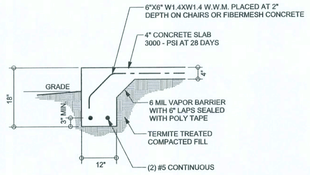
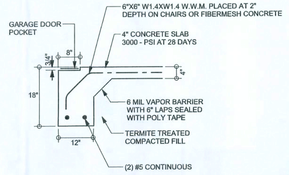
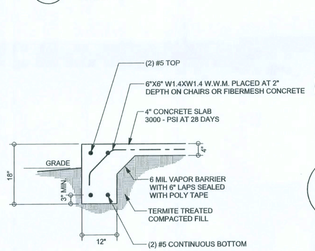
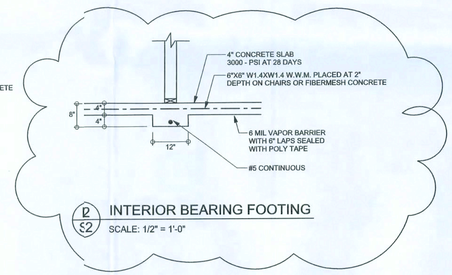


F1 S-2 MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"



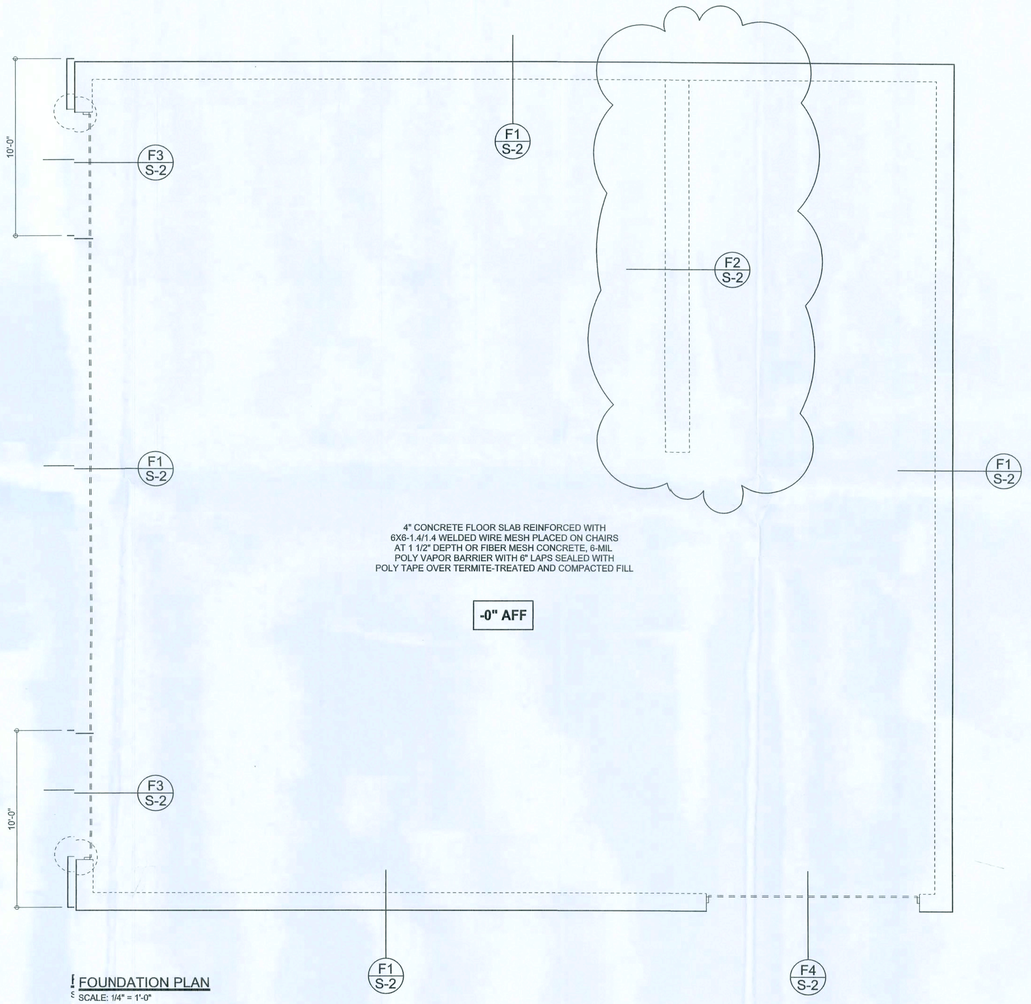
4 -2 GARAGE DOOR FOOTING
SCALE: 1/2" = 1'-0"





P S2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"

F3 S-2 MONOLITHIC FOOTING @ HANGAR DOOR WALL
SCALE: 1/2" = 1'-0"



FOUNDATION PLAN
SCALE: 1/4" = 1'-0"
DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

REVISONS	
30May08	

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POI 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS:
Stated dimensionsupercede scaled dimensions. Referall questions to Mark Disosway, P.E. for resolution. Do not proceed wihout clarification.

CERTIFICATION: hereby certify that I have examined this plar and that the applicable portions of the plas, relating to wind engineering comply with sectio R301.2.1, florida building code residential 204, to the best of my knowledge.

LIMITATION: Thisdesign is valid for one building, at specifid location.

MARK DISOSWAY
E_53915
30MAY08
SEAL

Fred & Ann
Elfers Hangar
Perm# 26978
Repare
ADDRESS:
Lot 31 Cannon Creek Airpark S/D
Columbia County, Florida

Mark Disosway P.E.
P.C. Box 868
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Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRNTED DATE:
May 30, 2008

