

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

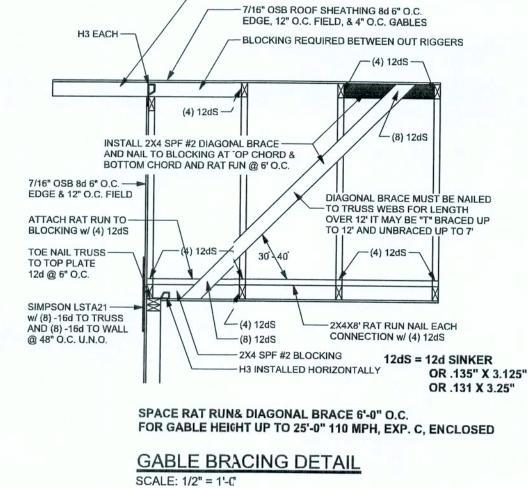
ONE STORY WALL SECTION

|) 11'-9" STUD HEIGHT |
|-----------------------|
| |
|) 13'-0" STUD HEIGHT |
|) 18'-10' STUD HEIGHT |
| 20.0' STUD HEIGHT |
| |

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS ESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

SIMPSON H2.5A U.N.O.-

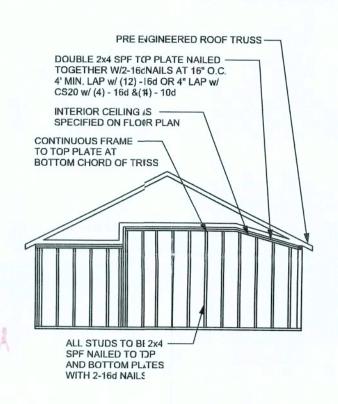
SEE STRUCTURAL PLAN



-2X4 OUTRIGGER @ 24" O.C.

GRADE & SPECIES TABLE

| | _ | Fb (psi) | E (10 ⁶ psi) | |
|------|--------------|----------|-------------------------|--|
| 2x8 | SYP #2 | 1200 | 1.6 | |
| 2x10 | SYP #2 | 1050 | 1.6 | |
| 2x12 | SYP #2 | 975 | 1.6 | |
| GLB | 24FV3 SP | 2400 | 1.8 | |
| LSL | TIMBERSTRAND | ID 1700 | 1.7 | |
| LVL | MICROLAM | 1600 | 1.9 | |
| PSL | PAFALAM | 2900 | 2.0 | |



CONTINUCUS FRAME TO CEILING DIAPHRAGM DETAIL SCALE: N.T.S.

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BULDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUND/TION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1/00 PSF BEARING CAPACITY UNLESS

VISUAL OBSERVATION OR SOILS TEST PROYES OTHERWISE

WELDED WIRE REINFORCED SLAB: 6" × 6'W1.4 × W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER LENGTH 1/2 INCH TO 2 INCHES. DOS/GE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD. PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

ACCORDANCE WITH ACI 302. JOINTS SHALL 3E CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TOCRACK ON A GIVEN LINE.)

