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ALPINE
AN ITW COMPANY
FL REG# 278, Yoonhwak Kim, FL PE #86367

6750 Forum Drive, Suite 305
Orlando, FL 32821
Phone: (800)755-6001
www.alpineitw.com

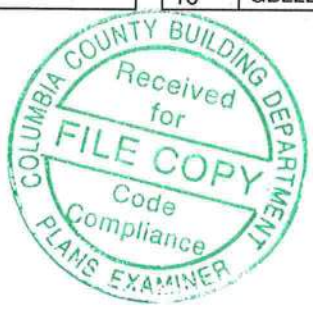
Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	*Job Number:* 20-4515
Job Description: Steedley Residence	
Address: FL	

Job Engineering Criteria:	
Design Code: FBC 2017 RES	*IntelliVIEW Version:* 20.01.01A *JRef #:* 1WYJ2150001
Wind Standard: ASCE 7-10 *Wind Speed (mph):* 130 *Building Type:* Closed	*Roof Load (psf):* 20.00-10.00- 0.00-10.00 *Floor Load (psf):* None

This package contains general notes pages, 13 truss drawing(s) and 3 detail(s).

Item	Drawing Number	Truss
1	253.20.1004.34020	A01
3	253.20.1004.37867	A03
5	253.20.1004.49150	A05
7	253.20.1004.59773	B02
9	253.20.1005.03637	B04
11	253.20.0953.13482	C02
13	253.20.1005.19807	J01
15	BRCLBSUB0119	

Item	Drawing Number	Truss
2	253.20.1004.35983	A02
4	253.20.1004.44700	A04
6	253.20.1004.58190	B01
8	253.20.1005.01493	B03
10	253.20.0953.13435	C01
12	253.20.1005.07093	HJ01
14	A14015ENC101014	
16	GBLLETIN0118	



General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:
The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:
Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:
The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:
Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:
Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:
Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

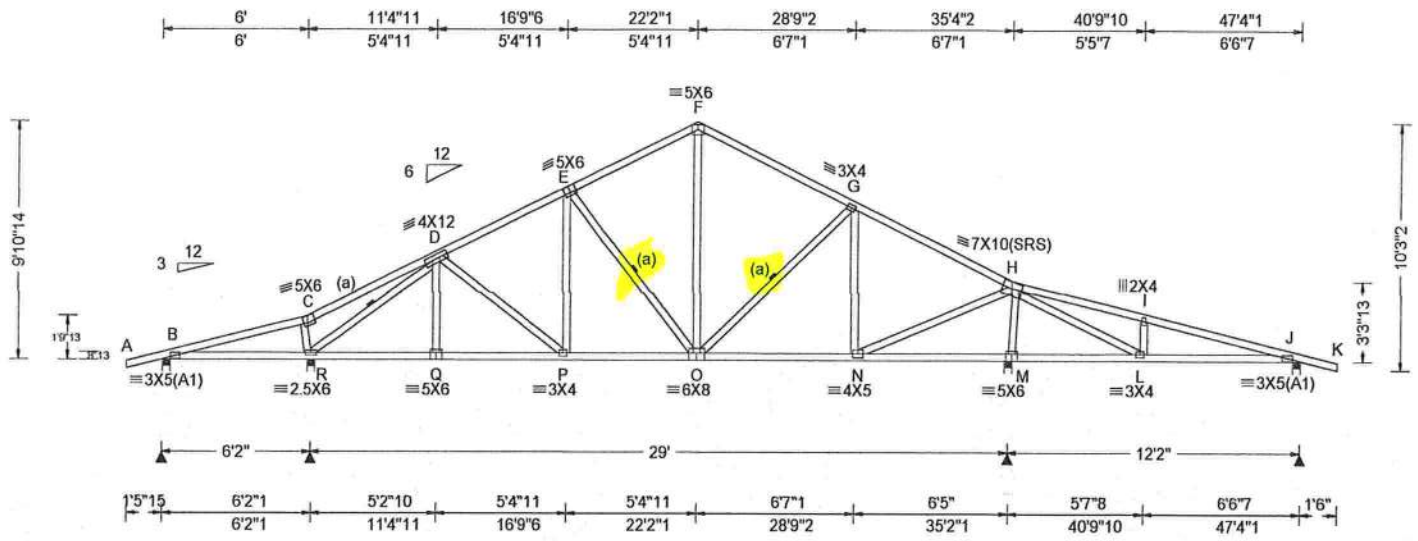
BCDL = Bottom Chord standard design Dead Load in pounds per square foot.
BCLL = Bottom Chord standard design Live Load in pounds per square foot.
CL = Certified lumber.
Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.
FRT = Fire Retardant Treated lumber.
FRT-DB = D-Blaze Fire Retardant Treated lumber.
FRT-DC = Dricon Fire Retardant Treated lumber.
FRT-FP = FirePRO Fire Retardant Treated lumber.
FRT-FL = FlamePRO Fire Retardant Treated lumber.
FRT-FT = FlameTech Fire Retardant Treated lumber.
FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.
g = green lumber.
HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.
HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.
HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.
Ic = Incised lumber.
FJ = Finger Jointed lumber.
L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.
L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.
Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.
Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.
Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.
Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for of all load cases.
NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.
PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.
PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds
PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.
PP = Panel Point.
R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).
Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).
TCDL = Top Chord standard design Dead Load in pounds per square foot.
TCLL = Top Chord standard design Live Load in pounds per square foot.
U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.
VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.
VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.
VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.
W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.
Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 514 Earth City Expressway, Suite 242, Earth City, MO 63045; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcindustry.com.

SEQN: 599981 SPEC	Ply: 1	**Job Number:** 20-4515	Cust: R 215 JRef: 1WYJ2150001 T11
FROM: CDM	Qty: 7	Steedley Residence	DrwNo: 253.20.1004.34020
		Truss Label: A01	/ YK 09/09/2020



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-10
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: > 2h
C&C Dist a: 4.73 ft
Loc. from endwall: not in 13.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 2017 RES
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.055	P	999	240
VERT(CL): 0.107	P	999	180
HORZ(LL): 0.022	N	-	-
HORZ(TL): 0.043	N	-	-

Creep Factor: 2.0
Max TC CSI: 0.504
Max BC CSI: 0.552
Max Web CSI: 0.616

VIEW Ver: 20.01.01A.0724.11

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	318	/-	/-	/142	/79	/242
R	1530	/-	/-	/874	/27	/-
M	2010	/-	/-	/1042	/-	/-
J	450	/-	/-	/233	/81	/-

Wind reactions based on MWFRS
B Brg Width = 4.0 Min Req = 1.5
R Brg Width = 4.0 Min Req = 1.5
M Brg Width = 4.0 Min Req = 2.4
J Brg Width = 4.0 Min Req = 1.5
Bearings B, R, M, & J are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
D - E	389	-1449	G - H	308	-1136
E - F	369	-1115	H - I	142	-438
F - G	380	-1140	I - J	107	-468

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
R - Q	1236	-175	N - M	209	-601
Q - P	1237	-175	M - L	205	-585
P - O	1221	-115	L - J	423	-61
O - N	959	-63			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
R - D	339	-1631	N - H	1616	-293
E - O	179	-484	M - H	491	-1902
F - O	577	-177	H - L	1030	-286
G - N	149	-472	L - I	159	-375

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Uplifts based on an elevation at or above 1000 ft.

Additional Notes

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

The overall height of this truss excluding overhang is 9-10-14.

FL REG# 278, Yoonhwak Kim, FL PE #86367
09/09/2020

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcindustry.com; ICC: www.iccsafe.org

ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
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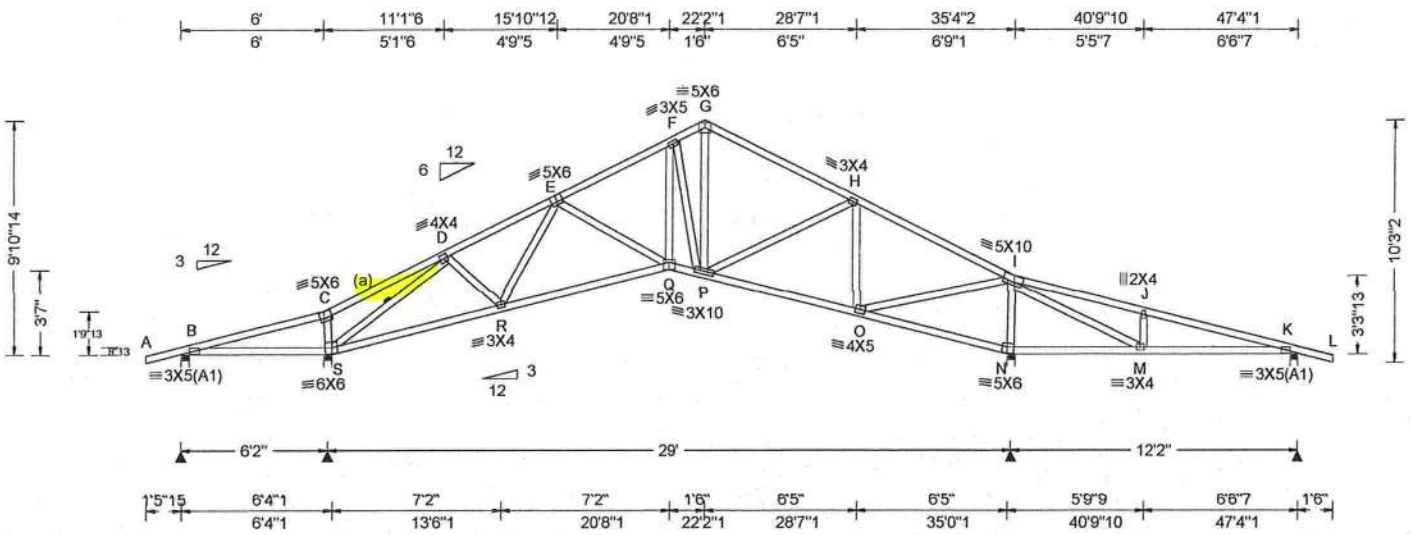
SEQN: 599977 SPEC	Ply: 1	**Job Number:** 20-4515	Cust: R 215 JRef: 1WYJ2150001 T16
FROM: CDM	Qty: 12	Steedley Residence	DrwNo: 253.20.1004.35983
		Truss Label: A02	/ YK 09/09/2020



