

Columbia County New Building Permit Application

For Office Use Only Application # 44681 Date Received 3/6 By MG Permit # 39491
 Zoning Official LW/CH Date 3-12-20 Flood Zone _____ Land Use Ag Zoning A-3
 FEMA Map # _____ Elevation _____ MFE _____ River _____ Plans Examiner T.C. Date 3-17-20
 Comments _____
☒ NOC ☒ EH ☐ Deed or PA ☒ Site Plan ☐ State Road Info ☒ Well letter ☒ 911 Sheet ☐ Parent Parcel # _____
☐ Dev Permit # _____ ☐ In Floodway ☐ Letter of Auth. from Contractor ☐ F W Comp. letter
☐ Owner Builder Disclosure Statement ☐ Land Owner Affidavit ☐ Ellisville Water ☒ App Fee Paid ☒ Sub VF Form

Septic Permit No. 20-0168 OR City Water ☐ Fax N/A

Applicant (Who will sign/pickup the permit) Kimmy Edgley Phone 386-752-0580

Address 320 SW Elk Hunter Glen, Fort White, FL 32038

Owners Name Wilburn & Sylvia Fender Phone 813-361-2826

911 Address 6046 SE County Rd 252, Lake City, FL 32025

Contractors Name Doug Edgley Phone 386-623-6654

Address 306 SW Main Blvd, Lake City, FL 32025

Contractor Email kimmy@edgleyconstruction.com ***Include to get updates on this job.

Fee Simple Owner Name & Address Wilburn & Sylvia Fender

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Mark Disoisway P.E., 163 SW Midtown Pl, Ste 103, Lake City, FL 32025

Mortgage Lenders Name & Address N/A

Circle the correct power company ☐ FL Power & Light ☒ Clay Elec. ☐ Suwannee Valley Elec. ☐ Duke Energy

Property ID Number 30-4S-18-10513-008 Estimated Construction Cost \$243,000.00

Subdivision Name N/A High Falls Unrecorded Lot _____ Block _____ Unit _____ Phase _____

Driving Directions from a Major Road Hwy 441 S, merge on 41 S, TL on Hwy 252, cross over Price Creek Road then go 1.9 miles TR.

Construction of Residential Home _____ Commercial OR ☒ Residential

Proposed Use/Occupancy _____ Number of Existing Dwellings on Property N/A

Is the Building Fire Sprinkled? _____ If Yes, blueprints included _____ Or Explain _____

Circle Proposed ☐ Culvert Permit or ☐ Culvert Waiver or ☐ D.O.T. Permit or ☒ Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 369' Side 60' Side 201' Rear 244'

Number of Stories 1 Heated Floor Area 1715 Total Floor Area 2763 Acreage 5.12

Zoning Applications applied for (Site & Development Plan, Special Exception, etc.) _____

sho spoke w/ Doug 3.19.20 sent 3.19.20

Columbia County Building Permit Application

CODE: Florida Building Code 2017 and the 2014 National Electrical Code.

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

TIME LIMITATIONS OF APPLICATION : An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless pursued in good faith or a permit has been issued.

TIME LIMITATIONS OF PERMITS: Every permit issued shall become invalid unless the work authorized by such permit is commenced within 180 days after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of 180 days after the time work is commenced. A valid permit receives an approved inspection every 180 days. Work shall be considered not suspended, abandoned or invalid when the permit has received an approved inspection within 180 days of the previous approved inspection.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment: According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO CONTRACTOR AND AGENT: **YOU ARE HEREBY NOTIFIED** as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: I CERTIFY THAT ALL THE FOREGOING INFORMATION IS ACCURATE AND THAT ALL WORK WILL BE DONE IN COMPLIANCE WITH ALL APPLICABLE LAWS REGULATING CONSTRUCTION AND ZONING.

NOTICE TO OWNER: There are some properties that may have deed restrictions recorded upon them. These restrictions may limit or prohibit the work applied for in your building permit. You must verify if your property is encumbered by any restrictions or face possible litigation and or fines.

Sylvia Fender
Print Owners Name

Sylvia Fender
Owners Signature

****Property owners must sign here
before any permit will be issued.**

****If this is an Owner Builder Permit Application then, ONLY the owner can sign the building permit when it is issued.**

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit including all application and permit time limitations.

[Signature]
Contractor's Signature

Contractor's License Number CRC1330689
Columbia County
Competency Card Number 44

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 2 day of March 2020

Personally known ☒ or Produced Identification _____

[Signature]
State of Florida Notary Signature (For the Contractor)

SEAL:



ALICE BURKE PEELER
Commission # GG 122000
Expires September 15, 2021
Bonded Thru Budget Notary Service

SUBCONTRACTOR VERIFICATION

APPLICATION/PERMIT # 44681 JOB NAME William + Sylvia Fender

THIS FORM MUST BE SUBMITTED BEFORE A PERMIT WILL BE ISSUED

Columbia County issues combination permits. One permit will cover all trades doing work at the permitted site. It is **REQUIRED** that we have records of the subcontractors who actually did the trade specific work under the general contractors permit.

NOTE: It shall be the responsibility of the general contractor to make sure that all of the subcontractors are licensed with the Columbia County Building Department.

Use website to confirm licenses: <http://www.columbiacountyfla.com/PermitSearch/ContractorSearch.aspx>

NOTE: If this should change prior to completion of the project, it is your responsibility to have a corrected form submitted to our office, before that work has begun.

Violations will result in stop work orders and/or fines.

ELECTRICAL <input checked="" type="checkbox"/> CC# <u>309</u>	Print Name <u>Matthew H. Burns</u> Company Name: <u>Matt Burns Electric Inc.</u> License #: <u>EC13006531</u> Phone #: <u>386-935-0444</u>	Signature <u>[Signature]</u>	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
MECHANICAL/A/C <input checked="" type="checkbox"/> CC# <u>568</u>	Print Name <u>David E. Hall</u> Company Name: <u>David Hall's A/C & Heating Svc. Inc.</u> License #: <u>CAC057424</u> Phone #: <u>386-755-9792</u>	Signature <u>[Signature]</u>	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
PLUMBING/GAS <input checked="" type="checkbox"/> CC# <u>298</u>	Print Name <u>Don Bills</u> Company Name: <u>Hometown Plumbing Services LLC</u> License #: <u>CFC1428890</u> Phone #: <u>386-754-6140</u>	Signature <u>[Signature]</u>	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input checked="" type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
ROOFING <input checked="" type="checkbox"/> CC# <u>534</u>	Print Name <u>Darin L. Summerlin</u> Company Name: <u>Summerlin Roofing Inc.</u> License #: <u>CCC1326192</u> Phone #: <u>386-288-5426</u>	Signature <u>[Signature]</u>	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
SHEET METAL <input type="checkbox"/> CC# _____	Print Name _____ Company Name: _____ License #: _____ Phone #: _____	Signature _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
FIRE SYSTEM/SPRINKLER <input type="checkbox"/> CC# _____	Print Name _____ Company Name: _____ License #: _____ Phone #: _____	Signature _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
SOLAR <input type="checkbox"/> CC# _____	Print Name _____ Company Name: _____ License #: _____ Phone #: _____	Signature _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
STATE SPECIALTY <input type="checkbox"/> CC# _____	Print Name _____ Company Name: _____ License #: _____ Phone #: _____	Signature _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE

When recorded, mail to:

Name: Wilburn Donald Fender

Address: 6050 SE CR 252

City/State/Zip Code: Lake City, FL 32025

Inst: 202012003490 Date: 02/12/2020 Time: 4:02PM
Page 1 of 3 B: 1405 P: 1274, P. DeWitt Cason, Clerk of Court
Columbia, County, By: BS
Deputy Clerk Doc Stamp-Deed: 0.70

30-45-18-10513-008

SPACE ABOVE THIS LINE FOR RECORDER'S USE

QUITCLAIM DEED

KNOW ALL MEN BY THESE PRESENTS:

That I(we), Richard Fender

the undersigned releasor(s), for the consideration of Ten Dollars (\$10.00), and other valuable considerations, by these presents, do hereby release, remise and forever quitclaim unto Wilburn Donald Fender
, Sylvia Fender

all rights, title and interest in that certain real property situated in the County of Columbia, State of Florida, and legally described as follows:

SEE Exhibit "A"

IN WITNESS WHEREOF, I(we) have hereunto set my(our) hand(s) and seal(s) this 12 day of Feb, 2020.

Richard Fender
Printed Name of Releasor

Richard Fender
Signature of Releasor

Printed Name of Co-Releasor

Signature of Co-Releasor

Karin Killee
Signature of Witness No. 1

Wanda J. Strickland
Signature of Witness No. 2

Karin Killee
Printed Name of Witness No. 1

Wanda J. Strickland
Printed Name of Witness No. 2

1468 SW Main Blvd. Ste 105
Address
Lake City, FL 32025
City/State/Zip Code

1468 SW. Main Blvd Ste 105
Address
Lake City, FL 32025
City/State/Zip Code

Acknowledgment

State of Florida)
County of Columbia) ss.

The foregoing instrument was acknowledged before me, the undersigned Notary Public, this 12 day
of February, 2020, by Richard Fender
, known to me to be the indi-
vidual(s) who executed the foregoing instrument and acknowledged the same to be his(her)(their) free act and deed.

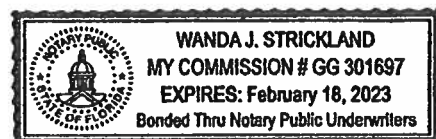
My Commission Expires: 2-18-2023

Wanda J. Strickland
Notary Public

If acknowledged in the State of Florida, complete the section
below:

(check one) [] Personally Known. [X] Produced Identification.

Type of Identification produced: FLDL-FS30-744-01-299-0



TOWNSHIP 4 SOUTH - RANGE 18 EAST

SECTION 30: A PART OF THE S ½ OF SECTION 30, TOWNSHIP 4 SOUTH, RANGE 18 EAST, COLUMBIA COUNTY, FLORIDA, BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS: COMMENCE AT THE SE CORNER OF THE SW ¼ OF SAID SECTION 30, AND RUN THENCE N 33°51'50" W, 426.21 FEET; THENCE N 53°11'39" E, 682.38 FEET TO THE POINT OF BEGINNING; THENCE CONTINUE N 53°11'39" E, 717.20 FEET TO A POINT ON THE SOUTHWESTERLY RIGHT-OF-WAY LINE OF COUNTY ROAD #252 (OLD LULU ROAD); THENCE N 43°49'59" W, ALONG SAID RIGHT-OF-WAY LINE, 313.22 FEET; THENCE S 53°46'55" W, 672.32 FEET; THENCE S 35°37'03" E, 317.83 FEET TO THE POINT OF BEGINNING.

SUBJECT TO: A 30 FOOT EASEMENT FOR INGRESS, EGRESS & UTILITIES LYING 30 FEET TO THE RIGHT OF THE FOLLOWING DESCRIBED LINE: COMMENCE AT THE SE CORNER OF THE SW ¼ OF SAID SECTION 30, AND RUN THENCE N 33°51'50" W, 751.30 FEET; THENCE N 53°46'55" E, 642.32 FEET TO THE POINT OF BEGINNING OF SAID LINE; THENCE CONTINUE N 53°46'55" E, 702.32 FEET TO A POINT ON THE SOUTHWESTERLY RIGHT-OF-WAY LINE OF COUNTY ROAD #252 (OLD LULU ROAD), SAID POINT BEING THE POINT OF TERMINATION OF SAID LINE.

THE ABOVE DESCRIBED PROPERTY IS NOT THE HOMESTEAD OF THE GRANTOR.

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

Exhibit "A"

NOTICE OF COMMENCEMENT

Tax Parcel Identification Number:

30-4S-18-10513-008

Clerk's Office Stamp

Inst: 202012005473 Date: 03/06/2020 Time: 12:33PM
Page 1 of 1 B: 1407 P: 964, P. DeWitt Cason, Clerk of Court Colur
County, By: BD
Deputy Clerk

THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes, the following information is provided in this NOTICE OF COMMENCEMENT.

1. Description of property (legal description): Comm SE cor of SW 1/4, Rn n 33 Deg W, 425.21', N 53 Deg E 682.38 ft to eury rly of CR-252, N 43 Deg W along RAW 313.22 ft, S 53 Deg W 672.32 ft, n 25 deg e 317.83 ft to f
a) Street (job) Address: 6046 SE County Rd 252, Lake City, FL 32025
2. General description of improvements: Residential Home
3. Owner Information or Lessee information if the Lessee contracted for the improvements:
a) Name and address: Wilburn & Sylvia Fender, 6050 SE County Rd 252, Lake City, FL 32025
b) Name and address of fee simple titleholder (if other than owner): N/A
c) Interest in property: Owner
4. Contractor Information
a) Name and address: Edgley Construction, 306 SW Main Blvd, Lake City, FL 32025
b) Telephone No.: 386-752-0580
5. Surety Information (if applicable, a copy of the payment bond is attached):
a) Name and address: N/A
b) Amount of Bond: _____
c) Telephone No.: _____
6. Lender
a) Name and address: N/A
b) Phone No.: _____
7. Person within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes:
a) Name and address: Doug Edgley 320 SW Elk Hunter Glen, Fort White FL 32038
b) Telephone No.: 386-623-6854
8. In addition to himself or herself, Owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes:
a) Name: Doug Edgley OF Edgley Construction
b) Telephone No.: 386-752-0580
9. Expiration date of Notice of Commencement (the expiration date will be 1 year from the date of recording unless a different date is specified): _____

WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY; A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

STATE OF FLORIDA
COUNTY OF COLUMBIA

10. Sylvia Fender
Signature of Owner or Lessee, or Owner's or Lessee's Authorized Office/Director/Partner/Manager
Sylvia Fender Owner
Printed Name and Signatory's Title/Office

The foregoing instrument was acknowledged before me, a Florida Notary, this 5 day of March, 2020, by:

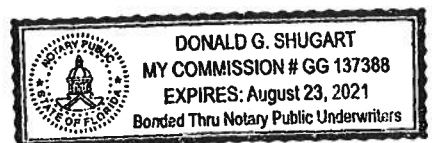
Sylvia Fender as owner for Sylvia Fender AND Wilburn Fender
(Name of Person) personally appeared (Type of Authority) (name of party on behalf of whom instrument was executed)

Personally Known _____ OR Produced Identification ☒ Type Florida Drivers license

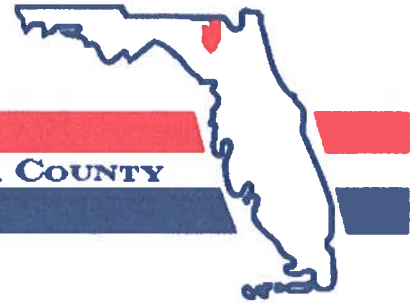
Notary Signature

[Signature]

Notary Stamp or Seal:



District No. 1 - Ronald Williams
District No. 2 - Rocky Ford
District No. 3 - Bucky Nash
District No. 4 - Toby Witt
District No. 5 - Tim Murphy



BOARD OF COUNTY COMMISSIONERS • COLUMBIA COUNTY

Address Assignment and Maintenance Document

To maintain the county wide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for addressing and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Services Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County

Date/Time Issued: **3/3/2020 8:41:24 PM**
Address: **6046 SE COUNTY ROAD 252**
City: **LAKE CITY**
State: **FL**
Zip Code **32025**

Parcel ID **10513-008**

REMARKS: Address for proposed structure on parcel.

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION AND ACCESS INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION AND/OR ACCESS INFORMATION BE FOUND TO BE IN ERROR OR CHANGED, THIS ADDRESS IS SUBJECT TO CHANGE.

Address Issued By: **Signed:/ Matt Crews**

Columbia County GIS/911 Addressing Coordinator

**COLUMBIA COUNTY
911 ADDRESSING / GIS DEPARTMENT**

263 NW Lake City Ave., Lake City, FL 32055 Telephone: (386) 758-1125
Email: gis@columbiacountyfla.com

-009 Columbia County 2020 R
 PRINTED 2/10/2020 14:17 CARD 001 of 001
 APPR 12/27/2016 JS BY JEFF

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#-----BLDG TRAV
# BAS2016=W56 S14 E56 N14$
#
#

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This ⁰⁵ permit replaced a
prior permit on 5 Ac,
making each 5 Ac a
legal lot of record. This is
Back 5 Ac piece - and

[illegible]

LAND	DESC	ZONE	ROAD	{UD1	{UD3	FRONT	DEPTH	FIELD	CR:	UNITS	UT	PRICE	ADJ	UT	PR	LAND
AE	CODE	TOPO	UTIL	{UD2	{UD4	BACK	DT	ADJUSTMENTS								VALUE
Y	000200	MBL	HM	A-1				1.00	1.00	1.00	1.00	5.120	AC	3271.950	3271.95	16,752
Y	000945	WELL/SEPT		0002	0003			1.00	1.00	1.00	1.00	1.000	UT	3250.000	3250.00	3,250

PAT LYNCH
LYNCH DRILLING CORP
P O Box 934
Branford, FL 32008
(386)935-1076

DATE 3-5-2020

CUSTOMER Wilburn & Sylvia Fender

LOCATION 6046 SE County Road 252
Lake City, FL 32025

WE WILL CONSTRUCT A 4" WATER WELL COMPLETE WITH 4" WATER WELL STEEL CASING, 1 1/2 SUBMERSIBLE PUMP WITH 1 1/4" DROP PIPE, AND AN 85 GALLON CAPTIVE AIR TANK (21.9 GALLON DRAWDOWN).

WELL WILL BE COMPLETE AT THE WELL SITE, WE DO NOT INCLUDE ELECTRICAL NOR PLUMBING CONNECTIONS FROM THE WELL TO THE HOME AND/OR POWER POLE.

ANY VARIATIONS OF THE ABOVE ARE SUBJECT TO APPROVAL FROM THE CUSTOMER AND/OR CONTRACTOR PRIOR TO COMMENSMENT OF THE INDIVIDUAL JOB.

THANK YOU



NOT RESPONSIBLE FOR THE QUALITY OF WATER

PART II - SITEPLAN

Notes:



STATE OF FLORIDA
DEPARTMENT OF HEALTH
ONSITE SEWAGE TREATMENT AND DISPOSAL
SYSTEM
APPLICATION FOR CONSTRUCTION PERMIT

PERMIT NO. 20-8168
DATE PAID: 3/3/20
FEE PAID: 378.00
RECEIPT #: 147184e7

APPLICATION FOR:

☒ New System ☐ Existing System ☐ Holding Tank ☐ Innovative
☐ Repair ☐ Abandonment ☐ Temporary ☐

APPLICANT: Wilburn & Sylvia FenderAGENT: ROCKY FORD, A & B CONSTRUCTIONTELEPHONE: 386-497-2311MAILING ADDRESS: 546 SW Dortch Street, FT. WHITE, FL, 32038

TO BE COMPLETED BY APPLICANT OR APPLICANT'S AUTHORIZED AGENT. SYSTEMS MUST BE CONSTRUCTED BY A PERSON LICENSED PURSUANT TO 489.105(3)(m) OR 489.552, FLORIDA STATUTES. IT IS THE APPLICANT'S RESPONSIBILITY TO PROVIDE DOCUMENTATION OF THE DATE THE LOT WAS CREATED OR PLATTED (MM/DD/YY) IF REQUESTING CONSIDERATION OF STATUTORY GRANDFATHER PROVISIONS.

PROPERTY INFORMATION

LOT: NA BLOCK: NA SUB: NA PLATTED: _____PROPERTY ID #: 30-4S-18-10513-009 ZONING: _____ I/M OR EQUIVALENT: ☒ Y / ☐ NPROPERTY SIZE: 5.12 ACRES WATER SUPPLY: ☒ PRIVATE PUBLIC ☐ ≤ 2000 GPD ☐ > 2000 GPDIS SEWER AVAILABLE AS PER 381.0065, FS? ☒ Y / ☐ N DISTANCE TO SEWER: NA FTPROPERTY ADDRESS: 6050 County Road 252, Lake City, FL

DIRECTIONS TO PROPERTY: Head W on NE Franklin St, TL at the 3rd
CROSS ST. onto N. Marion Ave, TL onto US-90E, slight
Rt onto FL-100E, TR onto SE County Rd 245, TL onto
CR 252 E.

BUILDING INFORMATION

☒ RESIDENTIAL ☐ COMMERCIAL

Unit No	Type of Establishment	No. of Bedrooms	Building Area Sqft	Commercial/Institutional System Design Table 1, Chapter 64E-6, FAC
1	SF Residential	3	1715	
2				
3				

☐ Floor/Equipment Drains ☐ Other (Specify) _____SIGNATURE: William A. Bishop Jr. DATE: 2/28/2020

DH 4015, 08/09 (Obsoletes previous editions which may not be used)
Incorporated 64E-6.001, FAC



fend u

**COLUMBIA COUNTY BUILDING DEPARTMENT
RESIDENTIAL CHECK LIST**

MINIMUM PLAN REQUIREMENTS: FLORIDA BUILDING CODE RESIDENTIAL 2017 EFFECTIVE 1 JANUARY 2018
AND THE NATIONAL ELECTRICAL 2014 EFFECTIVE 1 JANUARY 2018

ALL REQUIREMENTS ARE SUBJECT TO CHANGE

ALL BUILDING PLANS MUST INDICATE COMPLIANCE WITH THE CURRENT FLORIDA BUILDING CODES RESIDENTIAL AND THE NATIONAL ELECTRICAL CODE. ALL PLANS OR DRAWINGS SHALL PROVIDE CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS, FBC 1609.3.1 THRU 1609.3.3.

FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEEDS ARE PER FLORIDA BUILDING CODE FIGURE 1609-A THROUGH 1609-C ULTIMATE DESIGN WIND SPEEDS FOR RISK CATEGORY AND BUILDINGS AND OTHER STRUCTURES

Revised 7/1/18

Website: <http://www.columbiacountyfla.com/BuildingandZoning.asp>

Items to Include-
Each Box shall be
Circled as
Applicable

GENERAL REQUIREMENTS:

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

Select From Drop down

1	Two (2) complete sets of plans containing the following:	<input checked="" type="checkbox"/>		
2	All drawings must be clear, concise, drawn to scale, details that are not used shall be marked void	<input checked="" type="checkbox"/>		
3	Condition space (Sq. Ft.) <u>1715</u> Total (Sq. Ft.) under roof <u>2763</u>	Yes	No	NA

Designers name and signature shall be on all documents and a licensed architect or engineer, signature and official embossed seal shall be affixed to the plans and documents as per the FLORIDA BUILDING CODES RESIDENTIAL 107.1.

Site Plan information including:

4	Dimensions of lot or parcel of land	- <input checked="" type="checkbox"/>		
5	Dimensions of all building set backs	- <input checked="" type="checkbox"/>		
6	Location of all other structures (include square footage of structures) on parcel, existing or proposed well and septic tank and all utility easements.	- <input checked="" type="checkbox"/>		
7	Provide a full legal description of property. <u>Warranty Deed</u>	- <input checked="" type="checkbox"/>		

Wind-load Engineering Summary, calculations and any details are required.

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Items to Include- Each Box shall be Circled as Applicable		
8	Plans or specifications must show compliance with FBCR Chapter 3	Yes	No	NA
Select From Drop down				
9	Basic wind speed (3-second gust), miles per hour <u>110 MPH</u>	- <input checked="" type="checkbox"/>		
10	(Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated)	- <input checked="" type="checkbox"/>		
11	Wind importance factor and nature of occupancy	- <input checked="" type="checkbox"/>		
12	The applicable internal pressure coefficient, Components and Cladding	- <input checked="" type="checkbox"/>		
13	The design wind pressure in terms of psf (kN/m ²), to be used for the design of exterior component, cladding materials not specifiically designed by the registered design professional.	- <input checked="" type="checkbox"/>		

Elevations Drawing including:

14	All side views of the structure	- <input checked="" type="checkbox"/>		
15	Roof pitch	- <input checked="" type="checkbox"/>		
16	Overhang dimensions and detail with attic ventilation	- <input checked="" type="checkbox"/>		
17	Location, size and height above roof of chimneys	- <input checked="" type="checkbox"/>		<input checked="" type="checkbox"/>
18	Location and size of skylights with Florida Product Approval	- <input checked="" type="checkbox"/>		<input checked="" type="checkbox"/>
19	Number of stories	- <input checked="" type="checkbox"/>		
20	Building height from the established grade to the roofs highest peak	- <input checked="" type="checkbox"/>		

Floor Plan Including:

21	Dimensioned area plan showing rooms, attached garage, breeze ways, covered porches, deck, balconies	- ✓		
22	Raised floor surfaces located more than 30 inches above the floor or grade	- ✓		
23	All exterior and interior shear walls indicated	- ✓		
24	Shear wall opening shown (Windows, Doors and Garage doors)	- ✓		
25	Show compliance with Section FBCR 310 Emergency escape and rescue opening shown in each bedroom (net clear opening shown) and Show compliance with Section FBC 1405.13.2 where the opening of an operable window is located more than 72 inches above the finished grade or surface below, the lowest part of the clear opening of the window shall be a minimum of 24 inches above the finished floor of the room in which the window is located. Glazing between the floor and 24 inches shall be fixed or have openings through which a 4-inch-diameter sphere cannot pass.	- ✓		
26	Safety glazing of glass where needed	- ✓		
27	Fireplaces types (gas appliance) (vented or non-vented) or wood burning with Hearth (see chapter 10 and chapter 24 of FBCR)	-		✓
28	Show stairs with dimensions (width, tread and riser and total run) details of guardrails, Handrails	-		✓
29	Identify accessibility of bathroom (see FBCR SECTION 320)	- ✓		

All materials placed within opening or onto/into exterior walls, soffits or roofs shall have Florida product approval number and mfg. installation information submitted with the plans (see Florida product approval form)

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Items to Include- Each Box shall be Circled as Applicable
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FBCR 403: Foundation Plans

		Select From Drop down		
30	Location of all load-bearing walls footings indicated as standard, monolithic, dimensions, size and type of reinforcing.	- ✓		
31	All posts and/or column footing including size and reinforcing	- ✓		
32	Any special support required by soil analysis such as piling.	-		✓
33	Assumed load-bearing value of soil <u>1500</u> Pound Per Square Foot	- ✓		
34	Location of horizontal and vertical steel, for foundation or walls (include # size and type) For structures with foundation which establish new electrical utility companies service connection a Concrete Encased Electrode will be required within the foundation to serve as an grounding electrode system. Per the National Electrical Code article 250.52.3	- ✓		

FBCR 506: CONCRETE SLAB ON GRADE

35	Show Vapor retarder (6mil. Polyethylene with joints lap 6 inches and sealed)	- ✓		
36	Show control joints, synthetic fiber reinforcement or welded fire fabric reinforcement and Supports	- ✓		

FBCR 318: PROTECTION AGAINST TERMITES

37	Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or Submit other approved termite protection methods. Protection shall be provided by registered termiticides	- ✓		
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(Treat Soil below Slab)

FBCR 606: Masonry Walls and Stem walls (load bearing & shear Walls)

38	Show all materials making up walls, wall height, and Block size, mortar type	- ✓		
39	Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement	-		✓

Wood Frame

Metal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof. Engineer or Architect

Floor Framing System: First and/or second story

40	Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer	-	✓
41	Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, stem walls and/or piers	-	✓
42	Girder type, size and spacing to load bearing walls, stem wall and/or piers	-	✓
43	Attachment of joist to girder	-	✓
44	Wind load requirements where applicable	-	✓
45	Show required under-floor crawl space	-	✓
46	Show required amount of ventilation opening for under-floor spaces	-	✓
47	Show required covering of ventilation opening	-	✓
48	Show the required access opening to access to under-floor spaces	-	✓
49	Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges & intermediate of the areas structural panel sheathing	-	✓
50	Show Draftstopping, Fire caulking and Fire blocking	-	✓
51	Show fireproofing requirements for garages attached to living spaces, per FBCR section 302.6	-	✓
52	Provide live and dead load rating of floor framing systems (psf).	-	✓

FBCR CHAPTER 6 WOOD WALL FRAMING CONSTRUCTION

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Items to Include- Each Box shall be Circled as Applicable	
Select from Drop down			
53	Stud type, grade, size, wall height and oc spacing for all load bearing or shear walls	-	✓
54	Fastener schedule for structural members per table FBC-R602.3.2 are to be shown	-	✓
55	Show wood structural panel's sheathing attachment to studs, joist, trusses, rafters and structural members, showing fastener schedule attachment on the edges & intermediate of the areas structural panel sheathing	-	✓
56	Show all required connectors with a max uplift rating and required number of connectors and oc spacing for continuous connection of structural walls to foundation and roof trusses or rafter systems	-	✓
57	Show sizes, type, span lengths and required number of support jack studs, king studs for shear wall opening and girder or header per FBC-R602.7.	-	✓
58	Indicate where pressure treated wood will be placed	-	✓
59	Show all wall structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing edges & intermediate areas	-	✓
60	A detail showing gable truss bracing, wall balloon framing details or/ and wall hinge bracing detail	-	✓

FBCR :ROOF SYSTEMS:

61	Truss design drawing shall meet section FBC-R 802.10. 1 Wood trusses	-	✓
62	Include a layout and truss details, signed and sealed by Florida Professional Engineer	-	✓
63	Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters	-	✓
64	Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details	-	✓
65	Provide dead load rating of trusses	-	✓

FBCR 802:Conventional Roof Framing Layout

66	Rafter and ridge beams sizes, span, species and spacing	-	✓
67	Connectors to wall assemblies' include assemblies' resistance to uplift rating	-	✓
68	Valley framing and support details	-	✓
69	Provide dead load rating of rafter system	-	✓

FBCR 803 ROOF SHEATHING

70	Include all materials which will make up the roof decking, identification of structural panel sheathing, grade, thickness	-	✓
71	Show fastener Size and schedule for structural panel sheathing on the edges & intermediate areas	-	✓

ROOF ASSEMBLIES FRC Chapter 9

72	Include all materials which will make up the roof assembles covering	-	✓		
73	Submit Florida Product Approval numbers for each component of the roof assembles covering	-	✓		

FBCR Chapter 11 Energy Efficiency Code for Residential Building

Residential construction shall comply with this code by using the following compliance methods in the FBCR Chapter 11 Residential buildings compliance methods. **Two of the required forms are to be submitted, N1100.1.1.1 As an alternative to the computerized Compliance Method A, the Alternate Residential Point System Method hand calculation, Alternate Form 600A, may be used. All requirements specific to this calculation are located in Sub appendix C to Appendix G. Buildings complying by this alternative shall meet all mandatory requirements of this chapter. Computerized versions of the Alternate Residential Point System Method shall not be acceptable for code compliance.**

GENERAL REQUIREMENTS: APPLICANT - PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Items to Include- Each Box shall be Circled as Applicable			
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Select from Drop Down

74	Show the insulation R value for the following areas of the structure	-	✓		
75	Attic space <i>R-38</i>	-	✓		
76	Exterior wall cavity <i>R-19</i>	-	✓		
77	Crawl space <i>Concrete Floor.</i>	-			✓

HVAC information

78	Submit two copies of a Manual J sizing equipment or equivalent computation study	-	✓		
79	Exhaust fans shown in bathrooms Mechanical exhaust capacity of 50 cfm intermittent or 20 cfm continuous required <i>Shown on Elec. Plan p.3</i>	-	✓		
80	Show clothes dryer route and total run of exhaust duct	-	✓		

Plumbing Fixture layout shown

81	All fixtures waste water lines shall be shown on the foundation plan	-			✓
82	Show the location of water heater	-	✓		

Private Potable Water

83	Pump motor horse power <i>1 1/2 H.P.</i>	-	✓		
84	Reservoir pressure tank gallon capacity <i>86 Gall.</i>	-	✓		
85	Rating of cycle stop valve if used <i>36 PM.</i>	-	✓		

Electrical layout shown including

86	Show Switches, receptacles outlets, lighting fixtures and Ceiling fans	-	✓		
87	Show all 120-volt, single phase, 15- and 20-ampere branch circuits outlets required to be protected by Ground-Fault Circuit Interrupter (GFCI) Article 210.8 A	-	✓		
88	Show the location of smoke detectors & Carbon monoxide detectors	-	✓		
89	Show service panel, sub-panel, location(s) and total ampere ratings <i>200 AMP.</i>	-	✓		
90	On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Indicate if the utility company service entrance cable will be of the overhead or underground type. <i>Under Ground.</i>	-	✓		
	For structures with foundation which establish new electrical utility companies service connection a Concrete Encased Electrode will be required within the foundation to serve as an Grounding electrode system. Per the National Electrical Code article 250.52.3				
91	Appliances and HVAC equipment and disconnects	-	✓		
92	Show all 120-volt, single phase, 15- and 20-ampere branch circuits supplying outlets installed in dwelling unit family rooms, dining rooms, living rooms, parlors, libraries, dens, bedrooms, sunrooms, recreation rooms, closets, hallways, or similar rooms or areas shall be protected by a listed Combination arc-fault circuit interrupter, Protection device.	-	✓		

Notice Of Commencement:

A notice of commencement form **RECORDED** in the Columbia County Clerk Office is required to be filed with the Building Department **BEFORE ANY INSPECTIONS** can be performed.

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Circled as Applicable
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****ITEMS 95, 96, & 98 Are Required After APPROVAL from the ZONING DEPT.****

Select from Drop down

93	Building Permit Application A current Building Permit Application is to be completed, by following the Checklist all supporting documents must be submitted. There is a \$15.00 application fee. The completed application with attached documents and application fee can be mailed.	-	✓
94	Parcel Number The parcel number (Tax ID number) from the Property Appraisers Office (386) 758-1083 is required. A copy of property deed is also required. www.columbiacountyfla.com	-	✓
95	Environmental Health Permit or Sewer Tap Approval A copy of a approved Columbia County Environmental Health (386) 758-1058	-	✓
96	City of Lake City A City Water and/or Sewer letter. Call 386-752-2031	-	✓
97	Toilet facilities shall be provided for all construction sites	-	✓
98	Town of Fort White (386) 497-2321 If the parcel in the application for building permit is within the Corporate city limits of Fort White, an approval land use development letter issued by the Town of Fort is required to be submitted with the application for a building permit.	-	✓
99	Flood Information: All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting a application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.5.2 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.5.3 of the Columbia County Land Development Regulations (Municode.com)	-	✓
100	CERTIFIED FINISHED FLOOR ELEVATIONS will be required on any project where the approved FIRM Flood Maps show the property is in a AE, Floodway, and AH flood zones. Additionally One Foot Rise letters are required for AE and AH zones. In the Floodway Flood zones a Zero Rise letter is required.	-	✓
101	A Flood development permit is also required for AE, Floodway & AH. Development permit cost is \$50.00	-	✓
102	Driveway Connection: If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. County Public Works Dept. determines the size and length of every culvert before instillation and completes a final inspection before permanent power is granted. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00) Separate Check when issued. If the project is to be located on an F.D.O.T. maintained road, then an F.D.O.T. access permit is required. <i>Existing Drive</i>	-	✓
103	911 Address: An application for a 911 address must be applied for and received through the Columbia County Emergency Management Office of 911 Addressing Department (386) 758-1125.	-	✓

Ordinance Sec. 90-75. - Construction debris. (e) It shall be unlawful for any person to dispose of or discard solid waste, including construction or demolition debris at any place within the county other than on an authorized disposal site or at the county's solid waste facilities. The temporary storage, not to exceed seven days of solid waste (excluding construction and demolition debris) on the premises where generated or vegetative trash pending disposition as authorized by law or ordinance, shall not be deemed a violation of this section. The temporary storage of construction and demolition debris on the premises where generated or vegetative trash pending disposition as authorized by law or ordinance shall not be deemed in violation of this section; provided, however, such construction and demolition debris must be disposed of in accordance with this article prior to the county's issuance of a certificate of occupancy for the premises. The burning of lumber from a construction or demolition project or vegetative trash when done so with legal and proper permits from the authorized agencies and in accordance with such agencies' rules and regulations, shall not be deemed a violation of this section. No person shall bury, throw, place, or deposit, or cause to be buried, thrown, placed, or deposited, any solid waste, special waste, or debris of any kind into or on any of the public streets, road right-of-way, highways, bridges, alleys, lanes, thoroughfares, waters, canals, or vacant lots or lands within the county. No person shall bury any vegetative trash on any of the public streets, road right-of-way, highways, bridges, lanes, thoroughfares, waters, canals, or lots less than ten acres in size within the county.

Wulburn & Sylvia Fender

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed online @ www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
1. EXTERIOR DOORS			
A. SWINGING	MASONITE	IN swing & OUT swing STEEL	FL4904-R5
B. SLIDING			
C. SECTIONAL/ROLL UP			
D. OTHER			
2. WINDOWS			
A. SINGLE/DOUBLE HUNG	MI	VINYL 3540 SINGLE HUNG	FL12250-R10
B. HORIZONTAL SLIDER			
C. CASEMENT			
D. FIXED			
E. MULLION			
F. SKYLIGHTS			
G. OTHER			
3. PANEL WALL			
A. SIDING	CERTAINTED	WEATHERBOARD FIBERCEMENT	FL3148-R4
B. SOFFITS	KAYCAN	ALUM	FL16503
C. STOREFRONTS			
D. GLASS BLOCK			
E. OTHER	RHINO	UNDERLAYMENT	FL15216
4. ROOFING PRODUCTS			
A. ASPHALT SHINGLES	CERTAINTED	ARCH SHINGLES	FL5444
B. NON-STRUCT METAL			
C. ROOFING TILES			
D. SINGLE PLY ROOF			
E. OTHER	WOODLAND	#30 ROOFING FELT	FL1814-R6
5. STRUCT COMPONENTS			
A. WOOD CONNECTORS			
B. WOOD ANCHORS			
C. TRUSS PLATES			
D. INSULATION FORMS			
E. LINTELS			
F. OTHERS			
6. NEW EXTERIOR ENVELOPE PRODUCTS			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements.

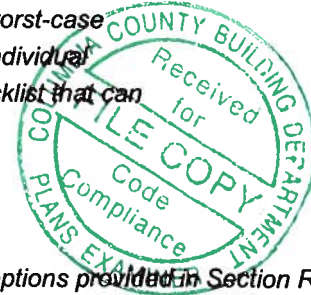
Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.

NOTES: _____

RESIDENTIAL ENERGY CONSERVATION CODE DOCUMENTATION CHECKLIST**Florida Department of Business and Professional Regulation
Simulated Performance Alternative (Performance) Method**

Applications for compliance with the 2017 Florida Building Code, Energy Conservation via the residential Simulated Performance Method shall include:

- ☐ This checklist
- ☐ A Form R405 report that documents that the Proposed Design complies with Section R405.3 of the Florida Energy Code. This form shall include a summary page indicating home address, e-ratio and the pass or fail status along with summary areas and types of components, whether the home was simulated as a worst-case orientation, name and version of the compliance software tool, name of individual completing the compliance report (one page) and an input summary checklist that can be used for field verification (usually four pages/may be greater).
- ☐ Energy Performance Level (EPL) Display Card (one page)
- ☐ HVAC system sizing and selection based on ACCA Manual S or per exceptions provided in Section R403.7
- ☐ Mandatory Requirements (five pages)



Required prior to CO for the Performance Method:

- ☐ Air Barrier and Insulation Inspection Component Criteria checklist (Table R402.4.1.1 - one page)
- ☐ A completed Envelope Leakage Test Report (usually one page)
- ☐ If Form R405 duct leakage type indicates anything other than "default leakage", then a completed Form R405 Duct Leakage Test Report (usually one page)


FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Business and Professional Regulation - Residential Performance Method

Project Name: 200191 Fender Street: City, State, Zip: Lake City, FL, 32025 Owner: Fender, Wilber & Sylvia Design Location: FL, Gainesville	Builder Name: Edgley Construction Permit Office: Permit Number: Jurisdiction: County: Columbia (Florida Climate Zone 2)
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Glass/Floor Area: 0.137	Total Proposed Modified Loads: 50.04	PASS
	Total Baseline Loads: 50.16	

<p>I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.</p> <p>PREPARED BY: <u>Evan Beamsley</u></p> <p>DATE: <u>2020-02-27</u></p> <p>I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.</p> <p>OWNER/AGENT: _____</p> <p>DATE: _____</p>	<p>Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.</p> <div style="text-align: center;">  </div> <p>BUILDING OFFICIAL: _____</p> <p>DATE: _____</p>
--	---

- Compliance requires certification by the air handler unit manufacturer that the air handler enclosure qualifies as certified factory-sealed in accordance with R403.3.2.1.
- Compliance requires an Air Barrier and Insulation Inspection Checklist in accordance with R402.4.1.1 and this project requires an envelope leakage test report with envelope leakage no greater than 7.00 ACH50 (R402.4.1.2).

INPUT SUMMARY CHECKLIST REPORT

PROJECT

Title:	200191 Fender	Bedrooms:	3	Address Type:	Street Address
Building Type:	User	Conditioned Area:	1715	Lot #	
Owner Name:	Fender, Wilber & Sylvia	Total Stories:	1	Block/Subdivision:	
# of Units:	1	Worst Case:	Yes	PlatBook:	
Builder Name:	Edgley Construction	Rotate Angle:	225	Street:	
Permit Office:		Cross Ventilation:		County:	Columbia
Jurisdiction:		Whole House Fan:		City, State, Zip:	Lake City , FL , 32025
Family Type:	Single-family				
New/Existing:	New (From Plans)				
Comment:					

CLIMATE

✓	Design Location	TMY Site	Design Temp 97.5 %	2.5 %	Int Design Temp Winter	Summer	Heating Degree Days	Design Moisture	Daily Temp Range
_____	FL, Gainesville	FL_GAINESVILLE_REGI	32	92	70	75	1305.5	51	Medium

BLOCKS

Number	Name	Area	Volume
1	Block1	1715	15435

SPACES

Number	Name	Area	Volume	Kitchen	Occupants	Bedrooms	Infil ID	Finished	Cooled	Heated
1	Main	1715	15435	Yes	6	3	1	Yes	Yes	Yes

FLOORS

✓	#	Floor Type	Space	Perimeter	R-Value	Area		Tile	Wood	Carpet
_____	1	Slab-On-Grade Edge Insulatio	Main	199 ft	0	1715 ft²	----	0.3	0.3	0.4

ROOF

✓	#	Type	Materials	Roof Area	Gable Area	Roof Color	Rad Barr	Solar Absor.	SA Tested	Emitt	Emitt Tested	Deck Insul.	Pitch (deg)
_____	1	Hip	Composition shingles	1918 ft²	0 ft²	Dark	N	0.92	No	0.9	No	0	26.6

ATTIC

✓	#	Type	Ventilation	Vent Ratio (1 in)	Area	RBS	IRCC
_____	1	Full attic	Vented	300	1715 ft²	N	N

CEILING

✓	#	Ceiling Type	Space	R-Value	Ins Type	Area	Framing Frac	Truss Type
_____	1	Under Attic (Vented)	Main	30	Blown	1715 ft²	0.11	Wood

INPUT SUMMARY CHECKLIST REPORT

WALLS

✓ #	Ornt	Adjacent To	Wall Type	Space	Cavity R-Value	Width Ft	In	Height Ft	In	Area	Sheathing R-Value	Framing Fraction	Solar Absor	Below Grade%
1	N=>SW	Exterior	Face Brick - Wood	Main	19	11	10	9		106.5 ft²	0	0.23	0.75	0
2	E=>NW	Exterior	Face Brick - Wood	Main	19	4		9		36.0 ft²		0.23	0.75	0
3	N=>SW	Exterior	Face Brick - Wood	Main	19	14	4	9		129.0 ft²		0.23	0.75	0
4	W=>SE	Exterior	Face Brick - Wood	Main	19	4		9		36.0 ft²		0.23	0.75	0
5	N=>SW	Exterior	Face Brick - Wood	Main	19	13	4	9		120.0 ft²		0.23	0.75	0
6	W=>SE	Exterior	Face Brick - Wood	Main	19	6		9		54.0 ft²		0.23	0.75	0
7	N=>SW	Exterior	Face Brick - Wood	Main	19	14	10	9		133.5 ft²		0.23	0.75	0
8	E=>NW	Exterior	Face Brick - Wood	Main	19	35	6	9		319.5 ft²		0.23	0.75	0
9	S=>NE	Garage	Frame - Wood	Main	13	25	4	9		228.0 ft²		0.23	0.75	0
10	E=>NW	Garage	Frame - Wood	Main	13	5	8	9		51.0 ft²		0.23	0.75	0
11	S=>NE	Exterior	Face Brick - Wood	Main	19	7		9		63.0 ft²		0.23	0.75	0
12	S=>NE	Exterior	Face Brick - Wood	Main	19	14	6	9		130.5 ft²		0.23	0.75	0
13	W=>SE	Exterior	Face Brick - Wood	Main	19	11	4	9		102.0 ft²		0.23	0.75	0
14	S=>NE	Exterior	Face Brick - Wood	Main	19	7	6	9		67.5 ft²		0.23	0.75	0
15	W=>SE	Exterior	Face Brick - Wood	Main	19	23	10	9		214.5 ft²		0.23	0.75	0

DOORS

✓ #	Ornt	Door Type	Space	Storms	U-Value	Width Ft	In	Height Ft	In	Area
1	W=>SE	Insulated	Main	None	.4	1		6	8	6.7 ft²
2	S=>NE	Insulated	Main	None	.4	2	8	6	8	17.8 ft²
3	S=>NE	Insulated	Main	None	.4	3		6	8	20 ft²

WINDOWS

Orientation shown is the entered orientation (=>) changed to Worst Case.

✓ #	Ornt	Wall ID	Frame	Panes	NFRC	U-Factor	SHGC	Imp	Area	Overhang Depth	Separation	Int Shade	Screening
1	N=>SW	1	Metal	Low-E Double	Yes	0.35	0.25	N	30.0 ft²	1 ft 6 in	4 ft 0 in	None	None
2	N=>SW	3	Metal	Low-E Double	Yes	0.35	0.25	N	45.0 ft²	17 ft 6 in	1 ft 0 in	None	None
3	W=>SE	4	Metal	Low-E Double	Yes	0.35	0.25	N	11.1 ft²	99 ft 0 in	1 ft 0 in	None	None
4	N=>SW	5	Metal	Low-E Double	Yes	0.35	0.25	N	45.0 ft²	13 ft 6 in	1 ft 0 in	None	None
5	N=>SW	7	Metal	Low-E Double	Yes	0.35	0.25	N	45.0 ft²	1 ft 6 in	1 ft 0 in	None	None
6	E=>NW	8	Metal	Low-E Double	Yes	0.35	0.25	N	15.0 ft²	1 ft 6 in	1 ft 0 in	None	None
7	E=>NW	8	Metal	Low-E Double	Yes	0.35	0.25	N	4.0 ft²	1 ft 6 in	1 ft 0 in	None	None
8	S=>NE	11	Metal	Low-E Double	Yes	0.35	0.25	N	13.3 ft²	8 ft 6 in	1 ft 0 in	None	None
9	S=>NE	12	Metal	Low-E Double	Yes	0.35	0.25	N	20.0 ft²	1 ft 6 in	1 ft 0 in	None	None
10	W=>SE	15	Metal	Low-E Double	Yes	0.35	0.25	N	6.0 ft²	1 ft 6 in	1 ft 0 in	None	None

INPUT SUMMARY CHECKLIST REPORT

GARAGE

✓	#	Floor Area	Ceiling Area	Exposed Wall Perimeter	Avg. Wall Height	Exposed Wall Insulation
✓	1	588 ft²	588 ft²	70 ft	9 ft	1

INFILTRATION

#	Scope	Method	SLA	CFM 50	ELA	EqLA	ACH	ACH 50
1	Wholehouse	Proposed ACH(50)	.0004	1800.8	98.86	185.92	.1579	7

HEATING SYSTEM

✓	#	System Type	Subtype	Efficiency	Capacity	Block	Ducts
✓	1	Electric Heat Pump/	None	HSPF:8.5	32 kBtu/hr	1	sys#1

COOLING SYSTEM

✓	#	System Type	Subtype	Efficiency	Capacity	Air Flow	SHR	Block	Ducts
✓	1	Central Unit/	None	SEER: 14	32 kBtu/hr	960 cfm	0.75	1	sys#1

HOT WATER SYSTEM

✓	#	System Type	SubType	Location	EF	Cap	Use	SetPnt	Conservation
✓	1	Electric	None	Main	0.95	40 gal	60 gal	120 deg	None

SOLAR HOT WATER SYSTEM

✓	FSEC Cert #	Company Name	System Model #	Collector Model #	Collector Area	Storage Volume	FEF
✓	None	None			ft²		

DUCTS

✓	#	--- Supply ---		--- Return ---		Leakage Type	Air Handler	CFM 25 TOT	CFM25 OUT	QN	RLF	HVAC #	
		Location	R-Value	Area	Location	Area						Heat	Cool
✓	1	Attic	6	343 ft²	Attic	85.75 ft	Default Leakage	Main	(Default)	(Default)		1	1

INPUT SUMMARY CHECKLIST REPORT

TEMPERATURES

Programable Thermostat: Y

Ceiling Fans:

Cooling	<input type="checkbox"/> Jan	<input type="checkbox"/> Feb	<input type="checkbox"/> Mar	<input type="checkbox"/> Apr	<input type="checkbox"/> May	<input checked="" type="checkbox"/> Jun	<input checked="" type="checkbox"/> Jul	<input checked="" type="checkbox"/> Aug	<input checked="" type="checkbox"/> Sep	<input type="checkbox"/> Oct	<input type="checkbox"/> Nov	<input type="checkbox"/> Dec
Heating	<input checked="" type="checkbox"/> Jan	<input checked="" type="checkbox"/> Feb	<input checked="" type="checkbox"/> Mar	<input checked="" type="checkbox"/> Apr	<input type="checkbox"/> May	<input type="checkbox"/> Jun	<input type="checkbox"/> Jul	<input type="checkbox"/> Aug	<input type="checkbox"/> Sep	<input checked="" type="checkbox"/> Oct	<input checked="" type="checkbox"/> Nov	<input checked="" type="checkbox"/> Dec
Venting	<input type="checkbox"/> Jan	<input type="checkbox"/> Feb	<input checked="" type="checkbox"/> Mar	<input checked="" type="checkbox"/> Apr	<input type="checkbox"/> May	<input type="checkbox"/> Jun	<input type="checkbox"/> Jul	<input type="checkbox"/> Aug	<input type="checkbox"/> Sep	<input checked="" type="checkbox"/> Oct	<input checked="" type="checkbox"/> Nov	<input checked="" type="checkbox"/> Dec

Thermostat Schedule: HERS 2006 Reference

Hours

Schedule Type		1	2	3	4	5	6	7	8	9	10	11	12
---------------	--	---	---	---	---	---	---	---	---	---	----	----	----

Cooling (WD)	AM	78	78	78	78	78	78	78	78	80	80	80	80
	PM	80	80	78	78	78	78	78	78	78	78	78	78

Cooling (WEH)	AM	78	78	78	78	78	78	78	78	78	78	78	78
	PM	78	78	78	78	78	78	78	78	78	78	78	78

Heating (WD)	AM	66	66	66	66	66	68	68	68	68	68	68	68
	PM	68	68	68	68	68	68	68	68	68	68	66	66

Heating (WEH)	AM	66	66	66	66	66	68	68	68	68	68	68	68
	PM	68	68	68	68	68	68	68	68	68	68	66	66

MASS

Mass Type

Area

Thickness

Furniture Fraction

Space

Default(8 lbs/sq.ft.)