DRAWN BY:	STRUCTURAL BY:
David Disoswa	David Disosway

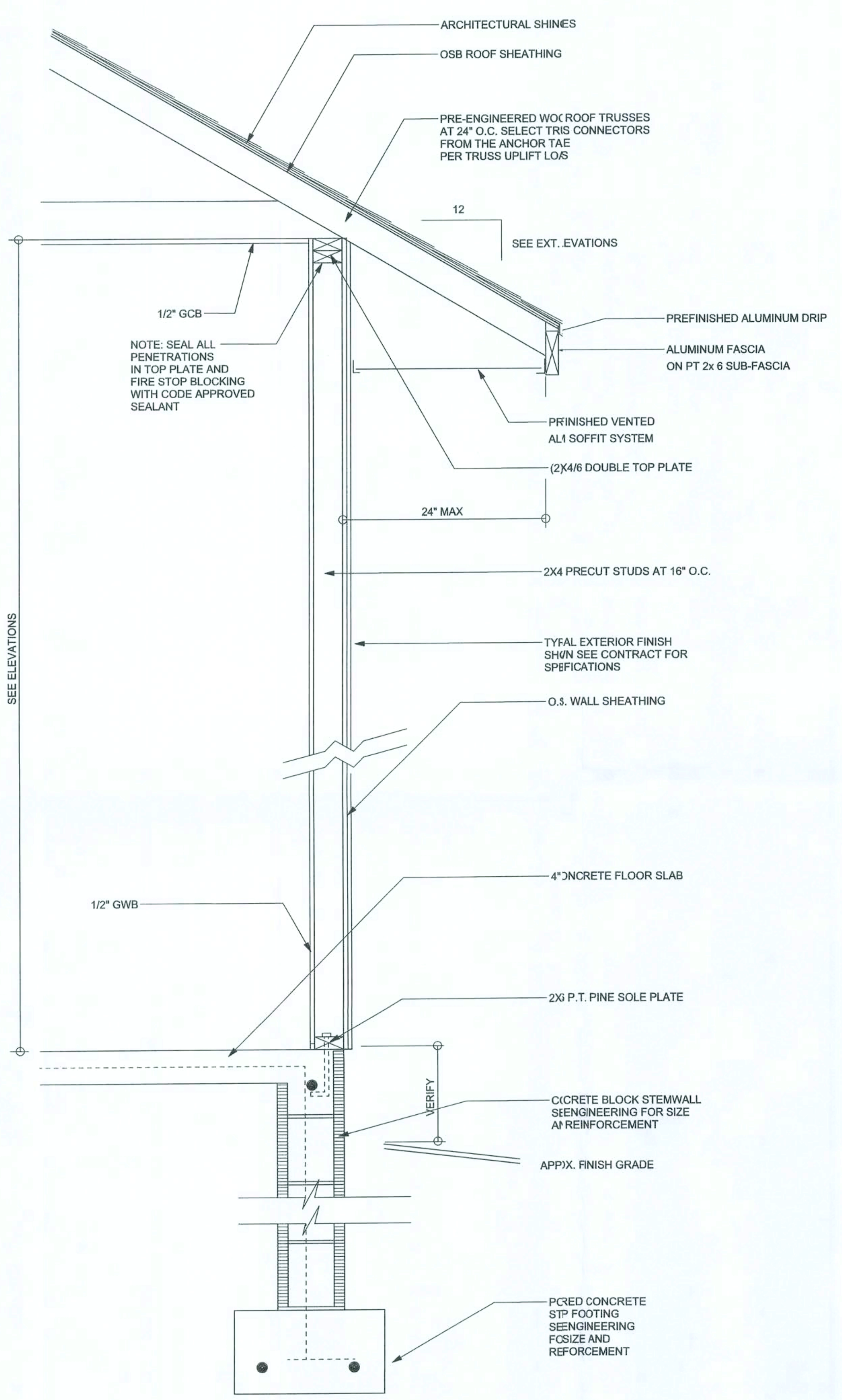
FINALS DATE
31 / Mar / 08

JOB NUMBER:
803121

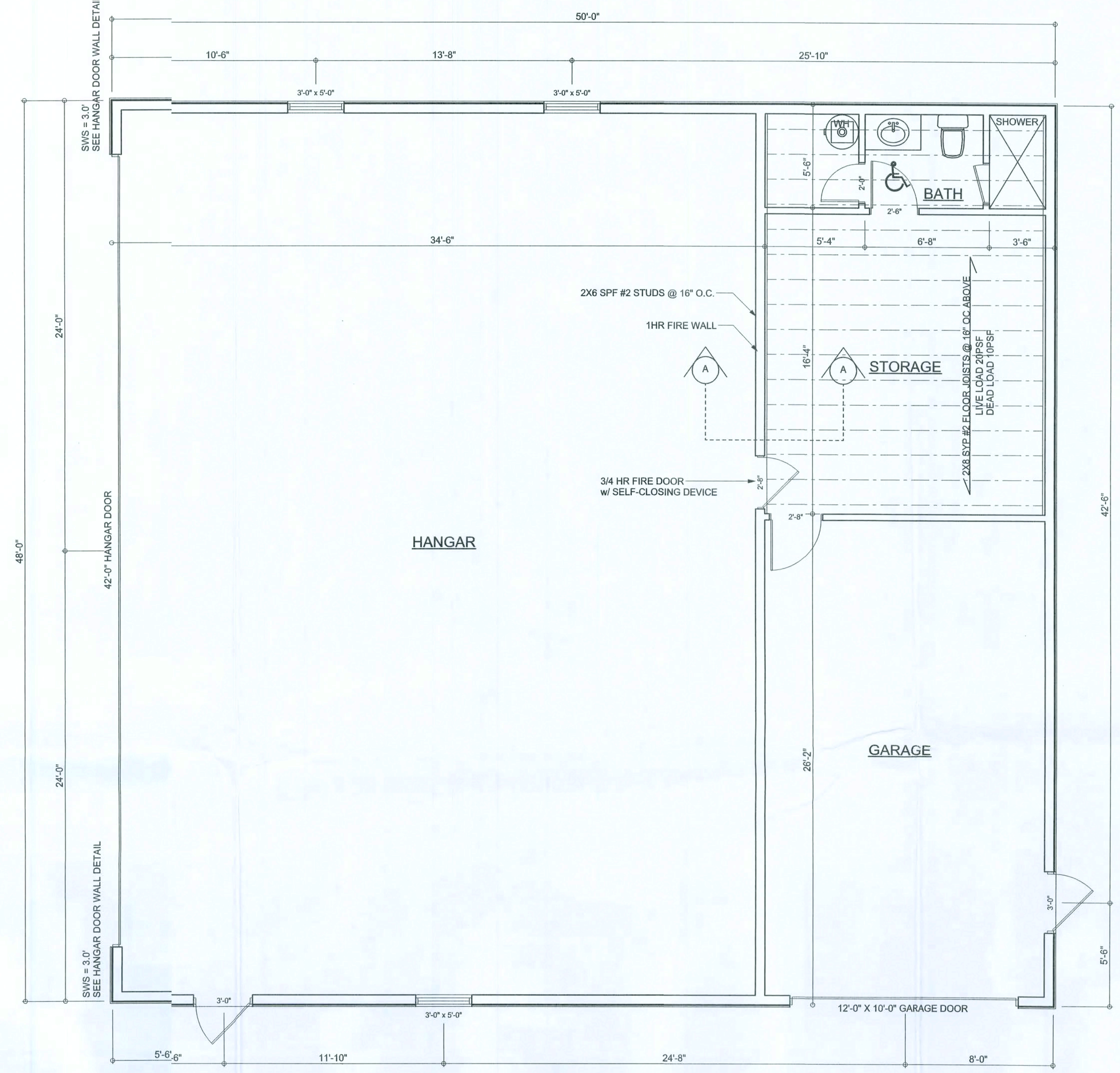
DRAVING NUMBER
S-2
OF6 SHEETS

REVISIONS	

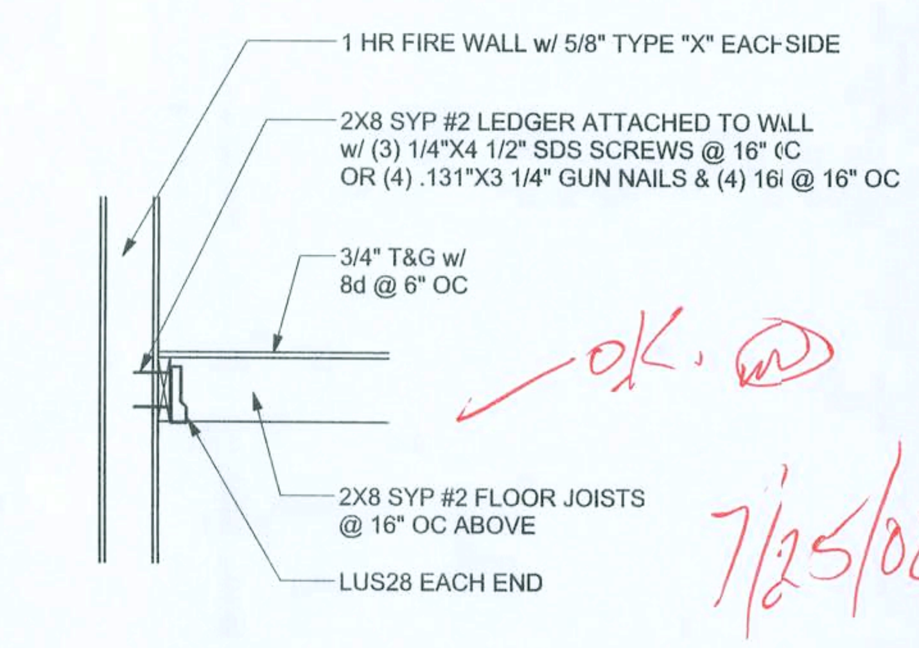
SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE



TYPICAL DESIGN WALL SECTION
NON - STRUCTURAL DATA
SCALE: 1" = 1'- 0"



FLOOR PLAN
SCALE: 1/4" = 1'-0"



SECTION A-A
SCALE: 1/2" = 1'-0"

OK. [initials] 7/25/08

AREA SUMMARY

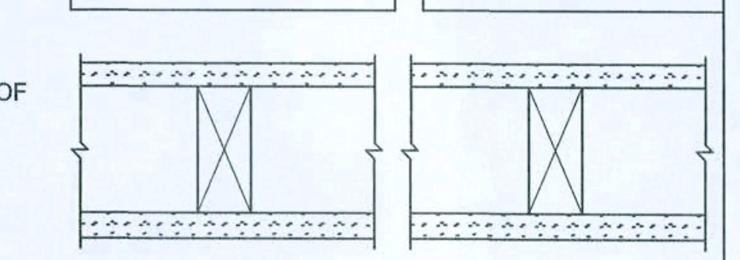
STORAGE / BATH AREA	351	S . F .
GARAGE AREA	409	S . F .
HANGAR AREA	1640	S . F .
TOTAL AREA	2400	S . F .

GA FILE NO. WP 3605	GENERIC	1 HOUR FIRE	30 TO 34 STC SOUND

GYPSUM WALLBOARD, WOOD STUDS

ONE LAYER 5/8" TYPE X PLAIN OR PREDECORATED GYPSUM WALLBOARD, WATER-RESISTANT GYPSUM BACKING BOARD, OR GYPSUM VENEER BASE APPLOED PARALLEL OR AT RIGHT ANGLES TO EACH SIDE OF 2x4 WOOD STUDS 16" oc w/ 6d COATED NAILS, 1 7/8" LONG, 0.0915" SHANK, 1/4" HEADS, 7" oc JOINTS OF SQUARE EDGE, BEVEL EDGE OR PREDCORATED WALLBOARD MAY BE LEFT EXPOSED

JOINTS STAGGERED 16" ON OPPOSITE SIDES. (LOAD-BEARING)



THICKNESS: 4 7/8"
APPROX. WEIGHT: 7 psf
FIRE TEST: UL R1319-4, -6, 6-17-52; UL R2717-39, 1-20-66; UL R3501-52, 3-15-66, UL DESIGN U305; ULC DESIGN W 301