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-10
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 4.73 ft
Loc. from endwall: not in 13.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 2017 RES
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.093	Q	999	240
VERT(CL): 0.193	Q	999	180
HORZ(LL): 0.061	N	-	-
HORZ(TL): 0.130	N	-	-

Creep Factor: 2.0
Max TC CSI: 0.549
Max BC CSI: 0.699
Max Web CSI: 0.710

VIEW Ver: 20.01.01A.0724.11

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	268	/-	/-	/72	/93	/242
S	1544	/-	/-	/945	/22	/-
N	2027	/-	/-	/1102	/30	/-
K	412	/-	/-	/195	/85	/-

Wind reactions based on MWFRS
B Brg Width = 4.0 Min Req = 1.5
S Brg Width = 4.0 Min Req = 1.8
N Brg Width = 4.0 Min Req = 2.4
K Brg Width = 4.0 Min Req = 1.5
Bearings B, S, N, & K are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	484	-96	F - G	353	-1180
C - D	595	-80	G - H	330	-1275
D - E	416	-1667	H - I	265	-994
E - F	340	-1516			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - S	121	-437	P - O	883	-17
S - R	1260	-188	O - N	319	-1169
R - Q	1561	-134	N - M	247	-935
Q - P	1313	0			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
S - D	443	-2215	O - I	1863	-319
Q - F	875	-86	N - I	431	-1640
F - P	215	-920	I - M	1044	-286
P - G	785	-237	M - J	158	-376
H - O	171	-725			

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Wind

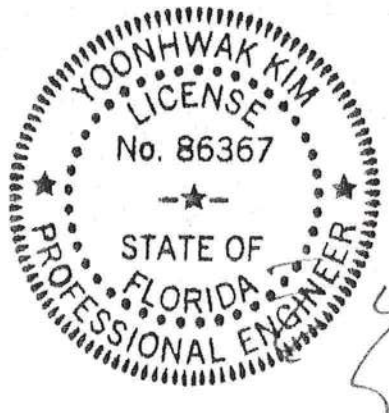
Wind loads based on MWFRS with additional C&C member design.

Uplifts based on an elevation at or above 1000 ft.

Additional Notes

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

The overall height of this truss excluding overhang is 9-10-14.



FL REG# 278, Yoonhwak Kim, FL PE #86367
09/09/2020

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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcindustry.com; ICC: www.iccsafe.org

ALPINE
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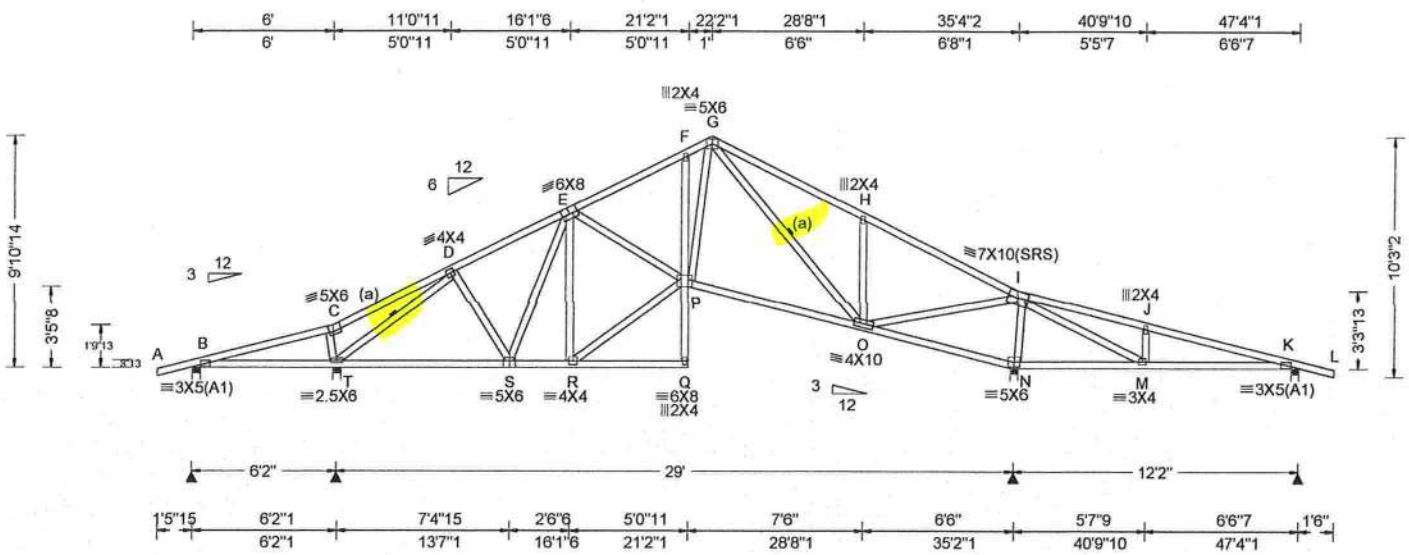
SEQN: 600000 SPEC	Ply: 1	**Job Number:** 20-4515	Cust: R 215 JRef: 1WYJ2150001 T10
FROM: CDM	Qty: 2	Steedley Residence	DrwNo: 253.20.1004.37867
		Truss Label: A03	/ YK 09/09/2020



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-10
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 4.73 ft
Loc. from endwall: not in 13.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 2017 RES
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.075	F	999	240
VERT(CL): 0.156	F	999	180
HORZ(LL): 0.047	N	-	-
HORZ(TL): 0.100	N	-	-

Creep Factor: 2.0
Max TC CSI: 0.505
Max BC CSI: 0.726
Max Web CSI: 0.698

VIEW Ver: 20.01.01A.0724.11

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	292	/-	/-	/104	/90	/265
T	1478	/-	/-	/902	/22	/-
N	2001	/-	/-	/1090	/35	/-
K	423	/-	/-	/199	/85	/-

Wind reactions based on MWFRS
B Brg Width = 4.0 Min Req = 1.5
T Brg Width = 4.0 Min Req = 1.5
N Brg Width = 4.0 Min Req = 2.3
K Brg Width = 4.0 Min Req = 1.5
Bearings B, T, N, & K are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
D - E	398	- 1234	G - H	410	- 1164
E - F	354	- 1595	H - I	282	- 1156
F - G	417	- 1505			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
T - S	992	- 145	O - N	306	- 980
S - R	1040	- 87	N - M	259	- 843
P - O	1204	0			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
T - D	336	- 1572	O - H	224	- 443
E - R	71	- 699	O - I	1833	- 326
E - P	395	0	N - I	437	- 1659
R - P	1280	- 106	I - M	1038	- 287
P - G	1116	- 227	M - J	160	- 376
G - O	143	- 407			

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Wind

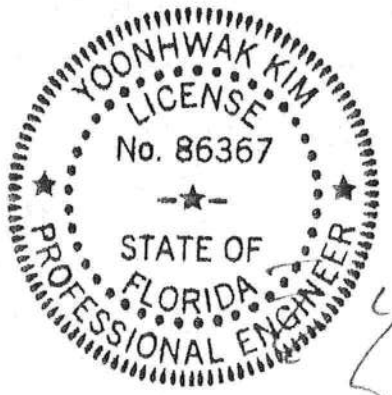
Wind loads based on MWFRS with additional C&C member design.

Uplifts based on an elevation at or above 1000 ft.

Additional Notes

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

The overall height of this truss excluding overhang is 9-10-14.



FL REG# 278, Yoonhwak Kim, FL PE #86367
09/09/2020

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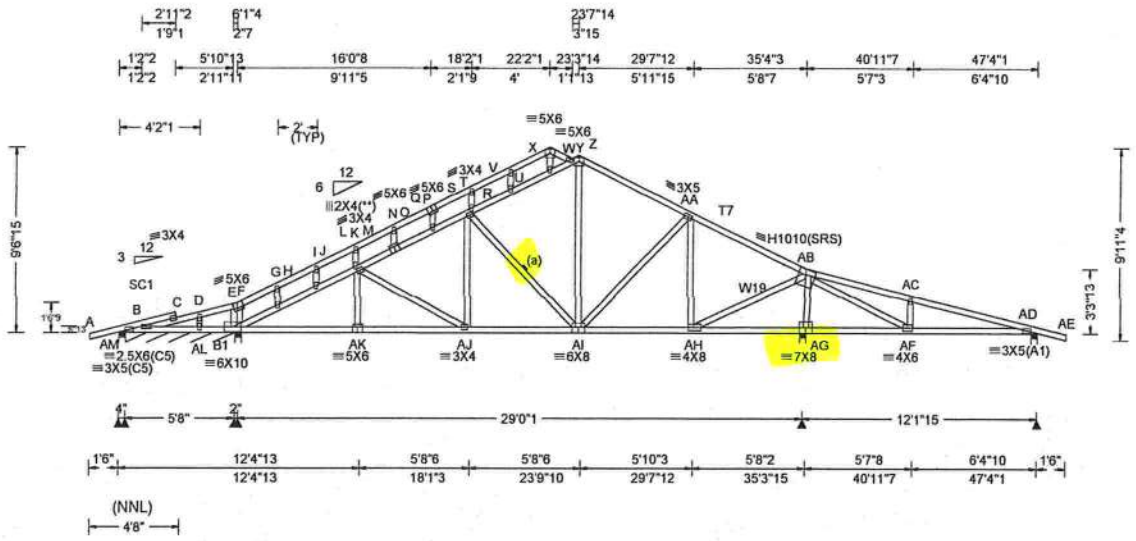
Orlando FL, 32821

SEQN: 600023 GABL FROM: CDM Page 1 of 2	Ply: 1 Qty: 1	**Job Number:** 20-4515 Steedley Residence **Truss Label:** A04		Cust: R 215 JRef: 1WYJ2150001 T14 DrwNo: 253.20.1004.44700 / YK 09/09/2020



Loading Criteria (psf)		**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**			
TCLL:	20.00	Wind Std: ASCE 7-10	Pg: NA Ct: NA CAT: NA	PP Deflection in	loc	L/defl	L/#
TCDL:	10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.167	Q	999	240
BCLL:	0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.317	Q	999	180
BCDL:	10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.068	P	-	-
Des Ld:	40.00	EXP: C Kzt: NA		HORZ(TL): 0.128	P	-	-
NCBCLL:	10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0			
Soffit:	2.00	TCDL: 5.0 psf	FBC 2017 RES	Max TC CSI: 0.881			
Load Duration:	1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.907			
Spacing:	24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Varies by Ld Case	Max Web CSI: 0.939			
		C&C Dist a: 4.73 ft	FT/RT:20(0)/10(0)				
		Loc. from endwall: not in 17.00 ft	Plate Type(s):				
		GCpi: 0.18	WAVE, HS				
		Wind Duration: 1.60		VIEW Ver: 20.01.01A.0724.11			

▲ Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
AM	669	/-	/-	/323	/65	/487
AM*	373	/-	/-	/140	/70	/-
E	502	/-	/-	/415	/45	/-
AG	3702	/-	/-	/1482	/947	/-
AD	744	/-	/-	/347	/261	/-

Wind reactions based on MWFRS
AM Brg Width = 4.0 Min Req = 1.5
AM Brg Width = 68.0 Min Req = -
E Brg Width = 4.0 Min Req = 1.5
AG Brg Width = 4.0 Min Req = 3.7
AD Brg Width = 4.0 Min Req = 1.5
Bearings AM, AM, E, AG, & AD are a rigid surface.

Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	153	-826	P - S	300	-855
C - D	0	-753	Q - R	734	-1873
D - F	0	-768	R - T	534	-1260
E - H	1122	-2795	S - U	395	-874
F - G	67	-1136	T - V	490	-1106
G - I	91	-1019	U - W	450	-794
H - J	1113	-2788	V - X	468	-1049
I - K	130	-945	W - Y	479	-777
J - L	1082	-2735	X - Y	529	-1170
K - N	220	-1008	Y - Z	734	-1625
L - M	996	-2543	Z -AA	806	-2070
M - O	773	-1984	AA-AB	613	-1710
N - P	258	-929	AB-AC	236	-439
O - Q	754	-1932	AC-AD	165	-508

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B -AM	1108	-276	AJ-AI	2467	-383
B -AL	751	-205	AI-AH	1420	-146
AL- E	738	-210	AH-AG	512	-1250
E -AK	6722	-1433	AG-AF	504	-1222
AK-AJ	3361	-717	AF-AD	422	-92

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
E - J	275	-854	AI-AA	381	-185
K - L	254	-568	AA-AH	356	-1339

Lumber

Top chord: 2x4 SP #2; T7 2x4 SP M-31;
Bot chord: 2x4 SP #2; B1 2x4 SP M-31;
Webs: 2x4 SP #3; W19 2x4 SP #2;
Stack Chord: SC1 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading

Truss designed to support 2-0-0 top chord outlookers and cladding load not to exceed 2.30 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C member design.

Uplifts based on an elevation at or above 1000 ft.

Additional Notes

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

The overall height of this truss excluding overhangs is 9-6-15.

YOONHWAK KIM LICENSE No. 86367 STATE OF FLORIDA PROFESSIONAL ENGINEER

FL REG# 278, Yoonhwak Kim, FL PE #86367
09/09/2020

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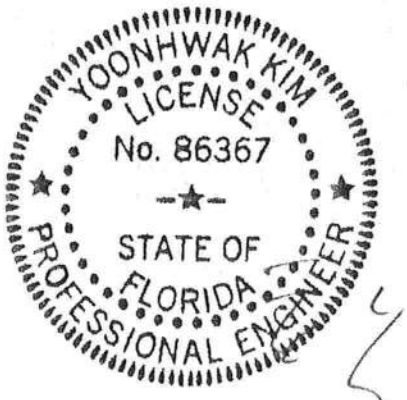
M -AJ	435	-1010	AH-AB	2962	-710
AJ- R	581	-206	AG-AB	1085	-3638
R -AI	440	-1188	AB-AF	1791	-624
S - T	225	-458	AF-AC	324	-796
Z -AI	685	-367			



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09/09/2020

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6750 Forum Drive
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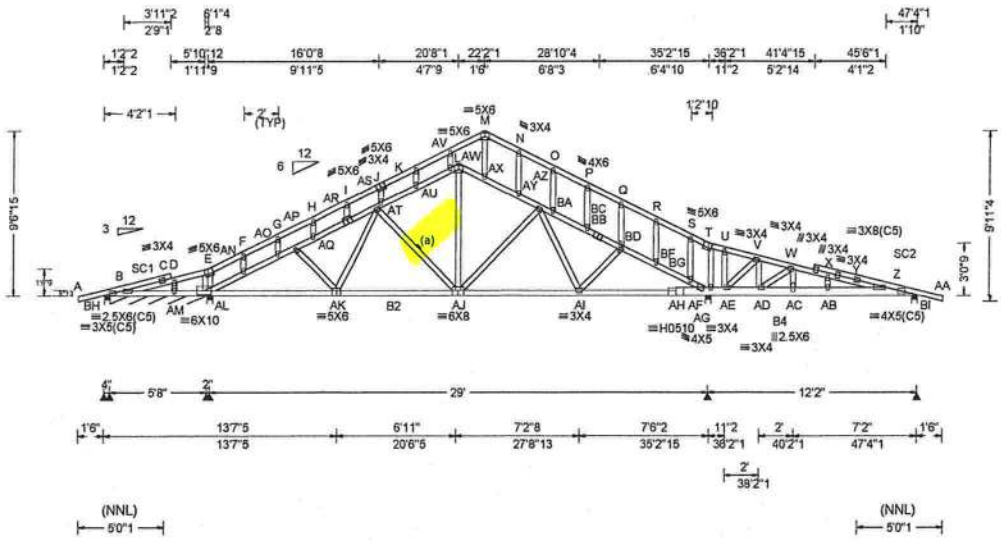
Orlando FL, 32821

SEQN: 600048 GABL FROM: CDM Page 1 of 2	Ply: 1 Qty: 1	**Job Number:** 20-4515 Steedley Residence **Truss Label:** A05		Cust: R 215 JRef: 1WYJ2150001 T8 DrwNo: 253.20.1004.49150 / YK 09/09/2020



Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL:	20.00	Wind Std: ASCE 7-10	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL:	10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.202 K 999 240
BCLL:	0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.384 K 906 180
BCDL:	10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.105 S - -
Des Ld:	40.00	EXP: C Kzt: NA		HORZ(TL): 0.199 S - -
NCBCLL:	10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit:	2.00	TCDL: 5.0 psf	FBC 2017 RES	Max TC CSI: 0.968
Load Duration:	1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.893
Spacing:	24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Varies by Ld Case	Max Web CSI: 0.728
		C&C Dist a: 4.73 ft	FT/RT:20(0)/10(0)	
		Loc. from endwall: not in 8.50 ft	Plate Type(s):	
		GCpi: 0.18	WAVE, HS	VIEW Ver: 20.01.01A.0724.11
		Wind Duration: 1.60		

▲ Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BH	667	/-	/-	/-	/95	/-
BH*	402	/-	/-	/-	/64	/-
AL	629	/-	/-	/105	/-	/-
AG	3171	/-	/0	/-	/321	/0
BI	1272	/-	/-	/-	/169	/-

Wind reactions based on MWFRS
BH Brg Width = 4.0 Min Req = 1.5
BH Brg Width = 68.0 Min Req = -
AL Brg Width = 4.0 Min Req = 1.5
AG Brg Width = 4.0 Min Req = 2.3
BI Brg Width = 4.0 Min Req = 1.5
Bearings BH, BH, AL, AG, & BI are a rigid surface.

Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	95	-758	N - O	84	-741
C - D	78	-668	O - P	81	-731
D - E	84	-703	P - Q	95	-805
E - F	135	-1045	Q - R	86	-779
F - G	114	-920	R - S	98	-827
G - H	113	-917	S - T	85	-728
H - I	109	-888	T - U	71	-667
I - J	96	-817	U - V	80	-706
J - K	102	-852	V - W	190	-1438
K - L	81	-739	W - X	303	-2491
L - M	83	-722	X - Y	301	-2466
M - N	80	-715	Y - Z	312	-2554

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B -BH	837	-23	AH-AG	3425	-361
B -AM	687	-81	AG-AF	1340	-149
AM-AL	674	-79	AF-AE	654	-71
AL-AK	7519	-795	AE-AD	1272	-168
AK-AJ	3102	-316	AD-AC	2340	-286
AJ-AI	2890	-293	AC-AB	2394	-292
AI-AH	3425	-361	AB- Z	2408	-293

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
AL- E	125	-799	AX-AY	180	-1906
AL-AN	336	-3310	AY-AZ	198	-2017

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP M-31; B2 2x4 SP #2;
B4 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Special Loads

------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	61 plf at	-1.50 to	61 plf at	6.10
TC: From	62 plf at	6.10 to	62 plf at	35.24
TC: From	61 plf at	35.24 to	61 plf at	37.61
TC: From	30 plf at	37.61 to	30 plf at	45.51
TC: From	61 plf at	45.51 to	61 plf at	48.84
BC: From	4 plf at	-1.50 to	4 plf at	0.00
BC: From	20 plf at	0.00 to	20 plf at	37.61
BC: From	10 plf at	37.61 to	10 plf at	47.34
BC: From	4 plf at	47.34 to	4 plf at	48.84

BC: 24 lb Conc. Load at 37.61,39.61,41.61,43.61
BC: 62 lb Conc. Load at 45.64

Plating Notes

All plates are 2X4 except as noted.

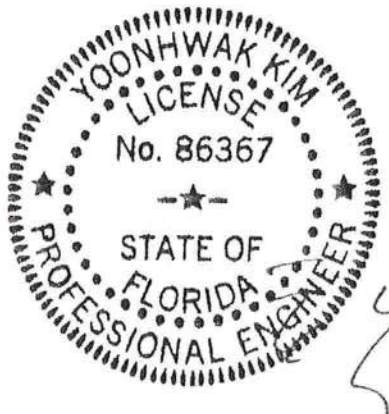
Loading

Truss designed to support 2-0-0 top chord outlookers and cladding load not to exceed 2.30 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads and reactions based on MWFRS.

