

30 yr architect shingle roofing

Cathedral ceilings in main area (Living room, dining room, Kitchen)

Aluminum soffit and lining

Vinyl solid soffit or ceiling of porch

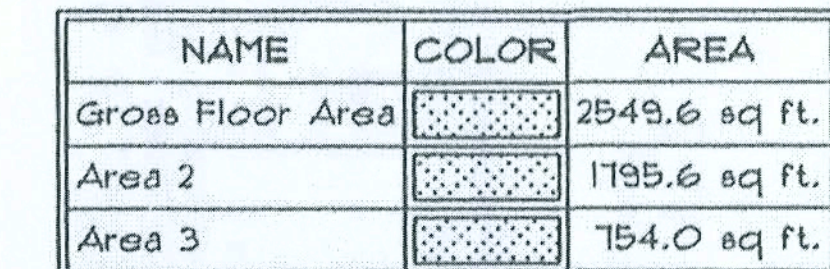
Hardie plank siding

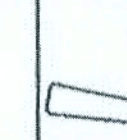














Low-E vinyl windows

Spray foam insulation in attic with battens in walls

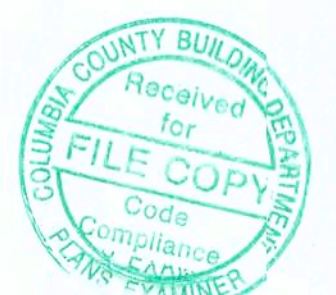
Pec w/ Manl block ystem

House to be built according to Florida Standard Code of engineered plans



ELECTRICAL LEGEND		
ELECTRICAL	COUNT	SYMBOL
ceiling fan 5 bladed 01	5	
can light 6inch	3	
ceiling dish square	2	
ceiling light 01	2	
ceiling light vent square	2	
fluorescent light 2 x 4	1	
exterior craftsman light fixture	1	
spotlight double with motion detector	4	
electrical meter	1	
electrical panel	1	
outlet	45	
outlet gfi	3	
smoke detector	5	
wall mounted 02 2 lights	2	
pendant cone	2	

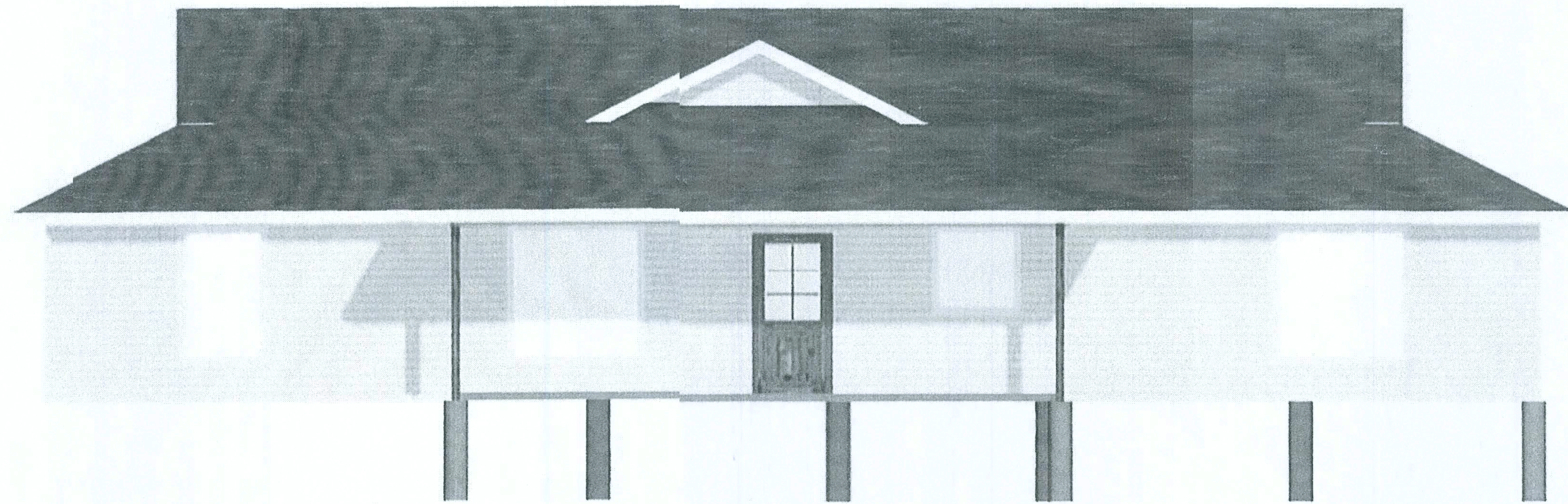
John Norris Construction  
Michael and Patricia Johnson  
Lake City, FL 32024



Michael and Patricia Johnson

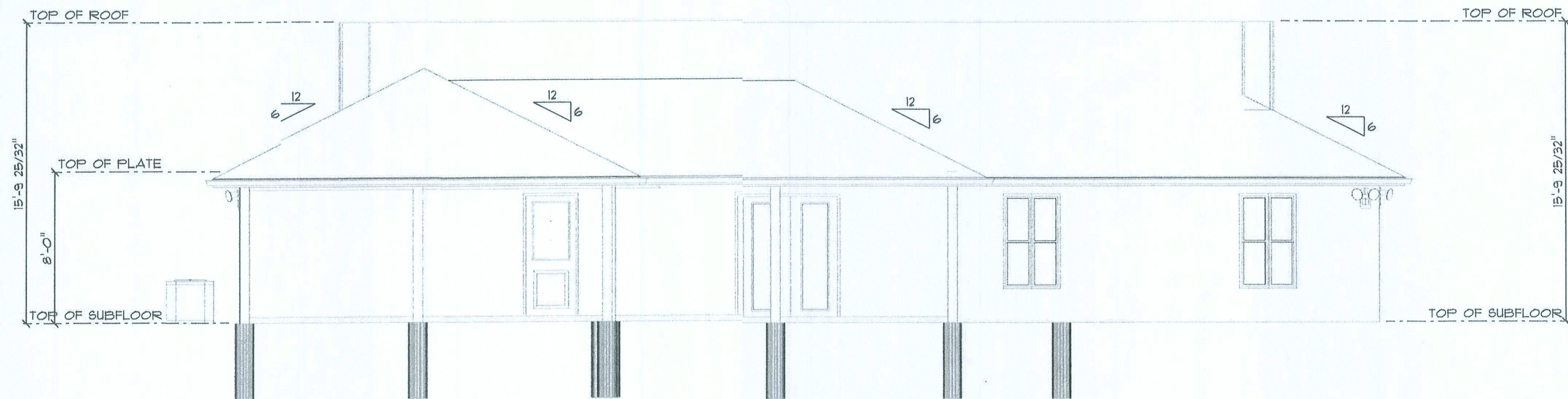
SCALE: 1" = 10'-0"