0 ft²

0 ft

0.3

Main

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE INDEX* = 100

The lower the Energy Performance Index, the more efficient the home.

1. New home or, addition	1. <u>New (From Plans)</u>	12. Ducts, location & insulation level	
2. Single-family or multiple-family	2. <u>Single-family</u>	a) Supply ducts	R <u>6.0</u>
3. No. of units (if multiple-family)	3. <u>1</u>	b) Return ducts	R <u>6.0</u>
4. Number of bedrooms	4. <u>3</u>	c) AHU location	Main
5. Is this a worst case? (yes/no)	5. <u>Yes</u>	13. Cooling system:	Capacity <u>32.0</u>
6. Conditioned floor area (sq. ft.)	6. <u>1715</u>	a) Split system	SEER <u> </u>
7. Windows, type and area		b) Single package	SEER <u> </u>
a) U-factor:(weighted average)	7a. <u>0.350</u>	c) Ground/water source	SEER/COP <u> </u>
b) Solar Heat Gain Coefficient (SHGC)	7b. <u>0.250</u>	d) Room unit/PTAC	EER <u> </u>
c) Area	7c. <u>234.4</u>	e) Other	<u>14.0</u>
8. Skylights		14. Heating system:	Capacity <u>32.0</u>
a) U-factor:(weighted average)	8a. <u>NA</u>	a) Split system heat pump	HSPF <u> </u>
b) Solar Heat Gain Coefficient (SHGC)	8b. <u>NA</u>	b) Single package heat pump	HSPF <u> </u>
9. Floor type, insulation level:		c) Electric resistance	COP <u> </u>
a) Slab-on-grade (R-value)	9a. <u>0.0</u>	d) Gas furnace, natural gas	AFUE <u> </u>
b) Wood, raised (R-value)	9b. <u> </u>	e) Gas furnace, LPG	AFUE <u> </u>
c) Concrete, raised (R-value)	9c. <u> </u>	f) Other	<u>8.50</u>
10. Wall type and insulation:		15. Water heating system	
A. Exterior:		a) Electric resistance	EF <u>0.95</u>
1. Wood frame (Insulation R-value)	10A1. <u>19.0</u>	b) Gas fired, natural gas	EF <u> </u>
2. Masonry (Insulation R-value)	10A2. <u> </u>	c) Gas fired, LPG	EF <u> </u>
B. Adjacent:		d) Solar system with tank	EF <u> </u>
1. Wood frame (Insulation R-value)	10B1. <u>13.0</u>	e) Dedicated heat pump with tank	EF <u> </u>
2. Masonry (Insulation R-value)	10B2. <u> </u>	f) Heat recovery unit	HeatRec% <u> </u>
11. Ceiling type and insulation level		g) Other	
a) Under attic	11a. <u>30.0</u>	16. HVAC credits claimed (Performance Method)	
b) Single assembly	11b. <u> </u>	a) Ceiling fans	<u> </u>
c) Knee walls/skylight walls	11c. <u> </u>	b) Cross ventilation	<u>No</u>
d) Radiant barrier installed	11d. <u>No</u>	c) Whole house fan	<u>No</u>
		d) Multizone cooling credit	<u> </u>
		e) Multizone heating credit	<u> </u>
		f) Programmable thermostat	<u>Yes</u>

*Label required by Section R303.1.3 of the Florida Building Code, Energy Conservation, if not DEFAULT.

I certify that this home has complied with the Florida Building Code, Energy Conservation, through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL display card will be completed based on installed code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: Lake City, FL 32025

Florida Building Code, Energy Conservation, 6th Edition (2017)

Mandatory Requirements for Residential Performance, Prescriptive and ERI Methods

ADDRESS:

Lake City , FL , 32025

Permit Number:

MANDATORY REQUIREMENTS See individual code sections for full details.

✓

SECTION R401 GENERAL

- ☐ **R401.3 Energy Performance Level (EPL) display card (Mandatory).** The building official shall require that an energy performance level (EPL) display card be completed and certified by the builder to be accurate and correct before final approval of the building for occupancy. Florida law (Section 553.9085, Florida Statutes) requires the EPL display card to be included as an addendum to each sales contract for both presold and nonpresold residential buildings. The EPL display card contains information indicating the energy performance level and efficiencies of components installed in a dwelling unit. The building official shall verify that the EPL display card completed and signed by the builder accurately reflects the plans and specifications submitted to demonstrate code compliance for the building. A copy of the EPL display card can be found in Appendix RD.

- ☐ **R402.4 Air leakage (Mandatory).** The building thermal envelope shall be constructed to limit air leakage in accordance with the requirements of Sections R402.4.1 through R402.4.5.

Exception: Dwelling units of R-2 Occupancies and multiple attached single family dwellings shall be permitted to comply with Section C402.5.

- ☐ **R402.4.1 Building thermal envelope.** The building thermal envelope shall comply with Sections R402.4.1.1 and R402.4.1.2. The sealing methods between dissimilar materials shall allow for differential expansion and contraction.

- ☐ **R402.4.1.1 Installation.** The components of the building thermal envelope as listed in Table R402.4.1.1 shall be installed in accordance with the manufacturer's instructions and the criteria listed in Table R402.4.1.1, as applicable to the method of construction. Where required by the code official, an approved third party shall inspect all components and verify compliance.

- ☐ **R402.4.1.2 Testing.** The building or dwelling unit shall be tested and verified as having an air leakage rate not exceeding seven air changes per hour in Climate Zones 1 and 2, and three air changes per hour in Climate Zones 3 through 8. Testing shall be conducted in accordance with ANSI/RESNET/ICC 380 and reported at a pressure of 0.2 inch w.g. (50 pascals). Testing shall be conducted by either individuals as defined in Section 553.993(5) or (7), Florida Statutes, or individuals licensed as set forth in Section 489.105(3)(f), (g) or (i) or an approved third party. A written report of the results of the test shall be signed by the party conducting the test and provided to the code official. Testing shall be performed at any time after creation of all penetrations of the building thermal envelope.

Exception: Testing is not required for additions, alterations, renovations, or repairs, of the building thermal envelope of existing buildings in which the new construction is less than 85 percent of the building thermal envelope.

During testing:

1. Exterior windows and doors, fireplace and stove doors shall be closed, but not sealed, beyond the intended weatherstripping or other infiltration control measures.
2. Dampers including exhaust, intake, makeup air, backdraft and flue dampers shall be closed, but not sealed beyond intended infiltration control measures.
3. Interior doors, if installed at the time of the test, shall be open.
4. Exterior doors for continuous ventilation systems and heat recovery ventilators shall be closed and sealed.
5. Heating and cooling systems, if installed at the time of the test, shall be turned off.
6. Supply and return registers, if installed at the time of the test, shall be fully open.

- ☐ **R402.4.2 Fireplaces.** New wood-burning fireplaces shall have tight-fitting flue dampers or doors, and outdoor combustion air. Where using tight-fitting doors on factory-built fireplaces listed and labeled in accordance with UL 127, the doors shall be tested and listed for the fireplace. Where using tight-fitting doors on masonry fireplaces, the doors shall be listed and labeled in accordance with UL 907.

- ☐ **R402.4.3 Fenestration air leakage.** Windows, skylights and sliding glass doors shall have an air infiltration rate of no more than 0.3 cfm per square foot (1.5 L/s/m²), and swinging doors no more than 0.5 cfm per square foot (2.6 L/s/m²), when tested according to NFRC 400 or AAMA/WDMA/CSA 101/I.S.2/A440 by an accredited, independent laboratory and listed and labeled by the manufacturer.

Exception: Site-built windows, skylights and doors.

MANDATORY REQUIREMENTS - (Continued)

- ☐ **R402.4.4 Rooms containing fuel-burning appliances.** In Climate Zones 3 through 8, where open combustion air ducts provide combustion air to open combustion fuel burning appliances, the appliances and combustion air opening shall be located outside the building thermal envelope or enclosed in a room, isolated from inside the thermal envelope. Such rooms shall be sealed and insulated in accordance with the envelope requirements of Table R402.1.2, where the walls, floors and ceilings shall meet not less than the basement wall R-value requirement. The door into the room shall be fully gasketed and any water lines and ducts in the room insulated in accordance with Section R403. The combustion air duct shall be insulated where it passes through conditioned space to a minimum of R-8.

Exceptions:

1. Direct vent appliances with both intake and exhaust pipes installed continuous to the outside.
2. Fireplaces and stoves complying with Section R402.4.2 and Section R1006 of the Florida Building Code, Residential.

- ☐ **R402.4.5 Recessed lighting.** Recessed luminaires installed in the building thermal envelope shall be sealed to limit air leakage between conditioned and unconditioned spaces. All recessed luminaires shall be IC-rated and labeled as having an air leakage rate not more than 2.0 cfm (0.944 L/s) when tested in accordance with ASTM E283 at a 1.57 psf (75 Pa) pressure differential. All recessed luminaires shall be sealed with a gasket or caulk between the housing and the interior wall or ceiling covering.

SECTION R403 SYSTEMS

R403.1 Controls.

- ☐ **R403.1.1 Thermostat provision (Mandatory).** At least one thermostat shall be provided for each separate heating and cooling system.

- ☐ **R403.1.3 Heat pump supplementary heat (Mandatory).** Heat pumps having supplementary electric-resistance heat shall have controls that, except during defrost, prevent supplemental heat operation when the heat pump compressor can meet the heating load.

- ☐ **R403.3.2 Sealing (Mandatory)** All ducts, air handlers, filter boxes and building cavities that form the primary air containment passageways for air distribution systems shall be considered ducts or plenum chambers, shall be constructed and sealed in accordance with Section C403.2.9.2 of the Commercial Provisions of this code and shall be shown to meet duct tightness criteria below.

Duct tightness shall be verified by testing in accordance with ANSI/RESNET/ICC 380 by either individuals as defined in Section 553.993(5) or (7), Florida Statutes, or individuals licensed as set forth in Section 489.105(3)(f), (g) or (i), Florida Statutes, to be "substantially leak free" in accordance with Section R403.3.3.

- ☐ **R403.3.2.1 Sealed air handler.** Air handlers shall have a manufacturer's designation for an air leakage of no more than 2 percent of the design airflow rate when tested in accordance with ASHRAE 193.

- ☐ **R403.3.3 Duct testing (Mandatory).** Ducts shall be pressure tested to determine air leakage by one of the following methods:

1. Rough-in test: Total leakage shall be measured with a pressure differential of 0.1 inch w.g. (25 Pa) across the system, including the manufacturer's air handler enclosure if installed at the time of the test. All registers shall be taped or otherwise sealed during the test.
2. Postconstruction test: Total leakage shall be measured with a pressure differential of 0.1 inch w.g. (25 Pa) across the entire system, including the manufacturer's air handler enclosure. Registers shall be taped or otherwise sealed during the test.

Exceptions:

1. A duct air leakage test shall not be required where the ducts and air handlers are located entirely within the building thermal envelope.
2. Duct testing is not mandatory for buildings complying by Section 405 of this code.

A written report of the results of the test shall be signed by the party conducting the test and provided to the code official.

- ☐ **R403.3.5 Building cavities (Mandatory).** Building framing cavities shall not be used as ducts or plenums.

- ☐ **R403.4 Mechanical system piping insulation (Mandatory).** Mechanical system piping capable of carrying fluids above 105°F (41°C) or below 55°F (13°C) shall be insulated to a minimum of R-3.

- ☐ **R403.4.1 Protection of piping insulation.** Piping insulation exposed to weather shall be protected from damage, including that caused by sunlight, moisture, equipment maintenance and wind, and shall provide shielding from solar radiation that can cause degradation of the material. Adhesive tape shall not be permitted.

- ☐ **R403.5.1 Heated water circulation and temperature maintenance systems (Mandatory)** Heated water circulation systems shall be in accordance with Section R403.5.1.1. Heat trace temperature maintenance systems shall be in accordance with Section R403.5.1.2. Automatic controls, temperature sensors and pumps shall be accessible. Manual controls shall be readily accessible.

- ☐ **R403.5.1.1 Circulation systems.** Heated water circulation systems shall be provided with a circulation pump. The system return pipe shall be a dedicated return pipe or a cold water supply pipe. Gravity and thermosiphon circulation systems shall be prohibited. Controls for circulating hot water system pumps shall start the pump based on the identification of a demand for hot water within the occupancy. The controls shall automatically turn off the pump when the water in the circulation loop is at the desired temperature and when there is no demand for hot water.

- ☐ **R403.5.1.2 Heat trace systems.** Electric heat trace systems shall comply with IEEE 515.1 or UL 515. Controls for such systems shall automatically adjust the energy input to the heat tracing to maintain the desired water temperature in the piping in accordance with the times when heated water is used in the occupancy.

MANDATORY REQUIREMENTS - (Continued)

- ☐ **R403.5.5 Heat traps (Mandatory).** Storage water heaters not equipped with integral heat traps and having vertical pipe risers shall have heat traps installed on both the inlets and outlets. External heat traps shall consist of either a commercially available heat trap or a downward and upward bend of at least 3 ½ inches (89 mm) in the hot water distribution line and cold water line located as close as possible to the storage tank.
- R403.5.6 Water heater efficiencies (Mandatory).**
- ☐ **R403.5.6.1.1 Automatic controls.** Service water-heating systems shall be equipped with automatic temperature controls capable of adjustment from the lowest to the highest acceptable temperature settings for the intended use. The minimum temperature setting range shall be from 100°F to 140°F (38°C to 60°C).
- ☐ **R403.5.6.1.2 Shut down.** A separate switch or a clearly marked circuit breaker shall be provided to permit the power supplied to electric service systems to be turned off. A separate valve shall be provided to permit the energy supplied to the main burner(s) of combustion types of service water-heating systems to be turned off.
- ☐ **R403.5.6.2 Water-heating equipment.** Water-heating equipment installed in residential units shall meet the minimum efficiencies of Table C404.2 in Chapter 4 of the Florida Building Code, Energy Conservation, Commercial Provisions, for the type of equipment installed. Equipment used to provide heating functions as part of a combination system shall satisfy all stated requirements for the appropriate water-heating category. Solar water heaters shall meet the criteria of Section R403.5.6.2.1.
- ☐ **R403.5.6.2.1 Solar water-heating systems.** Solar systems for domestic hot water production are rated by the annual solar energy factor of the system. The solar energy factor of a system shall be determined from the Florida Solar Energy Center Directory of Certified Solar Systems. Solar collectors shall be tested in accordance with ISO Standard 9806, Test Methods for Solar Collectors, and SRCC Standard TM-1, Solar Domestic Hot Water System and Component Test Protocol. Collectors in installed solar water-heating systems should meet the following criteria:
1. Be installed with a tilt angle between 10 degrees and 40 degrees of the horizontal; and
 2. Be installed at an orientation within 45 degrees of true south.
- ☐ **R403.6 Mechanical ventilation (Mandatory).** The building shall be provided with ventilation that meets the requirements of the Florida Building Code, Residential, or Florida Building Code, Mechanical, as applicable, or with other approved means of ventilation including: Natural, Infiltration or Mechanical means. Outdoor air intakes and exhausts shall have automatic or gravity dampers that close when the ventilation system is not operating.
- ☐ **R403.6.1 Whole-house mechanical ventilation system fan efficacy.** When installed to function as a whole-house mechanical ventilation system, fans shall meet the efficacy requirements of Table R403.6.1.
- Exception:** Where whole-house mechanical ventilation fans are integral to tested and listed HVAC equipment, they shall be powered by an electronically commutated motor.
- ☐ **R403.6.2 Ventilation air.** Residential buildings designed to be operated at a positive indoor pressure or for mechanical ventilation shall meet the following criteria:
1. The design air change per hour minimums for residential buildings in ASHRAE 62.2, Ventilation for Acceptable Indoor Air Quality, shall be the maximum rates allowed for residential applications.
 2. No ventilation or air-conditioning system make-up air shall be provided to conditioned space from attics, crawlspaces, attached enclosed garages or outdoor spaces adjacent to swimming pools or spas.
 3. If ventilation air is drawn from enclosed space(s), then the walls of the space(s) from which air is drawn shall be insulated to a minimum of R-11 and the ceiling shall be insulated to a minimum of R-19, space permitting, or R-10 otherwise.
- R403.7 Heating and cooling equipment (Mandatory).**
- ☐ **R403.7.1 Equipment sizing.** Heating and cooling equipment shall be sized in accordance with ACCA Manual S based on the equipment loads calculated in accordance with ACCA Manual J or other approved heating and cooling calculation methodologies, based on building loads for the directional orientation of the building. The manufacturer and model number of the outdoor and indoor units (if split system) shall be submitted along with the sensible and total cooling capacities at the design conditions described in Section R302.1. This Code does not allow designer safety factors, provisions for future expansion or other factors that affect equipment sizing. System sizing calculations shall not include loads created by local intermittent mechanical ventilation such as standard kitchen and bathroom exhaust systems. New or replacement heating and cooling equipment shall have an efficiency rating equal to or greater than the minimum required by federal law for the geographic location where the equipment is installed.

TABLE R403.6.1
WHOLE-HOUSE MECHANICAL VENTILATION SYSTEM FAN EFFICACY

FAN LOCATION	AIRFLOW RATE MINIMUM (CFM)	MINIMUM EFFICACY ^a (CFM/WATT)	AIRFLOW RATE MAXIMUM (CFM)
Range hoods	Any	2.8 cfm/watt	Any
In-line fan	Any	2.8 cfm/watt	Any
Bathroom, utility room	10	1.4 cfm/watt	<90
Bathroom, utility room	90	2.8 cfm/watt	Any

For SI: 1 cfm = 28.3 L/min.

a. When tested in accordance with HVI Standard 916

MANDATORY REQUIREMENTS - (Continued)

- ☐ **R403.7.1.1 Cooling equipment capacity.** Cooling only equipment shall be selected so that its total capacity is not less than the calculated total load but not more than 1.15 times greater than the total load calculated according to the procedure selected in Section 403.7, or the closest available size provided by the manufacturer's product lines. The corresponding latent capacity of the equipment shall not be less than the calculated latent load.
- The published value for AHRJ total capacity is a nominal, rating-test value and shall not be used for equipment sizing. Manufacturer's expanded performance data shall be used to select cooling-only equipment. This selection shall be based on the outdoor design dry-bulb temperature for the load calculation (or entering water temperature for water-source equipment), the blower CFM provided by the expanded performance data, the design value for entering wet-bulb temperature and the design value for entering dry-bulb temperature.

Design values for entering wet-bulb and dry-bulb temperatures shall be for the indoor dry bulb and relative humidity used for the load calculation and shall be adjusted for return side gains if the return duct(s) is installed in an unconditioned space.

Exceptions:

1. Attached single- and multiple-family residential equipment sizing may be selected so that its cooling capacity is less than the calculated total sensible load but not less than 80 percent of that load.
2. When signed and sealed by a Florida-registered engineer, in attached single- and multiple-family units, the capacity of equipment may be sized in accordance with good design practice.

R403.7.1.2 Heating equipment capacity.

- ☐ **R403.7.1.2.1 Heat pumps.** Heat pump sizing shall be based on the cooling requirements as calculated according to Section R403.7.1.1, and the heat pump total cooling capacity shall not be more than 1.15 times greater than the design cooling load even if the design heating load is 1.15 times greater than the design cooling load.
- ☐ **R403.7.1.2.2 Electric resistance furnaces.** Electric resistance furnaces shall be sized within 4 kW of the design requirements calculated according to the procedure selected in Section R403.7.1.
- ☐ **R403.7.1.2.3 Fossil fuel heating equipment.** The capacity of fossil fuel heating equipment with natural draft atmospheric burners shall not be less than the design load calculated in accordance with Section R403.7.1.
- ☐ **R403.7.1.3 Extra capacity required for special occasions.** Residences requiring excess cooling or heating equipment capacity on an intermittent basis, such as anticipated additional loads caused by major entertainment events, shall have equipment sized or controlled to prevent continuous space cooling or heating within that space by one or more of the following options:
1. A separate cooling or heating system is utilized to provide cooling or heating to the major entertainment areas.
 2. A variable capacity system sized for optimum performance during base load periods is utilized.

- ☐ **R403.8 Systems serving multiple dwelling units (Mandatory).** Systems serving multiple dwelling units shall comply with Sections C403 and C404 of the IECC—Commercial Provisions in lieu of Section R403.

- ☐ **R403.9 Snow melt and ice system controls (Mandatory)** Snow- and ice-melting systems, supplied through energy service to the building, shall include automatic controls capable of shutting off the system when the pavement temperature is above 50°F (10°C), and no precipitation is falling and an automatic or manual control that will allow shutoff when the outdoor temperature is above 40°F (4.8°C).

- ☐ **R403.10 Pools and permanent spa energy consumption (Mandatory).** The energy consumption of pools and permanent spas shall be in accordance with Sections R403.10.1 through R403.10.5.

- ☐ **R403.10.1 Heaters.** The electric power to heaters shall be controlled by a readily accessible on-off switch that is an integral part of the heater mounted on the exterior of the heater, or external to and within 3 feet (914 mm) of the heater. Operation of such switch shall not change the setting of the heater thermostat. Such switches shall be in addition to a circuit breaker for the power to the heater. Gas-fired heaters shall not be equipped with continuously burning ignition pilots.

- ☐ **R403.10.2 Time switches.** Time switches or other control methods that can automatically turn off and on according to a preset schedule shall be installed for heaters and pump motors. Heaters and pump motors that have built-in time switches shall be in compliance with this section.

Exceptions:

1. Where public health standards require 24-hour pump operation.
2. Pumps that operate solar- and waste-heat-recovery pool heating systems.
3. Where pumps are powered exclusively from on-site renewable generation.

- ☐ **R403.10.3 Covers.** Outdoor heated swimming pools and outdoor permanent spas shall be equipped with a vapor-retardant cover on or at the water surface or a liquid cover or other means proven to reduce heat loss.

Exception: Where more than 70 percent of the energy for heating, computed over an operation season, is from site-recovered energy, such as from a heat pump or solar energy source, covers or other vapor-retardant means shall not be required.

- ☐ **R403.10.4 Gas- and oil-fired pool and spa heaters.** All gas- and oil-fired pool and spa heaters shall have a minimum thermal efficiency of 82 percent for heaters manufactured on or after April 16, 2013, when tested in accordance with ANSI Z 21.56. Pool heaters fired by natural or LP gas shall not have continuously burning pilot lights.

☐ **R403.10.5 Heat pump pool heaters.** Heat pump pool heaters shall have a minimum COP of 4.0 when tested in accordance with AHRI 1160, Table 2, Standard Rating Conditions-Low Air Temperature. A test report from an independent laboratory is required to verify procedure compliance. Geothermal swimming pool heat pumps are not required to meet this standard.

☐ **R403.11 Portable spas (Mandatory)** The energy consumption of electric-powered portable spas shall be controlled by the requirements of APSP-14.

SECTION R404

ELECTRICAL POWER AND LIGHTING SYSTEMS

☐ **R404.1 Lighting equipment (Mandatory).** Not less than 75 percent of the lamps in permanently installed lighting fixtures shall be high-efficacy lamps or not less than 75 percent of the permanently installed lighting fixtures shall contain only high-efficacy lamps.

Exception: Low-voltage lighting.

R404.1.1 Lighting equipment (Mandatory) Fuel gas lighting systems shall not have continuously burning pilot lights.

2017 - AIR BARRIER AND INSULATION INSPECTION COMPONENT CRITERIA

**TABLE 402.4.1.1
AIR BARRIER AND INSULATION INSPECTION COMPONENT CRITERIA**

Project Name: 200191 Fender Street: City, State, Zip: Lake City , FL , 32025 Owner: Fender, Wilber & Sylvia Design Location: FL, Gainesville			Builder Name: Edgley Construction Permit Office: Permit Number: Jurisdiction:	CHECK
COMPONENT	AIR BARRIER CRITERIA	INSULATION INSTALLATION CRITERIA		
General requirements	A continuous air barrier shall be installed in the building envelope. The exterior thermal envelope contains a continuous air barrier. Breaks or joints in the air barrier shall be sealed.	Air-permeable insulation shall not be used as a sealing material.		
Ceiling/attic	The air barrier in any dropped ceiling/soffit shall be aligned with the insulation and any gaps in the air barrier shall be sealed. Access openings, drop down stairs or knee wall doors to unconditioned attic spaces shall be sealed.	The insulation in any dropped ceiling/soffit shall be aligned with the air barrier.		
Walls	The junction of the foundation and sill plate shall be sealed. The junction of the top plate and the top of exterior walls shall be sealed. Knee walls shall be sealed.	Cavities within corners and headers of frame walls shall be insulated by completely filling the cavity with a material having a thermal resistance of R-3 per inch minimum. Exterior thermal envelope insulation for framed walls shall be installed in substantial contact and continuous alignment with the air barrier.		
Windows, skylights and doors	The space between window/door jambs and framing, and skylights and framing shall be sealed.			
Rim joists	Rim joists shall include the air barrier.	Rim joists shall be insulated.		
Floors (including above-garage and cantilevered floors)	The air barrier shall be installed at any exposed edge of insulation.	Floor framing cavity insulation shall be installed to maintain permanent contact with the underside of subfloor decking, or floor framing cavity insulation shall be permitted to be in contact with the top side of sheathing, or continuous insulation installed on the underside of floor framing and extends from the bottom to the top of all perimeter floor framing members.		
Crawl space walls	Exposed earth in unvented crawl spaces shall be covered with a Class I vapor retarder with overlapping joints taped.	Where provided instead of floor insulation, insulation shall be permanently attached to the crawlspace		
Shafts, penetrations	Duct shafts, utility penetrations, and flue shafts opening to exterior or unconditioned space shall be sealed.			
Narrow cavities		Batts in narrow cavities shall be cut to fit, or narrow cavities shall be filled by insulation that on installation readily conforms to the available cavity spaces.		
Garage separation	Air sealing shall be provided between the garage and conditioned spaces.			
Recessed lighting	Recessed light fixtures installed in the building thermal envelope shall be sealed to the drywall.	Recessed light fixtures installed in the building thermal envelope shall be air tight and IC rated.		
Plumbing and wiring		Batt insulation shall be cut neatly to fit around wiring and plumbing in exterior walls, or insulation that on installation readily conforms to available space shall extend behind piping and wiring.		
Shower/tub on exterior wall	The air barrier installed at exterior walls adjacent to showers and tubs shall separate them from the showers and tubs.	Exterior walls adjacent to showers and tubs shall be insulated.		
Electrical/phone box on exterior walls	The air barrier shall be installed behind electrical or communication boxes or air-sealed boxes shall be installed.			
HVAC register boots	HVAC register boots that penetrate building thermal envelope shall be sealed to the sub-floor or drywall.			
Concealed sprinklers	When required to be sealed, concealed fire sprinklers shall only be sealed in a manner that is recommended by the manufacturer. Caulking or other adhesive sealants shall not be used to fill voids between fire sprinkler cover plates and walls or ceilings.			

a. In addition, inspection of log walls shall be in accordance with the provisions of ICC-400.

Envelope Leakage Test Report (Blower Door Test)

Residential Prescriptive, Performance or ERI Method Compliance

2017 Florida Building Code, Energy Conservation, 6th Edition

Jurisdiction:

Permit #:

Job Information

Builder: Edgley Construction

Community:

Lot: NA

Address:

City: Lake City

State: FL

Zip: 32025

Air Leakage Test Results

Passing results must meet either the Performance, Prescriptive, or ERI Method

☐ **PRESCRIPTIVE METHOD**-The building or dwelling unit shall be tested and verified as having an air leakage rate of not exceeding 7 air changes per hour at a pressure of 0.2 inch w.g. (50 Pascals) in Climate Zones 1 and 2.

☐ **PERFORMANCE or ERI METHOD**-The building or dwelling unit shall be tested and verified as having an air leakage rate of not exceeding the selected ACH(50) value, as shown on Form R405-2017 (Performance) or R406-2017 (ERI), section labeled as infiltration, sub-section ACH50.
ACH(50) specified on Form R405-2017-Energy Calc (Performance) or R406-2017 (ERI): 7.000

$$\frac{\text{CFM}(50)}{\text{Building Volume}} \times 60 \div 15435 = \text{ACH}(50)$$



PASS

☐ When ACH(50) is less than 3, Mechanical Ventilation installation must be verified by building department.

Method for calculating building volume:

☐ Retrieved from architectural plans

☒ Code software calculated

☐ Field measured and calculated

R402.4.1.2 Testing. Testing shall be conducted in accordance with ANSI/RESNET/ICC 380 and reported at a pressure of 0.2 inch w.g. (50 Pascals). Testing shall be conducted by either individuals as defined in Section 553.993(5) or (7), *Florida Statutes*, or individuals licensed as set forth in Section 489.105(3)(f), (g), or (i) or an approved third party. A written report of the results of the test shall be signed by the party conducting the test and provided to the code official. Testing shall be performed at any time after creation of all penetrations of the building thermal envelope.

During testing:

1. Exterior windows and doors, fireplace and stove doors shall be closed, but not sealed, beyond the intended weatherstripping or other infiltration control measures.
2. Dampers including exhaust, intake, makeup air, back draft and flue dampers shall be closed, but not sealed beyond intended infiltration control measures.
3. Interior doors, if installed at the time of the test, shall be open.
4. Exterior doors for continuous ventilation systems and heat recovery ventilators shall be closed and sealed.
5. Heating and cooling systems, if installed at the time of the test, shall be turned off.
6. Supply and return registers, if installed at the time of the test, shall be fully open.

Testing Company

Company Name: _____ Phone: _____

I hereby verify that the above Air Leakage results are in accordance with the 2017 6th Edition Florida Building Code Energy Conservation requirements according to the compliance method selected above.

Signature of Tester: _____ Date of Test: _____

Printed Name of Tester: _____

License/Certification #: _____ Issuing Authority: _____

Residential System Sizing Calculation

Summary

Fender, Wilber & Sylvia

Project Title:
200191 Fender

Lake City, FL 32025

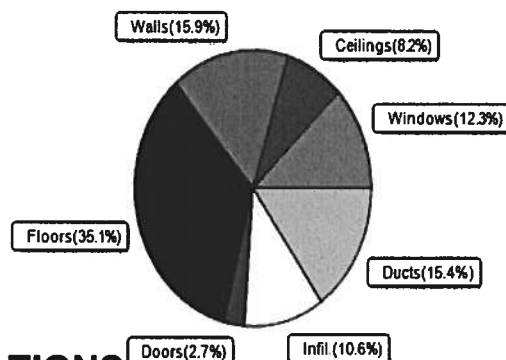
2020-02-27

Location for weather data: Gainesville, FL - Defaults: Latitude(29.7) Altitude(152 ft.) Temp Range(M)					
Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(51gr.)					
Winter design temperature(TMY3 99%)	30	F	Summer design temperature(TMY3 99%)	94	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	40	F	Summer temperature difference	19	F
Total heating load calculation	26789	Btuh	Total cooling load calculation	26784	Btuh
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh
Total (Electric Heat Pump)	119.5	32000	Sensible (SHR = 0.75)	105.0	24000
Heat Pump + Auxiliary(0.0kW)	119.5	32000	Latent	204.0	8000
			Total (Electric Heat Pump)	119.5	32000

WINTER CALCULATIONS

Winter Heating Load (for 1715 sqft)

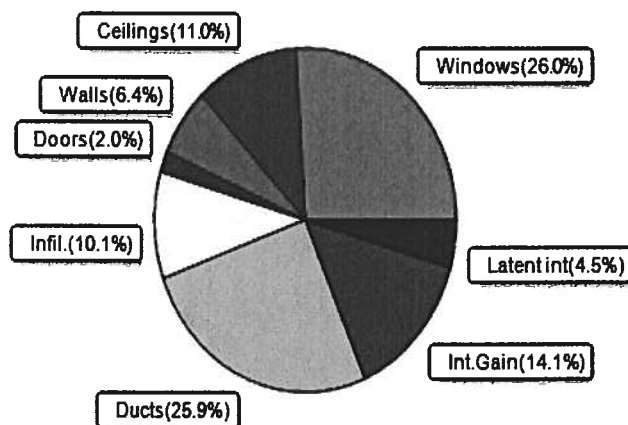
Load component			Load	
Window total	234	sqft	3282	Btuh
Wall total	1512	sqft	4257	Btuh
Door total	44	sqft	711	Btuh
Ceiling total	1715	sqft	2185	Btuh
Floor total	1715	sqft	9393	Btuh
Infiltration	65	cfm	2845	Btuh
Duct loss			4116	Btuh
Subtotal			26789	Btuh
Ventilation	0	cfm	0	Btuh
TOTAL HEAT LOSS			26789	Btuh



SUMMER CALCULATIONS

Summer Cooling Load (for 1715 sqft)

Load component			Load	
Window total	234	sqft	6973	Btuh
Wall total	1512	sqft	1722	Btuh
Door total	44	sqft	533	Btuh
Ceiling total	1715	sqft	2949	Btuh
Floor total			0	Btuh
Infiltration	49	cfm	1014	Btuh
Internal gain			3780	Btuh
Duct gain			5891	Btuh
Sens. Ventilation	0	cfm	0	Btuh
Blower Load			0	Btuh
Total sensible gain			22863	Btuh
Latent gain(ducts)			1039	Btuh
Latent gain(infiltration)			1682	Btuh
Latent gain(ventilation)			0	Btuh
Latent gain(internal/occupants/other)			1200	Btuh
Total latent gain			3921	Btuh
TOTAL HEAT GAIN			26784	Btuh



8th Edition

EnergyGauge® System Sizing
PREPARED BY: Evan Beamsley
DATE: 2020-02-27

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

Fender, Wilber & Sylvia

Project Title:
200191 Fender
Building Type: User

Lake City, FL 32025

2020-02-27

Reference City: Gainesville, FL (Defaults) Winter Temperature Difference: 40.0 F (TMY3 99%)
This calculation is for Worst Case. The house has been rotated 270 degrees.

Component Loads for Whole House

Window	Panes/Type	Frame	U	Orientation	Area(sqft)	X	HTM=	Load
1	2, NFRC 0.25	Metal	0.35	W	30.0		14.0	420 Btuh
2	2, NFRC 0.25	Metal	0.35	W	45.0		14.0	630 Btuh
3	2, NFRC 0.25	Metal	0.35	S	11.1		14.0	156 Btuh
4	2, NFRC 0.25	Metal	0.35	W	45.0		14.0	630 Btuh
5	2, NFRC 0.25	Metal	0.35	W	45.0		14.0	630 Btuh
6	2, NFRC 0.25	Metal	0.35	N	15.0		14.0	210 Btuh
7	2, NFRC 0.25	Metal	0.35	N	4.0		14.0	56 Btuh
8	2, NFRC 0.25	Metal	0.35	E	13.3		14.0	187 Btuh
9	2, NFRC 0.25	Metal	0.35	E	20.0		14.0	280 Btuh
10	2, NFRC 0.25	Metal	0.35	S	6.0		14.0	84 Btuh
Window Total					234.4(sqft)			3282 Btuh
Walls	Type	Ornt.	Ueff.	R-Value (Cav/Sh)	Area	X	HTM=	Load
1	Face Br - Wood	- Ext	(0.067)	19.0/0.0	77		2.66	204 Btuh
2	Face Br - Wood	- Ext	(0.067)	19.0/0.0	36		2.66	96 Btuh
3	Face Br - Wood	- Ext	(0.067)	19.0/0.0	84		2.66	224 Btuh
4	Face Br - Wood	- Ext	(0.067)	19.0/0.0	18		2.66	49 Btuh
5	Face Br - Wood	- Ext	(0.067)	19.0/0.0	75		2.66	200 Btuh
6	Face Br - Wood	- Ext	(0.067)	19.0/0.0	54		2.66	144 Btuh
7	Face Br - Wood	- Ext	(0.067)	19.0/0.0	89		2.66	236 Btuh
8	Face Br - Wood	- Ext	(0.067)	19.0/0.0	301		2.66	800 Btuh
9	Frame - Wood	- Adj	(0.089)	13.0/0.0	210		3.55	746 Btuh
10	Frame - Wood	- Adj	(0.089)	13.0/0.0	51		3.55	181 Btuh
11	Face Br - Wood	- Ext	(0.067)	19.0/0.0	30		2.66	79 Btuh
12	Face Br - Wood	- Ext	(0.067)	19.0/0.0	111		2.66	294 Btuh
13	Face Br - Wood	- Ext	(0.067)	19.0/0.0	102		2.66	271 Btuh
14	Face Br - Wood	- Ext	(0.067)	19.0/0.0	68		2.66	180 Btuh
15	Face Br - Wood	- Ext	(0.067)	19.0/0.0	209		2.66	555 Btuh
Wall Total					1512(sqft)			4257 Btuh
Doors	Type	Storm	Ueff.		Area	X	HTM=	Load
1	Insulated - Exterior,	n	(0.400)		7		16.0	107 Btuh
2	Insulated - Garage,	n	(0.400)		18		16.0	284 Btuh
3	Insulated - Exterior,	n	(0.400)		20		16.0	320 Btuh
Door Total					44(sqft)			711Btuh
Ceilings	Type/Color/Surface		Ueff.	R-Value	Area	X	HTM=	Load
1	Vented Attic/D/Shing		(0.032)	30.0/0.0	1715		1.3	2185 Btuh
Ceiling Total					1715(sqft)			2185Btuh
Floors	Type		Ueff.	R-Value	Size	X	HTM=	Load
1	Slab On Grade		(1.180)	0.0	199.0 ft(perim.)		47.2	9393 Btuh
Floor Total					1715 sqft			9393 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Fender, Wilber & Sylvia

Project Title:
200191 Fender
Building Type: User

Lake City, FL 32025

2020-02-27

	Envelope Subtotal:						19828 Btuh
Infiltration	Type	Wholehouse	ACH	Volume(cuft)	Wall Ratio	CFM=	
	Natural		0.25	15435	1.00	65.0	2845 Btuh
Duct load	Average sealed, R6.0, Supply(Att), Return(Att)					(DLM of 0.182)	4116 Btuh
All Zones	Sensible Subtotal All Zones						26789 Btuh

WHOLE HOUSE TOTALS

Totals for Heating	Subtotal Sensible Heat Loss						26789 Btuh
	Ventilation Sensible Heat Loss						0 Btuh
	Total Heat Loss						26789 Btuh

EQUIPMENT

1. Electric Heat Pump	#	32000 Btuh
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Key: Window types - NFRC (Requires U-Factor and Shading coefficient(SHGC) of glass as numerical values)
or - Glass as 'Clear' or 'Tint' (Uses U-Factor and SHGC defaults)
U - (Window U-Factor)
HTM - (ManualJ Heat Transfer Multiplier)



Version 8

System Sizing Calculations - Summer

Residential Load - Whole House Component Details

Fender, Wilber & Sylvia

Project Title:
200191 Fender

Lake City, FL 32025

2020-02-27

Reference City: Gainesville, FL Temperature Difference: 19.0F(TMY3 99%) Humidity difference: 51gr.
This calculation is for Worst Case. The house has been rotated 270 degrees.

Component Loads for Whole House

Window	Type*						Overhang		Window Area(sqft)			HTM		Load			
	Panes	SHGC	U	InSh	IS	Omt	Len	Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded				
1	2 NFRC	0.25, 0.35	No	No	W		1.5ft	4.0ft	30.0	0.0	30.0	12	31	923	Btuh		
2	2 NFRC	0.25, 0.35	No	No	W		17.5f	1.0ft	45.0	45.0	0.0	12	31	536	Btuh		
3	2 NFRC	0.25, 0.35	No	No	S		99.0f	1.0ft	11.1	11.1	0.0	12	14	132	Btuh		
4	2 NFRC	0.25, 0.35	No	No	W		13.5f	1.0ft	45.0	45.0	0.0	12	31	536	Btuh		
5	2 NFRC	0.25, 0.35	No	No	W		1.5ft	1.0ft	45.0	2.2	42.8	12	31	1342	Btuh		
6	2 NFRC	0.25, 0.35	No	No	N		1.5ft	1.0ft	15.0	0.0	15.0	12	12	179	Btuh		
7	2 NFRC	0.25, 0.35	No	No	N		1.5ft	1.0ft	4.0	0.0	4.0	12	12	48	Btuh		
8	2 NFRC	0.25, 0.35	No	No	E		8.5ft	1.0ft	13.3	12.1	1.2	12	31	182	Btuh		
9	2 NFRC	0.25, 0.35	No	No	E		1.5ft	1.0ft	20.0	1.0	19.0	12	31	597	Btuh		
10	2 NFRC	0.25, 0.35	No	No	S		1.5ft	1.0ft	6.0	6.0	0.0	12	14	71	Btuh		
	Excursion														2428	Btuh	
	Window Total								234 (sqft)							6973	Btuh
Walls	Type	U-Value			R-Value			Area(sqft)			HTM		Load				
1	Face Brick - Wood - Ext				0.07		19.0/0.0			76.5		1.0		78	Btuh		
2	Face Brick - Wood - Ext				0.07		19.0/0.0			36.0		1.0		37	Btuh		
3	Face Brick - Wood - Ext				0.07		19.0/0.0			84.0		1.0		86	Btuh		
4	Face Brick - Wood - Ext				0.07		19.0/0.0			18.2		1.0		19	Btuh		
5	Face Brick - Wood - Ext				0.07		19.0/0.0			75.0		1.0		77	Btuh		
6	Face Brick - Wood - Ext				0.07		19.0/0.0			54.0		1.0		55	Btuh		
7	Face Brick - Wood - Ext				0.07		19.0/0.0			88.5		1.0		91	Btuh		
8	Face Brick - Wood - Ext				0.07		19.0/0.0			300.5		1.0		308	Btuh		
9	Frame - Wood - Adj				0.09		13.0/0.0			210.2		1.7		355	Btuh		
10	Frame - Wood - Adj				0.09		13.0/0.0			51.0		1.7		86	Btuh		
11	Face Brick - Wood - Ext				0.07		19.0/0.0			29.7		1.0		30	Btuh		
12	Face Brick - Wood - Ext				0.07		19.0/0.0			110.5		1.0		113	Btuh		
13	Face Brick - Wood - Ext				0.07		19.0/0.0			102.0		1.0		105	Btuh		
14	Face Brick - Wood - Ext				0.07		19.0/0.0			67.5		1.0		69	Btuh		
15	Face Brick - Wood - Ext				0.07		19.0/0.0			208.5		1.0		214	Btuh		
	Wall Total								1512 (sqft)							1722	Btuh
Doors	Type	U-Value			R-Value			Area (sqft)			HTM		Load				
1	Insulated - Exterior									6.7		12.0		80	Btuh		
2	Insulated - Garage									17.8		12.0		213	Btuh		
3	Insulated - Exterior									20.0		12.0		240	Btuh		
	Door Total								44 (sqft)							533	Btuh
Ceilings	Type/Color/Surface	U-Value			R-Value			Area(sqft)			HTM		Load				
1	Vented Attic/DarkShingle				0.032		30.0/0.0			1715.0		1.72		2949	Btuh		
	Ceiling Total									1715 (sqft)				2949	Btuh		
Floors	Type	U-Value			R-Value			Size			HTM		Load				
1	Slab On Grade						0.0			1715 (ft-perimeter)		0.0		0	Btuh		
	Floor Total								1715.0 (sqft)							0	Btuh
	Envelope Subtotal:													12179		Btuh	

Manual J Summer Calculations

Residential Load - Component Details (continued)

Fender, Wilber & Sylvia

Project Title:
200191 Fender

Climate:FL_GAINESVILLE_REGIONAL_A

Lake City, FL 32025

2020-02-27

Infiltration	Type	Average ACH	Volume(cuft)	Wall Ratio	CFM=	Load
	Natural	0.19	15435	1	48.7	1014 Btuh
Internal gain		Occupants	Btuh/occupant		Appliance	Load
		6	X 230	+	2400	3780 Btuh
					Sensible Envelope Load:	16972 Btuh
Duct load	Average sealed, Supply(R6.0-Attic), Return(R6.0-Attic)				(DGM of 0.347)	5891 Btuh
					Sensible Load All Zones	22863 Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

Fender, Wilber & Sylvia

Project Title:
200191 Fender

Climate:FL_GAINESVILLE_REGIONAL_A

Lake City, FL 32025

2020-02-27

WHOLE HOUSE TOTALS

Whole House Totals for Cooling	Sensible Envelope Load All Zones	16972 Btuh
	Sensible Duct Load	5891 Btuh
	Total Sensible Zone Loads	22863 Btuh
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
	Total sensible gain	22863 Btuh
	Latent infiltration gain (for 51 gr. humidity difference)	1682 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	1039 Btuh
	Latent occupant gain (6.0 people @ 200 Btuh per person)	1200 Btuh
	Latent other gain	0 Btuh
	Latent total gain	3921 Btuh
	TOTAL GAIN	26784 Btuh

EQUIPMENT

1. Central Unit	#	32000 Btuh
-----------------	---	------------

*Key: Window types (Panels - Number and type of panes of glass)
 (SHGC - Shading coefficient of glass as SHGC numerical value)
 (U - Window U-Factor)
 (InSh - Interior shading device: none(No), Blinds(B), Draperies(D) or Roller Shades(R))
 - For Blinds: Assume medium color, half closed
 For Draperies: Assume medium weave, half closed
 For Roller shades: Assume translucent, half closed
 (IS - Insect screen: none(N), Full(F) or Half(½))
 (Omt - compass orientation)

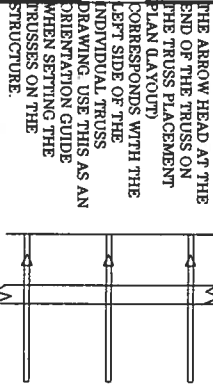
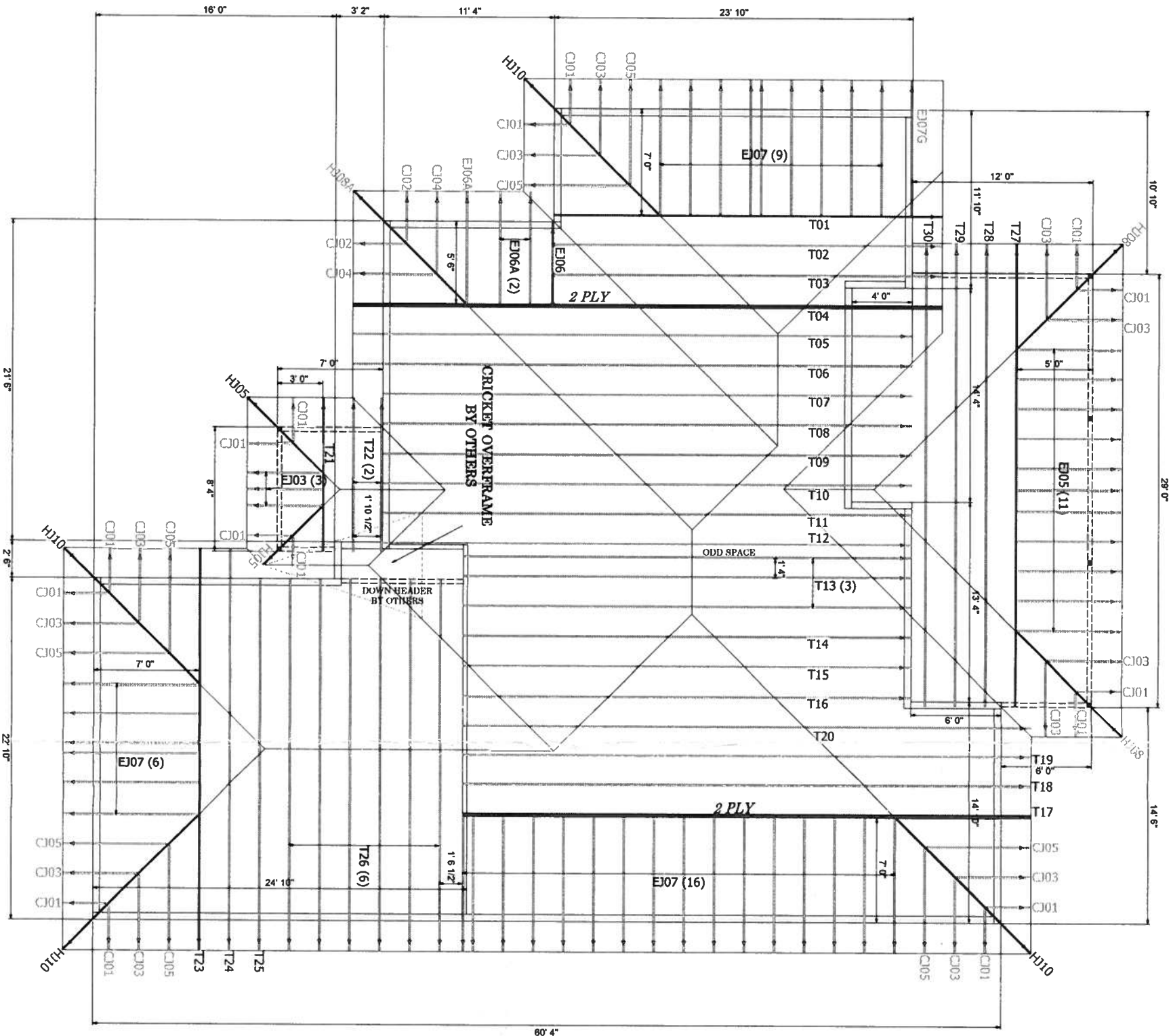


Version 8

Truss Connector Total List		
Manuf	Product	Qty
Simpson	HTU26	3

6/12 PITCH
24" PLUMB CUT O.H
3654. SQ FT

TRUSS TO TRUSS
CONNECTORS INCLUDED
WHERE SHOWN



General Notes:

- Per ANSI/TPI 1-2002 all "Truss to Wall" connections are the responsibility of the Building Designer, not the Truss Manufacturer.
- Use Manufacturer's specifications for all hanger connections unless noted otherwise.
- Trusses are to be 24" o.c. U.N.O.
- All hangers are to be Simpson or equivalent U.N.O.
- Use 10d x 1 1/2" Nail in hanger connections to single ply girders.
- Trusses are not designed to support brick U.N.O.
- Dimensions are Feet-Inches-Sixteenths

Notes:

No back charges will be accepted by Builders FirstSource unless approved in writing first.
850-836-4641

ACQ lumber is corrugate to truss plates. Any ACQ lumber that comes in contact with truss plates (i.e. scabbled on tails) must have an approved barrier applied first.