SOUND TEST: OR 64-8, 2-4-64

26978
O.K. [initials]

Edgley Const.

WINDLOAD ENGINEER Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

CERTIFICATION: I herely certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R30.2.1, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY
P.E. 5915
[signature] 24 JUL 08
SEAL

Fred & Ann Elfers Hangar

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PRINTED DATE: July 24, 2008	
DRAWN BY: David Disosway	STRUCTURAL BY: David Disosway
FINALS DATE: 31 / Mar / 08	

JOB NUMBER: 803121

DRAWING NUMBER
2
OF 6 SHEETS

← 150' →

PROP. LINE

PROPERTY LINE SET BACK 55'

55'

50'0"

48'0"

CONCRETE PAD

HANGAR BUILDING + STORAGE AREA

40.2"

48'0"

48'0"

HANGAR DOOR

48'0"

116.0'

CONCRETE PAD

48'0"

48'0"

50'0"

(LP GAS UNDER GROUND)?

19.19

PROPERTY LINE SET BACK 10'0"

3'

13'0"

23'0"

10'0"

82.8"

12'

Well

24'

11'

CONCRETE DRIVE WAY

GRASS

114'

Sep Sys

16'0"

HOUSE FOOTPRINT ±

CHECK ALL DEMINSIONS

8" = 1'0"

59'0"

36'10"

PROPERTY LINE

59'0"

12'

CONCRETE DRIVE TO ROAD

GRASS

LOT LINE

10' MIN

30'6"

82'8"

119'6"

Cessna Court

10'

12'

12'10"

LOT 31 CANON CREEK AIRPARK

FRED + ANN ELFERS 561-632-2009 CELL

BEECHV35 @ CHARTER.NET

FRONT ELEVATION
SCALE: 1/4" = 1'-0"

REQUIRED ROOF VENTILATION:

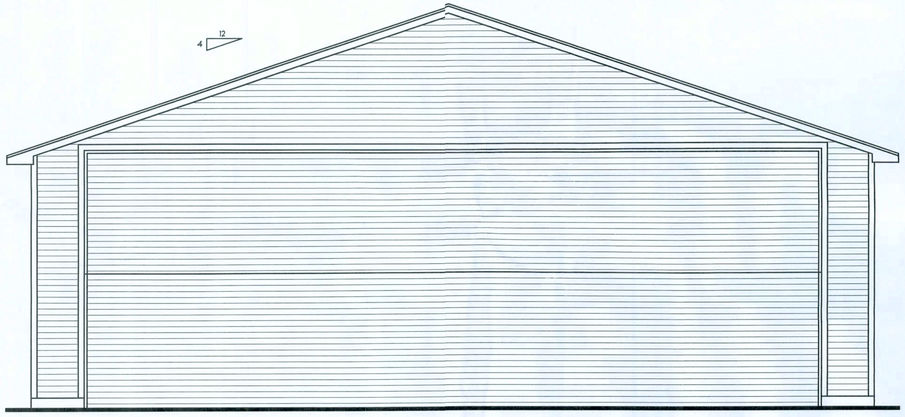
AS PER FLORIDA BUILDING CODE 2309.7

RIDGE VENT
MIN. 50% TOTAL VENT AREA
LOCATED IN THE UPPER PORTION OF ATTIC (MIN. 3' ABOVE EAVE)
2400 S.F. / 300 x 50% = 4 S.F. RIDGE VENT AREA REQUIRED
36.3 FEET OF RIDGE VENT REQUIRED

SOFFIT VENT
2400 S.F. / 300 x 50% = 4 S.F. SOFFIT VENT AREA REQUIRED
133.3 FEET OF SOFFIT VENT REQUIRED

BUILDER MUST VERIFY THE FOLLOWING MINIMUM NET FREE VENT AREAS:

1. RIDGE VENTS = 16 IN2/FT (.11 FT2/FT)
2. OFF-RIDGE VENTS = .70 FT2 PER 4' UNIT
3. SOFFIT VENTS = 4.3 IN2/FT (.03 FT2/FT)



LEFT ELEVATION
SCALE: 1/4" = 1'-0"

RIGHT ELEVATION
SCALE: 1/4" = 1'-0"

REAR ELEVATION
SCALE: 1/4" = 1'-0"

REVISIONS	

SOFPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB168, Lake City, FL 32056, 386-754-549

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer a questions to Mark Disosway, P.E for resolution. Do not proceed without clarification.

CERTIFICATION: I ereby certify that I have examined this plan, ind that the applicable portions of the plan, elating to wind engineering comply with section 6301.2.1, florida building code residential 200, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specifiedocation.

MARKDISOSWAY
P.E. 53915

07APR08

SEAL

Fred & Ann
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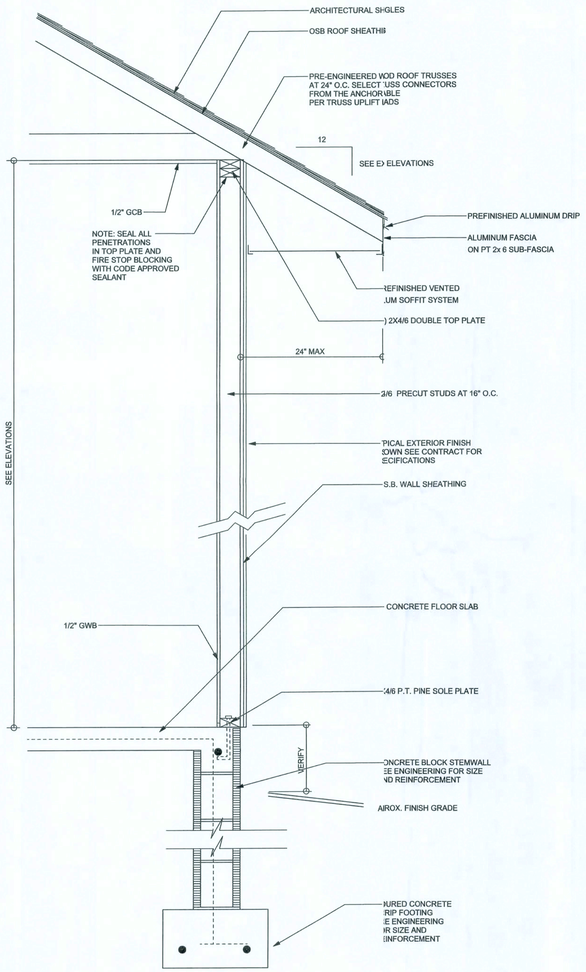
PRINTED DATE:
April 0, 2008

