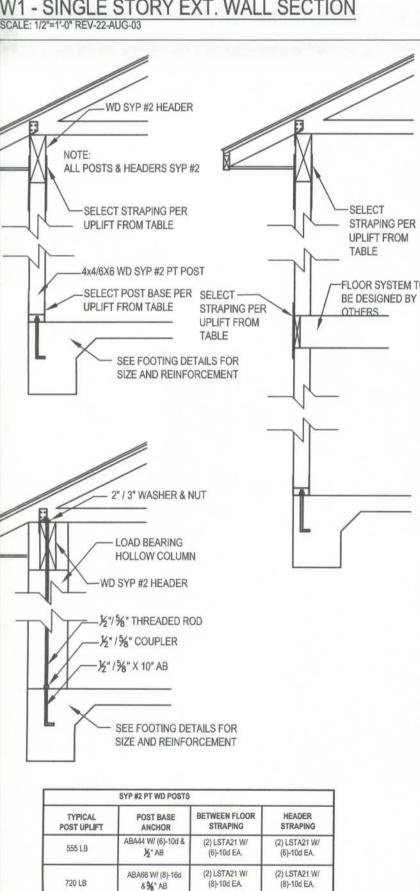




GREATER THAN 10'-0" AND LESS THAN 14'-0" SHALL BE 32" O.0



(2) LSTA21 W/ (16)-10d EA.

ABU66 W/ (12)-16d, (2) LSTA21 W/ (2) LSTA21 W/

(16)-10d EA.

(2) 1/2 BOLTS & 1/4"AB

(2) 1/2 BOLTS & 1/4"AB

光" X 10" AB ATTACHED TO人

THREADED ROD WITH **½**° COUPLER THRU COLUMN & HEADER WITH 2°

WASHER & NUT TOP

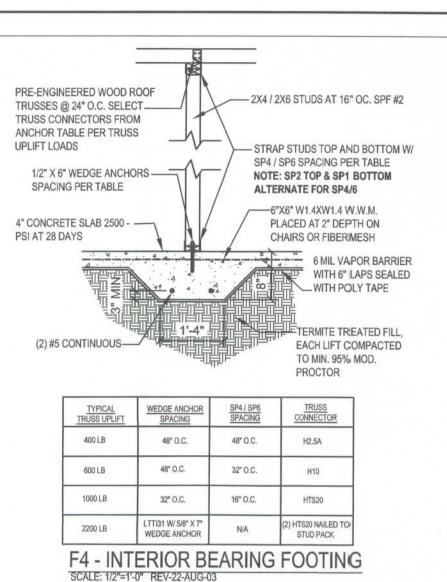
%" X 10" AB ATTACHED TO %

THREADED ROD WITH %" COUPLER THRU COLUMN & HEADER WITH 3"

WASHER & NUT TOP

W12 - PORCH HEADER ANCHORS

(2) LSTA21 W/ (16)-10d EA.



BOND BEAM @ SLAB EDGE

INTERSECTION w/ STEMWALL

(1) #5 STEEL DOWEL w/

24" HOOK BENT INTO

SLAB AND 6" HOOK AT

CORNER AT 96" OC

8x8x16, RUNNING

BOND, CMU STEM WALL,

MIN 2,MAX 5 COURSES

FOOTING @ EA.

APPROXIMATE

1'-8"

F1 - STEM WALL FOUNDATION

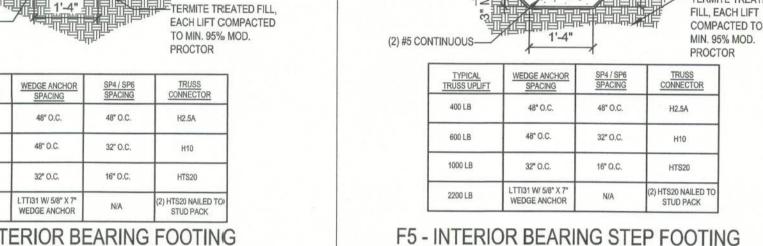
-SEE WALL SECTIONS

-6"x6"W1.4XW1.4| W.W.M.

