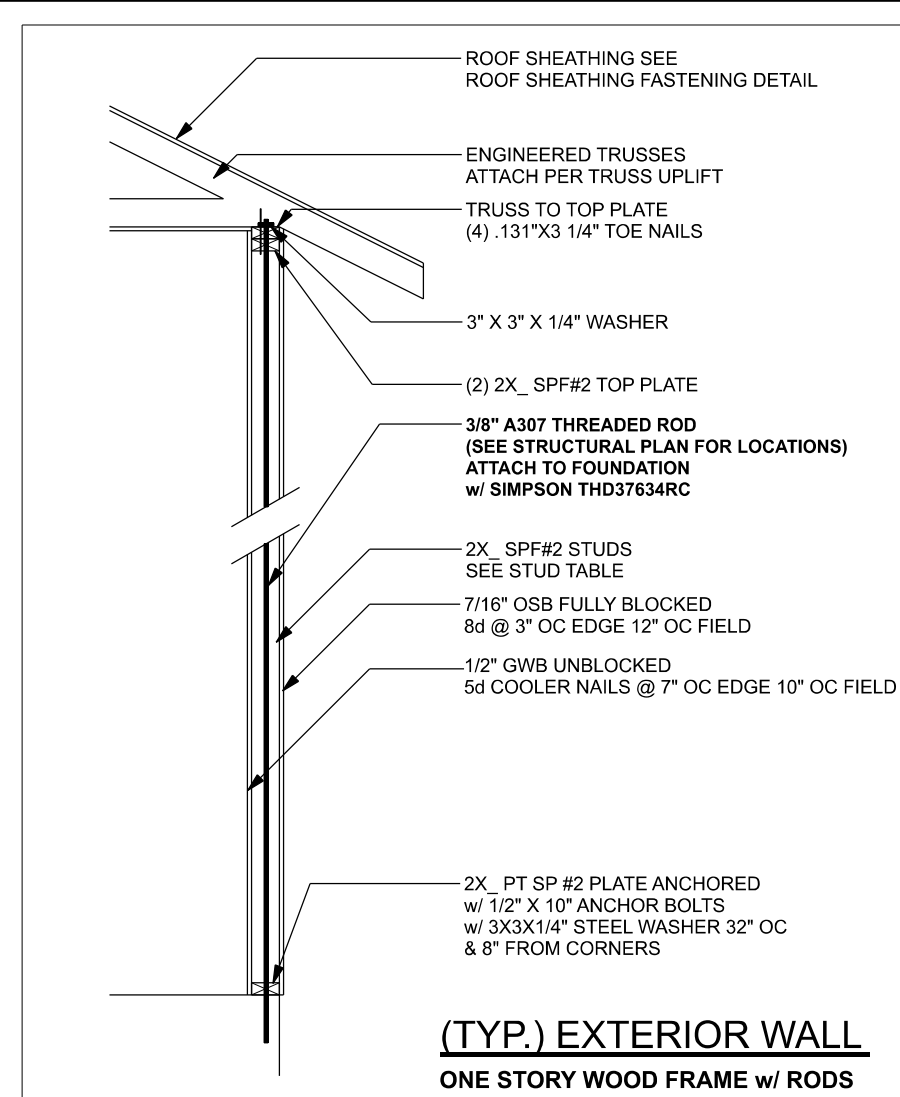
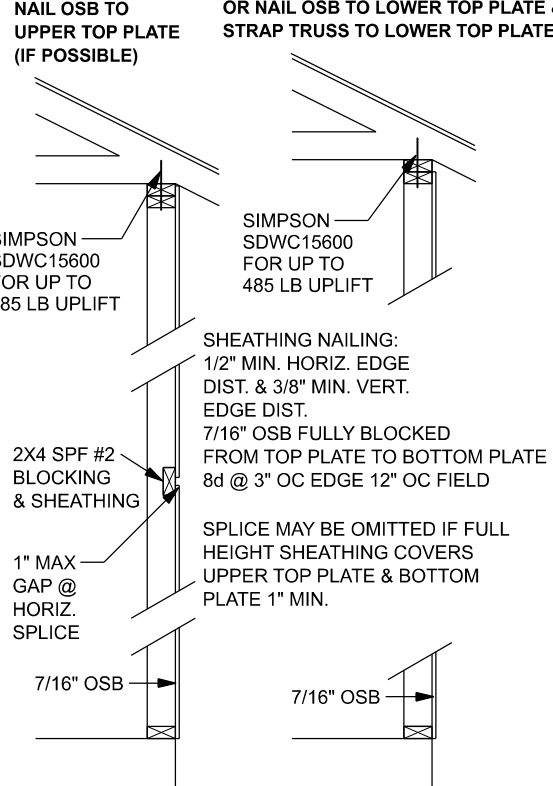