Refer to BCS3 B1 Summary Sheet-Guide for handling, installing and bracing of Metal Plate Connected Wood Truss prior to and during truss installation.

It is the responsibility of the Contractor to ensure of the proper orientation of the truss placement plans as to the construction documents and field conditions of the structure orientation. If a reversed or flipped layout is required it will be supplied at no extra cost by Builders FirstSource.

It is the responsibility of the Contractor to make sure the placement of trusses are adjusted for plumbing drops can rights, etc., so the trusses do not interfere with three type of items.

All common framed roof or floor systems must be designed as to NOT impose any loads on the floor trusses below. The floor trusses have not been designed to carry any additional loads from above.

This truss placement plan was not created by an engineer, but rather by the Builders FirstSource staff and is solely to be used as an installation guide and does not require a seal. Complete truss engineering and analysis can be found on the truss design drawings which may be scaled by the truss design engineer.

Gable end trusses require continuous bottom chord bearing. Refer to local codes for wall framing requirements.

Although all attempts have been made to do so, trusses may not be designed symmetrically. Please refer to the individual truss drawings and truss placement plans for proper orientation and placement.



Lake City
PHONE: 386-755-6894
FAX: 386-755-7973
Jacksonville
PHONE: 904-772-6100
FAX: 904-772-1973
Tallahassee
PHONE: 850-576-5177

Builder:
Edgeley Const.
Legal Address:
Fender Res.

Model:		
Custom		
Date	Drawn By	Original Ref #
2-19-2020	KLH	2256692
Floor 1 Job#	Floor 2 Job#	Roof Job #
		2256692



Lumber design values are in accordance with ANSI/TPI 1 section 6.3
These truss designs rely on lumber values established by others.

RE: 2256692 - Edgley Const. - Fender Res.

MiTek USA, Inc.

6904 Parke East Blvd.
Tampa, FL 33610-4115

Site Information:

Customer Info: Doug Edgley Const. Project Name: Fender Res. Model: Custom
Lot/Block: n/a Subdivision: n/a
Address: TBD, TBD
City: Columbia Cty State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: License #:
Address:
City: State:

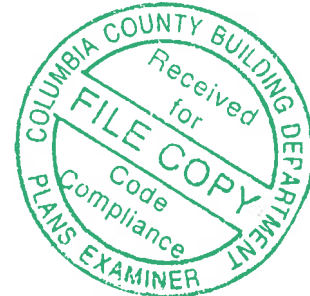
General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2017/TPI2014 Design Program: MiTek 20/20 8.2
Wind Code: ASCE 7-10 Wind Speed: 130 mph
Roof Load: 37.0 psf Floor Load: N/A psf

This package includes 45 individual, Truss Design Drawings and 0 Additional Drawings.

With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	T19453497	CJ01	2/19/20	23	T19453519	T08	2/19/20
2	T19453498	CJ02	2/19/20	24	T19453520	T09	2/19/20
3	T19453499	CJ03	2/19/20	25	T19453521	T10	2/19/20
4	T19453500	CJ04	2/19/20	26	T19453522	T11	2/19/20
5	T19453501	CJ05	2/19/20	27	T19453523	T12	2/19/20
6	T19453502	EJ03	2/19/20	28	T19453524	T13	2/19/20
7	T19453503	EJ05	2/19/20	29	T19453525	T14	2/19/20
8	T19453504	EJ06	2/19/20	30	T19453526	T15	2/19/20
9	T19453505	EJ06A	2/19/20	31	T19453527	T16	2/19/20
10	T19453506	EJ07	2/19/20	32	T19453528	T17	2/19/20
11	T19453507	EJ07G	2/19/20	33	T19453529	T18	2/19/20
12	T19453508	HJ05	2/19/20	34	T19453530	T19	2/19/20
13	T19453509	HJ08	2/19/20	35	T19453531	T20	2/19/20
14	T19453510	HJ08A	2/19/20	36	T19453532	T21	2/19/20
15	T19453511	HJ10	2/19/20	37	T19453533	T22	2/19/20
16	T19453512	T01	2/19/20	38	T19453534	T23	2/19/20
17	T19453513	T02	2/19/20	39	T19453535	T24	2/19/20
18	T19453514	T03	2/19/20	40	T19453536	T25	2/19/20
19	T19453515	T04	2/19/20	41	T19453537	T26	2/19/20
20	T19453516	T05	2/19/20	42	T19453538	T27	2/19/20
21	T19453517	T06	2/19/20	43	T19453539	T28	2/19/20
22	T19453518	T07	2/19/20	44	T19453540	T29	2/19/20

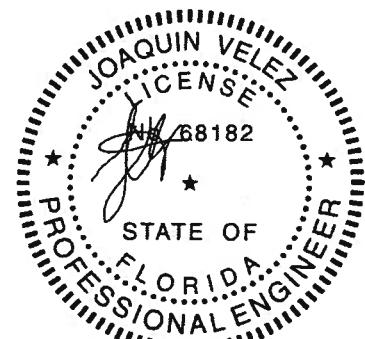


The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Jacksonville.

Truss Design Engineer's Name: Velez, Joaquin

My license renewal date for the state of Florida is February 28, 2021.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Joaquin Velez PE No. 68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

February 19, 2020

Velez, Joaquin

1 of 2



RE: 2256692 - Edgley Const. - Fender Res.

MiTek USA, Inc.

6904 Parke East Blvd.
Tampa, FL 33610-4115

Site Information:

Customer Info: Doug Edgley Const. Project Name: Fender Res. Model: Custom

Lot/Block: n/a Subdivision: n/a

Address: TBD, TBD

City: Columbia Cty

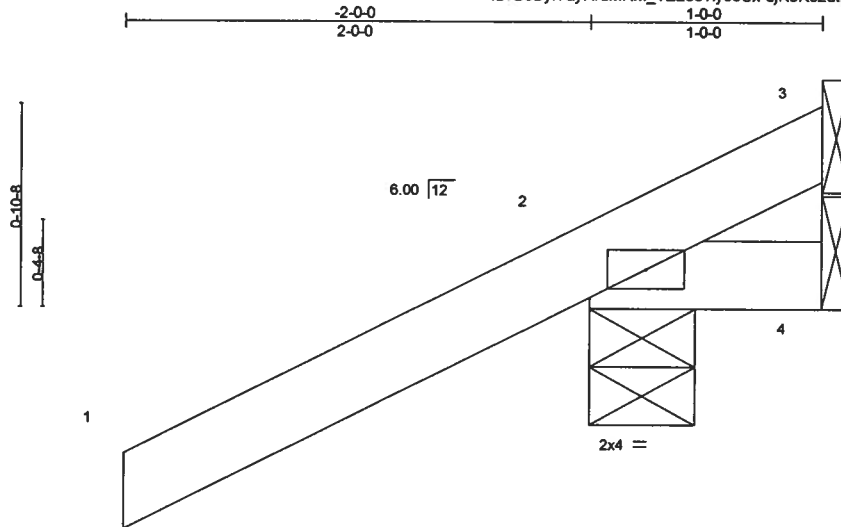
State: FL

No.	Seal#	Truss Name	Date
45	T19453541	T30	2/19/20

Job 2256692	Truss CJ01	Truss Type Jack-Open	Qty 16	Ply 1	Edgley Const. - Fender Res.	T19453497
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:15:54 2020 Page 1
ID:G3ByfYayRrdMRM_TE2867iye0Sx-djK6K52dtBYzhL3xJFTX8NGhPAobmLMU4Vb0zjdrZ



Scale = 1:9.5

LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc)	V/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.25	TC 0.25	Vert(LL)	-0.00 2	>999	240	MT20	244/190
TCDL 7.0	Lumber DOL 1.25	BC 0.01	Vert(CT)	-0.00 2	>999	180		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT)	-0.00 3	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014	Matrix-P					Weight: 7 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

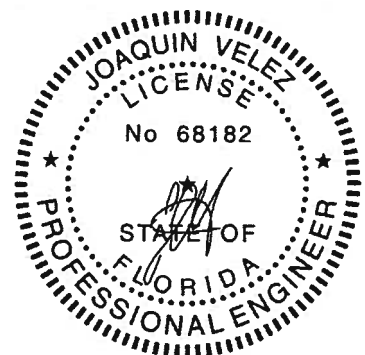
BRACING-
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 2=265/0-5-8, 4=9/Mechanical, 3=95/Mechanical
Max Horz 2=53(LC 12)
Max Uplift 2=126(LC 12), 4=6(LC 8), 3=95(LC 1)
Max Grav 2=265(LC 1), 4=19(LC 3), 3=50(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (7)

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 3 except (jt=lb) 2=126.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Joaquin Velez PE No.68182
MITek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

February 19,2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MH-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



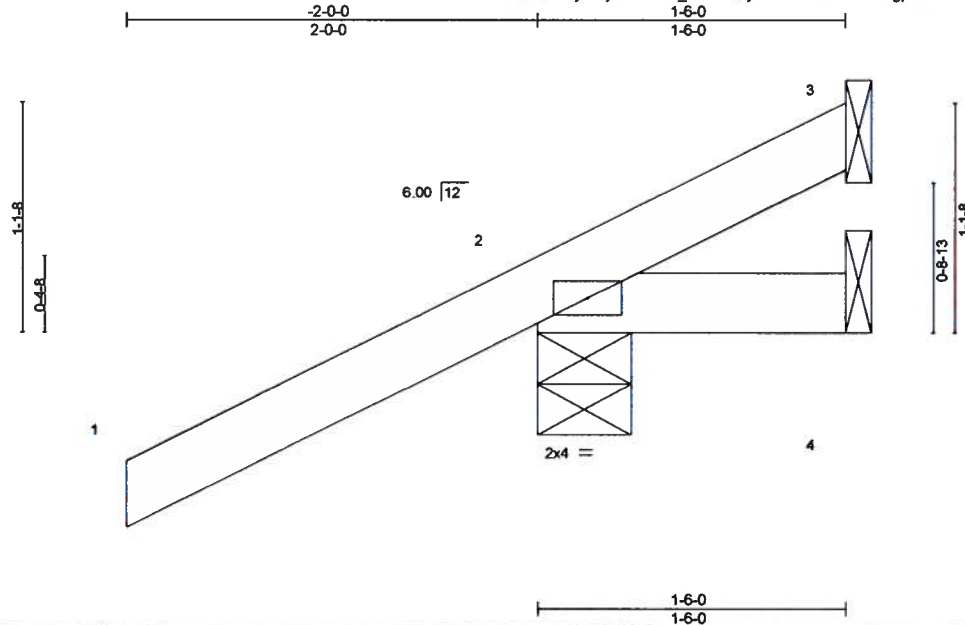
6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	Edgley Const. - Fender Res.	T19453498
2256692	CJ02	JACK-OPEN	2	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8 240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:15:55 2020 Page 1

ID: G3BylYayRdMRM_TE286?ye0Sx-5vtUYR3FeVgpJuYYceqU?kgY05kCX20Vb8p28SzdrY



LOADING (psf)	SPACING-		CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	2-0-0	TC 0.25	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.02	Vert(CT)	-0.00	2	>999	180		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(CT)	-0.00	3	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-P						Weight: 8 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING-

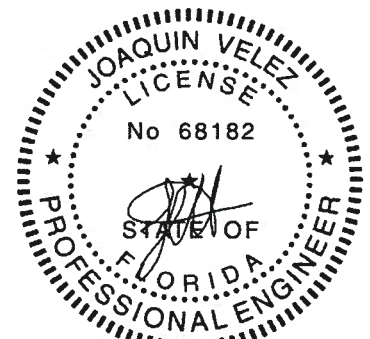
TOP CHORD Structural wood sheathing directly applied or 1-6-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=39/Mechanical, 2=242/0-5-8, 4=14/Mechanical
Max Horz 2=62(LC 12)
Max Uplift 3=39(LC 1), 2=105(LC 12)
Max Grav 3=26(LC 8), 2=242(LC 1), 4=29(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (7)

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3 except (ft=lb) 2=105.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

February 19,2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

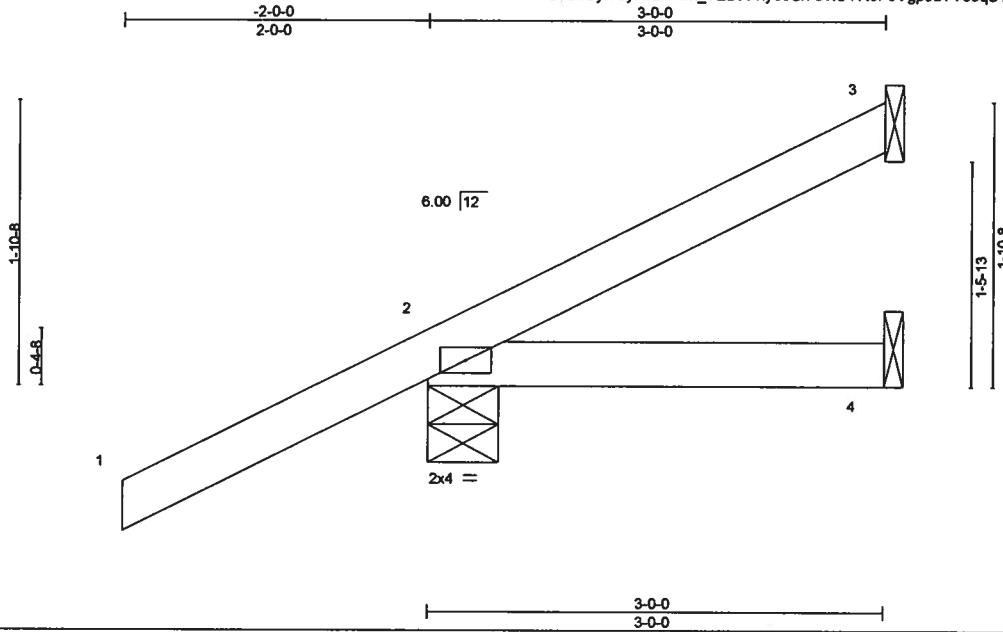


6904 Parke East Blvd.
Tampa, FL 36610

Job 2256692	Truss CJ03	Truss Type JACK-OPEN	Qty 12	Ply 1	Edgley Const. - Fender Res.	T19453499
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:15:55 2020 Page 1
ID: G3ByIYayRrdMRM_TE286?Iye0Sx-5vUYR3FeVgpJuYYceqU7kgXM5jFX20Vb8p28SzdY



Scale = 1:14.6

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.29	Vert(LL)	0.01	2-4	>999	240	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.08	Vert(CT)	-0.01	2-4	>999	180	
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(CT)	-0.00	3	n/a	n/a	
BCDL 10.0	Code FBC2017/TPI2014		Matrix-P						
								Weight: 13 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

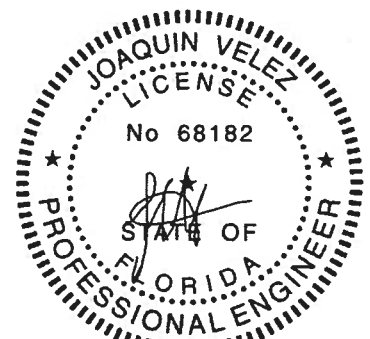
BRACING-
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=24/Mechanical, 2=270/0-5-8, 4=27/Mechanical
Max Horz 2=90(LC 12)
Max Uplift 3=30(LC 12), 2=99(LC 12), 4=17(LC 8)
Max Grav 3=24(LC 1), 2=270(LC 1), 4=54(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (7)

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2, 4.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 8634
6904 Parke East Blvd. Tampa FL 33610
Date:

February 19,2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

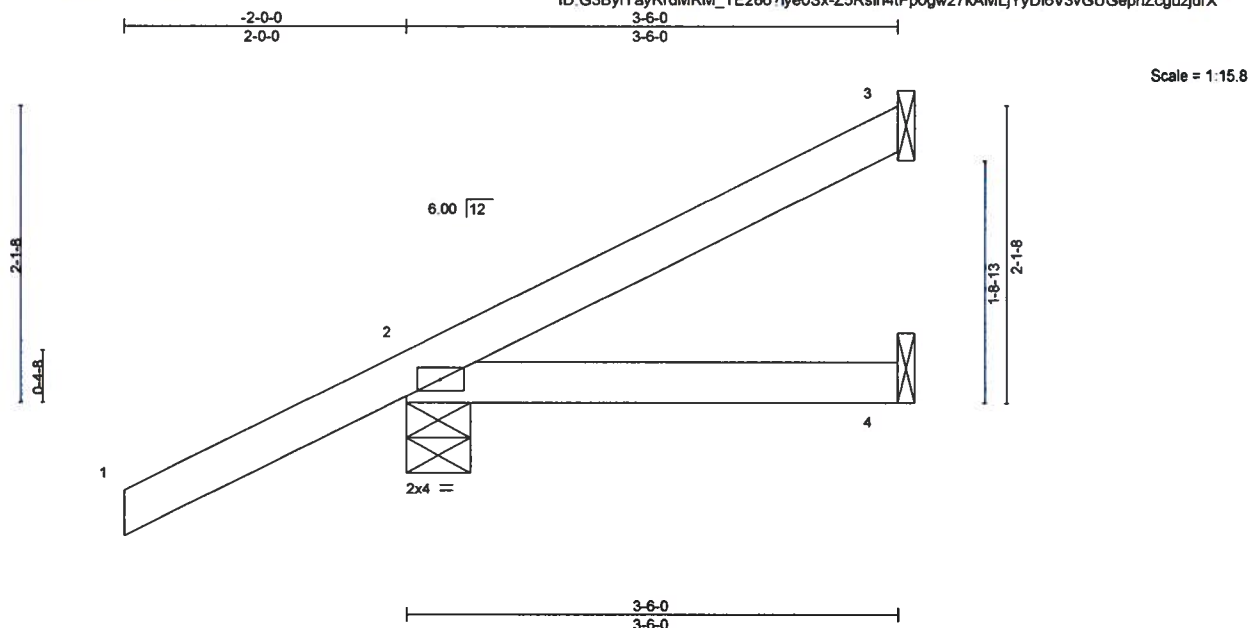


6904 Parke East Blvd.
Tampa, FL 33610

Job 2256692	Truss CJ04	Truss Type JACK-OPEN	Qty 2	Ply 1	Edgley Const. - Fender Res. T19453500
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:15:56 2020 Page 1
ID: G3By/YayRdMRM_TE286?ye0Sx-25Rsln4tPpogw27kAMLjYyDi6V3vGUGepnZcguzjdrX



LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.29	Vert(LL)	-0.01	2-4	>999	240	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.12	Vert(CT)	-0.01	2-4	>999	180	
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(CT)	-0.00	3	n/a	n/a	
BCDL 10.0	Code FBC2017/TPI2014		Matrix-P						
								Weight: 15 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING-

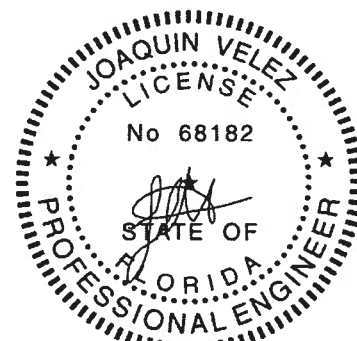
TOP CHORD Structural wood sheathing directly applied or 3-6-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=45/Mechanical, 2=281/0-5-8, 4=32/Mechanical
Max Horz 2=68(LC 12)
Max Uplift 3=-30(LC 12), 2=-48(LC 12)
Max Grav 3=45(LC 1), 2=281(LC 1), 4=64(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (7)

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

February 19,2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.

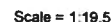
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



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Builders FirstSource, Jacksonville, FL - 32244.

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:15:57 2020 Page 1
ID:G3ByIYayRrdMRM_TE286?iye0Sx-1I?Ez64VA6wXYCiwk3ty49lruMk?xWn2RI9CKzidRW



Weight: 19 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING-

TOP CHORD	Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

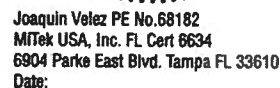
(lb/size) 3=99/Mechanical, 2=323/0-5-8, 4=47/Mechanical
Max Horz 2=88(LC 12)
Max Uplift 3=53(LC 12), 2=49(LC 12)
Max Grav 3=99(LC 1), 2=323(LC 1), 4=94(LC 3)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES. (7)

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



February 19, 2020



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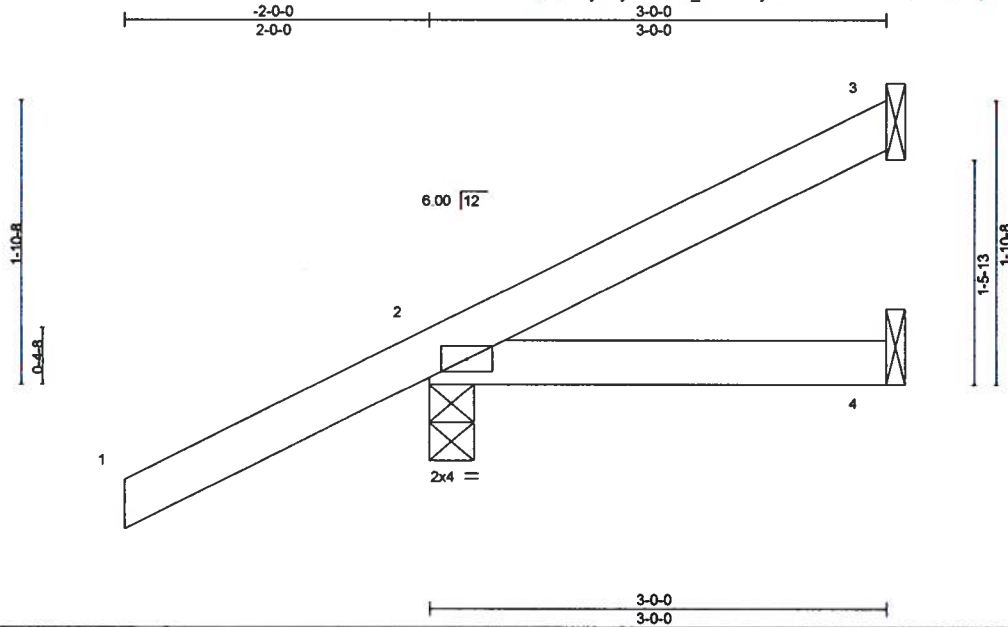
Design valid for use only with MitefK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCS1 Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	Edgley Const. - Fender Res.	T19453502
2256692	EJ03	Jack-Open	3	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:15:58 2020 Page 1
ID G3ByIYayRdMRM_TE2867iye0Sx-WUJzAS57xQ2OAMH7ImOBdNI2uIskOmxH52InzjdrV



Scale = 1:14.6

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.28	Vert(LL)	0.01	2-4	>999	240	MT20
TCDL 7.0	Lumber DOL	1.25	BC 0.09	Vert(CT)	-0.01	2-4	>999	180	244/190
BCLL 0.0	Rep Stress Incr	YES	WB 0.00	Horz(CT)	-0.00	3	n/a	n/a	
BCDL 10.0	Code FBC2017/TPI2014		Matrix-P						
								Weight: 13 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING-

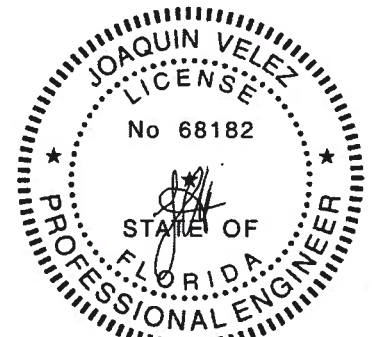
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=31/Mechanical, 2=264/0-3-8, 4=28/Mechanical
Max Horz 2=90(LC 12)
Max Uplift 3=-33(LC 12), 2=-96(LC 12), 4=-17(LC 8)
Max Grav 3=31(LC 1), 2=264(LC 1), 4=56(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (7)

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Endc., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2, 4.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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Date:

February 19,2020

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Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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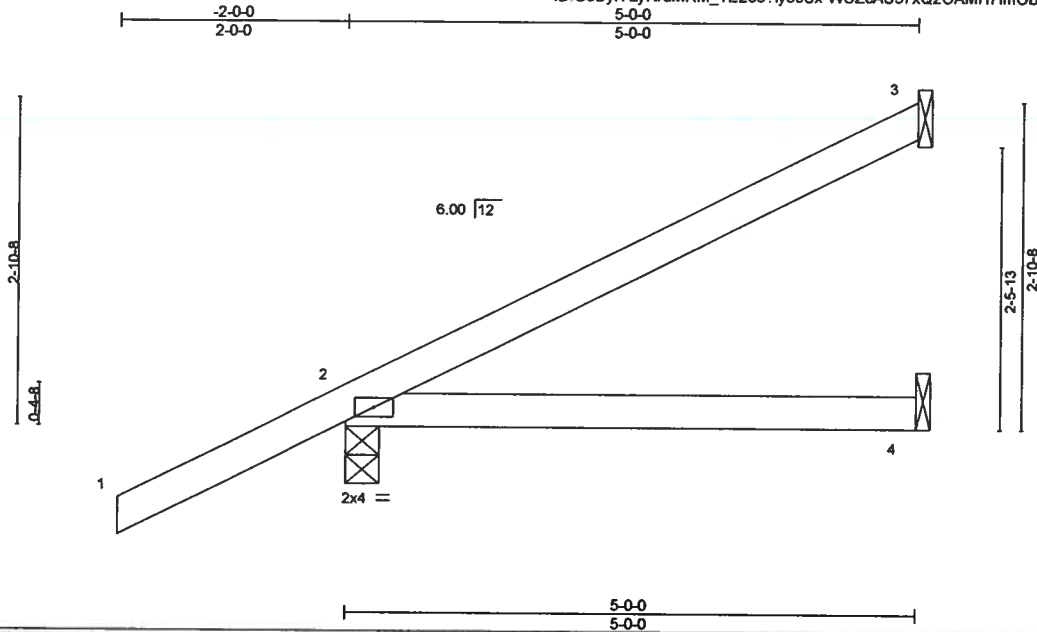
Job 2256692	Truss EJ05	Truss Type Jack-Open	Qty 11	Ply 1	Edgley Const. - Fender Res.	T19453503
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:15:58 2020 Page 1

ID:G3BylYayRrdMRM_TE2867iye0Sx-WUJZcAS57xQ2OAMH7ImOBdNI2ulipkOmxH52ilnzdrV

Job Reference (optional)



Scale = 1:19.5

LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.25	TC 0.28	Vert(LL) 0.06	2-4	>989	240	MT20	244/190
TCDL 7.0	Lumber DOL 1.25	BC 0.28	Vert(CT) -0.06	2-4	>909	180		
BCLL 0.0	Rep Stress Incr YES	WB 0.00	Horz(CT) -0.00	3	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014	Matrix-P						
							Weight: 19 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=103/Mechanical, 2=319/0-3-8, 4=48/Mechanical
Max Horz 2=129(LC 12)
Max Uplift 3=-83(LC 12), 2=-100(LC 12), 4=-30(LC 8)
Max Grav 3=103(LC 1), 2=319(LC 1), 4=96(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (7)

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (jt=lb) 2=100.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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MiTek USA, Inc. FL Cert 6634
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Date:

February 19,2020

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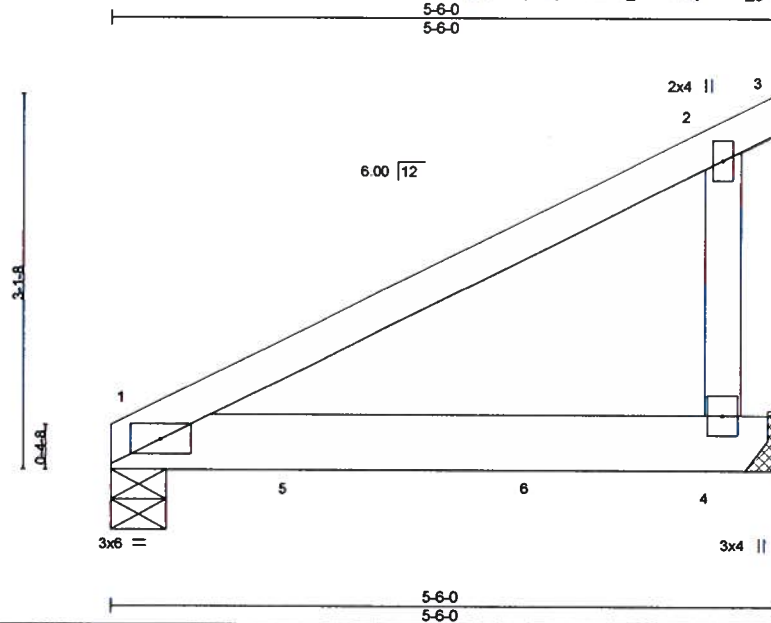
6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	Edgley Const. - Fender Res.	T19453504
2256692	EJ06	JACK-CLOSED GIRDER	1	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:15:59 2020 Page 1

ID: G3ByIYayRdMRM_TE286?iye0Sx-_g7?Oo6likAFnWJrUvQAarBCi_TTr04WnGHDzjdrU



Scale = 1:18.4

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.43	Vert(LL)	-0.07	1-4	>818	240	MT20
TCDL 7.0	Lumber DOL	1.25	BC 0.51	Vert(CT)	-0.13	1-4	>432	180	244/190
BCLL 0.0	Rep Stress Incr	NO	WB 0.00	Horz(CT)	0.00	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-P						
								Weight: 25 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP M 26
WEBS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 5-6-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 1=1052/0-5-8, 4=1020/Mechanical
Max Horz 1=104(LC 8)
Max Uplift 1=140(LC 8), 4=206(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (9)

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=140, 4=206.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 845 lb down and 118 lb up at 1-6-12, and 845 lb down and 128 lb up at 3-6-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=54, 2-3=54, 1-4=20

Concentrated Loads (lb)

Vert: 5=845(B) 6=845(B)



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MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

February 19,2020

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Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

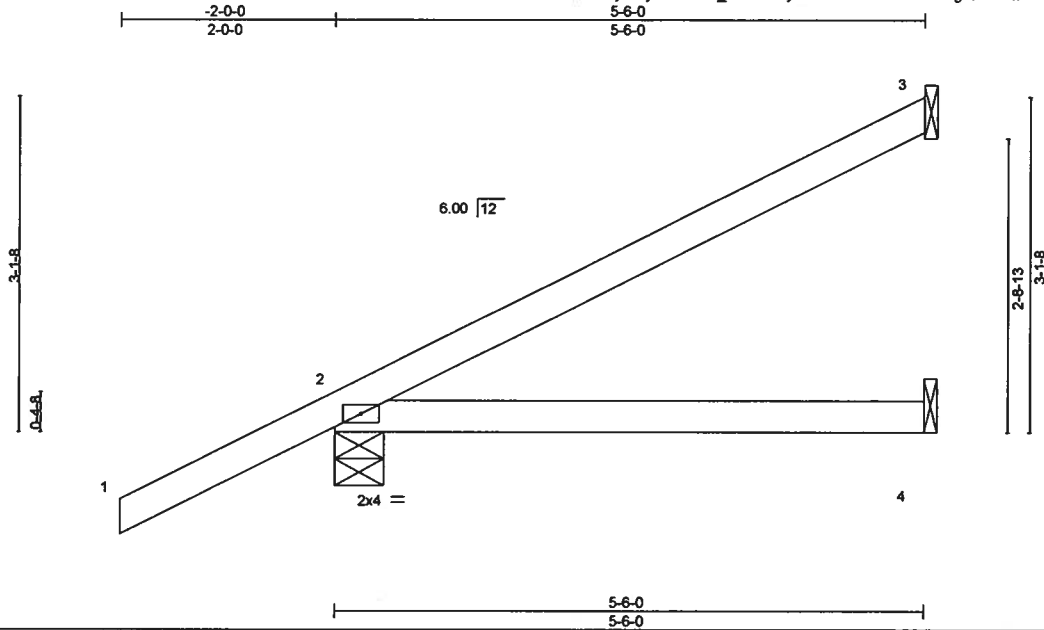
MiTek

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Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	Edgley Const. - Fender Res.	T19453505
2256692	EJ06A	Jack-Open	3	1	Job Reference (optional)	

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8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:00 2020 Page 1
ID: G3BylYayRrdMRM_TE2867iye0Sx-StnNb87NT116PgQVPBQfioNOp6NOCIFEkPXpfzdrT



LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.31	Vert(LL)	-0.04	2-4	>999	240	MT20
TCDL 7.0	Lumber DOL	1.25	BC 0.34	Vert(CT)	-0.09	2-4	>708	180	244/190
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(CT)	-0.00	3	n/a	n/a	
BCDL 10.0	Code FBC2017/TPI2014		Matrix-P						
								Weight: 21 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

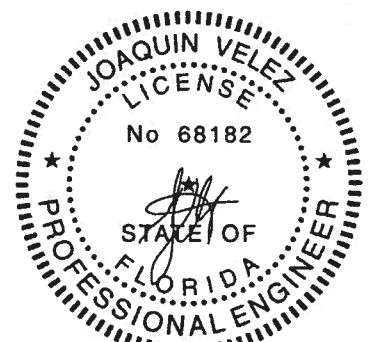
BRACING-
TOP CHORD Structural wood sheathing directly applied or 5-6-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=115/Mechanical, 2=339/0-5-8, 4=52/Mechanical
Max Horz 2=139(LC 12)
Max Uplift 3=93(LC 12), 2=104(LC 12)
Max Grav 3=115(LC 1), 2=339(LC 1), 4=104(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (7)

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3 except (jt=lb) 2=104.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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Date:

February 19,2020

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Job	Truss	Truss Type	Qty	Ply	Edgley Const. - Fender Res.	T19453506
2256692	EJ07	Jack-Open	31	1		

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8 240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:00 2020 Page 1
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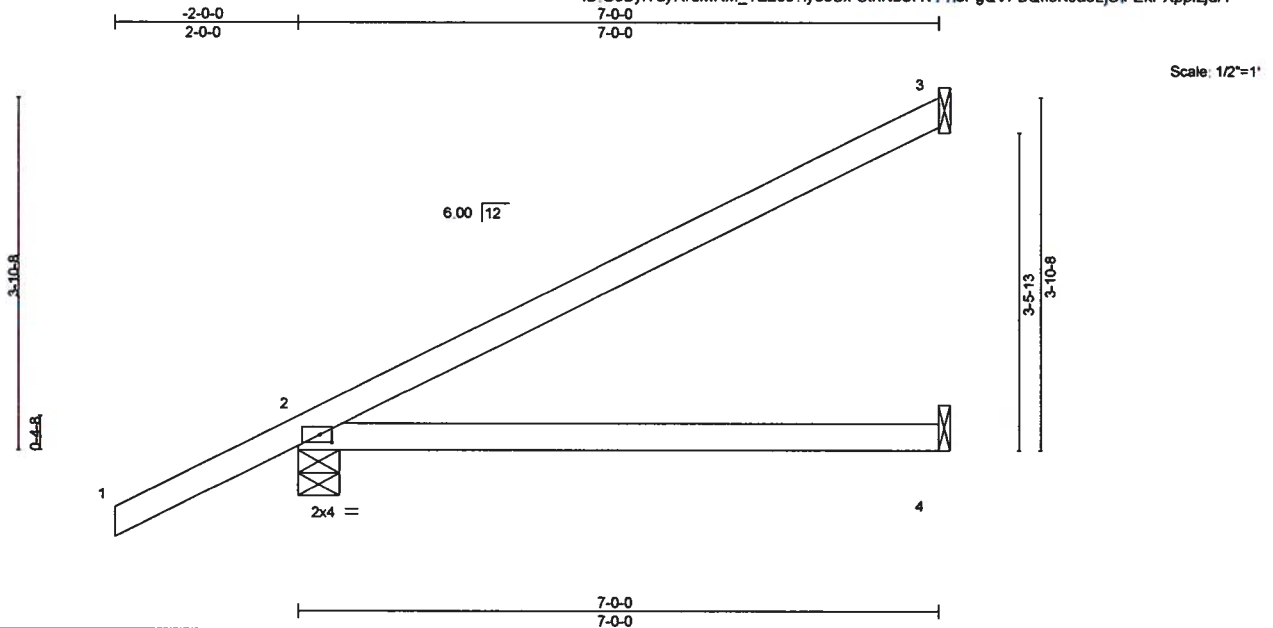


Plate Offsets (X,Y)-- [2:0-1-9,0-1-0]										
LOADING (psf)		SPACING- 2-0-0		CSI.		DEFL. in (loc) l/defl L/d		PLATES GRIP		
TCLL	20.0	Plate Grip DOL	1.25	TC	0.58	Vert(LL)	-0.10 2-4	>836	240	MT20 244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.51	Vert(CT)	-0.20 2-4	>395	180	
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.00 3	n/a	n/a	
BCDL	10.0	Code FBC2017/TPI2014		Matrix-S						
								Weight: 26 lb	FT = 20%	

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

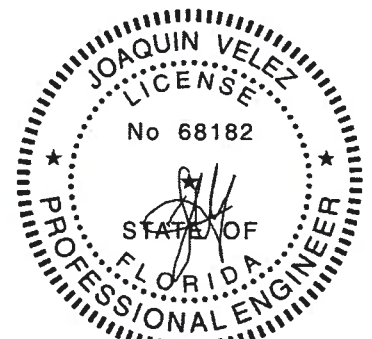
BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=157/Mechanical, 2=389/0-5-8, 4=71/Mechanical
Max Horz 2=115(LC 12)
Max Uplift 3=73(LC 12), 2=52(LC 12)
Max Grav 3=157(LC 1), 2=389(LC 1), 4=125(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (7)

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

February 19,2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd.
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	Edgley Const. - Fender Res.	T19453507
2256692	EJ07G	GABLE COMMON	1	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:01 2020 Page 1

ID:G3ByYayRdMRM_TE286?iye0Sx-w3FioU70ELQz1p?hzvuxF?waeWmkxIsNz3GML6zjdrS



Scale = 1:22.4

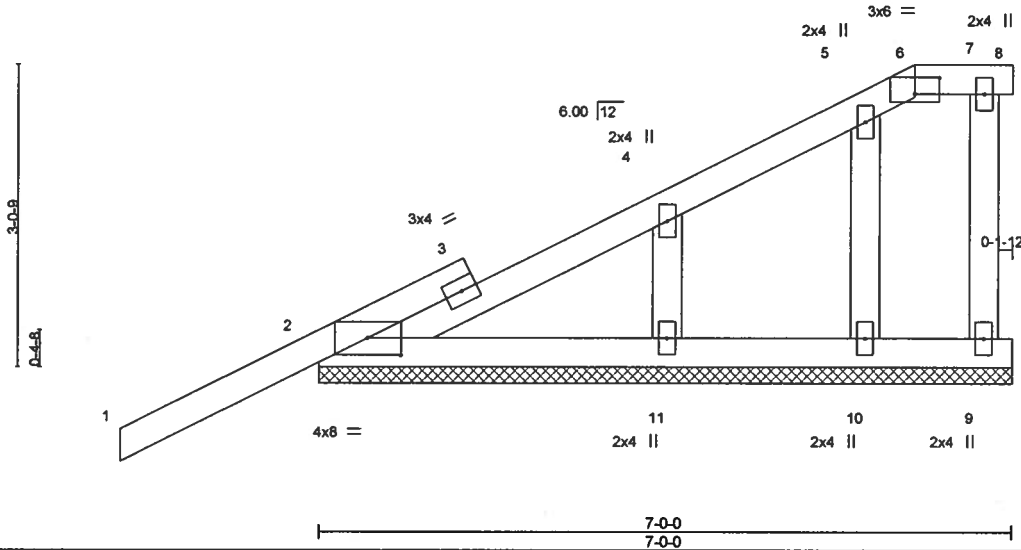


Plate Offsets (X,Y)- [2:0-4-0,0-2-1], [6:0-3-0,0-2-0]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.24	Vert(LL)	0.00	1	n/r	120	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.08	Vert(CT)	-0.01	1	n/r	120		
BCLL 0.0	Rep Stress Incr	YES	WB 0.04	Horz(CT)	-0.00	8	n/a	n/a		
BCDL 10.0	Code	FBC2017/TPI2014	Matrix-S						Weight: 36 lb	FT = 20%

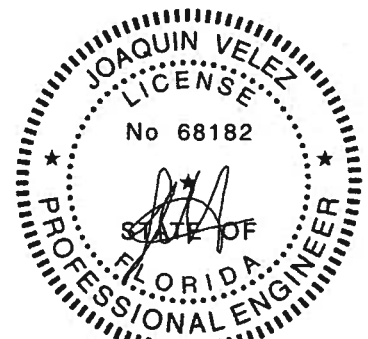
LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 7-0-0.
(lb) - Max Horz 2=137(LC 12)
Max Uplift All uplift 100 lb or less at joint(s) 2, 8, 9, 11, 10
Max Grav All reactions 250 lb or less at joint(s) 8, 9, 11, 10 except 2=254(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-** (11)
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - 4) Provide adequate drainage to prevent water ponding.
 - 5) Gable requires continuous bottom chord bearing.
 - 6) Gable studs spaced at 2-0-0 oc.
 - 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 9) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8, 9, 11, 10.
 - 11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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Date:

February 19,2020

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Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

MiTek

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Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	Edgley Const. - Fender Res.	T19453508
2256692	HJ05	Diagonal Hip Girder	2	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Feb 7 2020 Mitek Industries, Inc. Wed Feb 19 08:16:02 2020 Page 1
ID: G3ByfYayRrdMRM_TE286?ye0Sx-OFo70q8e?YqezauXcS7nDT9w4wgCIWCj0wuYzjdrR



Scale = 1:14.4

LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.39	Vert(LL)	-0.01	2-4	>999	MT20	244/190
TCDL 7.0	Plate Grip DOL 1.25	BC 0.21	Vert(CT)	-0.03	2-4	>999		
BCLL 0.0	Lumber DOL 1.25	WB 0.00	Horz(CT)	-0.00	3	n/a		
BCDL 10.0	Rep Stress Incr NO	Matrix-P						
	Code FBC2017/TPI2014						Weight: 18 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP M 31
BOT CHORD 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 4-2-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=17/Mechanical, 2=312/0-4-15, 4=40/Mechanical
Max Horz 2=108(LC 4)
Max Uplift 3=61(LC 19), 2=217(LC 4), 4=25(LC 4)
Max Grav 3=55(LC 35), 2=312(LC 1), 4=79(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (9)

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (it=lb) 2=217.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 101 lb down and 171 lb up at 1-5-4, and 101 lb down and 171 lb up at 1-5-4 on top chord, and 1 lb down and 9 lb up at 1-5-4, and 1 lb down and 9 lb up at 1-5-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard

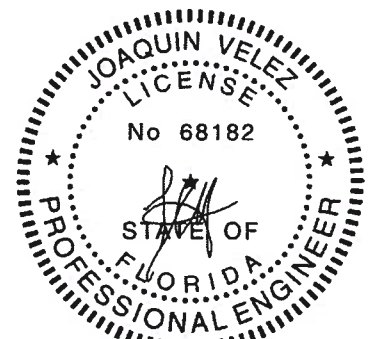
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=54, 2-4=20

Concentrated Loads (lb)

Vert: 5=89(F=44, B=44)



Joaquin Velez PE No.68182
Mitek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

