

Alpine, an ITW Company
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025
Phone: (800)755-6001
www.alpineitw.com

FL REG# 278, Yoonhwak Kim, FL PE #86367
Florida Certificate of Product Approval #FL 1999
11/18/2022

Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 22-8515
Job Description: Dave Blank	
Address: Little Rd, LAKE CITY	

Job Engineering Criteria:	
Design Code: FBC 7th Ed. 2020 Res.	IntelliVIEW Version: 21.02.01 JRef #: 1XKQ2150005
Wind Standard: ASCE 7-16 Wind Speed (mph): 150	Design Loading (psf): 47.00
Building Type: Closed	

This package contains general notes pages, 17 truss drawing(s) and 2 detail(s).

Item	Drawing Number	Truss
1	322.22.0643.33020	A01
3	322.22.0643.37427	A03
5	322.22.0643.41827	B01
7	322.22.0643.45550	C01
9	322.22.0644.00757	C03G
11	321.22.1656.48079	C05G
13	321.22.1656.48017	HJ01
15	321.22.1656.48096	J02
17	321.22.1656.48049	J04
19	GBLLETIN0118	

Item	Drawing Number	Truss
2	322.22.0643.34853	A02
4	322.22.0643.39703	A04
6	322.22.0643.43600	B02
8	322.22.0643.48570	C02
10	322.22.0644.03080	C04
12	322.22.0644.21477	G01
14	321.22.1656.48032	J01
16	321.22.1656.48018	J03
18	A16015ENC160118	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

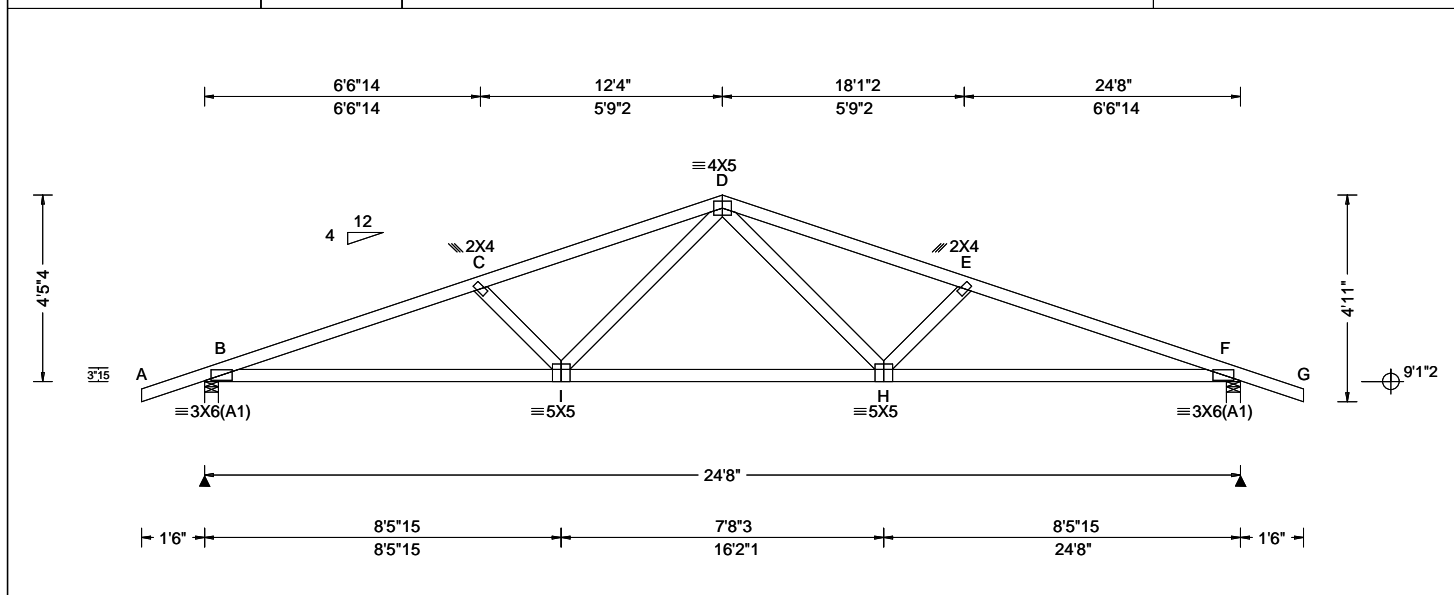
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com.

SEQN: 448842 FROM: CDM	COMN Ply: 1 Qty: 7	Job Number: 22-8515 Dave Blank Truss Label: A01	Cust: R 215 JRef: 1XKQ2150005 T6 DrwNo: 322.22.0643.33020 SSB / YK 11/18/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 17.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 47.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 150 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.122 D 999 360 VERT(CL): 0.286 D 999 298 HORZ(LL): 0.037 F - - HORZ(TL): 0.088 F - - Creep Factor: 2.0 Max TC CSI: 0.518 Max BC CSI: 0.854 Max Web CSI: 0.271 VIEW Ver: 21.02.01.1214.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1302 -/- /- /880 /367 /152 F 1302 -/- /- /880 /367 -/ Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 1.5 (Truss) F Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 1452 -2724 D - E 1309 -2397 C - D 1309 -2397 E - F 1453 -2724

Lumber

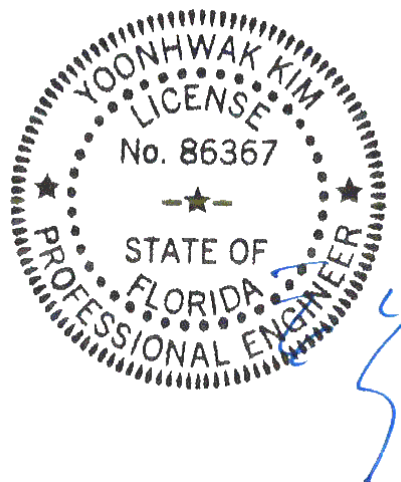
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 4-5-4.

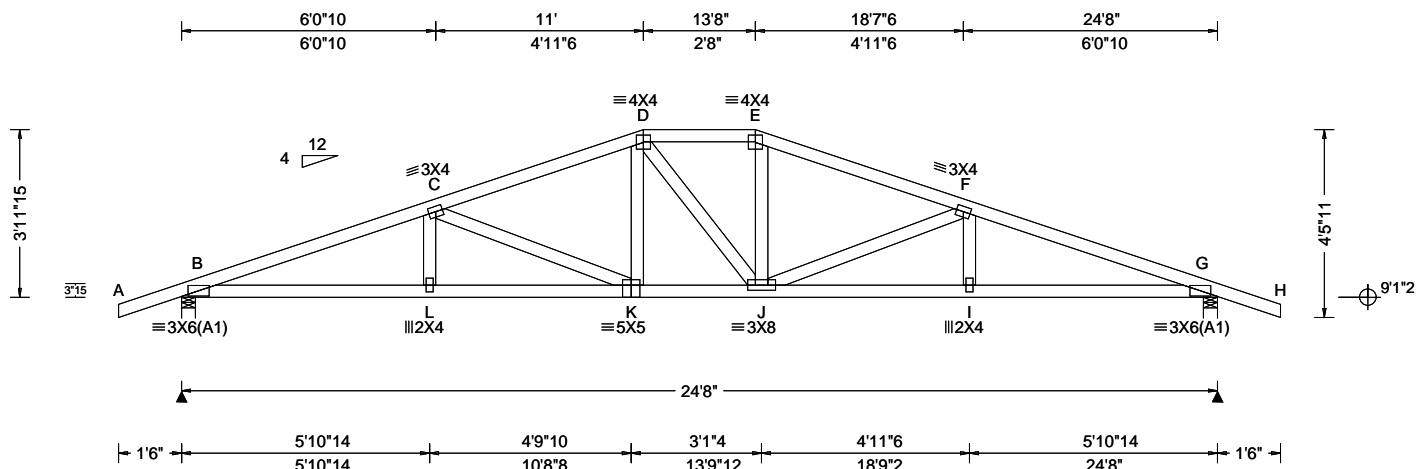


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****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

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SEQN: 448840 FROM: CDM	HIPS Ply: 1 Qty: 1	Job Number: 22-8515 Dave Blank Truss Label: A02	Cust: R 215 JRef: 1XKQ2150005 T5 DrwNo: 322.22.0643.34853 SSB / YK 11/18/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 17.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 47.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 150 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.125 K 999 360 VERT(CL): 0.295 K 990 298 HORZ(LL): 0.041 G - - HORZ(TL): 0.097 G - - Creep Factor: 2.0 Max TC CSI: 0.413 Max BC CSI: 0.671 Max Web CSI: 0.394 VIEW Ver: 21.02.01.1214.12	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 1302 - / - / /881 /368 /139 G 1302 - / - / /881 /368 - / - Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 1.5 (Truss) G Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings B & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 1337 -2786 E - F 1106 -2086 C - D 1112 -2097 F - G 1336 -2785 D - E 1095 -1903

Lumber

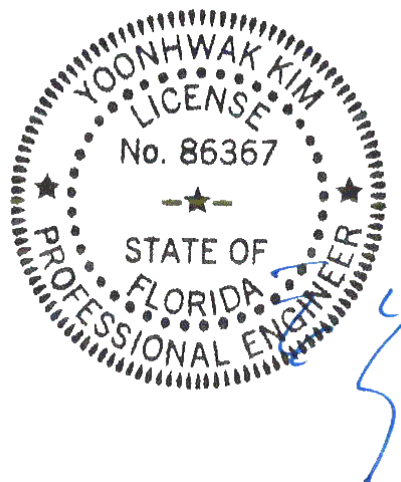
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 3-11-15.



