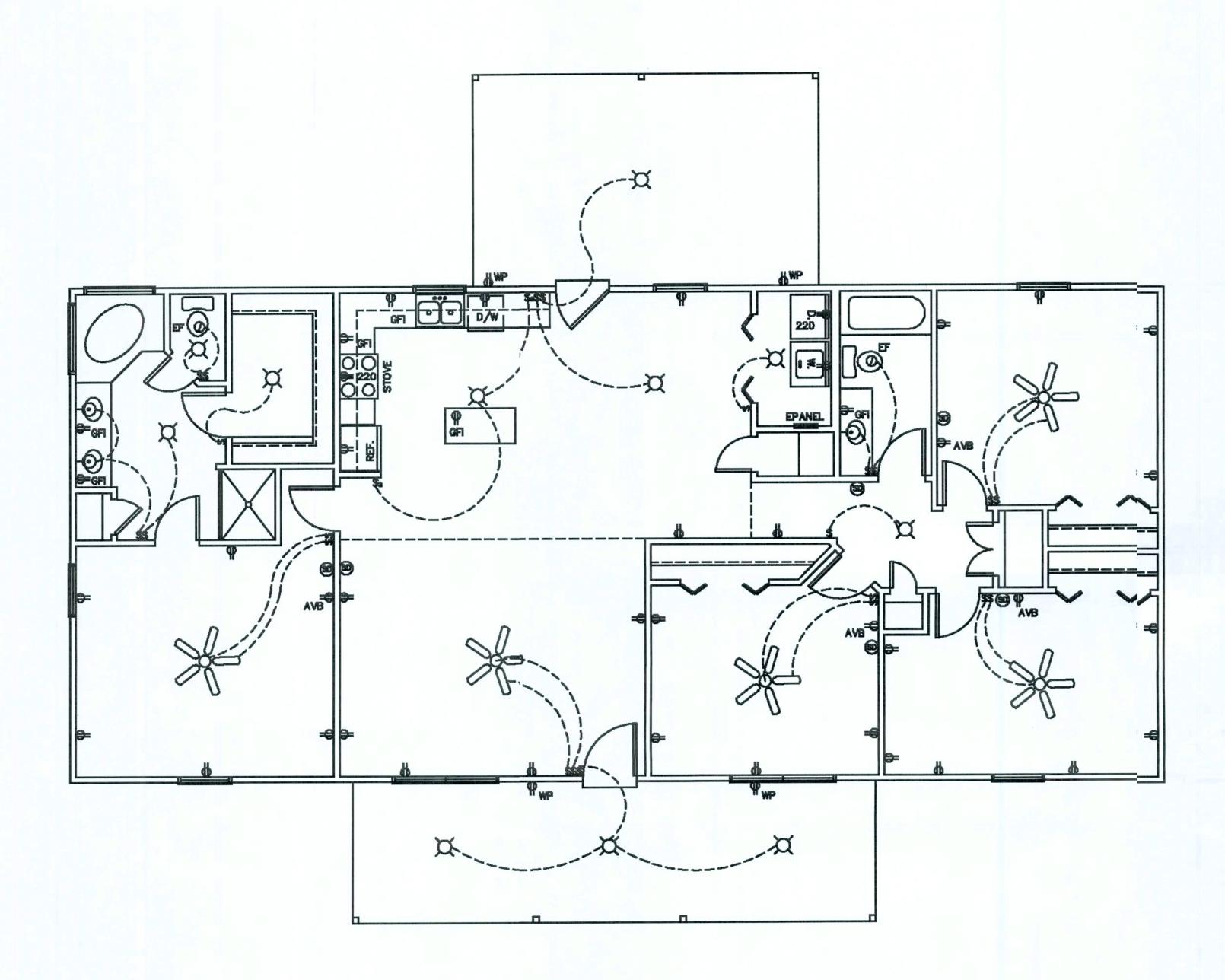
3/16"= 1'-0"





NEVENOVE:					
LEGEND  CO RUORESCENT FIXTURE  HO INCANDESCENT FIXTURE, WALL MTD	O NCANDESCENT FIXTURE, RECESSED	MCANDESCENT FIXTURE, SURFACE		EXISTING WALL	B
S SINGLE POLE SWITCH	Se DAMER SMITCH	_	A EXISTING RECEPTACLE	W TELEPHONE OUTLET	/ WRE PATHWAY
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DESIGN			. A #	THESE PLANS ARE FOR THE	NOT TO BE REPRODUCED.
RESIDENTIAL	P.O. BOX 123 DRAWN BY MTH	SANDERSON, FLORDA 32087 ROMEDI			NOT TO BE
MTH	304E: 1/4"=1"-0"	DATE 10/14/08	-se surane 170200	DESTINY LEE RESIDENCE	LAKE CITY, FLORIDA

A-3



ELECTRICAL PLAN

ELECTRICAL NOTES:

 WIRE ALL APPLIANCES. HVAC UNITS AND OTHER EQUIPMENT PER MANUFACTURE SPECIFICATIONS.

Consult the owner for the number of separate telephone lines to be installed and locations.

3.) ALL INSTALLATIONS SHALL BE PER NATIONAL ELECTRIC CODE.

4.) ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.

5.) TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE INTERLOCKED TOGETHER. INSIDE AND NEAR ALL BEDROOMS.

ELECTRIC CONTRACTORS SHALL BE RESPONSIBLE FOR THE DESIGN AND SIZING OF ELECTRICAL SERVICE AND CIRCUITS.

7.) ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY

ELECTF	RICAL LEGEND
¤	CEILING MOUNTED LIGHT
ю	WALL MOUNTED LIGHT
	CEILING LIGHT W/ BRACE FOR FAN
\$	SINGLE LIGHT SWITCH
\$2	2-WAY LIGHT SWITCH
<b>\$</b> 3	3-WAY LIGHT SWITCH
<b>⊕</b> =	110V OUTLET
<del>=</del>	220V OUTLET
<b>=</b>	WATERPROOF OUTLET
<b>⊜</b> =	DIRECT GROUNDED OUTLET
⊕= ANB	ARC VAULT BREAKER
•	SMOKE DETECTOR
EPANEL	ELECTRIC SERVICE PANEL

 		,					_	
PEASONS								
LEGEND	CO PLUORESCENT FIXTURE	HO INCANDESCENT FIXTURE, WALL MTD		X INCANDESCENT FIXTURE, SURFACE	NEW WALL	EXSTING WALL	OFFICE	THE PROPERTY OF WENTER
	S SINGLE POLE SWITCH	S. THREE WAY SWITCH	S. DEMER SWITCH		SX EXISTING RECEPTACLE	OFFICIAL PAULT INTERUPT	THE FORMER DITTET	ANTHUE DATHWAY
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TATMINERATION	KENIDEN LIAL		P.O. BOX 123 Devell Brit MTH	SANDERSON, FLORDA 32067 REMEDI:	PHONE (904) 275-2327 SPECT MARKET A-4			SE CL. LON
	HIM		804E: 1/4"=1"-0"	DATE 10/14/08	JUN MINNESS 170208	TOTAL AND	DESIRI LEE RESIDENCE	LAKE CITY. FLORIDA

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