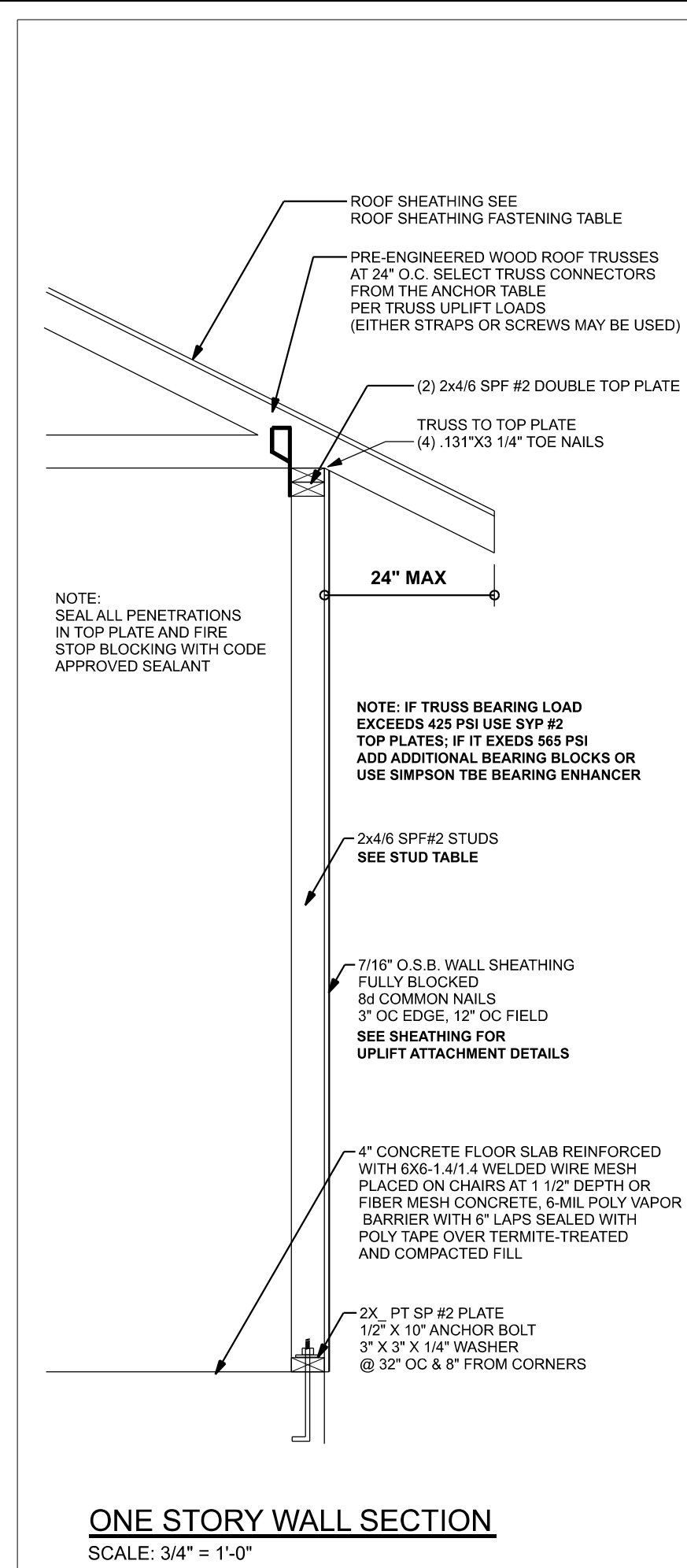
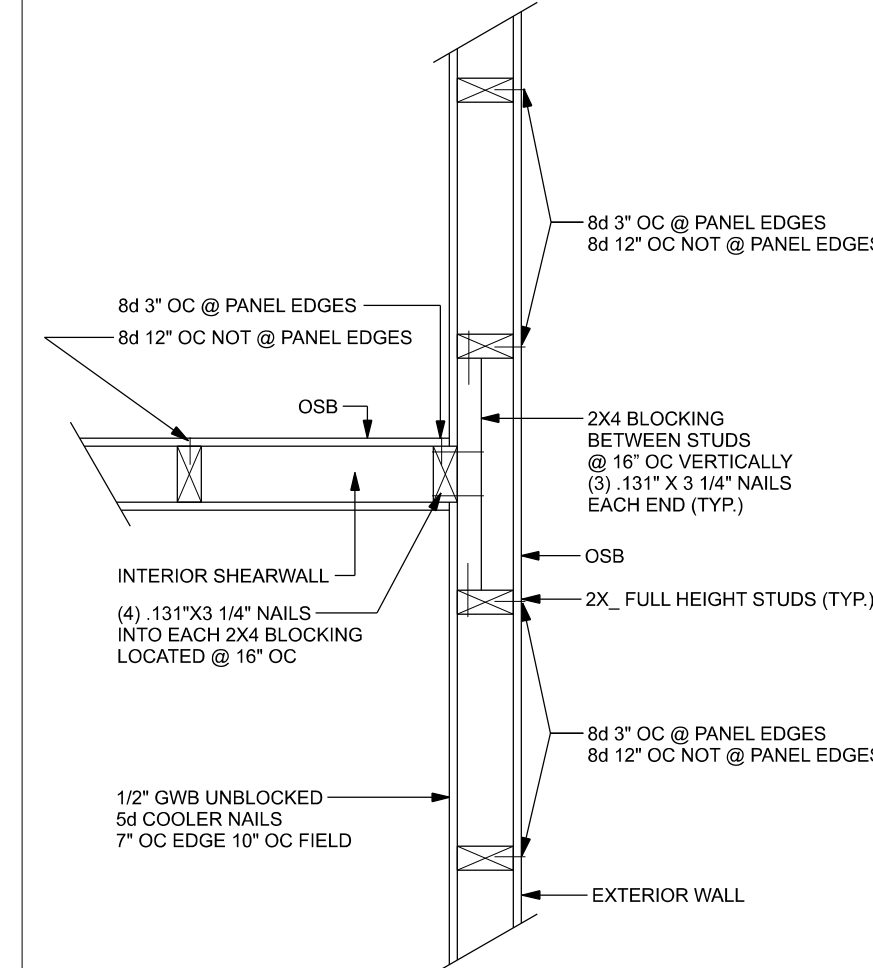
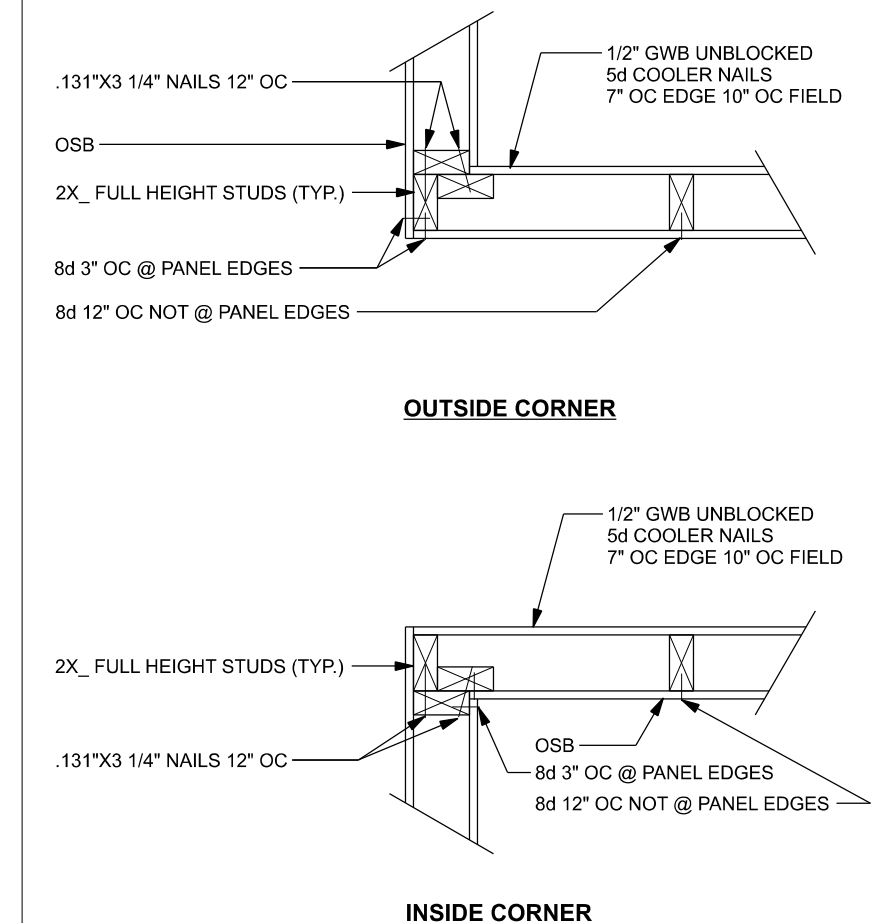