- PSI @ 28 DA'YS

(2) #5 CONTINUIOUS

PROCTOR



PRE-ENGINEERED WOOD ROOF

TRUSSES @ 24" O.C. SELECT _

TRUSS CONNECTORS FROM

ANCHOR TABLE PER TRUSS

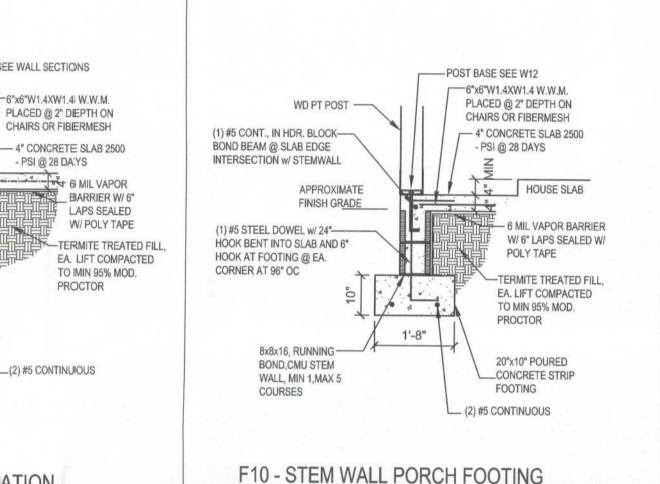
SPACING PER TABLE

4" CONCRETE SLAB 2500 -

1/2" X 6" WEDGE ANCHORS -

UPLIFT LOADS

PSI AT 28 DAYS



-2X4 / 2X6 STUDS AT 16" OC. SPF #2

ALTERNATE FOR SP4/6

STRAP STUDS TOP AND BOTTOM

NOTE: SP2 TOP & SP1 BOTTOM

- 6"X6" W1.4XW1.4 W.W.M.