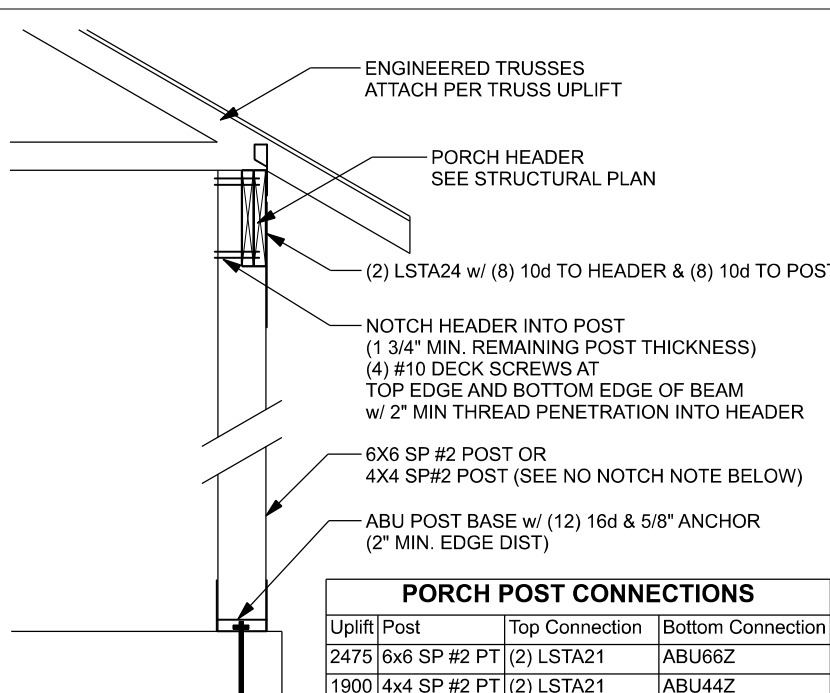
ROOF SHEATHING FASTENING TABLE (RAFTER / TRUSS SG = 0.49)					
Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field	
120 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.113")	6" oc	12" oc	
120 mph Exp. C	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.113")	6" oc	6" oc	
120 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc	
130 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.113")	6" oc	6" oc	
130 mph Exp. C	15/32"	ASTM F1667 RRS-01 (2 3/8" x 0.113")	6" oc	6" oc	
130 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc	
140 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.113")	6" oc	6" oc	
140 mph Exp. C	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc	
140 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc	

Note:
For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field.
Note:
This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.



SHEATHING FOR UPLIFT ATTACHMENT DETAILS ONE STORY WOOD FRAME

(TYP.) INTERIOR BEARING WALL ONE STORY WOOD FRAME w/ RODS



POSTS CUT FLUSH WITHOUT NOTCH - BRACE TO PREVENT ROTATION:	
EAVE / HIP ROOF:	
- STRAP TRUSS W/ H2.5A OR EQUAL TO EACH SIDE OF HEADER (FRONT & BACK) USE LONGER STRAP/STRAPS NEEDED IF TOP PLATES ARE INSTALLED	
- (2) L2S24 16-10d OR (2) MTS20 14-10d ON FRONT AND REAR OF POST TO HEADER	
- ATTACH HEADER TO POST W/ (8) .131" x 3.25" TOE-NAIL	
GABLE END:	
- EXTEND GABLE SHEATHING TO BOTTOM OF HEADER NAILED WITH 8d NAILS @ 6" M	
INTO TRUSS BOTTOM CHORD AND INTO BEAM AT 1 1/2" FROM TOP & BOTTOM EDGE	
- ATTACH HEADER TO POST W/ (8) .131" x 3.25" TOE-NAIL	