Uplifts based on an elevation at or above 1000 ft.



FL REG# 278, Yoonhwak Kim, FL PE #86367
09/09/2020

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6750 Forum Drive
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SEQN: 600048 GABL	Ply: 1	**Job Number:** 20-4515	Cust: R 215 JRef: 1WYJ2150001 T8	
FROM: CDM	Qty: 1	Steedley Residence	DrwNo: 253.20.1004.49150	
Page 2 of 2		**Truss Label:** A05	/ YK 09/09/2020	

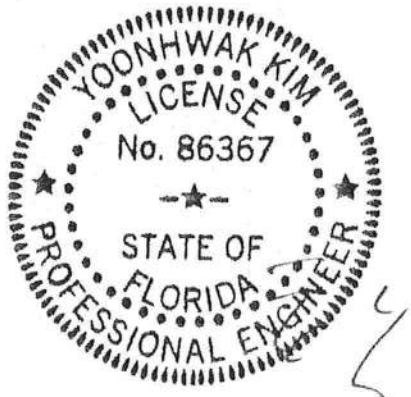
Additional Notes

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

The overall height of this truss excluding overhang is 9-6-15.

AN-AO	334	-3310	AZ-BA	221	-2565
AO-AP	312	-3187	AZ-AI	399	0
AP-AQ	280	-3013	BA-BB	247	-2710
AP-AK	116	-631	AI-BD	104	-534
AQ-AR	262	-2913	BB-BC	256	-2769
AK-AS	552	-9	BC-BD	256	-2771
AR-AS	252	-2853	BD-BF	318	-3081
AS-AT	207	-2095	BF-BG	329	-3162
AS-AJ	130	-1107	BG- S	61	-406
J -AT	84	-499	BG-AG	359	-3348
AT-AU	179	-1927	AE- V	146	-935
AU-AV	177	-1905	V -AD	636	-102
AV-AW	153	-1786	AD- W	137	-1285
AJ-AZ	96	-800	W -AC	722	-82
AW-AJ	1477	-97	AB- X	35	-378
AW-AX	185	-1990			



FL REG# 278, Yoonhwak Kim, FL PE #86367
09/09/2020

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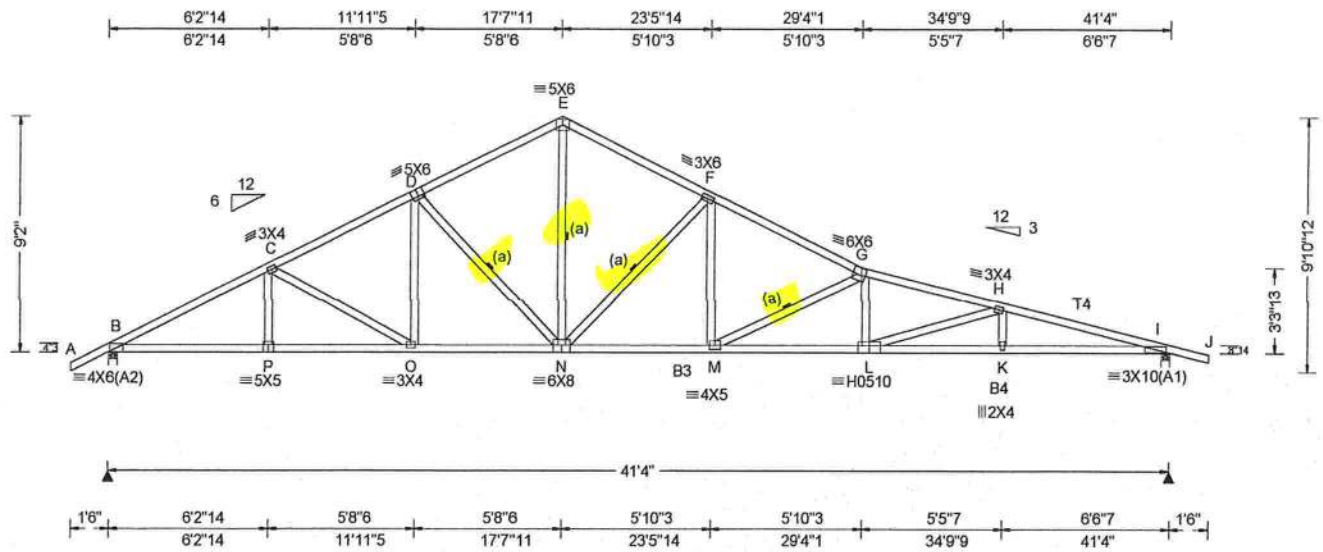
SEQN: 599957 MONO FROM: CDM	Ply: 1 Qty: 10	**Job Number:** 20-4515 Steedley Residence **Truss Label:** B01	Cust: R 215 JRef: 1WYJ2150001 T13 DrwNo: 253.20.1004.58190 / YK 09/09/2020	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-10
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 4.13 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 2017 RES
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE, HS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.388	G	999	240
VERT(CL): 0.784	G	628	180
HORZ(LL): 0.110	E	-	-
HORZ(TL): 0.223	E	-	-

Creep Factor: 2.0
Max TC CSI: 0.503
Max BC CSI: 0.832
Max Web CSI: 0.600

VIEW Ver: 20.01.01A.0724.11

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1799	/-	/-	/1063	/318	/256
I	1781	/-	/-	/957	/327	/-

Wind reactions based on MWFRS
B Brg Width = 4.0 Min Req = 2.1
I Brg Width = 4.0 Min Req = 1.5
Bearings B & I are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	1431	-3198	F - G	1694	-3320
C - D	1341	-2742	G - H	2435	-4923
D - E	1209	-2228	H - I	2654	-5585
E - F	1252	-2232			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - P	2783	-1129	M - L	4710	-2195
P - O	2781	-1130	L - K	5371	-2511
O - N	2370	-926	K - I	5376	-2508
N - M	2860	-1191			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - O	263	-460	F - M	1078	-500
O - D	409	-117	M - G	1115	-2046
D - N	395	-658	G - L	391	-113
E - N	1574	-825	L - H	374	-663
N - F	769	-1350			

Lumber

Top chord: 2x4 SP #2; T4 2x4 SP M-31;
Bot chord: 2x4 SP #2; B3,B4 2x4 SP M-31;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

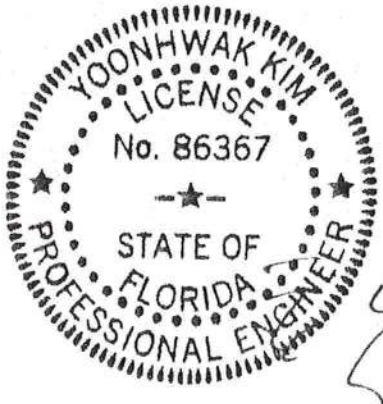
Wind

Wind loads based on MWFRS with additional C&C member design.

Uplifts based on an elevation at or above 1000 ft.

Additional Notes

The overall height of this truss excluding overhang is 9-2-0.



FL REG# 278, Yoonhwak Kim, FL PE #86367
09/09/2020

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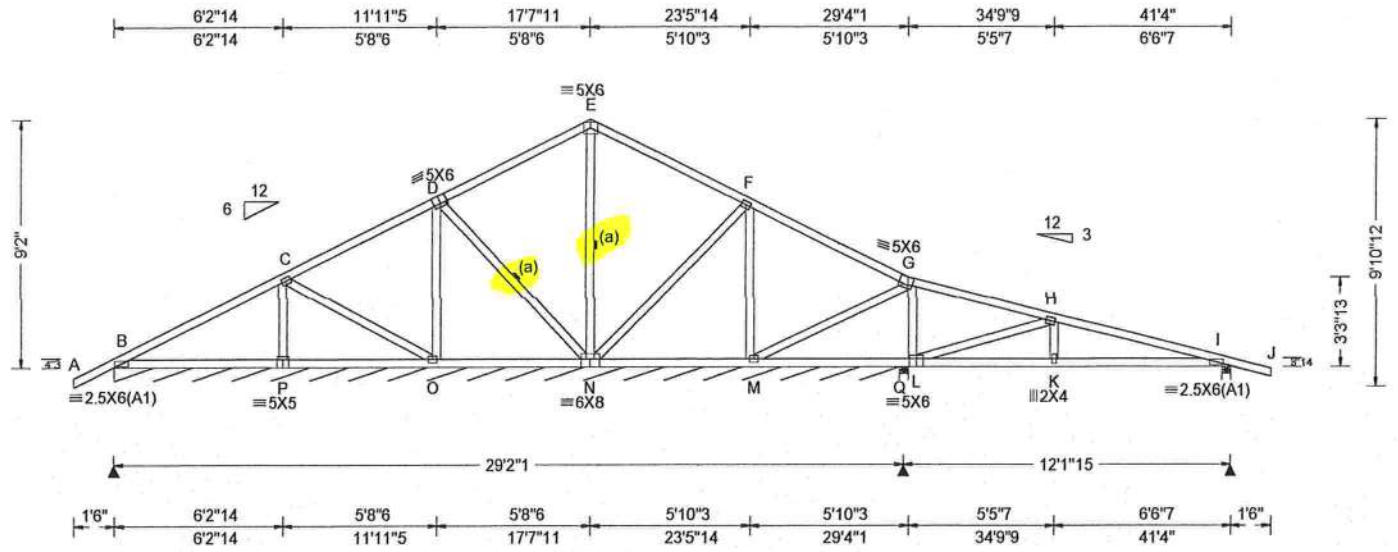
ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 599963 MONO FROM: CDM	Ply: 1 Qty: 1	**Job Number:** 20-4515 Steedley Residence **Truss Label:** B02	Cust: R 215 JRef: 1WYJ2150001 T3 DrwNo: 253.20.1004.59773 / YK 09/09/2020



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-10
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 4.13 ft
Loc. from endwall: not in 13.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 2017 RES
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.032	K	999	240
VERT(CL): 0.060	K	999	180
HORZ(LL): 0.005	F	-	-
HORZ(TL): 0.011	F	-	-

Creep Factor: 2.0
Max TC CSI: 0.682
Max BC CSI: 0.456
Max Web CSI: 0.580

VIEW Ver: 20.01.01A.0724.11

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B*	88	/-	/-	/49	/2	/9
Q	507	/-	/-	/304	/8	/-
I	533	/-	/-	/282	/90	/-

Wind reactions based on MWFRS
B Brg Width = 348 Min Req = -
Q Brg Width = 4.0 Min Req = 1.5
I Brg Width = 4.0 Min Req = 1.5
Bearings B, Q, & I are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
F - G	482	-88	H - I	202	-794

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
M - L	198	-455	K - I	737	-152
L - K	726	-153			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
E - N	77	-469	L - H	269	-975
F - M	183	-646			

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 3X4 except as noted.