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Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - L	2585 -1156	J - I	2580 -1169
L - K	2580 -1159	I - G	2584 -1166
K - J	1899 -825		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
C - K	354 -711	E - J	377 -118
K - D	376 -109	J - F	358 -718

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 17.00	Speed: 150 mph	Pf: NA Ce: NA	VERT(LL): 0.127 D 999 360	Loc R+ /R- /Rh /Rw /U /RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.300 D 975 298	B 1302 -/- /- /878 /369 /119
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.041 G - -	G 1302 -/- /- /878 /369 -/-
Des Ld: 47.00	EXP: C Kzt: NA		HORZ(TL): 0.097 G - -	Wind reactions based on MWFRS
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0	B Brg Wid = 4.0 Min Req = 1.5 (Truss)
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.785	G Brg Wid = 4.0 Min Req = 1.5 (Truss)
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.754	Bearings B & G are a rigid surface.
Spacing: 24.0 "	MWFRS Parallel Dist: h/2 to h	Rep Fac: Yes	Max Web CSI: 0.150	Members not listed have forces less than 375#
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)		Maximum Top Chord Forces Per Ply (lbs)
	Loc. from endwall: not in 9.00 ft	Plate Type(s):		Chords Tens.Comp. Chords Tens. Comp.
	GCpi: 0.18	WAVE	VIEW Ver: 21.02.01.1214.12	B - C 1587 -2806 E - F 1490 -2390
	Wind Duration: 1.60			

The overall height of this truss excluding overhang is 3-3-15.



Maximum Web Forces Per Ply (lbs)					
Webs	Tens.Comp.		Webs	Tens. Comp.	
C - K	172	-400	E - J	384	-17
K - D	384	-20	J - F	176	-407

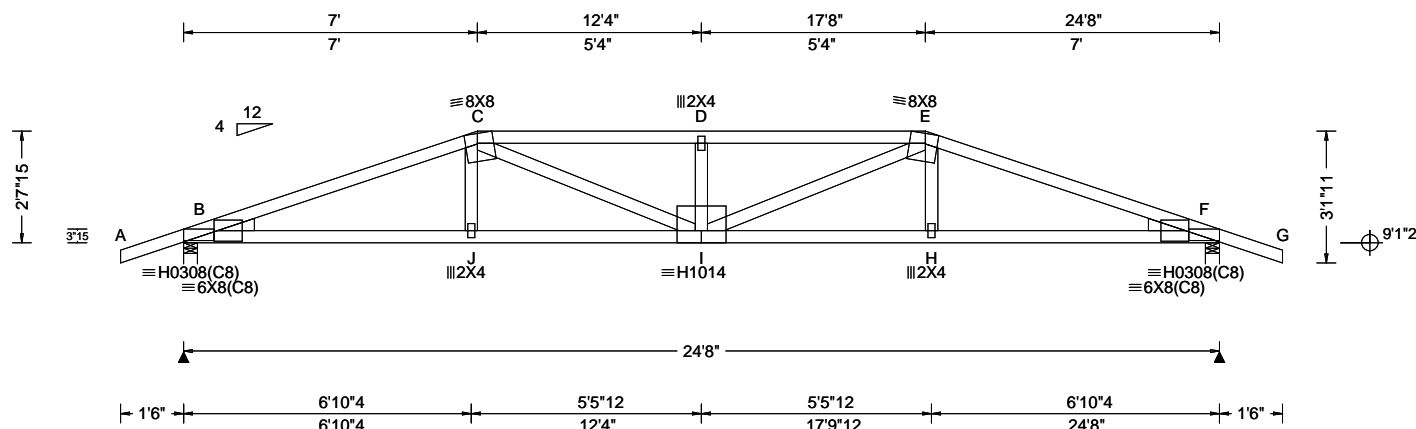
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SEQN: 448847 FROM: CDM	HIPS Ply: 1 Qty: 1	Job Number: 22-8515 Dave Blank Truss Label: A04	Cust: R 215 JRef: 1XKQ2150005 T8 DrwNo: 322.22.0643.39703 SSB / YK 11/18/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 17.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 47.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 150 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT: 20(0)/10(0) Plate Type(s): HS, WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.345 D 846 360 VERT(CL): 0.811 D 360 298 HORZ(LL): 0.082 F - - HORZ(TL): 0.193 F - - Creep Factor: 2.0 Max TC CSI: 0.966 Max BC CSI: 0.881 Max Web CSI: 0.742 VIEW Ver: 21.02.01.1214.12	Gravity Loc R+ / R- / Rh / Rw / U / RL B 2749 - / - / - / 819 - / - F 2749 - / - / - / 819 - / - Non-Gravity B Brg Wid = 4.0 Min Req = 2.3 (Truss) F Brg Wid = 4.0 Min Req = 2.3 (Truss) Wind reactions based on MWFRS Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 2087 - 7109 D - E 2464 - 8406 C - D 2464 - 8406 E - F 2087 - 7109

Lumber

Top chord: 2x4 SP M-31;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP #3;
Lt Wedge: 2x4 SP #3; Rt Wedge: 2x4 SP #3;

Special Loads

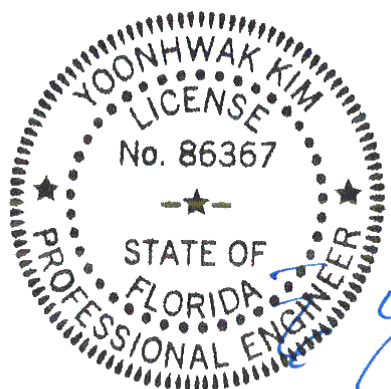
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 76 plf at -1.50 to 76 plf at 7.00
TC: From 38 plf at 7.00 to 38 plf at 17.67
TC: From 76 plf at 17.67 to 76 plf at 26.17
BC: From 4 plf at -1.50 to 4 plf at 0.00
BC: From 20 plf at 0.00 to 20 plf at 7.03
BC: From 10 plf at 7.03 to 10 plf at 17.64
BC: From 20 plf at 17.64 to 20 plf at 24.67
BC: From 4 plf at 24.67 to 4 plf at 26.17
TC: 308 lb Conc. Load at 7.03, 17.64
TC: 224 lb Conc. Load at 9.06, 11.06, 12.33, 13.60
15.60
BC: 504 lb Conc. Load at 7.03, 17.64
BC: 132 lb Conc. Load at 9.06, 11.06, 12.33, 13.60
15.60

Wind

Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 27-15.

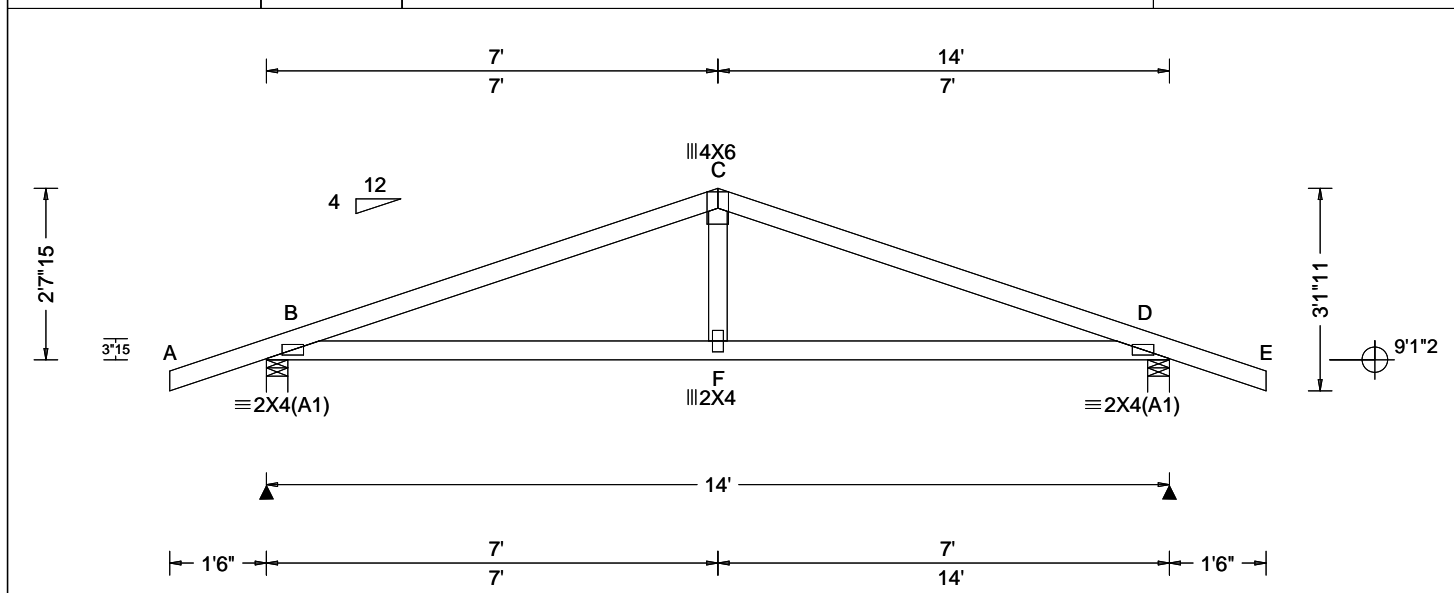


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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 448832 FROM: CDM	COMN Ply: 1 Qty: 4	Job Number: 22-8515 Dave Blank Truss Label: B01	Cust: R 215 JRef: 1XKQ2150005 T1 DrwNo: 322.22.0643.41827 SSB / YK 11/18/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 17.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 47.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 150 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.028 F 999 360 VERT(CL): 0.066 F 999 298 HORZ(LL): 0.009 D - - HORZ(TL): 0.022 D - - Creep Factor: 2.0 Max TC CSI: 0.555 Max BC CSI: 0.664 Max Web CSI: 0.119 VIEW Ver: 21.02.01.1214.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 791 -/- /- /552 /225 /100 D 791 -/- /- /552 /225 -/ Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 1.5 (Truss) D Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings B & D are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 1016 - 1197 C - D 1016 - 1197

Lumber

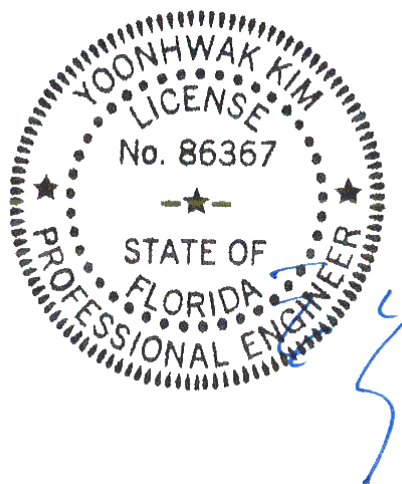
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 2-7-15.