PLACED AT 2" DEPTH ON

CHAIRS OR FIBERMESH

WITH POLY TAPE

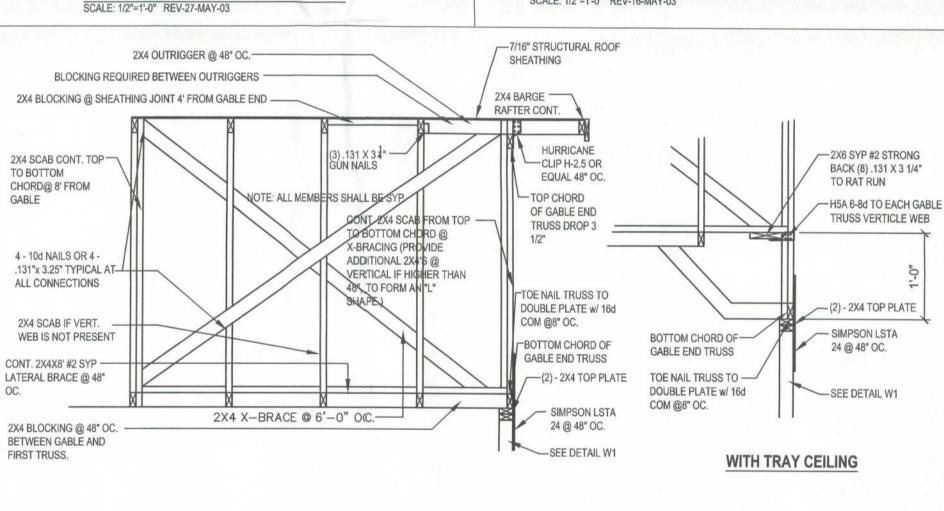
- 6 MIL VAPOR BARRIER

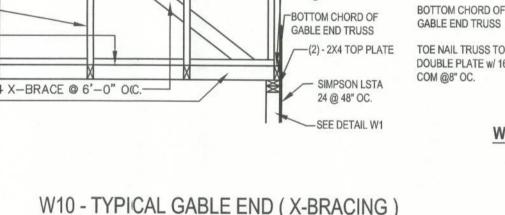
WITH 6" LAPS SEALED

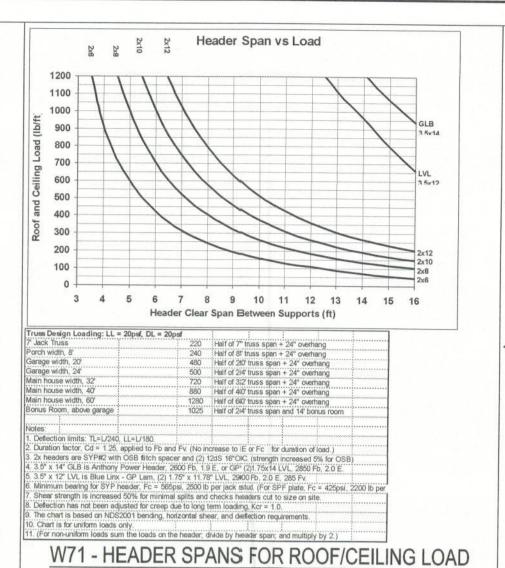
TERMITE TREATED

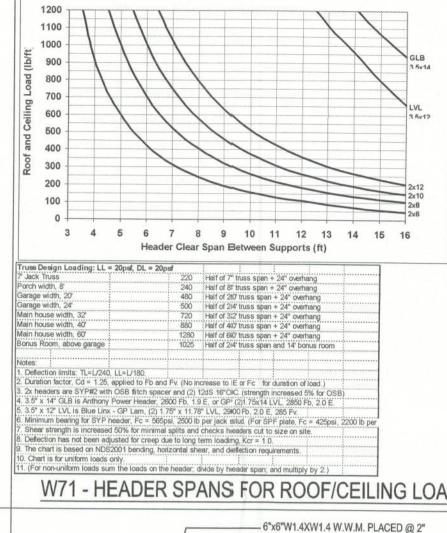
MIN. 95% MOD.