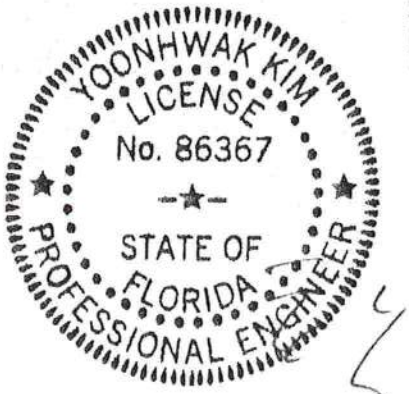
Wind

Wind loads based on MWFRS with additional C&C member design.

Uplifts based on an elevation at or above 1000 ft.

Additional Notes

The overall height of this truss excluding overhang is 9-2-0.



FL REG# 278, Yoonhwak Kim, FL PE #86367
09/09/2020

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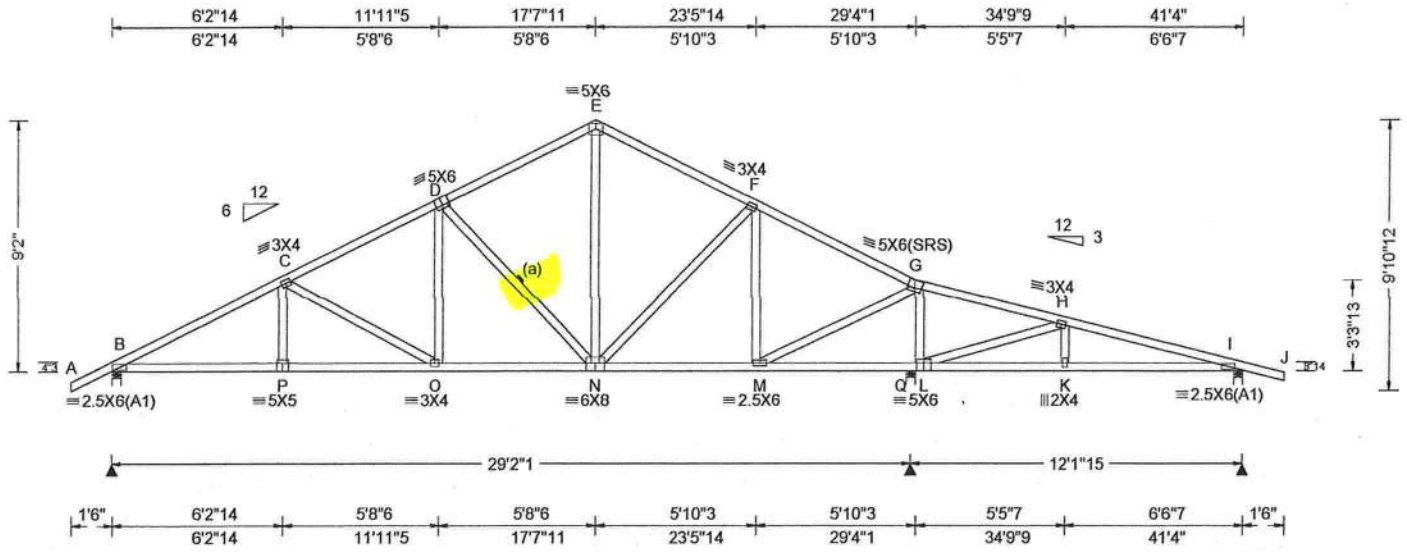
ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 599960 MONO FROM: CDM	Ply: 1 Qty: 3	**Job Number:** 20-4515 Steedley Residence **Truss Label:** B03	Cust: R 215 JRef: 1WYJ2150001 T5 DrwNo: 253.20.1005.01493 / YK 09/09/2020	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00
Load Duration:	1.25
Spacing:	24.0 "

Wind Criteria
Wind Std: ASCE 7-10
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 4.13 ft
Loc. from endwall: not in 13.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 2017 RES
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.092	O	999	240
VERT(CL): 0.181	O	999	180
HORZ(LL): 0.031	K	-	-
HORZ(TL): 0.064	K	-	-

Creep Factor: 2.0
Max TC CSI: 0.519
Max BC CSI: 0.896
Max Web CSI: 0.638

VIEW Ver: 20.01.01A.0724.11

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1288	/-	/-	/804	/35	/256
Q	1798	/-	/-	/964	/36	/-
I	514	/-	/-	/275	/89	/-

Wind reactions based on MWFRS
B Brg Width = 4.0 Min Req = 1.5
Q Brg Width = 4.0 Min Req = 1.7
I Brg Width = 4.0 Min Req = 1.5
Bearings B, Q, & I are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	466	-2106	F - G	312	-1008
C - D	425	-1620	G - H	403	-56
D - E	368	-1101	H - I	197	-743
E - F	390	-1106			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - P	1811	-312	M - L	201	-480
P - O	1808	-313	L - K	675	-146
O - N	1365	-189	K - I	688	-147
N - M	855	-78			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - O	144	-500	F - M	139	-533
O - D	414	-54	M - G	1129	-189
D - N	217	-663	G - L	372	-1342
E - N	571	-186	L - H	296	-1071

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

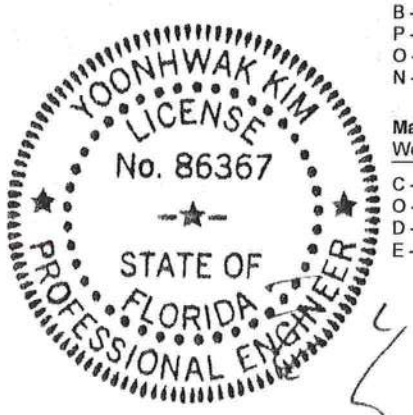
Wind

Wind loads based on MWFRS with additional C&C member design.

Uplifts based on an elevation at or above 1000 ft.

Additional Notes

The overall height of this truss excluding overhang is 9-2-0.



FL REG# 278, Yoonhwak Kim, FL PE #86367
09/09/2020

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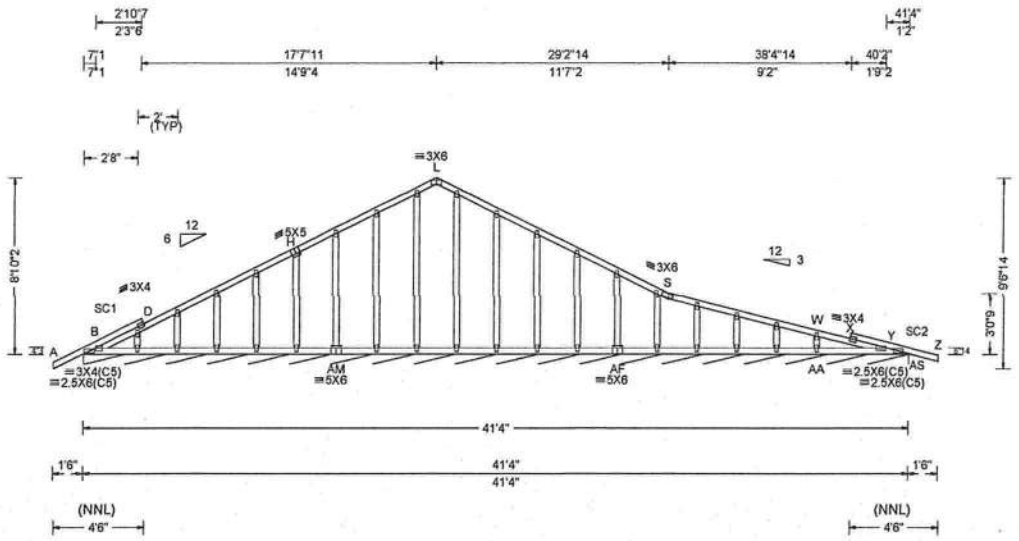
ALPINE AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 599994 GABL FROM: CDM	Ply: 1 Qty: 1	**Job Number:** 20-4515 Steedley Residence **Truss Label:** B04		Cust: R 215 JRef: 1WYJ2150001 T1 DrwNo: 253.20.1005.03637 / YK 09/09/2020



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-10
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 4.13 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 2017 RES
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.018	X	999	240
VERT(CL): 0.034	X	999	180
HORZ(LL): 0.008	N	-	-
HORZ(TL): 0.013	N	-	-

Creep Factor: 2.0
Max TC CSI: 0.563
Max BC CSI: 0.207
Max Web CSI: 0.324

VIEW Ver: 20.01.01A.0724.11

▲ Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
AS*	163	/-	/-	/78	/-	/7

Wind reactions based on MWFRS
AS Brg Width = 496 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
D - H	389	-107	W - X	57	-516
H - L	405	-90	X - Y	227	-709
S - W	57	-479			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
AM-AF	710	-71	AA- Y	681	-61
AF-AA	695	-66			

Maximum Gable Forces Per Ply (lbs)

Gables	Tens.	Comp.
AA- W	186	-426

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

Loading

Truss designed to support 2-0-0 top chord outlookers and cladding load not to exceed 2.30 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Purlins

In lieu of structural panels use purlins to brace TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

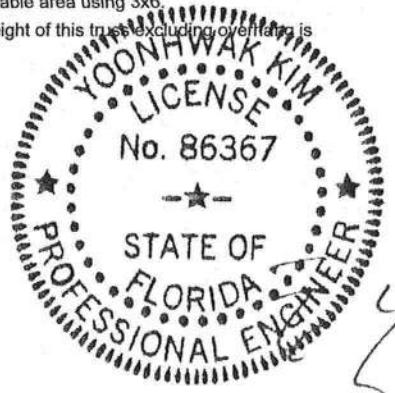
Uplifts based on an elevation at or above 1000 ft.