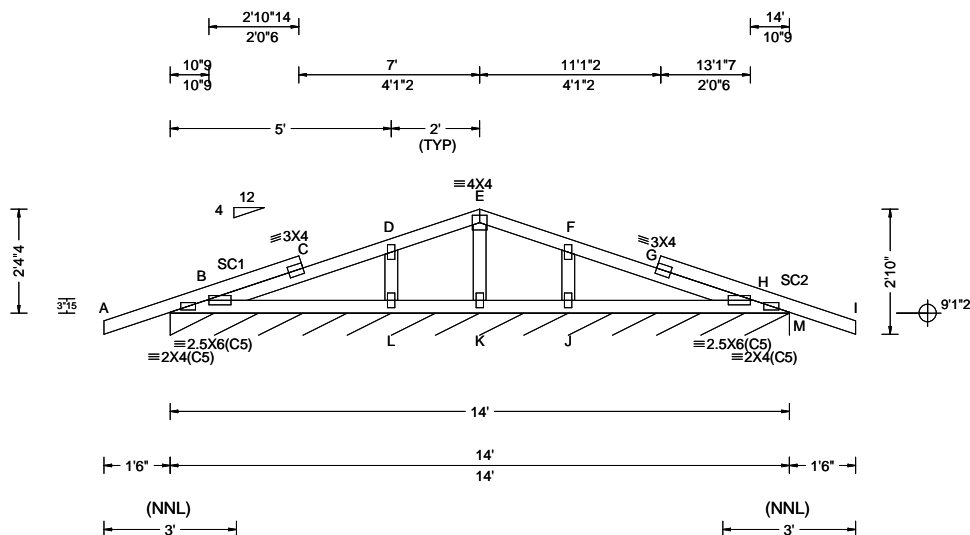


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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 448830 FROM: CDM	GABL Ply: 1 Qty: 1	Job Number: 22-8515 Dave Blank Truss Label: B02	Cust: R 215 JRef: 1XKQ2150005 T2 DrwNo: 322.22.0643.43600 SSB / YK 11/18/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 17.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 47.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 150 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.013 C 999 360 VERT(CL): 0.031 C 999 298 HORZ(LL): 0.004 C - - HORZ(TL): 0.010 C - - Creep Factor: 2.0 Max TC CSI: 0.319 Max BC CSI: 0.143 Max Web CSI: 0.084 VIEW Ver: 21.02.01.1214.12	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity M* 113 /- /- /65 /31 /7 Wind reactions based on MWFRS M Brg Wid = 168 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Gable Forces Per Ply (lbs) Gables Tens.Comp. Gables Tens. Comp. D - L 393 -280 J - F 394 -280

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.

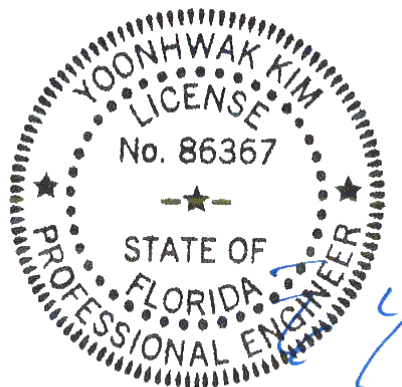
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A16015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in noticable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in noticable area using 3x6.

The overall height of this truss excluding overhang is 2'-4"-4."



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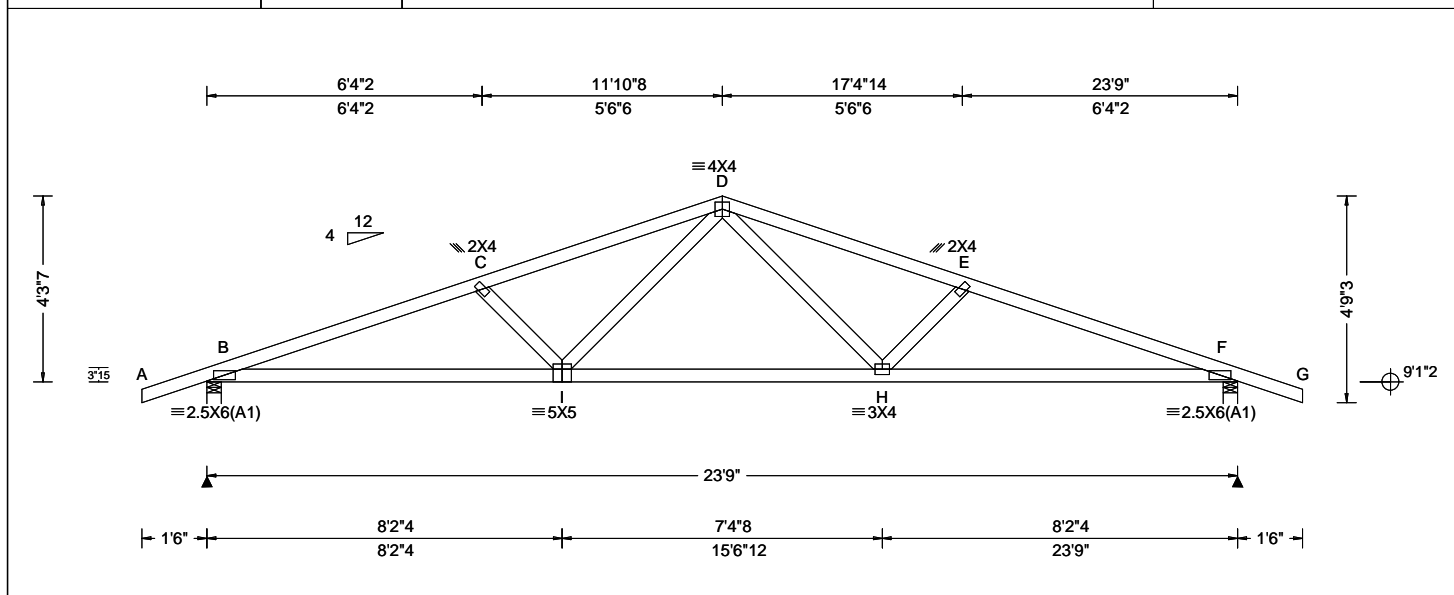
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 448850 FROM: CDM	COMN Ply: 1 Qty: 8	Job Number: 22-8515 Dave Blank Truss Label: C01	Cust: R 215 JRef: 1XKQ2150005 T3 DrwNo: 322.22.0643.45550 SSB / YK 11/18/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 17.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 47.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 150 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.113 D 999 360 VERT(CL): 0.265 D 999 298 HORZ(LL): 0.035 F - - HORZ(TL): 0.081 F - - Creep Factor: 2.0 Max TC CSI: 0.462 Max BC CSI: 0.803 Max Web CSI: 0.259 VIEW Ver: 21.02.01.1214.12	Gravity Loc R+ / R- / Rh / Rw / U / RL B 1258 - / - / /852 /355 /147 F 1258 - / - / /852 /355 - Non-Gravity Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 1.5 (Truss) F Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 1450 -2610 D - E 1307 -2299 C - D 1307 -2297 E - F 1450 -2611

Lumber

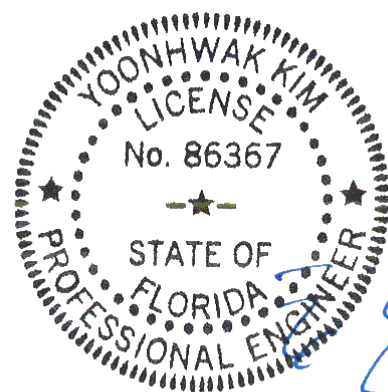
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 4-3-7.

