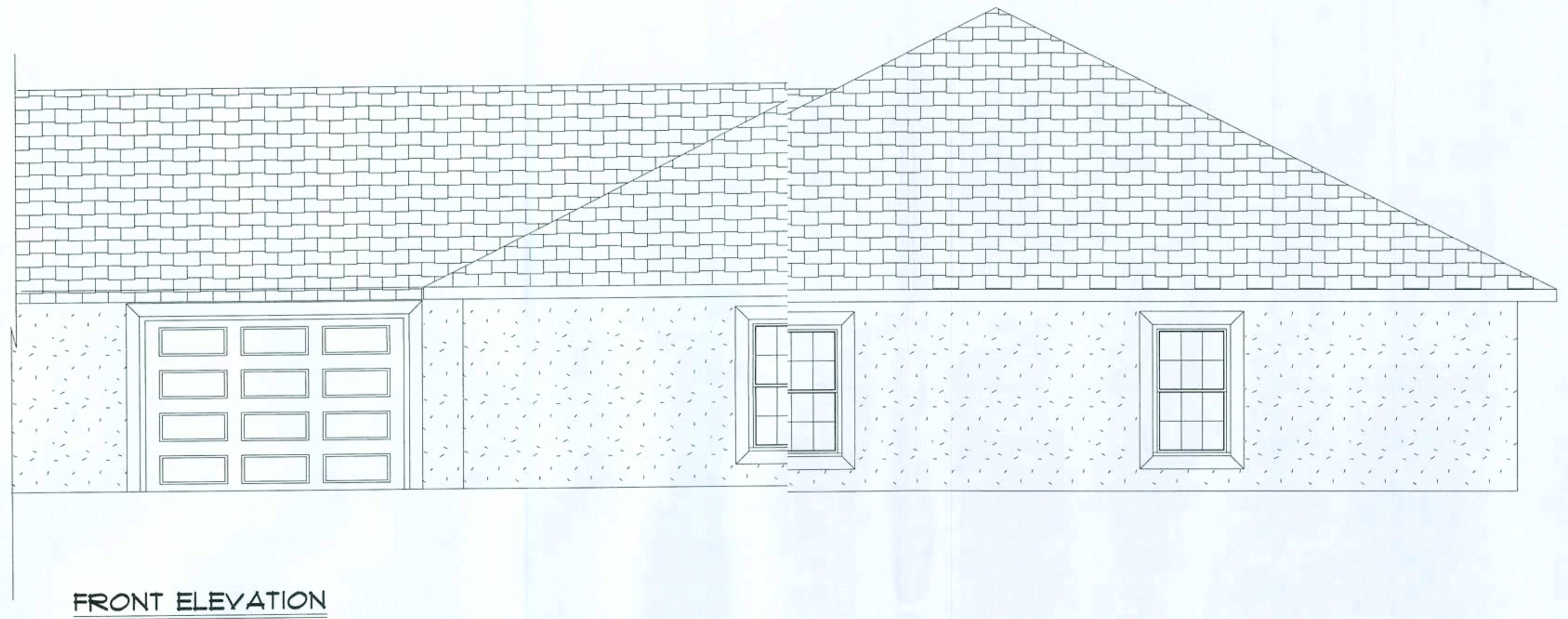
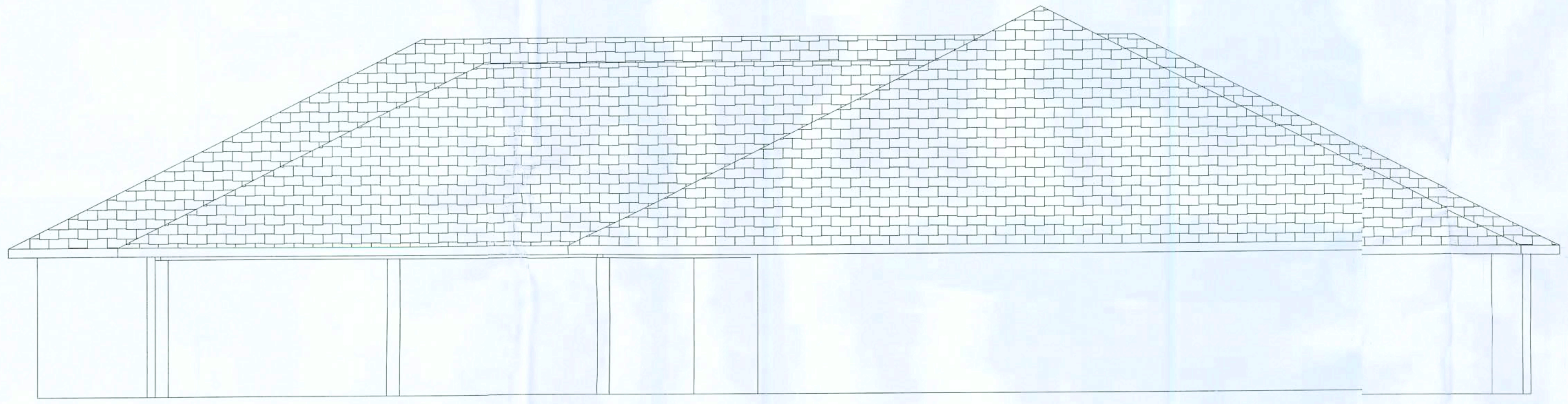
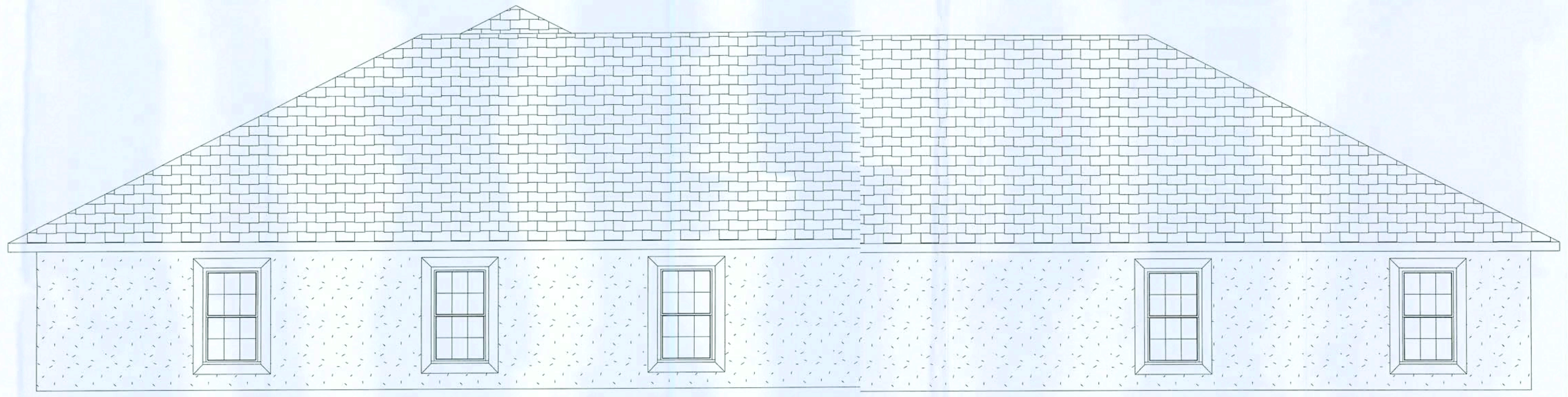