(TYP.) GABLE BRACING DETAIL
WOOD FRAME



(TYP.) CORNER FRAMING
WOOD FRAME

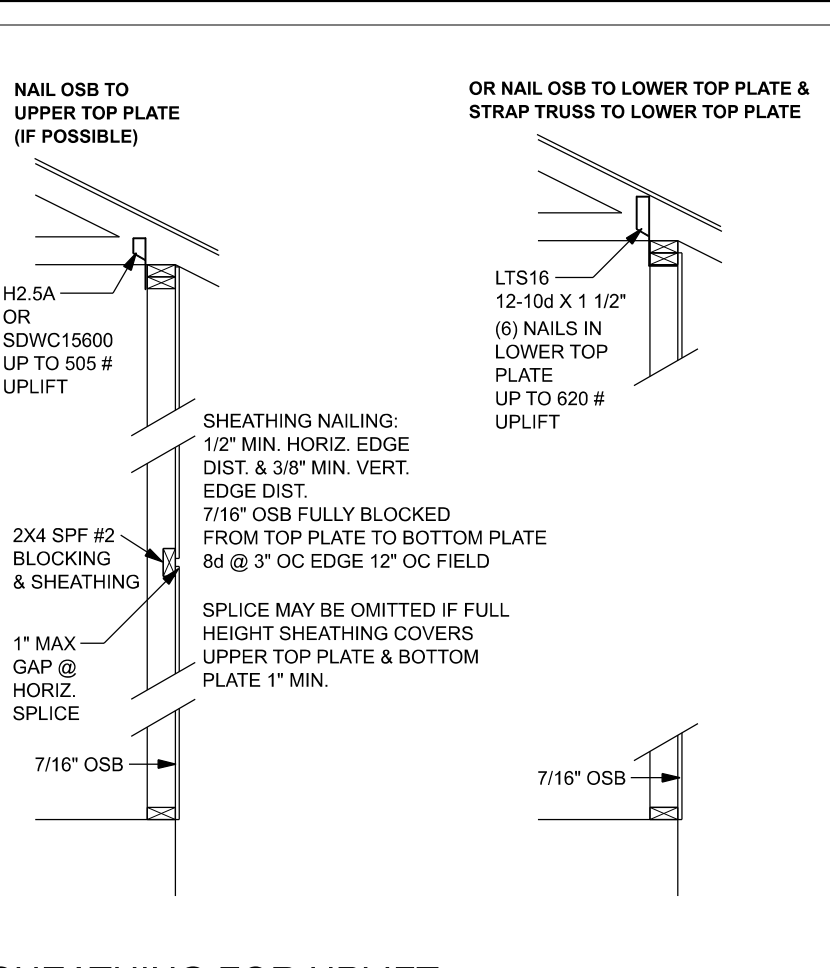


(TYP.) CORNER FRAMING
WOOD FRAME

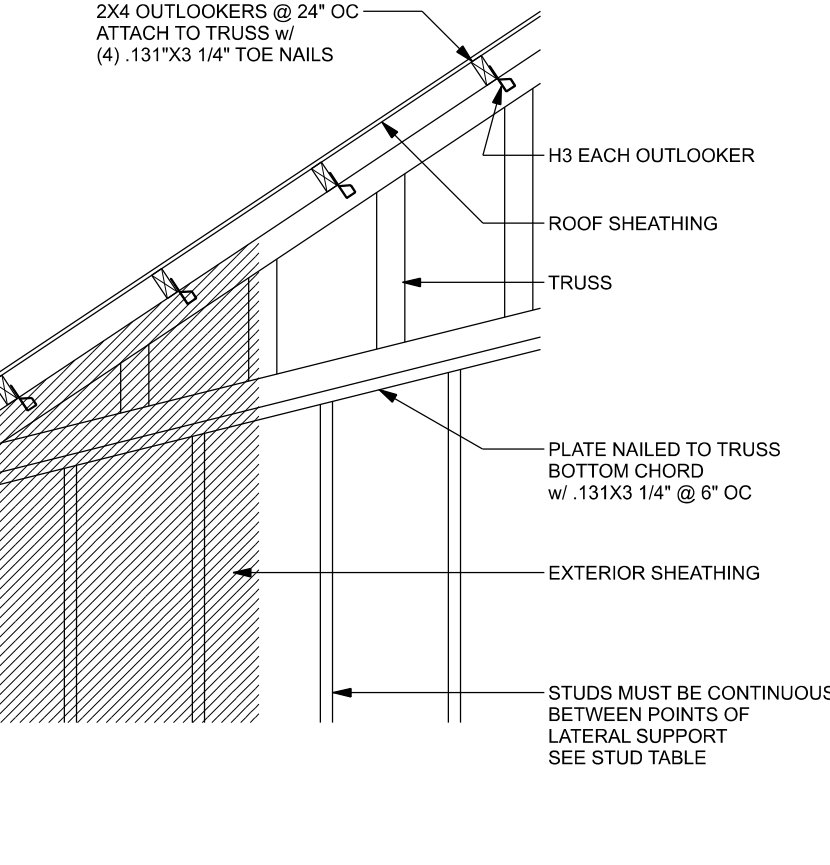
ROOF SHEATHING FASTENING TABLE (RAFTER / TRUSS SG = 0.49)

Wind Speed	Sheathing Thickness Plywood or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field
120 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.131")	6" oc	12" oc
120 mph Exp. C	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.131")	6" oc	12" oc
130 mph Exp. B	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
130 mph Exp. C	15/32"	ASTM F1667 RRS-01 (2 3/8" x 0.131")	6" oc	6" oc
130 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.131")	6" oc	6" oc
140 mph Exp. C	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. C	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	4" oc	4" oc

Note: For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field. Note: This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.



(TYP.) GABLE WALL w/ VAULTED CEILING
WOOD FRAME



(TYP.) GABLE WALL w/ VAULTED CEILING
WOOD FRAME

HEADER STRAP TABLE

Option/Uplift	Top Connection	Bottom Connection
#1	Attach king stud to top plate w/ (1) Simpson SDWC15600	Attach king stud to bottom plate w/ (2) Simpson SDWC15450
#2	Attach king stud to top plate w/ (2) Simpson SDWC15600	Attach king stud to bottom plate w/ (3) Simpson SDWC15450

HEADER SCREWS TABLE

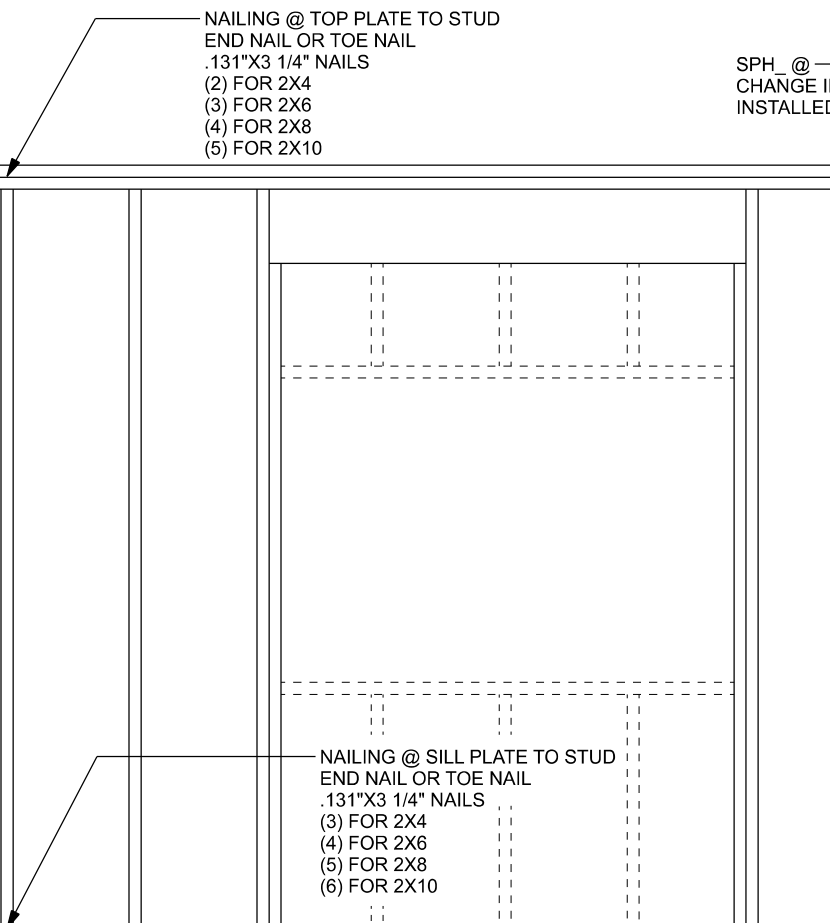
Option/Uplift	Top Connection	Bottom Connection
#1	Attach king stud to top plate w/ (1) Simpson SDWC15600	Attach king stud to bottom plate w/ (2) Simpson SDWC15450
#2	Attach king stud to top plate w/ (2) Simpson SDWC15600	Attach king stud to bottom plate w/ (3) Simpson SDWC15450