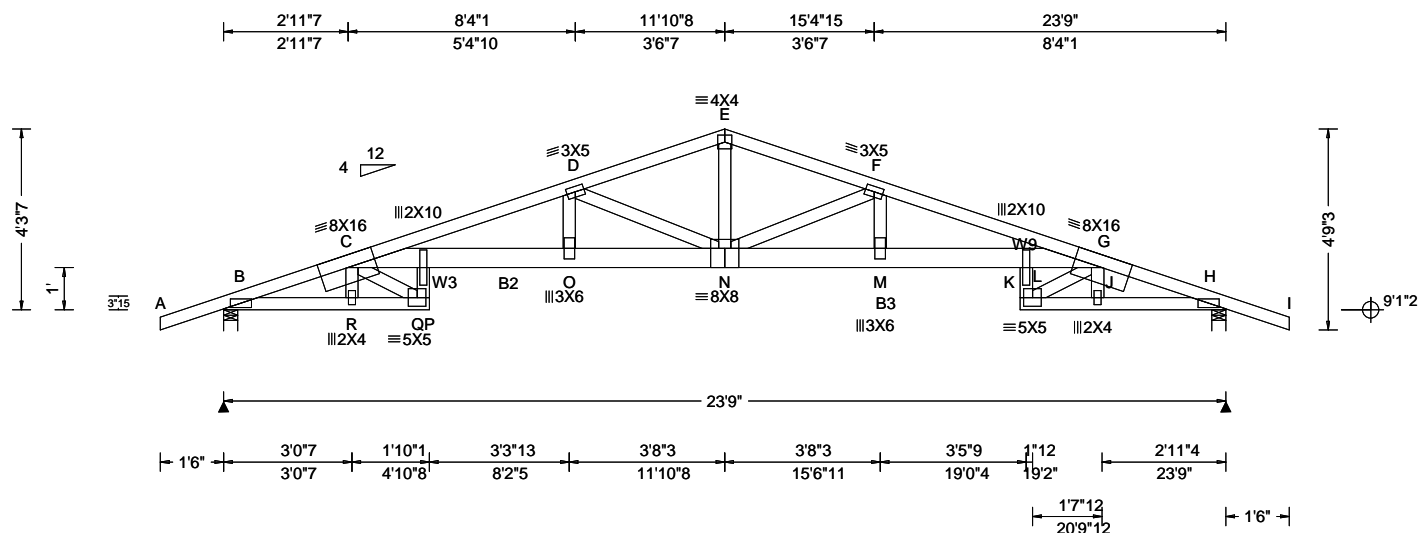


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SEQN: 448859 FROM: CDM	COMN Ply: 1 Qty: 6	Job Number: 22-8515 Dave Blank Truss Label: C02	Cust: R 215 JRef: 1XKQ2150005 T16 DrwNo: 322.22.0643.48570 SSB / YK 11/18/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 17.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 47.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 150 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.329 L 854 360 VERT(CL): 0.775 L 363 298 HORZ(LL): 0.174 H - - HORZ(TL): 0.408 H - - Creep Factor: 2.0 Max TC CSI: 0.443 Max BC CSI: 0.813 Max Web CSI: 0.571 VIEW Ver: 21.02.01.1214.12	Gravity Loc R+ / R- / Rh / Rw / U / RL B 1258 - / - / - / 851 / 354 / 184 H 1258 - / - / - / 851 / 356 - / - Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 1.5 (Truss) H Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings B & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 1087 - 2557 E - F 1062 - 2380 C - D 1698 - 4062 F - G 1652 - 4062 D - E 1053 - 2380 G - H 1096 - 2557

Lumber

Top chord: 2x4 SP M-31;
Bot chord: 2x4 SP #2; B2, B3 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3; W3, W9 2x4 SP #2;

Plating Notes

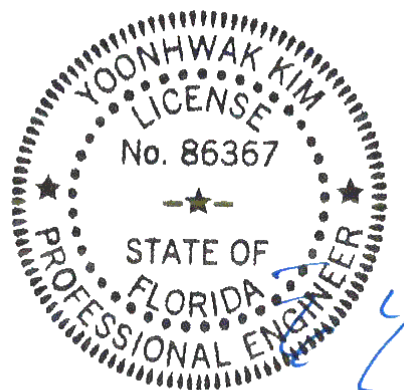
All plates are 2.5X6(A1) except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 4-3-7.

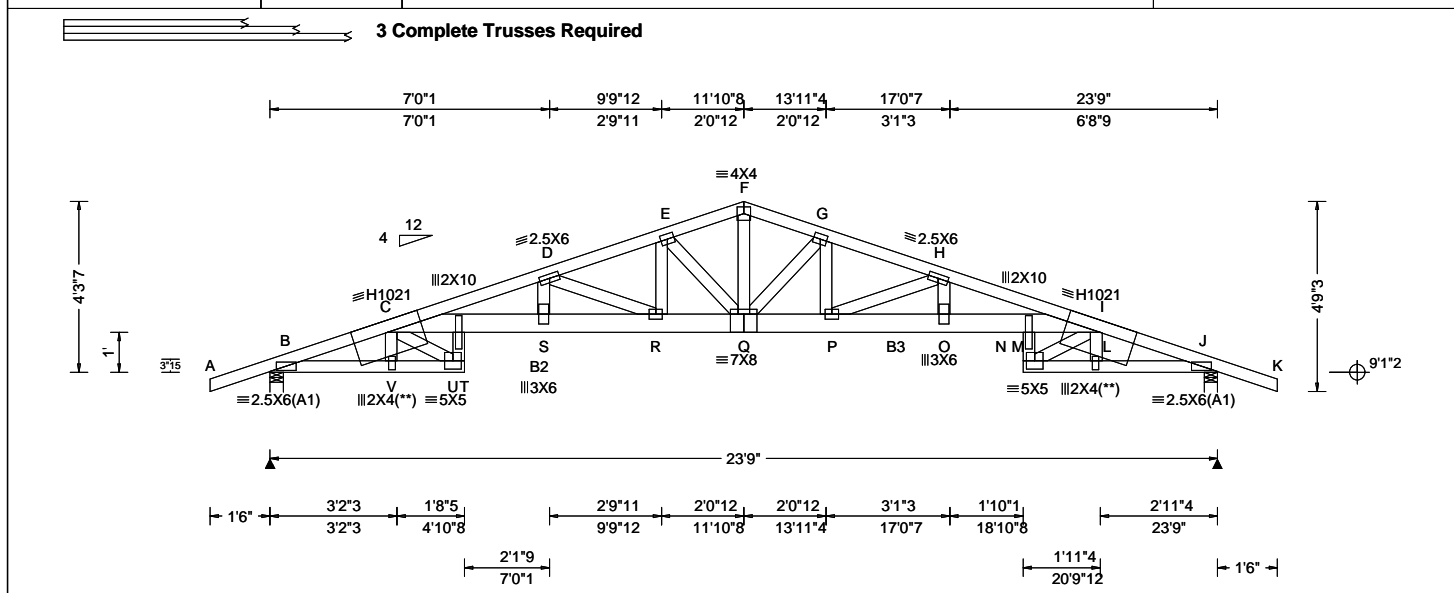


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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 450673 FROM: CDM	COMN Ply: 3 Qty: 2	Job Number: 22-8515 Dave Blank Truss Label: C03G	Cust: R 215 JRRef: 1XKQ2150005 T14 DrwNo: 322.22.0644.00757 SSB / YK 11/18/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 17.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 47.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 150 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/def L/# VERT(LL): 0.279 Q 999 360 VERT(CL): 0.680 Q 413 298 HORZ(LL): 0.136 J - - HORZ(TL): 0.332 J - - Creep Factor: 2.0 Max TC CSI: 0.636 Max BC CSI: 0.651 Max Web CSI: 0.518 VIEW Ver: 21.02.01.1214.12	Gravity Loc R+ / R- / Rh / Rw / U / RL B 2654 -/- /- /- /663 -/ J 2654 -/- /- /- /663 -/ Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 1.5 (Truss) J Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings B & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 495 -2072 F - G 580 -2509 C - D 993 -4203 G - H 728 -3115 D - E 728 -3115 H - I 993 -4203 E - F 580 -2509 I - J 495 -2072

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2; B2, B3 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

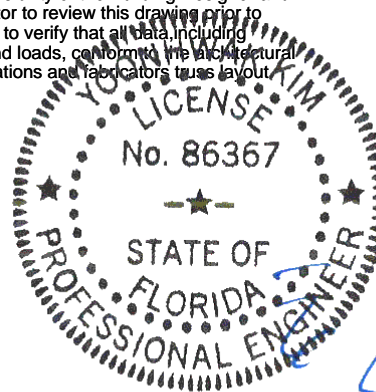
Nailnote
Nail Schedule: 0.128"x3", min. nails
Top Chord: 1 Row @ 8.50" o.c.
Bot Chord: 1 Row @ 12.00" o.c.
Webs : 1 Row @ 4" o.c.
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads
----- (Lumber Dur. Fac. = 1.25 / Plate Dur. Fac. = 1.25)
TC: From 76 plf at -1.50 to 76 plf at 9.67
TC: From 138 plf at 9.67 to 138 plf at 9.69
TC: From 266 plf at 9.69 to 266 plf at 14.06
TC: From 138 plf at 14.06 to 138 plf at 14.08
TC: From 76 plf at 14.08 to 76 plf at 25.25
BC: From 4 plf at -1.50 to 4 plf at 0.00
BC: From 20 plf at 0.00 to 20 plf at 23.75
BC: From 4 plf at 23.75 to 4 plf at 25.25
BC: 978 lb Conc. Load at 9.81, 13.94

Plating Notes
All plates are 3X4 except as noted.
(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind
Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

Additional Notes
The overall height of this truss excluding overhang is 4-3-7.
It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data, including dimensions and loads, conform to the architectural plans/specifications and fabricator's truss layout.