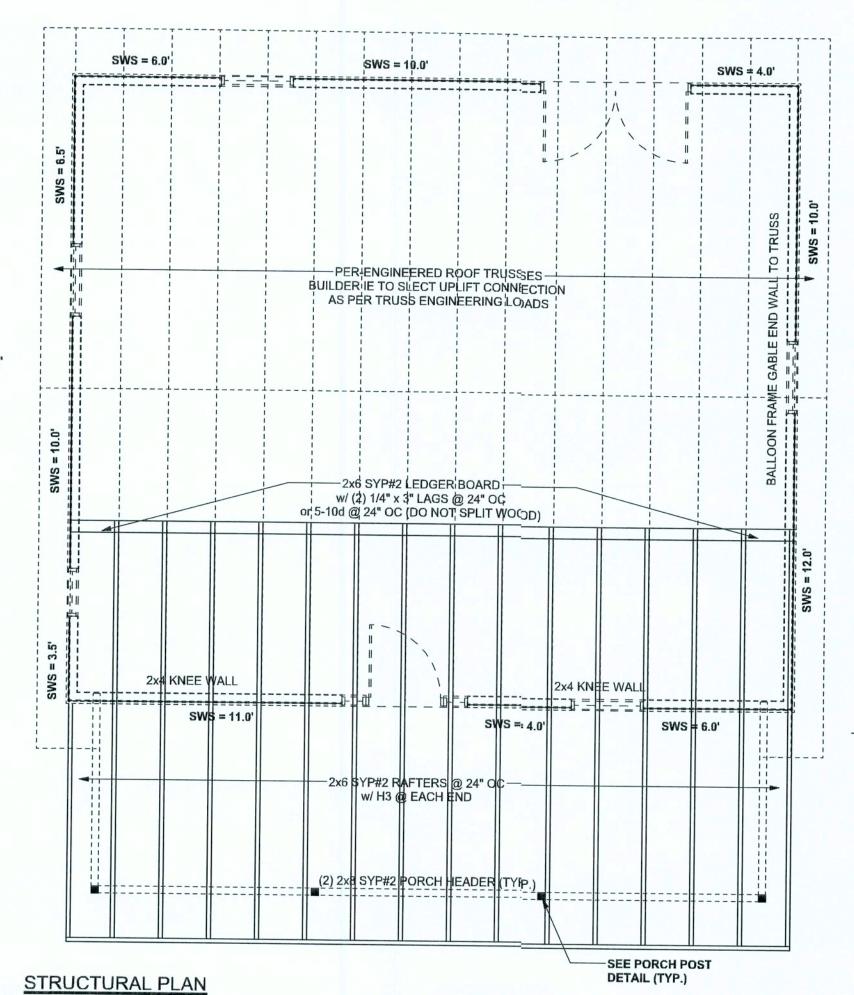
REBAR: ASTM A 615, GRADE 60, DEFORMEDBARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); JNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F√3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH ENUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZ(NTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON IAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SJBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHEVE RATED LOADS.

LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.



HEADER LEGEND

STRUCTURAL PLAN NOTES

ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X8 SYP #2 (U.N.O.)

ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

> PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03. BCSI-B1, BCSI-B2, & BCSI-B3, BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

TOTAL SHEAR WALL SEGMENTS

—SIZE OF HEADER MATERIAL

-NUMBER OF PLIES IN HEADER

-NUMBER OF KING STUDS (FULL LENGTH)

—NUMBER OF JACK STUDS (UNDER HEADER)

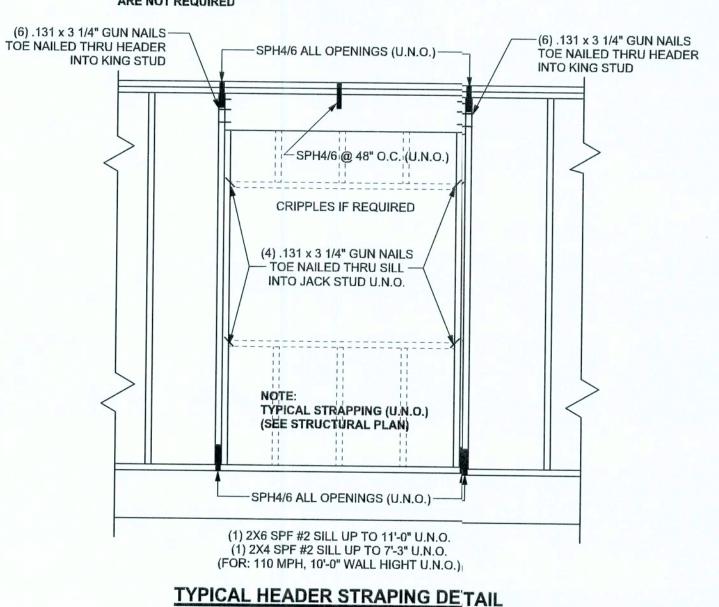
(2) 2X12X0',1J 1K HEADER/BEAM CALL-OUT (U.N.O.)