Additional Notes

See DWGS A14015ENC101014 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhangs is 8-10-2.



FL REG# 278, Yoonhwak Kim, FL PE #86367
09/09/2020

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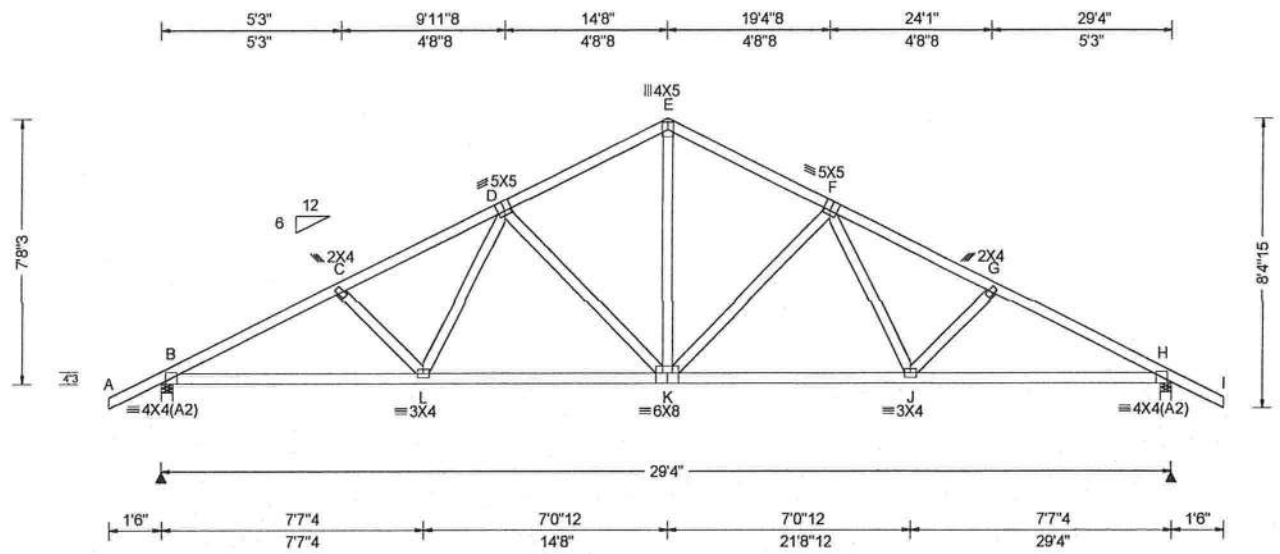
ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 599985 / COMN FROM: CDM	Ply: 1 Qty: 5	**Job Number:** 20-4515 Steedley Residence **Truss Label:** C01	Cust: R 215 JRef: 1WYJ2150001 T9 / DrwNo: 253.20.0953.13435 / YK 09/09/2020



Loading Criteria (psf)
TCLL: 20.00
TCDL: 10.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 40.00
NCBCLL: 10.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-10
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 2017 RES
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.097	K	999	240
VERT(CL): 0.196	K	999	180
HORZ(LL): 0.040	J	-	-
HORZ(TL): 0.080	J	-	-

Creep Factor: 2.0
Max TC CSI: 0.295
Max BC CSI: 0.728
Max Web CSI: 0.472

VIEW Ver: 20.01.01A.0724.11

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1308	/-	/-	/782	/234	/226
H	1308	/-	/-	/782	/234	/-

Wind reactions based on MWFRS
B Brg Width = 4.0 Min Req = 1.5
H Brg Width = 4.0 Min Req = 1.5
Bearings B & H are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	903	-2167	E - F	734	-1399
C - D	876	-1964	F - G	876	-1964
D - E	734	-1399	G - H	903	-2167

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - L	1876	-670	K - J	1550	-513
L - K	1550	-499	J - H	1876	-684

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
L - D	405	-114	K - F	316	-518
D - K	316	-518	F - J	405	-114
E - K	896	-407			

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Uplifts based on an elevation at or above 1000 ft.

Additional Notes

The overall height of this truss excluding overhang is 7-8-3.

YOONHWAK KIM LICENSE No. 86367 STATE OF FLORIDA PROFESSIONAL ENGINEER

FL REG# 278, Yoonhwak Kim, FL PE #86367
09/09/2020

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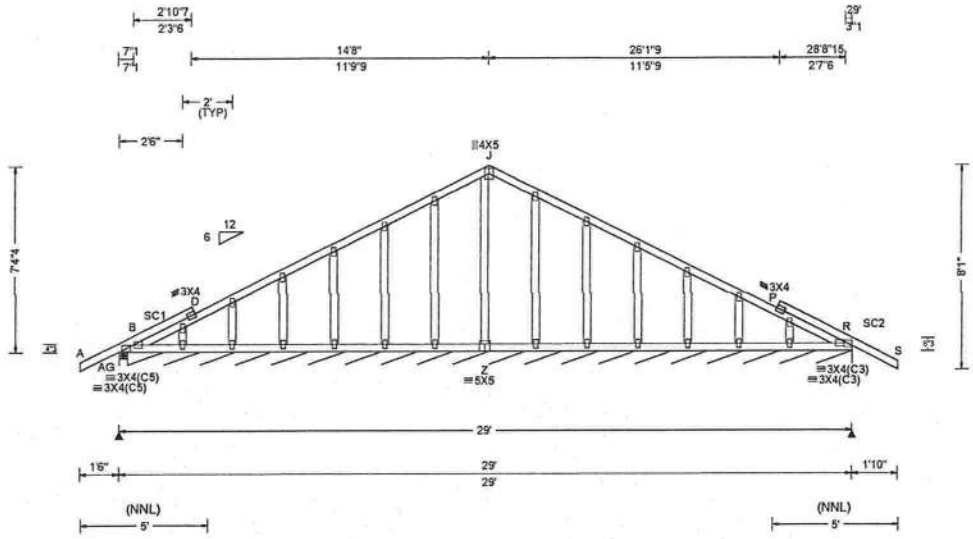
ALPINE AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 599987 / GABL FROM: CDM	Ply: 1 Qty: 1	**Job Number:** 20-4515 Steedley Residence **Truss Label:** C02	Cust: R 215 JRef: 1WYJ2150001 T19 / DrwNo: 253.20.0953.13482 / YK 09/09/2020



Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL:	20.00	Wind Std: ASCE 7-10	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL:	10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.004 T 804 240
BCLL:	0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.008 T 421 180
BCDL:	10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.001 D - -
Des Ld:	40.00	EXP: C Kzt: NA		HORZ(TL): 0.002 D - -
NCBCLL:	10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit:	2.00	TCDL: 5.0 psf	FBC 2017 RES	Max TC CSI: 0.702
Load Duration:	1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.227
Spacing:	24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Varies by Ld Case	Max Web CSI: 0.107
		C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
		Loc. from endwall: Any	Plate Type(s):	
		GCpi: 0.18	WAVE	VIEW Ver: 20.01.01A.0724.11
		Wind Duration: 1.60		

▲ Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
AG	523	/-	/-	/341	/82	/90
R*	154	/-	/-	/76	/-	/-

Wind reactions based on MWFRS
AG Brg Width = 4.0 Min Req = 1.5
R Brg Width = 343 Min Req = -
Bearings AG & B are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
D - J	408	-70	P - R	455	-136
P - R	142	-494			

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

Loading

Truss designed to support 2-0-0 top chord outlookers and cladding load not to exceed 2.30 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C member design.

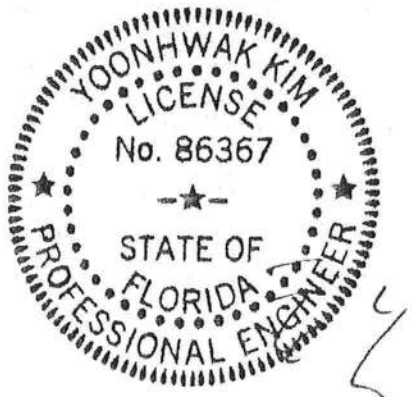
Uplifts based on an elevation at or above 1000 ft.

Additional Notes

See DWGS A14015ENC101014 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is 7-4-4.



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09/09/2020

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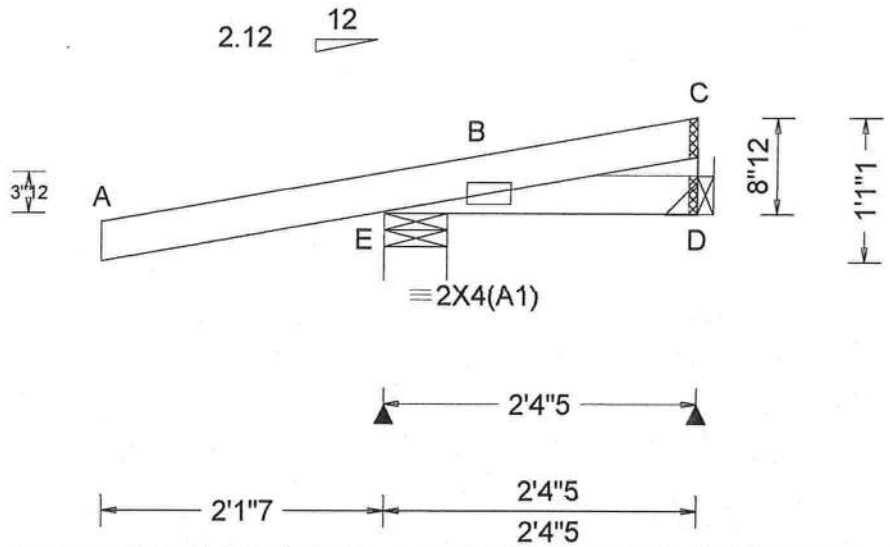
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 600045 HIP_ FROM: CDM	Ply: 1 Qty: 1	**Job Number:** 20-4515 Steedley Residence **Truss Label:** HJ01		Cust: R 215 JRef: 1WYJ2150001 T12 DrwNo: 253.20.1005.07093 / YK 09/09/2020



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-10
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 2017 RES
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.001	C	999	240
VERT(CL): 0.006	C	999	180
HORZ(LL): -0.000	C	-	-
HORZ(TL): 0.001	C	-	-

Creep Factor: 2.0
Max TC CSI: 0.129
Max BC CSI: 0.042
Max Web CSI: 0.000

VIEW Ver: 20.01.01A.0724.11

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
E	156	/-	/-	/-	/35	/-
D	38	/-22	/-	/13	/-	/-

Wind reactions based on MWFRS
E Brg Width = 5.7 Min Req = 1.5
D Brg Width = - Min Req = -
Bearing E is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Hangers / Ties

(J) Hanger Support Required, by others

Wind

Wind loads and reactions based on MWFRS.