February 19,2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

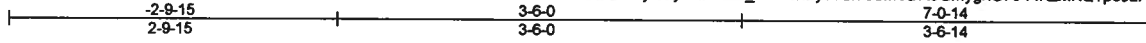
MITEK

6904 Parke East Blvd.
Tampa, FL 33610

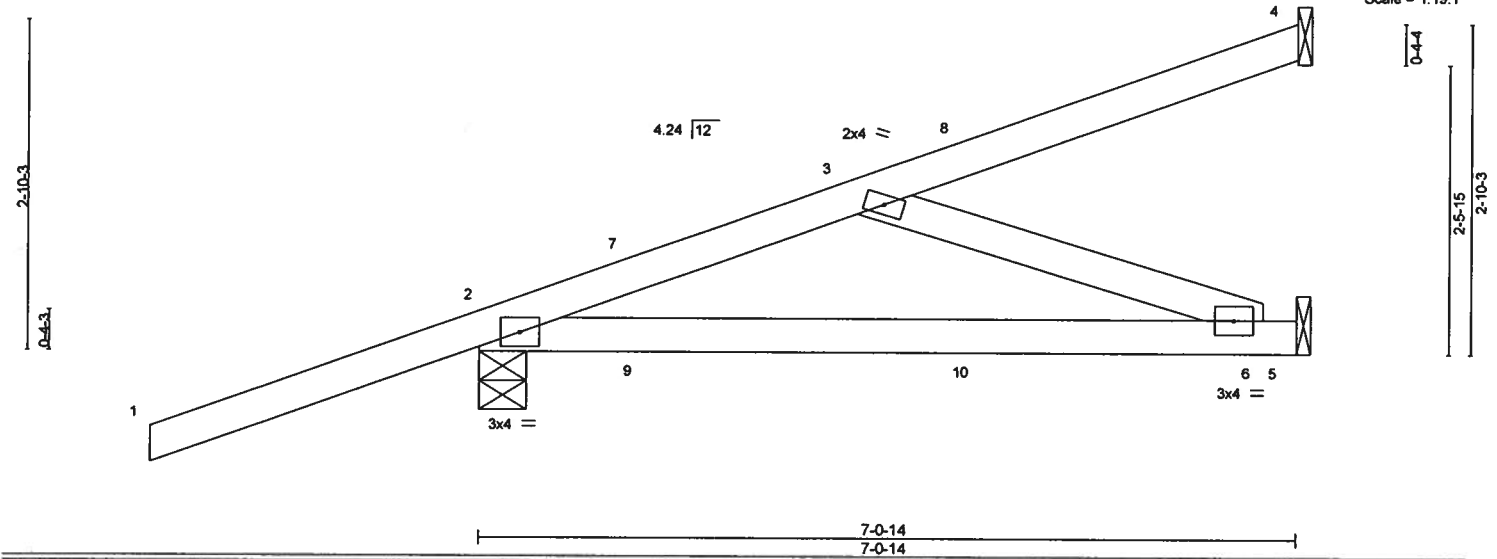
Job 2256692	Truss HJ08	Truss Type Diagonal Hip Girder	Qty 2	Ply 1	Edgley Const. - Fender Res.	T19453509
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:03 2020 Page 1
ID: G3ByIYayRrdMRM_TE2867iye0Sx-sSMVDA9GmyghG7944KzMKQ?poJLPPehgQNITQ_zdrQ



Scale = 1:19.1



LOADING (psf)	SPACING-		CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.25		TC 0.65	Vert(LL) -0.10	2-6	>853	240		MT20	244/190
TCDL 7.0	Lumber DOL 1.25		BC 0.51	Vert(CT) -0.18	2-6	>461	180			
BCLL 0.0 *	Rep Stress Incr NO		WB 0.08	Horz(CT) 0.00	5	n/a	n/a			
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S							
									Weight: 32 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. (lb/size) 4=96/Mechanical, 2=380/0-4-15, 5=103/Mechanical
Max Horz 2=147(LC 4)
Max Uplift 4=66(LC 4), 2=241(LC 4), 5=68(LC 5)
Max Grav 4=109(LC 19), 2=380(LC 1), 5=137(LC 34)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-390/66
BOT CHORD 2-6=-116/283
WEBS 3-6=-299/123

NOTES- (9)

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 5 except (it=lb) 2=241.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 101 lb down and 171 lb up at 1-5-4, 101 lb down and 171 lb up at 1-5-4, and 80 lb down and 18 lb up at 4-3-4, and 80 lb down and 18 lb up at 4-3-4 on top chord, and 1 lb down and 9 lb up at 1-5-4, 1 lb down and 9 lb up at 1-5-4, and 19 lb down and 25 lb up at 4-3-4, and 19 lb down and 25 lb up at 4-3-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard

- 1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 2-5=-20
Concentrated Loads (lb)
Vert: 7=89(F=44, B=44)



Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

February 19,2020

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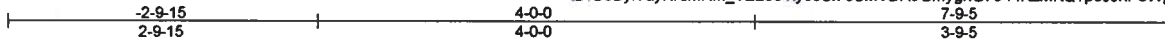
MiTek

6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	Edgley Const. - Fender Res.	T19453510
2256692	HJ08A	Diagonal Hip Girder	1	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:03 2020 Page 1
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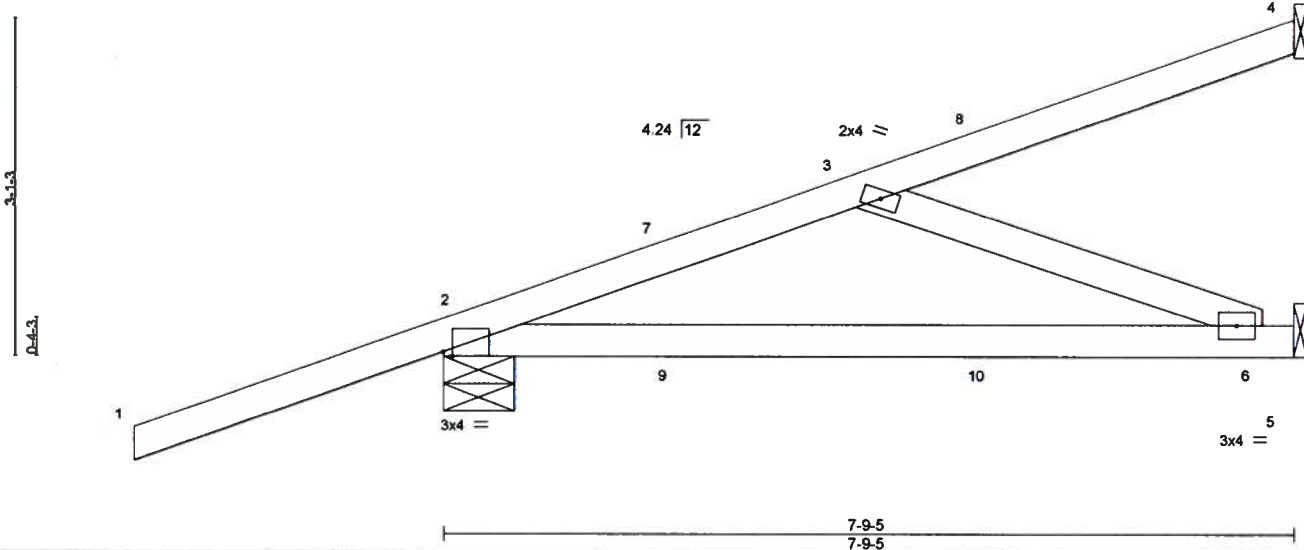


Plate Offsets (X,Y) [2:0-1-0,Edge]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.70	Vert(LL)	-0.14	2-6	>616	240	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.69	Vert(CT)	-0.27	2-6	>324	180		
BCLL 0.0	Rep Stress Incr	NO	WB 0.10	Horz(CT)	0.00	5	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S							
									Weight: 34 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(lb/size) 4=99/Mechanical, 2=438/0-7-12, 5=128/Mechanical
Max Horz 2=156(LC 4)
Max Uplift 4=-73(LC 4), 2=-179(LC 4), 5=-6(LC 8)
Max Grav 4=110(LC 19), 2=438(LC 1), 5=171(LC 32)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

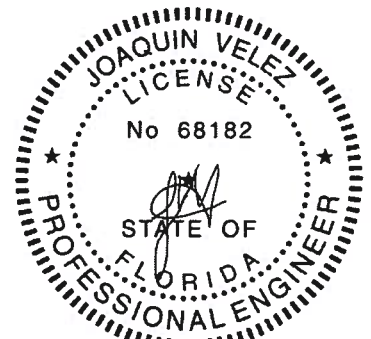
TOP CHORD 2-3=-379/39
BOT CHORD 2-6=-87/283
WEBS 3-6=-302/93

NOTES- (9)

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 5 except (jt=lb) 2=179.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 79 lb down and 116 lb up at 2-1-12, 79 lb down and 116 lb up at 2-1-12, and 79 lb down and 22 lb up at 4-11-11, and 79 lb down and 22 lb up at 4-11-11 on top chord, and 6 lb down at 2-1-12, 6 lb down at 2-1-12, and 24 lb down at 4-11-11, and 24 lb down at 4-11-11 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard

- Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=54, 2-5=20
Concentrated Loads (lb)
Vert: 7=60(F=30, B=30) 10=-8(F=-4, B=-4)



Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

February 19,2020

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6904 Parke East Blvd.
Tampa, FL 33610

Job 2256692	Truss HJ10	Truss Type Diagonal Hip Girder	Qty 4	Ply 1	Edgley Const. - Fender Res.	T19453511
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:04 2020 Page 1
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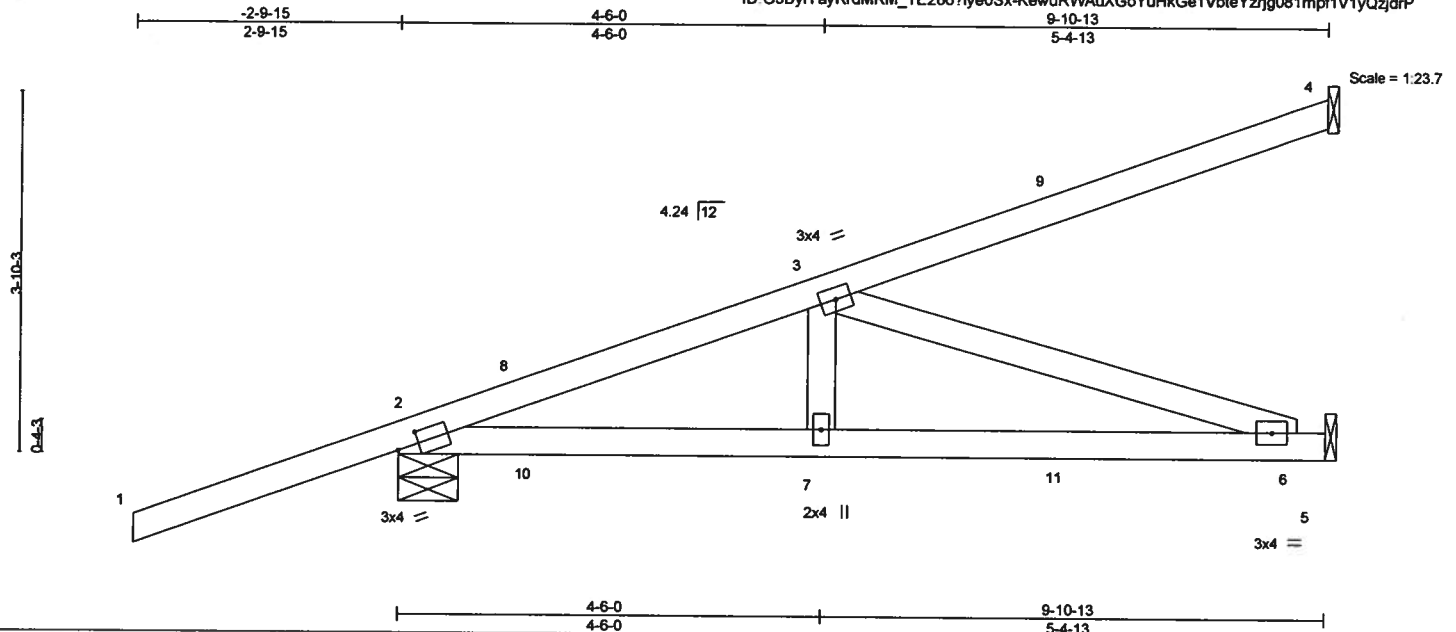


Plate Offsets (X,Y) - [2:0-2-12,0-1-8]

LOADING (psf)	SPACING-		CSI.	DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	2-0-0	TC 0.76	Vert(LL)	-0.06	6-7	>999	240	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.62	Vert(CT)	-0.12	6-7	>925	180		
BCLL 0.0 *	Rep Stress Incr	NO	WB 0.35	Horz(CT)	0.01	5	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S							
									Weight: 45 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(lb/size) 4=145/Mechanical, 2=496/0-7-12, 5=227/Mechanical
Max Horz 2=185(LC 4)
Max Uplift 4=91(LC 4), 2=243(LC 4), 5=63(LC 8)
Max Grav 4=145(LC 1), 2=496(LC 1), 5=259(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

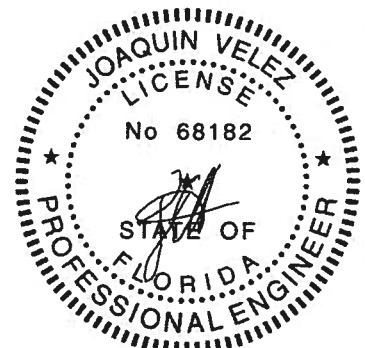
TOP CHORD 2-3=748/172
BOT CHORD 2-7=254/586, 6-7=254/586
WEBS 3-7=5/285, 3-6=616/266

NOTES- (9)

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 5 except (it=lb) 2=243.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 101 lb down and 171 lb up at 1-5-4, 101 lb down and 171 lb up at 1-5-4, 80 lb down and 18 lb up at 4-3-4, 80 lb down and 18 lb up at 4-3-4, and 89 lb down and 53 lb up at 7-1-3, and 89 lb down and 53 lb up at 7-1-3 on top chord, and 9 lb up at 1-5-4, 9 lb up at 1-5-4, 25 lb up at 4-3-4, 25 lb up at 4-3-4, and 39 lb down at 7-1-3, and 39 lb down at 7-1-3 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard

- 1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=54, 2-5=20
Concentrated Loads (lb)
Vert: 8=89(F=44, B=44) 9=45(F=22, B=22) 11=38(F=19, B=19)



Joaquin Velez PE No.68182
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February 19,2020

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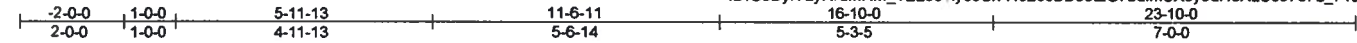
6904 Parke East Blvd.
Tampa, FL 36610

Job 2256692	Truss T01	Truss Type Hip Girder	Qty 1	Ply 1	Edgley Const. - Fender Res.	T19453512
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:06 2020 Page 1

ID:G3BylYayRrdMRM_TE2867ye0Sx-H02esBB83l2G7bufrSX3y3dH8XLSc0767L_71JzdrN



Scale = 1:43.0

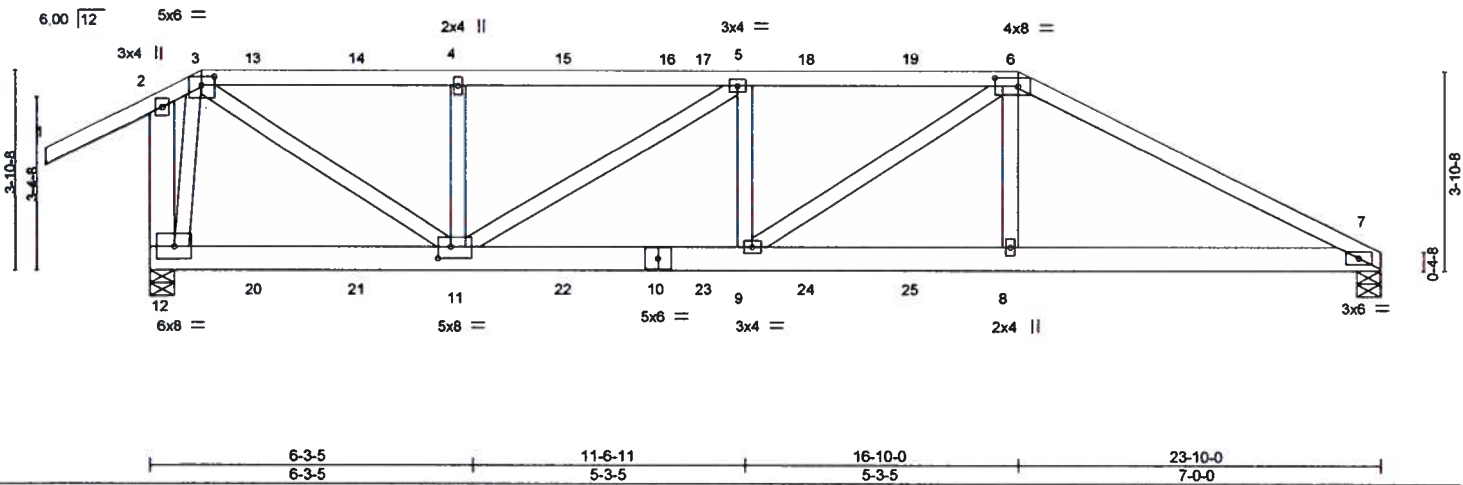


Plate Offsets (X, Y) - [3-0-3-0, 0-2-0], [6-0-5-4, 0-2-0], [11-0-3-0, 0-2-12]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.83	Vert(LL)	-0.12	9	>999	240	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.68	Vert(CT)	-0.22	9-11	>999	180		
BCLL 0.0	Rep Stress Incr	NO	WB 0.94	Horz(CT)	0.05	7	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S							
									Weight: 148 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2 *Except*
6-7: 2x4 SP M 31
BOT CHORD 2x6 SP No.2
WEBS 2x4 SP No.3 *Except*
2-12: 2x6 SP No.2

BRACING-
TOP CHORD Structural wood sheathing directly applied or 2-4-11 oc purtins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-6-11 oc bracing.

REACTIONS. (lb/size) 7=1597/0-5-8, 12=1909/0-5-8
Max Horz 12=-97(LC 6)
Max Uplift 7=-364(LC 9), 12=-423(LC 4)
Max Grav 7=1600(LC 20), 12=1909(LC 1)

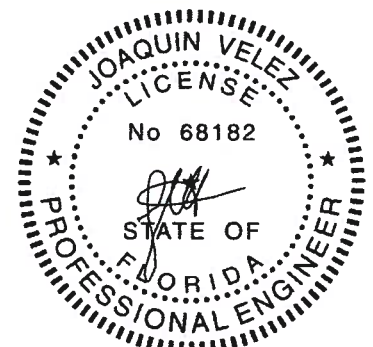
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 3-4=2407/570, 4-5=2407/570, 5-6=3256/782, 6-7=3089/731, 2-12=257/48
BOT CHORD 11-12=-107/403, 9-11=-745/3256, 8-9=-605/2691, 7-8=-605/2673
WEBS 3-11=-555/2457, 4-11=-606/264, 5-11=-1008/271, 6-9=-172/750, 6-8=-17/625, 3-12=-1619/422

NOTES- (10)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope); end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7=364, 12=423.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 103 lb down and 81 lb up at 2-0-12, 103 lb down and 81 lb up at 4-0-12, 103 lb down and 81 lb up at 6-0-12, 103 lb down and 81 lb up at 8-0-12, 103 lb down and 81 lb up at 10-0-12, 103 lb down and 81 lb up at 10-9-4, 103 lb down and 81 lb up at 12-9-4, and 103 lb down and 81 lb up at 14-9-4, and 218 lb down and 184 lb up at 16-10-0 on top chord, and 85 lb down at 2-0-12, 85 lb down at 4-0-12, 85 lb down at 6-0-12, 85 lb down at 8-0-12, 85 lb down at 10-0-12, 85 lb down at 10-9-4, 85 lb down at 12-9-4, and 85 lb down at 14-9-4, and 288 lb down and 91 lb up at 16-9-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard

Continued on page 2



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Job	Truss	Truss Type	Qty	Ply	Edgley Const. - Fender Res.	T19453512
2256692	T01	Hip Girder	1	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:06 2020 Page 2
ID: G3ByIYayRdMRM_TE286?iye0Sx-H02esBB83t2G7bufmSX3y3dH8XLSc0767L_71JzdrN

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=54, 2-3=54, 3-6=54, 6-7=54, 7-12=20

Concentrated Loads (lb)

Vert: 6=171(F) 10=51(F) 11=51(F) 4=103(F) 8=250(F) 13=103(F) 14=103(F) 15=103(F) 16=103(F) 17=103(F) 18=103(F) 19=103(F) 20=51(F) 21=51(F)
22=51(F) 23=51(F) 24=51(F) 25=51(F)



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Job 2256692	Truss T02	Truss Type Hip	Qty 1	Ply 1	Edgley Const. - Fender Res.	T19453513
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ID: G3BylYayRrdMRRM_TE286?ye0Sx-IDc03XCnpBB6ikTrJ92IUGAZmweJLPJGL?hZizjdrM

-2-0-0	3-0-0	8-11-0	14-10-0	19-0-8	23-10-0
2-0-0	3-0-0	5-11-0	5-11-0	4-2-8	4-9-8

Scale = 1:42.6

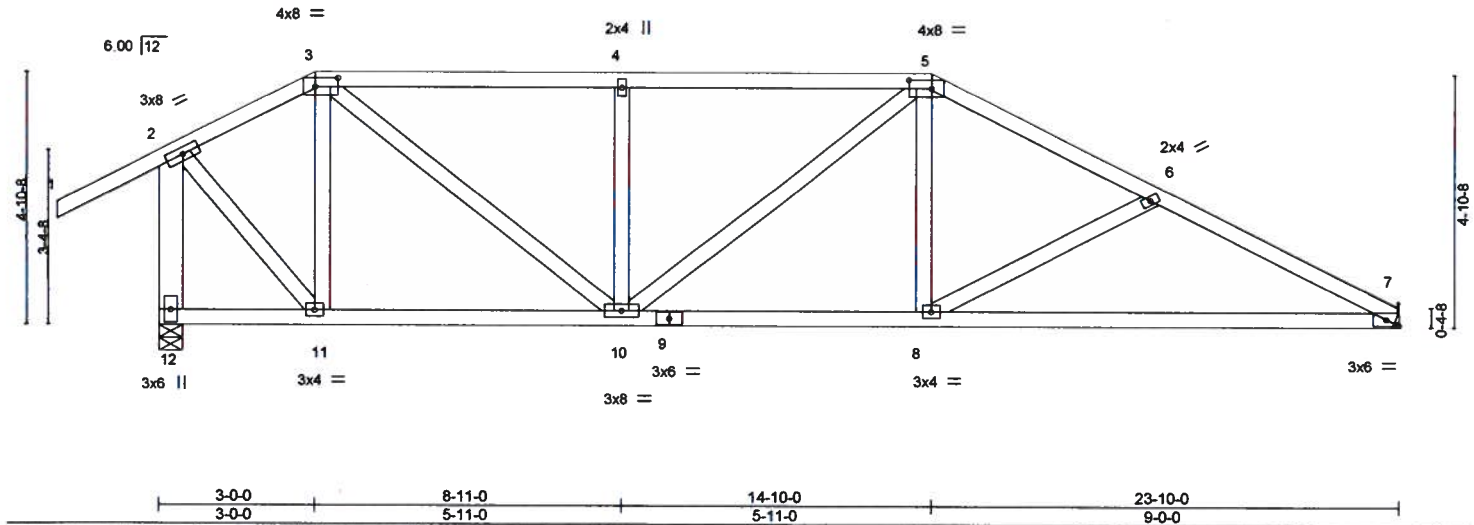


Plate Offsets (X,Y)-		[3:0-5-4, 0-2-0], [5:0-5-4, 0-2-0], [7:0-2-15, Edge]			
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc) l/defl L/d
TCLL 20.0	Plate Grip DOL	1.25	TC 0.39	Vert(LL)	-0.20 7-8 >999 240
TCDL 7.0	Lumber DOL	1.25	BC 0.83	Vert(CT)	-0.42 7-8 >670 180
BCLL 0.0	Rep Stress Incr	YES	WB 0.30	Horz(CT)	0.03 7 n/a n/a
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S		
					Weight: 137 lb FT = 20%

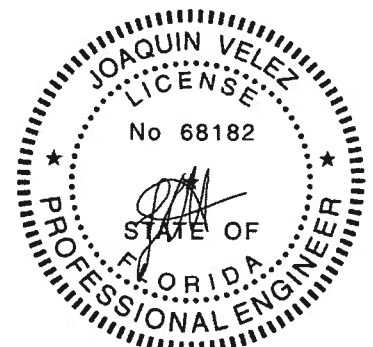
LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3 *Except*
2-12: 2x6 SP No.2

BRACING-
TOP CHORD Structural wood sheathing directly applied or 4-3-11 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. (lb/size) 7=865/Mechanical, 12=997/0-5-8
Max Horz 12=111(LC 10)
Max Uplift 7=98(LC 13), 12=124(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=572/276, 3-4=1074/483, 4-5=1074/483, 5-6=1266/488, 6-7=1531/607,
2-12=985/458
BOT CHORD 10-11=68/465, 8-10=278/1090, 7-8=485/1330
WEBS 3-11=436/194, 3-10=296/773, 4-10=368/210, 5-8=61/392, 6-8=282/236,
2-11=223/738

- NOTES-** (9)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf, BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7 except (jt=lb) 12=124.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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MiTek USA, Inc. FL Cert 6634
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February 19,2020

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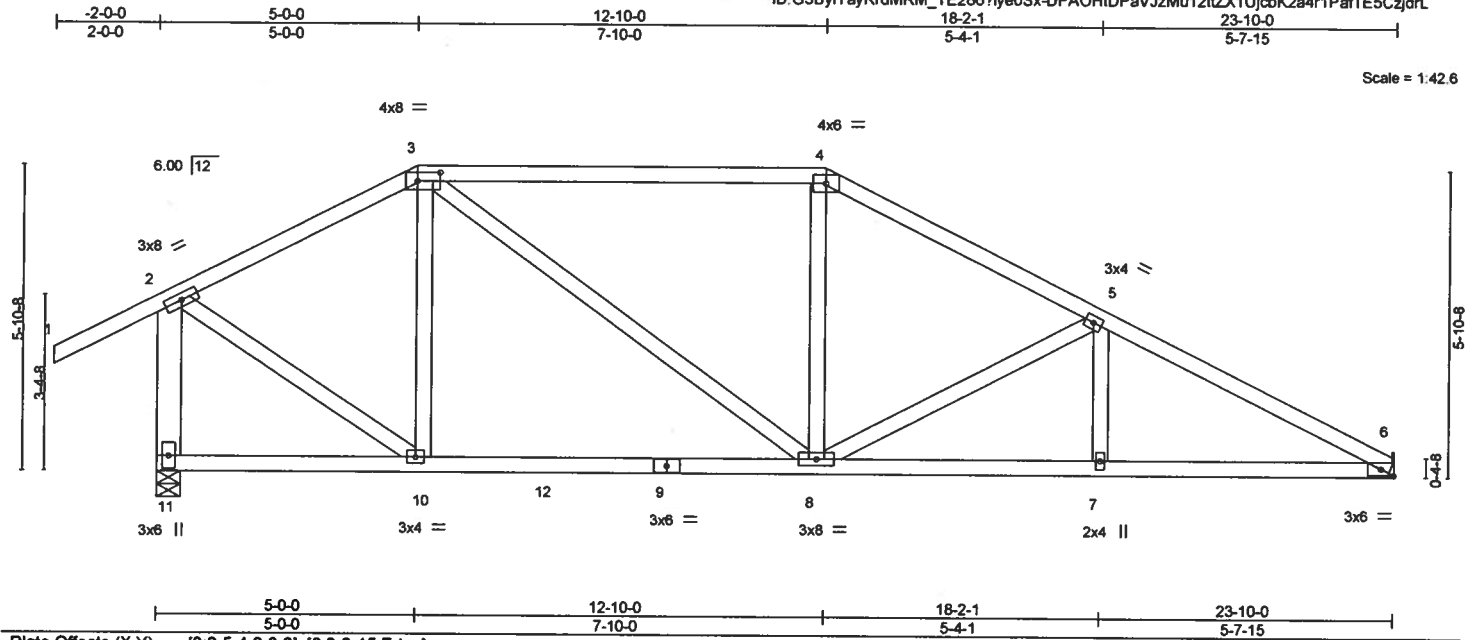
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Job 2256692	Truss T03	Truss Type Hip	Qty 1	Ply 1	Edgley Const. - Fender Res.	T19453514
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8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:08 2020 Page 1
ID: G3ByfYayRdMRM_TE286?iye0Sx-DPAOHIDPaVJzMu12tZX1UjcbK2a4r1PafTE5CzjdrL



LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.90	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plate Grip DOL 1.25	BC 0.51	Vert(LL) -0.08 8-10 >999 240		
BCLL 0.0 *	Lumber DOL 1.25	WB 0.33	Vert(CT) -0.17 8-10 >999 180		
BCDL 10.0	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.03 6 n/a n/a		
	Code FBC2017/TPI2014			Weight: 134 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3 *Except*
2-11: 2x6 SP No.2

BRACING-
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. (lb/size) 6=865/Mechanical, 11=997/0-5-8
Max Horz 11=121(LC 10)
Max Uplift 6=108(LC 13), 11=101(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=752/338, 3-4=939/458, 4-5=1112/462, 5-6=1553/581, 2-11=962/471
BOT CHORD 8-10=103/617, 7-8=451/1339, 6-7=451/1339
WEBS 3-10=268/164, 3-8=162/426, 4-8=0/255, 5-8=456/262, 2-10=224/751

- NOTES-** (9)
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - 7) Refer to girder(s) for truss to truss connections.
 - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=108, 11=101.
 - 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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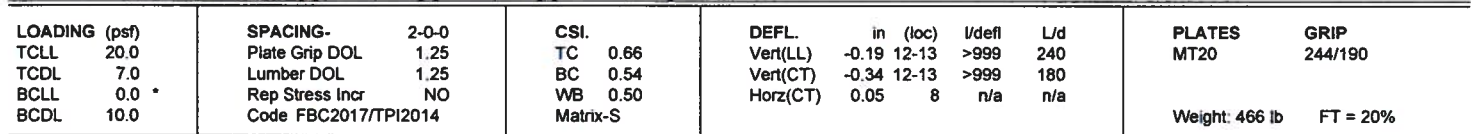
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 ID: G3ByIYayRrdMRM_TE2867?ye0Sx-hbknUDE1LoRq_2cERa4mahFr6kOQpGdYpJCodezdrK

 Scale = 1:64.8



REACTIONS. (lb/size) 16=2035/0-5-8, 8=2187/0-5-8
 Max Horz 16=144(LC 6)
 Max Uplift 16=301(LC 4), 8=543(LC 9)
 Max Grav 16=2035(LC 1), 8=2207(LC 20)

FORCES.	(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD	2-3=-54/346, 3-4=-942/245, 4-5=-2652/561, 5-6=-6026/1428, 6-7=-6026/1428, 7-8=-4169/1048
BOT CHORD	15-16=-63/554, 14-15=-158/1302, 13-14=-1077/5352, 12-13=-1076/5358, 10-12=-861/3655, 8-10=-862/3639
WEBS	3-16=-1830/295, 3-15=-268/1371, 4-15=-1279/298, 4-14=-481/2256, 5-14=-3620/867, 5-12=-539/979, 6-12=-376/221, 7-12=-524/2633, 7-10=0/487, 2-16=-361/138

NOTES: (11)

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc.
Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDD=4.2psf; BCDL=3.0psf; h=18ft, Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope); cantilever left exposed; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 16=301, 8=543.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 101 lb down and 101 lb up at 25-4-12, and 101 lb down and 101 lb up at 27-4-12, and 191 lb down and 183 lb up at 29-8-0 on top chord, and 1000 lb down and 226 lb up at 23-10-12, 64 lb down at 25-4-12, and 64 lb down at 27-4-12, and 207 lb down and 27 lb up at 29-7-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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6904 Parke East Blvd. Tampa FL 33610
Date: February 19,2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see *ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information* available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

Job 2256692	Truss T04	Truss Type Roof Special Girder	Qty 1	Ply 2	Edgley Const. - Fender Res. T19453515
Builders FirstSource, Jacksonville, FL - 32244,					Job Reference (optional)

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:09 2020 Page 2
ID:G3ByIYayRrdMRM_TE286?iye0Sx-hbknUDE1LoRq_2cERa4mahFr6kOQpGdYpJCodezdrK

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-54, 2-3=-54, 3-4=-54, 4-5=-54, 5-7=-54, 7-9=-54, 8-17=-20

Concentrated Loads (lb)

Vert: 7=95(F) 12=1000(F) 10=132(F) 18=61(F) 19=61(F) 20=32(F) 21=32(F)

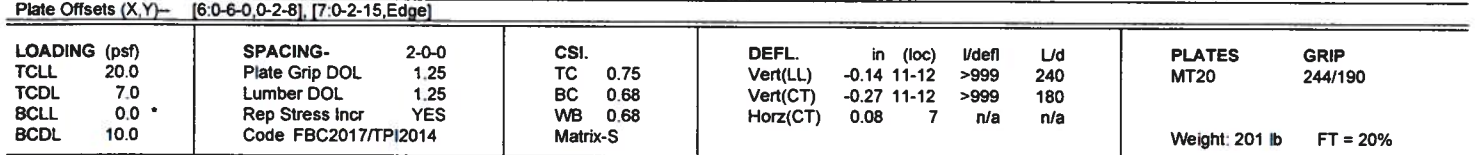
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.

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Tampa, FL 36610

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:10 2020 Page 1

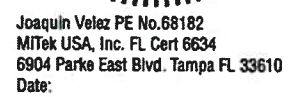


REACTIONS. (lb/size) 15=1447/0-5-8, 7=1242/0-5-8
Max Horz 15=-159(LC 8)
Max Uplift 15=-168(LC 13), 7=-208(LC 13)

NOTES- (8)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TC DL=4.2psf; BC DL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; cantilever left exposed ; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 15=168, 7=208.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

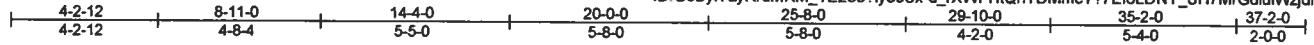
JOAQUIN VELEZ
LICENSE
No 68182
PROF. OF ENGINEERING
STATE OF TEXAS



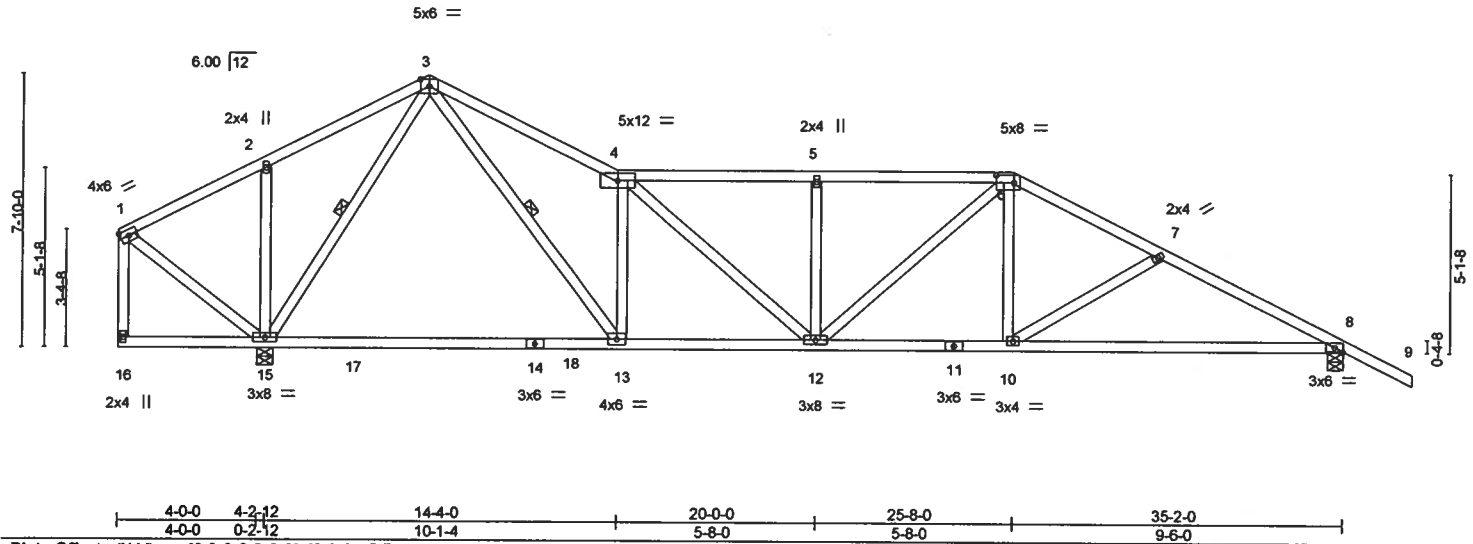
Mii
MiTek
6904 Parke East Blvd.
Tampa, FL 36610

Job 2256692	Truss T06	Truss Type Roof Special	Qty 1	Ply 1	Edgley Const. - Fender Res.	T19453517
Builders FirstSource, Jacksonville, FL - 32244,						

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:11 2020 Page 1
ID:G3ByfYayRrdMRM_TE2867iye0Sx-d_rXvwFhtQhYDMmcY77E6LDNY_SH7MrGdiuVzjdri



Scale: 3/16"=1'



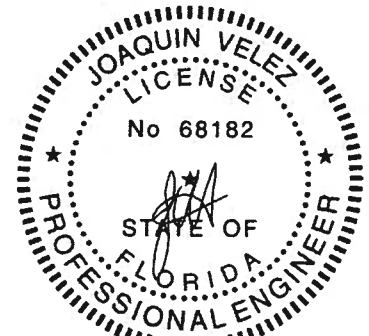
LOADING (psf)		SPACING-		CSI.		DEFL.		PLATES		GRIP	
TCLL	20.0	Plate Grip DOL	1.25	TC	0.48	Vert(LL)	-0.36 13-15 >999	MT20		244/190	
TCDL	7.0	Lumber DOL	1.25	BC	0.95	Vert(CT)	-0.63 13-15 >589				
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.68	Horz(CT)	0.06 8 n/a n/a				
BCDL	10.0	Code FBC2017/TPI2014		Matrix-S							
								Weight: 206 lb FT = 20%			

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 3-7-12 oc purlins, except end verticals.
BOT CHORD	2x4 SP No.2	BOT CHORD	Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS	2x4 SP No.3	WEBS	1 Row at midpt 3-15, 3-13

REACTIONS. (lb/size) 15=1454/0-5-8, 8=1241/0-5-8
Max Horz 15=148(LC 13)
Max Uplift 15=171(LC 13), 8=206(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 3-4=1835/727, 4-5=1763/682, 5-6=1763/682, 6-7=1735/618, 7-8=1976/709
BOT CHORD 13-15=0/530, 12-13=326/1547, 10-12=349/1510, 8-10=522/1688
WEBS 2-15=280/220, 3-15=1114/429, 3-13=660/1786, 4-13=1257/591, 4-12=151/288, 5-12=339/191, 6-12=127/333, 6-10=49/399

- NOTES- (8)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 15=171, 8=206.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

February 19, 2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MH-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

MiTek

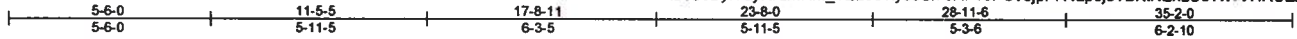
6904 Parke East Blvd.
Tampa, FL 33610

Job 2256692	Truss T07	Truss Type Hip	Qty 1	Ply 1	Edgley Const. - Fender Res.	T19453518
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:12 2020 Page 1

ID: G3BylYayRrdMRM_TE286?iye0Sx-5APv6FGvejPrVWlp6jeTBKINExLU0WV7VHRSEzjdrH



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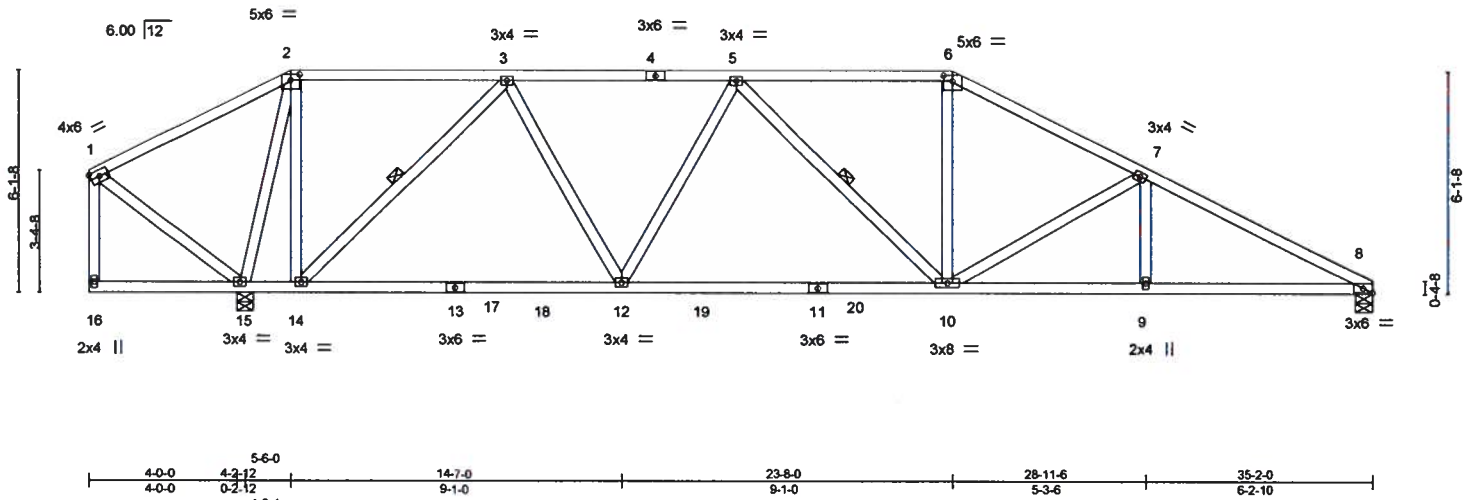


Plate Offsets (X,Y) - [2:0-3-0,0-2-0], [6:0-3-0,0-2-0], [8:0-2-15,Edge]

LOADING (psf)	SPACING-		CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	2-0-0	TC 0.54	Vert(LL)	-0.16	10-12	>999	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.83	Vert(CT)	-0.34	10-12	>999		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.92	Horz(CT)	0.07	8	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S						
								Weight: 200 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

REACTIONS.

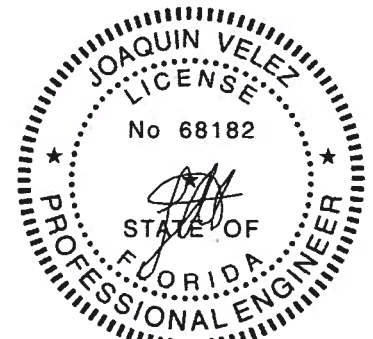
(lb/size) 8=1118/0-5-8, 15=1457/0-5-8
Max Horz 15=111(LC 13)
Max Uplift 8=122(LC 13), 15=215(LC 8)
Max Grav 8=1127(LC 24), 15=1457(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 3-5=1317/493, 5-6=1407/576, 6-7=1634/597, 7-8=2061/709
BOT CHORD 12-14=174/1062, 10-12=343/1469, 9-10=556/1778, 8-9=556/1778
WEBS 2-15=1332/448, 2-14=225/987, 3-14=1168/458, 3-12=147/589, 5-12=334/210,
6-10=106/447, 7-10=443/267

NOTES- (8)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCPI=0.18; MWFRS (envelope) and C-C Exterior(2) zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=122, 15=215.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

February 19,2020

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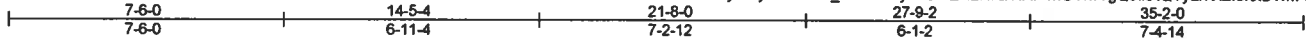
6904 Parke East Blvd.
Tampa, FL 33610

Job 2256692	Truss T08	Truss Type Hip	Qty 1	Ply 1	Edgley Const. - Fender Res.	T19453519
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:13 2020 Page 1

ID: G3ByfYayRrdMRM_TE2867iye0Sx-ZNzHKbHXP1xGTfw?gQ9ikXQTyLhVl2i8kx8?mPzjdrG



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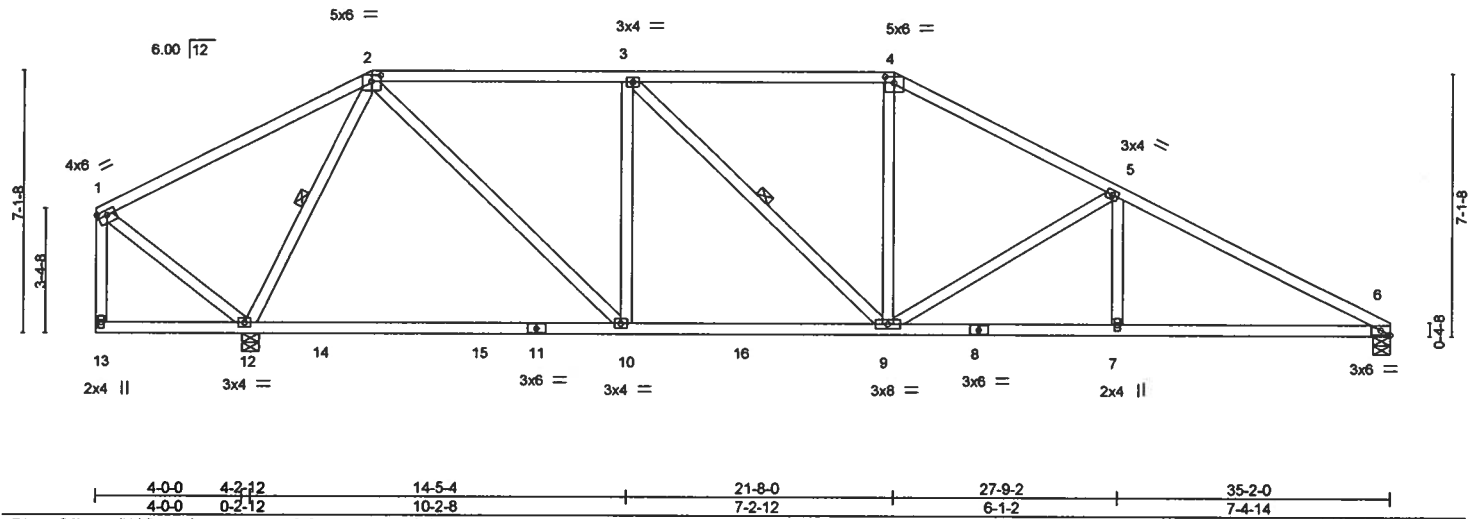


Plate Offsets (X,Y)~ [1:0-3-0,0-1-8], [2:0-3-0,0-2-0], [4:0-3-0,0-2-0], [6:0-2-15,Edge]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.86	Vert(LL)	-0.29 10-12	>999	240	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.85	Vert(CT)	-0.52 10-12	>705	180		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.63	Horz(CT)	0.06 6	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S						
								Weight: 194 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 2-12, 3-9

REACTIONS.