## SOUTH ELEVATION

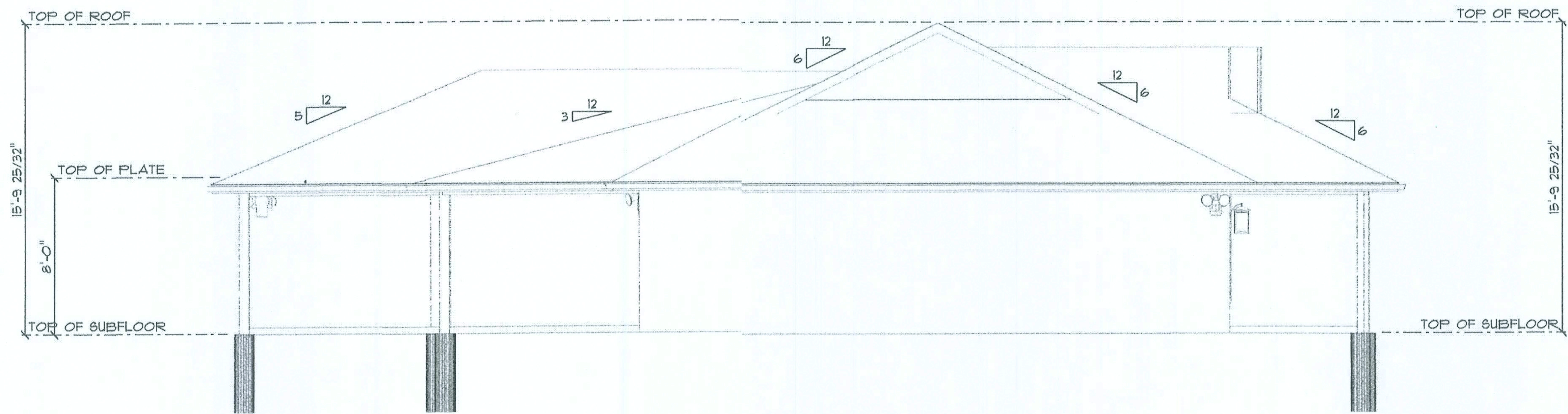
SCALE:  $\frac{1}{8}" = 1'-0"$



## NORTH ELEVATION

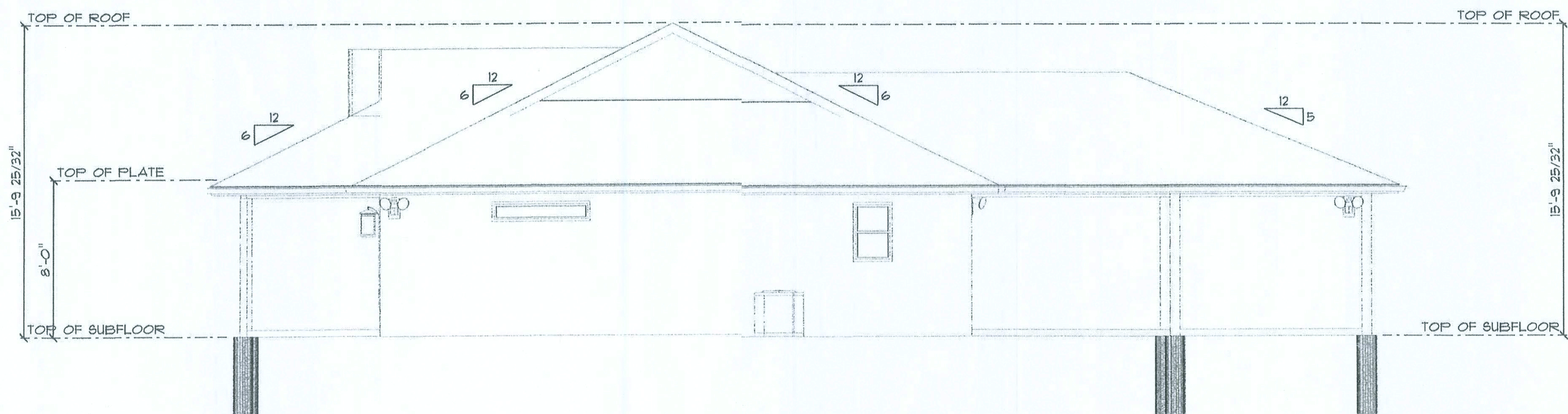
SCALE:  $\frac{1}{8}" = 1'-0"$





## WEST ELEVATION

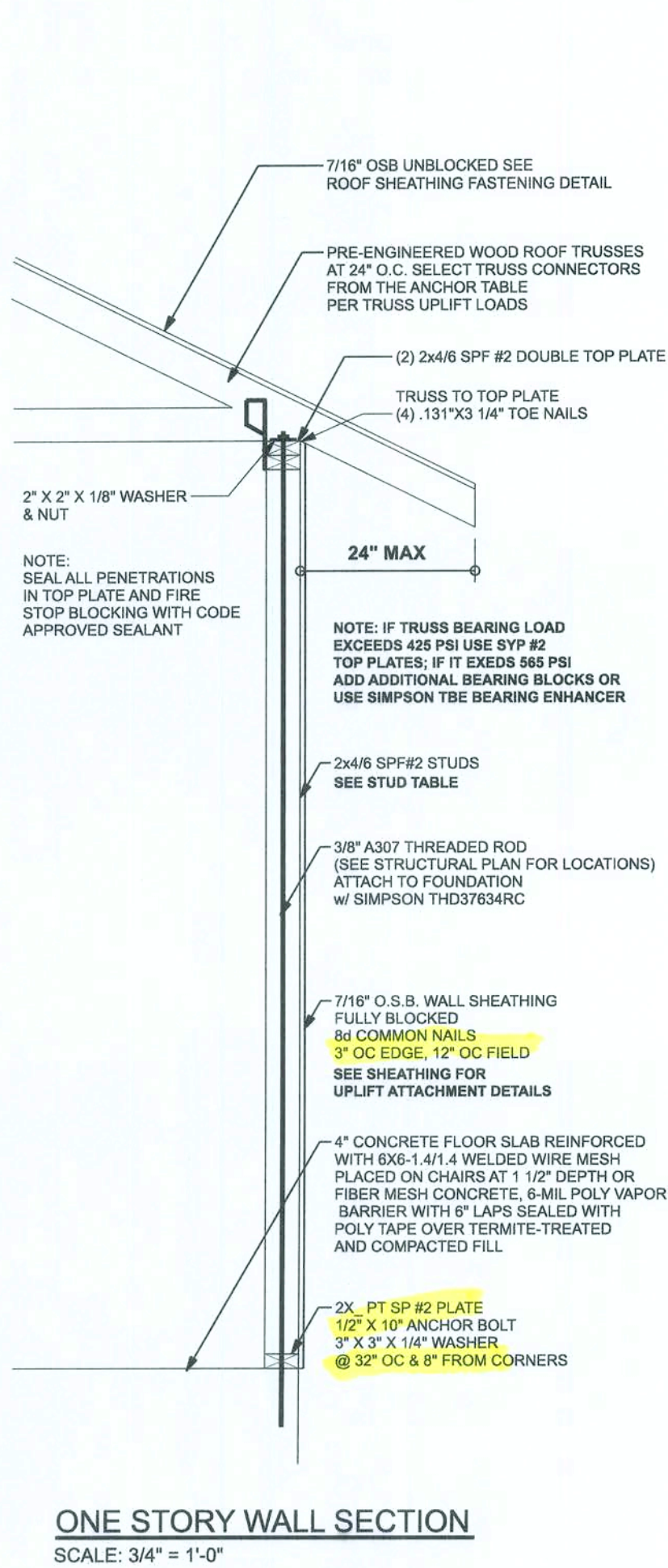
SCALE: 1/8" = 1'-0"



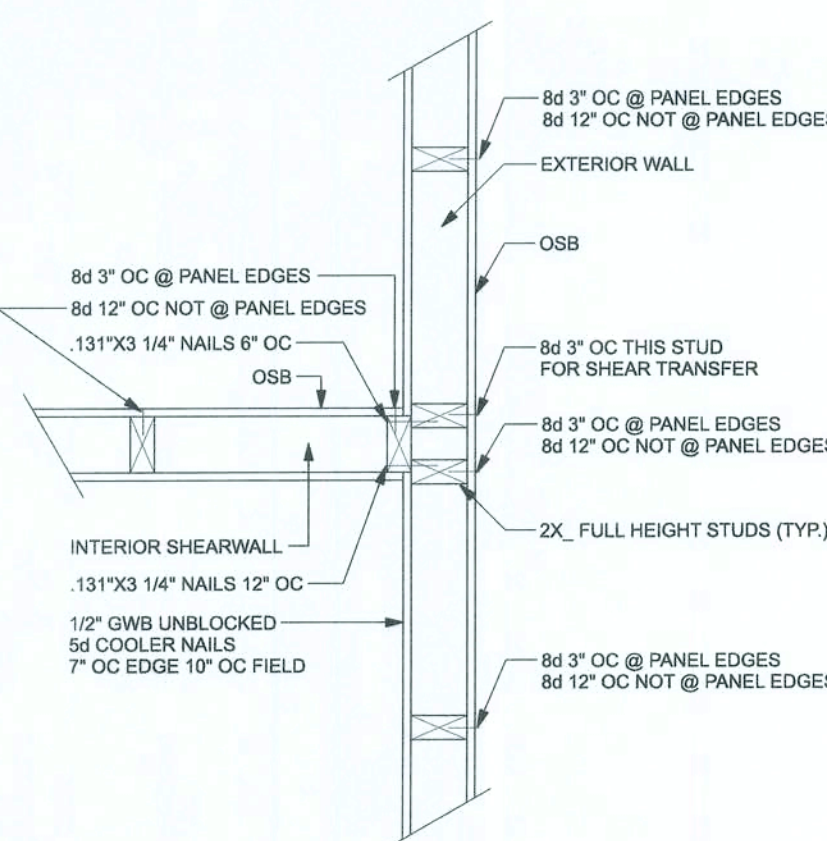
## EAST ELEVATION

SCALE: 1/8" = 1'-0"