POSTS CUT FLUSH WITHOUT NOTCH - BRACE TO PREVENT ROTATION:
EAVE / HIP ROOF:
- STRAP TRUSS w/ H2 5A OR EQUAL TO EACH SIDE OF HEADER (FRONT & BACK)
- USE LONGER STRAP AS NEEDED IF TOP PLATES ARE INSTALLED.
- (2) LSTA24 16-10d OR (2) MTS20 14-10d ON FRONT AND REAR OF POST TO HEADER
- ATTACH HEADER TO POST w/ (8) 131" x 3.25" TOE-NAIL
- GABLE END:
- EXTEND GABLE SHEATHING TO BOTTOM OF HEADER NAILED WITH 8d NAILS @ 6" MIN. INTO TRUSS BOTTOM CHORD AND INTO BEAM AT 1-1/2" FROM TOP & BOTTOM EDGE
- ATTACH HEADER TO POST w/ (8) 131" x 3.25" TOE-NAIL

CONNECTOR TABLE					
Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter	
615	485	SDWC15600	-	-	
415	290	H3	4-8d x 1 1/2"	4-8d x 1 1/2"	
575	496	H2 5A	5-8d x 1 1/2"	5-8d x 1 1/2"	
1340	1015	H10A	9-10d x 1 1/2"	9-10d x 1 1/2"	
720	620	LTS12-20	6-10d1 1/2"	6-10d1 1/2"	
1000	860	MTS12-30	7-10d1 1/2"	7-10d1 1/2"	
1450	1245	HTS20-30	12-10d1 1/2"	12-10d1 1/2"	
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member	
1235	1235	LSTA21	6-10d	6-10d	
1640	1455	MSTA24	9-10d	9-10d	
1030	1030	CS20	7-10d	7-10d	
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate	
585	535	SP1	6-10d	4-10d	
1065	605	SP2	6-10d	6-10d	
771	771	LSTA24	10-10d	wrap under or over plate	
1235	1235	LSTA24	14-10d	wrap under or over plate	
Uplift SP	Uplift SPF	Holdowns @ Stenwall	To Stud / Post	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD	
4235	3640	HTT4	16-16d x 1/2"	1/2"x12" Titen HD	
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x6" Titen HD	
4235	3640	HTT4	16-16d x 1/2"	1/2"x12" Titen HD	
Uplift SP	Uplift SPF	Post Bases @ Stenwall	To Post	To Post	Anchor
2200		ABU44	12-16d	5/8"x12" Drill & Epoxy	
2300		ABU66	12-16d	5/8"x12" Drill & Epoxy	
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	To Post	Anchor
2200		ABU44	12-16d	5/8"x7" Drill & Epoxy	
2300		ABU66	12-16d	5/8"x7" Drill & Epoxy	

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20BS, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

	(1) 2x4 @ 16" OC	(1) 2x4 @ 12" OC	(1) 2x6 @ 16" OC	(1) 2x6 @ 12" OC
TO 10'-1" STUD HEIGHT				
TO 11'-2" STUD HEIGHT				
TO 15'-7" STUD HEIGHT				
TO 17'-3" STUD HEIGHT				

GRADE & SPECIES TABLE			
	SP #2	Fb	E
2x8		925	1.4
2x10		800	1.4
2x12		750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBOR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER'S DESIGN REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN
FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1600 PSF BEARING CAPACITY UNLESS USUAL OBSERVATION OR SOILS TEST PROVIDES OTHERWISE)
CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 2500 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W14 x W14, FB = 8KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A186, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C-1116. SUPPLIER TO PROVIDE ASTM C-1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH/WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR APPROVAL). THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, FY = 40 KSI. ALL LAP SPLICES 40" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-86, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL. DIAPHRAGMS, ROOF SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