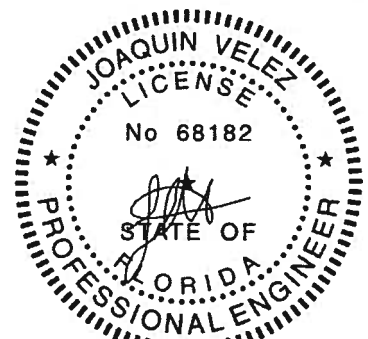
(lb/size) 6=1120/0-5-8, 12=1455/0-5-8
Max Horz 12=121(LC 13)
Max Uplift 6=132(LC 13), 12=177(LC 8)
Max Grav 6=1123(LC 24), 12=1455(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=1157/492, 3-4=1249/558, 4-5=1475/572, 5-6=2007/699
BOT CHORD 10-12=49/461, 9-10=201/1157, 7-9=535/1726, 6-7=535/1726
WEBS 2-12=1271/568, 2-10=343/1017, 3-10=526/300, 4-9=65/371, 5-9=565/315,
5-7=0/304, 1-12=216/259

NOTES- (8)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf, BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=132, 12=177.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

February 19,2020

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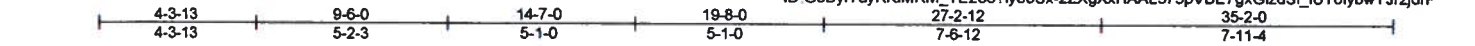


6904 Parke East Blvd.
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	Edgley Const. - Fender Res.	T19453520
2256692	T09	HIP	1	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8 240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:14 2020 Page 1
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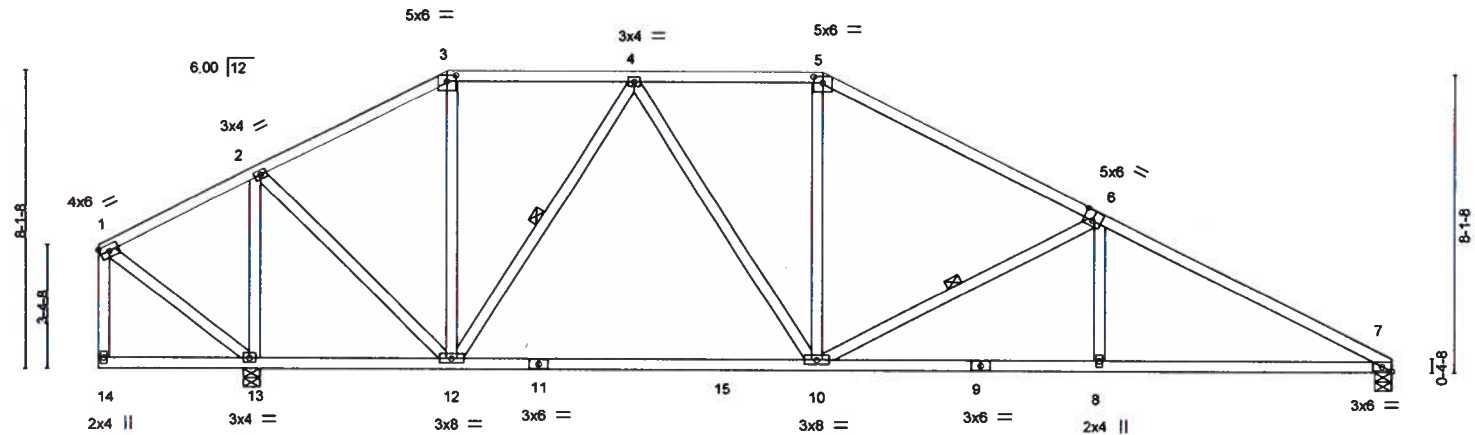


Plate Offsets (X,Y) =	[3:0-3-0,0-2-0], [5:0-3-0,0-2-0], [6:0-3-0,0-3-4], [7:0-2-15,Edge]
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LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.94	Vert(LL)	-0.37 10-12	>992	240	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.98	Vert(CT)	-0.60 10-12	>608	180		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.70	Horz(CT)	0.06 7	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S					Weight: 206 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 4-12, 6-10

REACTIONS.

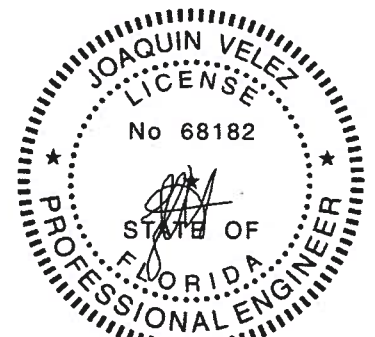
(lb/size) 13=1463/0-5-8, 7=1112/0-5-8
Max Horz 13=131(LC 13)
Max Uplift 13=145(LC 8), 7=140(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=730/305, 3-4=599/314, 4-5=1095/523, 5-6=1326/520, 6-7=1968/698
BOT CHORD 10-12=135/935, 8-10=531/1692, 7-8=531/1692
WEBS 2-13=1304/559, 2-12=296/959, 4-12=658/291, 4-10=102/331, 5-10=38/327,
6-10=679/386, 6-8=0/320

NOTES- (8)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; cantilever left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 13=145, 7=140.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

February 19,2020

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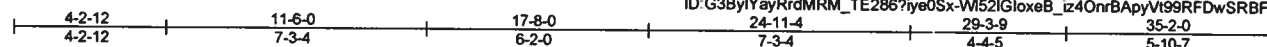
6904 Parke East Blvd.
Tampa, FL 33610

Job 2256692	Truss T10	Truss Type Hip	Qty 1	Ply 1	Edgley Const. - Fender Res.	T19453521
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:15 2020 Page 1

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5x8 =

5x6 =

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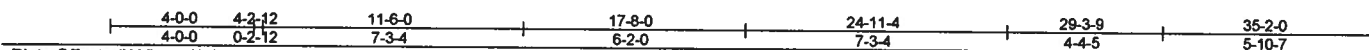
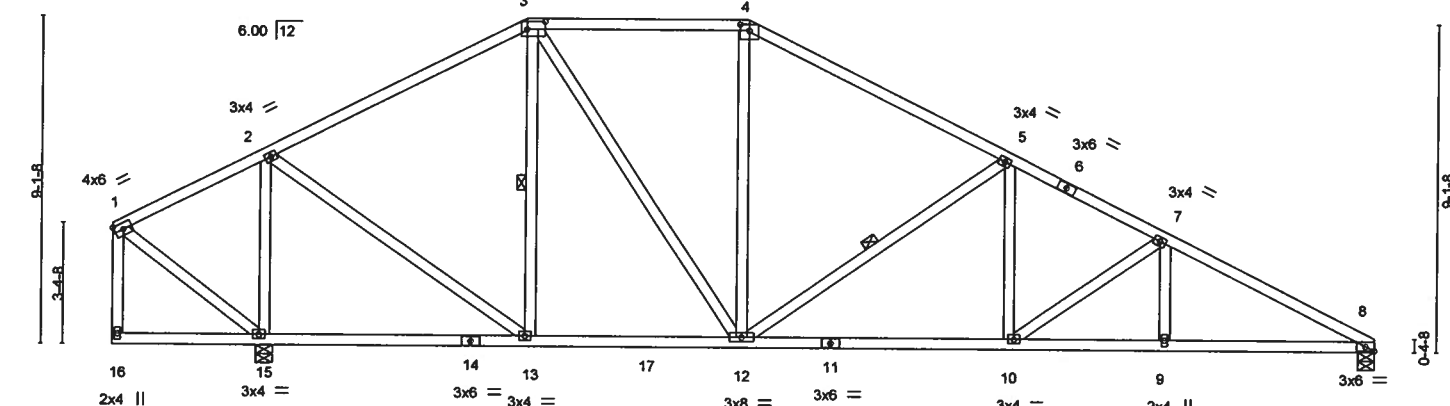


Plate Offsets (X,Y) - [3:0-6-0,0-2-8], [4:0-3-0,0-2-0], [8:0-2-15,Edge]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.62	Vert(LL)	-0.10 10-12	>999	240	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.57	Vert(CT)	-0.23 10-12	>999	180		
BCLL 0.0	Rep Stress Incr	YES	WB 0.67	Horz(CT)	0.05 8	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S						
								Weight: 214 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 3-3-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 3-13, 5-12

REACTIONS.

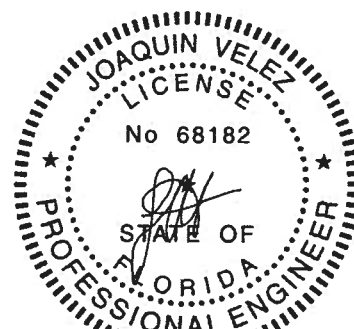
(lb/size) 15=1458/0-5-8, 8=1116/0-5-8
Max Horz 15=141(LC 13)
Max Uplift 15=150(LC 12), 8=147(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-867/376, 3-4=-952/505, 4-5=-1155/497, 5-7=-1711/669, 7-8=-2037/728
BOT CHORD 12-13=-19/684, 10-12=-424/1492, 9-10=-572/1753, 8-9=-572/1753
WEBS 2-15=-1275/599, 2-13=-286/913, 3-13=-393/211, 3-12=-199/516, 4-12=-4/256, 5-12=-662/361, 5-10=-62/379, 7-10=-319/179

NOTES- (8)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 15=150, 8=147.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

February 19,2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MIT-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



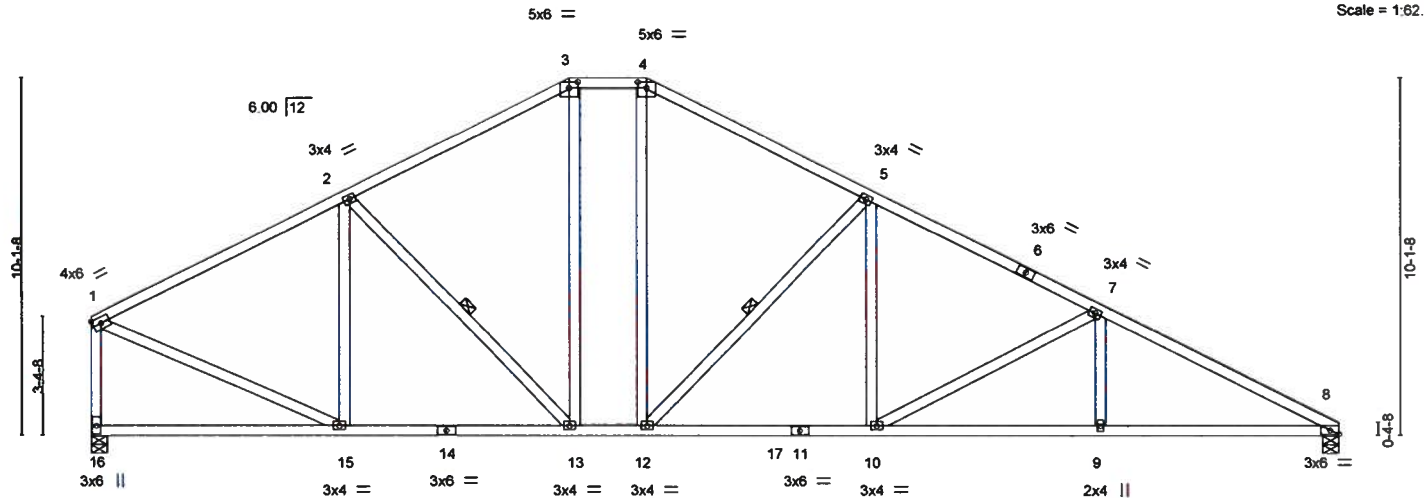
6904 Parke East Blvd.
Tampa, FL 33610

Job 2256692	Truss T11	Truss Type Hip	Qty 1	Ply 1	Edgley Const. - Fender Res.	T19453522
Builders FirstSource, Jacksonville, FL - 32244,						Job Reference (optional)

8 240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08 16:16 2020 Page 1
ID: G3ByIYayRrdMRM_TE286?iye0Sx-_yfQycJQiyJrK7faLYIPMA2?YzjhyQQaQvPINKzjdrD



Scale = 1:62.6



Job 2256692	Truss T12	Truss Type Common	Qty 1	Ply 1	Edgley Const. - Fender Res.	T19453523
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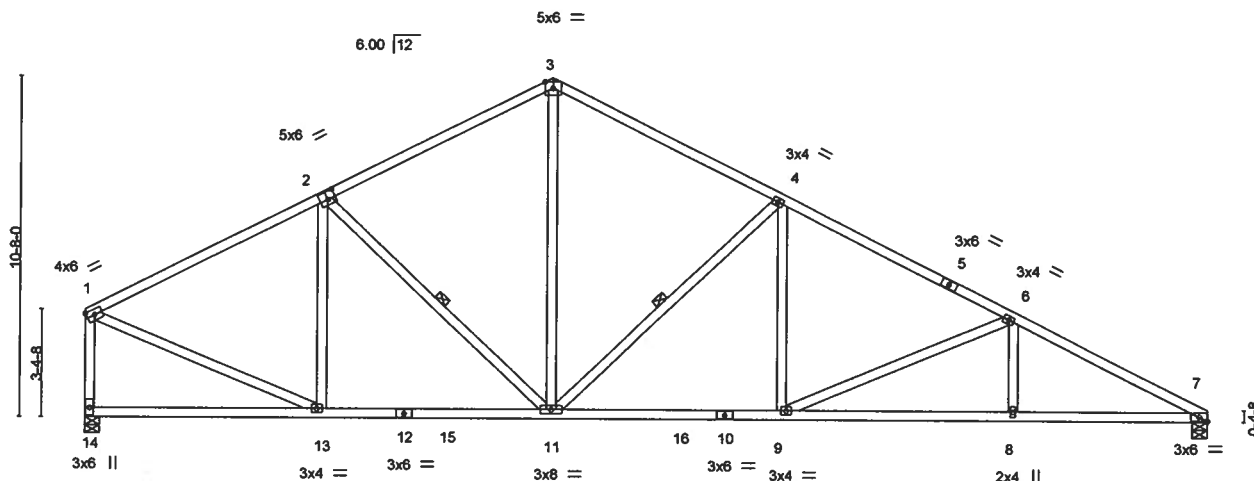
Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:17 2020 Page 1

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7-4-12	14-7-0	21-9-2	28-10-14	35-2-0
7-4-12	7-2-3	7-2-3	7-1-12	6-3-2

Scale = 1:69.4



		7-4-12		14-7-0		21-9-2		28-10-14		35-2-0	
		7-4-12		7-2-3		7-2-3		7-1-12		6-3-2	
Plate Offsets (X,Y)-- [2:0-3-0,0-3-0], [7:0-2-15,Edge]											
LOADING (psf)		SPACING- 2-0-0		CSI.		DEFL.		in (loc)		l/defl L/d	
TCLL	20.0	Plate Grip DOL 1.25		TC 0.60		Vert(LL)		-0.13	9-11	>999	240
TCDL	7.0	Lumber DOL 1.25		BC 0.67		Vert(CT)		-0.25	9-11	>999	180
BCLL	0.0	Rep Stress Incr YES		WB 0.69		Horz(CT)		0.08	7	n/a	n/a
BCDL	10.0	Code FBC2017/TPI2014		Matrix-S							
										Weight: 205 lb	FT = 20%
										PLATES	GRIP
										MT20	244/190

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 3-2-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-10-3 oc bracing.
WEBS 1 Row at midpt 2-11, 4-11

REACTIONS.

(lb/size) 14=1287/0-5-8, 7=1287/0-5-8
Max Horz 14=156(LC 13)
Max Uplift 14=147(LC 12), 7=170(LC 13)

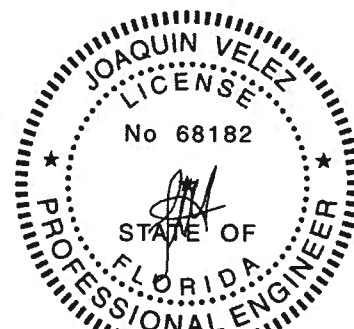
FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=1360/552, 2-3=1293/627, 3-4=1292/626, 4-6=1883/769, 6-7=2426/914, 1-14=1221/515
BOT CHORD 11-13=280/1157, 9-11=467/1609, 8-9=740/2103, 7-8=740/2103
WEBS 2-13=326/219, 3-11=332/752, 4-11=753/401, 4-9=72/450, 1-13=426/1204, 6-9=543/297, 6-8=0/283

NOTES- (7)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 14=147, 7=170.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

February 19,2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

MiTek

6904 Parke East Blvd.
Tampa, FL 36610

Job 2256692	Truss T13	Truss Type Common	Qty 3	Ply 1	Edgley Const. - Fender Res.	T19453524
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:18 2020 Page 1

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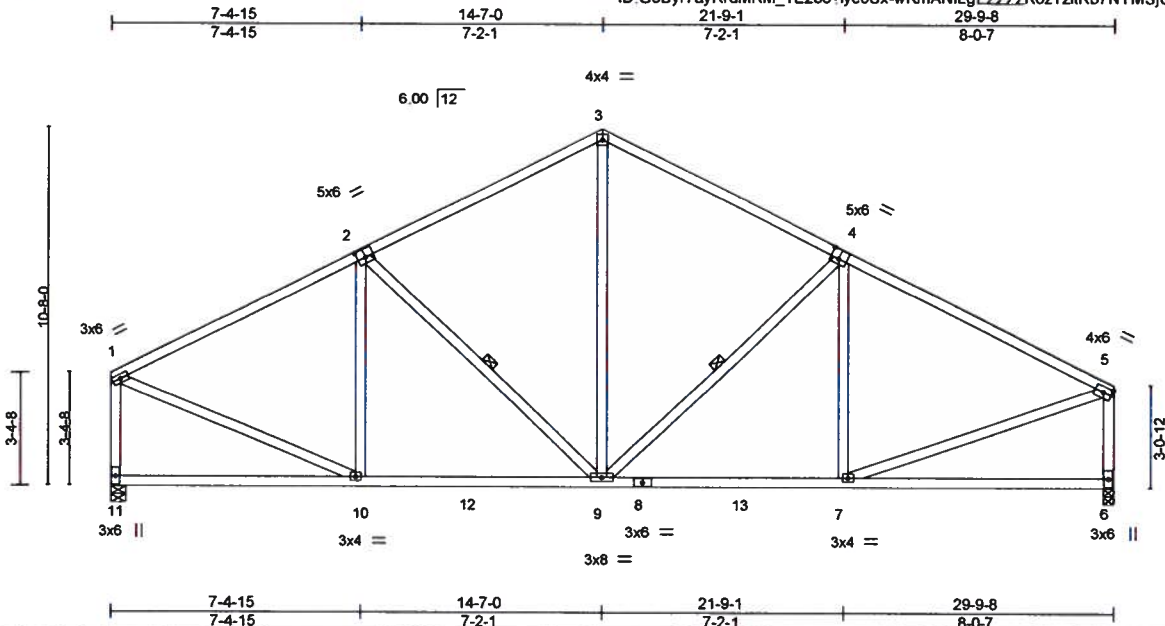


Plate Offsets (X,Y) - [2-0-3-0 0-3-0], [4-0-3-0 0-3-4]		7-4-15 7-4-15		14-7-0 7-2-1		21-9-1 7-2-1		29-9-8 8-0-7	
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.68	Vert(LL)	-0.10 6-7	>999	240	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.59	Vert(CT)	-0.21 6-7	>999	180		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.49	Horz(CT)	0.02 6	n/a	n/a		
BCDL 10.0	Code	FBC2017/TPI2014	Matrix-S					Weight: 188 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 3-8-6 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 2-9, 4-9

REACTIONS. (lb/size) 11=1091/0-5-8, 6=1091/0-3-8
Max Horz 11=-84(LC 8)
Max Uplift 11=-130(LC 12), 6=-132(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-1125/456, 2-3=-986/504, 3-4=-988/506, 4-5=-1194/477, 1-11=-1025/435, 5-6=-1018/434
BOT CHORD 9-10=-315/955, 7-9=-339/994
WEBS 2-9=-277/181, 3-9=-236/510, 4-9=-340/215, 1-10=-330/973, 5-7=-324/987

- NOTES-** (7)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 11=130, 6=132.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

February 19,2020

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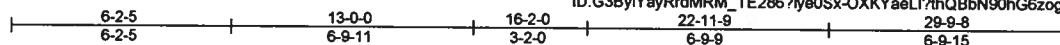
MiTek

6904 Parke East Blvd.
Tampa, FL 36610

Job 2256692	Truss T14	Truss Type Hip	Qty 1	Ply 1	Edgley Const. - Fender Res.	T19453525
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:19 2020 Page 1
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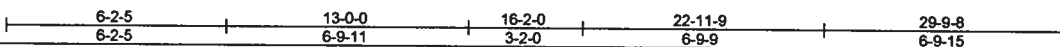
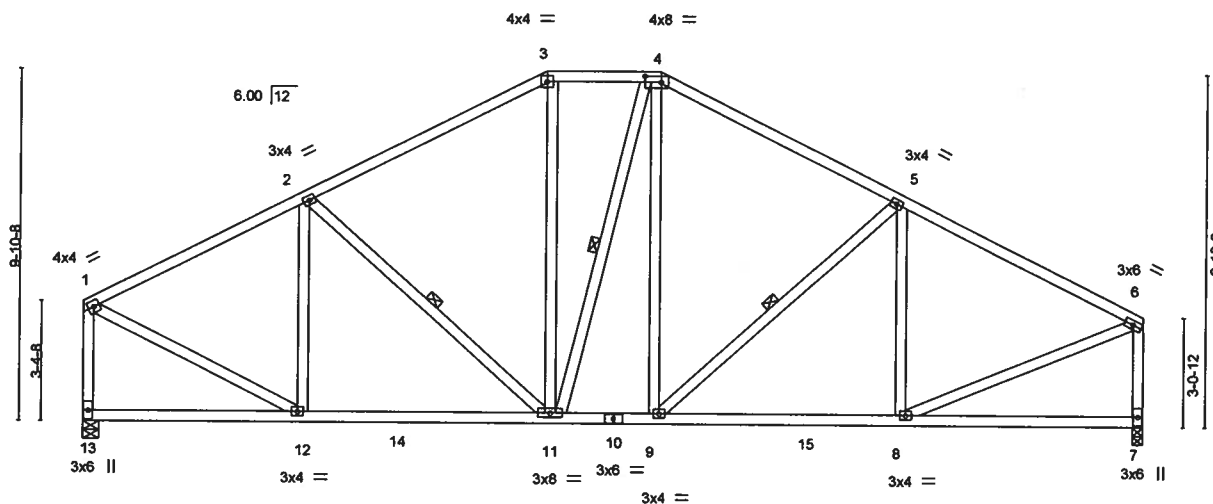


Plate Offsets (X,Y) - [4:0-5-8,0-2-0]

LOADING (psf)	SPACING-		CSI.	DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	2-0-0	TC 0.51	Vert(LL)	-0.07	8-9	>999	240	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.52	Vert(CT)	-0.14	8-9	>999	180		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.39	Horz(CT)	0.03	7	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S							
									Weight: 208 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 4-10-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 2-11, 5-9, 4-11

REACTIONS.

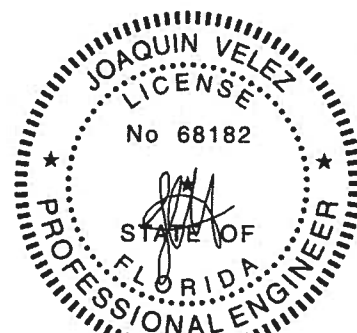
(lb/size) 13=1091/0-5-8, 7=1091/0-3-8
Max Horz 13=75(LC 8)
Max Uplift 13=125(LC 12), 7=127(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=1070/427, 2-3=1032/497, 3-4=842/499, 4-5=1039/501, 5-6=1155/456,
1-13=1039/426, 6-7=1032/426
BOT CHORD 11-12=305/904, 9-11=213/847, 8-9=335/972
WEBS 2-12=313/205, 4-9=83/259, 1-12=344/989, 6-8=341/1012

NOTES- (8)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 13=125, 7=127.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

February 19,2020

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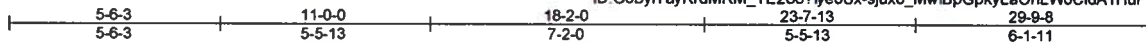
6904 Parke East Blvd.
Tampa, FL 33610

Job 2256692	Truss T15	Truss Type Hip	Qty 1	Ply 1	Edgley Const. - Fender Res.	T19453526
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:20 2020 Page 1

ID:G3BytYayRrdMRM_TE286?ye0Sx-sjuxo_MwIBpGpkyLaOnLW0CidA1HuFwALWNlWVzjdr9



5x6 =

5x6 =

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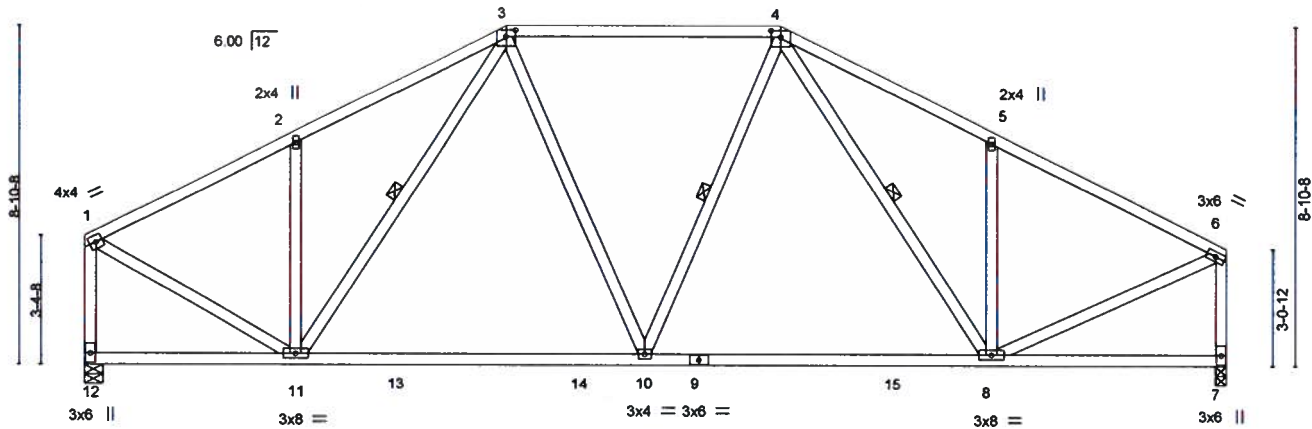


Plate Offsets (X,Y) [3:0-3-0,0-2-0], [4:0-3-0,0-2-0]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.71	Vert(LL)	-0.21	8-10	>999	240	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.97	Vert(CT)	-0.34	8-10	>999	180		
BCLL 0.0	Rep Stress Incr	YES	WB 0.40	Horz(CT)	0.03	7	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S							
									Weight: 192 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

REACTIONS.

(lb/size) 12=1091/0-5-8, 7=1091/0-3-8
Max Horz 12=-64(LC 8)
Max Uplift 12=-116(LC 12), 7=-118(LC 13)
Max Grav 12=1095(LC 2), 7=1091(LC 1)

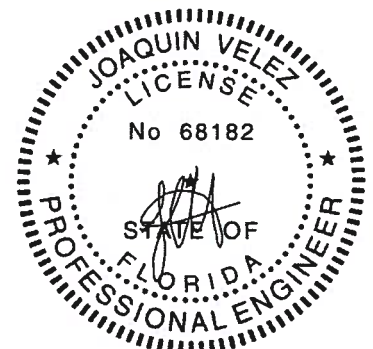
FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-1055/396, 2-3=-1065/539, 3-4=-1016/483, 4-5=-1152/578, 5-6=-1147/428,
1-12=-1073/415, 6-7=-1058/416
BOT CHORD 10-11=-245/926, 8-10=-253/946
WEBS 2-11=-318/273, 3-10=-9/301, 5-8=-338/289, 1-11=-327/1029, 6-8=-328/1056,
4-10=-11/257

NOTES- (8)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=116, 7=118.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

February 19,2020

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Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-69 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd.
Tampa, FL 36610

Job 2256692	Truss T16	Truss Type Hip	Qty 1	Ply 1	Edgley Const. - Fender Res. Job Reference (optional)	T19453527
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:21 2020 Page 1

ID: G3ByIYayRrdMRM_TE286?iye0Sx-KvSJ7KNZWUx7QuXX86la3Dlv8aQpdakJZA7Q3xjdr8

4-2-2	9-0-0	14-7-0	20-2-0	24-11-14	29-9-8
4-2-2	4-9-14	5-7-0	5-7-0	4-9-14	4-9-10

Scale = 1:52.9

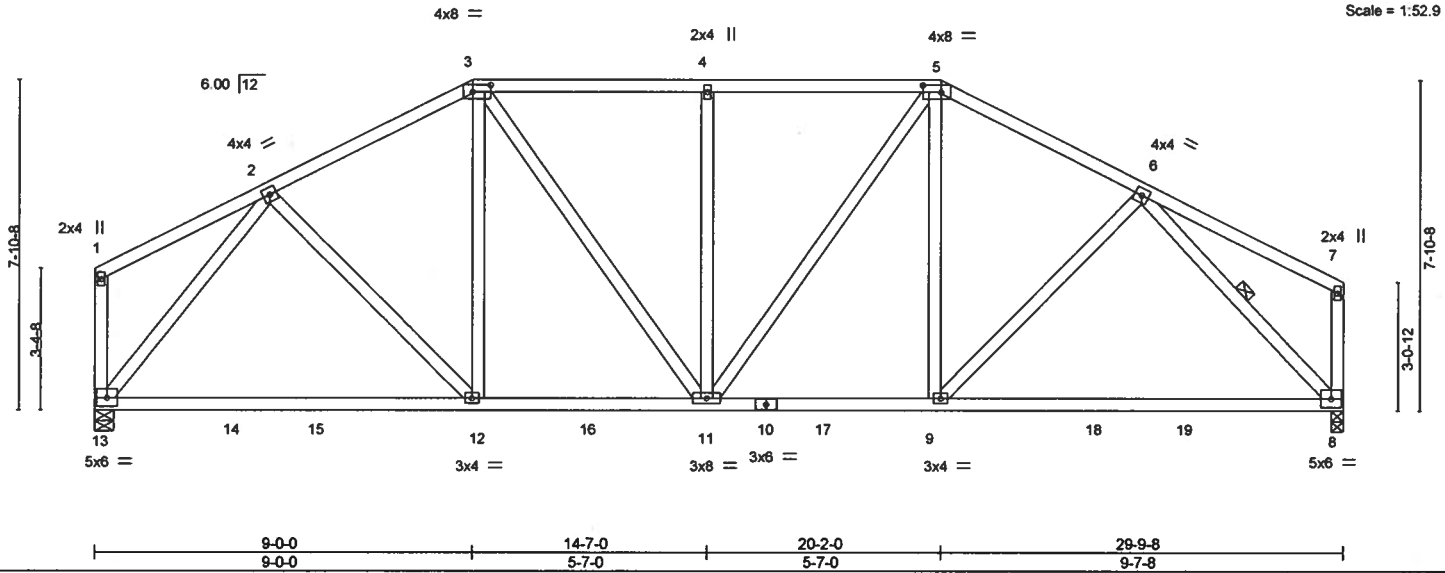


Plate Offsets (X,Y)- [3:0-5-4,0-2-0], [5:0-5-4,0-2-0]

LOADING (psf)	SPACING-	CSL	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.59	Vert(LL)	-0.23	8-9	>999	240	MT20	244/190
TCDL 7.0	Plate Grip DOL 1.25	BC 0.82	Vert(CT)	-0.47	8-9	>756	180		
BCLL 0.0 *	Lumber DOL 1.25	WB 0.94	Horz(CT)	0.04	8	n/a	n/a		
BCDL 10.0	Rep Stress Incr YES	Matrix-S							
	Code FBC2017/TPI2014							Weight: 199 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 5-4-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 6-8

REACTIONS.

(lb/size) 13=1091/0-5-8, 8=1091/0-3-8
Max Horz 13=-54(LC 8)
Max Uplift 13=-105(LC 12), 8=-108(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1100/467, 3-4=-1085/527, 4-5=-1085/527, 5-6=-1142/483
BOT CHORD 12-13=-263/736, 11-12=-253/940, 9-11=-267/978, 8-9=-306/829
WEBS 2-12=-30/349, 3-11=-120/346, 4-11=-342/191, 5-11=-97/295, 6-9=-23/288, 2-13=-1146/449, 6-8=-1173/452

NOTES- (8)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 13=105, 8=108.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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February 19,2020

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6904 Parke East Blvd.
Tampa, FL 33610

Job 2256692	Truss T17	Truss Type Half Hip Girder	Qty 1	Ply 2	Edgley Const. - Fender Res.	T19453528
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:24 2020 Page 1
ID: G3BylYayRrdMRM_TE286?iye0Sx-IU8RdLPKIHMG6pEsHgsNR4nU4q_smG8L4fGzdr5



Scale: 3/16"=1'

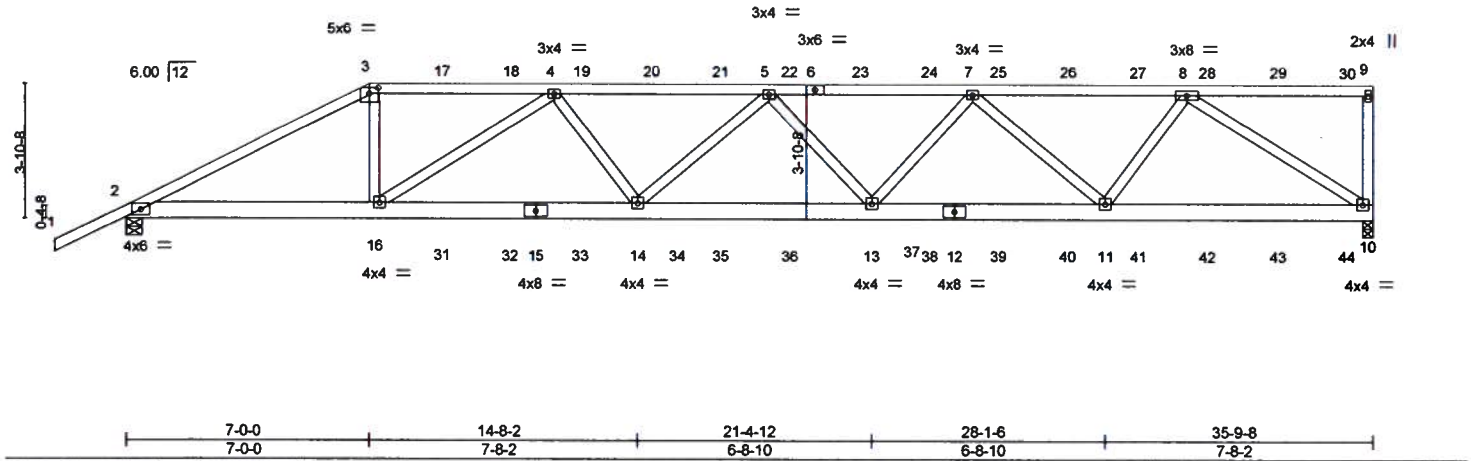


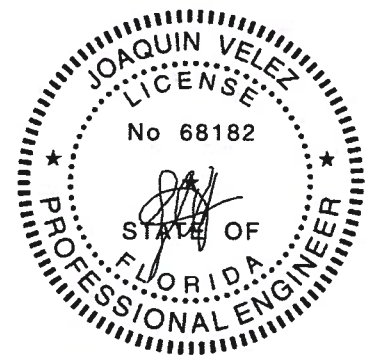
Plate Offsets (X,Y)=[3:0-3:0,0-2:0]											
LOADING (psf)		SPACING-2-0-0		CSI.	DEFL.		in (loc)	l/defl	L/d	PLATES GRIP	
TCLL 20.0		Plate Grip DOL 1.25		TC 0.55	Vert(LL)		-0.22 13-14	>999	240	MT20 244/190	
TCDL 7.0		Lumber DOL 1.25		BC 0.66	Vert(CT)		-0.43 13-14	>992	180		
BCLL 0.0 *		Rep Stress Incr NO		WB 0.73	Horz(CT)		0.10 10	n/a	n/a		
BCDL 10.0		Code FBC2017/TPI2014		Matrix-S							
										Weight: 415 lb FT = 20%	

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 4-4-1 oc purlins, except end verticals.
BOT CHORD	2x6 SP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2x4 SP No.3		

REACTIONS.		(lb/size)	10=2867/0-3-8, 2=2603/0-5-8
		Max Horz	2=116(LC 8)
		Max Uplift	10=672(LC 5), 2=556(LC 8)

FORCES.		(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD	2-3=-4972/1144, 3-4=-4411/1049, 4-5=-6595/1474, 5-7=-6533/1445, 7-8=-4557/974, 9-10=-394/176	
BOT CHORD	2-16=-1038/4340, 14-16=-1469/6202, 13-14=-1587/6858, 11-13=-1369/5973, 10-11=-802/3442	
WEBS	3-16=-277/1698, 4-16=-2163/533, 4-14=-18/794, 5-14=-358/161, 5-13=-507/221, 7-13=-118/875, 7-11=-1924/537, 8-11=-307/1993, 8-10=-4097/957	

- NOTES-** (10)
- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
 - All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 - Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (if=lb) 10=672, 2=556.



Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
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February 19,2020

Continued on page 2

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6904 Parke East Blvd.
Tampa, FL 36610

Job 2256692	Truss T17	Truss Type Half Hip Girder	Qty 1	Ply 2	Edgley Const. - Fender Res. T19453528
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:24 2020 Page 2
ID: G3ByfYayRrdMRM_TE286?iye0Sx-IU8RdLPRpPKiHMG6pEsHgsNR4nU4q_smG8L4fGzjdr5

NOTES- (10)

9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 171 lb down and 184 lb up at 7-0-0, 103 lb down and 81 lb up at 9-0-12, 103 lb down and 81 lb up at 11-0-12, 103 lb down and 81 lb up at 13-0-12, 103 lb down and 81 lb up at 15-0-12, 103 lb down and 81 lb up at 17-0-12, 103 lb down and 81 lb up at 19-0-12, 103 lb down and 81 lb up at 21-0-12, 103 lb down and 81 lb up at 23-0-12, 103 lb down and 81 lb up at 25-0-12, 103 lb down and 81 lb up at 27-0-12, 103 lb down and 81 lb up at 29-0-12, 103 lb down and 81 lb up at 31-0-12, 103 lb down and 81 lb up at 33-0-12, and 114 lb down and 79 lb up at 35-0-12, and 132 lb down and 77 lb up at 35-7-12 on top chord, and 288 lb down and 91 lb up at 7-0-0, 85 lb down at 9-0-12, 85 lb down at 11-0-12, 85 lb down at 13-0-12, 85 lb down at 15-0-12, 85 lb down at 17-0-12, 85 lb down at 19-0-12, 85 lb down at 21-0-12, 85 lb down at 23-0-12, 85 lb down at 25-0-12, 85 lb down at 27-0-12, 85 lb down at 29-0-12, 85 lb down at 31-0-12, and 85 lb down at 33-0-12, and 93 lb down at 35-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-9=-54, 2-10=-20

Concentrated Loads (lb)

Vert: 3=-171(B) 9=-132(B) 16=-250(B) 17=-103(B) 18=-103(B) 19=-103(B) 20=-103(B) 21=-103(B) 22=-103(B) 23=-103(B) 24=-103(B) 25=-103(B) 26=-103(B) 27=-103(B) 28=-103(B) 29=-103(B) 30=-114(B) 31=-51(B) 32=-51(B) 33=-51(B) 34=-51(B) 35=-51(B) 36=-51(B) 37=-51(B) 38=-51(B) 39=-51(B) 40=-51(B) 41=-51(B) 42=-51(B) 43=-51(B) 44=-55(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MH-1473 rev. 10/03/2015 BEFORE USE.

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Tampa, FL 36610

Job 2256692	Truss T18	Truss Type Hip	Qty 1	Ply 1	Edgley Const. - Fender Res.	T19453529
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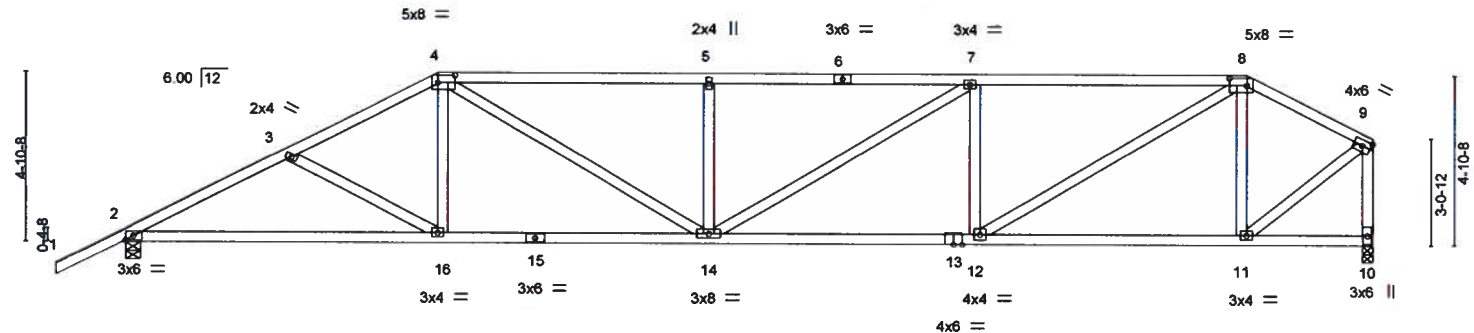
Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08 16:25 2020 Page 1

ID:G3BylYayRrdMRM_TE286?ye0Sx-DghqrhQ3ajSZvWwJNxnWD3weHBmNZQovUo5eCjzdr4



Scale: 3/16"=1'



	9-0-0	16-9-4	24-4-12	32-2-0	35-9-8
	9-0-0	7-9-4	7-7-8	7-9-4	3-7-8

Plate Offsets (X,Y)– [4:0-6-0,0-2-8], [8:0-6-0,0-2-8]									
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.39	Vert(LL)	-0.19	2-16	>999	240	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.91	Vert(CT)	-0.41	2-16	>999	180	
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.81	Horz(CT)	0.09	10	n/a	n/a	
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S						
								Weight: 193 lb	FT = 20%

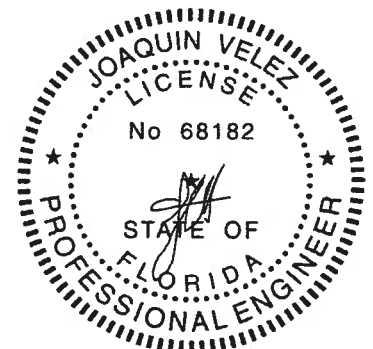
LUMBER-
TOP CHORD 2x4 SP No.2 *Except*
 4-6,6-8: 2x4 SP M 31
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 3-7-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-5-14 oc bracing.

REACTIONS. (lb/size) 2=1435/0-5-8, 10=1307/0-3-8
 Max Horz 2=113(LC 12)
 Max Uplift 2=169(LC 9), 10=174(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=2391/865, 3-4=2190/784, 4-5=2585/974, 5-7=2585/974, 7-8=2269/853,
 8-9=1102/397, 9-10=1289/465
BOT CHORD 2-16=818/2052, 14-16=667/1922, 12-14=778/2269, 11-12=309/938
WEBS 4-16=25/377, 4-14=275/853, 5-14=440/251, 7-14=141/381, 7-12=635/325,
 8-12=545/1561, 8-11=612/282, 9-11=394/1204

- NOTES-** (8)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=169, 10=174.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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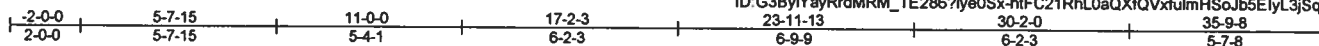
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Job 2256692	Truss T19	Truss Type Hip	Qty 1	Ply 1	Edgley Const. - Fender Res.	T19453530
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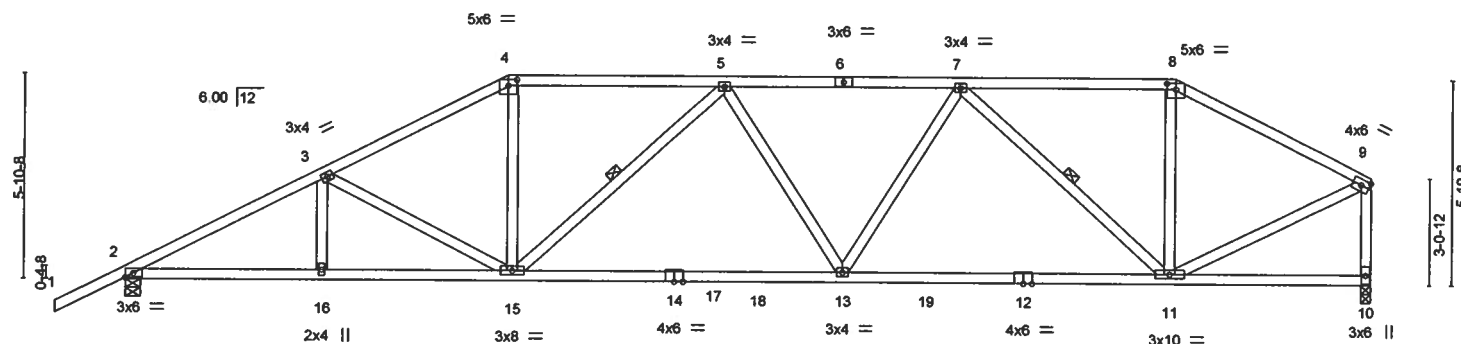
Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:26 2020 Page 1

ID: G3BylYayRrdMRM_TE2867iye0Sx-hlFC21RhL0aQXfQVxfulmHSoJb5ElyL3jSqBk9zjdr3



Scale: 3/16"=1'



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LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 3-6-6 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 5-15, 7-11

REACTIONS.

(lb/size) 2=1435/0-5-8, 10=1307/0-3-8
Max Horz 2=123(LC 12)
Max Uplift 2=160(LC 12), 10=150(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=2435/855, 3-4=2052/762, 4-5=1790/729, 5-7=2031/784, 7-8=1141/484,
8-9=1333/485, 9-10=1266/478
BOT CHORD 2-16=805/2089, 15-16=805/2089, 13-15=729/2087, 11-13=640/1853
WEBS 3-15=358/223, 4-15=168/609, 5-15=507/163, 7-13=51/407, 7-11=1005/372,
8-11=28/345, 9-11=387/1235

NOTES- (8)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCPi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 2=160, 10=150.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6804 Parke East Blvd. Tampa FL 33610
Date:

February 19,2020



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MIT-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



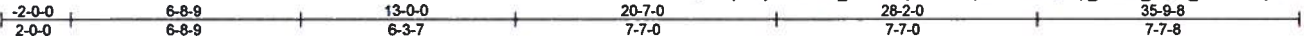
6904 Parke East Blvd.
Tampa, FL 38610

Job 2256692	Truss T20	Truss Type Hip	Qty 1	Ply 1	Edgley Const. - Fender Res. T19453531
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Builders FirstSource, Jacksonville, FL - 32244,

8 240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:27 2020 Page 1

ID:G3ByYayRrdMRM_TE2867ye0Sx-93paGNSJ6KIH8p_hUMP_IU7rJ_W81P5Cy6akGbzdr2



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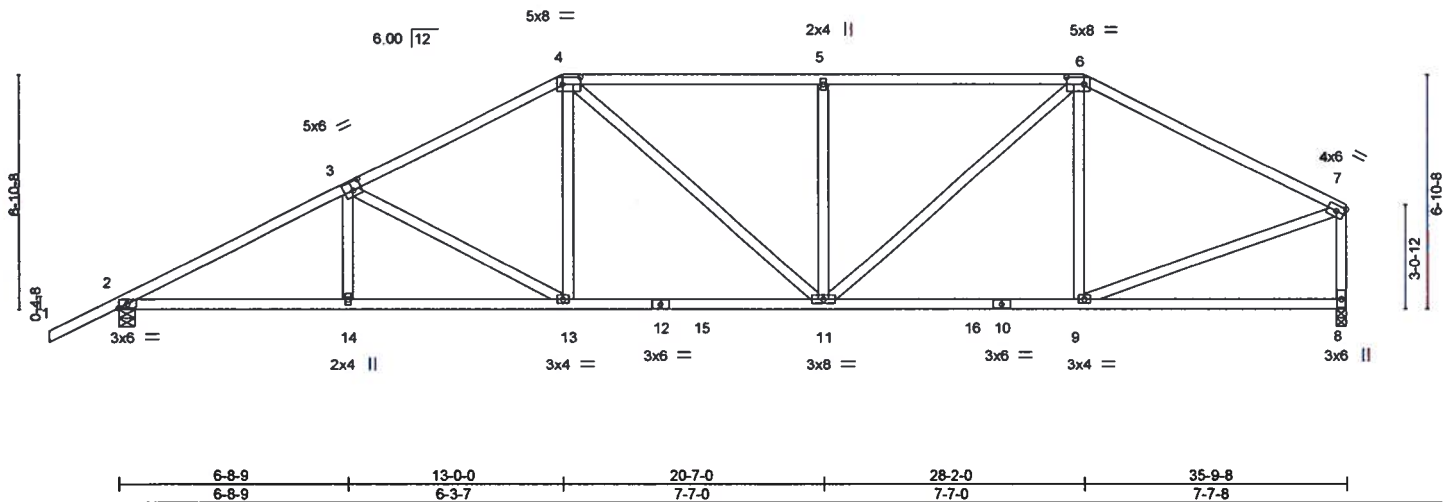


Plate Offsets (X,Y) - [3:0-3-0,0-3-0], [4:0-6-0,0-2-8], [6:0-6-0,0-2-8]

LOADING (psf)	SPACING-		CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0		TC 0.93	Vert(LL)	-0.14 11-13	>999	240	MT20	244/190
TCDL 7.0	Plate Grip DOL 1.25		BC 0.63	Vert(CT)	-0.29 11-13	>999	180		
BCLL 0.0 *	Lumber DOL 1.25		WB 0.50	Horz(CT)	0.08 8	n/a	n/a		
BCDL 10.0	Rep Stress Incr YES		Matrix-S						
	Code FBC2017/TPI2014								
								Weight: 201 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

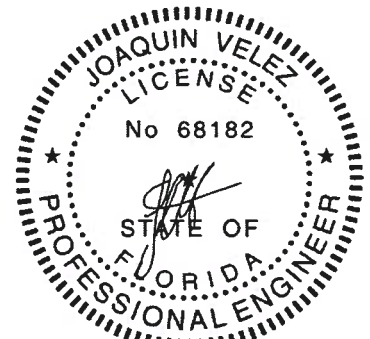
BRACING-
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-7-7 oc bracing.