DRAWN BY:	STRUCTURAL BY:
David Disosway	David Disosway

FINALS DATE:
31 / Mar / 08

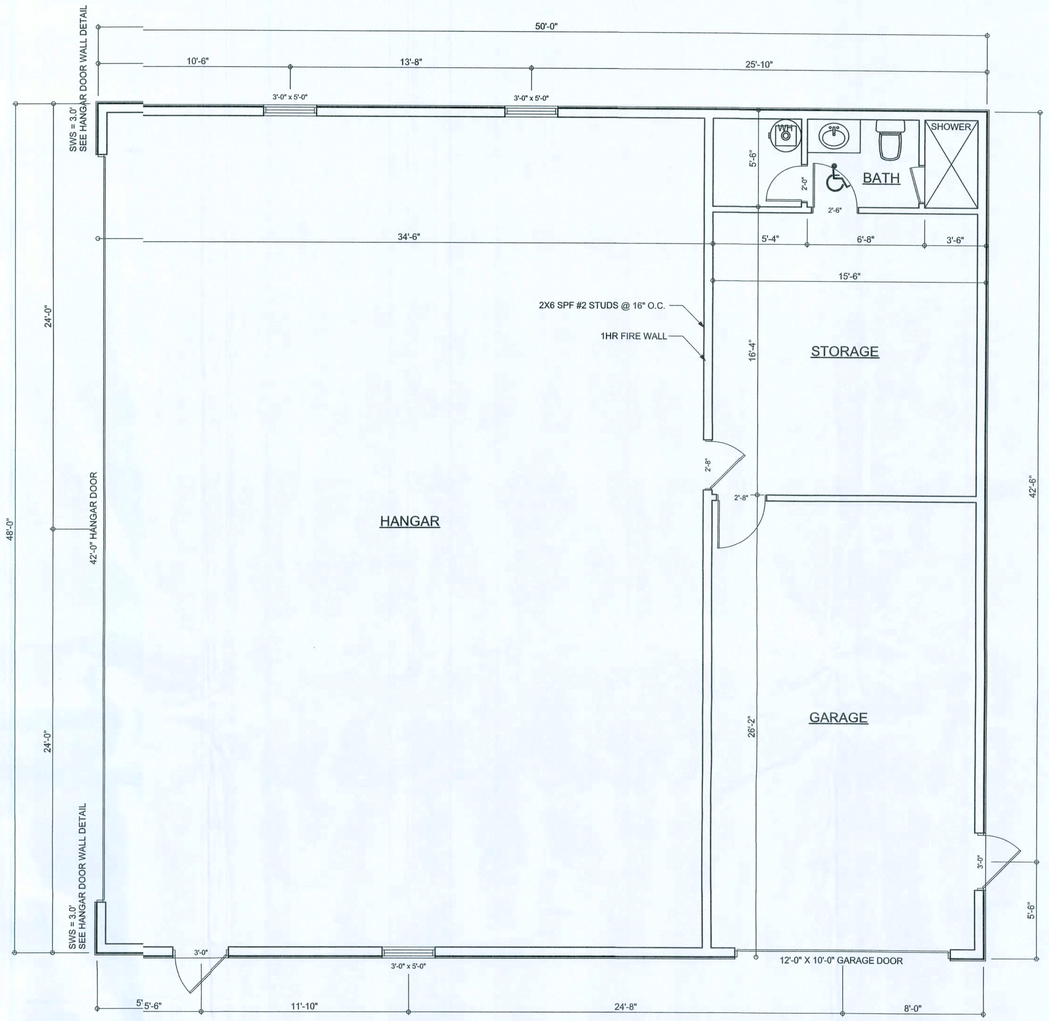
JOB NUMBER:
803121

DRAWING NUMBER
1
OF (SHEETS



TYPICAL DESIGN WALL SECTION
NON - STRUCTURAL DATA

SCALE: 1" = 1'- 0"



FLOOR PLAN
SCALE: 1/4"= 1'-0"

AREA SUMMARY

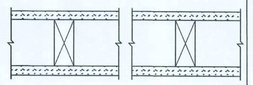
STORAGE / BATH AREA	351	S . F .
GARAGE AREA	409	S . F .
HANGAR AREA	1640	S . F .
TOTAL AREA	2400	S . F .

GA A FILE NO. WP 3605 | GENERIC | 1 HOUR FIRE | 30 TO 34 STC SOUND

GYPSUM WALLBOARD, WOOD STUDS

ONE LA AYER 5/8" TYPE X PLAIN OR PREDECORATED GYPSUM WALLBOARD, WATER-RESISTANT GYPSUM BACKIN NG BOARD, OR GYPSUM VENEER BASE APPLOED PARALLEL OR AT RIGHT ANGLES TO EACH SIDE OF 2x4 WO OOD STUDS 16" oc w/ 6d COATED NAILS, 1 7/8" LONG, 0.0915" SHANK, 1/4" HEADS, 7" oc JOINTS OF SQUARE RE EDGE, BEVEL EDGE OR PREDCORATED WALLBOARD MAY BE LEFT EXPOSED

JOINTS S STAGGERED 16" ON OPPOSITE SIDES. (LOAD-BEARING)



THICKNESS: 4 7/8"
APPROX. WEIGHT: 7 psf
FIRE TEST: UL R1319-4, -6, 6-17-52; UL R2717-39, 1-20-66; UL R3501-52, 3-15-66, UL DESIGN U305; ULC DESIGN W 301
SOUND TEST: OR 64-8, 2-4-64

REVISIONS

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 88, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions suercede scaled dimensions. Refer all questions to Mark Disosway, P.E. or resolution. Do not proceed withot clarification.

CERTIFICATION: I hreby certify that I have examined this plan, ad that the applicable portions of the plan, dating to wind engineering comply with section R01.2.1, florida building code residential 2004 to the best of my knowledge.

LIMITATION: This deign is valid for one building, at specified lcation.

MARK HSOSWAY
P.E 53915

SEAL

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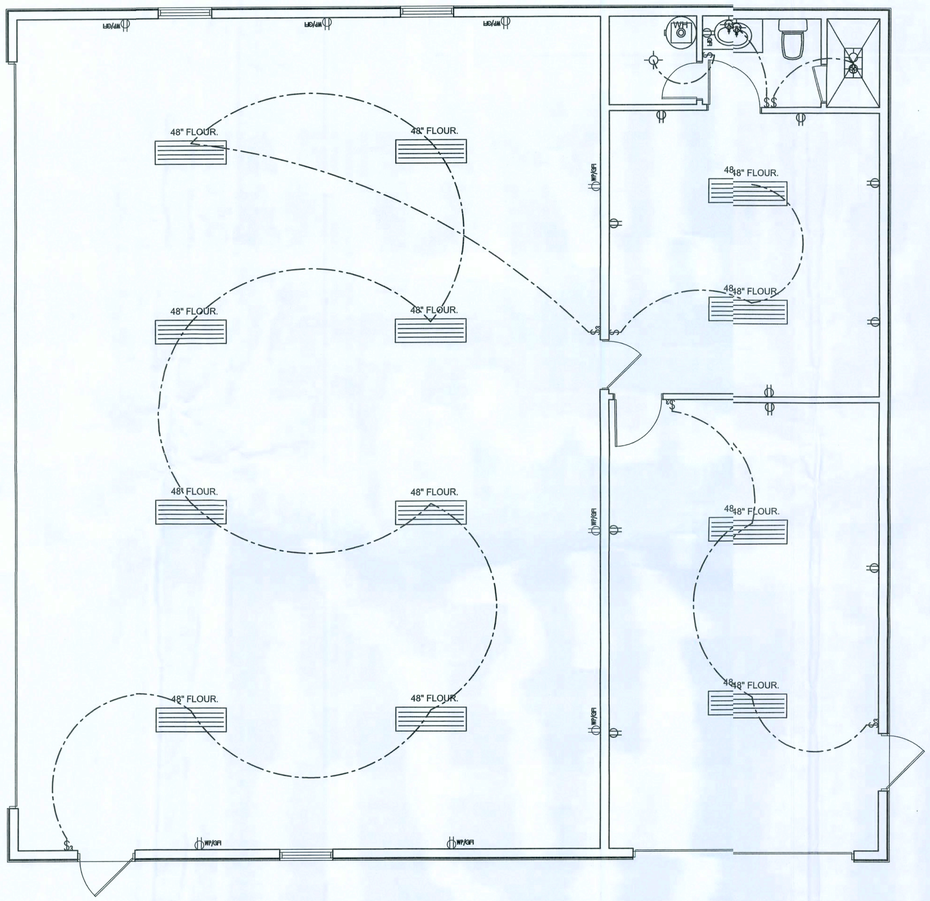
DRAWN BY: David Disosway
STRUCTURAL BY: David Disosway

FINALS DATE: 31 / Mar / 08

JOB NUMBER: 803121

DRAWING NUMBER
2
OF 6 SHEETS



ELECTRICAL PLAN

SCALE: 1/4" = 1'-0"

ELECTRICAL PLAN NOTES

E -1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.

E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.

E -3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.

E -4 ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.

E -5 TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.

E -6 ELECTRICAL CONT'R SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.

E -7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.