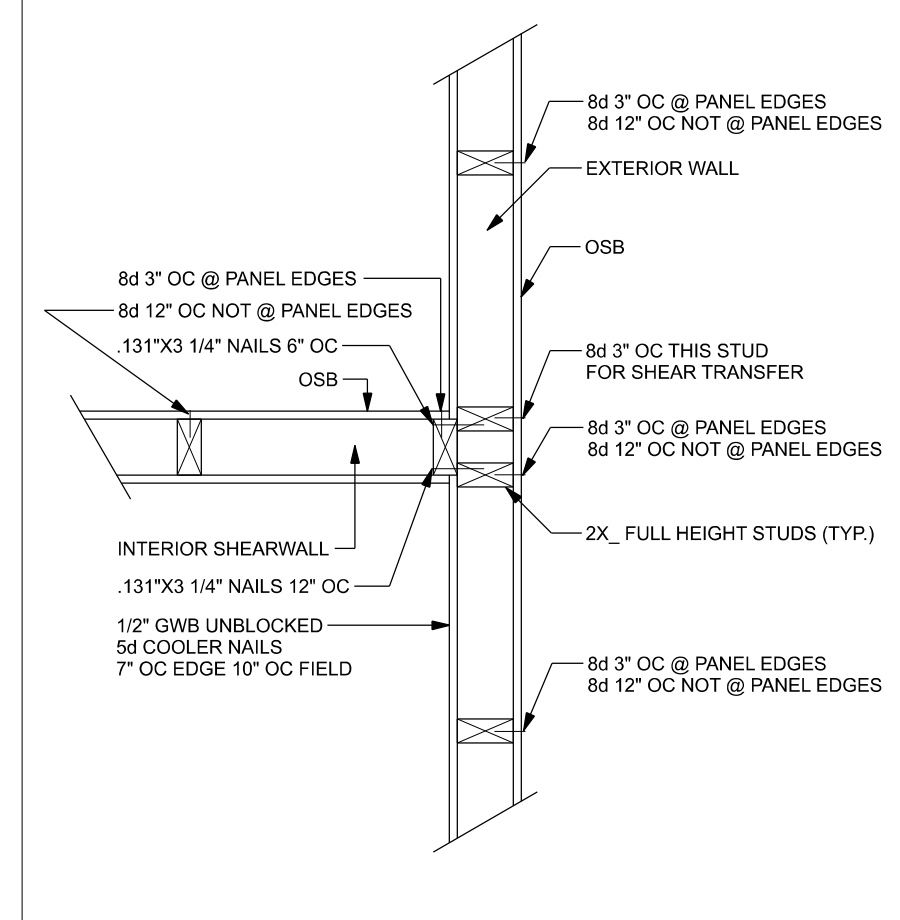
STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 10" IN GROUTED CMU.