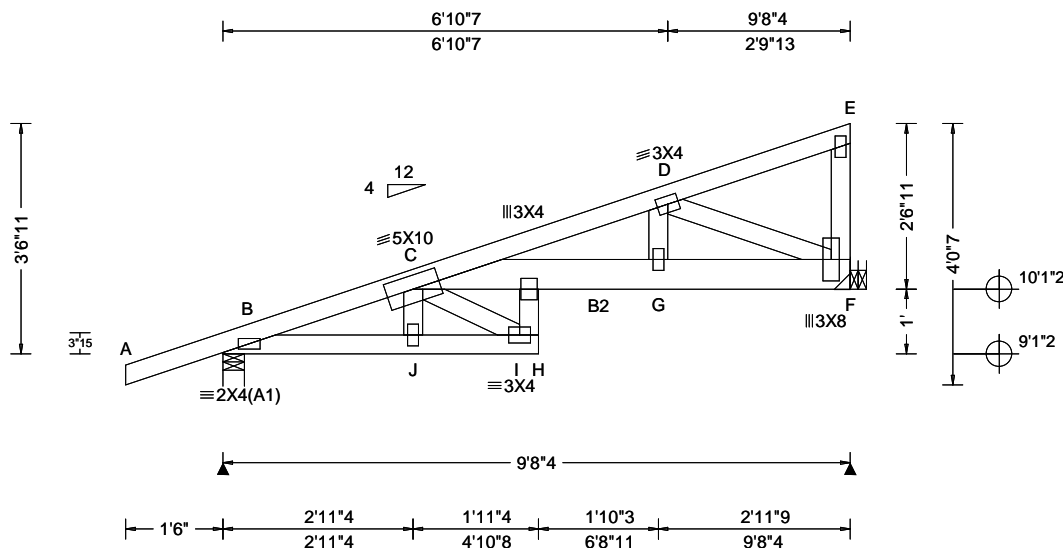
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 448865 FROM: CDM	MONO Ply: 1 Qty: 4	Job Number: 22-8515 Dave Blank Truss Label: C04	Cust: R 215 JRef: 1XKQ2150005 T15 DrwNo: 322.22.0644.03080 SSB / YK 11/18/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 17.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 47.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 150 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.054 H 999 360 VERT(CL): 0.126 H 910 298 HORZ(LL): 0.015 F - - HORZ(TL): 0.035 F - - Creep Factor: 2.0 Max TC CSI: 0.266 Max BC CSI: 0.536 Max Web CSI: 0.227 VIEW Ver: 21.02.01.1214.12	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 603 - / - / 459 / 148 / 171 F 446 - / - / 331 / 149 / - Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 1.5 (Truss) F Brg Wid = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 446 -857 C - D 608 -976

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2; B2 2x6 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=9'5"4 uses the following support conditions: 9'5"4

Bearing F (9'5"4, 10'1"2) LUS26
Supporting Member: (2)2x6 SP #2
(4) 0.148"x3" nails into supporting member,
(3) 0.148"x3" nails into supported member.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

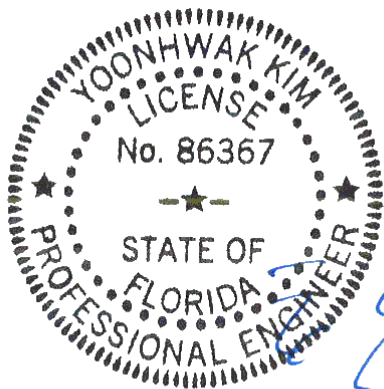
The overall height of this truss excluding overhang is 3-6-11.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - J	783 -648	I - G	887 -718
C - I	922 -746	G - F	858 -702
J - H	783 -645		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
C - H	762 -927	G - D	380 -189
I - H	458 -356	D - F	761 -931



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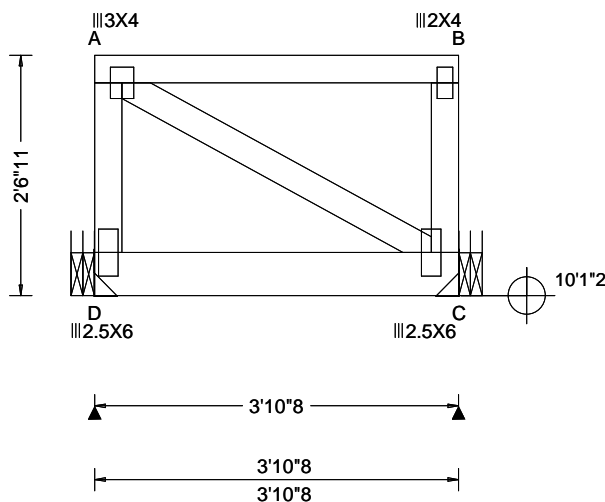
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 450661 FROM: CDM	FLAT Ply: 2 Qty: 2	Job Number: 22-8515 Dave Blank Truss Label: G01	Cust: R 215 JRef: 1XKQ2150005 T19 DrwNo: 322.22.0644.21477 SSB / YK 11/18/2022
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2 Complete Trusses Required



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 17.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 47.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 150 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 10.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.000 B 999 360 VERT(CL): 0.001 B 999 298 HORZ(LL): -0.000 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.580 Max BC CSI: 0.439 Max Web CSI: 0.111 VIEW Ver: 21.02.01.1214.12	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity Loc R+ / R- / Rh / Rw / U / RL D 978 - / - / - /239 - C 978 - / - / - /239 - Wind reactions based on MWFRS D Brg Wid = - Min Req = - C Brg Wid = - Min Req = - Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x6 SP #2;
Webs: 2x4 SP #3;

Nailnote

Nail Schedule: 0.128"x3", min. nails
Top Chord: 1 Row @ 11.50" o.c.
Bot Chord: 1 Row @ 6.50" o.c.
Webs: 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 264 plf at 0.00 to 264 plf at 3.88
BC: From 10 plf at 0.00 to 10 plf at 3.88
BC: 446 lb Conc. Load at 1.81, 2.06

Hangers / Ties

(J) Hanger Support Required, by others

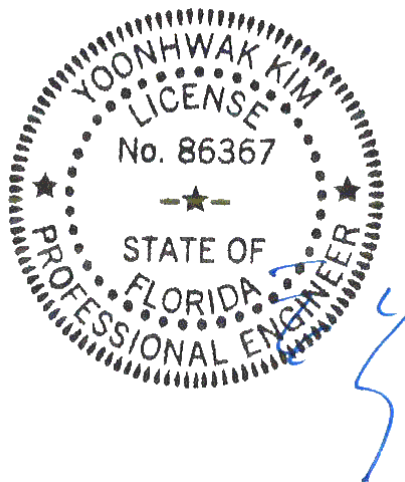
Wind

Wind loads and reactions based on MWFRS.
End verticals not exposed to wind pressure.

It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data, including dimensions and loads, conform to the architectural plans/specifications and fabricators truss layout.

Additional Notes

Truss must be installed as shown with top chord up.
Wall girder loading on this truss.
The overall height of this truss excluding overhang is 2-6-11.



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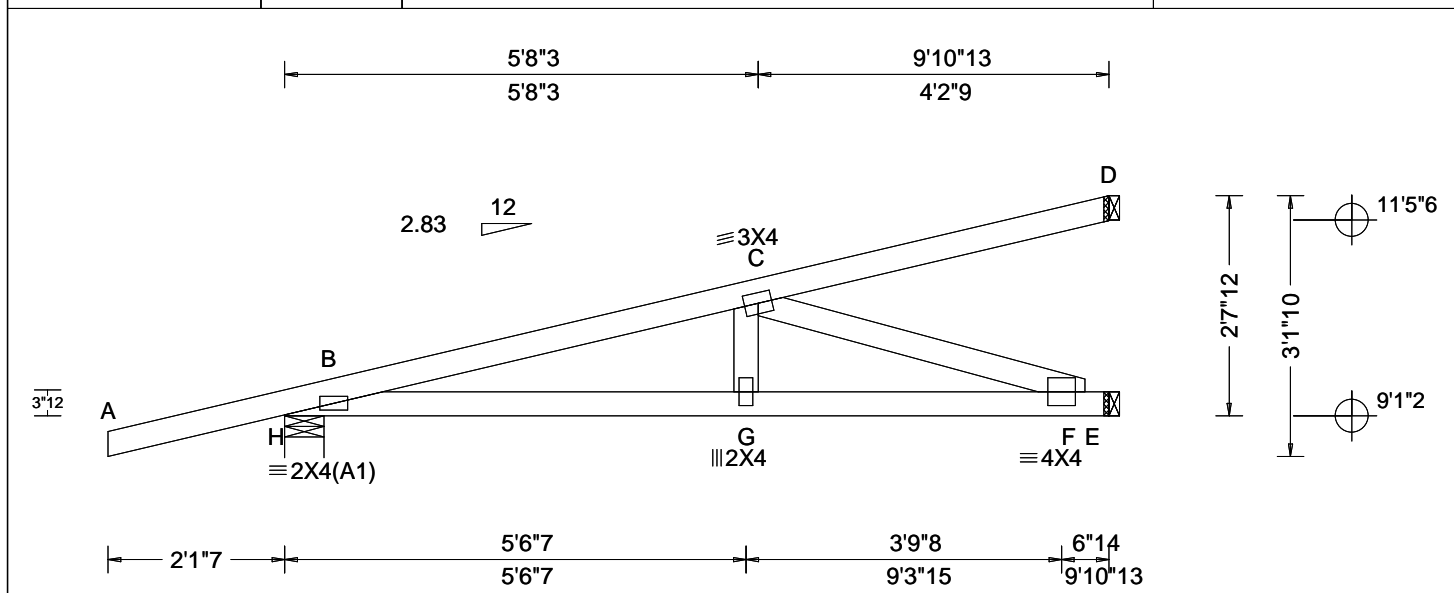
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 448843 / FROM: CDM	HIP_	Ply: 1 Qty: 2	Job Number: 22-8515 Dave Blank Truss Label: HJ01	Cust: R 215 JRef: 1XKQ2150005 T13 DrwNo: 321.22.1656.48017 SSB / YK 11/17/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 17.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 47.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 150 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.034 G 999 360 VERT(CL): 0.075 G 999 298 HORZ(LL): 0.007 F - - HORZ(TL): 0.016 F - - Creep Factor: 2.0 Max TC CSI: 0.602 Max BC CSI: 0.734 Max Web CSI: 0.411 VIEW Ver: 21.02.01.1214.12	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity H 470 -/- /- /270 -/ E 371 -/- /- /118 -/ D 84 -/- /- /30 -/ Wind reactions based on MWFRS H Brg Wid = 5.7 Min Req = 1.5 (Truss) E Brg Wid = 1.5 Min Req = - D Brg Wid = 1.5 Min Req = - Bearing H is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. B - C 439 - 1040 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - G 1016 -403 G - F 996 -402 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. C - F 425 - 1053