SILL PLATE SPANS FOR 10'-0" WALL HEIGHT

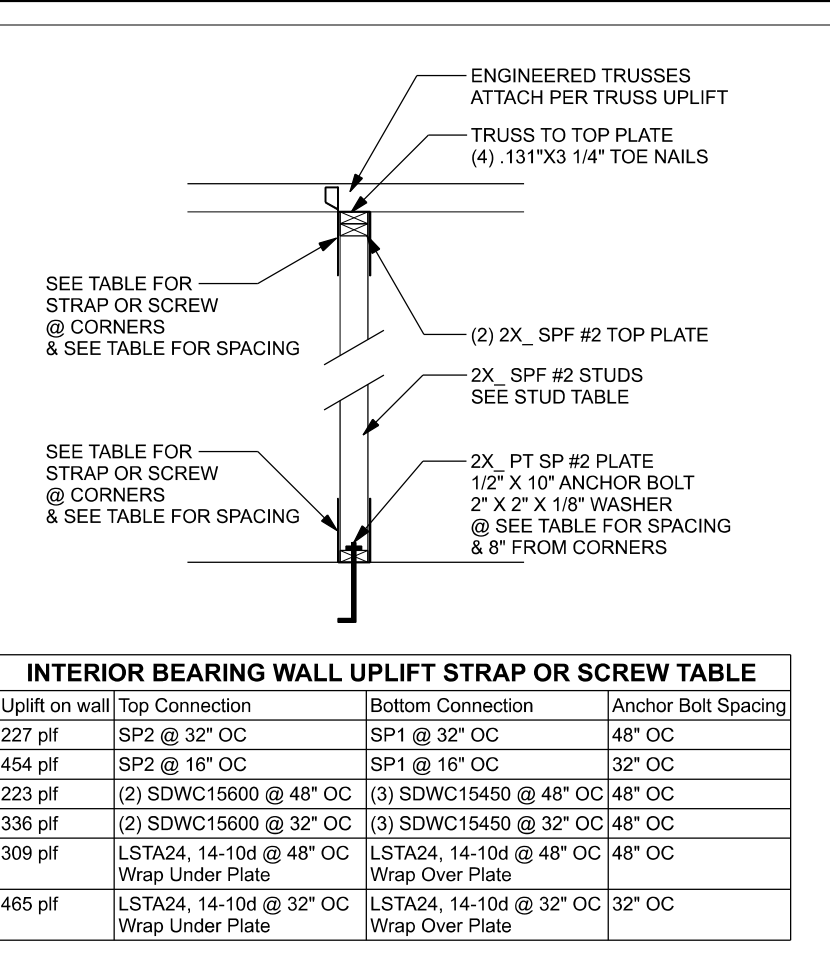
DESIGN WIND SPEED	MAX. SPANS FOR SPF #2	BASED ON WFCM NAIL #3.238
130 MPH EXP. C	5'-2"	7'-9"
	7'-9"	11'-3"

(TYP.) GABLE WALL w/ VAULTED CEILING
WOOD FRAME

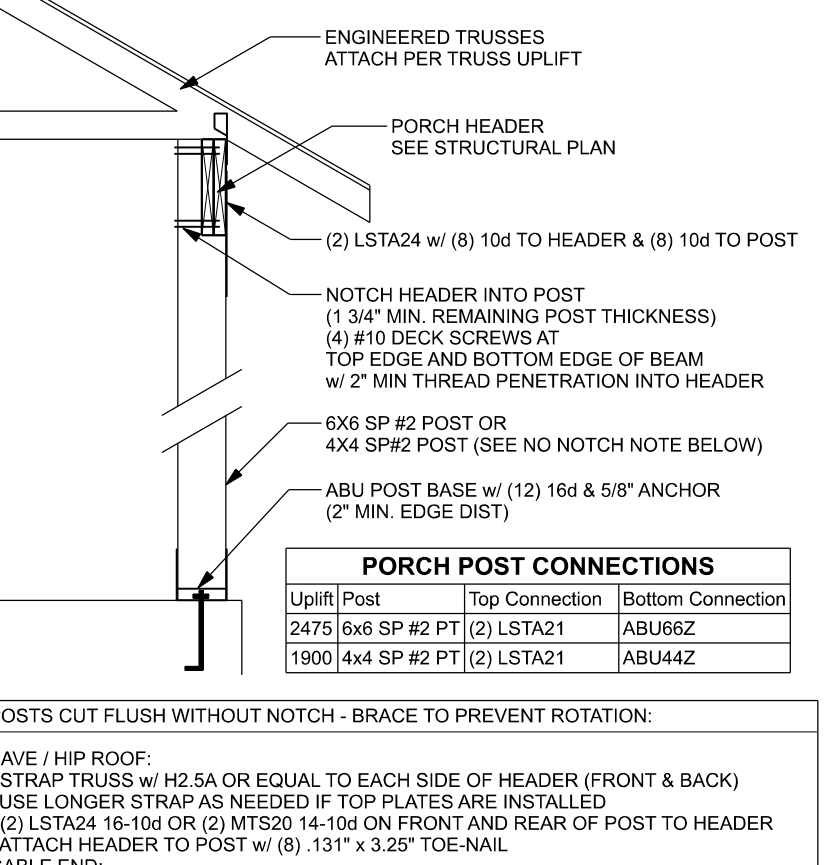
(TYP.) GABLE WALL w/ VAULTED CEILING
WOOD FRAME



(TYP.) GABLE WALL w/ VAULTED CEILING
WOOD FRAME

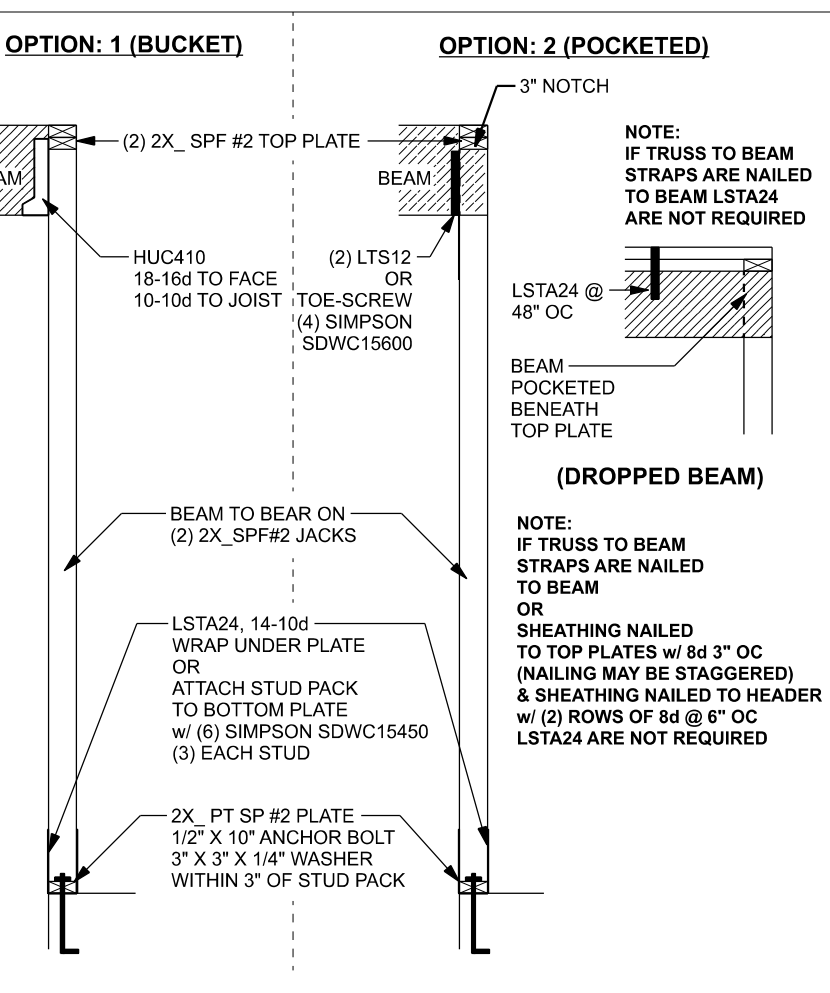


(TYP.) INTERIOR BEARING WALL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS

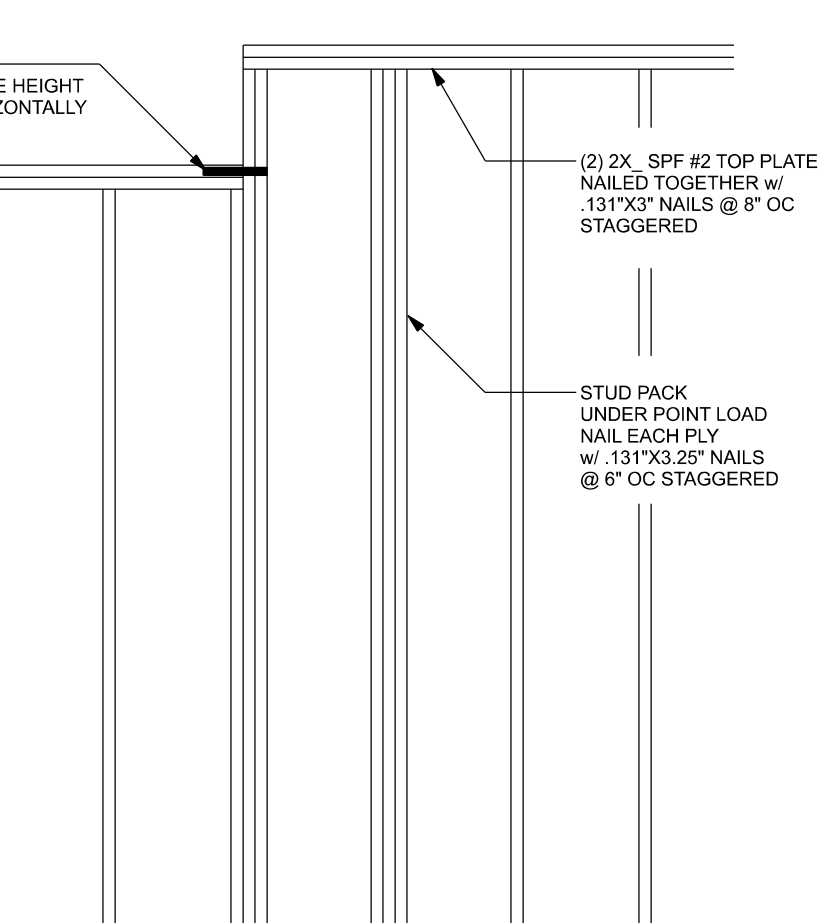


(TYP.) PORCH POST
ONE STORY WOOD

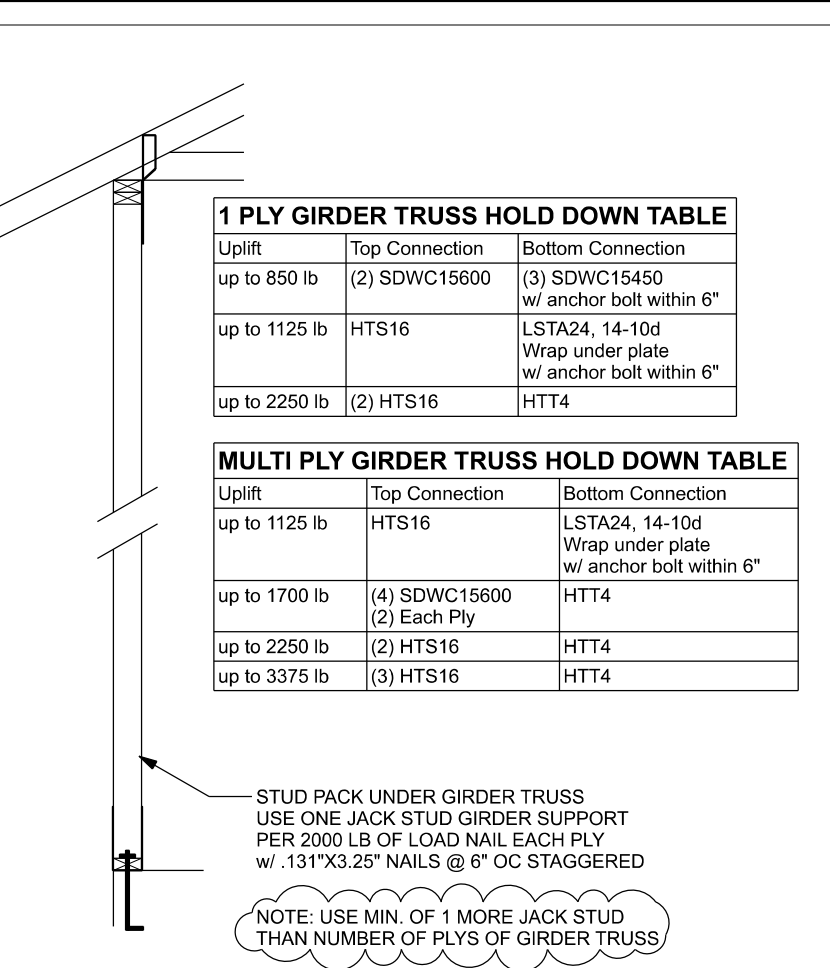
(TYP.) PORCH POST
ONE STORY WOOD



(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS

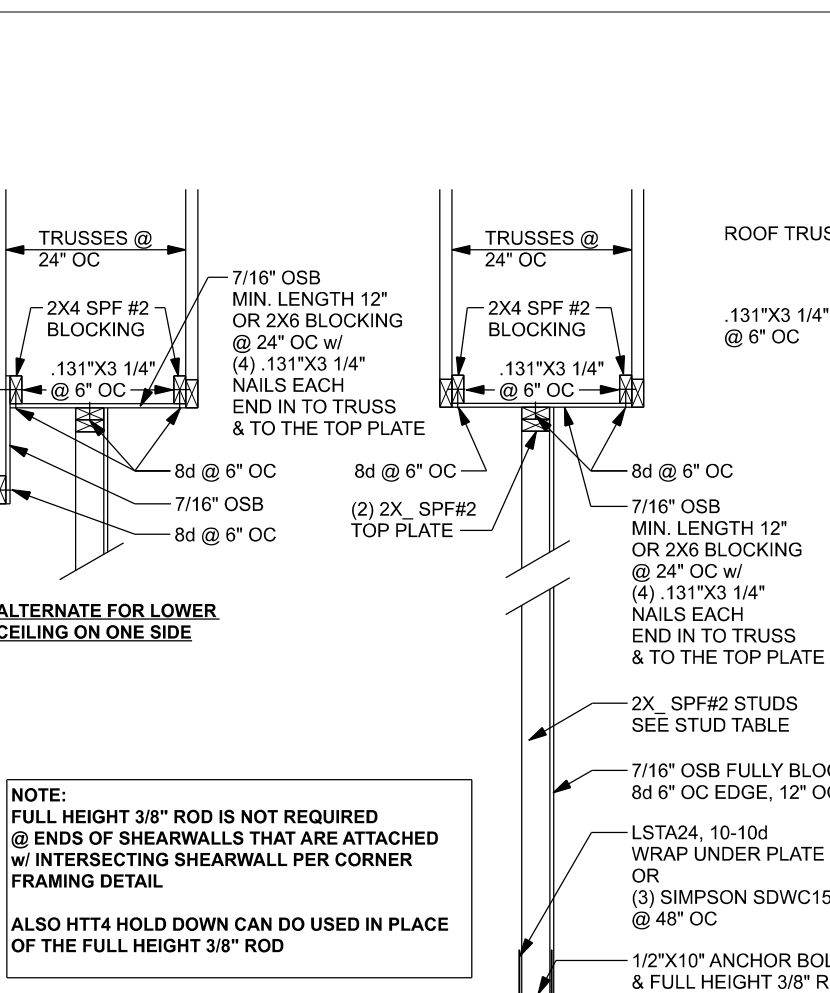


(TYP.) GIRDER TRUSS HOLD DOWN DETAIL
WOOD FRAME w/ STRAPS & ANCHORS



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WOOD FRAME w/ STRAPS & ANCHORS