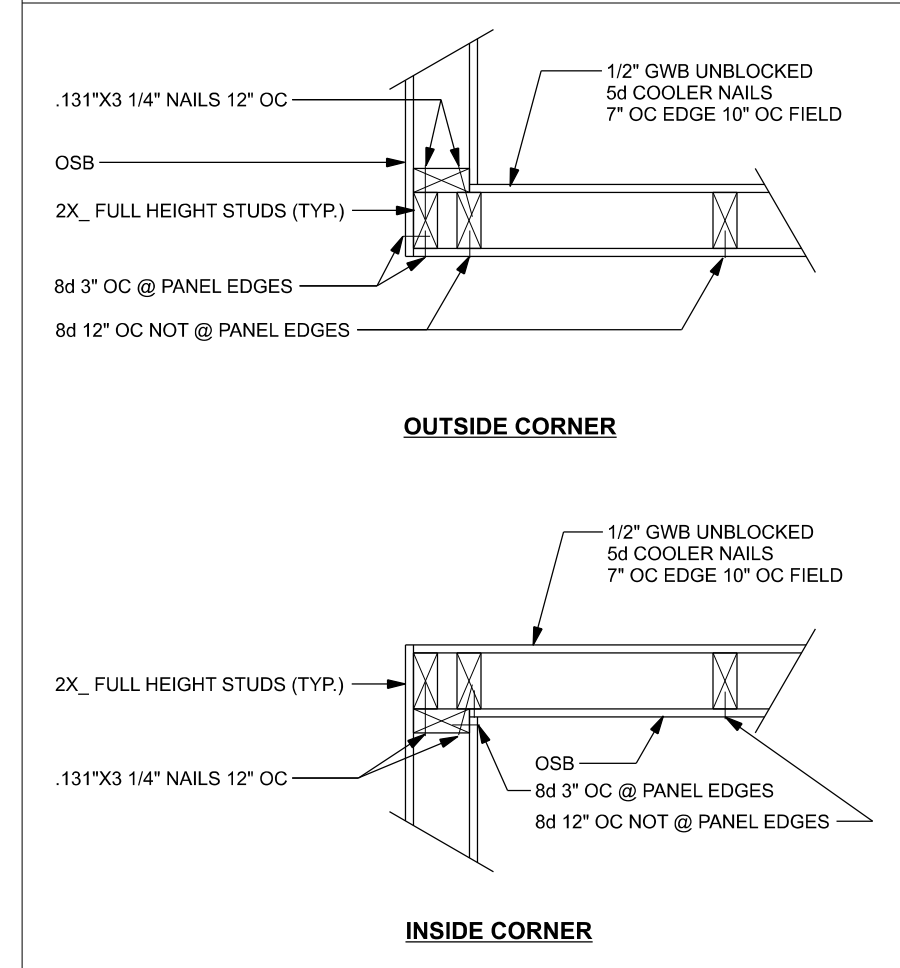
BUILDER'S RESPONSIBILITY:			
THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.			
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.			
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES WHICH COMPLY WITH FBOR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.			
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.			
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.			

ROOF SYSTEM DESIGN:

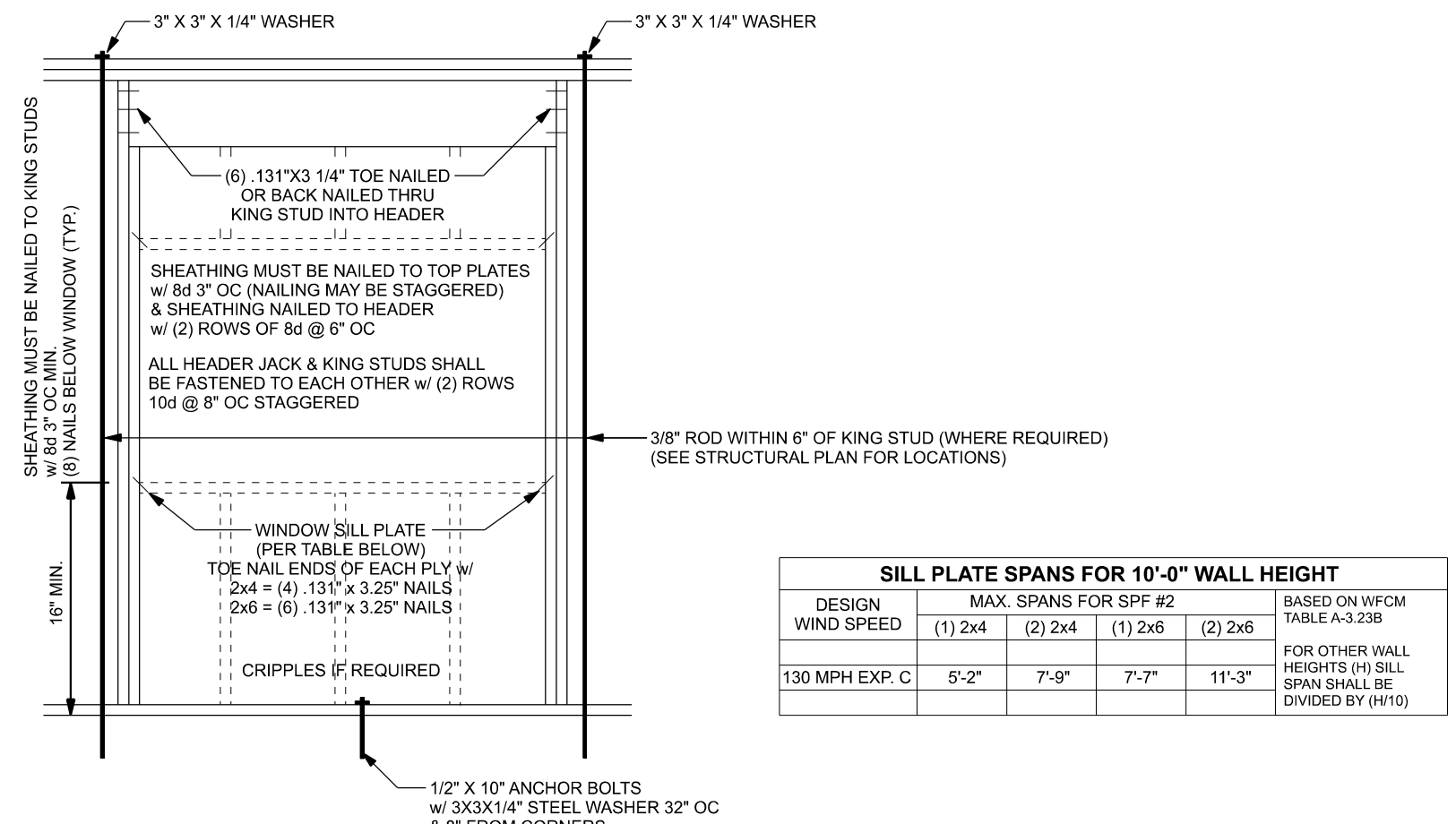
THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBOR. IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBOR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER. ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.