Uplifts based on an elevation at or above 1000 ft.

Additional Notes

The overall height of this truss excluding overhang is 0-8-12.

FL REG# 278, Yoonhwak Kim, FL PE #86367
09/09/2020

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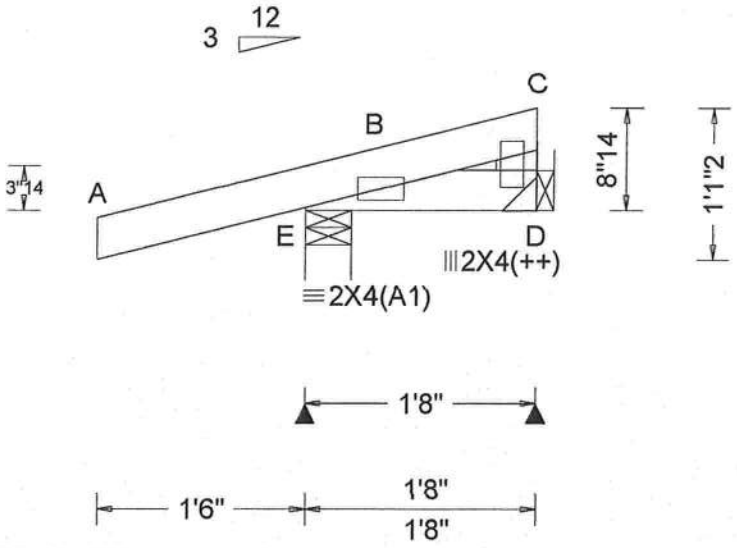
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 600043 EJAC FROM: CDM	Ply: 1 Qty: 5	**Job Number:** 20-4515 Steedley Residence **Truss Label:** J01		Cust: R 215 JRef: 1WYJ2150001 T6 DrwNo: 253.20.1005.19807 / YK 09/09/2020



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-10
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 2017 RES
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): NA			
VERT(CL): NA			
HORZ(LL): 0.000	D	-	-
HORZ(TL): 0.000	D	-	-

Creep Factor: 2.0
Max TC CSI: 0.251
Max BC CSI: 0.041
Max Web CSI: 0.005

VIEW Ver: 20.01.01A.0724.11

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
E	228	/-	/-	/134	/98	/28
D	24	/-	/-	/42	/21	/-

Wind reactions based on MWFRS
E Brg Width = 4.0 Min Req = 1.5
D Brg Width = - Min Req = -
Bearing E is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

(++) - This plate works for both joints covered.

Hangers / Ties

(J) Hanger Support Required, by others

Wind

Wind loads based on MWFRS with additional C&C member design.

Uplifts based on an elevation at or above 1000 ft.

Additional Notes

The overall height of this truss excluding overhang is 0-8-14.

FL REG# 278, Yoonhwak Kim, FL PE #86367
09/09/2020

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Gable Stud Reinforcement Detail

ASCE 7-10: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Or: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00
Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 "L" Brace * Group A	(1) 1x4 "L" Brace * Group B	(1) 2x4 "L" Brace * Group A	(1) 2x4 "L" Brace * Group B	(2) 2x4 "L" Brace ** Group A	(2) 2x4 "L" Brace ** Group B	(1) 2x6 "L" Brace * Group A	(1) 2x6 "L" Brace * Group B	(2) 2x6 "L" Brace ** Group A	(2) 2x6 "L" Brace ** Group B
24" o.c.	SPF HF	#1 / #2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"
		#3	4' 1"	6' 7"	7' 1"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
		Stud	4' 1"	6' 7"	7' 0"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
		Standard	4' 1"	5' 8"	6' 0"	7' 7"	8' 1"	10' 1"	10' 6"	11' 10"	12' 8"	14' 0"	14' 0"
	SP DFL	#1	4' 6"	7' 4"	7' 8"	8' 8"	9' 0"	10' 4"	10' 9"	13' 8"	14' 0"	14' 0"	14' 0"
		#2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"
		#3	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"
		Stud	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"
		Standard	4' 0"	5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	10' 2"	11' 0"	11' 10"	14' 0"	14' 0"
16" o.c.	SPF HF	#1 / #2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 8"	8' 1"	8' 8"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 8"	8' 1"	8' 6"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 8"	6' 11"	7' 5"	9' 3"	9' 11"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 1"	8' 5"	8' 9"	9' 11"	10' 4"	11' 10"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 8"	6' 5"	6' 10"	8' 7"	9' 2"	11' 7"	12' 1"	13' 6"	14' 0"	14' 0"	14' 0"
12" o.c.	SPF HF	#1 / #2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	11' 8"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 1"	8' 0"	8' 6"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 8"	9' 3"	9' 8"	10' 11"	11' 4"	13' 0"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	12' 11"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 1"	7' 5"	7' 11"	9' 11"	10' 7"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:

Group A:

Spruce-Pine-Fir	
#1 / #2	Standard
#3	Stud

Hem-Fir	
#2	Stud
#3	Standard

Douglas Fir-Larch
#3
Stud
Standard

Southern Pine***
#3
Stud
Standard

Group B:

Hem-Fir
#1 & Btr
#1

Douglas Fir-Larch
#1
#2

Southern Pine***
#1
#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 55 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

Attach "L" braces with 10d (0.128"x3.0" min) nails.

* For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones.

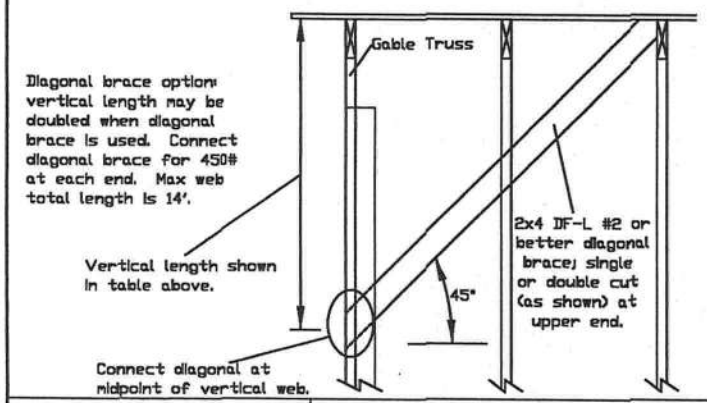
** For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

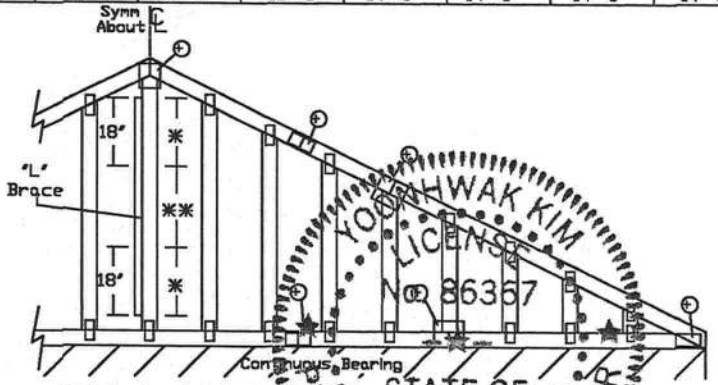
"L" bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0"	3X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.





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For more information see this job's general notes page and these web sites:
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514 Earth City Expressway
Suite 242
Earth City, MO 63045

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-10-GAB14015
DATE	10/01/14
DRWG	A14015ENC101014

10/09/2020

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforecement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

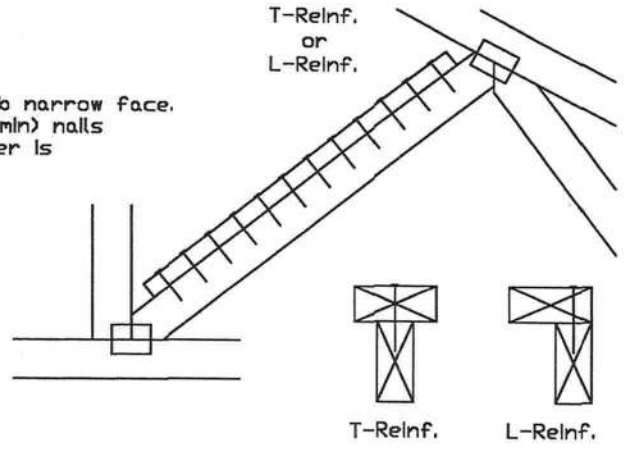
Web Member Size	Specified CLR Restraint	Alternative Reinforecement T- or L- Reinf.	Alternative Reinforecement Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4 ⊛⊛
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6 ⊛⊛

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

⊛⊛ Center scab on wide face of web. Apply (1) scab to each face of web.