E -8 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)

E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION

E -10 A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC CONDUCTORS ENTER THE BUILDING. SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL APPROVAL OF THE BUILDING OFFICIAL

	ELECTRICAL LEGEND
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	2X4 FLUORESCENT LIGHT FIXTURE
	RECESSED CAN LIGHT
	BATH EXAUST FAN WITH LIGHT
	BATH EXAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET
	220v OUTLET
WP/GFI	GFI DUPLEX OUTLET
	SMOKE DETECTOR
$	WALL SWITCH
$3	3 WAY WALL SWITCH
$4	4 WAY WALL SWITCH
WP/GFI	WATER PROOF GFI OUTLET
	PHONE JACK
	TELEVISION JACK
	GARAGE DOOR OPENER
	WALL HEATER

REVISIONS	

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGNEER: Mark Disosway, PE No.53915, PcB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimension supercede scaled dimensions. Refe all questions to Mark Disosway, I.E. for resolution. Do not proceed wihout clarification.

CERTIFICATION I hereby certify that I have examined this plai, and that the applicable portions of the plri, relating to wind engineering comply with sectin R301.2.1, florida building code residential 204, to the best of my knowledge.

LIMITATION: Thi design is valid for one building, at specifid location.

MARK DISOSWAY
P.E. 53915

SEAL

Frd & Ann
Elfrs Hangar

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PRINTED DATE:
April08, 2008

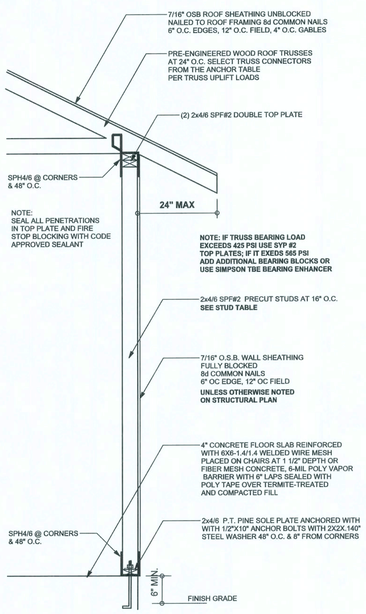
DRAWN BY:	STRUCTURAL BY:
David Disosway	David Disosway

FINALS DATE
31 / Mar / 0

JOBNUMBER:
303121

DRAWING NUMBER

0/ 6 SHEETS



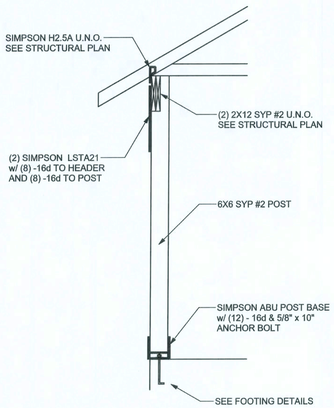
ONE STORY WALL SECTION

SCALE: 3/4" = 1'-0"

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

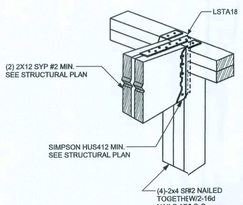
(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10' STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20.0' STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.



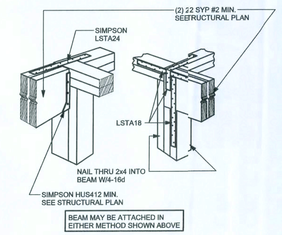
TYPICAL PORCH POST DETAIL

SCALE: 1/2" = 1'-0"



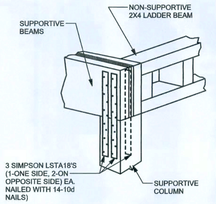
BEAM MID-WALL CONECTION DETAIL

SCALE: N.T.S.



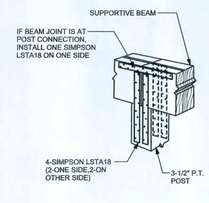
BEAM CORNER CONNECTIN. DETAIL

SCALE: N.T.S.



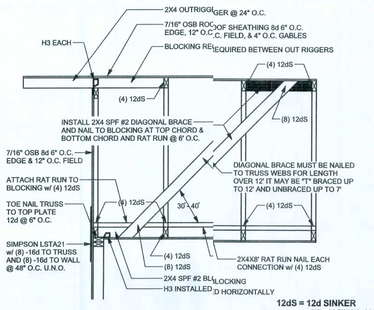
SUPPORTIVE POST TO BEAM DETAIL FOR SINGLE BEAM

SCALE: N.T.S.



SUPPORTIVE CENTER POST TO BEAM DETAIL

SCALE: N.T.S.



12dS = 12d SINKER
OR .135" X 3.125"
OR .131 X 3.25"

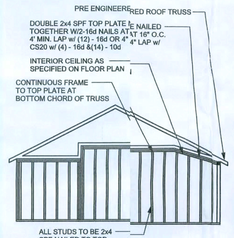
SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C.
FOR GABLE HEIGHT UP TO 25'-0" 110 MPH, EXP. C, ENCLOSED

GABLE BRACING DETAIL

SCALE: 1/2" = 1'-0"

GRADE & SPECIES TABLE

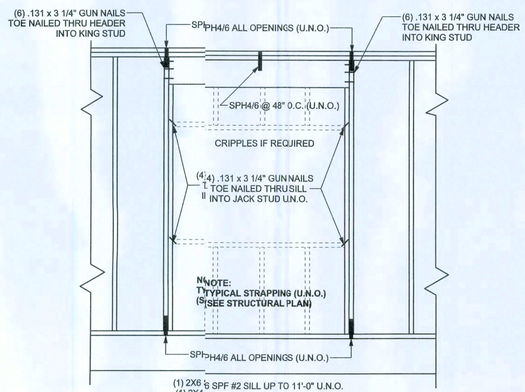
		Fb (psi)	E (10^6 psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0



CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL

SCALE: N.T.S.

NOTE:
IF TRUSS TO WALL STRAPS ARE NAILED
TO THE HEADER THE SPH4/6 @ 48" O.C.
ARE NOT REQUIRED



TYPICAL HEADER STRAPING DETAIL

SCALE: 1/2" = 1'-0"

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 80KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THR THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE;
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

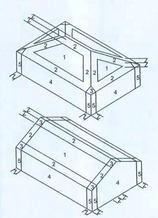
ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			**TO FOUNDATION**
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
		STUD STRAP CONNECTOR*			**TO STUDS**
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	**TO STUDS**		**TO FOUNDATION**
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTT31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT, NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)
BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION
1.) BASIC WIND SPEED = 110 MPH
2.) WIND EXPOSURE = B
3.) WIND IMPORTANCE FACTOR = 1.0
4.) BUILDING CATEGORY = II
5.) ROOF ANGLE = 10-45 DEGREES
6.) MEAN ROOF HEIGHT = <30 FT
7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



Zone	Effective Wind Area (ft2)			
	10		100	
1	19.9	-21.8	18.1	-18.1
2	19.9	-25.5	18.1	-21.8
2 Othg		-40.6		-40.6
3	19.9	-25.5	18.1	-21.8
3 Othg		-68.3		-42.4
4	21.8	-23.6	18.5	-20.4
5	21.8	-29.1	18.5	-22.6
Doors & Windows Worst Case (Zone 5, 10 ft2)	21.8	-29.1		
8x7 Garage Door	19.5	-22.9		
16x7 Garage Door	18.5	-21.0		

DESIGN LOADS	
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
SOIL BEARING CAPACITY	1000PSF
NOT IN FLOOD ZONE (BUILDER TO VERIFY)	

REVISIONS	

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENNINEER: Mark Disosway, PE No.53915, PIB 868, Lake City, FL 32056, 386-754-419

DIMENSIONS:
Stated dimensios supercede scaled dimensions. Refr all questions to Mark Disosway, *.E. for resolution. Do not proceed ithout clarification.