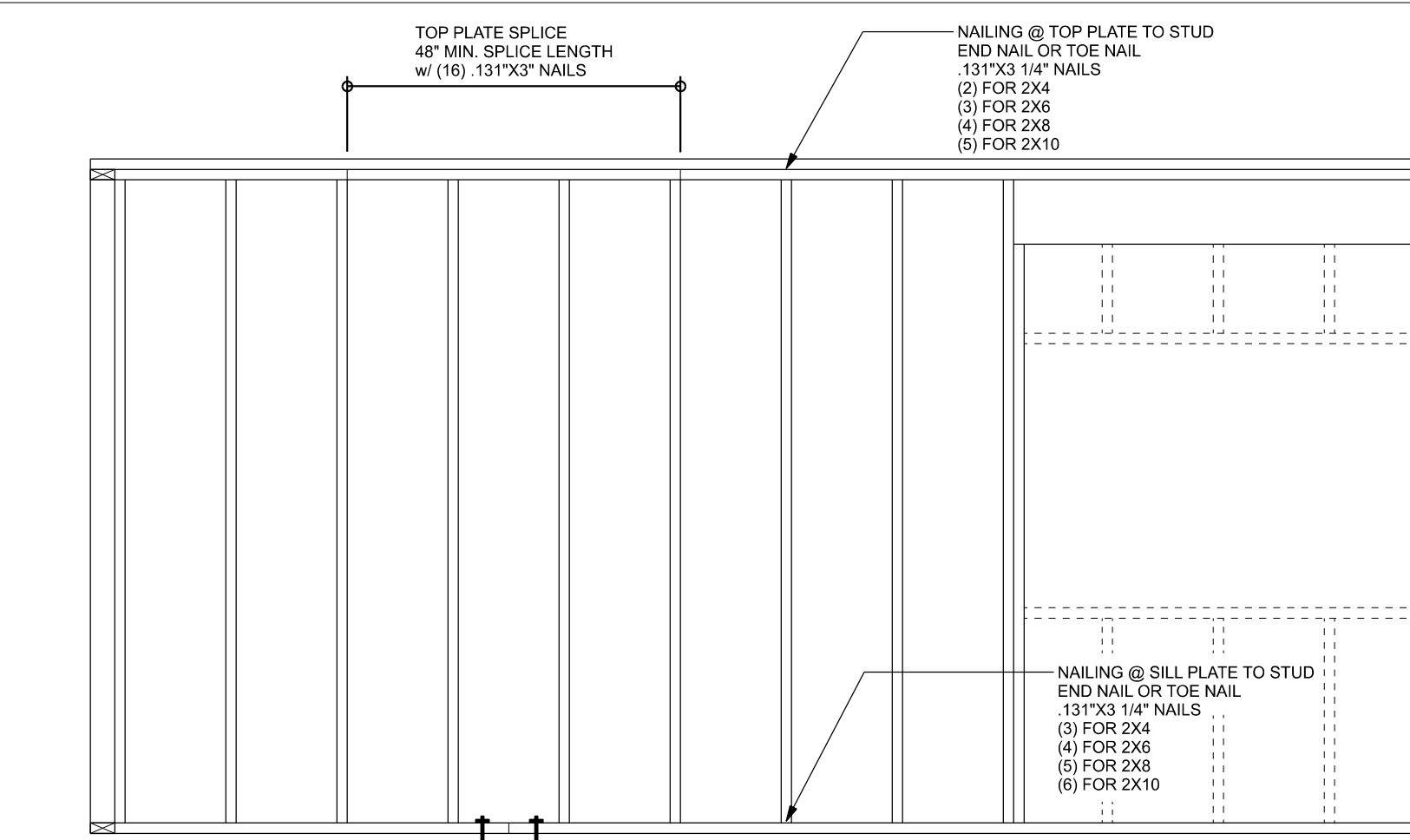
(TYP.) INTERSECTING WALL FRAMING WOOD FRAME



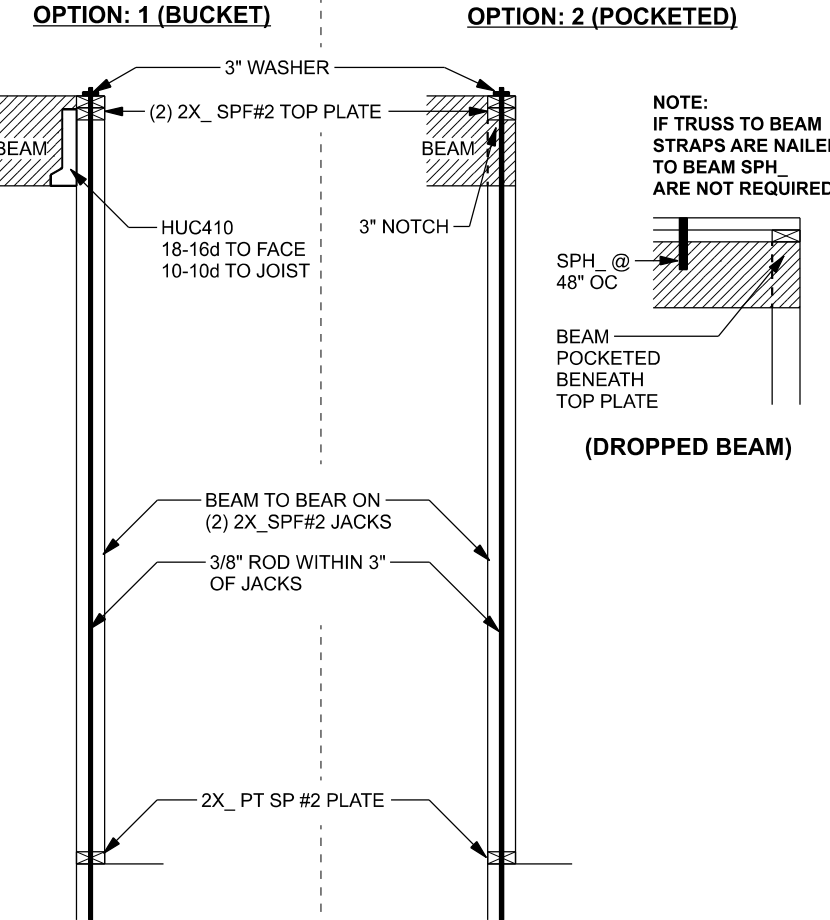
(TYP.) CORNER FRAMING WOOD FRAME



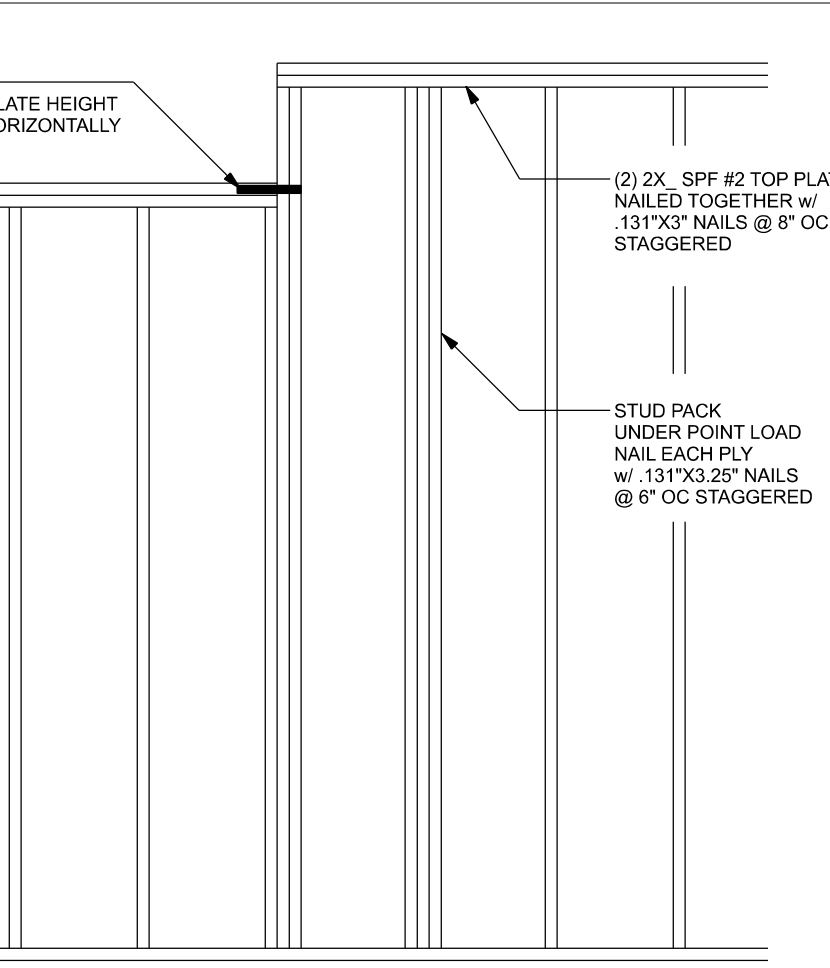
TYPICAL HEADER STRAPPING DETAIL ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) WALL CONNECTIONS ONE STORY WOOD FRAME



(TYP.) BEAM TO WALL WOOD FRAME w/ RODS



(TYP.) BEAM TO WALL WOOD FRAME w/ RODS

ALLOWABLE UPLIFT: 1770 LB

Blake Construction
Emery - Accessory Dwelling
PROJECT ADDRESS:
The Oaks
Columbia County, FL

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY P.E. 53915

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Monday, June 7, 2021

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JOB NUMBER:
210735

S-1
OF 4 SHEETS