CONNECTOR TABLE

Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
805	505	SDWC15600	-	-
415	290	H3	4-8x1 1/2"	4-8x1 1/2"
415	540	H2.5A	5-8x1 1/2"	5-8x1 1/2"
1340	1015	H10A	9-10d1 1/2"	9-10d1 1/2"
720	620	LTS12-20	6-10d1 1/2"	6-10d1 1/2"
1000	860	MTS12-30	7-10d1 1/2"	7-10d1 1/2"
1450	1245	HTS20-30	12-10d1 1/2"	12-10d1 1/2"
1235	1235	LSTA21	To One Member	To Other Member
1640	1455	MSTA24	6-10d	9-10d
1030	1030	CS20	7-10d	7-10d
585	535	SP1	6-10d	4-10d
1065	605	SP2	6-10d	6-10d
771	771	LSTA24	10-10d	wrap under or over plate w/ anchor bolt within 6"
1235	1235	LSTA24	14-10d	wrap under or over plate
1825	1800	DTT22	8-SDS 1/4"x12"	1/2"x12" Titen HD
4235	3640	HTT4	16-16d2x12"	1/2"x12" Titen HD
1825	1800	DTT22	8-SDS 1/4"x12"	1/2"x12" Titen HD
4235	3640	HTT4	16-16d2x12"	1/2"x12" Titen HD
1900	ABU4Z	Post Bases @ Stewall	To Stud	Anchor
2475	ABU6Z	Post Bases @ Mono	To Stud	Anchor
1900	ABU4Z	Post Bases @ Mono	To Stud	Anchor
2475	ABU6Z	Post Bases @ Mono	To Stud	Anchor
1900	ABU4Z	Post Bases @ Mono	To Stud	Anchor
2475	ABU6Z	Post Bases @ Mono	To Stud	Anchor

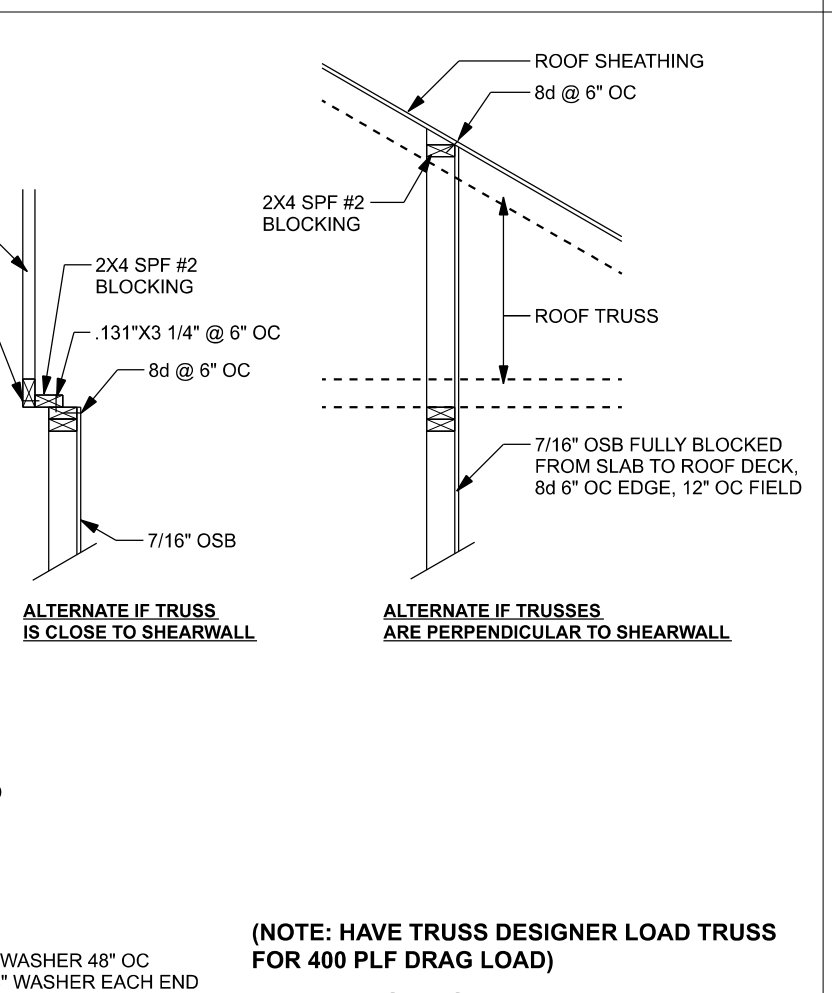
(TYP.) GIRDER TRUSS HOLD DOWN DETAIL
WOOD FRAME w/ STRAPS & ANCHORS

GRADE & SPECIES TABLE

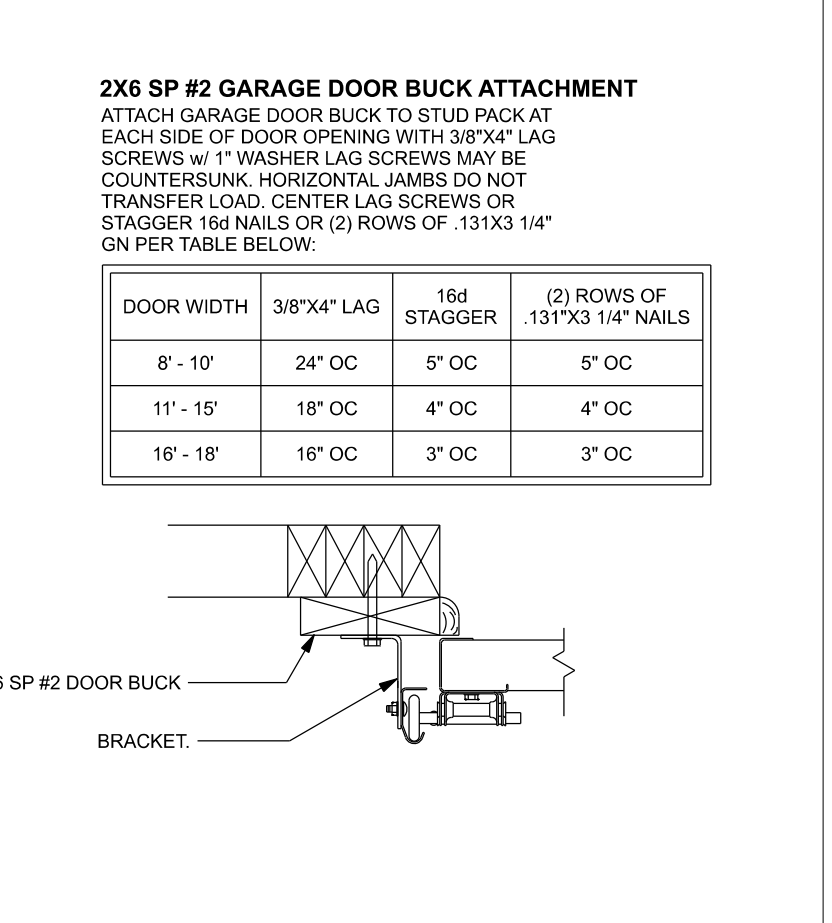
	SP #2	Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LVL	TIMBERSTRAND	1700	1.7
LSL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

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WOOD FRAME w/ STRAPS & ANCHORS

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT AND TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED AND SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS CONNECTIONS ON THE BUILDING STRUCTURE. STRAP 2x6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END; 2x8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 2500 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, F_y = 8KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A186, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 12 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH/WIDTH RATIOS OF SLAB RAFTERS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 30'. DO NOT CUT C/W OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR APPROVALS. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 40, DEFORMED BARS, F_y = 40 KSI. ALL LAP SPLICES 40" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILLED AND PLACED IN ACCORDANCE WITH ACI 318-08, UNO.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL. DIAPHRAGMS, SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 10" IN GROUDED CMU.

BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

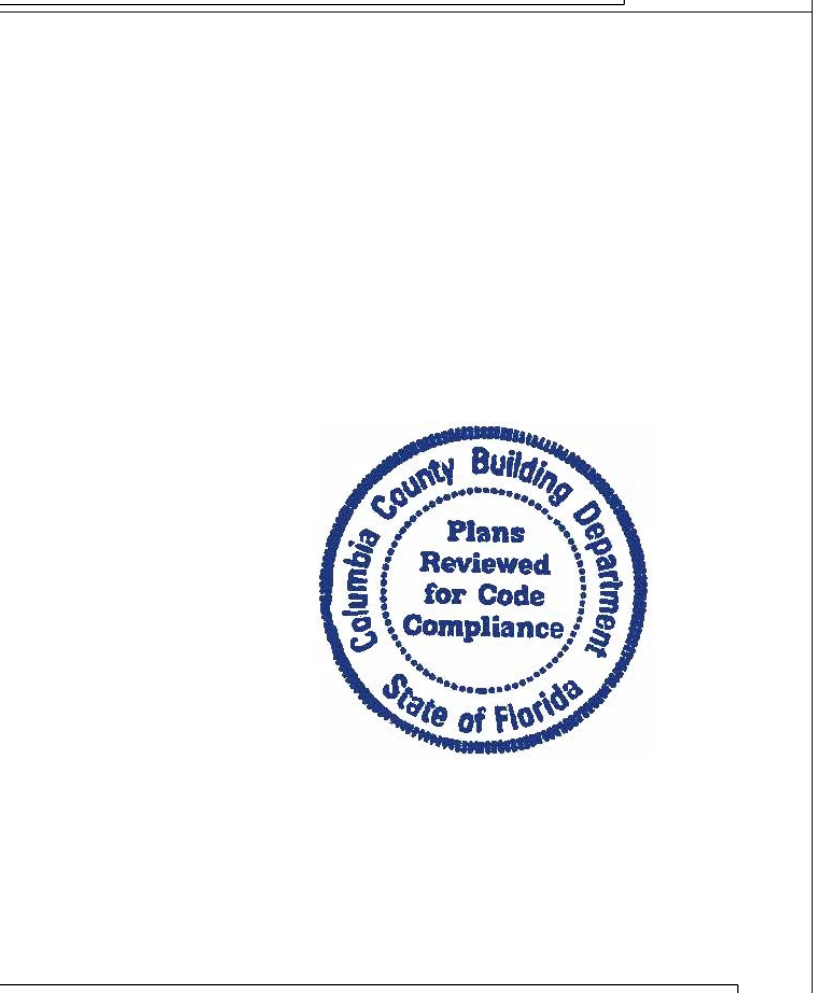
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION.

THE WIND LOAD ENGINEER IMMEDIATELY.

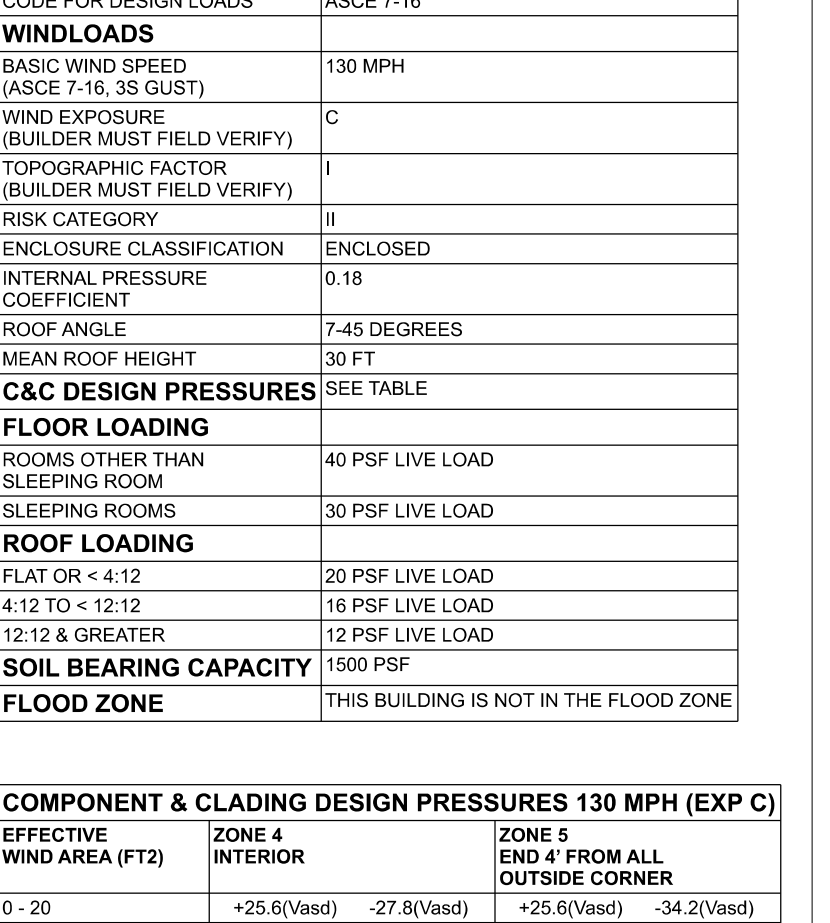
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

(TYP.) GIRDER TRUSS HOLD DOWN DETAIL
WOOD FRAME w/ STRAPS & ANCHORS

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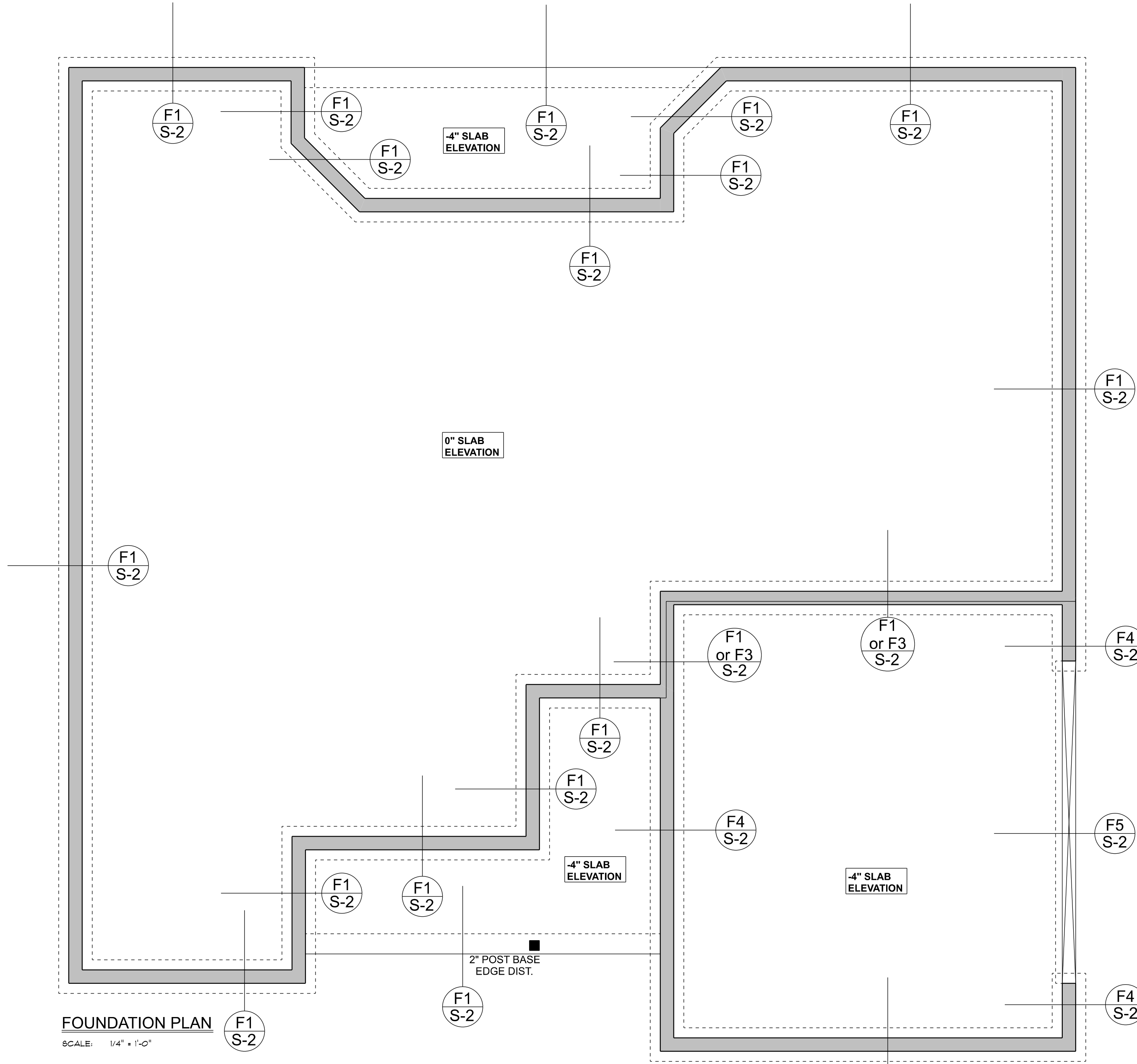
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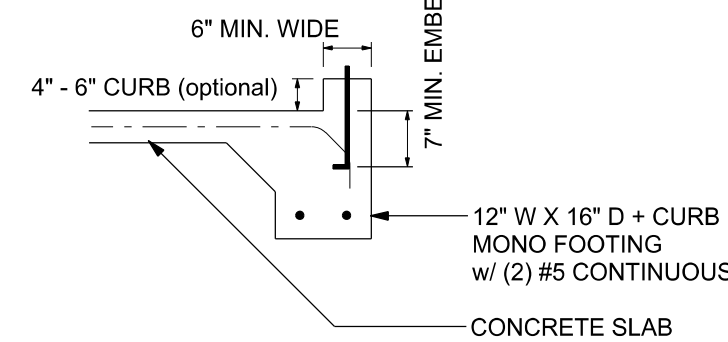
(TYP.) GIRDER TRUSS HOLD DOWN DETAIL
WOOD FRAME w/ STR