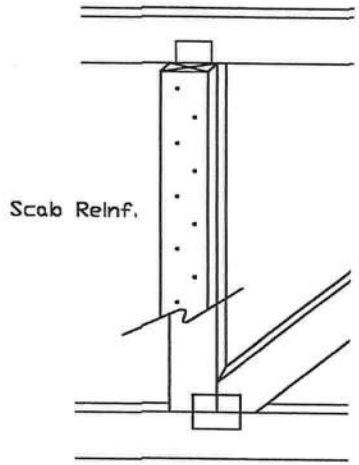
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



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514 Earth City Expressway
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⊛⊛WARNING⊛⊛ READ AND FOLLOW ALL NOTES ON THIS DRAWING
⊛⊛IMPORTANT⊛⊛ FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

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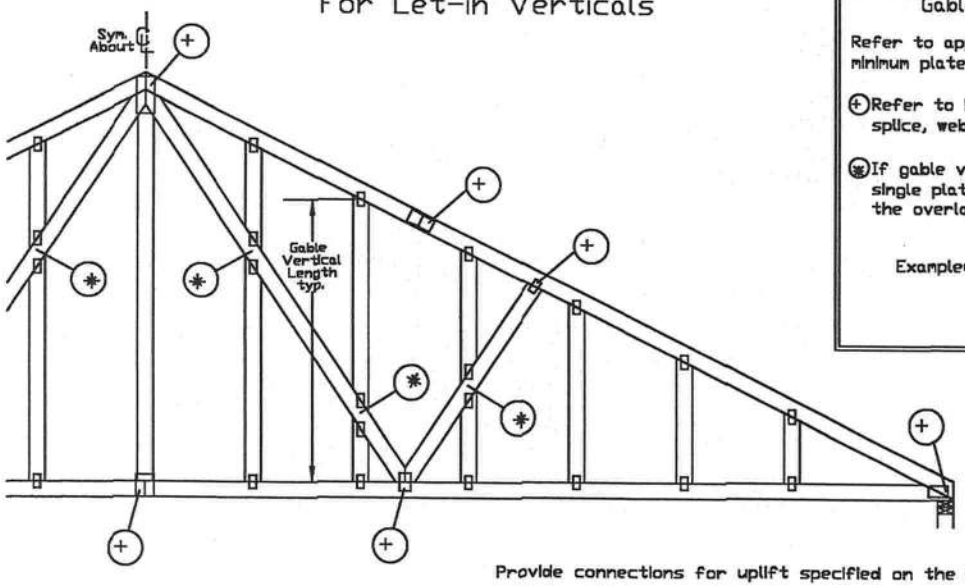
For more information see this job's general notes page and these web sites:
ALPINE: www.alpineitw.com) TPI: www.tpinst.org) SBCA: www.sbcindustry.org) ICC: www.iccsafe.org

TC LL	PSF	REF CLR Subst.
TC DL	PSF	DATE 01/02/19
BC DL	PSF	DRWG BRCLBSUB0119
BC LL	PSF	
TOT. L.D.	PSF	
DUR. FAC.		
SPACING		

1/30/2020
E 86367 278 Yoonhwak Kim FL PE #86367

Gable Detail
For Let-In Verticals



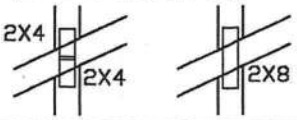
Gable Truss Plate Sizes

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

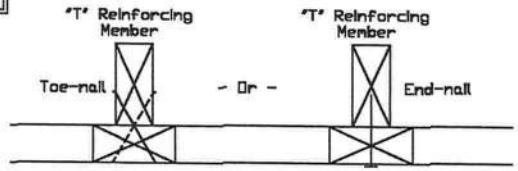
(+) Refer to Engineered truss design for peak, splice, web, and heel plates.

(*) If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



"T" Reinforcement Attachment Detail



Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with

End Driven Nails:
- 10d Common (0.148"x 3",min) Nails at 4" o.c. plus
- (4) nails in the top and bottom chords.

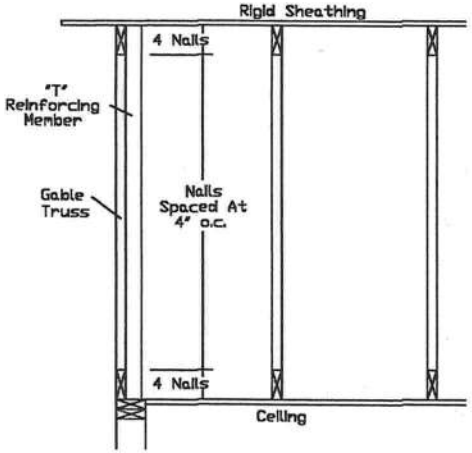
Toenailed Nails:
- 10d Common (0.148"x3",min) Toenails at 4" o.c. plus
- (4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

ASCE 7-05 Gable Detail Drawings
A13015051014, A12015051014, A11015051014, A10015051014, A14015051014, A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings
A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118, A18015ENC100118, A20015ENC100118, A20015END100118, A20015PED100118, A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118, A18030ENC100118, A20030ENC100118, A20030END100118, A20030PED100118, S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118, S18015ENC100118, S20015ENC100118, S20015END100118, S20015PED100118, S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118, S18030ENC100118, S20030ENC100118, S20030END100118, S20030PED100118

See appropriate Alpine gable detail for maximum unreinforced gable vertical length.



To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate Alpine gable detail).

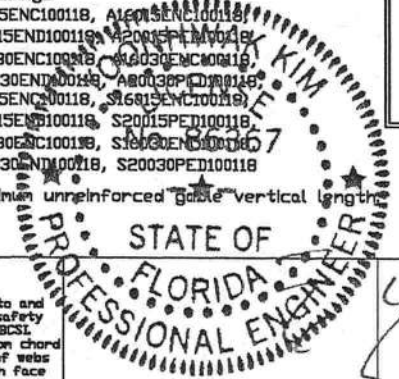
Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member.

Web Length Increase w/ "T" Brace

"T" Reinf. Mbr. Size	"T" Increase
2x4	30 %
2x6	20 %

Example:
- ASCE 7-10 Wind Speed = 120 mph
- Mean Roof Height = 30 ft, Kzt = 1.00
- Gable Vertical = 24"o.c. SP #3
- "T" Reinforcing Member Size = 2x4
- "T" Brace Increase (From Above) = 30% = 1.30
- (1) 2x4 "L" Brace Length = 8' 7"
- Maximum "T" Reinforced Gable Vertical Length
  1.30 x 8' 7" = 11' 2"



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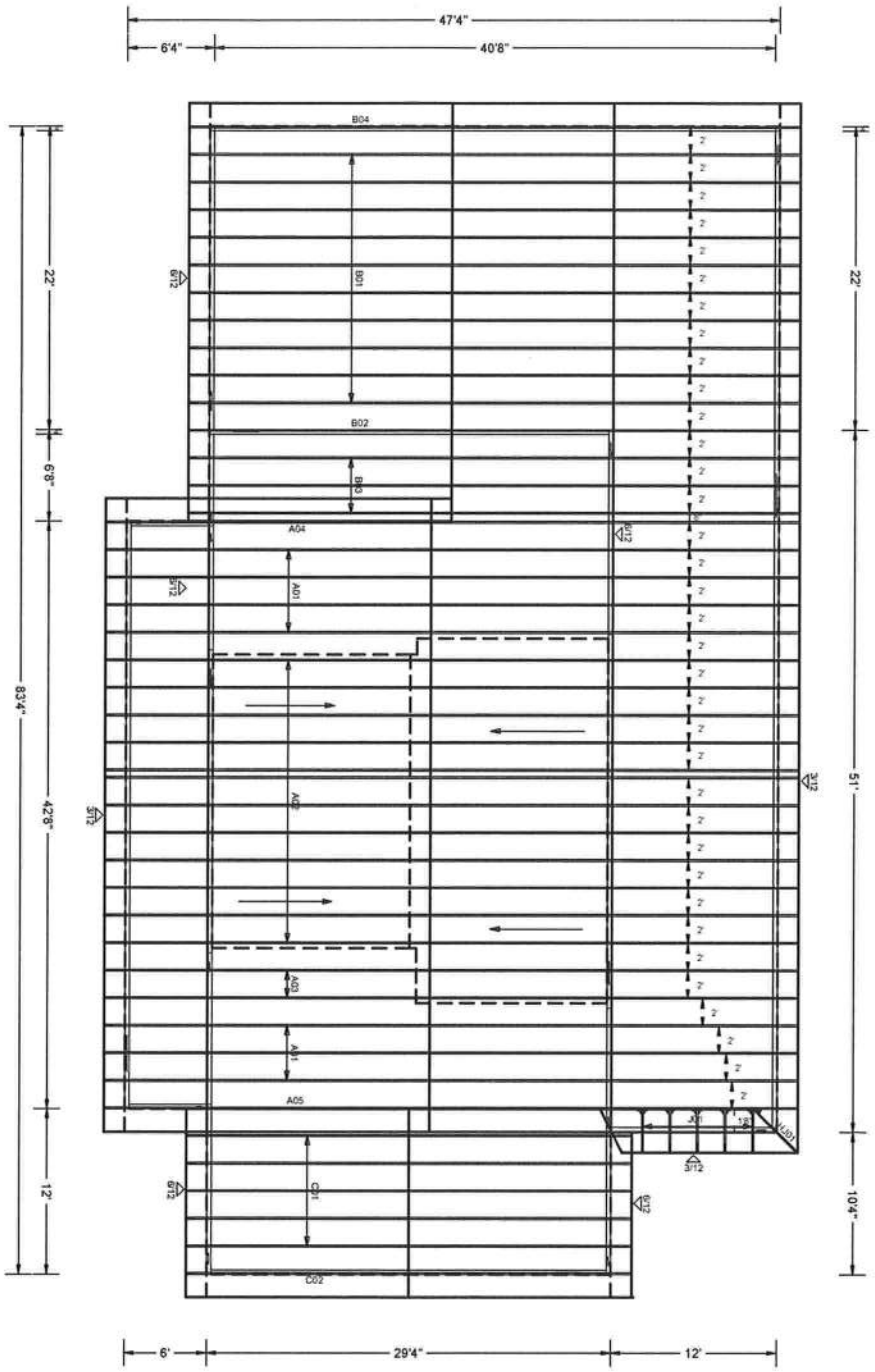

W.B. Howland Truss Co.
610 11th St. SW
Live Oak, FL 32064
(386) 362-1235
(386) 362-7124 (Fax)
howlandtruss@gmail.com

ROOF PITCH: 6/12, 3/12
OVERHANG: 18"
CEILING: 9"
EXT. WALLS:
LOADING: 40psf
WIND LOAD: 130mph
EXPOSURE: C
DATE: 9/9/20

PAGE NO: 1 OF 1	JOB NO: 20-4515	Job Name: Steedley Residence Customer: OWNER BUILDER Designer: Kelly Caudill ADDRESS: SALESMAN: HOUSE : <Not Found>	JOB #: 20-4515	WB HOWLAND