FRONT ELEVATION



SIDE ELEVATION

REAR ELEVATION



SIDE ELEVATION

ELEVATION PLAN

SCALE: 3/16" = 1'-0"

ADDITION FOR:
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LICENSE NUMBER: CBC1251118

SHEET:

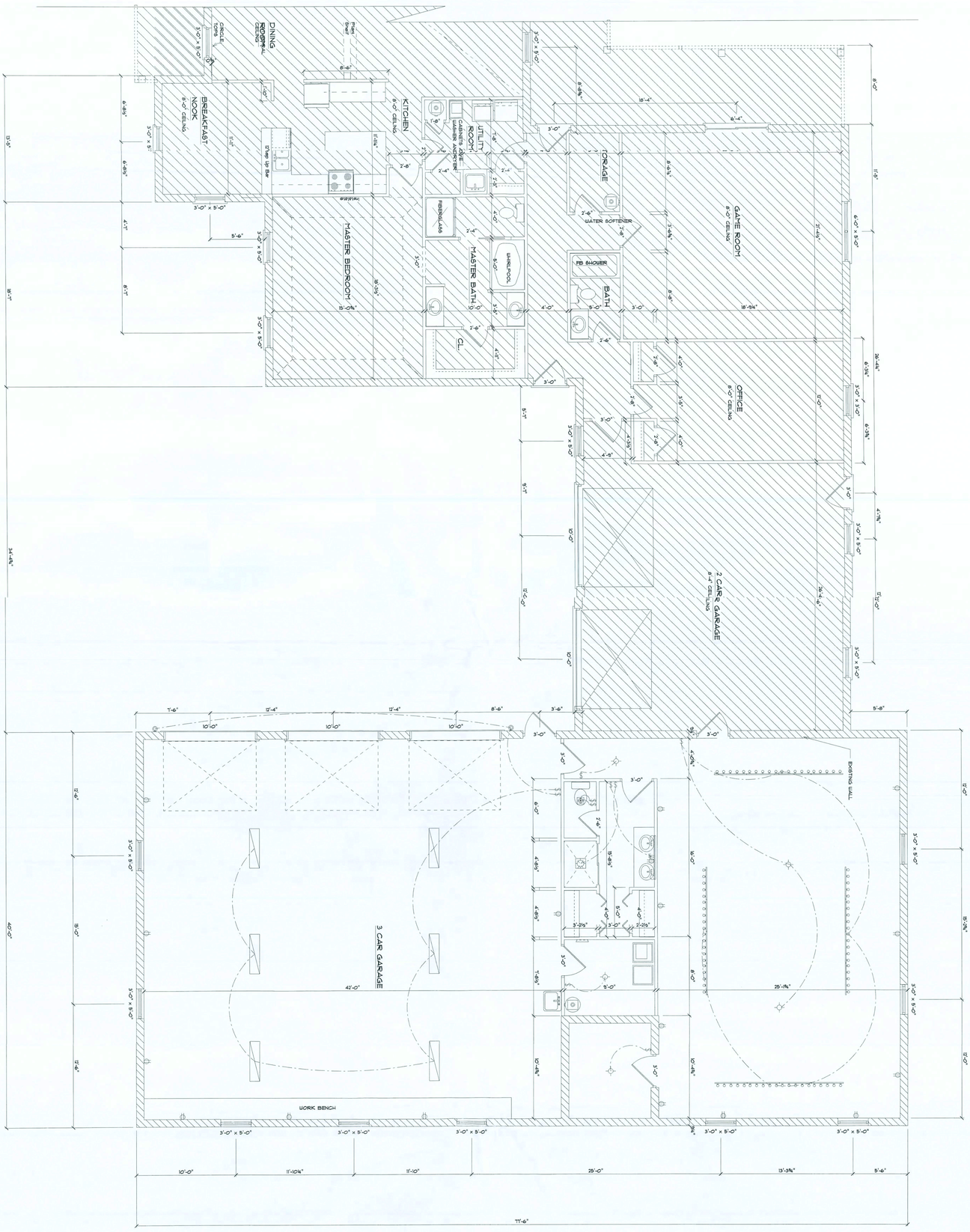
A - 1

1 OF 2

SEAL

FLOOR PLAN

SCALE: 3/16" = 1'-0"



SHEET:
A - 2
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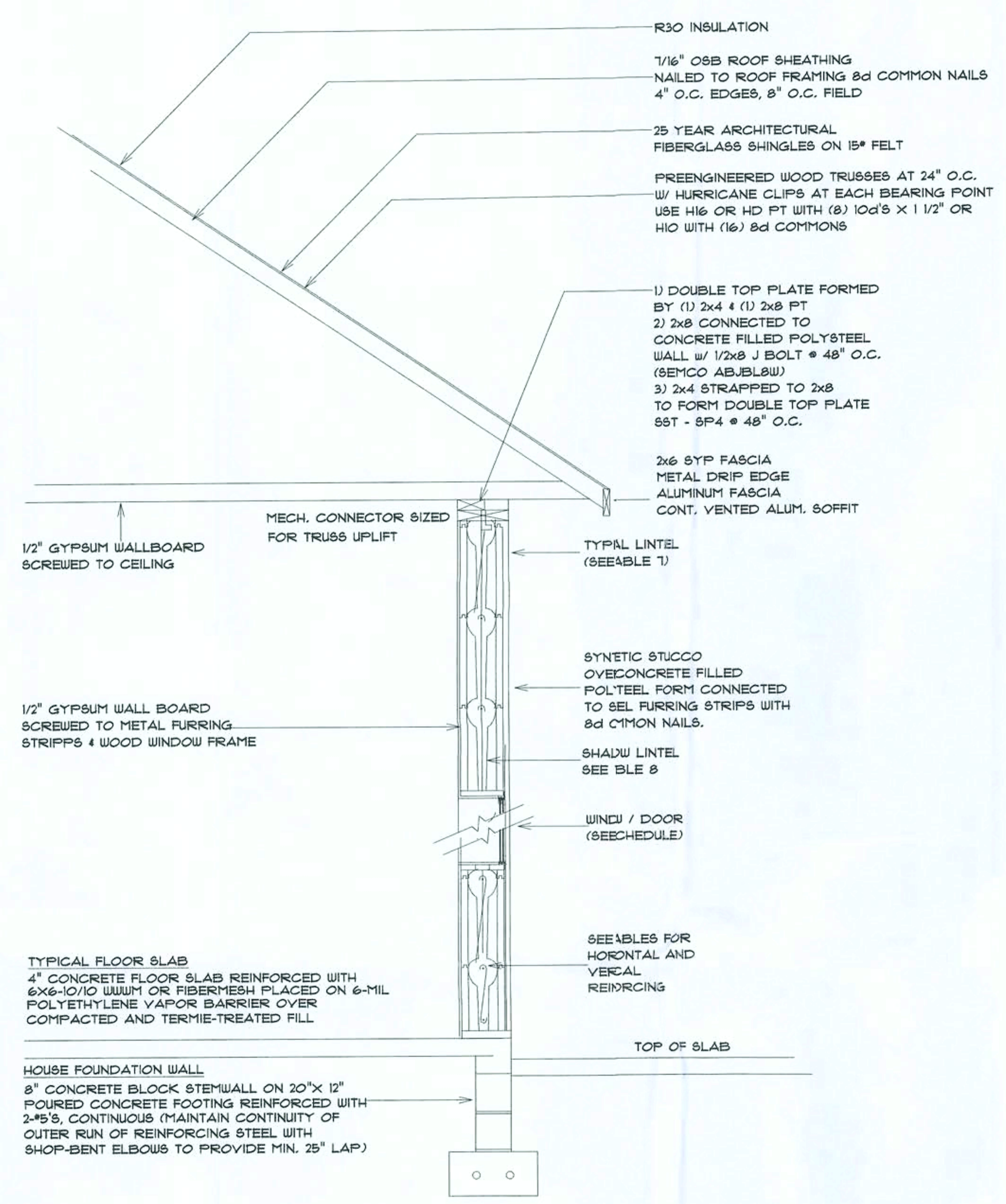