FOUNDATION PLAN

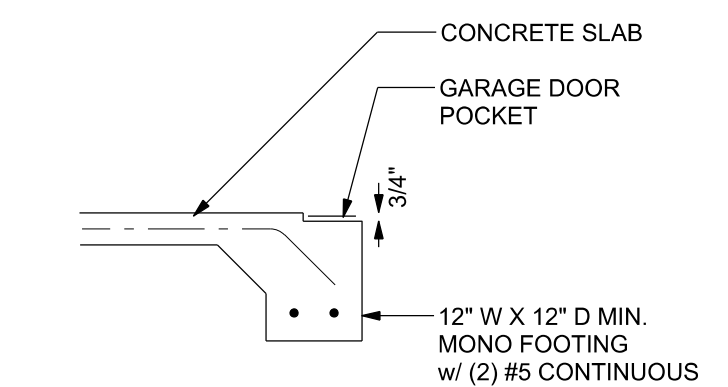
SCALE: 1/4" = 1'-0"

FOUNDATION NOTES	
FN - 1	DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS. RECESSES IN SLAB STEP DOWNS, ETC. DISOWAY DESIGN GROUP OR MARK DISOWAY P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
FN - 2	CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING WALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.
FN - 3	THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED W/ 6X8-14I A WELDED WIRE MESH PLACED ON CHAIRS @ 1'12" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER W/ 6" LAPS SEALED W/ POLY TAPE OVER TERMITES. TREATED & COMPACTED FILL (ALSO, ANY OTHER CODE APPROVED TERMITE-TREATMENT METHOD CAN BE USED INSTEAD)



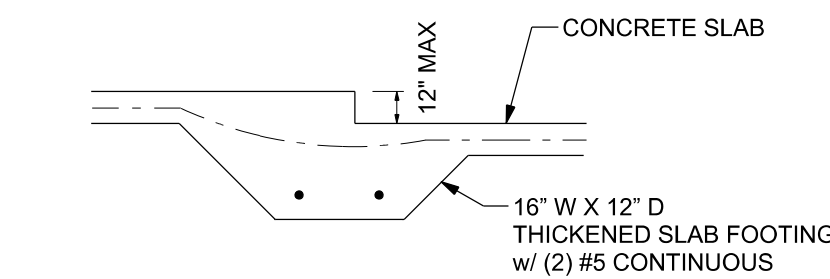
F4 S-2 MONOLITHIC CURB FOOTING

SCALE: 1/2" = 1'-0"



F5 S-2 GARAGE DOOR POCKET FOOTING

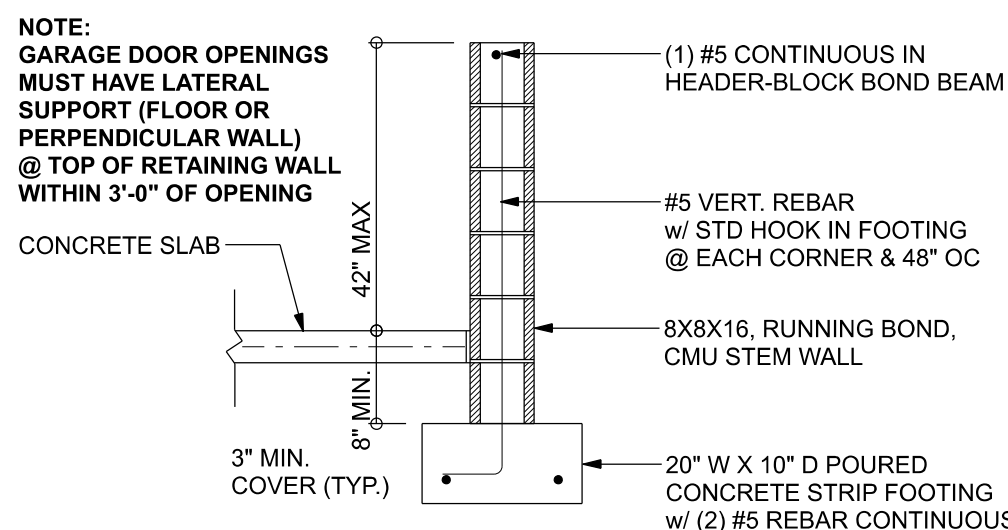
SCALE: 1/2" = 1'-0"



F3 S-2 INTERIOR BEARING STEP FOOTING

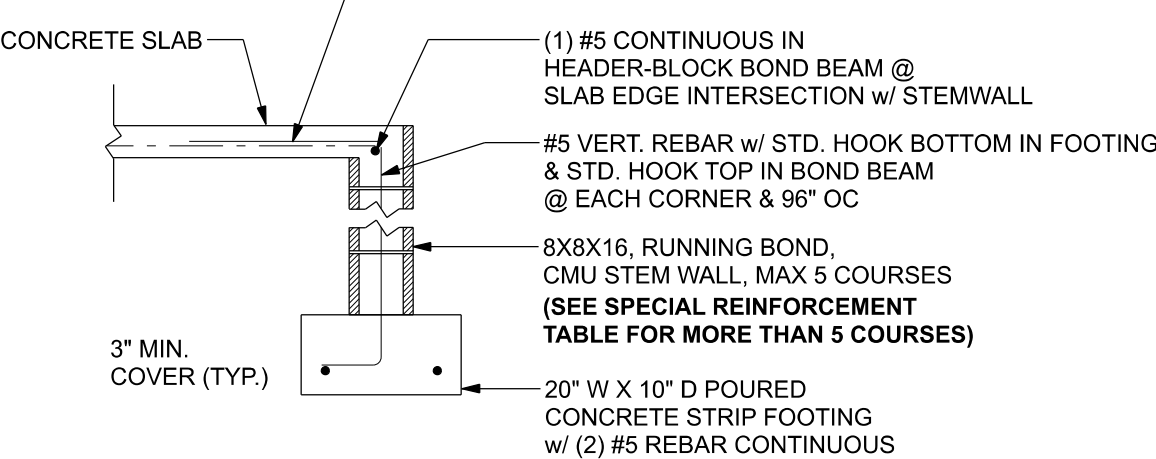
SCALE: 1/2" = 1'-0"

BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL



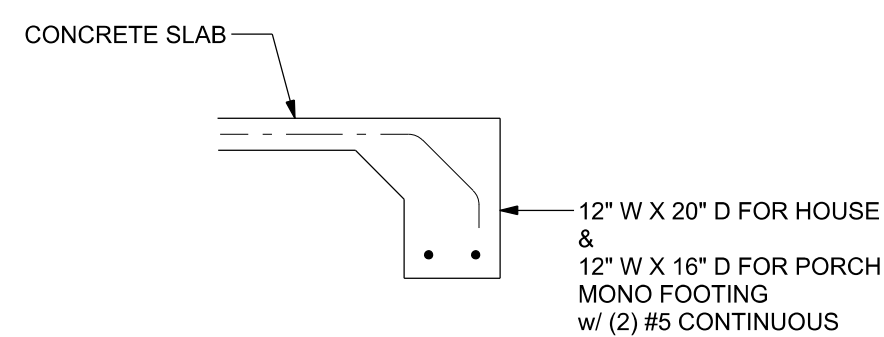
F4 S-2 OPTIONAL STEM WALL CURB FOOTING

SCALE: 1/2" = 1'-0"