REACTIONS. (lb/size) 2=1435/0-5-8, 8=1307/0-3-8
Max Horz 2=133(LC 12)
Max Uplift 2=172(LC 12), 8=124(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=2418/857, 3-4=1914/739, 4-5=1775/751, 5-6=1775/751, 6-7=1452/541, 7-8=1237/494
BOT CHORD 2-14=798/2070, 13-14=798/2070, 11-13=553/1646, 9-11=387/1213
WEBS 3-14=0/270, 3-13=492/280, 4-13=91/449, 4-11=102/315, 5-11=467/255, 6-11=270/809, 6-9=267/182, 7-9=376/1229

NOTES- (8)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=172, 8=124.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

February 19,2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

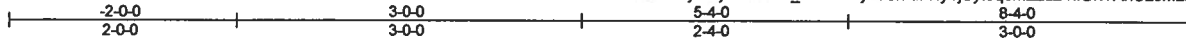
MiTek

6904 Parke East Blvd.
Tampa, FL 33610

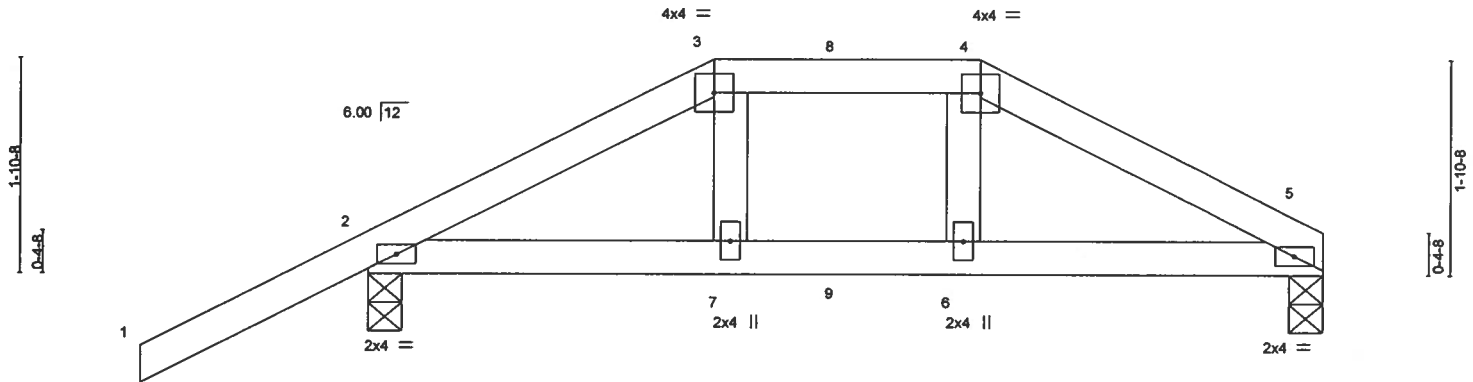
Job	Truss	Truss Type	Qty	Ply	Edgley Const. - Fender Res.	T19453532
2256692	T21	Hip Girder	1	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:28 2020 Page 1
ID:G3ByYayRdMRM_TE286?ye0Sx-dFnyTjSyteq8mzZu24wDriYAnOzmzHLAmJlo1zjdr1



Scale = 1:19.4



LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.31	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plate Grip DOL 1.25	BC 0.14	Vert(LL) -0.01 6 >999 240		
BCLL 0.0 *	Lumber DOL 1.25	WB 0.06	Vert(CT) -0.02 5-6 >999 180		
BCDL 10.0	Rep Stress Incr NO	Matrix-S	Horz(CT) 0.00 5 n/a n/a		
	Code FBC2017/TPI2014			Weight: 33 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 5=270/0-3-8, 2=416/0-3-8
Max Horz 2=36(LC 12)
Max Uplift 5=133(LC 9), 2=167(LC 8)
Max Grav 5=296(LC 42), 2=416(LC 1)

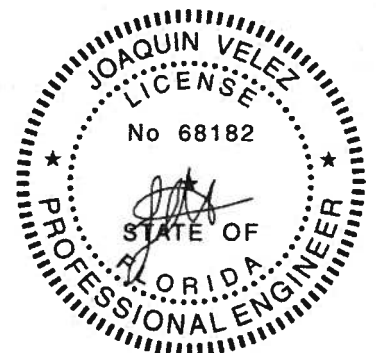
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-478/253, 3-4=-384/234, 4-5=-461/244
BOT CHORD 2-7=-192/390, 6-7=-197/395, 5-6=-192/391

NOTES- (10)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope); porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 5=133, 2=167.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 133 lb down and 138 lb up at 3-0-0, and 50 lb down and 32 lb up at 4-2-0, and 133 lb down and 138 lb up at 5-4-0 on top chord, and 52 lb down and 62 lb up at 3-0-0, and 21 lb down and 26 lb up at 4-2-0, and 52 lb down and 62 lb up at 5-3-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard

- 1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-54, 4-5=-54, 2-5=-20
Concentrated Loads (lb)
Vert: 3=36(F) 4=36(F) 7=-19(F) 6=-19(F) 9=-8(F)



Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

February 19,2020

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Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd.
Tampa, FL 36610

Job 2256692	Truss T22	Truss Type Common	Qty 2	Ply 1	Edgley Const. - Fender Res.	T19453533
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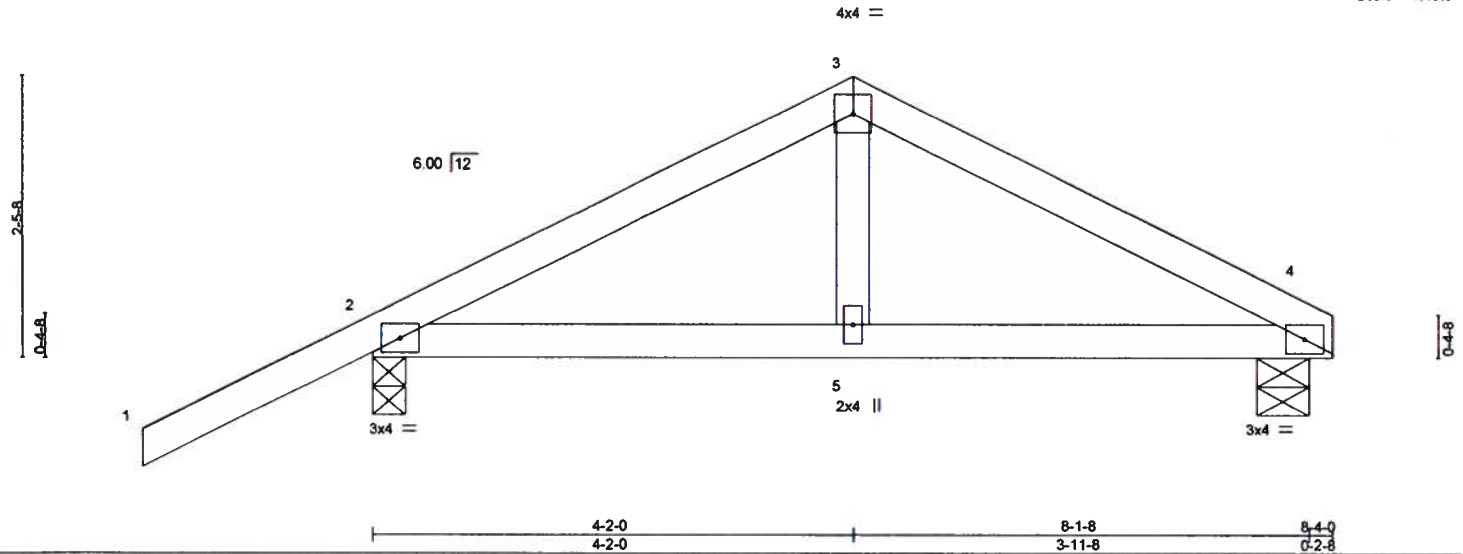
Builders FirstSource, Jacksonville, FL - 32244,

8 240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:29 2020 Page 1

ID: G3ByfYayRrdMRM_TE286?iye0Sx-5SxKh3Taexy?O784cnRSNv4KNoEIVLPVQ3rLUzjdr0



Scale = 1:19.3



LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.32	Vert(LL)	0.02	4-5	>999	240	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.46	Vert(CT)	-0.02	4-5	>999	180		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.07	Horz(CT)	0.00	4	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S							
									Weight: 33 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(lb/size) 4=279/0-5-8, 2=426/0-3-8
Max Horz 2=41(LC 16)
Max Uplift 4=74(LC 8), 2=97(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=373/425, 3-4=365/419
BOT CHORD 2-5=308/278, 4-5=308/278

NOTES- (7)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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February 19,2020

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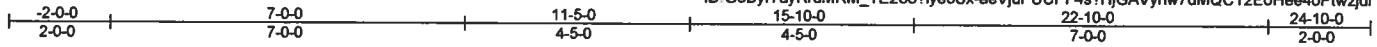


6904 Parke East Blvd.
Tampa, FL 33610

Job 2256692	Truss T23	Truss Type Hip Girder	Qty 1	Ply 1	Edgley Const. - Fender Res.	T19453534
Builders FirstSource, Jacksonville, FL - 32244,						Job Reference (optional)

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:30 2020 Page 1

ID: G3ByfYayRrdMRM_TE2867iye0Sx-aeVjuPUCPF4s?HjGAVyhw7dMQCT2EoHee4oPtwzjdr?



Scale = 1:43.7

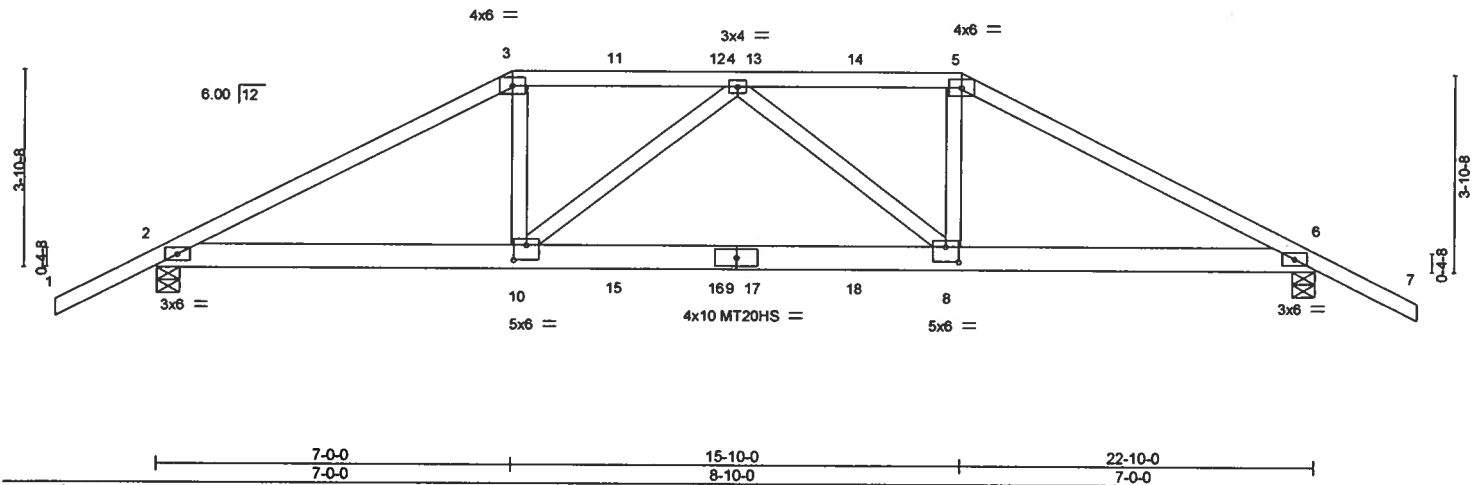


Plate Offsets (X,Y)~		[8:0-3-0,0-3-8], [10:0-3-0,0-3-8]	
LOADING (psf)	SPACING-	2-0-0	CSI.
TCLL 20.0	Plate Grip DOL	1.25	TC 0.94
TCDL 7.0	Lumber DOL	1.25	BC 0.94
BCLL 0.0	Rep Stress Incr	NO	WB 0.35
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S
			DEFL.
			in (loc) l/defl L/d
			Vert(LL) -0.14 8-10 >999 240
			Vert(CT) -0.31 8-10 >859 180
			Horz(CT) 0.06 6 n/a n/a
			PLATES GRIP
			MT20 244/190
			MT20HS 187/143
			Weight: 122 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2
WEBS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 8-6-6 oc bracing.

REACTIONS.

(lb/size) 2=1678/0-5-8, 6=1678/0-5-8
Max Horz 2=48(LC 26)
Max Uplift 2=407(LC 8), 6=407(LC 9)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

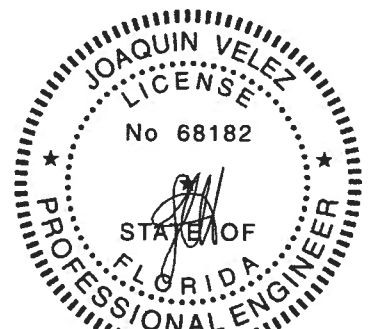
TOP CHORD 2-3=-2988/728, 3-4=-2609/683, 4-5=-2609/683, 5-6=-2988/728
BOT CHORD 2-10=-608/2574, 8-10=-746/2947, 6-8=-582/2574
WEBS 3-10=-110/911, 4-10=-500/231, 4-8=-500/231, 5-8=-110/911

NOTES- (11)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=407, 6=407.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 218 lb down and 184 lb up at 7-0-0, 103 lb down and 81 lb up at 9-0-12, 103 lb down and 81 lb up at 11-0-12, 103 lb down and 81 lb up at 11-9-4, and 103 lb down and 81 lb up at 13-9-4, and 218 lb down and 184 lb up at 15-10-0 on top chord, and 288 lb down and 91 lb up at 7-0-0, 85 lb down at 9-0-12, 85 lb down at 11-0-12, 85 lb down at 11-9-4, and 85 lb down at 13-9-4, and 288 lb down and 91 lb up at 15-9-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard

- 1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-54, 5-7=-54, 2-6=-20



Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

February 19,2020

Continued on page 2



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Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	Edgley Const. - Fender Res.	T19453534
2256692	T23	Hip Girder	1	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:30 2020 Page 2
ID: G3BylYayRrdMRM_TE286?iye0Sx-aeVjuPUCPF4s?HjGAVyhw7dMQCT2EoHee4oPtwzjdr?

LOAD CASE(S) Standard

Concentrated Loads (lb)

Vert: 3=171(B) 5=171(B) 10=250(B) 8=250(B) 11=103(B) 12=103(B) 13=103(B) 14=103(B) 15=51(B) 16=51(B) 17=51(B) 18=51(B)

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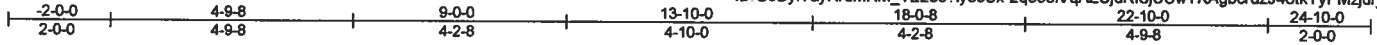


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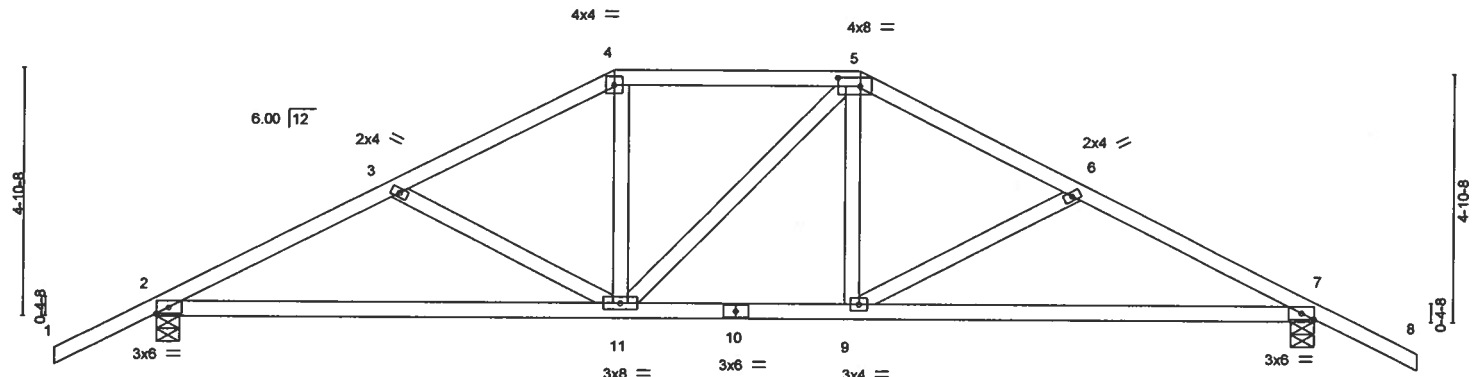
Job	Truss	Truss Type	Qty	Ply	Edgley Const. - Fender Res.	T19453535
2256692	T24	Hip	1	1		

Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:31 2020 Page 1
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Scale = 1:43.7



		9-0-0		13-10-0		22-10-0			
		9-0-0		4-10-0		9-0-0			
Plate Offsets (X,Y)– [5:0-5-4,0-2-0], [7:0-2-15,Edge]									
LOADING (psf)		SPACING- 2-0-0		CSI.		DEFL. in (loc) l/defl L/d		PLATES GRIP	
TCLL	20.0	Plate Grip DOL 1.25		TC	0.34	Vert(LL)	-0.20 7-9 >999	240	MT20 244/190
TCDL	7.0	Lumber DOL 1.25		BC	0.79	Vert(CT)	-0.40 7-9 >670	180	
BCLL	0.0 *	Rep Stress Incr YES		WB	0.12	Horz(CT)	0.04 7 n/a	n/a	
BCDL	10.0	Code FBC2017/TPI2014		Matrix-S					
								Weight: 114 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 4-10-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-6-9 oc bracing.

REACTIONS.

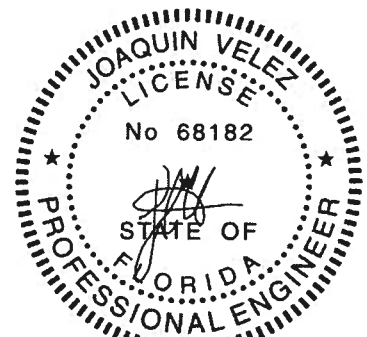
(lb/size) 2=948/0-5-8, 7=948/0-5-8
Max Horz 2=59(LC 10)
Max Uplift 2=124(LC 12), 7=124(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=1374/529, 3-4=1135/433, 4-5=973/426, 5-6=1134/433, 6-7=1374/529
BOT CHORD 2-11=341/1161, 9-11=196/972, 7-9=369/1160
WEBS 4-11=46/313, 5-9=51/313

NOTES- (8)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 2=124, 7=124.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

February 19,2020

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Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	Edgley Const. - Fender Res.	T19453536
2256692	T25	Hip	1	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:32 2020 Page 1

ID: G3BylYayRrdMRRM_TE286?ie0Sx-W1cTJ4VSxsKZFattHv?9?YrT?Hfidx5OHVwpjdqz



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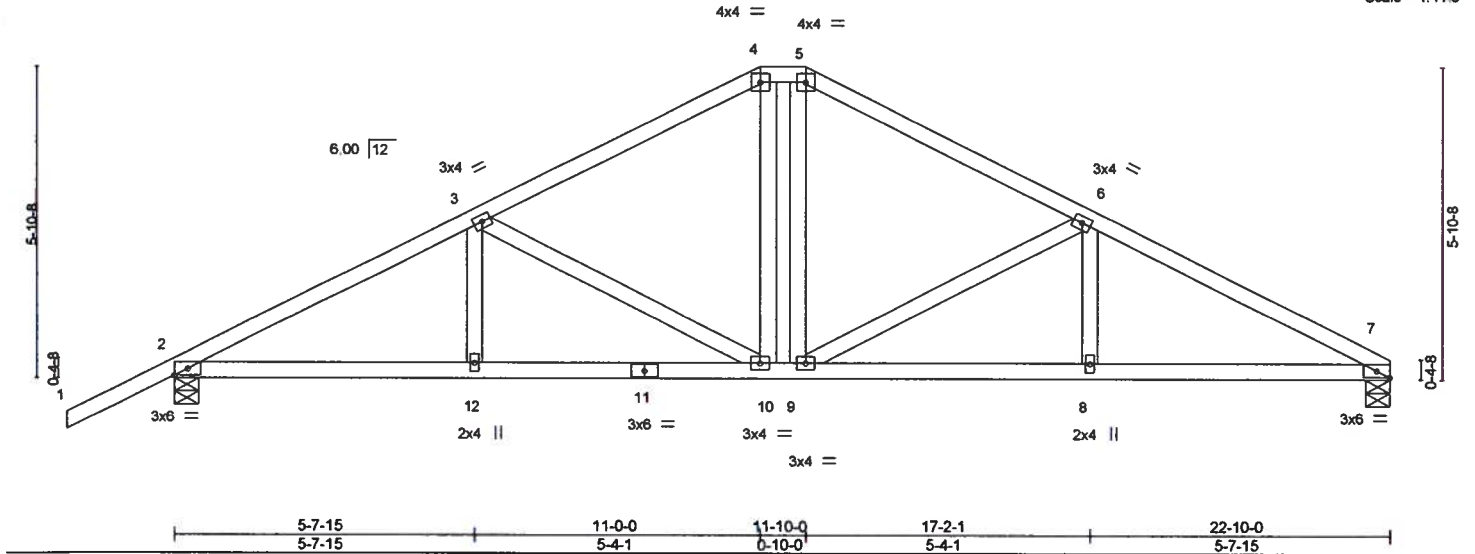


Plate Offsets (X,Y)-- [7-0-2-15,Edge]											
LOADING (psf)		SPACING- 2-0-0		CSI.		DEFL. in (loc) l/defl L/d				PLATES GRIP	
TCLL 20.0		Plate Grip DOL 1.25		TC 0.33		Vert(LL) -0.05 10-12 >999 240				MT20 244/190	
TCDL 7.0		Lumber DOL 1.25		BC 0.42		Vert(CT) -0.11 10-12 >999 180					
BCLL 0.0 *		Rep Stress Incr YES		WB 0.36		Horz(CT) 0.04 7 n/a n/a					
BCDL 10.0		Code FBC2017/TPI2014		Matrix-S						Weight: 117 lb FT = 20%	

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 4-5-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-0-13 oc bracing.

REACTIONS.

(lb/size) 7=822/0-5-8, 2=954/0-5-8
Max Horz 2=75(LC 12)
Max Uplift 7=103(LC 13), 2=134(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1430/528, 3-4=-999/433, 4-5=-836/432, 5-6=-1001/434, 6-7=-1446/547
BOT CHORD 2-12=-397/1203, 10-12=-397/1203, 9-10=-194/836, 8-9=-420/1238, 7-8=-420/1238
WEBS 3-10=-444/232, 4-10=-92/275, 5-9=-104/278, 6-9=-485/259

NOTES- (8)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7=103, 2=134.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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Date:

February 19,2020

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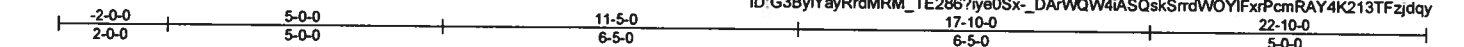
MiTek

6904 Parke East Blvd.
Tampa, FL 33610

Job 2256692	Truss T26	Truss Type Common	Qty 6	Ply 1	Edgley Const. - Fender Res.	T19453537
Builders FirstSource, Jacksonville, FL - 32244,						Job Reference (optional)

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:33 2020 Page 1

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Scale = 1:40.3

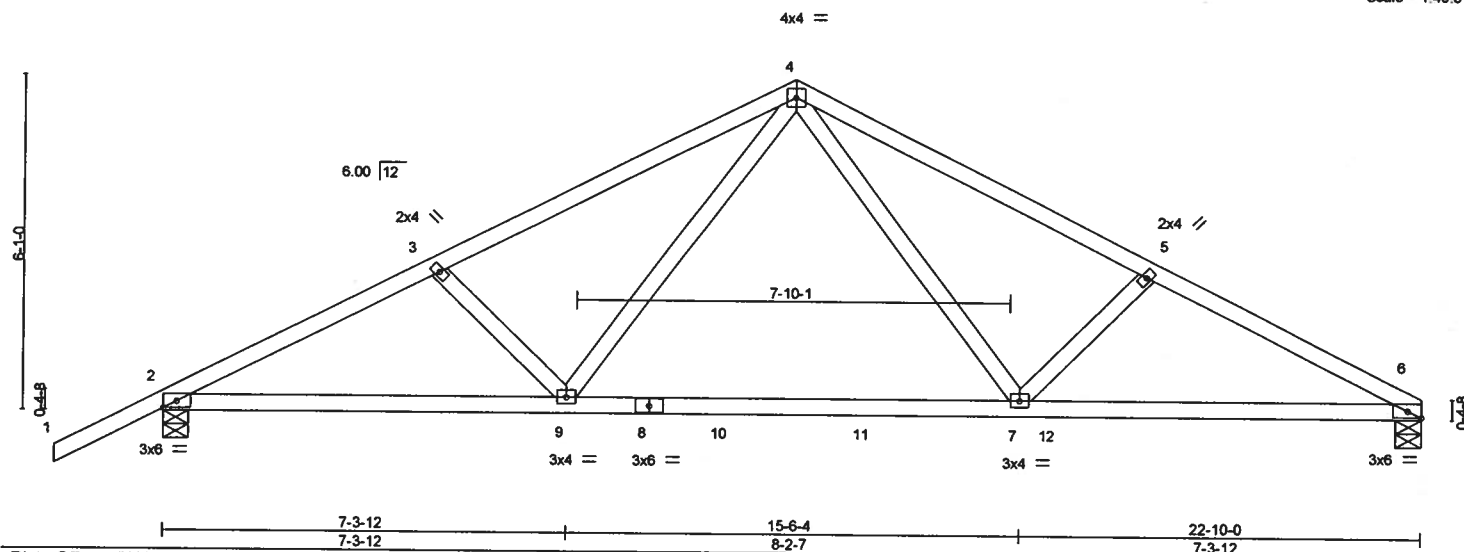


Plate Offsets (X,Y)--		[6:0-2-15,Edge]		7-3-12		8-2-7		7-3-12	
LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d
TCLL 20.0		Plate Grip DOL	1.25	TC 0.67		Vert(LL)	-0.12 7-9	>999	240
TCDL 7.0		Lumber DOL	1.25	BC 0.43		Vert(CT)	-0.38 7-9	>714	180
BCLL 0.0	•	Rep Stress Incr	NO	WB 0.25		Horz(CT)	0.04 6	n/a	n/a
BCDL 10.0		Code	FBC2017/TPI2014	Matrix-S					
						PLATES		GRIP	
						MT20		244/190	
						Weight: 106 lb		FT = 20%	

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 3-8-0 oc purlins.
BOT CHORD	2x4 SP M 31	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2x4 SP No.3		

REACTIONS. (lb/size) 6=956/0-5-8, 2=1082/0-5-8
Max Horz 2=77(LC 16)
Max Uplift 2=7(LC 12)

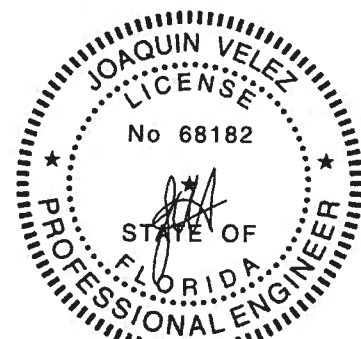
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=1743/269, 3-4=1553/221, 4-5=1589/223, 5-6=1774/276
BOT CHORD 2-9=183/1487, 7-9=14/979, 6-7=195/1534
WEBS 4-7=0/656, 5-7=287/270, 4-9=0/611, 3-9=264/255

- NOTES-** (8)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.
 - Load case(s) 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 21, 22, 23, 24, 25, 26 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard

- Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=54, 4-6=54, 2-9=20, 9-12=50, 6-12=20
- Dead + 0.75 Roof Live (balanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=44, 4-6=44, 2-9=20, 9-10=50, 10-11=80, 11-12=50, 6-12=20
- Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=14, 4-6=14, 2-9=40, 9-12=70, 6-12=40

Continued on page 2



Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

February 19,2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.

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MiTek

6904 Parke East Blvd.
Tampa, FL 36610

Job 2256692	Truss T26	Truss Type Common	Qty 6	Ply 1	Edgley Const. - Fender Res. T19453537
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:33 2020 Page 2
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LOAD CASE(S) Standard

- 4) Dead + 0.6 C-C Wind (Pos. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=60, 2-4=34, 4-6=34, 2-9=6, 9-12=36, 6-12=6
Horz: 1-2=68, 2-4=43, 4-6=43
- 5) Dead + 0.6 C-C Wind (Pos. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=29, 2-4=34, 4-6=34, 2-9=6, 9-12=36, 6-12=6
Horz: 1-2=37, 2-4=43, 4-6=43
- 6) Dead + 0.6 C-C Wind (Neg. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=5, 2-4=29, 4-6=29, 2-9=20, 9-12=50, 6-12=20
Horz: 1-2=9, 2-4=15, 4-6=15
- 7) Dead + 0.6 C-C Wind (Neg. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=23, 2-4=29, 4-6=29, 2-9=20, 9-12=50, 6-12=20
Horz: 1-2=9, 2-4=15, 4-6=15
- 8) Dead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=13, 2-4=0, 4-6=11, 2-9=6, 9-12=36, 6-12=6
Horz: 1-2=21, 2-4=9, 4-6=19
- 9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=5, 2-4=11, 4-6=0, 2-9=6, 9-12=36, 6-12=6
Horz: 1-2=14, 2-4=19, 4-6=9
- 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=11, 2-4=16, 4-6=6, 2-9=20, 9-12=50, 6-12=20
Horz: 1-2=3, 2-4=2, 4-6=8
- 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=0, 2-4=6, 4-6=16, 2-9=20, 9-12=50, 6-12=20
Horz: 1-2=14, 2-4=8, 4-6=2
- 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=13, 2-4=18, 4-6=9, 2-9=6, 9-12=36, 6-12=6
Horz: 1-2=21, 2-4=27, 4-6=17
- 13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=3, 2-4=9, 4-6=18, 2-9=6, 9-12=36, 6-12=6
Horz: 1-2=11, 2-4=17, 4-6=27
- 14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=13, 2-4=18, 4-6=9, 2-9=6, 9-12=36, 6-12=6
Horz: 1-2=21, 2-4=27, 4-6=17
- 15) Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=3, 2-4=9, 4-6=18, 2-9=6, 9-12=36, 6-12=6
Horz: 1-2=11, 2-4=17, 4-6=27
- 16) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=7, 2-4=2, 4-6=8, 2-9=20, 9-12=50, 6-12=20
Horz: 1-2=21, 2-4=16, 4-6=6
- 17) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=3, 2-4=8, 4-6=2, 2-9=20, 9-12=50, 6-12=20
Horz: 1-2=11, 2-4=6, 4-6=16
- 18) Dead + Uninhabitable Attic Storage: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=14, 4-6=14, 2-9=20, 9-10=50, 10-11=90, 11-12=50, 6-12=20
- 19) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=42, 2-4=46, 4-6=38, 2-9=20, 9-10=50, 10-11=80, 11-12=50, 6-12=20
Horz: 1-2=2, 2-4=2, 4-6=6
- 20) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=34, 2-4=38, 4-6=46, 2-9=20, 9-10=50, 10-11=80, 11-12=50, 6-12=20
Horz: 1-2=10, 2-4=6, 4-6=2
- 21) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=28, 2-4=32, 4-6=40, 2-9=20, 9-10=50, 10-11=80, 11-12=50, 6-12=20
Horz: 1-2=16, 2-4=12, 4-6=4

Continued on page 3

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6904 Parke East Blvd.
Tampa, FL 36610

Job 2256692	Truss T26	Truss Type Common	Qty 6	Ply 1	Edgley Const. - Fender Res. T19453537
Builders FirstSource, Jacksonville, FL - 32244,					Job Reference (optional)

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:33 2020 Page 3
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LOAD CASE(S) Standard

22) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=35, 2-4=40, 4-6=32, 2-9=20, 9-10=50, 10-11=80, 11-12=50, 6-12=20

Horz: 1-2=9, 2-4=4, 4-6=12

23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=54, 4-6=14, 2-9=20, 9-12=50, 6-12=20

24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=14, 4-6=54, 2-9=20, 9-12=50, 6-12=20

25) 3rd Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=44, 4-6=14, 2-9=20, 9-10=50, 10-11=80, 11-12=50, 6-12=20

26) 4th Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=14, 4-6=44, 2-9=20, 9-10=50, 10-11=80, 11-12=50, 6-12=20

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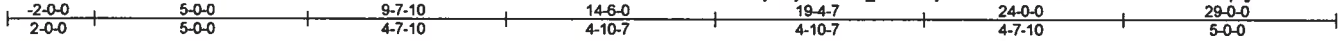
6904 Parke East Blvd.
Tampa, FL 36610

Job 2256692	Truss T27	Truss Type HIP GIRDER	Qty 1	Ply 1	Edgley Const. - Fender Res.	T19453538
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Feb 7 2020 MITek Industries, Inc. Wed Feb 19 08:16:34 2020 Page 1

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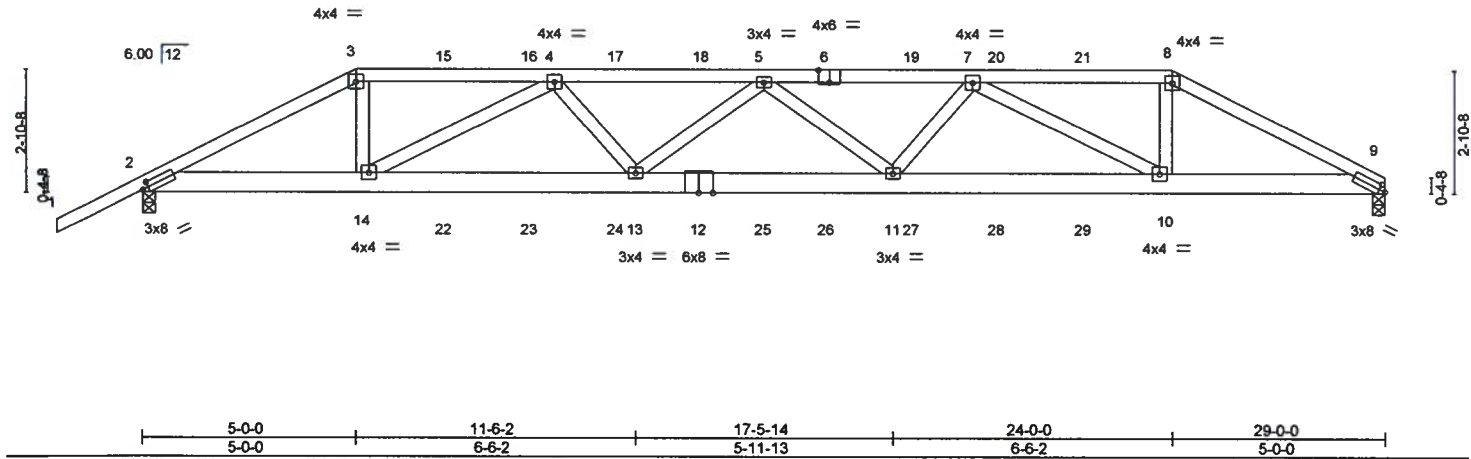


Plate Offsets (X,Y)-- [2-0-1-12,0-1-8], [6-0-3-0,Edge], [9-0-1-12,0-1-8]									
LOADING (psf)		SPACING- 2-0-0		CSI.		DEFL. in (loc) l/defl L/d		PLATES GRIP	
TCLL 20.0		Plate Grip DOL 1.25		TC 0.69		Vert(LL) 0.46 11-13 >756 240		MT20 244/190	
TCDL 7.0		Lumber DOL 1.25		BC 0.96		Vert(CT) -0.54 11-13 >635 180			
BCLL 0.0		Rep Stress Incr NO		WB 0.88		Horz(CT) 0.12 9 n/a n/a			
BCDL 10.0		Code FBC2017/TPI2014		Matrix-S				Weight: 156 lb FT = 20%	

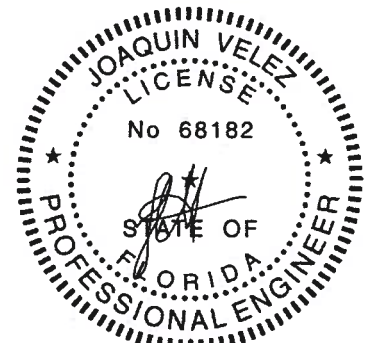
LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 2-4-11 oc purlins.
BOT CHORD 2x6 SP No.2	BOT CHORD Rigid ceiling directly applied or 4-1-10 oc bracing.
WEBS 2x4 SP No.3	

REACTIONS. (lb/size) 9=1590/0-3-8, 2=1715/0-3-8
Max Horz 2=46(LC 31)
Max Uplift 9=961(LC 4), 2=975(LC 5)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-3140/1995, 3-4=-2797/1831, 4-5=-4581/2981, 5-7=-4589/2979, 7-8=-2839/1837, 8-9=-3179/2000
BOT CHORD 2-14=-1769/2735, 13-14=-2753/4259, 11-13=-3123/4829, 10-11=-2744/4275, 9-10=-1748/2776
WEBS 3-14=-691/1068, 4-14=-1696/1104, 4-13=-353/623, 5-13=-329/225, 5-11=-322/222, 7-11=-348/618, 7-10=-1682/1097, 8-10=-696/1068

- NOTES- (10)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCp=0.18; MWFRS (envelope); porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 9=961, 2=975.
 - Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 129 lb down and 167 lb up at 5-0-0, 49 lb down and 92 lb up at 7-0-12, 49 lb down and 92 lb up at 9-0-12, 49 lb down and 92 lb up at 11-0-12, 49 lb down and 92 lb up at 13-0-12, 49 lb down and 92 lb up at 14-6-0, 49 lb down and 92 lb up at 15-11-4, 49 lb down and 92 lb up at 17-11-4, 49 lb down and 92 lb up at 19-11-4, and 49 lb down and 92 lb up at 21-11-4, and 129 lb down and 167 lb up at 24-0-0 on top chord, and 165 lb down and 131 lb up at 5-0-0, 56 lb down and 44 lb up at 7-0-12, 56 lb down and 44 lb up at 9-0-12, 56 lb down and 44 lb up at 11-0-12, 56 lb down and 44 lb up at 13-0-12, 56 lb down and 44 lb up at 14-6-0, 56 lb down and 44 lb up at 15-11-4, 56 lb down and 44 lb up at 17-11-4, 56 lb down and 44 lb up at 19-11-4, and 56 lb down and 44 lb up at 21-11-4, and 165 lb down and 131 lb up at 23-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 - in the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard
Continued on page 2



Joaquin Velez PE No.68182
MITek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

February 19,2020

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MITek
6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	Edgley Const. - Fender Res.	T19453538
2256692	T27	HIP GIRDER	1	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:34 2020 Page 2
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LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-8=-54, 8-9=-54, 2-9=-20

Concentrated Loads (lb)

Vert: 3=-82(B) 6=-49(B) 8=-82(B) 12=-28(B) 14=-102(B) 5=-49(B) 10=-102(B) 15=-49(B) 16=-49(B) 17=-49(B) 18=-49(B) 19=-49(B) 20=-49(B) 21=-49(B) 22=-28(B)
23=-28(B) 24=-28(B) 25=-28(B) 26=-28(B) 27=-28(B) 28=-28(B) 29=-28(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 rev. 10/03/2015 BEFORE USE.

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6904 Parke East Blvd.
Tampa, FL 36610

Job 2256692	Truss T28	Truss Type Hip	Qty 1	Ply 1	Edgley Const. - Fender Res.	T19453539
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Builders FirstSource, Jacksonville, FL - 32244,

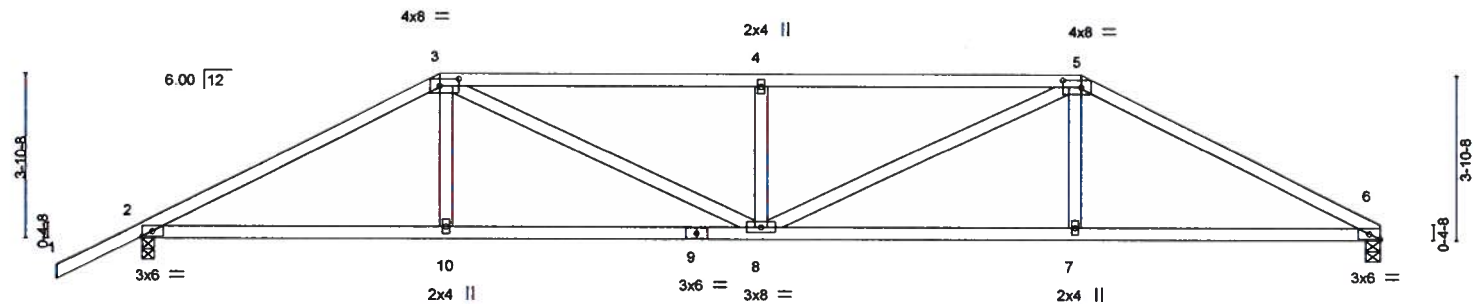
8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:35 2020 Page 1

ID:G3By/YayRdMRM_TE286?iye0Sx-wclcx6YLDni862cEy2YsdAKDHDEZvyjNnMWAX7zjdqw

Job Reference (optional)



Scale = 1:52.0



	7-0-0	14-6-0	22-0-0	29-0-0
	7-0-0	7-6-0	7-6-0	7-0-0
Plate Offsets (X,Y)-	[3-0-5-4, 0-2-0], [5-0-5-4, 0-2-0], [6-0-2-15, Edge]			

LOADING (psf)	SPACING-		CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	2-0-0	TC 0.93	Vert(LL)	0.22	8-10	>999	240	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.66	Vert(CT)	-0.27	8-10	>999	180		
BCLL 0.0	Rep Stress Incr	YES	WB 0.72	Horz(CT)	0.07	6	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S						Weight: 130 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 4-6-12 oc bracing.

REACTIONS.

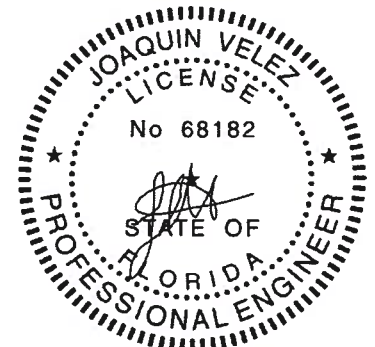
(lb/size) 6=1057/0-4-0, 2=1182/0-3-8
Max Horz 2=55(LC 12)
Max Uplift 6=379(LC 8), 2=393(LC 9)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=1875/1626, 3-4=2208/2022, 4-5=2208/2022, 5-6=1892/1637
BOT CHORD 2-10=1355/1586, 8-10=1366/1593, 7-8=1379/1613, 6-7=1367/1606
WEBS 3-10=276/315, 3-8=571/773, 4-8=454/243, 5-8=543/763, 5-7=280/319

NOTES- (8)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf, BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=379, 2=393.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

February 19,2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-88 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

MiTek

6904 Parke East Blvd.
Tampa, FL 33610

Job 2256692	Truss T29	Truss Type Hip	Qty 1	Ply 1	Edgley Const. - Fender Res.	T19453540
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Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:36 2020 Page 1

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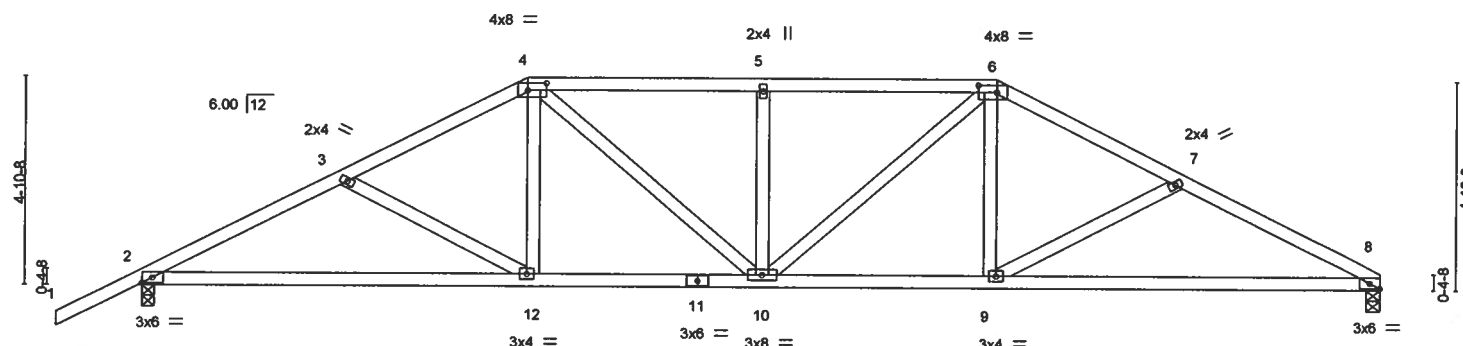


Plate Offsets (X,Y)~	[4:0-5-4,0-2-0], [6:0-5-4,0-2-0], [8:0-2-15,Edge]
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LOADING (psf)	SPACING-	CSL	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.35	Vert(LL) -0.20	8-9	>999	240	MT20	244/190
TCDL 7.0	Plate Grip DOL 1.25	BC 0.87	Vert(CT) -0.42	8-9	>814	180		
BCLL 0.0	Lumber DOL 1.25	WB 0.15	Horz(CT) 0.07	8	n/a	n/a		
BCDL 10.0	Rep Stress Incr YES	Matrix-S						
	Code FBC2017/TPI2014							
							Weight: 146 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 3-10-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-7-1 oc bracing.

REACTIONS.