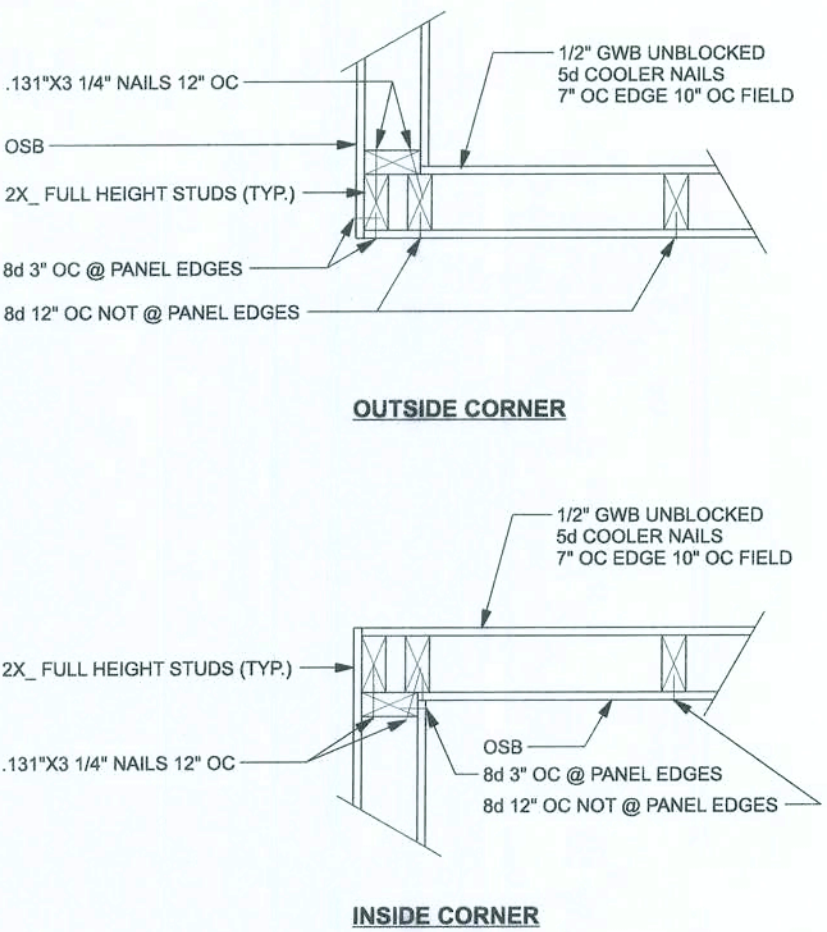




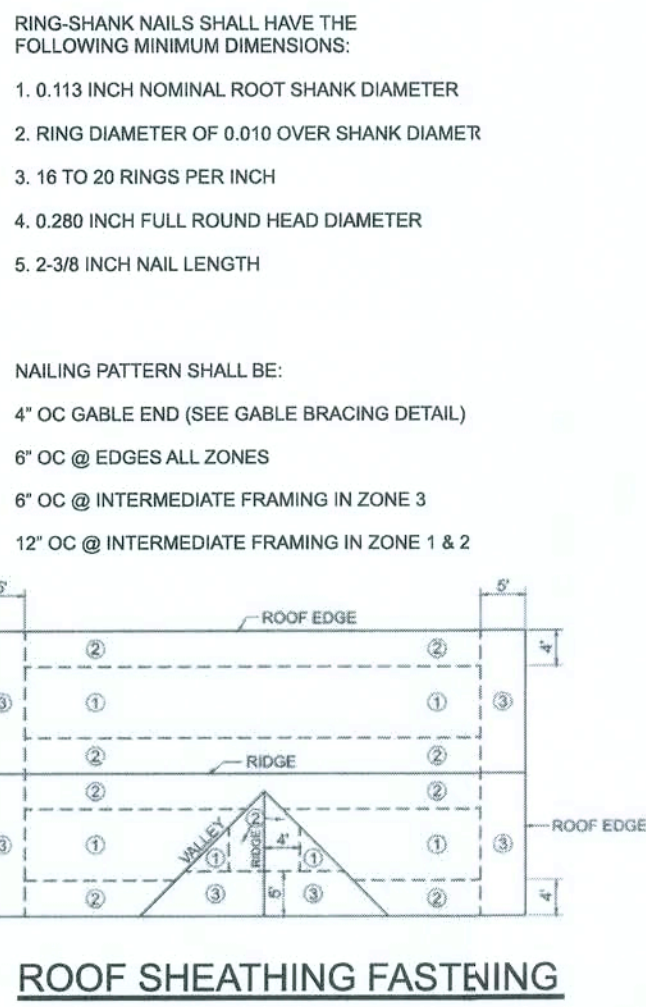
**ONE STORY WALL SECTION**  
SCALE: 3/4" = 1'-0"