TYPICAL WALL SECTION

SHALLOW LINTEL MINIMUM REINFORCEMENT

TABLE 8

(LESS THAN ONE FULL 6 INCH POLYSTEEL FORM IN HEIGHT)

HEIGHT "H" OF POLYSTEEL LINTEL	LINTEL SPAN IN FEET							
	1.5	2	3	4	5	6	7	8
8 INCHES	#5	#5	#5	#5				
10 INCHES	#5	#5	#5	#5	#5	2-#5		
12 INCHES	#5	#5	#5	#5	#5	#5	2-#5	
14 INCHES	#5	#5	#5	#5	#5	#5	2-#5	2-#5

NOTES:

1. THIS TABLE IS FOR A FACTORED LOAD OF UP TO A MAXIMUM OF 2000 LBS. PER LINEAR FOOT OF LINTEL (U = 1.4 x DEAD LOADS + 1.7 LIVE LOADS)
2. RE-CREATE THE CONTINUOUS HORIZONTAL CONCRETE BEAM WHICH HAS BEEN CUT OFF. DO THIS BY REMOVING A PORTION OF THE BULKY PARTS OF THE EXPANDED POLYSTRENE AND THE EXPANDED STEEL (SEE SKETCH).
3. CENTER THE REQUESTED HORIZONTAL REBAR (AS SHOWN IN THE TABLE ABOVE) IN BOTH THE BOTTOM AND TOP HORIZONTAL CONCRETE BEAMS OF THE LINTEL FORM.
4. SHEAR REINFORCEMENT STIRRUPS ARE REQUIRED FOR ALL "SHALLOW" POLYSTEEL FORM LINTELS. SPACE STIRRUPS AT 6 INCHES ON CENTER OVER THE ENTIRE LINTEL SPAN.
5. THIS TABLE IS BASED ON 40,000 PSI REFORMED STEEL REBAR AND 3,000 PSI CONCRETE.
6. REFER TO TABLE 7 FOR POLYSTEEL FORM LINTELS WHICH ARE A FULL 16" IN HEIGHT.
7. REFER TO DRAWING "REINFORCEMENT AROUND OPENINGS" FOR OTHER REINFORCEMENT REQUIRED AROUND OPENINGS.

6" AND 8" POLYSTEEL 16" DEEP LINTEL - TABLE 7

GIVEN GRADE 60 REBAR (f_y = 60,000 psi)

LOAD "U" PER FOOT OF LINTEL	1.5	2	3	4	5	6	8	10	12	14	16	18
300	#5 (0)	#5 (0)	#5 (0)	#5 (0)	#5 (0)	#5 (2)	#5 (2)	#5 (2)	#5 (2)	#5 (2)	#5 (2)	#5 (2)
500	#5 (0)	#5 (0)	#5 (0)	#5 (0)	#5 (0)	#5 (2)	#5 (2)	#5 (2)	#5 (2)	#5 (2)	#5 (2)	2-#5 (2)
1000	#5 (0)	#5 (0)	#5 (1)	#5 (1)	#5 (2)	#5 (2)	#5 (2)	#5 (2)	#5 (2)	#5 (2)	2-#5 (2)	2-#5 (2)
1500	#5 (0)	#5 (1)	#5 (1)	#5 (1)	#5 (1)	#5 (1)	#5 (2)	#5 (2)	2-#5 (2)	2-#5 (2)	N	N
2000	#5 (1)	#5 (1)	#5 (1)	#5 (1)	#5 (2)	#5 (2)	#5 (2)	2-#5 (2)	N	N	N	N

NOTES:

1. U = 1.4 x DEAD LOADS + 1.7 x LIVE LOADS.
2. CENTER THE REQUIRED HORIZONTAL REBAR (AS SHOWN IN THE TABLE ABOVE) IN THE BOTTOM HORIZONTAL CORES OF THE FORM LINTEL.
3. THE HORIZONTAL REBAR MUST EXTEND 24" BEYOND EACH SIDE OF THE OPENING.
4. REFER TO DRAWINGS "REINFORCEMENT" AROUND OPENINGS" FOR OTHER REINFORCING REQUIRED AROUND OPENINGS
5. THIS TABLE IS BASED ON 3000 PSI CONCRETE
6. N = NOT RECOMENDED, DESIGN BY A STRUCTURAL ENGINEER IS REQUIRED
7 (0) = #4 VERTICAL REBAR IS REQUIRED AT 24" O.C.
8. (1) = #3 VERTICLE STIRRUPS ARE REQUIRED @ 12" O.C. (ONE IN EACH VERTICAL CORE)
9. (2) = #3 VERTICLE STIRRUPS ARE REQUIRED @ 6" O.C. (TWO IN EACH VERTICLE CORE)
10. SEE THE DRAWING ENTITLED "TYPICAL LINTEL STIRRUPS FOR PROPER STIRRUP INSTALLATION
11. (1) - #7 REBAR MAY BE SUBSTITUTED FOR (2)-#5

GENERAL NOTES

1. THIS STRUCTURE UTILIZES POLYSTEEL ICF WALL SYSTEM IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
2. ALL CONCRETE SHALL BE 3000 PSI
3. ALL REINFORCING STEEL SHALL BE GRADE 60.

EXTREME WIND REINFORCEMENT

GIVEN GRADE 60 REBAR (f_y = 60,000 psi)

FOR 1-STORY (OR TOP OF 2-STORY)

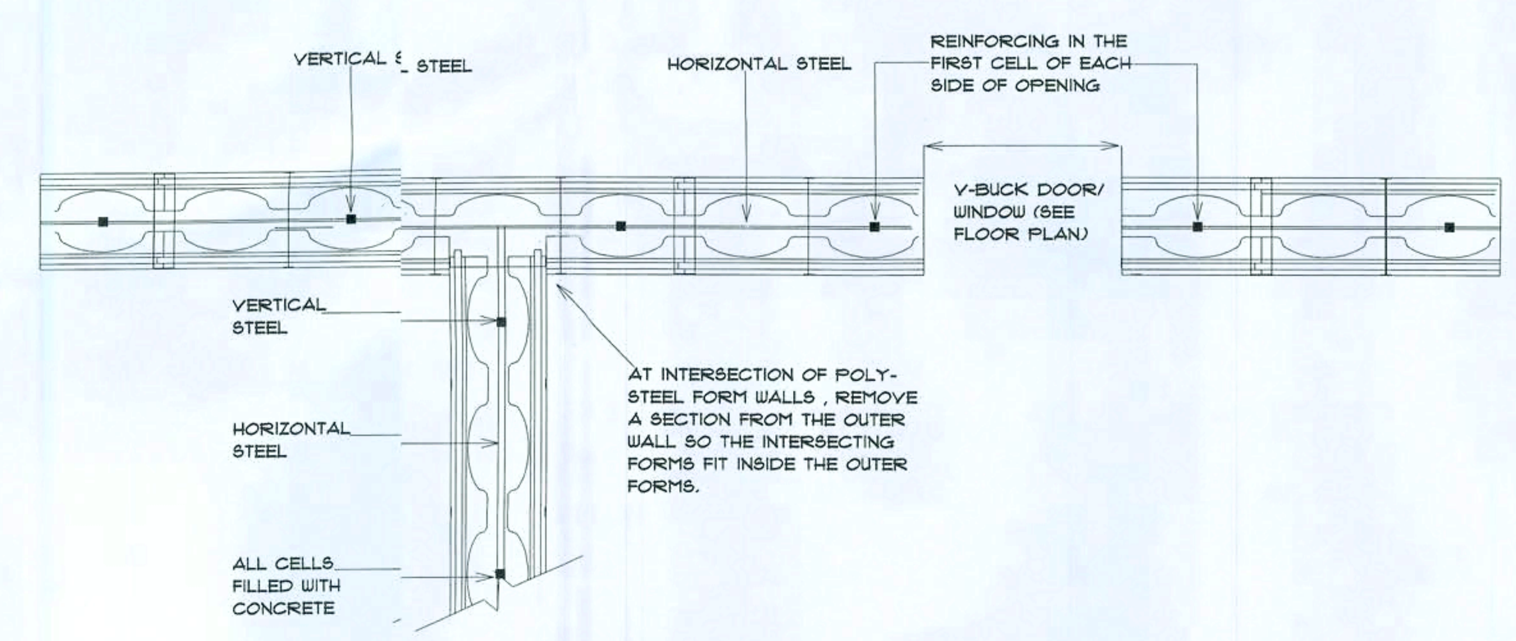
MAXIMUM DESIGN PRESSURE "P"	WIND SPEED MPH	6 INCH FORMS SEISMIC ZONES 0, 1, 2A, 2B	8 INCH FORMS SEISMIC ZONES 0, 1, 2A, 2B
64.2	120	V = #4 @ 24" O.C. H = #4 @ 32" O.C.	V = #4 @ 24" O.C. H = #4 @ 32" O.C.
75.3	130	V = #4 @ 24" O.C. H = #4 @ 32" O.C.	V = #4 @ 24" O.C. H = #4 @ 32" O.C.
100.2	150	V = #4 @ 12" O.C. H = #4 @ 32" O.C.	V = #4 @ 24" O.C. H = #4 @ 32" O.C.
178.2	200	V = #4 @ 12" O.C. H = #4 @ 32" O.C.	V = #4 @ 24" O.C. H = #4 @ 32" O.C.