CERTIFICATION I hereby certify that I have examined this *plan, and that the applicable portions of the plin, relating to wind engineering comply with sectn R301.2.1, florida building code residential 004, to the best of my knowledge.

LIMITATION: Ths design is valid for one building, at specied location.

MARK DISOSWAY
P.E. 53915

SEAL

Fred & Ann
Elfrs Hangar

„DDRESS:
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PRINTED DATE:
Apri08, 2008

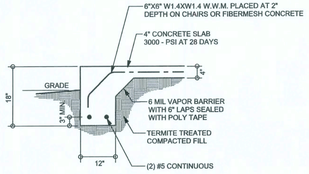
DRAWN BY: David Disosway
STRUCTURAL BY: David Disosway

FINALS DATE: 31 / Mar / 08

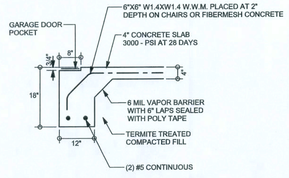
JOBNUMBER: 303121

DRAWING NUMBER
S-1

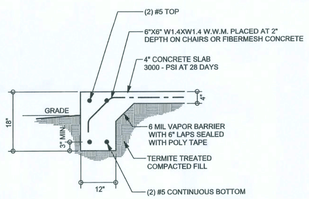
O' 6 SHEETS



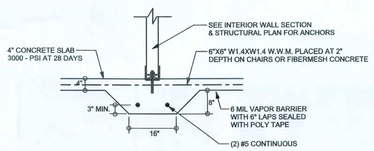
F1 S-2 MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"



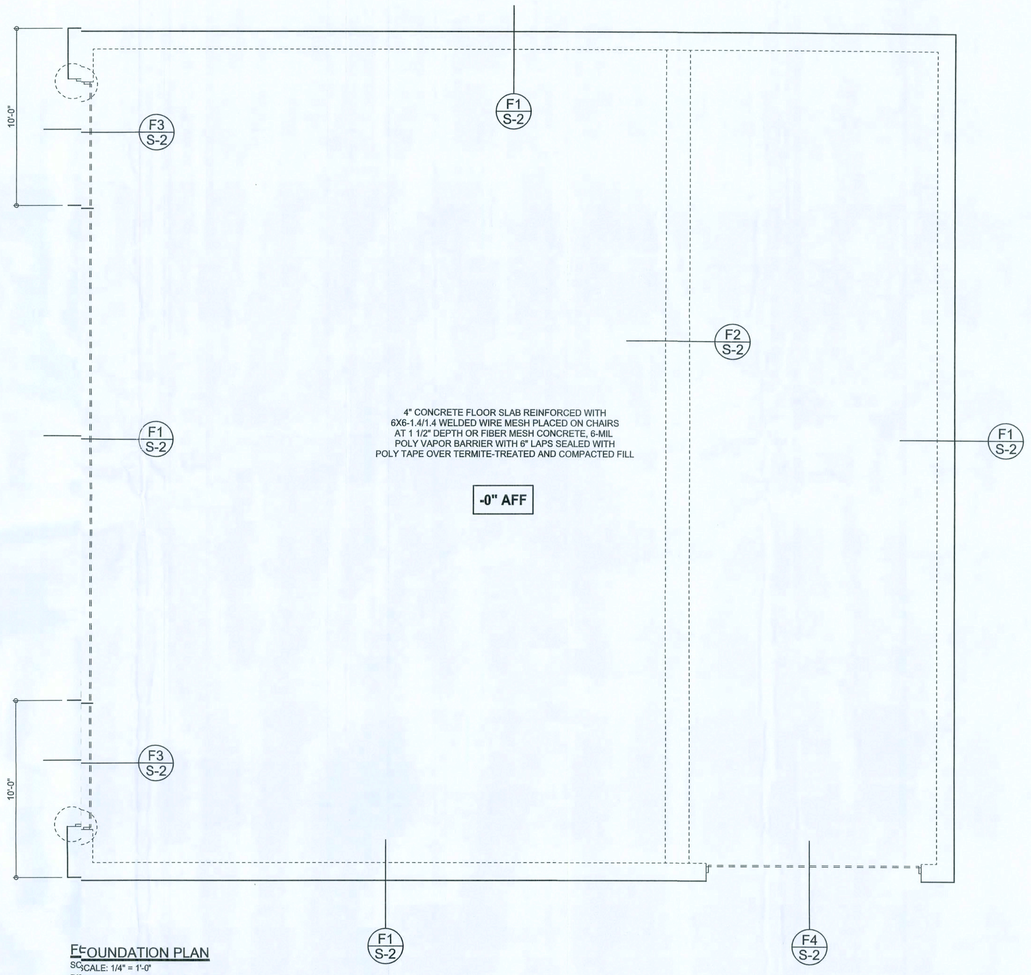
F GARAGE DOOR FOOTING
S2 SCALE: 1/2" = 1'-0"



F3 S-2 MONOLITHIC FOOTING @ HANGAR DOOR WAL
SCALE: 1/2" = 1'-0"



F2 S-2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



FOUNDATION PLAN
SCALE: 1/4" = 1'-0"
DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

REVISIONS	

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer a questions to Mark Disosway, P.E for resolution. Do not proceed without clarification.

CERTIFICATION: I kereby certify that I have examined this plan, nd that the applicable portions of the plan, elating to wind engineering comply with section 1301.2.1, florida building code residential 200, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specifieclocation.