(lb/size) 8=1057/0-4-0, 2=1182/0-3-8
Max Horz 2=65(LC 12)
Max Uplift 8=107(LC 13), 2=137(LC 12)

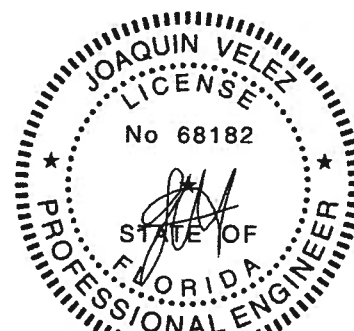
FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=1901/719, 3-4=1661/621, 4-5=1684/699, 5-6=1684/699, 6-7=1674/630,
7-8=1918/741
BOT CHORD 2-12=573/1630, 10-12=399/1444, 9-10=405/1454, 8-9=600/1664
WEBS 4-12=40/382, 4-10=143/397, 5-10=336/188, 6-10=129/390, 6-9=53/386,
7-9=252/222

NOTES- (8)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl.,
GCPi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber
DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb)
8=107, 2=137.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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Date:

February 19,2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIH-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

MiTek

6904 Parke East Blvd.
Tampa, FL 33610

Job 2256692	Truss T30	Truss Type HIP	Qty 1	Ply 1	Edgley Const. - Fender Res.	T19453541
Builders FirstSource, Jacksonville, FL - 32244,						Job Reference (optional)

8 240 s Feb 7 2020 MiTek Industries, Inc. Wed Feb 19 08:16:37 2020 Page 1
ID: G3ByfYayRdMRM_TE286?ie0Sx-s_QMMoZblPysLMlc4TbKibQct0yBNyPgFg?Gc0zjdqu



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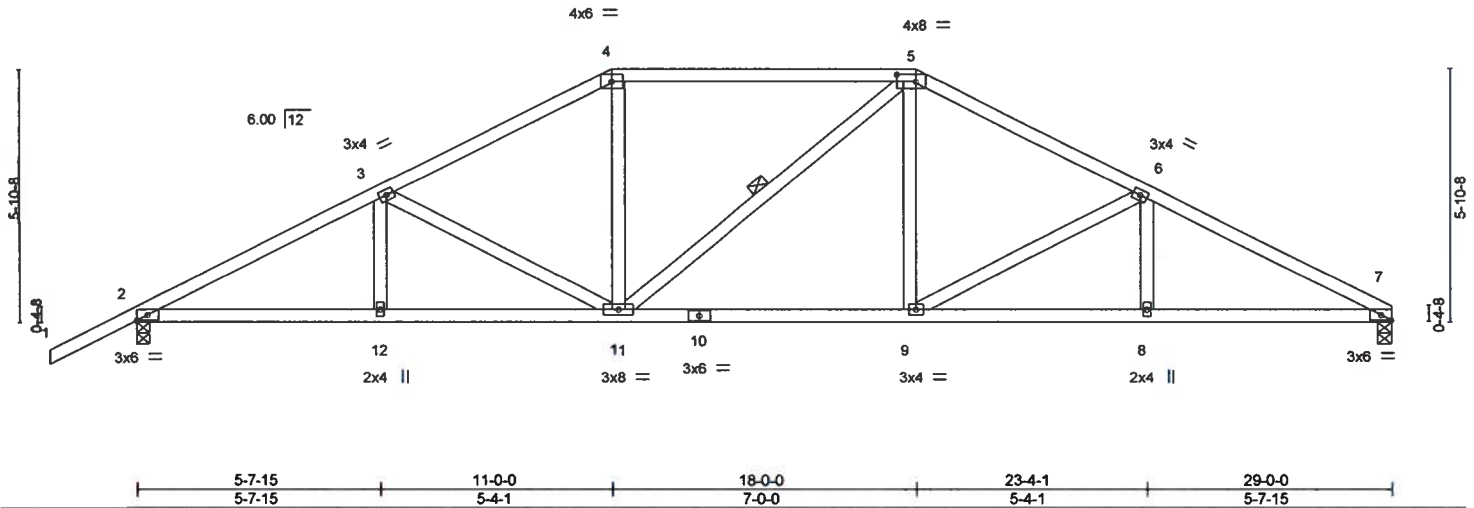


Plate Offsets (X,Y) - [5:0-5-4,0-2-0], [7:0-2-15,Edge]

LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.73	Vert(LL)	0.18	9-11	>999	MT20	244/190
TCDL 7.0	Plate Grip DOL 1.25	BC 0.52	Vert(CT)	-0.20	9-11	>999		
BCLL 0.0	Lumber DOL 1.25	WB 0.32	Horz(CT)	0.07	7	n/a		
BCDL 10.0	Rep Stress Incr YES	Matrix-S						
	Code FBC2017/TPI2014						Weight: 147 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 3-1-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-6-15 oc bracing.
WEBS 1 Row at midpt 5-11

REACTIONS.

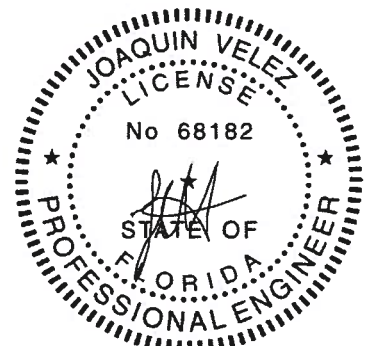
(lb/size) 7=1057/0-4-0, 2=1182/0-3-8
Max Horz 2=75(LC 12)
Max Uplift 7=332(LC 8), 2=346(LC 9)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1934/1722, 3-4=-1517/1395, 4-5=-1306/1303, 5-6=-1522/1398, 6-7=-1949/1741
BOT CHORD 2-12=-1458/1652, 11-12=-1458/1652, 9-11=-1067/1309, 8-9=-1478/1685, 7-8=-1478/1685
WEBS 3-11=-405/456, 4-11=-426/393, 5-9=-432/394, 6-9=-439/478

NOTES- (8)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7=332, 2=346.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Joaquin Velez PE No.68182
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

February 19,2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

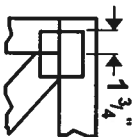
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



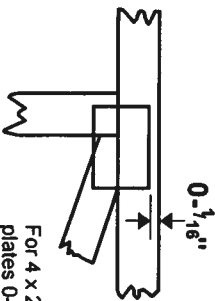
6904 Parke East Blvd.
Tampa, FL 33610

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/8" from outside edge of truss.

— This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MITek 20/20 software or upon request.

PLATE SIZE

4 X 4

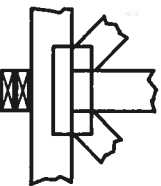
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



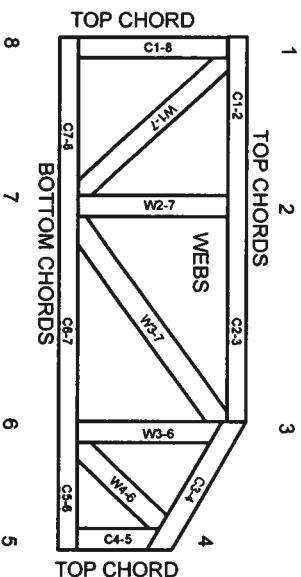
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MITek Engineering Reference Sheet: MII-7473 rev. 10/03/2015

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

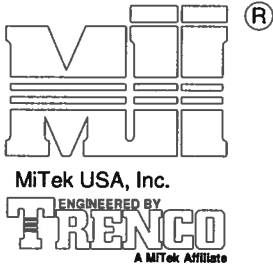
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

AUGUST 1, 2016

T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

MII-T-BRACE 2

MiTek USA, Inc. Page 1 of 1



Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

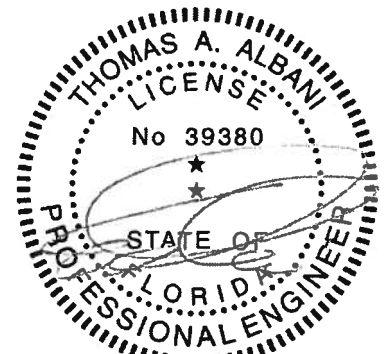
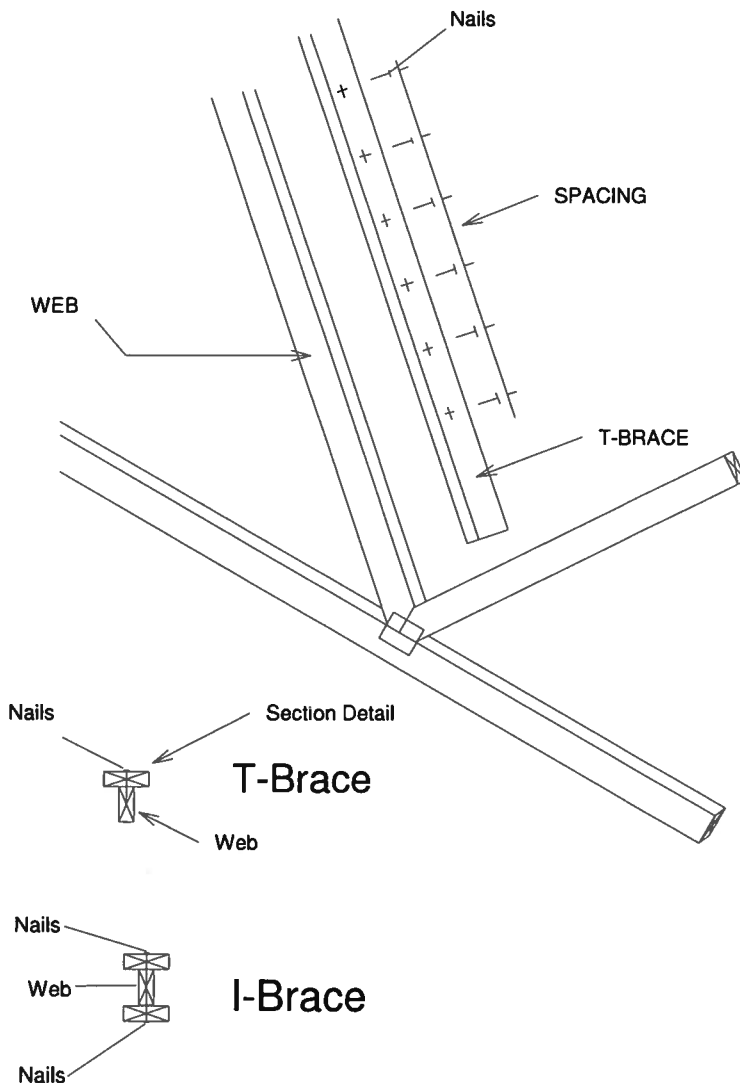
Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

Nailing Pattern		
T-Brace size	Nail Size	Nail Spacing
2x4 or 2x6 or 2x8	10d (0.131" X 3")	6" o.c.
Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)		

Brace Size for One-Ply Truss		
Specified Continuous Rows of Lateral Bracing		
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

Brace Size for Two-Ply Truss		
Specified Continuous Rows of Lateral Bracing		
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

T-Brace / I-Brace must be same species and grade (or better) as web member.



Thomas A. Albani PE No.39380
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Date:

February 12, 2018

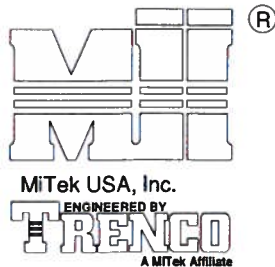
AUGUST 1, 2016

SCAB-BRACE DETAIL

MII-SCAB-BRACE

MiTek USA, Inc.

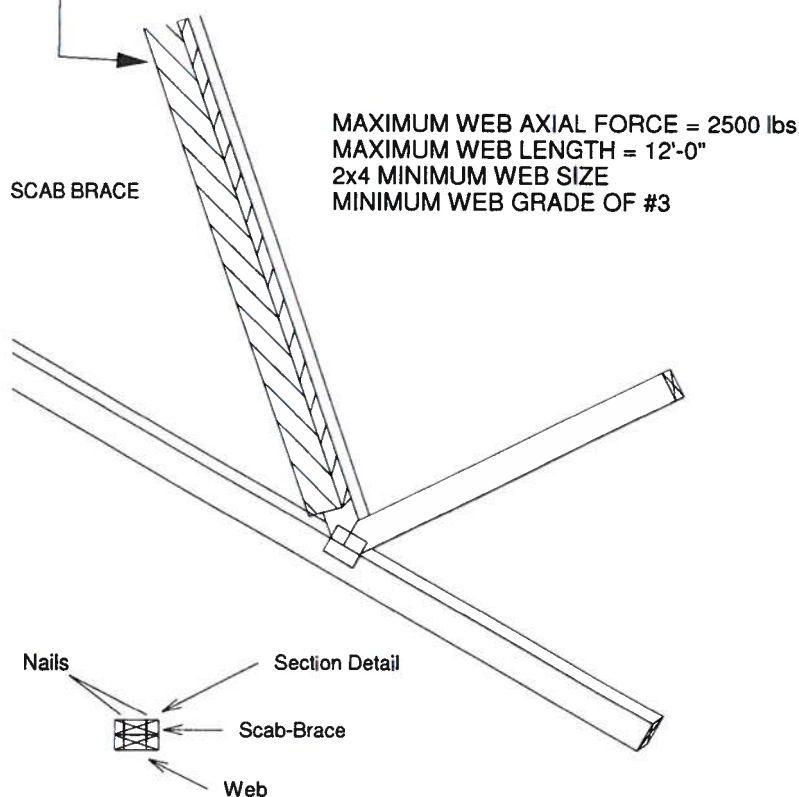
Page 1 of 1



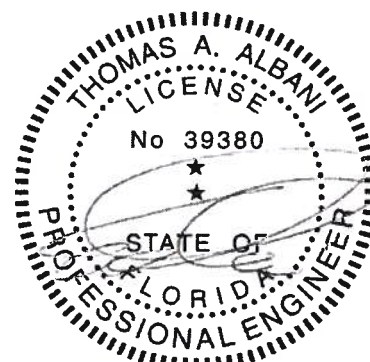
Note: Scab-Bracing to be used when continuous lateral bracing at midpoint (or T-Brace) is impractical.
Scab must cover full length of web +/- 6".

*** THIS DETAIL IS NOT APPLICABLE WHEN BRACING IS ***
REQUIRED AT 1/3 POINTS OR I-BRACE IS SPECIFIED.

APPLY 2x SCAB TO ONE FACE OF WEB WITH
2 ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C.
SCAB MUST BE THE SAME GRADE, SIZE AND
SPECIES (OR BETTER) AS THE WEB.



Scab-Brace must be same species grade (or better) as web member.



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Date:

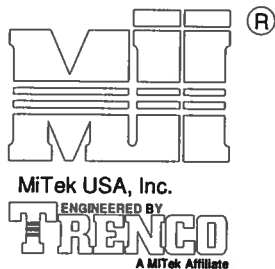
February 12, 2018

AUGUST 1, 2016

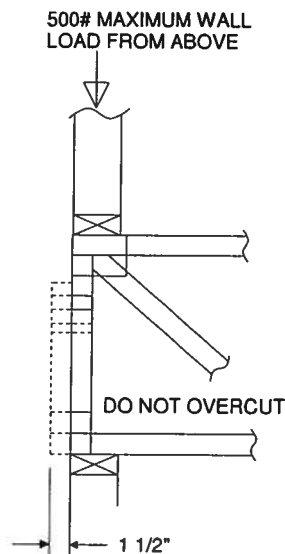
STANDARD REPAIR TO REMOVE END VERTICAL (RIBBON NOTCH VERTICAL)

MII-REP05

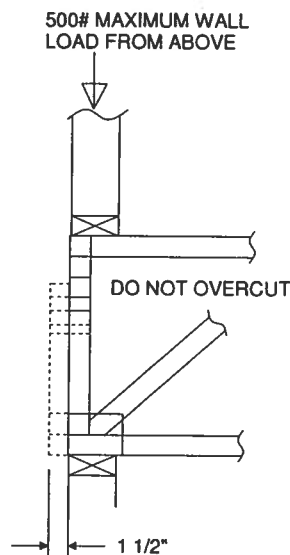
MiTek USA, Inc. Page 1 of 1



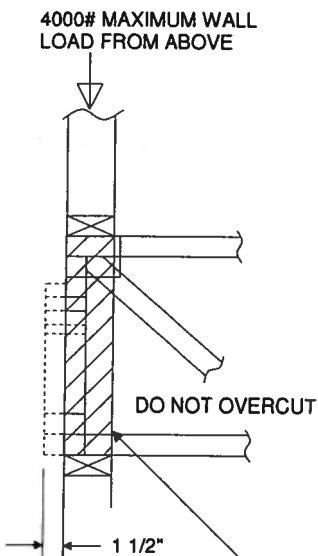
1. THIS IS A SPECIFIC REPAIR DETAIL TO BE USED ONLY FOR ITS ORIGINAL INTENTION. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.
2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLYING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.
3. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID SPLITTING OF THE WOOD.
4. LUMBER MUST BE CUT CLEANLY AND ACCURATELY AND THE REMAINING WOOD MUST BE UNDAMAGED.
5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 4X ORIENTATION ONLY.
6. CONNECTOR PLATES MUST BE FULLY IMBEDDED AND UNDISTURBED.



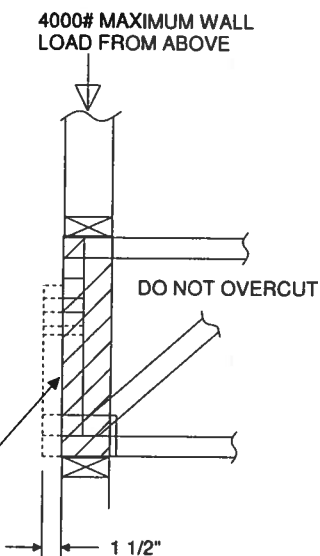
REFER TO INDIVIDUAL TRUSS DESIGN FOR PLATE SIZES AND LUMBER GRADES



TRUSSES BUILT WITH 4x2 MEMBERS

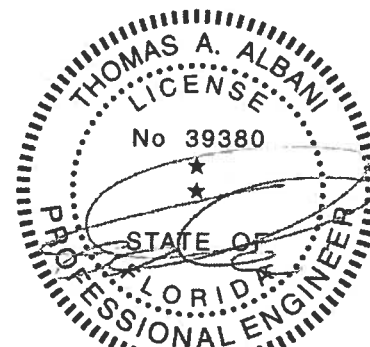


REFER TO INDIVIDUAL TRUSS DESIGN FOR PLATE SIZES AND LUMBER GRADES



TRUSSES BUILT WITH 4x2 MEMBERS

ATTACH 2x4 SQUASH BLOCK (CUT TO FIT TIGHTLY) TO BOTH SIDES OF THE TRUSS AS SHOWN WITH 10d (0.131" X 3") NAILS SPACED 3" O.C.



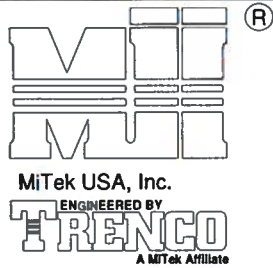
Thomas A. Albani PE No. 39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

February 12, 2018

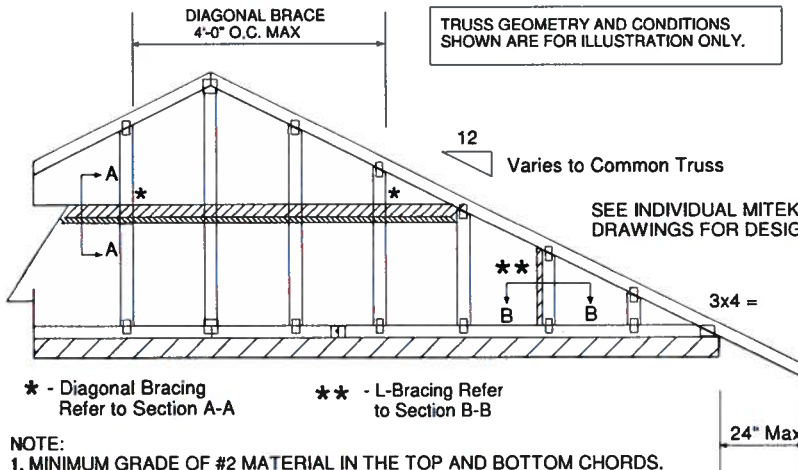
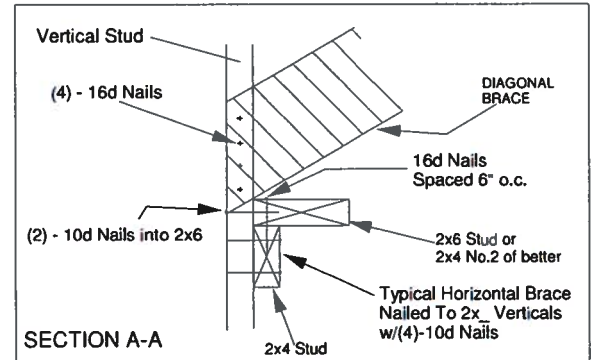
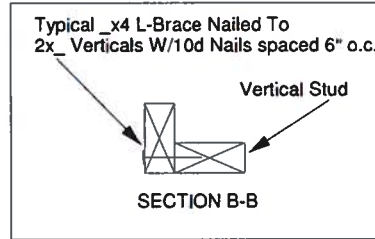
AUGUST 1, 2016

Standard Gable End Detail

MII-GE130-D-SP



MiTek USA, Inc. Page 1 of 2

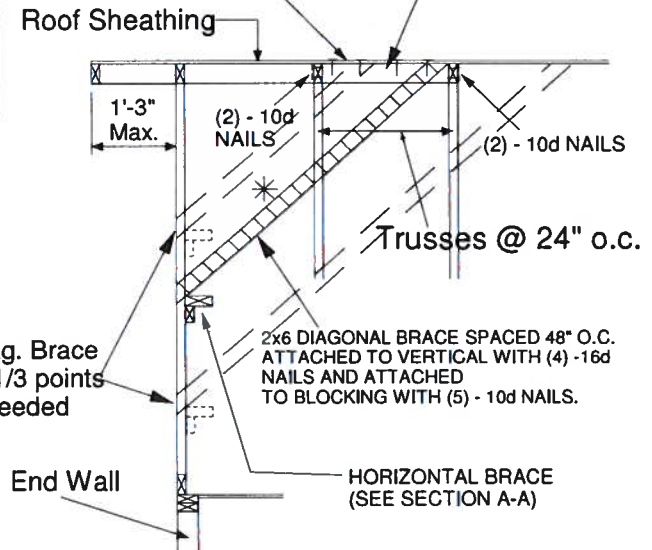


NOTE:

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

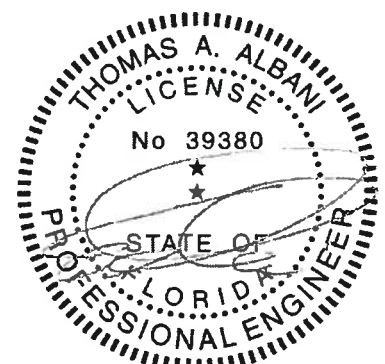


Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
			Maximum Stud Length			
2x4 SP No. 3 / Stud	12" O.C.	3-9-13	4-1-1	5-9-6	7-1-3	11-5-7
2x4 SP No. 3 / Stud	16" O.C.	3-5-4	3-6-8	5-0-2	6-10-8	10-3-13
2x4 SP No. 3 / Stud	24" O.C.	2-9-11	2-10-11	4-1-1	5-7-6	8-5-1

- * Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE D
ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH
ASCE 7-10 160 MPH
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.
CONNECTION OF BRACING IS BASED ON MWFRS.



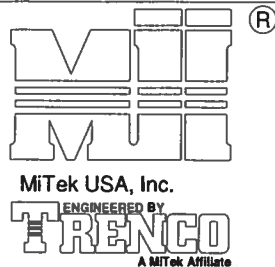
Thomas A. Albani PE No.39380
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6904 Parke East Blvd. Tampa FL 33610
Date:

February 12, 2018

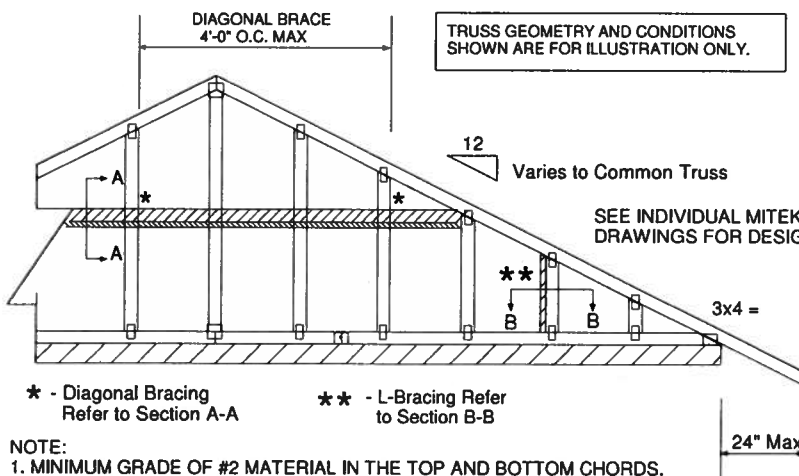
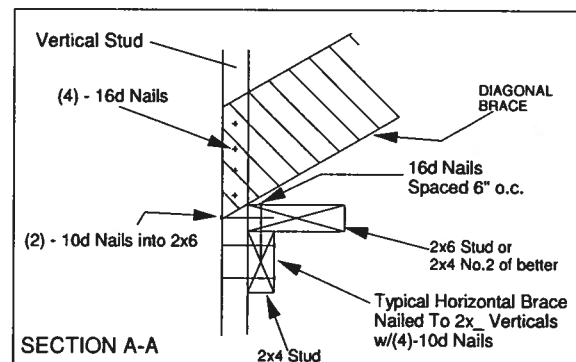
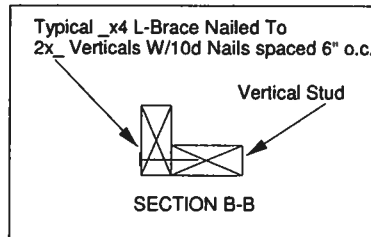
AUGUST 1, 2016

Standard Gable End Detail

MII-GE130-SP

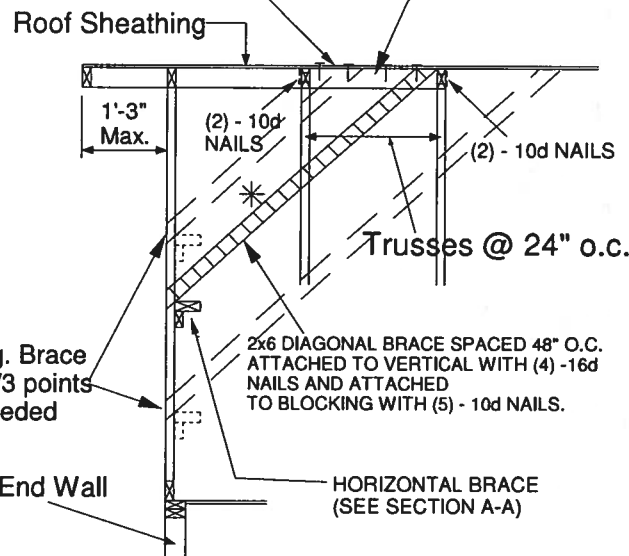


MiTek USA, Inc. Page 1 of 2



PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK



NOTE:

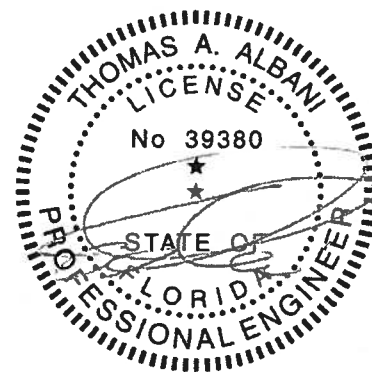
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
			Maximum Stud Length			
2x4 SP No. 3 / Stud	12" O.C.	4-0-7	4-5-6	6-3-8	8-0-15	12-1-6
2x4 SP No. 3 / Stud	16" O.C.	3-8-0	3-10-4	5-5-6	7-4-1	11-0-1
2x4 SP No. 3 / Stud	24" O.C.	3-0-10	3-1-12	4-5-6	6-1-5	9-1-15

- * Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH
ASCE 7-10 160 MPH
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.
CONNECTION OF BRACING IS BASED ON MWFRS.



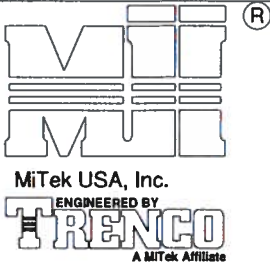
Thomas A. Albani PE No.39380
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6904 Parke East Blvd. Tampa FL 33610
Date:

February 12, 2018

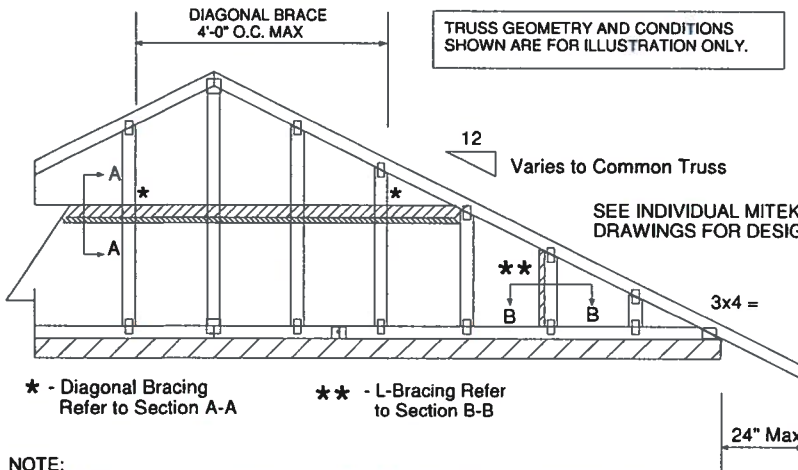
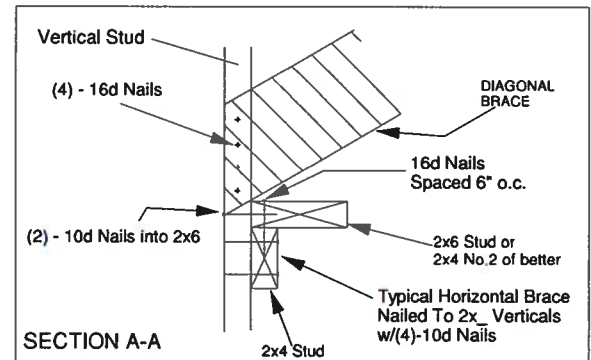
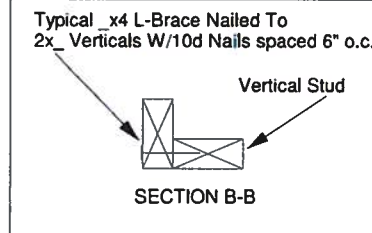
JANUARY 6, 2017

Standard Gable End Detail

MII-GE140-001



MiTek USA, Inc. Page 1 of 2

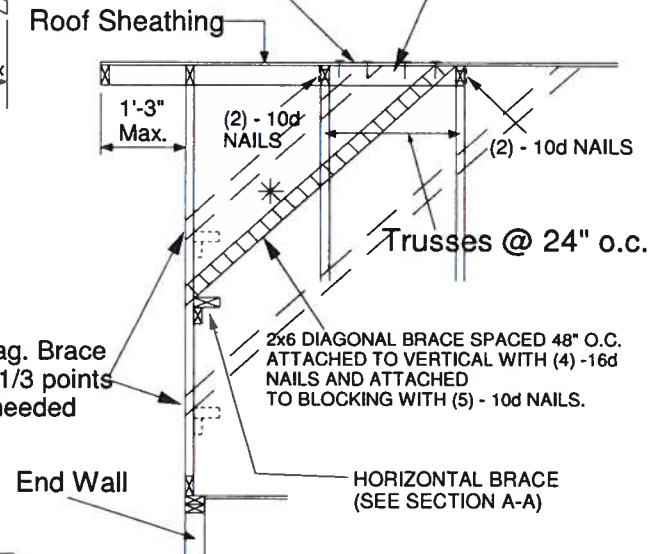


NOTE:

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
10. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD DF/SPF BLOCK

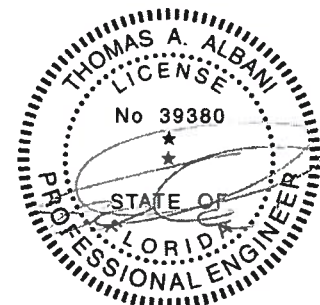


Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
		Maximum Stud Length				
2x4 DF/SPF Std/Stud	12" O.C.	3-10-1	3-11-7	5-7-2	7-8-2	11-6-4
2x4 DF/SPF Std/Stud	16" O.C.	3-3-14	3-5-1	4-10-2	6-7-13	9-11-11
2x4 DF/SPF Std/Stud	24" O.C.	2-8-9	2-9-8	3-11-7	5-5-2	8-1-12

- * Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAXIMUM WIND SPEED = 140 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-98, ASCE 7-02, ASCE 7-05
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.
CONNECTION OF BRACING IS BASED ON MWFRS.



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Date:

January 19, 2018

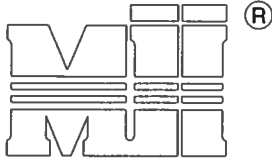
AUGUST 1, 2016

Standard Gable End Detail

MII-GE170-D-SP

MiTek USA, Inc.

Page 1 of 2



MiTek USA, Inc.

 ENGINEERED BY
TRENCO
 A MiTek Affiliate

 Typical 2x4 L-Brace Nailed To
 2x4 Verticals W/10d Nails spaced 6" o.c.

Vertical Stud

SECTION B-B

 TRUSS GEOMETRY AND CONDITIONS
 SHOWN ARE FOR ILLUSTRATION ONLY.

 12
 Varies to Common Truss

 SEE INDIVIDUAL MITTEK ENGINEERING
 DRAWINGS FOR DESIGN CRITERIA

3x4 =

24" Max

 * - Diagonal Bracing
 Refer to Section A-A

 ** - L-Bracing Refer
 to Section B-B

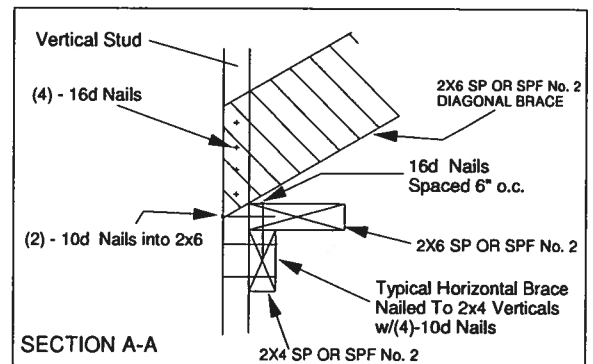
NOTE:

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3 OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
		Maximum Stud Length			
2x4 SP No. 3 / Stud	12" O.C.	3-9-7	5-8-8	6-11-1	11-4-4
2x4 SP No. 3 / Stud	16" O.C.	3-4-12	4-11-15	6-9-8	10-2-3
2x4 SP No. 3 / Stud	24" O.C.	2-9-4	4-0-7	5-6-8	8-3-13
2x4 SP No. 2	12" O.C.	3-11-13	5-8-8	6-11-1	11-11-7
2x4 SP No. 2	16" O.C.	3-7-7	4-11-5	6-11-1	10-10-5
2x4 SP No. 2	24" O.C.	3-1-15	4-0-7	6-3-14	9-5-14

- * Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

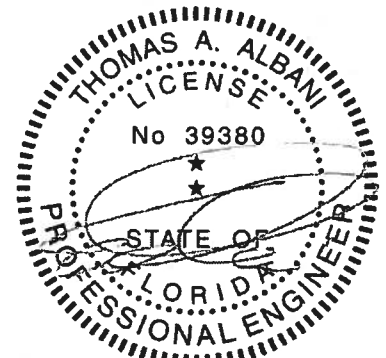
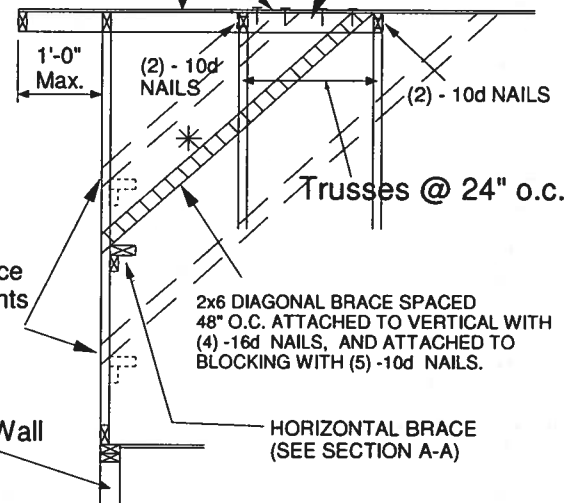
 MAX MEAN ROOF HEIGHT = 30 FEET
 EXPOSURE D
 ASCE 7-10 170 MPH
 DURATION OF LOAD INCREASE : 1.60

 STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.
 CONNECTION OF BRACING IS BASED ON MWFRS.


PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

Roof Sheathing


 Thomas A. Albani PE No.39380
 MiTek USA, Inc. FL Cert 6634
 6904 Parke East Blvd. Tampa FL 33610
 Date:

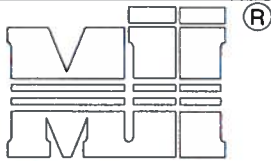
February 12, 2018

AUGUST 1, 2016

Standard Gable End Detail

MII-GE180-D-SP

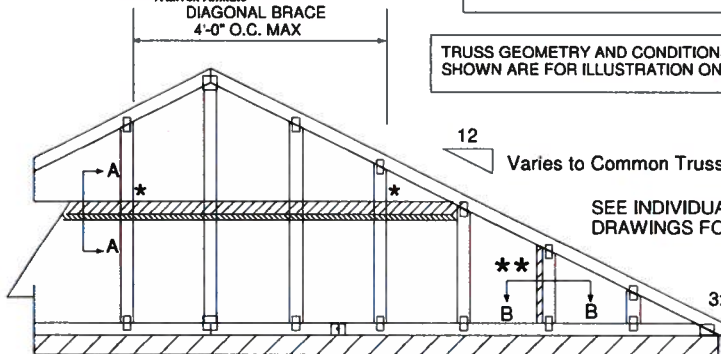
MiTek USA, Inc. Page 1 of 2



MiTek USA, Inc.

ENGINEERED BY
TRENCO

A MiTek Affiliate

DIAGONAL BRACE
4'-0" O.C. MAX* - Diagonal Bracing
Refer to Section A-A** - L-Bracing Refer
to Section B-B

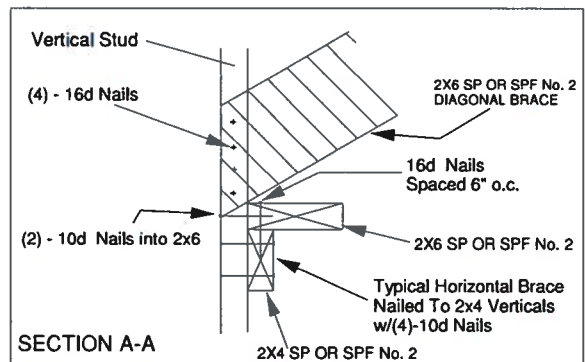
NOTE:

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3 OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Typical 2x4 L-Brace Nailed To
2x4 Verticals W/10d Nails spaced 6" o.c.

Vertical Stud

SECTION B-B

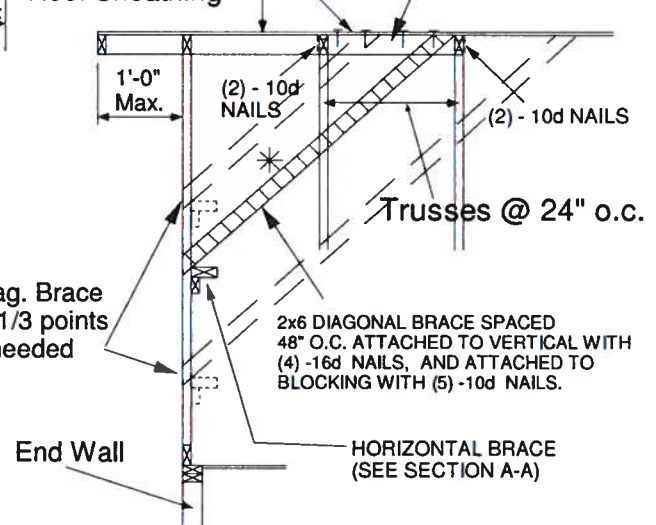
TRUSS GEOMETRY AND CONDITIONS
SHOWN ARE FOR ILLUSTRATION ONLY.

SECTION A-A

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

Roof Sheathing



Diag. Brace
at 1/3 points
if needed

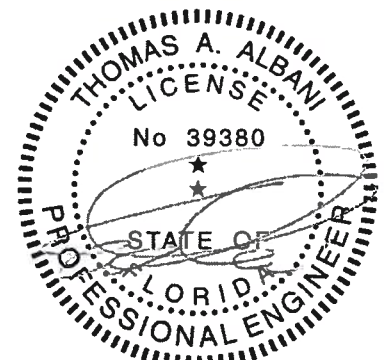
End Wall

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
		Maximum Stud Length			
2x4 SP No. 3 / Stud	12" O.C.	3-7-12	5-4-11	6-2-1	10-11-3
2x4 SP No. 3 / Stud	16" O.C.	3-2-8	4-8-1	6-2-1	9-7-7
2x4 SP No. 3 / Stud	24" O.C.	2-7-7	3-9-12	5-2-13	7-10-4
2x4 SP No. 2	12" O.C.	3-10-0	5-4-11	6-2-1	11-6-1
2x4 SP No. 2	16" O.C.	3-5-13	4-8-1	6-2-1	10-5-7
2x4 SP No. 2	24" O.C.	3-0-8	3-9-12	6-1-1	9-1-9

- * Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 l-braces attached to both edges. Fasten T and l braces to narrow edge of diagonal brace with 10d nails 6in o.c., with 3in minimum end distance. Brace must cover 90% of diagonal length. T or l braces must be 2x4 SPF No. 2 or SP No. 2.

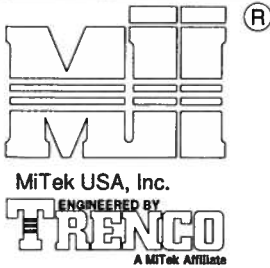
MAX MEAN ROOF HEIGHT = 30 FEET
EXPOSURE D
ASCE 7-10 180 MPH
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.
CONNECTION OF BRACING IS BASED ON MWFRS.



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Date:

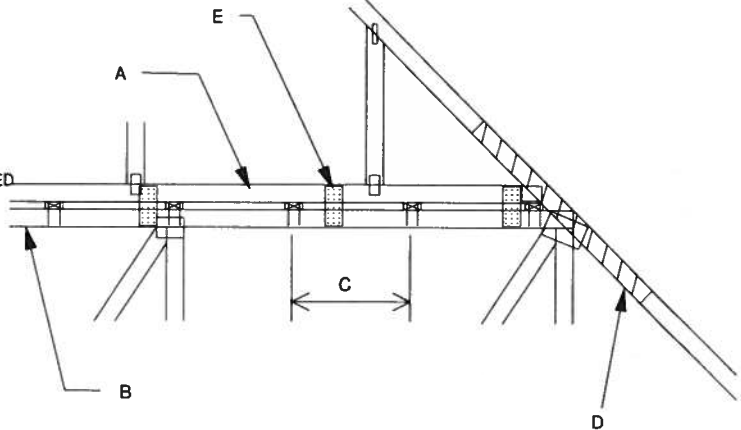
February 12, 2018



MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E
 MAX MEAN ROOF HEIGHT = 30 FEET
 MAX TRUSS SPACING = 24" O.C.
 CATEGORY II BUILDING
 EXPOSURE B or C
 ASCE 7-10
 DURATION OF LOAD INCREASE : 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES
 TRANSFERRING DRAG LOADS (SHEAR TRUSSES).
 ADDITIONAL CONSIDERATIONS BY BUILDING
 ENGINEER/DESIGNER ARE REQUIRED.

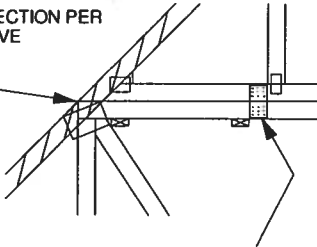
- A - PIGGYBACK TRUSS. REFER TO MITEK TRUSS DESIGN DRAWING. SHALL BE CONNECTED TO EACH PURLIN WITH (2) (0.131" X 3.5") TOE-NAILED.
- B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
- C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH.
- D - 2 X X 4'-0" SCAB, SIZE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:
1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
 2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.
- E - FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C. W/ (4) (0.131" X 1.5") NAILS PER MEMBER. STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5" EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)



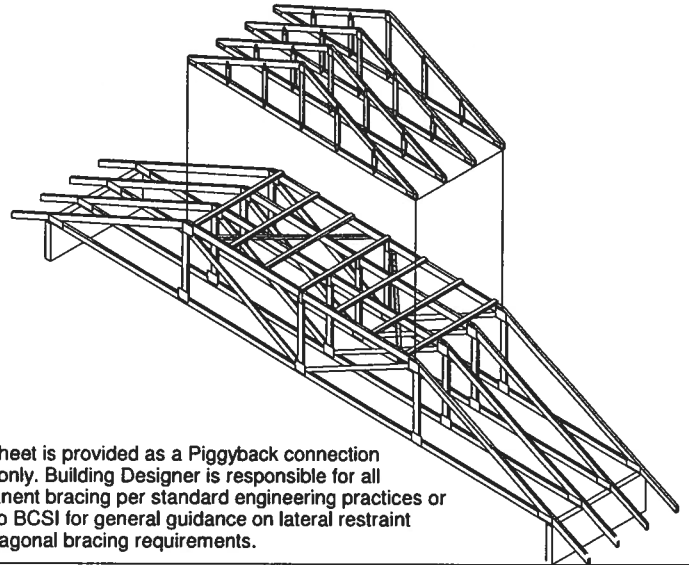
WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.

SCAB CONNECTION PER
NOTE D ABOVE

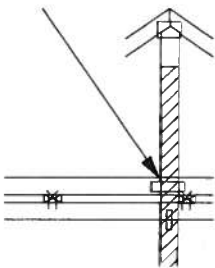


FOR ALL WIND SPEEDS, ATTACH MITEK 3X6 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) (0.131" X 1.5") PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE.



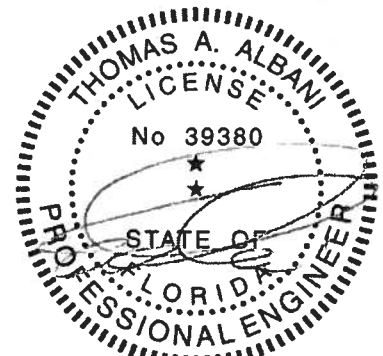
This sheet is provided as a Piggyback connection detail only. Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements.

VERTICAL WEB TO
EXTEND THROUGH
BOTTOM CHORD
OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

- 1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.
- 2) ATTACH 2 x x 4'-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
- 3) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
- 4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
- 5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN.



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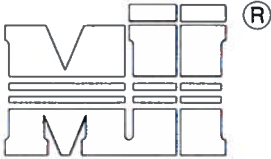
February 12, 2018

AUGUST 1, 2016

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

MII-PIGGY-ALT
7-10

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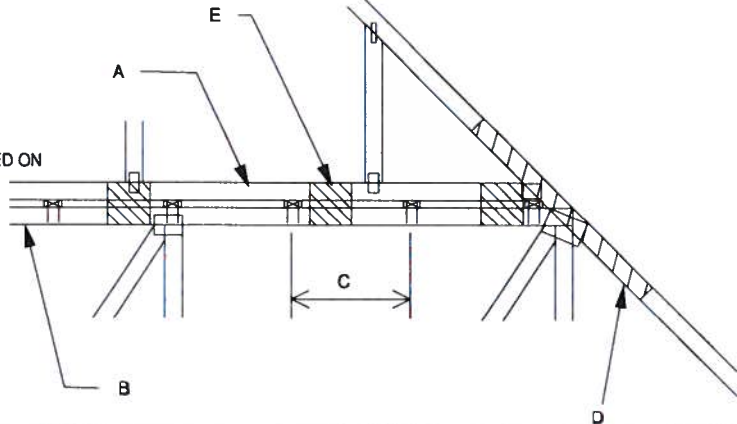


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MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E
MAX MEAN ROOF HEIGHT = 30 FEET
MAX TRUSS SPACING = 24" O.C.
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-10
DURATION OF LOAD INCREASE : 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES
TRANSFERING DRAG LOADS (SHEAR TRUSSES).
ADDITIONAL CONSIDERATIONS BY BUILDING
ENGINEER/DESIGNER ARE REQUIRED.

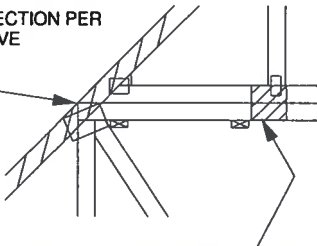
- A - PIGGYBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING. SHALL BE CONNECTED TO EACH PURLIN WITH (2) 0(0.131" X 3.5") TOE-NAILED.
- B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
- C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH.
- D - 2 X 4-0" SCAB, SIZE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:
1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
 2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.
- E - FOR WIND SPEED IN THE RANGE 126 MPH - 160 MPH ADD 9" x 9" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 48" O.C. OR LESS. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)



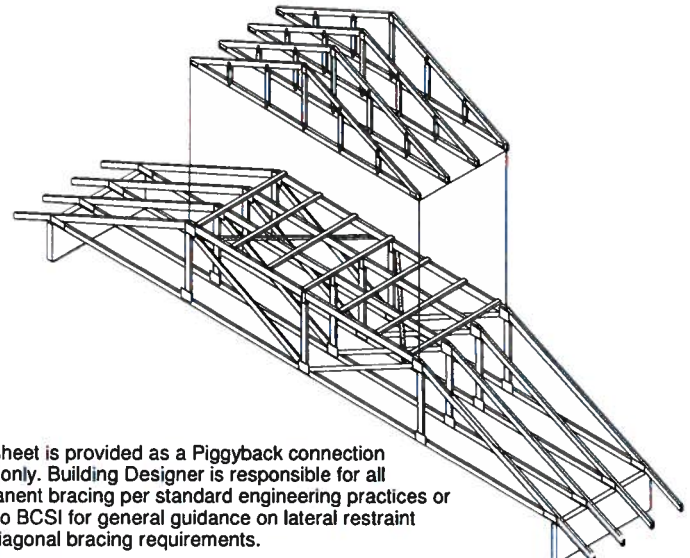
WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH PLYWOOD GUSSETS AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.