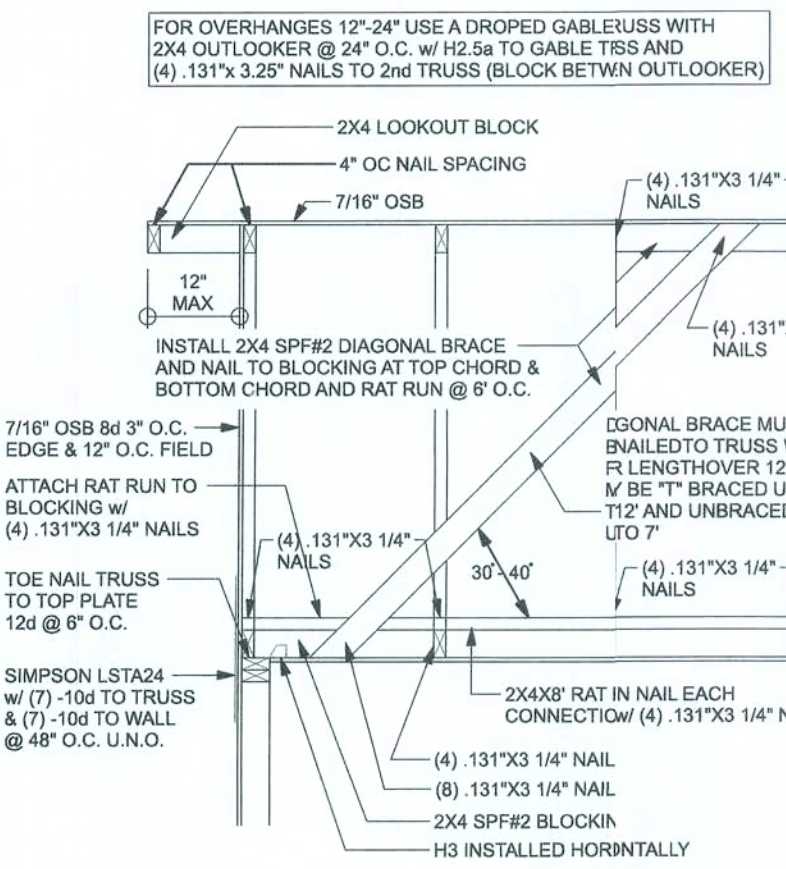
**(TYP.) INTERSECTING WALL FRAMING**  
WOOD FRAME



**(TYP.) CORNER FRAMING**  
WOOD FRAME

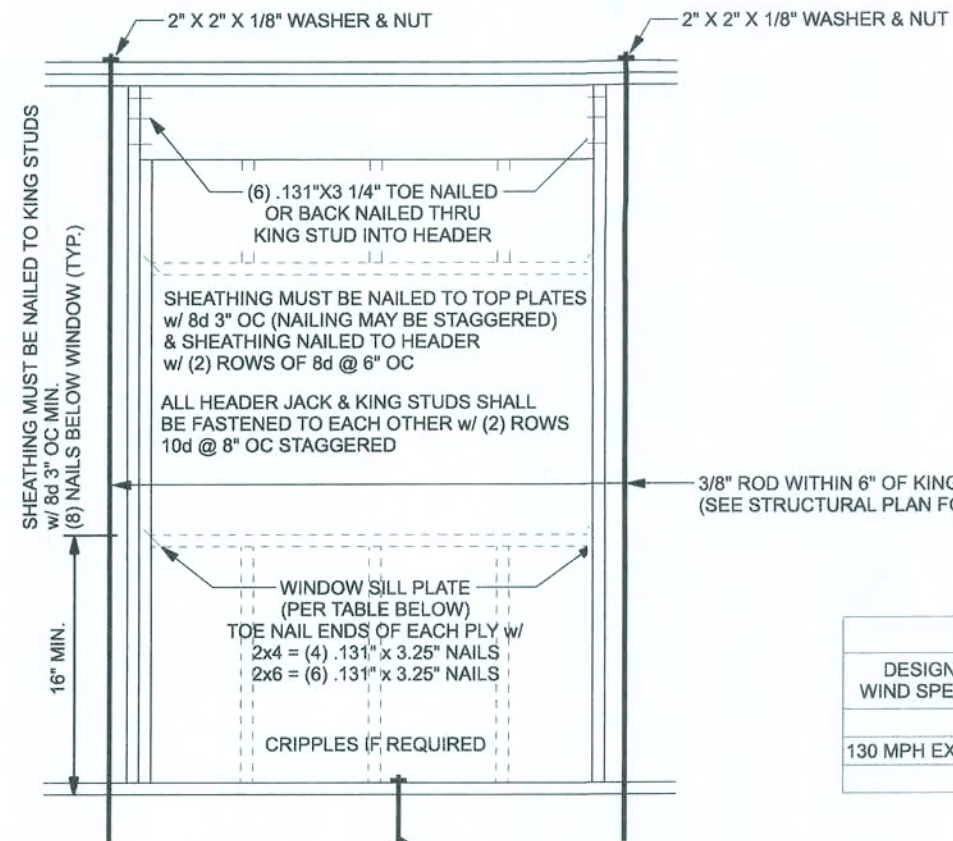


**ROOF SHEATHING FASTENING**

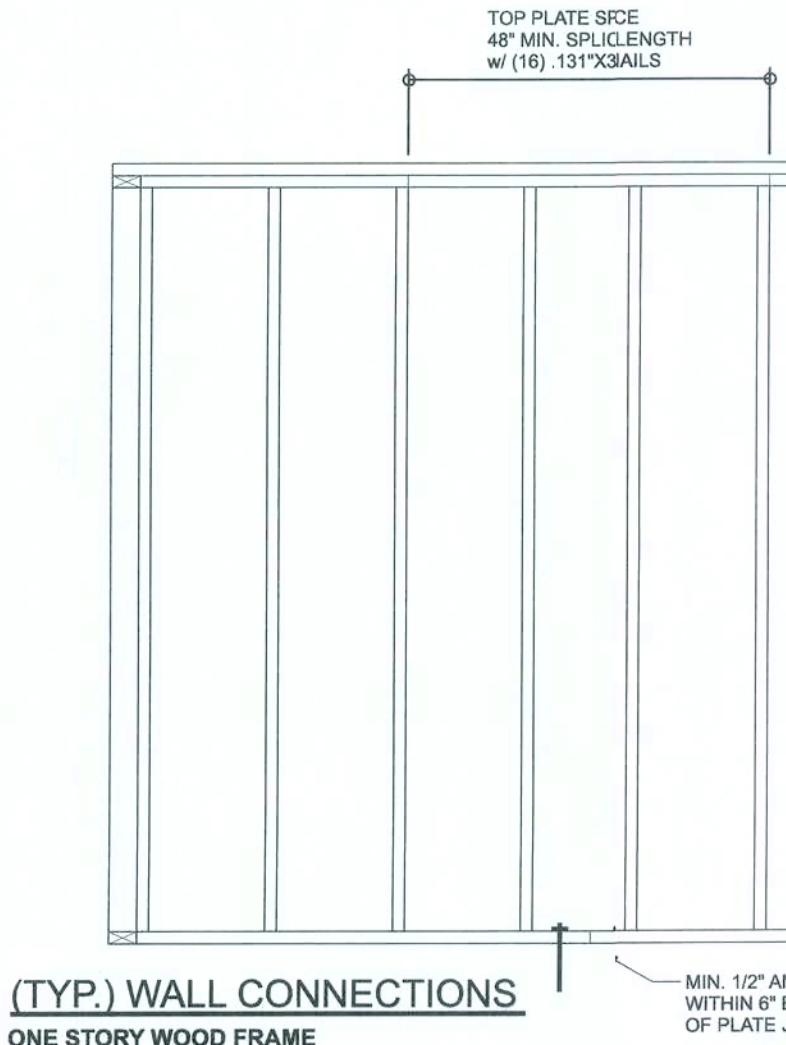


**SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C.**  
FOR GABLE HEIGHT UP TO 25'-0" 130 MPH, EXP. C.NCLOSED

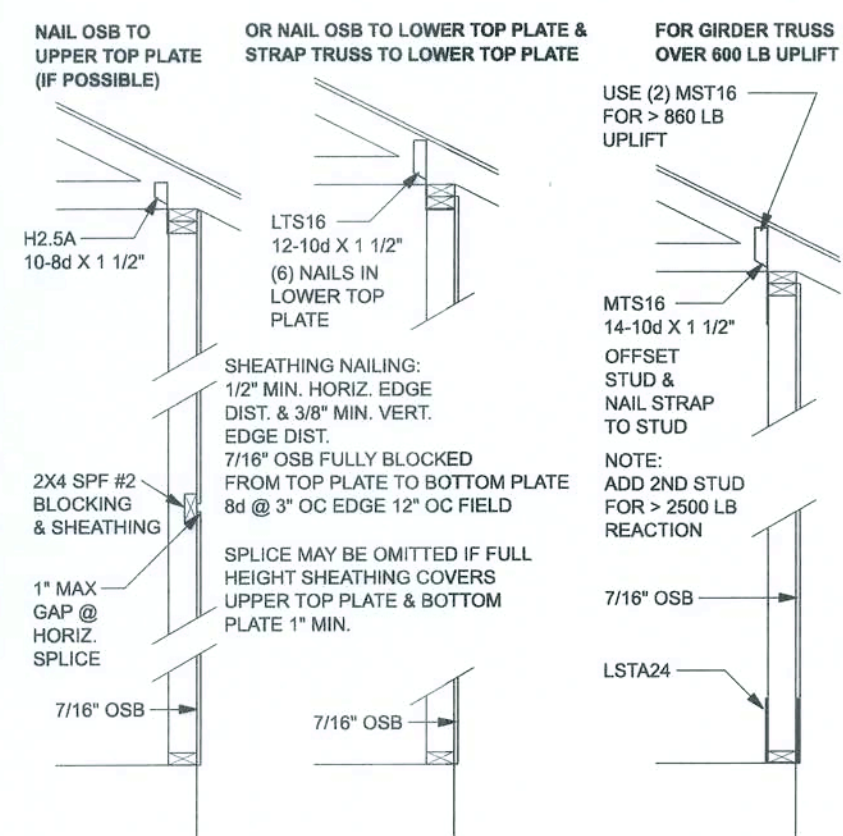
**(TYP.) GABLE BRACING DETAIL**  
WOOD FRAME



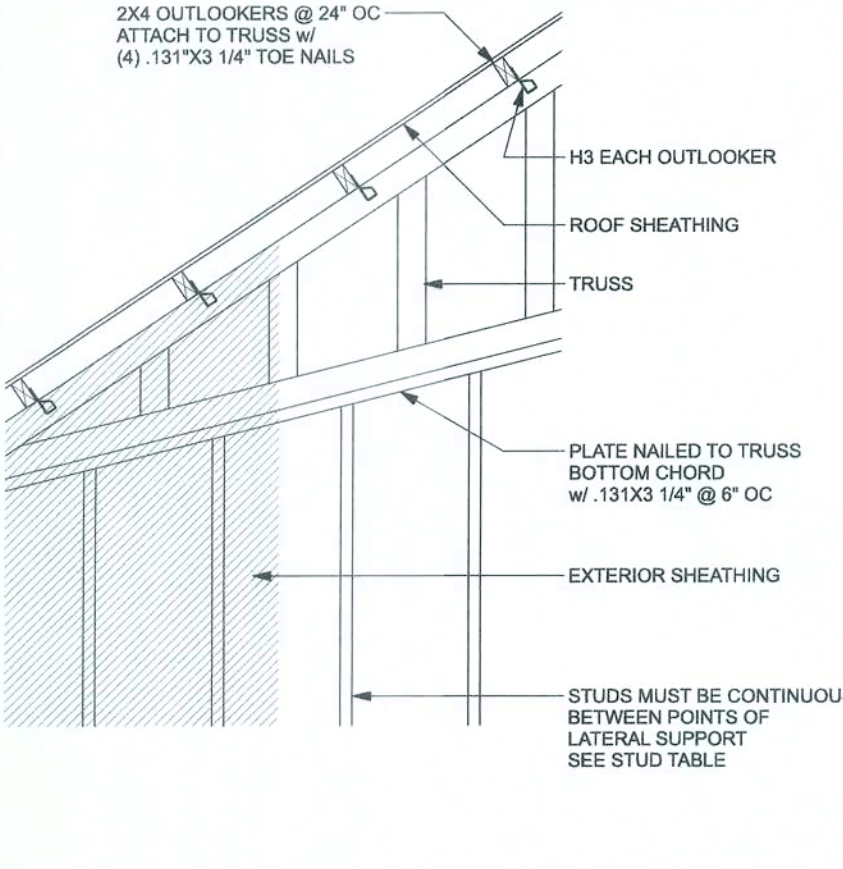
**TYPICAL HEADER STRAPPING DETAIL**  
ONE STORY WOOD FRAME w/ STRAPS & ANCHRS



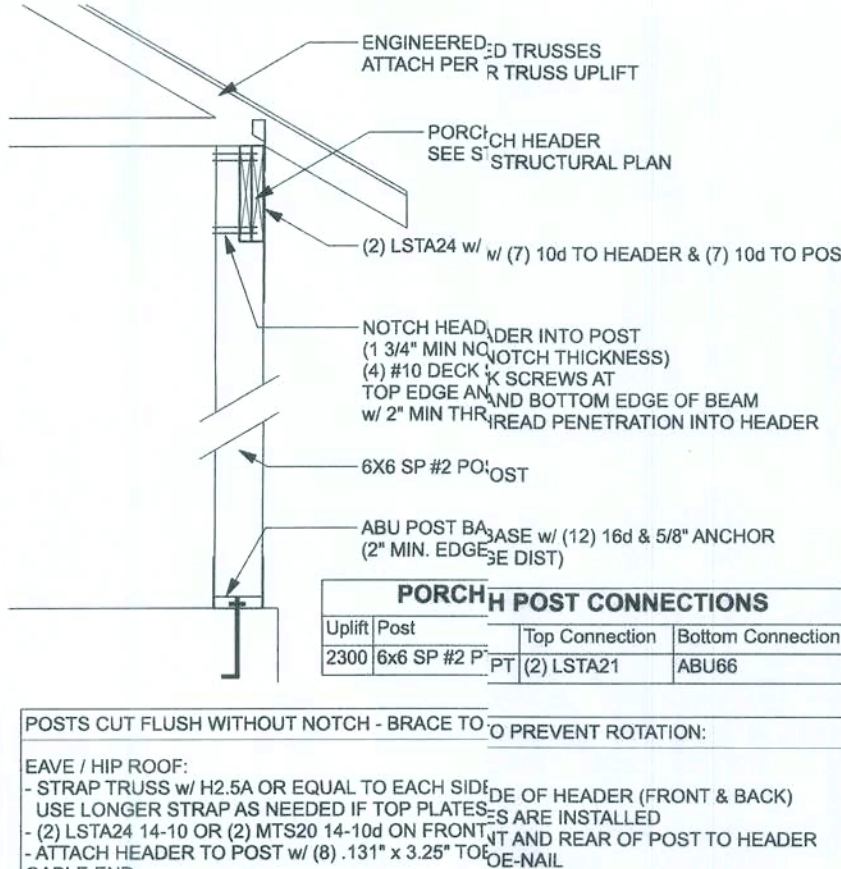
**(TYP.) WALL CONNECTIONS**  
ONE STORY WOOD FRAME