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Special Loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

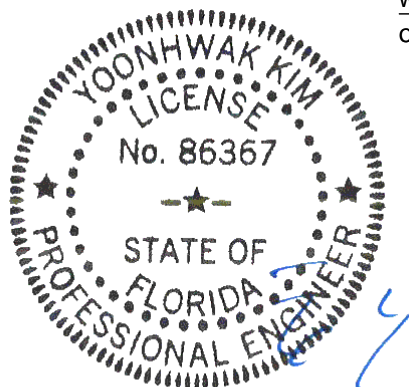
TC: From 0 plf at -2.12 to 75 plf at 0.00
TC: From 2 plf at 0.00 to 2 plf at 9.90
BC: From 0 plf at -2.12 to 4 plf at 0.00
BC: From 2 plf at 0.00 to 2 plf at 9.90
TC: -55 lb Conc. Load at 1.48
TC: 148 lb Conc. Load at 4.31
TC: 304 lb Conc. Load at 7.13
BC: -8 lb Conc. Load at 1.48
BC: 95 lb Conc. Load at 4.31
BC: 182 lb Conc. Load at 7.13

Wind

Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 27'-7.12.

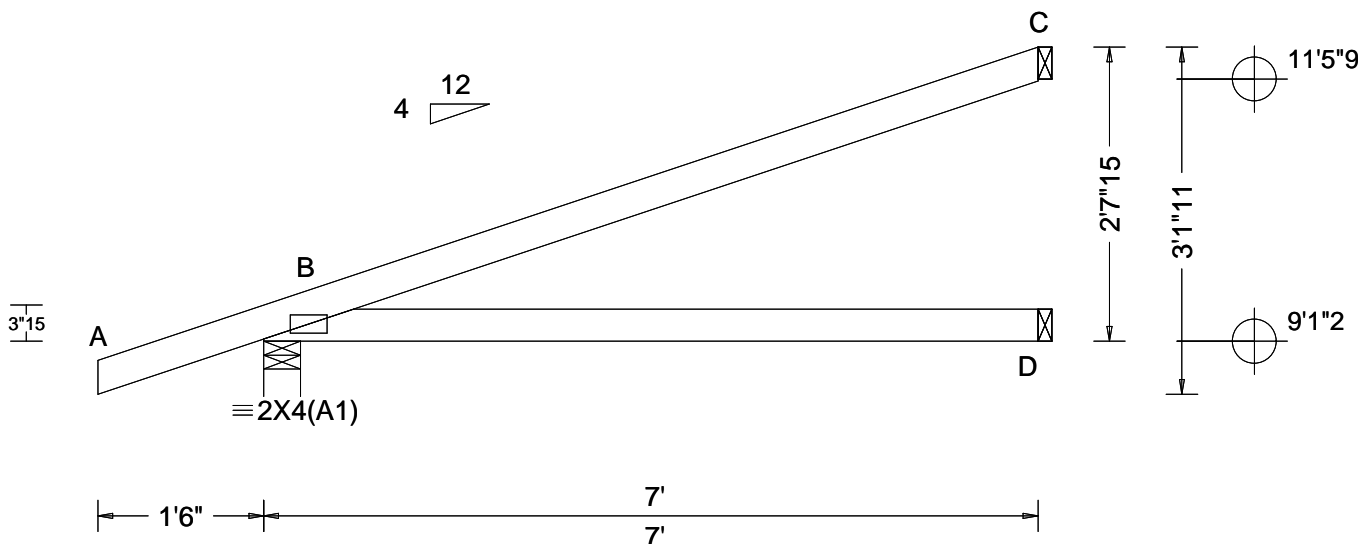


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Florida State Seal of Professional Engineer

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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 448836 / FROM: CDM	EJAC Ply: 1 Qty: 7	Job Number: 22-8515 Dave Blank Truss Label: J01	Cust: R 215 JRRef: 1XKQ2150005 T12 DrwNo: 321.22.1656.48032 SSB / YK 11/17/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 17.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 47.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 150 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.015 B - - HORZ(TL): 0.034 B - - Creep Factor: 2.0 Max TC CSI: 0.850 Max BC CSI: 0.538 Max Web CSI: 0.000 VIEW Ver: 21.02.01.1214.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 478 - / - / - /371 /121 /128 D 132 - / - / - /77 - / - C 224 - / - / - /154 /116 - Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

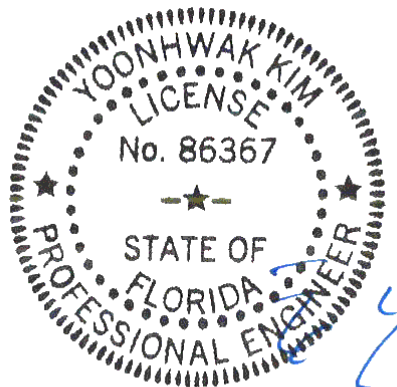
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 2'-7-15.

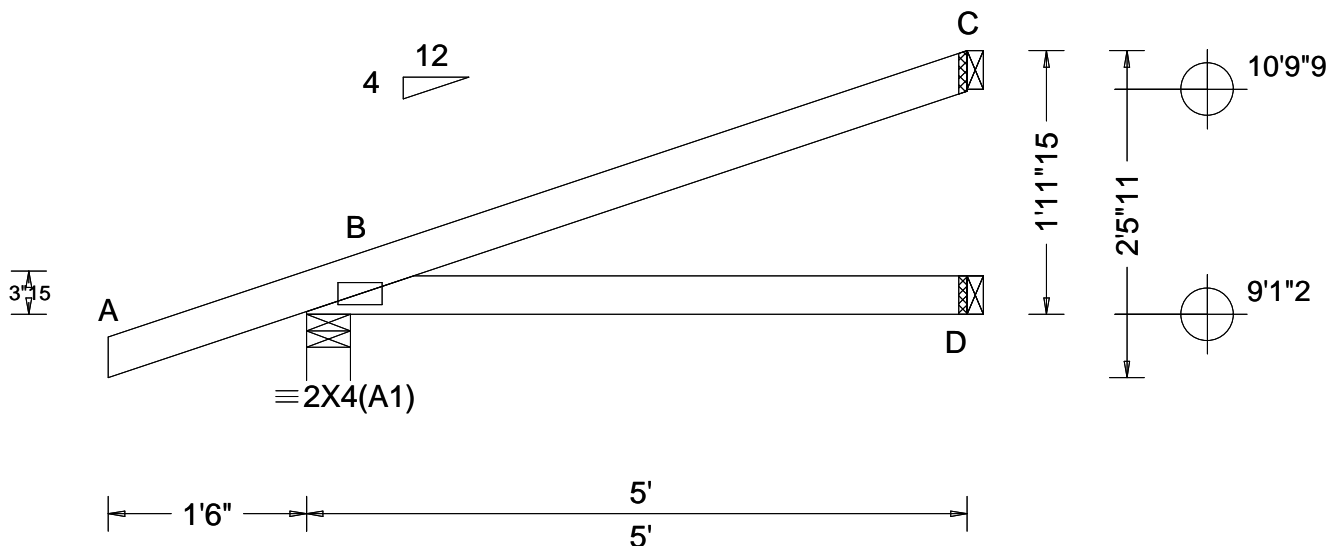


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North Building, 4th Floor
Glenview, IL 60025

SEQN: 448835 / FROM: CDM	JACK Ply: 1 Qty: 4	Job Number: 22-8515 Dave Blank Truss Label: J02	Cust: R 215 JRef: 1XKQ2150005 T9 DrwNo: 321.22.1656.48096 SSB / YK 11/17/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 17.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 47.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 150 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.004 B - - HORZ(TL): 0.010 B - - Creep Factor: 2.0 Max TC CSI: 0.367 Max BC CSI: 0.239 Max Web CSI: 0.000 VIEW Ver: 21.02.01.1214.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 389 - / - /309 /102 /97 D 91 - / - /51 - / - C 152 - / - /103 /79 - Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 1-11-15.