SCAB CONNECTION PER
NOTE D ABOVE

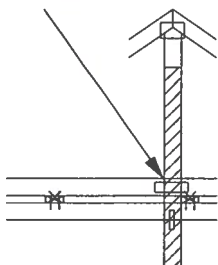


7" x 7" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 24" O.C. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)



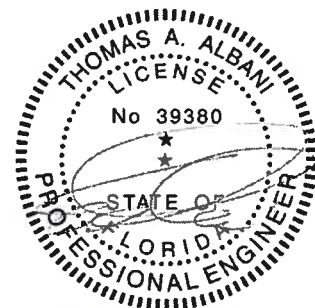
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VERTICAL WEB TO
EXTEND THROUGH
BOTTOM CHORD
OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

- 1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.
- 2) ATTACH 2 x 4-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
- 3) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
- 4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
- 5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN.



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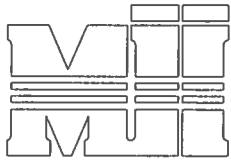
January 19, 2018

AUGUST 1, 2016

STANDARD REPAIR DETAIL FOR BROKEN CHORDS, WEBS
AND DAMAGED OR MISSING CHORD SPLICE PLATES

MII-REP01A1

MiTek USA, Inc. Page 1 of 1



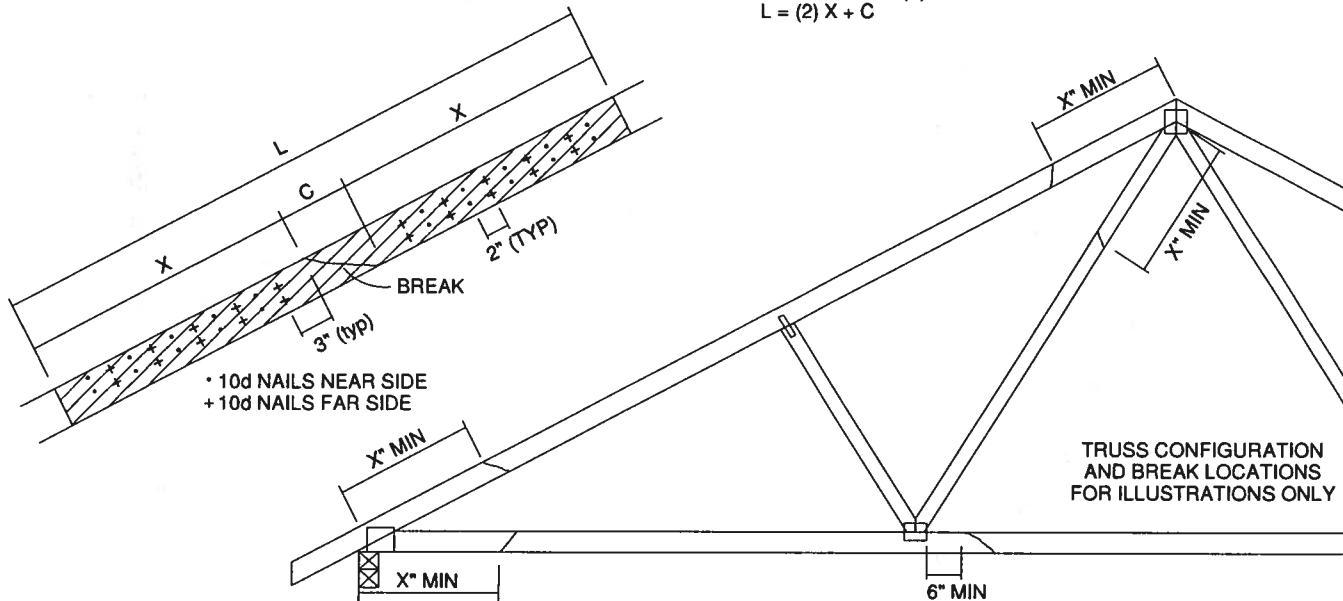
MiTek USA, Inc.
ENGINEERED BY
TRENCO
A MiTek Affiliate

TOTAL NUMBER OF NAILS EACH SIDE OF BREAK *		X INCHES	MAXIMUM FORCE (lbs) 15% LOAD DURATION							
			SP		DF		SPF		HF	
2x4	2x6		2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6
20	30	24"	1706	2559	1561	2342	1320	1980	1352	2028
26	39	30"	2194	3291	2007	3011	1697	2546	1738	2608
32	48	36"	2681	4022	2454	3681	2074	3111	2125	3187
38	57	42"	3169	4754	2900	4350	2451	3677	2511	3767
44	66	48"	3657	5485	3346	5019	2829	4243	2898	4347

* DIVIDE EQUALLY FRONT AND BACK

ATTACH 2x SCAB OF THE SAME SIZE AND GRADE AS THE BROKEN MEMBER TO EACH
FACE OF THE TRUSS (CENTER ON BREAK OR SPLICE) WITH 10d (0.131" X 3") NAILS
(TWO ROWS FOR 2x4, THREE ROWS FOR 2x6) SPACED 4" O.C. AS SHOWN.
STAGGER NAIL SPACING FROM FRONT FACE AND BACK FACE FOR A NET 0-2-0 O.C.
SPACING IN THE MAIN MEMBER. USE A MIN. 0-3-0 MEMBER END DISTANCE.

THE LENGTH OF THE BREAK (C) SHALL NOT EXCEED 12". (C=PLATE LENGTH FOR SPLICE REPAIRS)
THE MINIMUM OVERALL SCAB LENGTH REQUIRED (L) IS CALCULATED AS FOLLOWS:
 $L = (2) X + C$

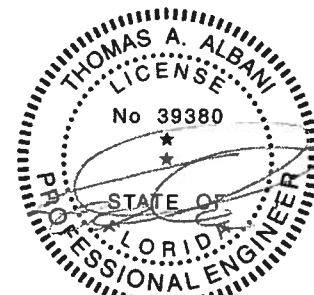


THE LOCATION OF THE BREAK MUST BE GREATER THAN OR EQUAL TO THE REQUIRED X DIMENSION FROM ANY
PERIMETER BREAK OR HEEL JOINT AND A MINIMUM OF 6" FROM ANY INTERIOR JOINT (SEE SKETCH ABOVE)

DO NOT USE REPAIR FOR JOINT SPLICES

NOTES:

1. THIS REPAIR DETAIL IS TO BE USED ONLY FOR THE APPLICATION SHOWN. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.
2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLYING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.
3. THE END DISTANCE, EDGE DISTANCE AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
4. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.
5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 2x ORIENTATION ONLY.
6. THIS REPAIR IS LIMITED TO TRUSSES WITH NO MORE THAN THREE BROKEN MEMBERS.



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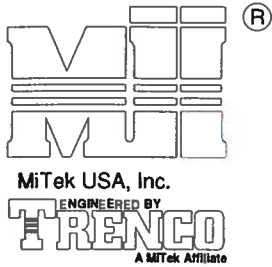
January 19, 2018

AUGUST 1, 2016

LATERAL TOE-NAIL DETAIL

MII-TOENAIL_SP

MiTek USA, Inc. Page 1 of 1



NOTES:

1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.)
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

THIS DETAIL APPLICABLE TO THE
THREE END DETAILS SHOWN BELOW

VIEWS SHOWN ARE FOR
ILLUSTRATION PURPOSES ONLY

TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)

	DIAM.	SP	DF	HF	SPF	SPF-S
3.5" LONG	.131	88.0	80.6	69.9	68.4	59.7
	.135	93.5	85.6	74.2	72.6	63.4
	.162	108.8	99.6	86.4	84.5	73.8
3.25" LONG	.128	74.2	67.9	58.9	57.6	50.3
	.131	75.9	69.5	60.3	59.0	51.1
	.148	81.4	74.5	64.6	63.2	52.5

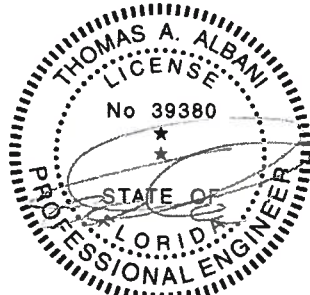
VALUES SHOWN ARE CAPACITY PER TOE-NAIL.
APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

EXAMPLE:

(3) - 16d (0.162" X 3.5") NAILS WITH SPF SPECIES BOTTOM CHORD

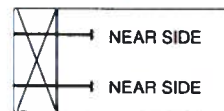
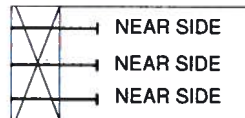
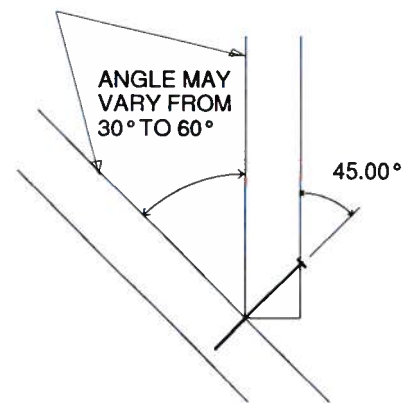
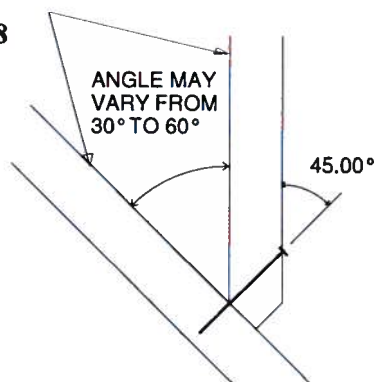
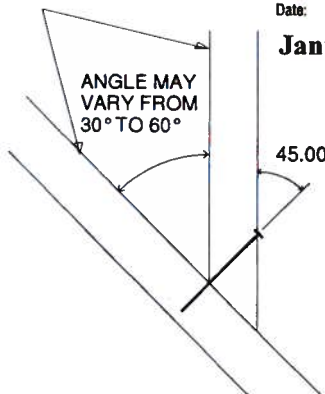
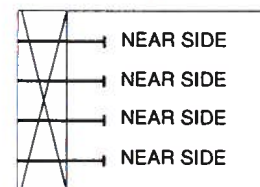
For load duration increase of 1.15:

3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity



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January 19, 2018

SIDE VIEW
(2x3)
2 NAILSSIDE VIEW
(2x4)
3 NAILSSIDE VIEW
(2x6)
4 NAILS

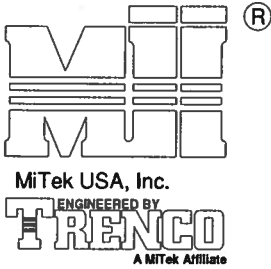
AUGUST 1, 2016

TRUSSED VALLEY SET DETAIL

MII-VALLEY HIGH WIND1

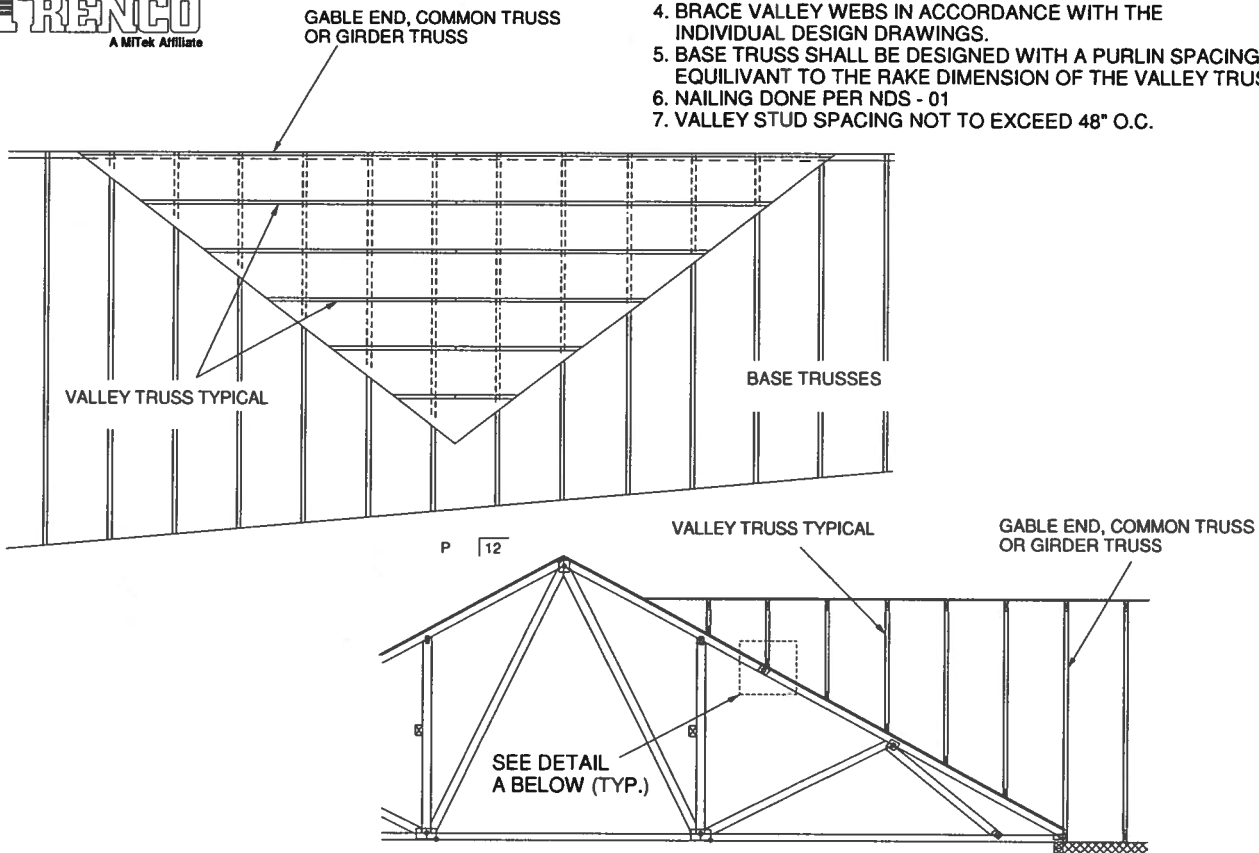
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Page 1 of 1

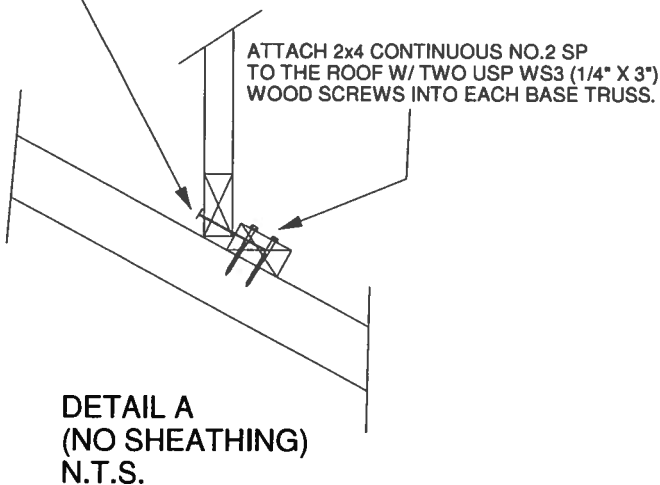


GENERAL SPECIFICATIONS

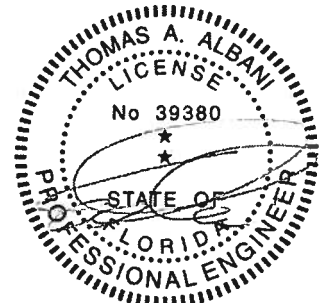
1. NAIL SIZE 10d (0.131" X 3")
2. WOOD SCREW = 3" WS3 USP OR EQUIVALENT
DO NOT USE DRYWALL OR DECKING TYPE SCREW
3. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
4. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
5. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUIVALENT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
6. NAILING DONE PER NDS - 01
7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.



SECURE VALLEY TRUSS
W/ ONE ROW OF 10d
NAILS 6" O.C.

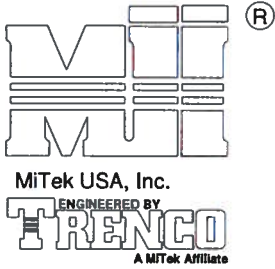


WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH
WIND DESIGN PER ASCE 7-10 160 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12
CATEGORY II BUILDING
EXPOSURE C
WIND DURATION OF LOAD INCREASE : 1.60
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 6 PSF
ON THE TRUSSES



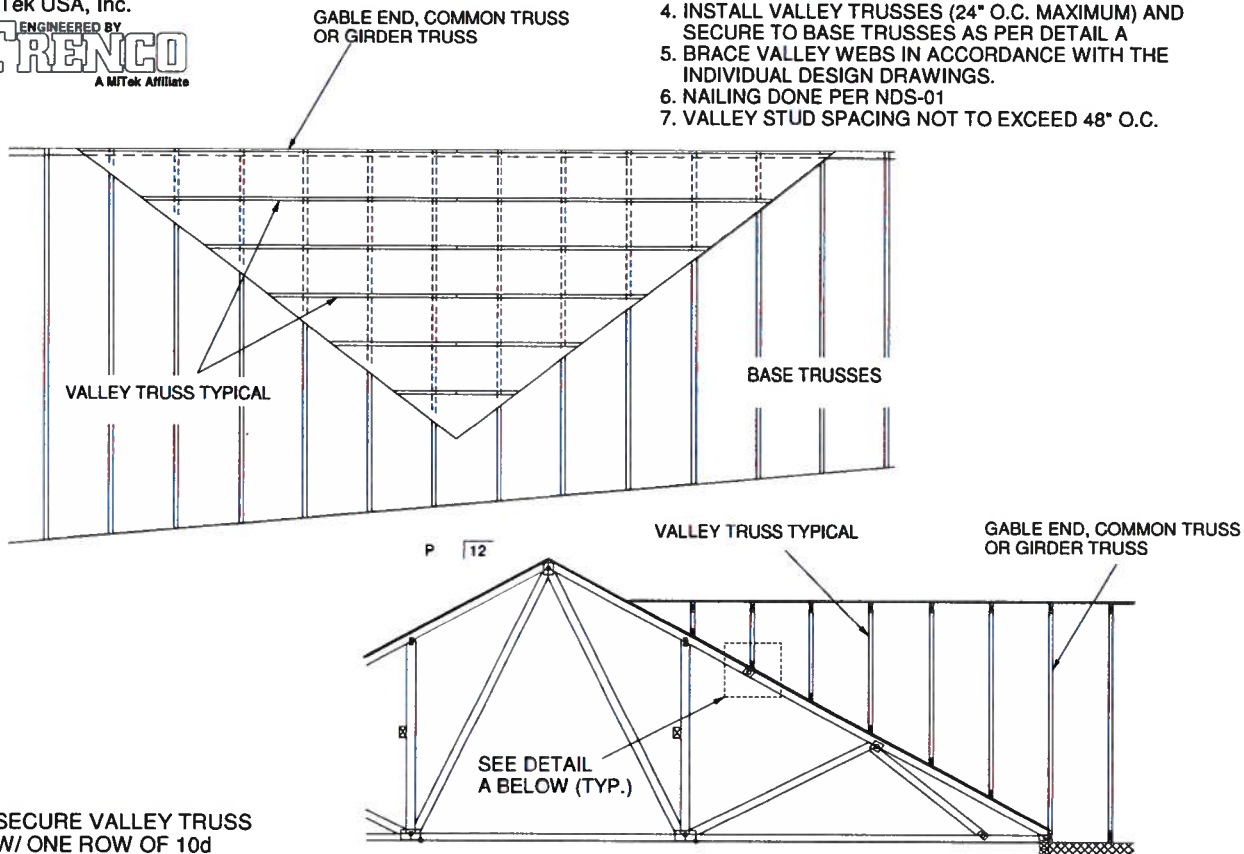
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January 19, 2018

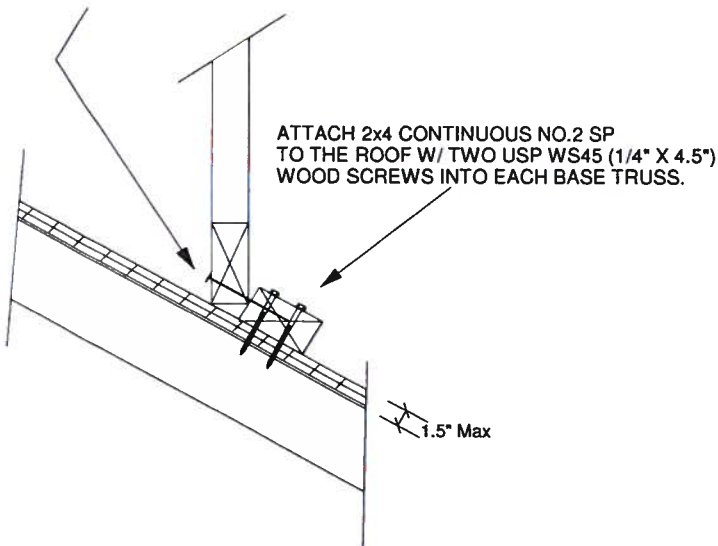


GENERAL SPECIFICATIONS

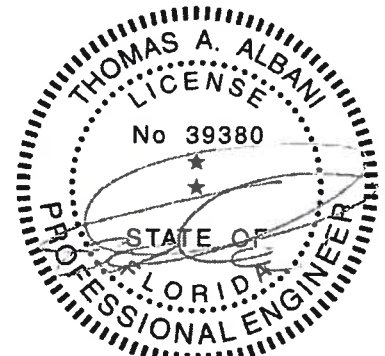
1. NAIL SIZE 10d (0.131" X 3")
2. WOOD SCREW = 4.5" WS45 USP OR EQUIVANT
3. INSTALL SHEATHING TO TOP CHORD OF BASE TRUSSES.
4. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE TO BASE TRUSSES AS PER DETAIL A
5. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
6. NAILING DONE PER NDS-01
7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.



SECURE VALLEY TRUSS
W/ ONE ROW OF 10d
NAILS 6" O.C.

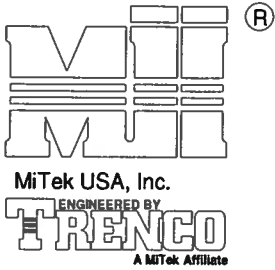


WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH
WIND DESIGN PER ASCE 7-10 160 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12
CATEGORY II BUILDING
EXPOSURE C
WIND DURATION OF LOAD INCREASE : 1.60
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 6 PSF
ON THE TRUSSES



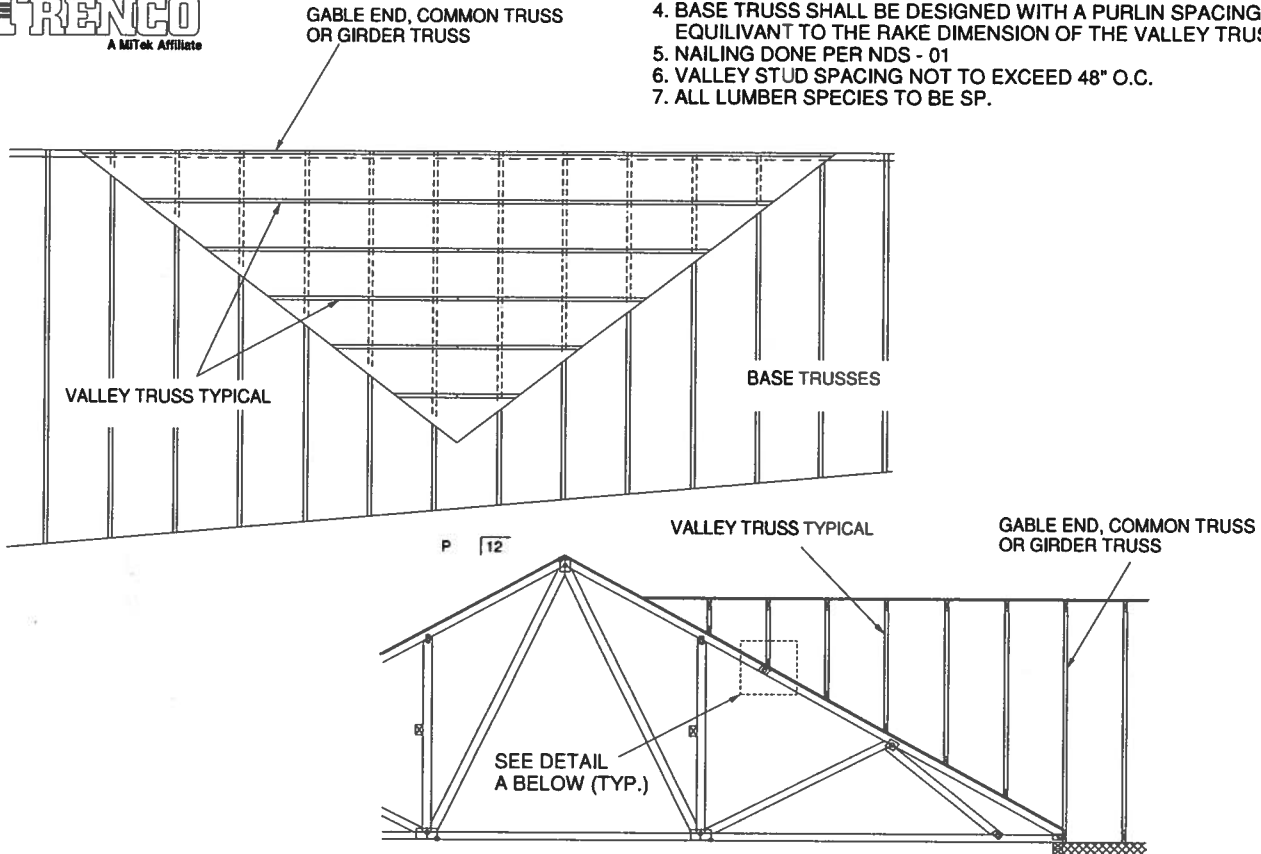
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Date:

February 12, 2018

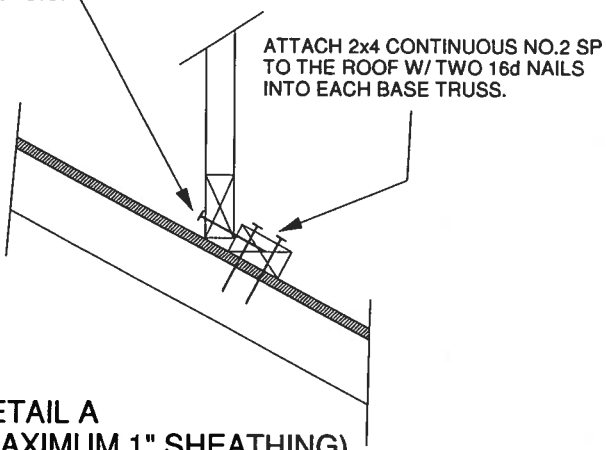


GENERAL SPECIFICATIONS

1. NAIL SIZE 16d (0.131" X 3.5")
2. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
3. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
4. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUIVARIANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
5. NAILING DONE PER NDS - 01
6. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.
7. ALL LUMBER SPECIES TO BE SP.

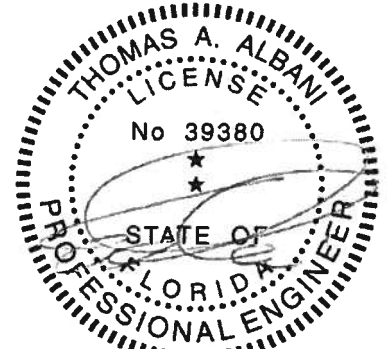


SECURE VALLEY TRUSS
W/ ONE ROW OF 16d
NAILS 6" O.C.



DETAIL A
(MAXIMUM 1" SHEATHING)
N.T.S.

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 120 MPH
WIND DESIGN PER ASCE 7-10 150 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
ROOF PITCH = MINIMUM 3/12 MAXIMUM 10/12
CATEGORY II BUILDING
EXPOSURE C OR B
WIND DURATION OF LOAD INCREASE : 1.60
MAX TOP CHORD TOTAL LOAD = 60 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 4.2 PSF
ON THE TRUSSES



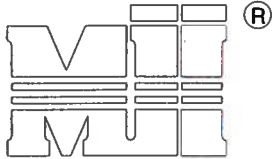
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Date:

February 12, 2018

AUGUST 1, 2016

TRUSSED VALLEY SET DETAIL
(HIGH WIND VELOCITY)

MII-VALLEY

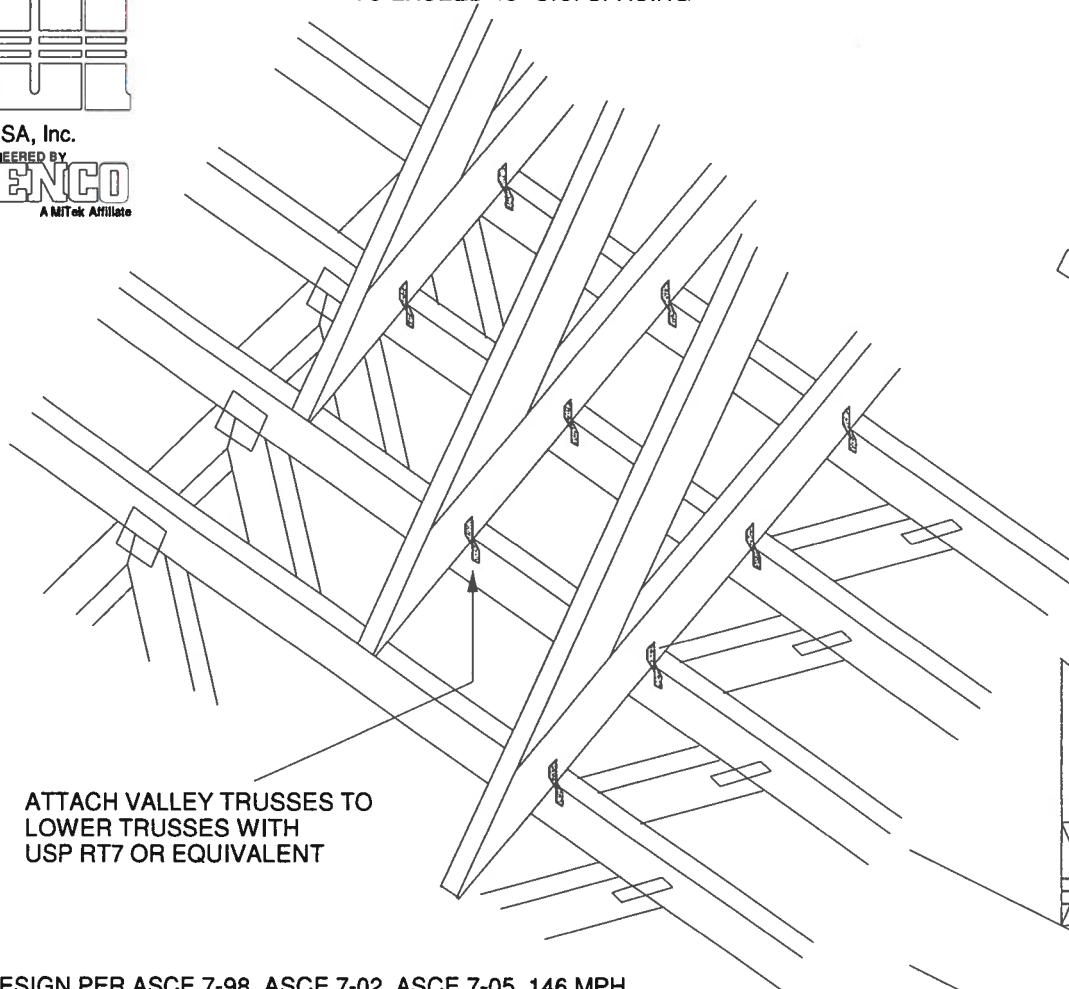


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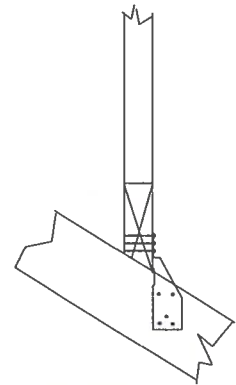
ENGINEERED BY
TRENCO
A MiTek Affiliate

NOTE: VALLEY STUD SPACING NOT
TO EXCEED 48" O.C. SPACING

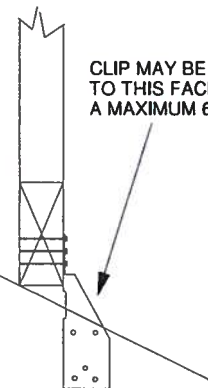
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ATTACH VALLEY TRUSSES TO
LOWER TRUSSES WITH
USP RT7 OR EQUIVALENT



FOR BEVELED BOTTOM
CHORD, CLIP MAY BE
APPLIED TO EITHER FACE



CLIP MAY BE APPLIED
TO THIS FACE UP TO
A MAXIMUM 6/12 PITCH

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH
WIND DESIGN PER ASCE 7-10 160 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE B or C
WIND DURATION OF LOAD INCREASE : 1.6
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)

SUPPORTING TRUSSES DIRECTLY UNDER
VALLEY TRUSSES MUST BE DESIGNED
WITH A MAXIMUM UNBRACED LENGTH OF
2'-10" ON AFFECTED TOP CHORDS.

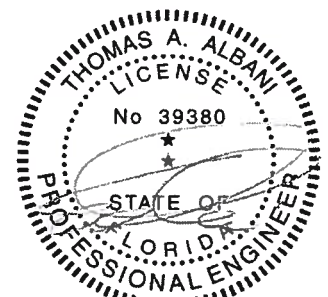
NOTES:

- SHEATHING APPLIED AFTER
INSTALLATION OF VALLEY TRUSSES
- THIS DETAIL IS NOT APPLICABLE FOR
SPF-S SPECIES LUMBER.

NON-BEVELED
BOTTOM CHORD

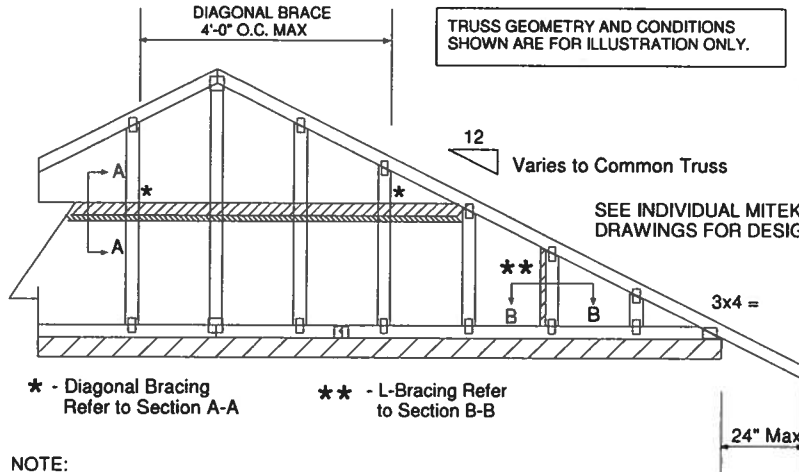
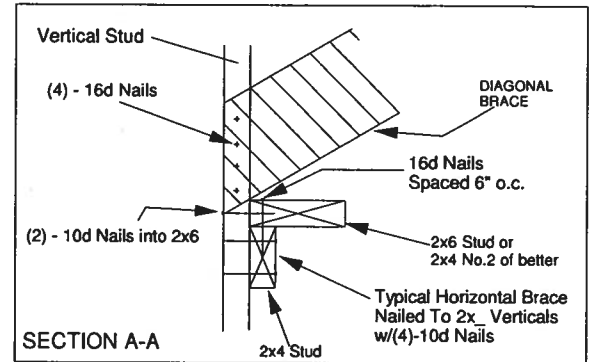
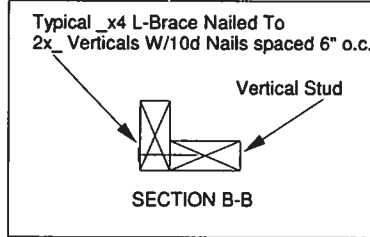
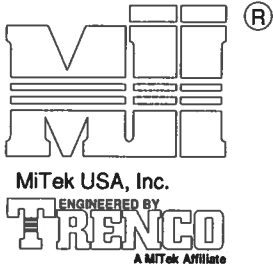
NON-BEVELED
BOTTOM CHORD

CLIP MUST BE APPLIED
TO THIS FACE WHEN
PITCH EXCEEDS 6/12.
(MAXIMUM 12/12 PITCH)



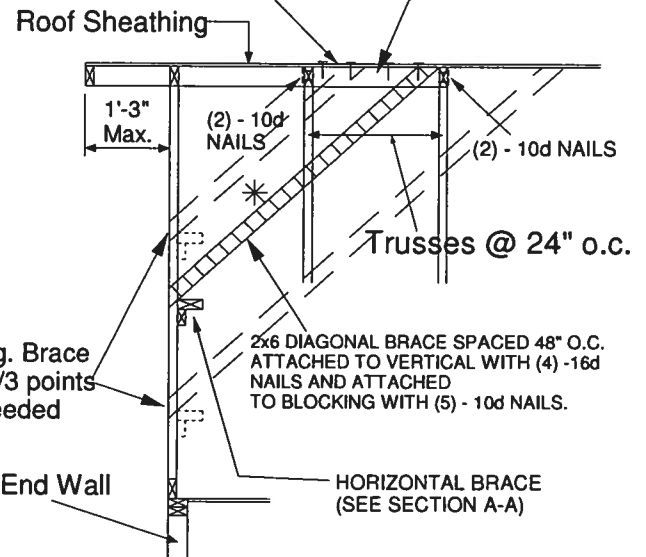
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PROVIDE 2×4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2×4 STD SP BLOCK



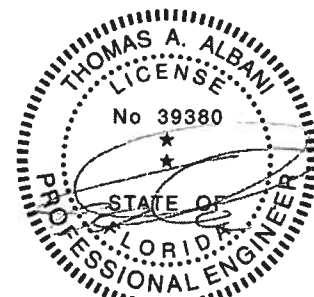
- NOTE:
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
 3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
 4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 2×4 No 3/STUD SP OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2×6 STUD AND A 2×4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2×4 . (REFER TO SECTION A-A)
 7. GABLE STUD DEFLECTION MEETS OR EXCEEDS $L/240$.
 8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
 9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
 10. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
		Maximum Stud Length			
2x4 SP No 3/Stud	12" O.C.	3-11-3	6-8-0	7-2-14	11-9-10
2x4 SP No 3/Stud	16" O.C.	3-6-14	5-9-5	7-1-13	10-8-11
2x4 SP No 3/Stud	24" O.C.	3-1-8	4-8-9	6-2-15	9-4-7

- * Diagonal braces over 6'-3" require a 2×4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2×4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

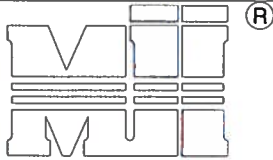
MAXIMUM WIND SPEED = 146 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-98, ASCE 7-02, ASCE 7-05
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.
CONNECTION OF BRACING IS BASED ON MWFRS.



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Page 1 of 1

TRUSS CRITERIA:

LOADING: 40-10-0-10

DURATION FACTOR: 1.15

SPACING: 24" O.C.

TOP CHORD: 2x4 OR 2x6

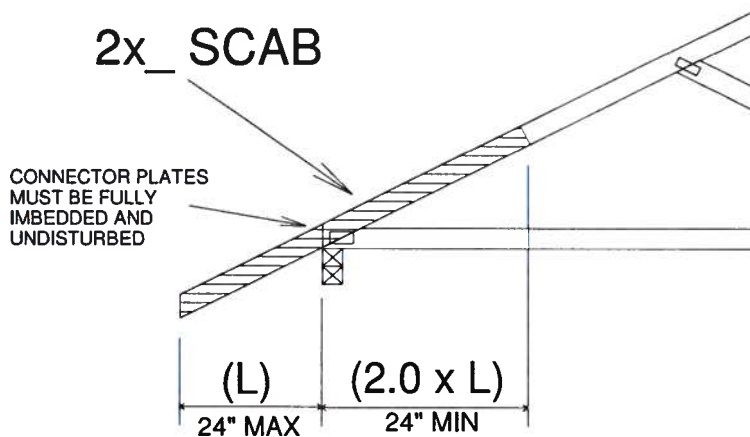
PITCH: 4/12 - 12/12

HEEL HEIGHT: STANDARD HEEL UP TO 12" ENERGY HEEL

END BEARING CONDITION

NOTES:

1. ATTACH 2x SCAB (MINIMUM NO.2 GRADE SPF, HF, SP, DF) TO ONE FACE OF TRUSS WITH TWO ROWS OF 10d (0.131" X 3") SPACED 6" O.C.
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.

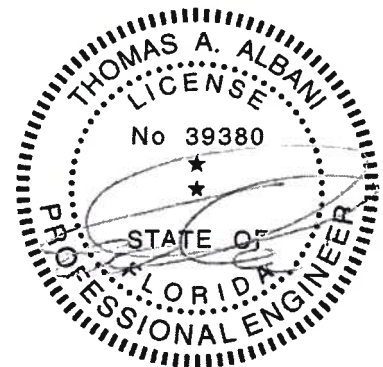


IMPORTANT

This detail to be used only with trusses (spans less than 40') spaced 24" o.c. maximum and having pitches between 4/12 and 12/12 and total top chord loads not exceeding 50 psf.

Trusses not fitting these criteria should be examined individually.

REFER TO INDIVIDUAL TRUSS DESIGN
FOR PLATE SIZES AND LUMBER GRADES



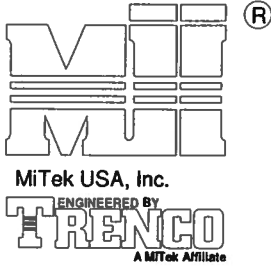
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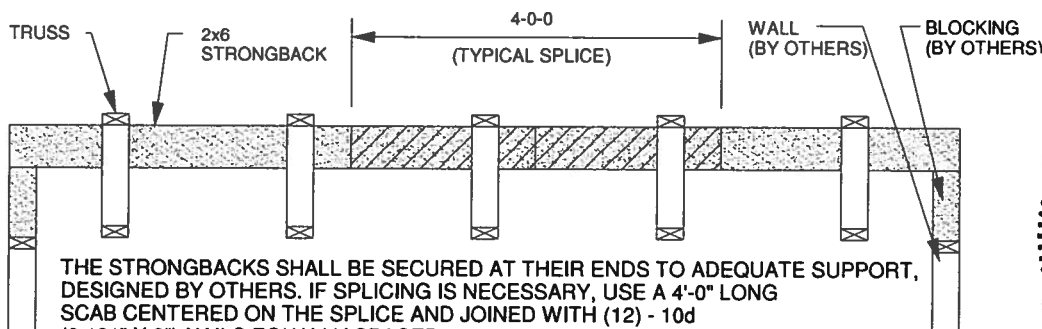
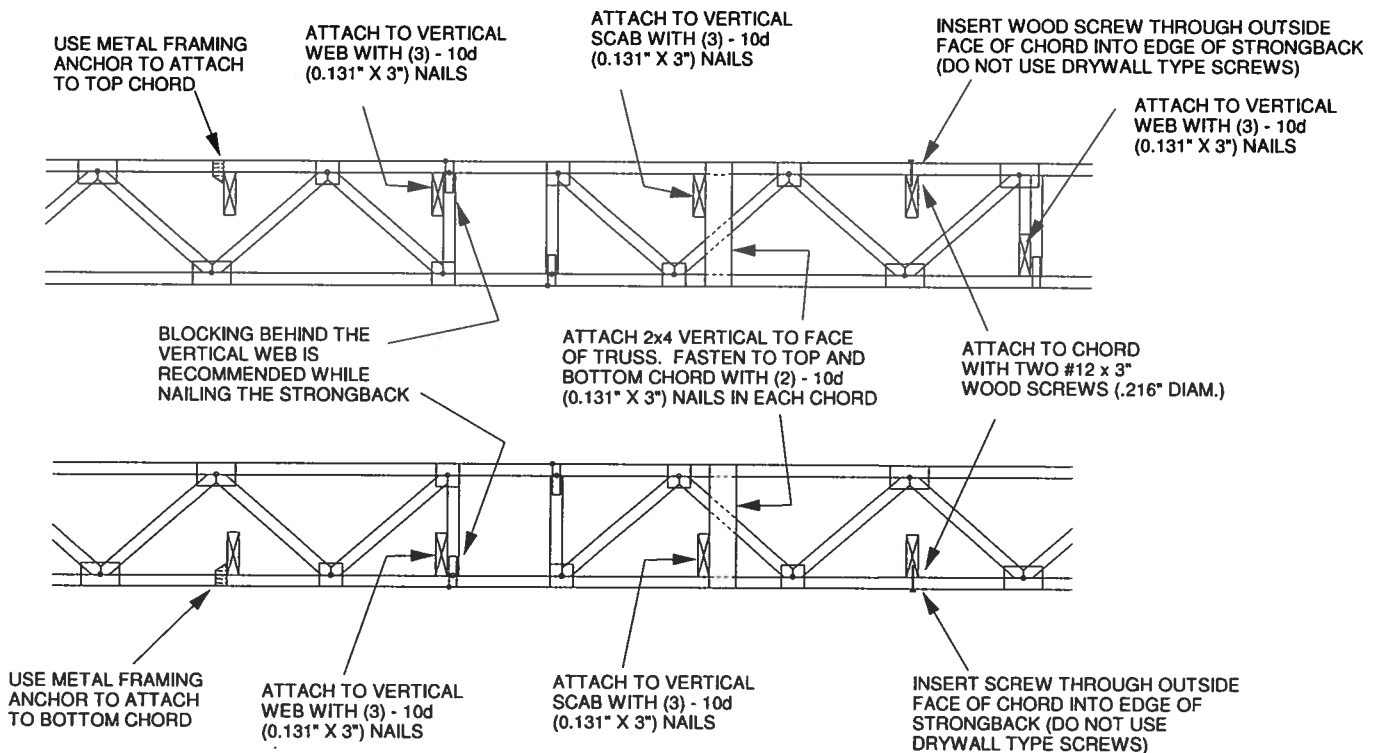
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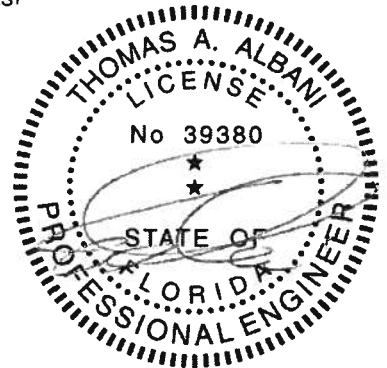
TO MINIMIZE VIBRATION COMMON TO ALL SHALLOW FRAMING SYSTEMS, 2x6 "STRONGBACK" IS RECOMMENDED, LOCATED EVERY 8 TO 10 FEET ALONG A FLOOR TRUSS.

NOTE 1: 2X6 STRONGBACK ORIENTED VERTICALLY MAY BE POSITIONED DIRECTLY UNDER THE TOP CHORD OR DIRECTLY ABOVE THE BOTTOM CHORD. SECURELY FASTENED TO THE TRUSS USING ANY OF THE METHODS ILLUSTRATED BELOW.

NOTE 2: STRONGBACK BRACING ALSO SATISFIES THE LATERAL BRACING REQUIREMENTS FOR THE BOTTOM CHORD OF THE TRUSS WHEN IT IS PLACED ON TOP OF THE BOTTOM CHORD, IS CONTINUOUS FROM END TO END, CONNECTED WITH A METHOD OTHER THAN METAL FRAMING ANCHOR, AND PROPERLY CONNECTED, BY OTHERS, AT THE ENDS.



ALTERNATE METHOD OF SPLICING:
OVERLAP STRONGBACK MEMBERS A MINIMUM OF 4'-0" AND FASTEN WITH (12) - 10d (0.131" X 3") NAILS STAGGERED AND EQUALLY SPACED.
(TO BE USED ONLY WHEN STRONGBACK IS NOT ALIGNED WITH A VERTICAL)



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