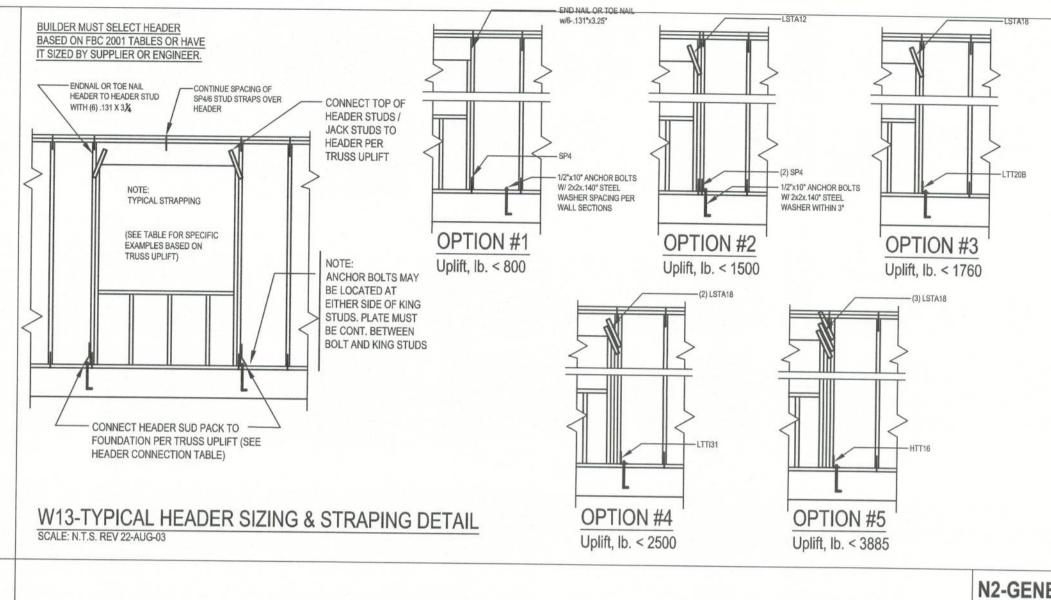
W/ SP4 / SP6 SPACING PER TABLE

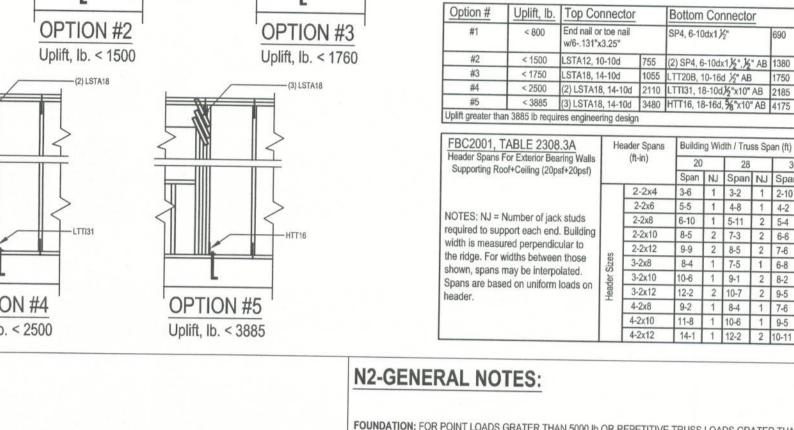


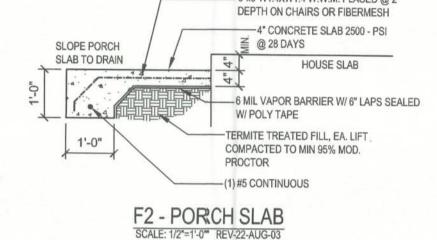


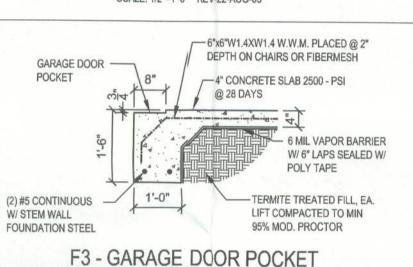


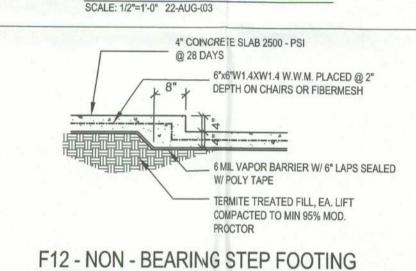


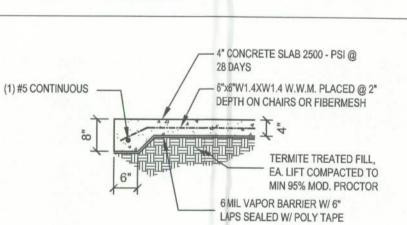














6-10dx11/3"

9-8d or 7-10d

14-8d or 11-10d

%"x 16" AB

Manufacturer and product number are listed for example not endorsement. An equivalent device of the same or other manufacturer can be substituted

for any devices listed in the example tables as long as it meets the required load capacities. Manufacturer's installation instructions must be followed to

duration. Strap uplift may be reduced proportionally to number of nails. See spec sheet for alternate nail sizes (10d=.84*16d, 10dx1½*=.80*10d, 10d=12d=16d sinker). SPF=.86*SYP

hieve rated loads. All connections exposed directly to the weather shall be hot dipped galvanized after fabrication. Loads are increased for wind

14-8d or 11-10d

8-16d Sinkers

18-10dx 11/2

2-% Bolts

18-16d

7-10d

8-10d

SPF SYP Column Anchor To Foundation

3000 lb of reaction. Check the minimum bearing requirements of the truss and top plate (SPF, Fc=425psi=2230lb/pty)

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N5 - TRUSS UPLIFT CONNECTOR TABLE