F1 S-2 OPTIONAL STEM WALL FOOTING

SCALE: 1/2" = 1'-0"

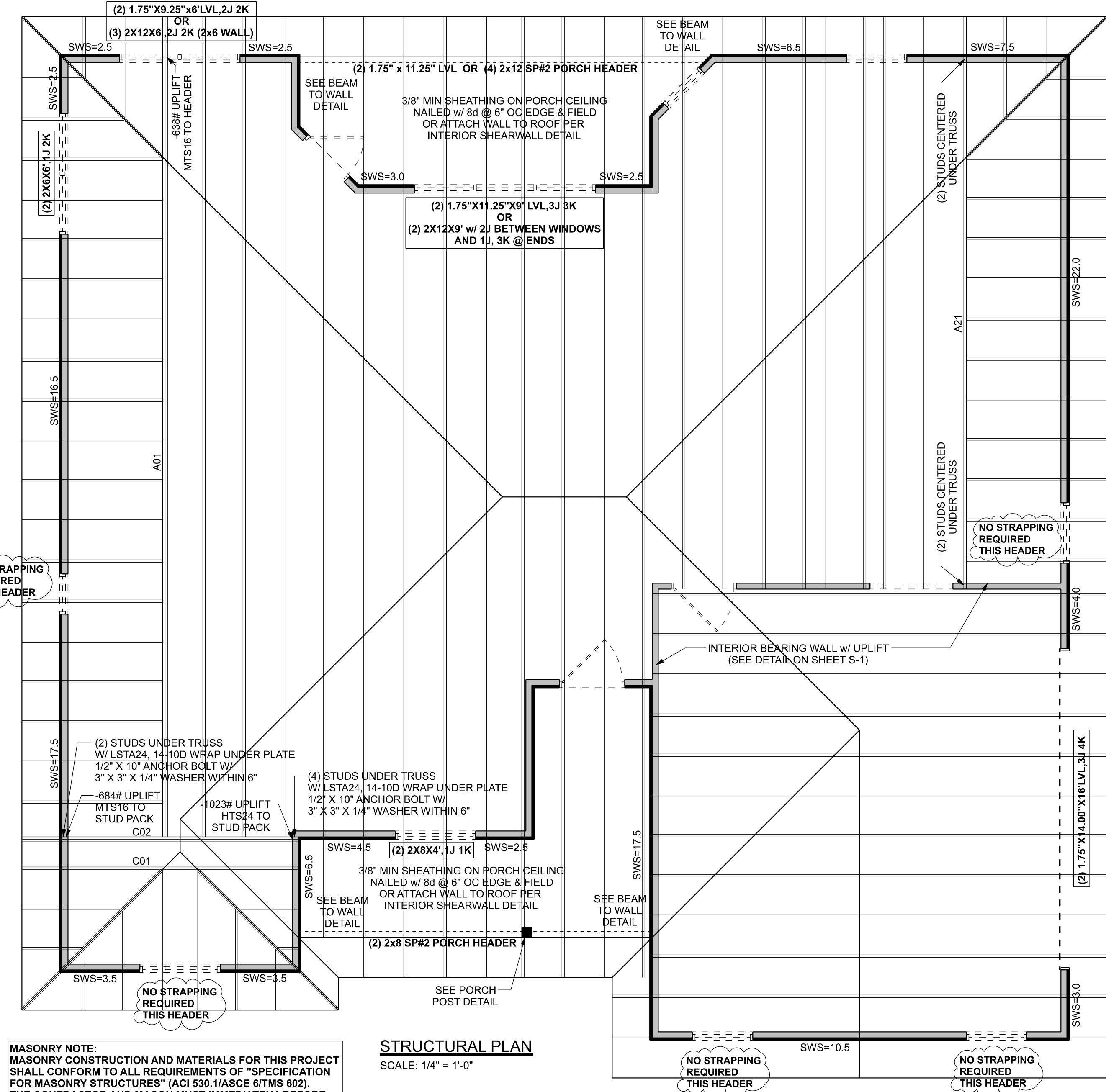


F1 S-2 MONOLITHIC FOOTING

SCALE: 1/2" = 1'-0"

NOTE: FOR STEM WALL FOUNDATIONS OVER 5 COURSES IN HEIGHT THE SLAB IS REQUIRED TO BE ATTACHED TO THE STEM WALL @ BOND BEAM W/ 2" X 2" #5 CORNER REBARS (1) LEG EXTENDING INTO SLAB & (1) LEG LAPPED W/ THE HORIZONTAL BOND BEAM REBAR SPACED THE SAME AS VERTICAL REBAR

(1) #5 CONTINUOUS IN HEADER-BLOCK BOND BEAM @ SLAB EDGE INTERSECTION W/ STEM WALL
#5 VERT. REBAR W/ STD. HOOK BOTTOM IN FOOTING & STD. HOOK TOP IN BOND BEAM @ EACH CORNER & 96" OC
8X8X16, RUNNING BOND, CMU STEM WALL, MAX 5 COURSES (SEE SPECIAL REINFORCEMENT TABLE FOR MORE THAN 5 COURSES)
20" W X 10" D POURED CONCRETE STRIP FOOTING W/ (2) #5 REBAR CONTINUOUS



MASONRY NOTE: MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI 530.1-02 Section		Specific Requirements
1.4A	Compressive strength	8" block bearing walls 17m = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 40, Fy = 40 ksi, Lap splices min 40 bar dia. (25" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/lb2 or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb2 or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

STRUCTURAL PLAN

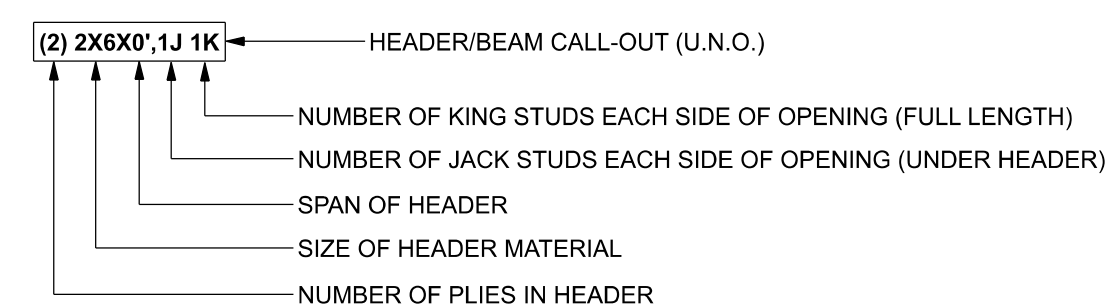
SCALE: 1/4" = 1'-0"

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER, W.B. HOWLAND COMPANY JOB #21-6637 (SEALED 02/07/2022)

ACTUAL vs REQUIRED SHEARWALL

	TRANSVERSE	LONGITUDINAL
ACTUAL	35442 LBF	19404 LBF
REQUIRED	12011 LBF	15468 LBF

HEADER LEGEND



STRUCTURAL PLAN NOTES

SN-1 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

SN-2 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

UNLESS NOTED OTHERWISE (MINIMUM REQUIREMENTS)	
SEE STRUCTURAL PLAN FOR ANY SPECIFIC CALL OUTS	
BEAM / HEADERS (SIZE)	ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SP #2 (UNO)
HEADERS (JACK & KING STUDS)	ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (UNO)
HEADERS (STRAPING)	ALL HEADERS W/ UPLIFT TO BE STRAPPED OR SCREWED DOWN W/ OPTION #1 (SEE DETAIL ON SHEET S-1) (U.N.O.) 1/2" X 10" ANCHOR BOLT W/ 3" X 3" X 1/4" WASHER MUST BE LOCATED WITHIN 6" OF KING STUD @ ALL DOOR LOCATIONS (U.N.O.)
JACK STUDS UNDER GIRDER TRUSS	USE ONE JACK STUD GIRDER SUPPORT PER 2000 LB LOAD

Bryan Zecher Construction

Kellum, Amaya Res

PROJECT ADDRESS:
267 SW Timberland Ct,
Lake City, Florida 32024

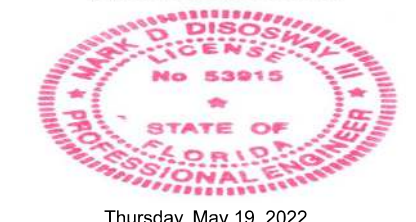
DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY P.E. 53915
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JOB NUMBER:
220603

S-2

OF 2 SHEETS