-----SPAN OF HEADER

SWS; = 0.0' INDICATES SHEAR WALL SEGMENTS REQUIRED ACTUAL

TRANSVERSE 12.5' LONGITUDINAL 10.1' 41.0'

IF TRUSS TO WALL STRAPS ARE NAILED TO THE HEADER THE SPH4/6 @ 48" O.C. ARE NOT REQUIRED



-6" AFF **FOUNDATION PLAN** DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

TO PLATES TO RAFTER/TRUSS

4" CONCRETE FLOOR SLAB REINFORCED WITH

6X6-1.4/1.4 WELDED WIRE MESH PLACED ON CHAIRS

POLY VAPOR BARRIER WITH 6" LAPS SEALED WITH

POLY TAPE OVER TERMITE-TREATED AND COMPACTED FILL

AT 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL

ANCHOR TABLE OBTAIN UPLIFT REQUIREMENTS FROM TRUSS

UPLIFT LBS. SYP UPLIFT LBS. SPF

< 245

TRUSS CONNECTOR*

H5A

MANUFACTURER'S ENGINEERING

< 420

< 2300

< 2320

< 2300

< 2320

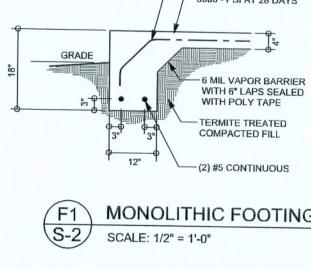
< 455 < 265 4-8d 4-8d < 360 < 235 4-8d 4-8d < 455 < 320 4-8d 4-8d < 415 < 365 H2.5 5-8d 5-8d < 535 H2.5A 5-8d 5-8d < 950 < 820 8-8d 8-8d < 745 < 565 5-10d, 1 1/2" 5-10d, 1 1/2" < 1465 < 1050 H14-1 13-8d 12-8d, 1 1/2 < 1465 < 1050 H14-2 15-8d 12-8d, 1 1/2' < 990 < 850 H10-1 8-8d, 1 1/2" 8-8d, 1 1/2" < 760 < 655 H10-2 6-10d 6-10d < 1470 < 1265 H16-1 10-10d, 1 1 2-10d, 1 1/2' < 1470 < 1265 H16-2 10-10d, 1 1/2 2-10d, 1 1/2' < 1000 < 860 MTS24C 7-10d 1 1/2 7-10d 1 1/2" < 1245 HTS24 12-10d 1 1/2" 12-10d 1 1/2° < 2900 < 2490 2 - HTS24 < 2050 < 1785 LGT2 14 -16d 14 -16d **EAVY GIRDER TIEDOWNS** TO FOUNDATION < 3965 -5/8" THREADED ROI MGT 22 -10d 12" EMBEDMENT -5/8" THREADED ROD HGT-2 16 -10d 12" EMBEDMENT < 10530 < 9035 HGT-3 2-5/8" THREADED ROD 16 -10d 12" EMBEDMENT < 9250 < 9250 2-5/8" THREADED ROD HGT-4 16 -10d 12" EMBEDMENT STUD STRAP CONNECTOR* TO STUDS < 435 < 435 SSP DOUBLE TOP PLATE 4 -10d < 455 < 420 SSP SINGLE SILL PLATE 4 -10d < 825 < 825 DSP DOUBLE TOP PLATE 6 -10d 8 -10d < 825 < 600 DSP SINGLE SILL PLATE 2 -10d 8 -10d < 885 < 760 6-10d, 1 1/2" < 1240 < 1065 SPH4 10-10d, 1 1/2" < 885 SP6 6-10d, 1 1/2" < 1240 < 1065 SPH6 10-10d, 1 1/2" < 1235 < 1165 LSTA18 14-10d < 1235 < 1235 LSTA21 16-10d < 1030 < 1030 CS20 18-8d < 1705 < 1705 CS16 STUD ANCHORS* TO STUDS TO FOUNDATION < 1350 < 1305 8-16d 1/2" AB < 2310 < 2310 LTTI31 18-10d, 1 1/2 1/2" AB < 2775 < 2570 HD2A 2-5/8" BOLTS 5/8" AB < 4175 < 3695 HTT16 18 - 16d 5/8" AB < 1400 < 1400 PAHD42 16-16d < 3335 < 3335 HPAHD22 16-16d < 2200 < 2200 ABU44 12-16d 1/2" AB