FOR BOTTOM STORY OF 2-STORY (NOT FOR BASEMENTS)

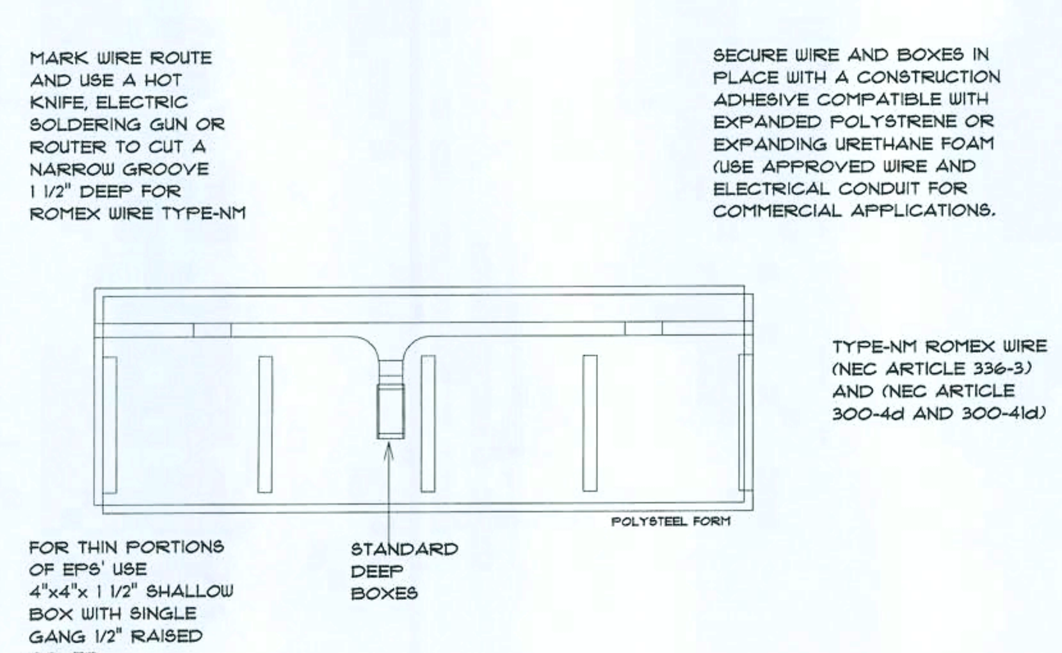
MAXIMUM DESIGN PRESSURE "P"	WIND SPEED MPH	6 INCH FORMS SEISMIC ZONES 0, 1, 2A, 2B	8 INCH FORMS SEISMIC ZONES 0, 1, 2A, 2B
64.2	120	V = #4 @ 24" O.C. H = #4 @ 32" O.C.	V = #4 @ 24" O.C. H = #4 @ 32" O.C.
75.3	130	V = #4 @ 24" O.C. H = #4 @ 32" O.C.	V = #4 @ 24" O.C. H = #4 @ 32" O.C.
100.2	150	V = #4 @ 24" O.C. H = #4 @ 32" O.C.	V = #4 @ 24" O.C. H = #4 @ 32" O.C.
178.2	200	V = #4 @ 12" O.C. H = #4 @ 32" O.C.	V = #4 @ 24" O.C. H = #4 @ 32" O.C.