Uplift Uplift

320 455 H3

245 350 H5A

535 600 H2.5A

620 720 H10 850 990 LTS12

1785 2050 LGT2 3655 4200 MGT

865 1005 CS20 1085 1265 LSTA18-24 1170 1360 SPH4

420 455 SSP

600 825 DSP

1160 | 1350 | LTT19

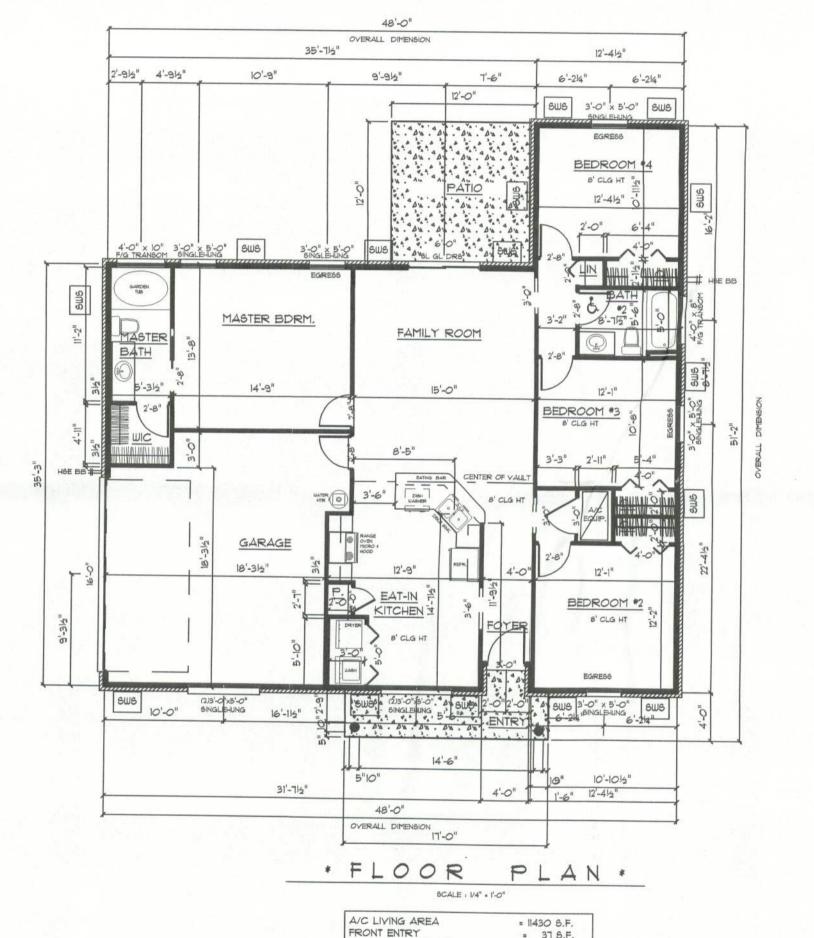
1985 2310 LTTI31

2385 2775 HD2A

3590 4175 HTT16 1975 2300 ABU66

1265 1470 H16, H16-2

SPF SYP Connector



FOUNDATION: FOR POINT LOADS GRATER THAN 5000 Ib OR REPETITIVE TRUSS LOADS GRATER THAN 2000 Ib PER TRUSS PROVIDE A THICKENED SLAB OR PAD FOOTING 1'-0"D X 1 sq ft. FOR EVERY 1000 Ib OF BEARING REINFORCE WITH #5 @ 8" O.C. EACH WAY

oad Bearing Header Sizing Methods (BY BUILDER)

lack Studs and King Studs (BY BUILDER)

leader Uplift Connections (BY BUILDER)

dividing by the length of the header.

5. Use one jack stud for every 3000 lb vertical load.

connection) and stud to foundation (bottom connection).

Use supplier published data or Southern pine span tables.

For engineered lumber beams have suppliers engineer size beam.

Lookup jack studs from FBC 2001, Tables 2308.3 A, B, & C, or 2308.5.

Total king plus jack studs = studs needed to be there if no opening was there.

Calculate the uplift at each end of the header by summing the moments of all truss uplifts and

8. Select header connections from table below or mfg. catalog to connect header to stud (top

Determine header size from FBC 2001, Tables 2308.3 A, B, & C, or 2308.5.

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS SHALL BE F'c = 3000 PSI. WHERE EXCESS WATER IS ADDED TO THE CONCRETE SO THAT ITS SERVICABILITY IS DEGRADED, THE ATTAINMENT OF REQUIRED STRENGTH SHALL NOT RELEASE THE CONTRACTOR FROM PROVIDING SUCH MODIFICATIONS AS MAY BE REQUIRED BY THE ENGINEER TO PROVIDE A SERVICEABLE MEMBER OR SURFACE. ALL CONCRETE SHALL BE VIBRATED. NO REPAIR OR RUBBING OF CONCRETE SURFACES SHALL BE MADE PRIOR TO INSPECTION BY AND APPROVAL OF THE ENGINEER, OWNER OR HIS

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTHS SHALL BE 1/2 INCH TO 2 INCHES IN LENGTH. DOSAGE AMOUNTS SHALL BE FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. SYNTHETIC FIBERS SHALL COMPLY WITH ASTM C 1116. THE MANUFACTURER OR SUPPLIER SHALL PROVIDE CERTIFICATION OF COMPLIANCE WITH ASTM C 1116 WHEN REQUESTED BY THE BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAPS SPLICES 48 * db (30" FOR #5 3ARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-WITH ACI 315-96 UNLESS NOTED OTHERWISE. ALL TENSION DEVELOPMENT LENGTHS SHALL, BE 30

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; NO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