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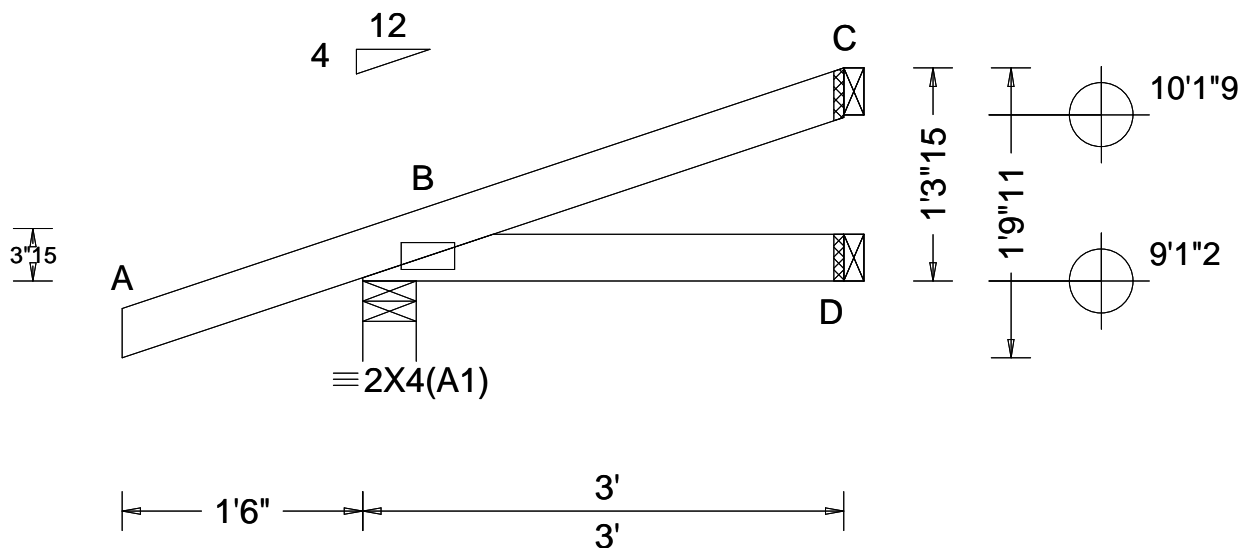
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 448834 / FROM: CDM	JACK Ply: 1 Qty: 4	Job Number: 22-8515 Dave Blank Truss Label: J03	Cust: R 215 JRef: 1XKQ2150005 T10 DrwNo: 321.22.1656.48018 SSB / YK 11/17/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 17.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 47.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 150 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.001 B - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.209 Max BC CSI: 0.059 Max Web CSI: 0.000 VIEW Ver: 21.02.01.1214.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 309 /- /- /256 /96 /65 D 48 /- /- /27 /- /- C 74 /- /- /46 /40 /- Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

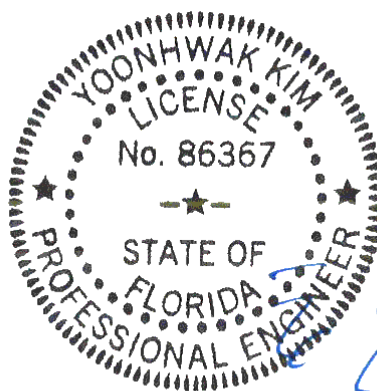
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 1'-3-15.



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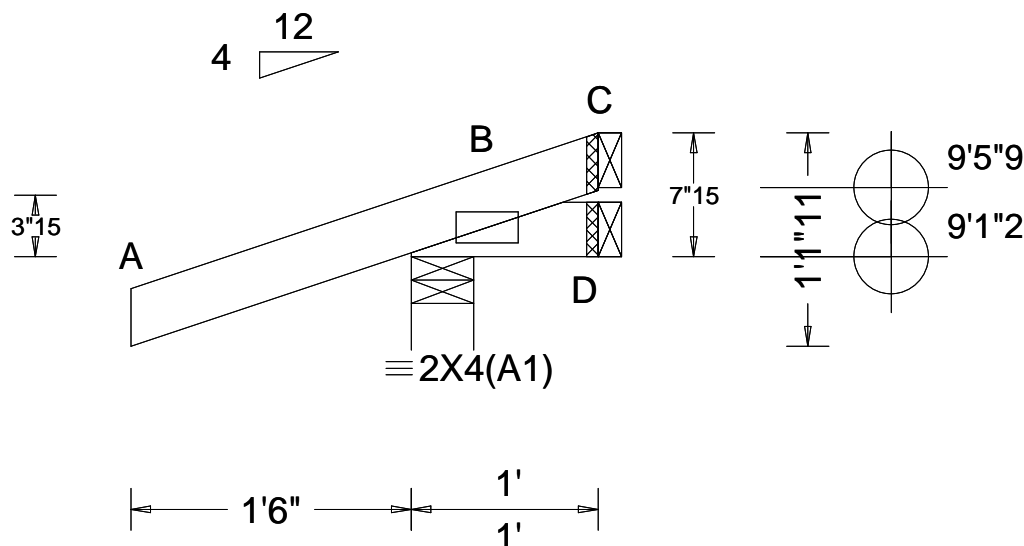
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 448833 / FROM: CDM	JACK Ply: 1 Qty: 4	Job Number: 22-8515 Dave Blank Truss Label: J04	Cust: R 215 JRef: 1XKQ2150005 T11 DrwNo: 321.22.1656.48049 SSB / YK 11/17/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 17.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 47.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 150 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.000 D - - HORZ(TL): 0.000 D - - Creep Factor: 2.0 Max TC CSI: 0.353 Max BC CSI: 0.045 Max Web CSI: 0.000 VIEW Ver: 21.02.01.1214.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 302 /- /- /272 /141 /37 D - /-25 /- /27 /27 /- C - /-61 /- /48 /63 /- Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = - C Brg Wid = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

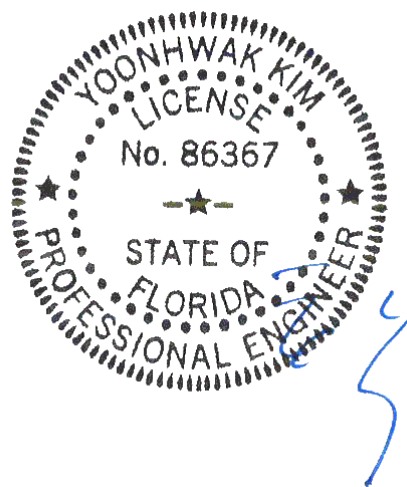
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 0'-7-15.



FL REG# 278, Yoonhwak Kim, FL PE #86367
Florida Certificate of Product Approval #FL 1999

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ALPINE
AN ITW COMPANY
155 Harlem Ave
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Glenview, IL 60025

Gable Stud Reinforcement Detail

ASCE 7-16: 160 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 140 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Or: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

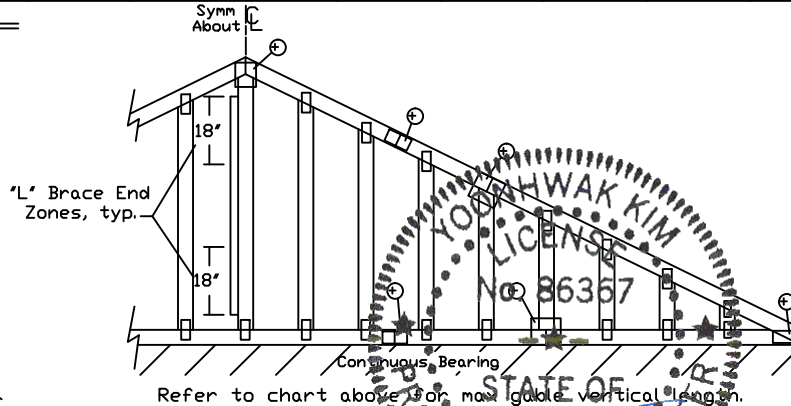
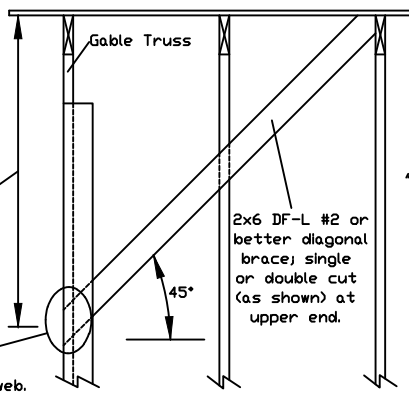
Or: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length	2x4 Gable Vertical		Brace Grade	No Braces	(1) 1x4 'L' Brace *		(1) 2x4 'L' Brace *		(2) 2x4 'L' Brace **		(1) 2x6 'L' Brace *		(2) 2x6 'L' Brace **	
	Spacing	Species			Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" O.C.	SPF	#1 / #2	#1	3' 10"	6' 7"	6' 10"	7' 9"	8' 1"	9' 3"	9' 7"	12' 2"	12' 8"	14' 0"	14' 0"
			#3	3' 8"	5' 9"	6' 2"	7' 8"	7' 11"	9' 1"	9' 6"	12' 0"	12' 6"	14' 0"	14' 0"
			Stud	3' 8"	5' 9"	6' 1"	7' 8"	7' 11"	9' 1"	9' 6"	12' 0"	12' 6"	14' 0"	14' 0"
		Standard	#1	3' 8"	4' 11"	5' 3"	6' 7"	7' 1"	8' 11"	9' 6"	10' 4"	11' 1"	14' 0"	14' 0"
			#2	4' 0"	6' 8"	6' 11"	7' 10"	8' 2"	9' 4"	9' 8"	12' 4"	12' 9"	14' 0"	14' 0"
			#3	3' 10"	6' 7"	6' 10"	7' 9"	8' 1"	9' 3"	9' 7"	12' 2"	12' 8"	14' 0"	14' 0"
	SP	DFL	#1	3' 9"	5' 3"	5' 7"	6' 11"	7' 5"	9' 2"	9' 7"	10' 11"	11' 8"	14' 0"	14' 0"
			Stud	3' 9"	5' 3"	5' 7"	6' 11"	7' 5"	9' 2"	9' 7"	10' 11"	11' 8"	14' 0"	14' 0"
			Standard	3' 6"	4' 7"	4' 11"	6' 2"	6' 7"	8' 4"	8' 11"	9' 8"	10' 4"	13' 1"	14' 0"
		DFL	#1 / #2	4' 5"	7' 6"	7' 9"	8' 10"	9' 3"	10' 7"	11' 0"	13' 11"	14' 0"	14' 0"	14' 0"
			#3	4' 2"	7' 1"	7' 9"	8' 9"	9' 1"	10' 5"	10' 10"	13' 9"	14' 0"	14' 0"	14' 0"
			Stud	4' 2"	7' 1"	7' 6"	8' 9"	9' 1"	10' 5"	10' 10"	13' 9"	14' 0"	14' 0"	14' 0"
16" O.C.	SPF	#1 / #2	#1	4' 2"	6' 1"	6' 5"	8' 1"	8' 8"	10' 5"	10' 10"	12' 8"	13' 7"	14' 0"	14' 0"
			#3	4' 2"	6' 1"	6' 5"	8' 1"	8' 8"	10' 5"	10' 10"	12' 8"	13' 7"	14' 0"	14' 0"
			Stud	4' 2"	6' 1"	6' 5"	8' 1"	8' 8"	10' 5"	10' 10"	12' 8"	13' 7"	14' 0"	14' 0"
		Standard	#1	4' 7"	7' 7"	7' 11"	9' 0"	9' 4"	10' 8"	11' 1"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	4' 5"	7' 6"	7' 9"	8' 10"	9' 3"	10' 7"	11' 0"	13' 11"	14' 0"	14' 0"	14' 0"
			#3	4' 4"	6' 5"	6' 10"	8' 6"	9' 1"	10' 6"	10' 11"	13' 4"	14' 0"	14' 0"	14' 0"
	SP	DFL	#1	4' 4"	6' 5"	6' 10"	8' 6"	9' 1"	10' 6"	10' 11"	13' 4"	14' 0"	14' 0"	14' 0"
			Stud	4' 4"	6' 5"	6' 10"	8' 6"	9' 1"	10' 6"	10' 11"	13' 4"	14' 0"	14' 0"	14' 0"
			Standard	4' 2"	5' 8"	6' 0"	7' 6"	8' 0"	10' 2"	10' 10"	11' 10"	12' 7"	14' 0"	14' 0"
	SPF	#1 / #2	#1	4' 10"	8' 3"	8' 7"	9' 9"	10' 2"	10' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 7"	8' 2"	8' 5"	9' 8"	10' 0"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 7"	8' 2"	8' 5"	9' 8"	10' 0"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	DFL	#1	4' 7"	7' 0"	7' 5"	9' 4"	10' 0"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	4' 7"	7' 0"	7' 5"	9' 4"	10' 0"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 7"	7' 0"	7' 5"	9' 4"	10' 0"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
12" O.C.	SPF	#1 / #2	#1	5' 1"	8' 5"	8' 8"	9' 11"	10' 3"	11' 9"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	4' 10"	8' 3"	8' 7"	9' 9"	10' 2"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 9"	7' 4"	7' 10"	9' 8"	10' 1"	11' 7"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	DFL	#1	4' 9"	7' 4"	7' 10"	9' 8"	10' 1"	11' 7"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 9"	7' 4"	7' 10"	9' 8"	10' 1"	11' 7"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			Standard	4' 7"	6' 6"	6' 11"	8' 8"	9' 3"	11' 6"	12' 0"	13' 7"	14' 0"	14' 0"	14' 0"

Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 600# at each end. Max web total length is 14'.

Vertical length shown in table above.

Connect diagonal at midpoint of vertical web.



Bracing Group Species and Grades:

Group A:			
Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard

Group B:			
Hem-Fir			
#1 & Btr			
#1			

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 75 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12' plywood overhang.

Attach 'L' braces with 10d (0.128"x3.0" min) nails.

* For (1) 'L' brace: space nails at 2' o.c.

in 18' end zones and 4' o.c. between zones.

** For (2) 'L' braces: space nails at 3' o.c.

in 18' end zones and 6' o.c. between zones.

'L' bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	2X3
Greater than 4' 0", but less than 11' 6"	3X4
Greater than 11' 6"	4X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

Yoonhwak Kim, FL PE #86367

MAX. TOT. LD. 60 PSF

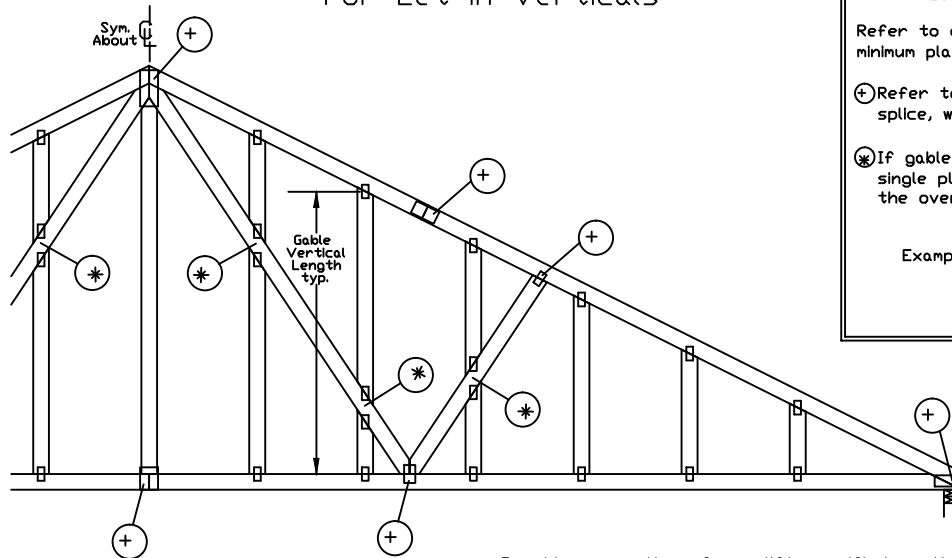
MAX. SPACING 24.0"

REF ASCE7-16-GAB16015

DATE 01/26/2018

DRWG A16015ENC160118

Gable Detail For Let-in Verticals



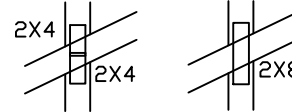
Gable Truss Plate Sizes

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

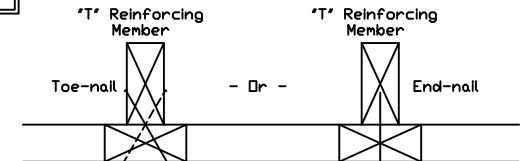
① Refer to Engineered truss design for peak, splice, web, and heel plates.

⊗ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



'T' Reinforcement Attachment Detail



To convert from 'L' to 'T' reinforcing members, multiply 'T' increase by length (based on appropriate Alpine gable detail).

Maximum allowable 'T' reinforced gable vertical length is 14' from top to bottom chord.

'T' reinforcing member material must match size, specie, and grade of the 'L' reinforcing member.

Web Length Increase w/ 'T' Brace

'T' Reinf. Mbr. Size	'T' Increase
2x4	30 %
2x6	20 %

Example:

ASCE 7-10 Wind Speed = 120 mph

Mean Roof Height = 30 ft, Kzt = 1.00

Gable Vertical = 24' o.c. SP #3

'T' Reinforcing Member Size = 2x4

'T' Brace Increase (From Above) = 30% = 1.30

(1) 2x4 'L' Brace Length = 8' 7"

Maximum 'T' Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"

Provide connections for uplift specified on the engineered truss design.

Attach each 'T' reinforcing member with

End Driven Nails:

10d Common (0.148"x 3", min) Nails at 4' o.c. plus
(4) nails in the top and bottom chords.

Toenailed Nails:

10d Common (0.148"x 3", min) Toenails at 4' o.c. plus
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

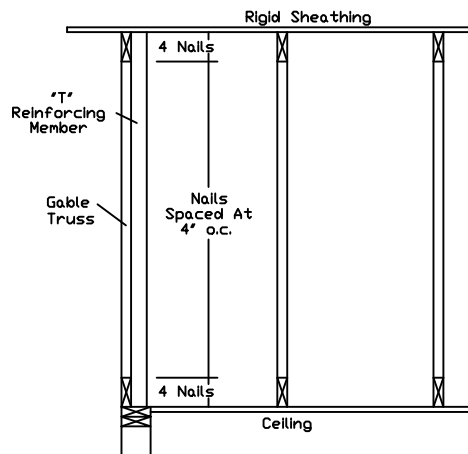
ASCE 7-05 Gable Detail Drawings

A13015051014, A12015051014, A11015051014, A10015051014, A14015051014,
A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings

A11515ENC100118, A12015ENC100118, A14015ENC100118, A10015ENC100118,
A18015ENC100118, A20015ENC100118, A20015END100118, A20015P100118,
A11530ENC100118, A12030ENC100118, A14030ENC100118, A10030ENC100118,
A18030ENC100118, A20030ENC100118, A20030END100118, A20030P100118,
S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118,
S18015ENC100118, S20015ENC100118, S20015END100118, S20015P100118,
S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118,
S18030ENC100118, S20030ENC100118, S20030END100118, S20030P100118

See appropriate Alpine gable detail for maximum unreinforced gable vertical length.



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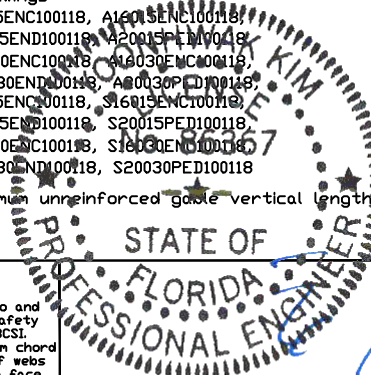
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ALPINE
AN ITW COMPANY

155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



REF LET-IN VERT

DATE 01/02/2018

DRWG GBLLETIN0118

MAX. TOT. LD. 60 PSF

DUR. FAC. ANY

MAX. SPACING 24.0"



PRELIMINARY LAYOUT

11/8/22

JOB #: 22-8515

```
Job Name: Dave Blank
Customer: OWNER BUILDER
Designer: Fill in later
ADDRESS: Little Rd
SALESMAN: HOUSE
: <Not Found>
```

JOB NO:
22-8515

PAGE NO:
1 OF 1