MARK DISOSWAY
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April 0, 2008

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FINALS DATE:
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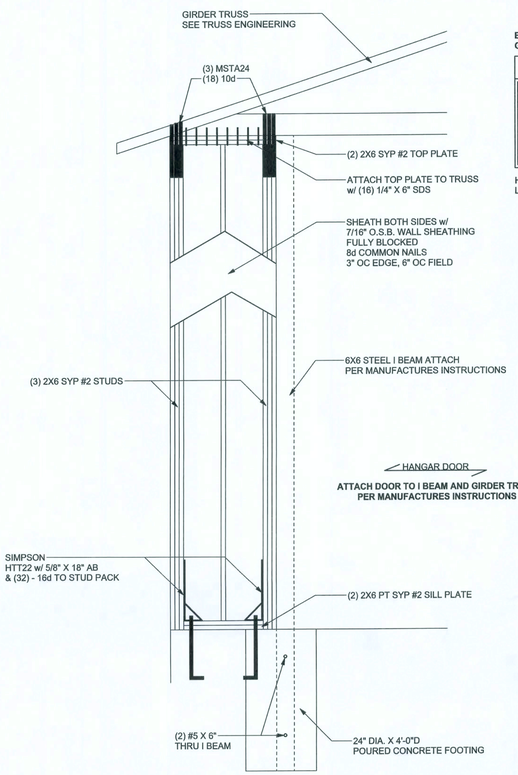
JOB NUMBER:
803121

DRAWING NUMBER
S-2

OF SHEETS



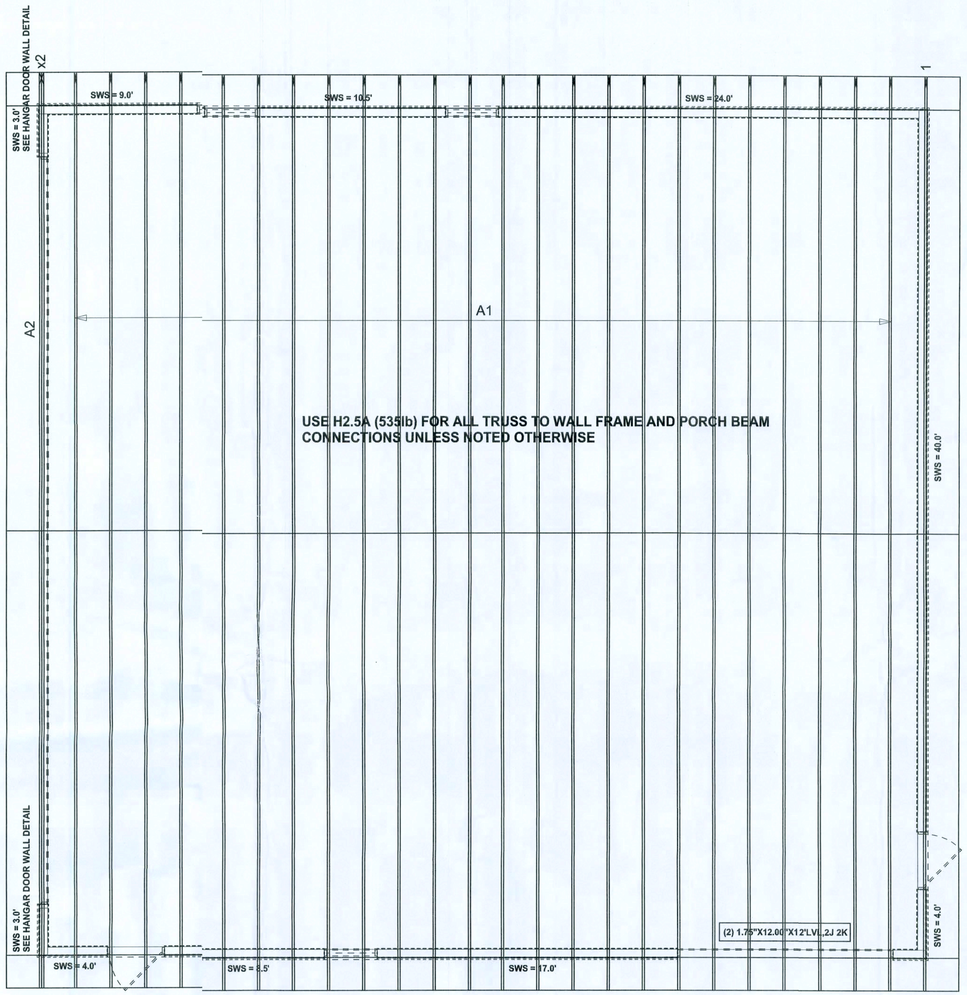
HANGAR DOOR WALL DETAIL

SCALE: 1/2" = 1'-0"

BUIDER MUST VERIFT THAT TRUSS ENGINEER LOADS GAIE GIRDER FOR SHEARWALL LOADS AS FOLLOWS:

	VERTICAL				HORIZONTAL	
	0'	3'	45'	48'	0-3'	45-48'
CSE #1	-4000 LB	+4000 LB	-4000 LB	+4000 LB	+2000 LB	+2000 LB
CSE #2	+4000 LB	-4000 LB	+4000 LB	-4000 LB	-2000 LB	-2000 LB

HOZONTAL LOADS ARE DRAG LOADS INTO BOTTOM CHORD & OUT THE TOP CHORD.
LEFTO RIGHT IS POSITIVE



STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 S SYP #2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL I HEADERS SHALL HAVE (1) JACK STUD & (1) K KING STUD EACH SIDE (U.N.O.)

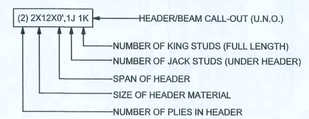
SN-3 DIMENSIONS ON STRUCTURAL SHI SHEETS ARE NOT EXACT. REFER TO ARCHI HITECTURAL FLOOR PLAN FOR ACTUAL DIMEN SIONS

SN-4 PERMANENT TRUSS BRACING IS T TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE REST TRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI SI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SU SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR WALL
IBW	1ST FLOOR INTERIOR BEARING WALL
IBW	2ND FLOOR INTERIOR BEARING WALL

HEADER LEGEND



TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	44.0'	50.0'
LONGITUDINAL	34.2'	73.0'

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. W.B. HOWLAND JOB #5350

REVISIONS	

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINER: Mark Disosway, PE No.53915, POB 68, Lake City, FL 32056, 386-754-5411

DIMENSIONS:
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CERTIFICATION: I hreby certify that I have examined this plan, nd that the applicable portions of the plan, elating to wind engineering comply with section I301.2.1, florida building code residential 200, to the best of my knowledge.

LIMITATION: This deign is valid for one building, at specifiedlocation.

MARKDISOSWAY
P.E. 53915

07 MAR 08

SEAL

Frec & Ann
Elfers Hangar

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PRINTED DATE: April 0, 2008

DRAWN BY:	STRUCTURAL BY:
David Disosway	David Disosway

FINALS DATE: 31 / Mar / 08

JOB NUMBER: 8#3121

DRAWING NUMBER

S-3

OF t SHEETS