All connectors are Simpson Strongtie, uno. Seelect top and bottom connections from this table or engineer of record for the trusses and did not design the trusses or delegate to the truss designer. SST catalog to meet truss uplift. Use fasteners as specified. (Wind loads are per FBC 2004, Section 1609 for enclosed simple diaphragm buildings with mean roof height less than 60' or the least horizontal dimension; not sited on the upper half of an unobstructed 60' high hill with BUILDER'S RESPONSIBILITY: The builder and owner are responsible for the following, which are specifically not part To Plate To Truss / Rafter >10% slope. of the wind load engineer's scope of work. * Confirm that the foundation design & site conditions meet gravity load requirements (assume 1000 PSF bearing Basic Wind Speed 110 MPH capacity unless visual observation or soils test proves otherwise Wind Exposure Provide materials and construction techniques, which comply with FBC 2004 requirements for the stated wind Wind Importance Factor 1.0 elocity and design pressures. **Building Category** 6-10dx 11/3" * Provide a continuous load path from roof to foundation. If you believe the plan omits a continuous load path Internal pressure Coefficier N/A (Enclosed) connection, call the wind load engineer immediately. 8-8dx 11/2" Verify the truss engineering includes truss design, placement plans, temporary and permanent bracing details, Building not in the high velocity hurricane zone 10-10d or 12-10dx11/2" 10-10d or 12-10dx 1) truss-to-truss connections, and load reactions for all bearing locations. Building not in the wind-borne debris region 2-10dx 11/6" Select uplift connections, walls, columns, and footings based on truss engineering bearing locations and reactions Mean Roof Height 14-10d Sinker 16-16d Sinker < 30 ft ncluding interior bearing walls. Roof Angle 10-45 degrees %" Thid. Rod 22-10d * Size headers for gravity loads; headers sized by the builder for gravity loads will also satisfy wind loads. SPF SYP Strap Connector To One Member Components And Cladding Wind Pressures (FBC Table 1609 B&C To Other Member Zone Effective Wind Area (ft2) DOCUMENT CONTROL and PRIORITY: Structural requirements on S-1 control unless the building code or chitectural sheets have more stringent requirements. Non-structural requirements on architectural sheets control. Specific requirements take precedence over general requirements. Revision control is by the latest signature date and 4 21.8 -23.6 18.5 -20.4 5 21.8 -29.1 18.5 -22.6 is the responsibility of the builder. COPYRIGHTS AND PROPERTY RIGHTS: Mark Disosway, P.E. hereby expressly reserves 3-10d to double plate or 1-10d to single Total Shear Wall Segments ts common law copy/rights and property right in these instruments of service. This document is 6-10d to double plate or 2-10d to single -4"min for 8'-0"H wall 2'-10"min for 10'-0"H wall not to be reproducedl, altered or copied in any form or manner without first the express written permission and conser

Transverse Longitudinal

All exterior walls are type II shear walls

ACTUAL SHEAR WALL length is the total

sheathing and width to height ratio greate

of all wall segments with full height

than 1: 3.5 (plus special shear wall

segments if noted.) REQUIRED SHEA

VALL length is from WFCM-2001, table

17A & 3.17B with table 3.17E

adjustment for type II shear wall (or

Required 19.0'

Actual 69.0"

equivalent calculation)

N4-WIND LOAD DESIGN DATA

2-CAR GARAGE & MECH, AREA =: 400 S.F.

= 1867 S.F.

TOTAL AREA UNDER ROOF

Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clariffication.

N3-WINDLOAD ENGINEER'S SCOPE OF WORK: The wind load engineer is engineer of record for compliance of the

structure to wind load requirements of FBC 2004, Section 1609. If trusses are used, the wind load engineer is not

WINDLOAD ENGINEER: Mark Disosway, PE No.53915

CERTIFICATION: The attached plans and "Windload Engineering", sheet S-1, comply with FBC 2004, Section 1609 wind loads, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location. is drawing is not valid for construction unless raised seal is affixed.

REV-27-Jul-04 WINDLOAD ENGINEERING

"EVERYTHING YOU NEED FOR YOUR BUILDING PERMIT" Mark Disosway P.E.

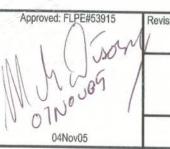
POB 868, Lake City, FL 32056 Phone: (386) 754-5419 Fax: (386) 754-6749 Email: windloadengineer@bellsouth.net

ocation: Lot #95 Emerald Cove S/D, Lake City, Florida

The Victria Model

Builder: Cornerstone Development

Designer: Teena Ruffo



Sheet S-1 of 2 Sheet Windload Engineering Job # 511012