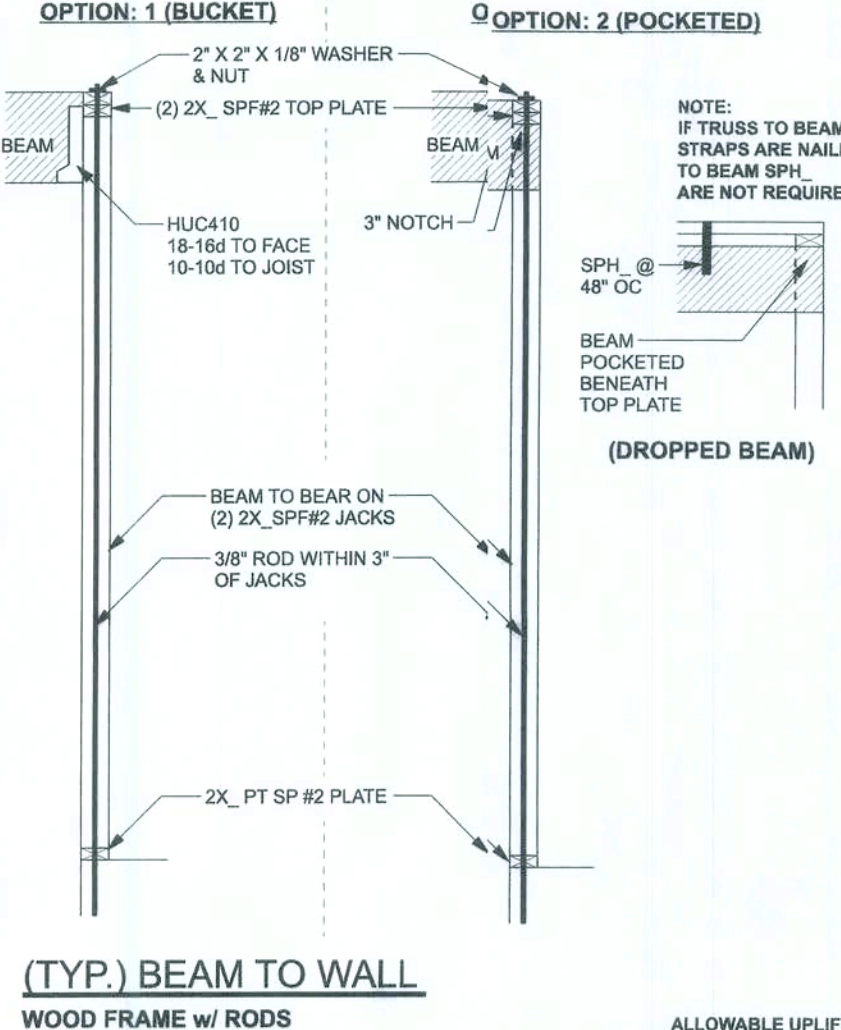
**SHEATHING FOR UPLIFT ATTACHMENT DETAILS**  
ONE STORY WOOD FRAME



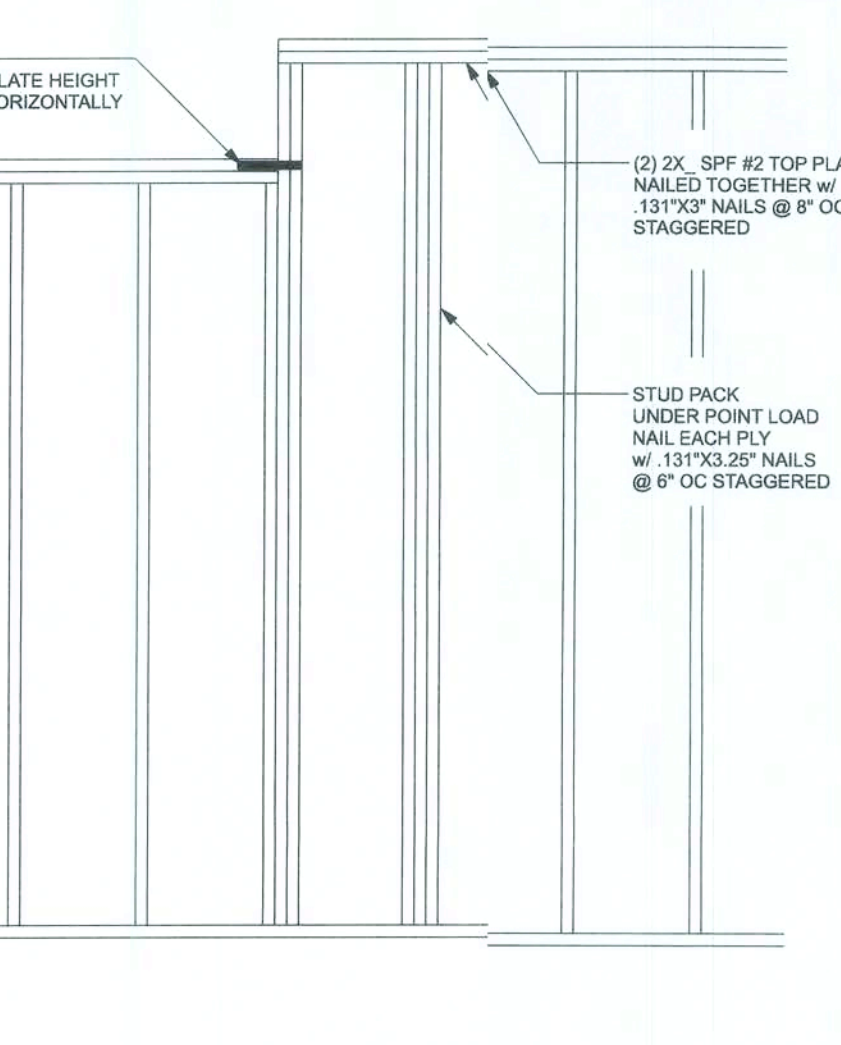
**(TYP.) GABLE WALL w/ VAULTED CEILING**  
WOOD FRAME



**(TYP.) PORCH POST**  
ONE STORY WOOD



**(TYP.) BEAM TO WALL**  
WOOD FRAME w/ RODS



**(TYP.) WALL CONNECTIONS**  
ONE STORY WOOD FRAME

CONNECTOR TABLE					
Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter	
615	485	SDWC15600			
415	290	H3	4-8d x 1 1/2"	4-8d x 1 1/2"	
575	495	H2.5A	5-8d x 1 1/2"	5-8d x 1 1/2"	
1340	1015	H10A	9-10d1 1/2"	9-10d1 1/2"	
720	620	LTS12-30	4-10d1 1/2"	8-10d1 1/2"	
1000	860	MTS12-30	7-10d1 1/2"	7-10d1 1/2"	
1450	1245	HTS20-30	12-10d1 1/2"	12-10d1 1/2"	
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member	
1235	1235	LSTA21	8-10d	8-10d	
1440	1455	MS1A24	9-10d	9-10d	
1030	1030	CS20	7-10d	7-10d	
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate	
885	535	SP1	8-10d	4-10d	
1065	665	SP2	8-10d	8-10d	
771	771	LSTA24	10-10d	wrap under or over plate	
1235	1235	LSTA24	14-10d	wrap under or over plate	
Uplift SP	Uplift SPF	Holdowns @ Stenwall	To Stud / Post	Anchor	
1625	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titan HD	
4235	3640	HT14	18-16d2 1/2"	1/2"x12" Titan HD	
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor	
1625	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x6" Titan HD	
4235	3640	HT14	18-16d2 1/2"	1/2"x12" Titan HD	
Uplift SP	Uplift SPF	Post Bases @ Stenwall	To Post	Anchor	
2200		ABU44	12-16d	5/8"x12" Drill & Epoxy	
2300		ABU66	12-16d	5/8"x12" Drill & Epoxy	
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	Anchor	
2200		ABU44	12-16d	5/8"x7" Drill & Epoxy	
2300		ABU66	12-16d	5/8"x7" Drill & Epoxy	

**EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:**

**THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)**

(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

GRADE & SPECIES TABLE			
	SP #2	Fd	E
2x8	925	1.4	
2x10	800	1.4	
2x12	750	1.4	
GLB	2600	1.9	
LSL	1700	1.7	
LVL	2950	2.0	
PSL	2900	2.0	

## GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 418LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F<sub>c</sub> = 2500 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, F<sub>y</sub> = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C-1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WMM OR REINFORCING STEEL. RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 40, DEFORMED BARS, F<sub>y</sub> = 40 KSI, ALL LAP SPLICES 40 " DB (2" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL, DIAPHRAGMS, 7/16" OSB SHEATHING, UNLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT, AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

## BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DERRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

## ROOF SYSTEM DESIGN:

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE, AND TO PROVIDE RESISTANT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

## DESIGN CRITERIA & LOADS:

BUILDING CODE	6TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2017)
CODE FOR DESIGN LOADS	ASCE 7-10
<b>WINDLOADS</b>	
BASIC WIND SPEED (ASCE 7-10, 38 GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	I
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
<b>C &amp; C DESIGN PRESSURES</b> SEE TABLE	
<b>FLOOR LOADING</b>	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
<b>ROOF LOADING</b>	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
<b>SOIL BEARING CAPACITY</b>	
FLOOD ZONE	
THIS BUILDING IS NOT IN THE FLOOD ZONE	

COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C) (Vult)			
EFFECTIVE WIND AREA (Ft <sup>2</sup> )	ZONE 4 INTERIOR	ZONE 5 END 4' FROM ALL OUTSIDE CORNER	
0 - 20	+42.6 -46.2	+42.6 -57	
<b>GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C) (ASD)</b>			
8x7 GARAGE DOOR	+22.6 -25.5		
16x7 GARAGE DOOR	+21.7 -24.1		

John Norris Construction L.L.C.