NOTES:

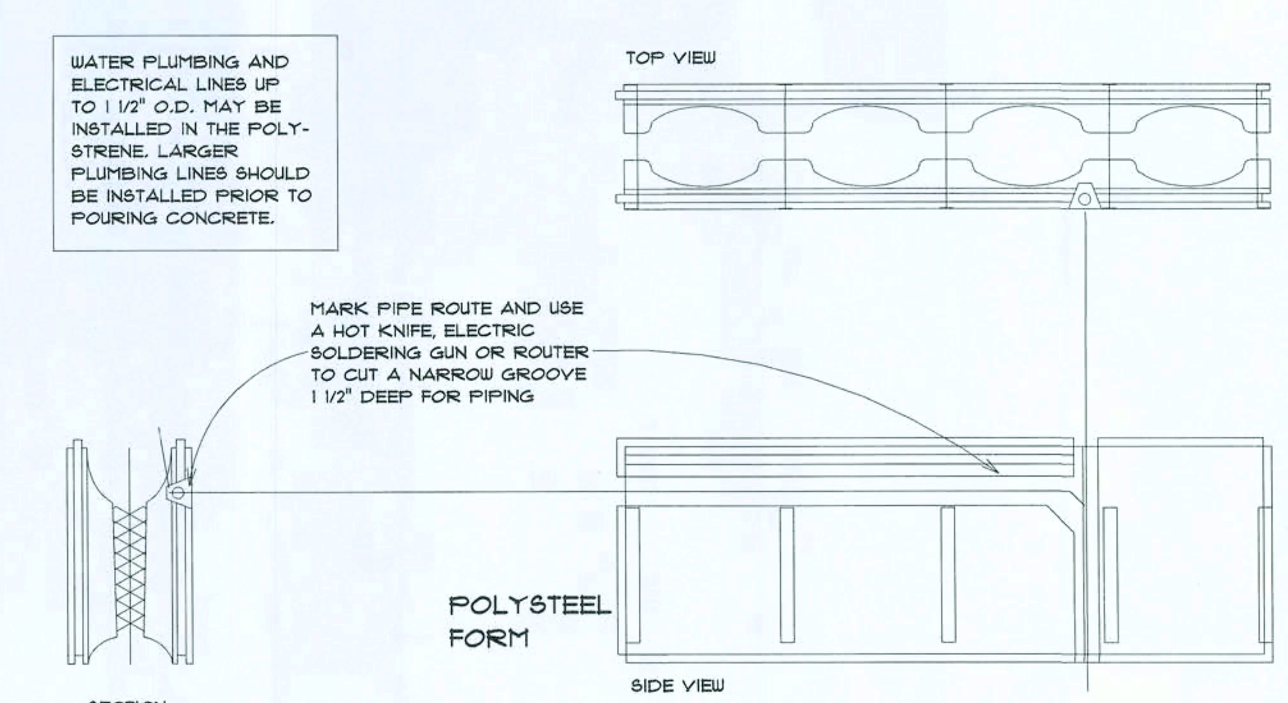
1. V= VERTICAL REINFORCEMENT. H= HORIZONTAL REINFORCEMENT.
2. TABLE REQUIRES A MINIMUM OF 3,000 PSI CONCRETE.
3. TABLE REQUIRES GRADE 60 (60,000 PSI) REINFORCING STEEL.
4. TABLE ASSUMES UNSUPPORTED WALL HEIGHTS DO NOT EXCEED 10 FEET FROM FLOOR TO CEILING.
5. TABLE ASSUMES A ROOF LIVE LOAD OF 20 LBS./SQ.FT. AND A FLOOR LIVE LOAD OF 40 LBS./SQ.FT.
6. TABLE ASSUMES ROOF AND FLOOR DEAD LOADS OF 15 LBS./SQ.FT.
7. TABLE ASSUMES MAXIMUM ROOF SPANS OF 60 FEET AND FLOOR SPANS OF 32 FEET.
8. WHERE OPENINGS ARE REQUIRED, ADDITIONAL REBAR SHOULD BE ADDED PER DESIGN AND CODE REQUIREMENTS.
9. ROOF AND FLOOR DIAPHRAGMS SHALL BE DESIGNED TO WITHSTAND THESE EXTREME LOADS.



TYPICAL WALL - TOP VIEW



TYPICAL ELECTRICAL INSTALLATION



TYPICAL PLUMBING INSTALLATION

SECTIONS PLAN

SCALE: 3/16" = 1'-0"

ADDITION FOR: STOINOFF

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SHEET:

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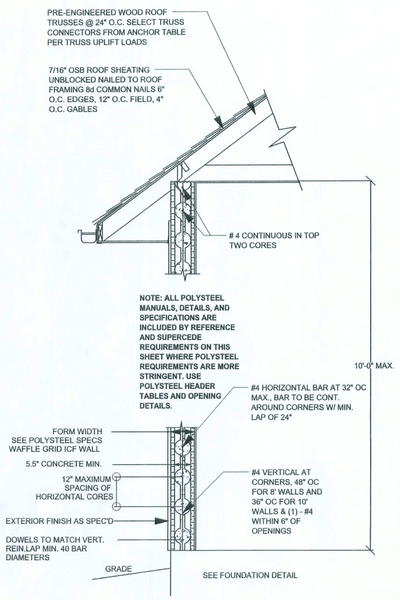
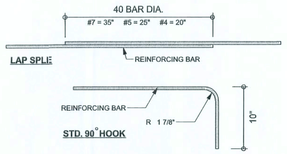
1 OF 1