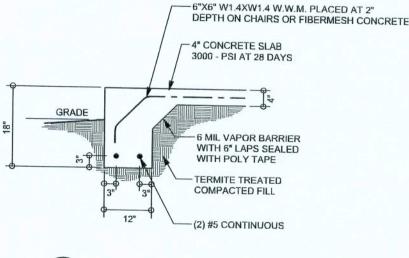
12-16d

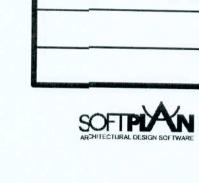
18 - 16d

1/2" AB

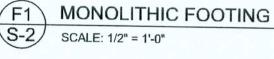
2-5/8" AB

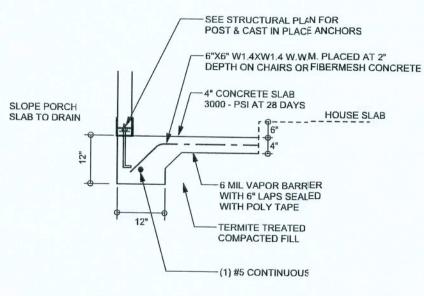






REVISIONS





PORCH FOOTING

S-2 SCALE: 1/2" = 1'-0"

BUILDER'S RESPONSIBILITY THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS,

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1 (ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.) BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION 1.) BASIC WIND SPEED = 110 MPH 2.) WIND EXPOSURE = B 3.) WIND IMPORTANCE FACTOR = 1.0 4.) BUILDING CATEGORY = II ROOF ANGLE = 10-45 DEGREES 6.) MEAN ROOF HEIGHT = <30 FT</p> .) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING) 8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2)) Zone Effective Wind Area (ft2) 19.9 -21.8 18.1 -18.1 2 19.9 -25.5 18.1 -21.8 -40.6 3 O'hg -25.5 18.1 -21.8 -68.3 -42.4 4 21.8 -23.6 18.5 -20.4 5 21.8 -29.1 18.5 -22.6

| | | Doors & Windows Worst Case (Zone 5, 10 ft2) | 21.8 | -29.1 |
|--------|--|---|------|-------|
| 5 | 2 | 8x7 Garage Door | 19.5 | -22.9 |
| 2 | 4 3 4 5 | 16x7 Garage Door | 18.5 | -21.0 |
| | 55 | | | |
| | | | | |
| DESIGN | LOADS | | | |
| FLOOR | 40 PSF (ALL OTHER DWELLING ROOMS) | | | |
| | 30 PSF (SLEEPING ROOMS) | | | |
| | 30 PSF (ATTICS WITH STORAGE) | | | |
| | 10 PSF (ATTICS WITHOUT STORAGE, <3:12) | | | |
| ROOF | 20 PSF (FLAT OR <4:12) | | | |
| | 16 PSF (4:12 TO <12:12) | | | |
| | 12 PSF (12:12 AND GREATER) | | | |
| STAIRS | 40 PSF (ONE & TWO FAMILY DWELLINGS) | | | |
| | ARING CAPACITY 1000PSF | | | |

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

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WINDLOAD ENGINEER: Mark Disosway,

PE No.53915, POB 868, Lake City, FL

tated dimensions supercede scaled

mensions. Refer all questions to

Mark Disosway, P.E. for resolution.

2056, 386-754-5419

IMENSIONS

P.E. 53915

SEAL

Willber Brown

McCloud Mother-in-law's

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November 05, 2007 DRAWN BY: STRUCTURAL B Evan Beamsley Evan Beamsley

FINALS DATE: Nov. 2, 2007

JOB NUMBER: 710247 DRAWING NUMBER

> 5-1 OF 2 SHEETS

-6X6 SYP #2 POST w/ (12) - 16d & 5/8" x 10" ANCHOR BOLT

> TYPICAL PORCH POST DETAIL SCALE: 1/2" = 1'-0"

-(2) 2X12 SYP #2 U.N.O. SEE STRUCTURAL PLAN (2) SIMPSON LSTA21w/(8)-16d TO HEADER AND (8) -16d TO POST -SIMPSON ABU POST BASE SEE FOOTING DETAILS

CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

CONCRETE: MINIMUM COMPRESSIVE STREIGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

FIBER CONCRETE SLAB: CONCRETE SLABSON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT.

CONTROL JOINTS: WHERE SPECIFIED, SAVN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO WASHERS: WASHERS USED WITH 1/2" BOLI'S TO BE 2" x 2" x 9/64"; WITH 5/8" BOLITS TO BE 3" x 3" x 9/64"; WITH

NAILS: ALL NAILS ARE COMMON NAILS UNLISS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.