Michael & Patricia Johnson Res.

PROJECT ADDRESS:  
339 NW 14th Ave  
Lake City, FL 32805

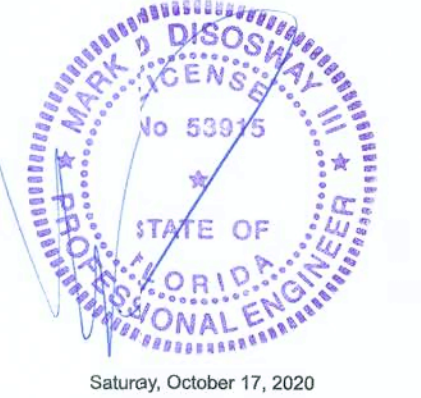
DIMENSIONS:  
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 6th Edition Florida Building Code Residential (2017) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915



Saturday, October 17, 2020

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Lake City, Florida 32025  
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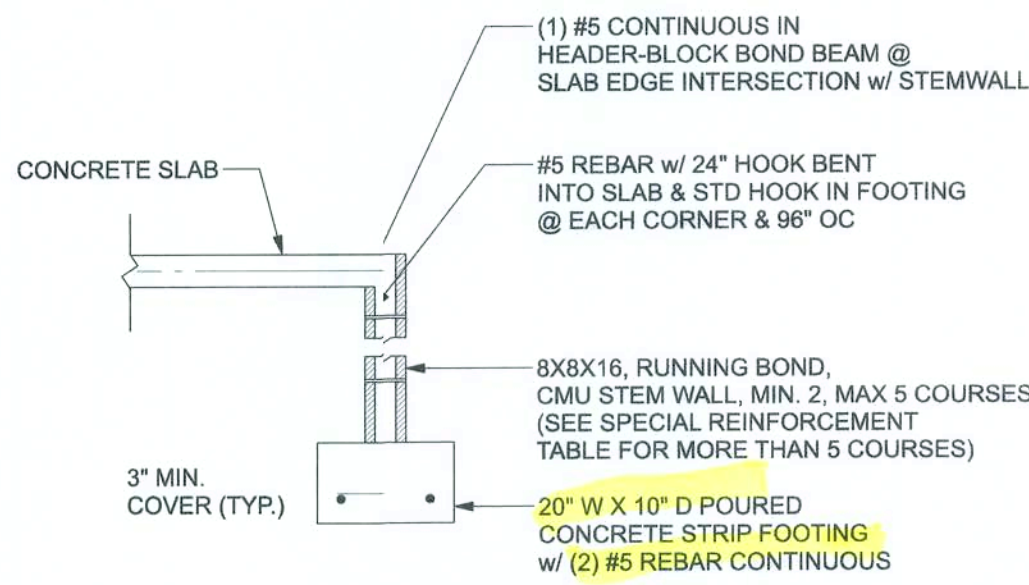
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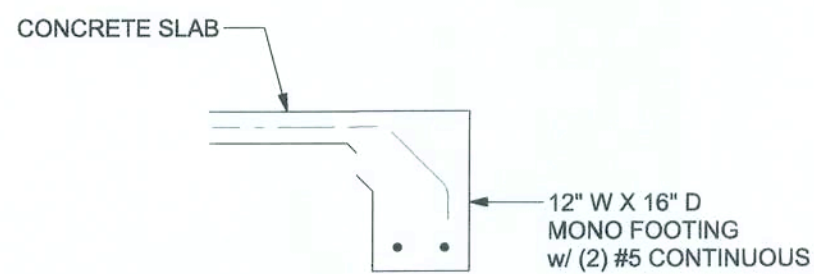
OF 3 SHEETS



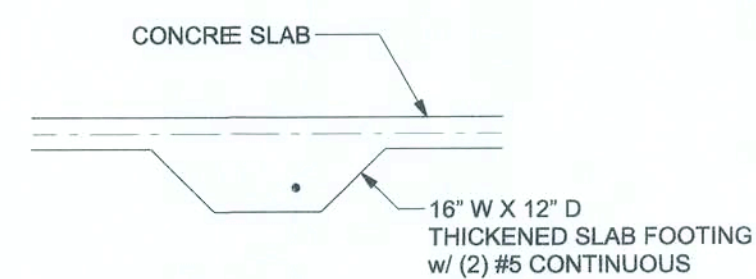


**F1 S-2 STEMWALL FOOTING**  
SCALE: 2" = 1'-0"

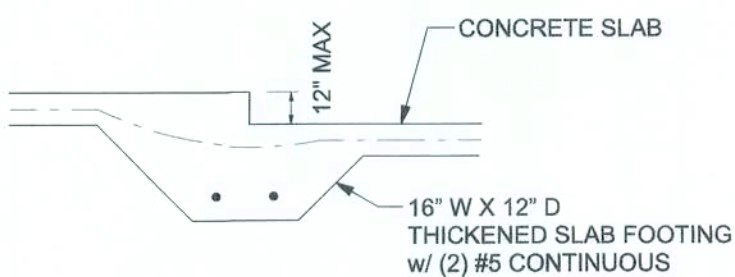
BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL PER FBC 2017-RES. SECTN R403.1.4



**F2 S-2 MONOLITHIC FOOTING @ PORCH**  
SCALE: 2" = 1'-0"



**F3 S-2 INTERIOR BEARING FOOTING**  
SCALE: 1/2" = 1'-0"



**F4 S-2 INTERIOR BEARING STEP FOOTING**  
SCALE: 1/2" = 1'-0"

**TALL STEM WALL TABLE:**  
The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Duowall ladder reinforcement at 16" OC vertically or a horizontal 1nd beam with #5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

**MASONRY NOTE:**  
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 8/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

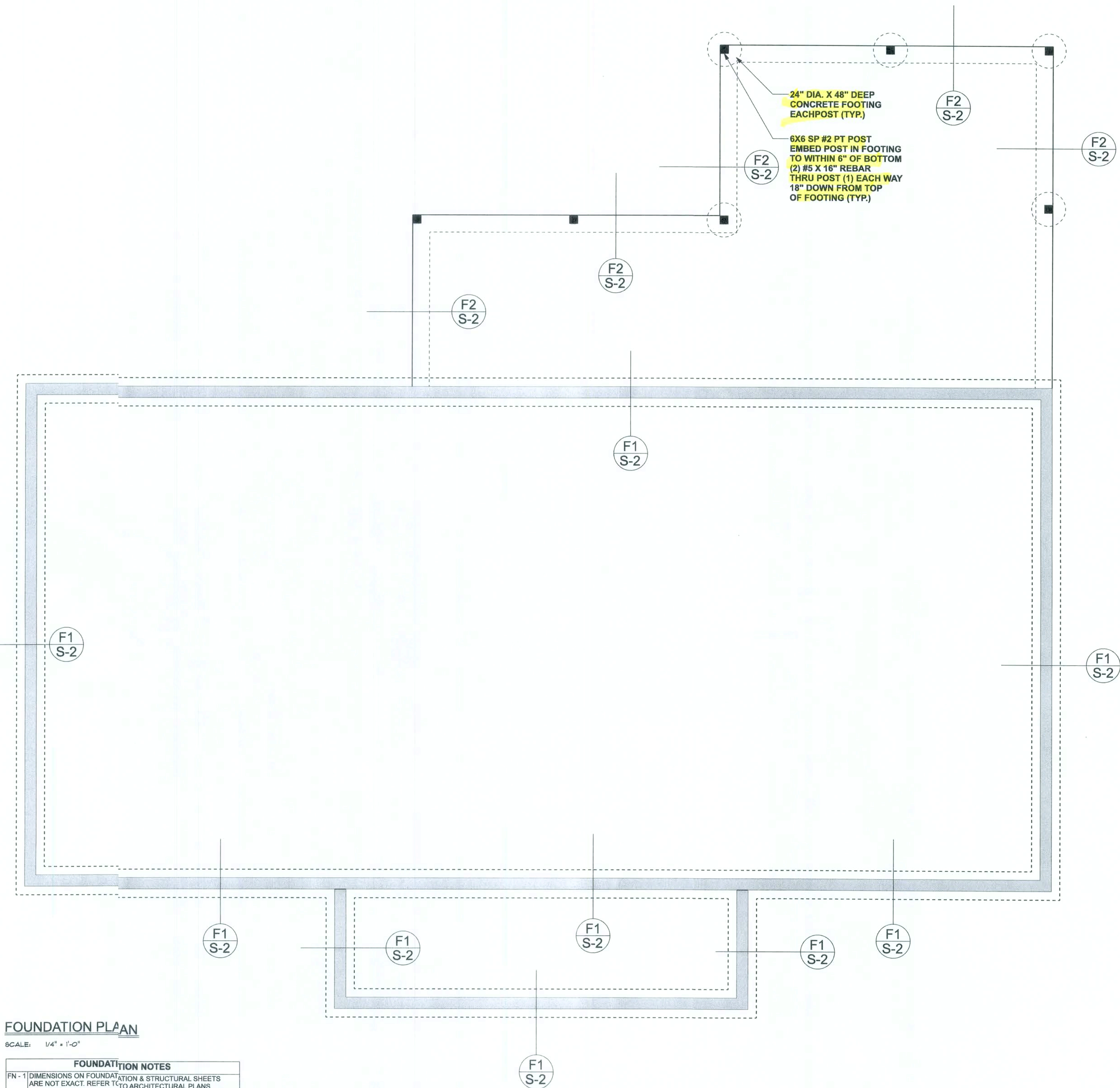
ACI 530.1-02 Section	Specific Requirements
1.4A Compressive strength	8" block bearing walls Pm = 1500 psi
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block.
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 6"x2 1/2"x11 5/8"
2.4 Reinforcing bars, #3 - #11	ASTM 615, Grade 40, Fy = 40 ksi, Lap splices min 40 bar dia. (25" for #5)
2.4F Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G80, 0.60 oz/lb or 304SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wet soils, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

## FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

**FOUNDATION NOTES**

1. DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS. RECESSES IN SLAB, STEP DOWNS, ETC. DISOWAY DESIGN GROUP OR MARK DISOWAY P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
2. CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.
3. THE SLAB SHALL BE 4" CONCRETE SLAB REINFORCED W/ 6X8-1/4 WELDED WIRE MESH PLACED ON CHAIRS @ 12" DEPTH OR FIBER MESH CONCRETE, 5-MIL POLY VAPOR BARRIER W/ 6" LAPS SEALED W/ POLY TAPE OVER TERMINALS & TREATED & COMPACTED FILL.



John Norris Construction L.L.C.

Michael & Patricia Johnson Res.

PROJECT ADDRESS:  
163 SW Midtown Place  
Suite 103  
Lake City, FL 32055

**DIMENSIONS:**  
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway P.E. for resolution. Do not proceed without clarification.

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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the law, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2017) to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.

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**JOB NUMBER:**  
201058

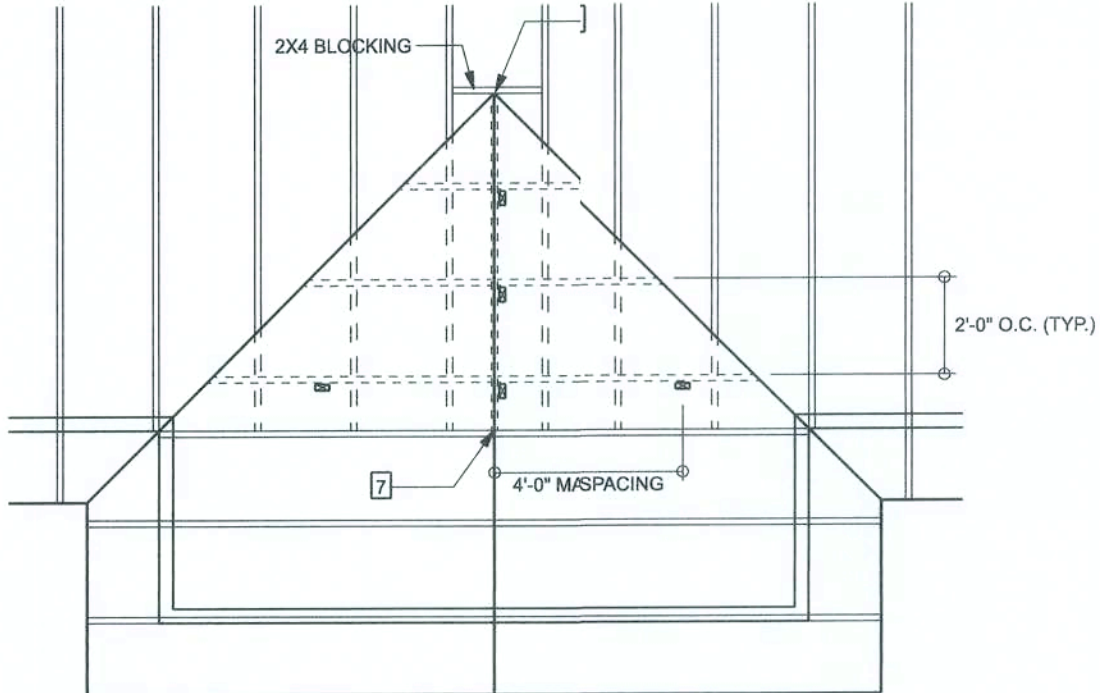
**S-2**

OF 3 SHEETS



LUMBER SIZE & GRADE MINIMUM REQUIREMENTS

RIDGE BOARD	2X6 SYP #2
RAFTER SPANS 20'-0" OR LESS	2X4 SYP #2
PURLINS / LATERAL BRACING	2X4 SPF #2
SLEEPERS	2X (WIDTH OF RAFTER SEAT CUT) SPF #3 OR 2 PARALLEL 2X4 SPF #3
CRIPPLES & BLOCKING	2X4 SPF #2 OR BETTER
TRUSS BELOW	SEE TRUSS DESIGN - SOUTHERN PINE MATERIAL



VALLEY ROOF PLAN MEMBER LEGEND

- TRUSS
- TRUSS UNDER VALLEY FRAMING
- VALLEY RAFTER OR RIDGE
- CRIPPLE

CRIPPLES 4'-0" O.C. FOR 20 psf (TL) AND 10 psf (TD) (TYP. SHINGLE ROOF) MAX

CONNECTION REQUIREMENT NOTES

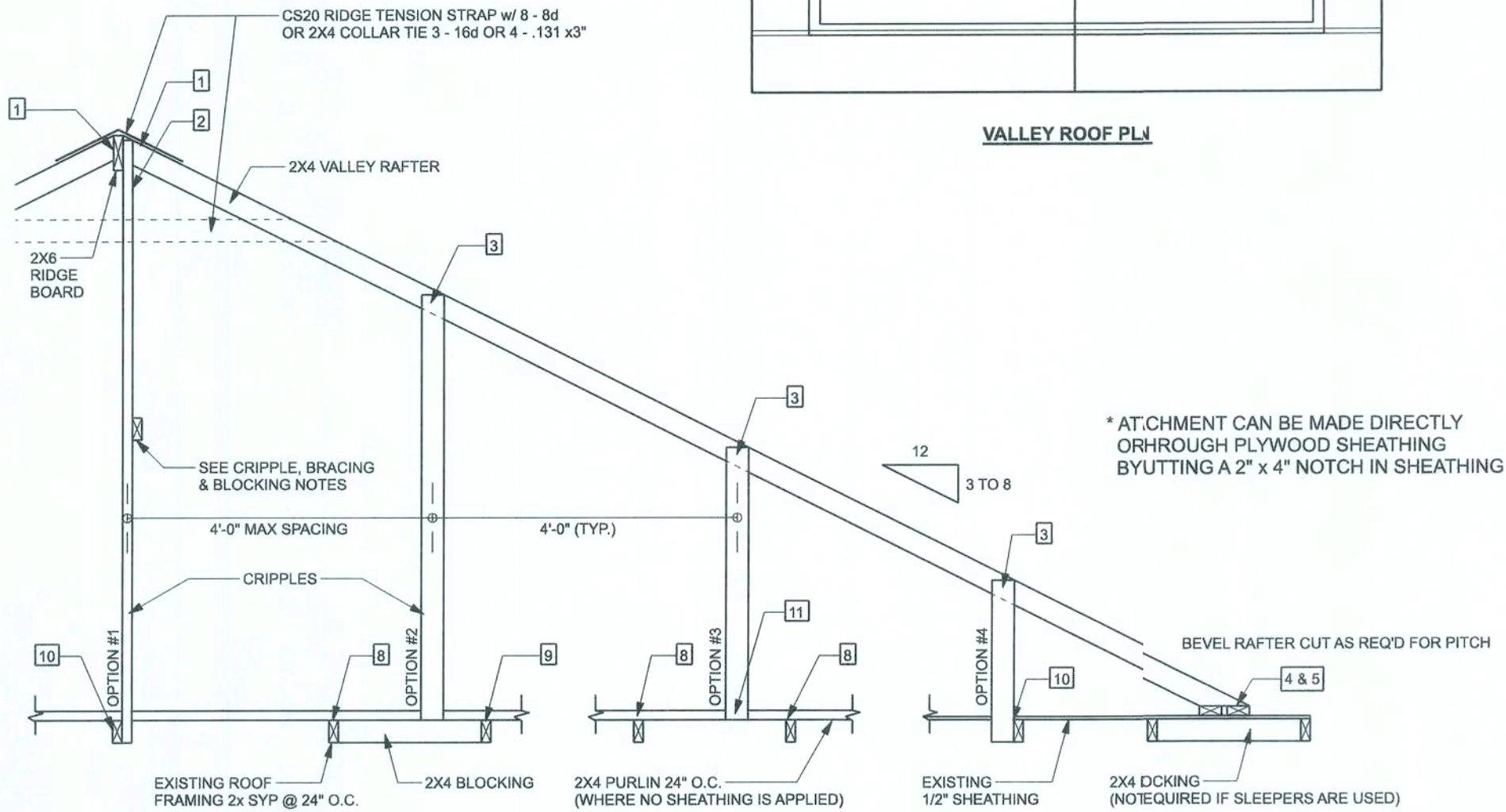
1 2X4 RAFTERS TO RIDGE	3-16d OR 6-131 x 3" TOE NAILS
2 CRIPPLE TO RIDGE	3-16d OR 6-131 x 3" FACE NAILS
3 CRIPPLE TO RAFTERS	3-16d OR 6-131 x 3" FACE NAILS
4 RAFTER TO SLEEPER OR BLOCKING	6-16d OR 12-131 x 3" TOE NAILS
5 SLEEPER TO TRUSS	4-16d OR 8-131 x 3" FACE NAILS EACH TRUSS
6 RIDGE BOARD TO ROOF BLOCK	3-16d OR 6-131 x 3" TOE NAILS
7 RIDGE BOARD TO TRUSS	3-16d OR 6-131 x 3" TOE NAILS
8 PURLIN TO TRUSS (TYP)	3-16d OR 6-131 x 3" TOE NAILS
9 PURLIN TO TRUSS (IF CRIPPLE IS ATTACHED TO PURLIN)	4-16d OR 8-131 x 3" NAILS
10 TRUSS TO BLOCKING	3-16d OR 6-131 x 3" END NAILS
11 CRIPPLE TO TRUSS	3-16d OR 6-131 x 3" FACE NAILS
12 CRIPPLE TO PURLIN	3-16d OR 6-131 x 3" FACE NAILS

GENERAL NOTES

MAXIMUM RAFTER SPANS  
6'-0" FOR 2X4, 9'-0" FOR 2X6 SPF #2 OR SYP #2.  
MAXIMUM ROOF AREA PER SUPPORT  
1602 IN ZONES 2 & 3, 2402 IN ZONE 1, (EXAMPLE: 4'-0" O.C. X 4'-0" SPAN = 1602 OR 2'-0" X 8'-0" SPAN = 1602)  
PURLINS REQUIRED 2'-0" O.C. IF EXISTING SHEATHING IS REMOVED.  
PURLINS SHOULD OVERLAP SHEATHING ONE TRUSS SPACING MINIMUM.  
IN CASES THAT THIS IS IMPRACTICAL, OVERLAP SHEATHING A MINIMUM OF 6" AND NAIL UPWARDS THROUGH SHEATHING INTO PURLIN WITH A MINIMUM OF 3-8d COMMON WIRE NAILS.  
THIS DRAWING APPLIES TO VALLEYS WITH THE FOLLOWING CONDITIONS:  
- SPANS (DISTANCES BETWEEN HEELS) 40'-0" OR LESS  
- MAXIMUM VALLEY HEIGHT: 14'-0" OR LESS  
- MAXIMUM WIND SPEED: 130 MPH  
- MAXIMUM MEAN ROOF HEIGHT: 30 FEET  
- MAXIMUM TOTAL LOADING: 40 psf  
- MEETS IRC 2018/ASCE 7-10 WIND REQUIREMENTS  
- EXPOSURE CATEGORY "C", I = 1.0, Kd = 1.0  
- ENCLOSED BUILDING

CRIPPLE, BRACING & BLOCKING NOTES

2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 5'-0" TO 10'-0" LONG NAILED w/ 2-10d NAILS OR 2X4 "T" OR SCAB BRACE NAILED TO FLAT EDGE OF CRIPPLE WITH 8d NAILS @ 8" O.C. "T" OR SCAB MUST BE 90% OF CRIPPLE LENGTH. CRIPPLES OVER 10'-0" LONG REQUIRE TWO CLBs OR BOTH FACES w/ "T" OR SCAB. USE STRESS GRADED LUMBER & BOX OR COMMON NAILS.  
NARROW EDGE OF CRIPPLE CAN FACE RIDGE OR RAFTER, AS LONG AS THE PROPER NUMBER OF NAILS ARE INSTALLED INTO RIDGE BOARD.  
INSTALL BLOCKING UNDER RAFTER IF SLEEPERS ARE NOT USED.  
INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FALL BETWEEN LOWER TRUSS TOP CHORDS AND LATERAL BRACING IS NOT USED.  
APPLY ALL NAILING IN ACCORDANCE TO NDS-1997 SECTION 12. NAILS ARE COMMON WIRE NAILS UNLESS NOTED OTHERWISE.



SECTION CUT PARALLEL TO VALLEY RAFTER

ROOF OVER FRAMING & BRACING DETAIL

SCALE: N.T.S.

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 USE ONE JACK STD GIRDER SUPPORT PER 2500 LB LOAD
- SN-4 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-5 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACINGS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, BCSI-B3, BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

HEADER LEGEND

- (2) 2X10X8', 1J 1K - HEADER/BEAM CALL-OUT (U.N.O.)
- NUMER OF KING STUDS (FULL LENGTH)
- NUMER OF JACK STUDS (UNDER HEADER)
- SPAN# HEADER
- SIZE # HEADER MATERIAL
- NUMER OF PLIES IN HEADER

THREADED ROD LEGEND

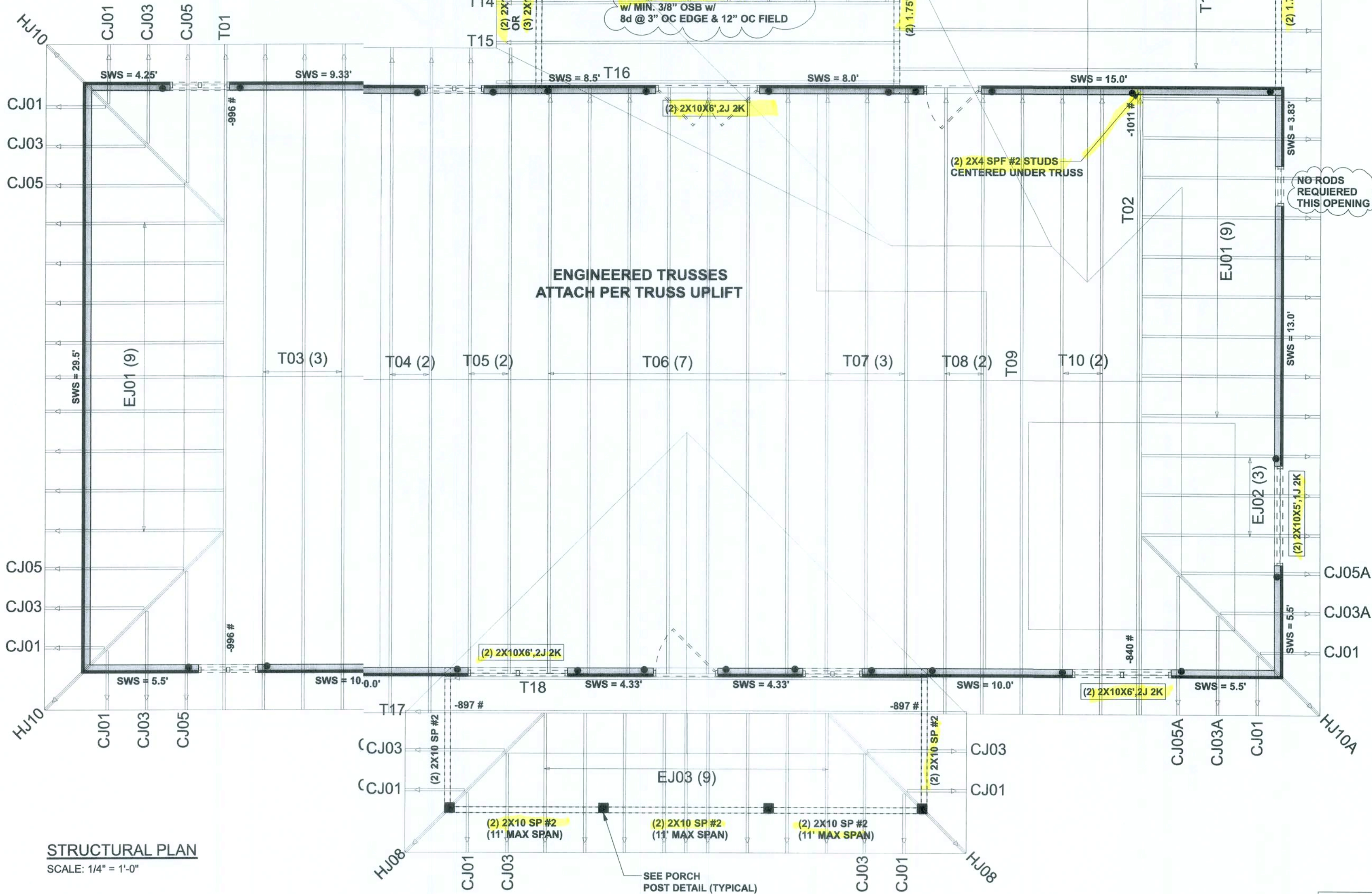
- INDICATES LOCATION OF: 3/8" ANCHOR THREADED ROD

ACTUAL vs REQUIRED SHEARWALL

	TRANSVERSE	LONGITUDINAL
ACTUAL	15549 LBF	25422 LBF
REQUIRED	11796 LBF	9559 LBF

STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"



CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #2490421

John Norris Construction L.L.C.

Michael & Patricia Johnson Res.

PROJECT ADDRESS:  
338 NW Holland Dr.  
Lake City, FL 32055

DIMENSIONS:  
Stated dimensions are approximate. Refer to architectural drawings for exact dimensions. Refer to questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: hereby certify that I have examined this plan and that the applicable portions of the plan, relating to wind engineering comply with the 6th Edition Florida Building Code Residential (2017) to the best of my knowledge.

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S-3  
OF 3 SHEETS