SEAL





REBAR DETAILS

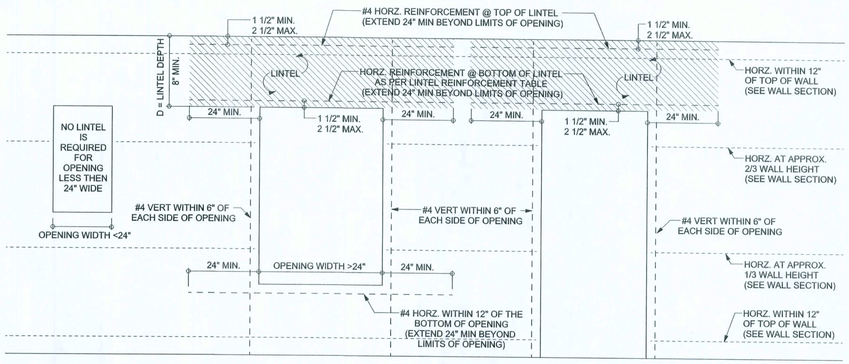
SCALE: N.T.S.

MAX. ALLOWABLE CLEAR SPANS FOR ICF LINTELS WITHOUT STIRRUPS IN GABLE END (NON-LOAD-BEARING WALLS) (#4 BOTTOM LINTEL REINFORCEMENT)		
MIN. LINTEL DEPTH, D	SUPPORTING FRAME GABLE END WALL ONLY	SUPPORTING ICF SECOND STORY & GABLE END WALL
8"	16'-3"	4'-4"
12"	16'-3"	7'-0"
16"	16'-3"	9'-7"
20"	16'-3"	12'-0"
22"	16'-3"	14'-3"

MIMIMUM BOTTOM BAR ICF LINTEL REINFORCEMENT FOR LARGE CLEAR SPANS WITH STIRRUPS IN LOAD-BEARING WALL				
MAX. LINTEL CLEAR SPAN	MIN. LINTEL DEPTH	SUPPORTING FRAME ROOF ONLY	SUPPORTING FRAME 2nd STORY & ROOF	SUPPORTING ICF 2nd STORY & FRAME ROOF
		MAX. ROOF / FLOOR CLEAR SPAN = 34'		
12'-3"	20"	1-#6 ; 2-#4		
	24"	1-#5	2-#5	2-#6
16'-3"	24"	2-#5		

MAX. ALLOWABLE CLEAR SPANS FOR ICF LINTELS WITHOUT STIRRUPS IN LOAD-BEARING WALLS (#4 BOTTOM LINTEL REINFORCEMENT)									
MIN. LINTEL DEPTH, D	SUPPORTING FRAME ROOF ONLY			SUPPORTING FRAME 2nd STORY & ROOF			SUPPORTING ICF 2nd STORY & FRAME ROOF		
	MAX. ROOF / FLOOR CLEAR SPAN								
	24'	28'	34'	24'	28'	34'	24'	28'	34'
8"	3'-10"	2'-11"	2'-5"	2'-9"	2'-7"	2'-6"	2'-9"	2'-7"	2'-6"
12"	5'-1"	4'-10"	4'-7"	4'-8"	4'-5"	4'-3"	4'-2"	4'-0"	3'-10"
16"	7'-0"	7'-5"	6'-5"	5'-1"	4'-10"	4'-8"	4'-8"	4'-5"	4'-3"
20"	8'-11"	8'-6"	8'-2"	6'-7"	6'-3"	6'-0"	5'-11"	5'-8"	5'-5"
24"	10'-7"	10'-1"	9'-8"	8'-5"	7'-8"	7'-4"	7'-2"	6'-10"	6'-7"

MAX. ALLOWABLE CLEAR SPANS FOR ICF LINTELS WITH STIRRUPS IN LOAD-BEARING WALLS (#4 BOTTOM LINTEL REINFORCEMENT)									
ALLOWABLE LINTELS CLEAR SPANS: 4'-8" / 1'-5" — MIDDLE PORTION OF SPAN WHERE STIRRUPS ARE NOT REQUIRED									
MIN. LINTEL DEPTH, D	SUPPORTING FRAME ROOF ONLY			SUPPORTING FRAME 2nd STORY & ROOF			SUPPORTING ICF 2nd STORY & FRAME ROOF		
	MAX. ROOF / FLOOR CLEAR SPAN								
	24'	28'	34'	24'	28'	34'	24'	28'	34'
8"	5'-8" / 2'-2"	5'-5" / 2'-0"	5'-2" / 1'-10"	4'-2" / 1'-2"	4'-0" / 1'-1"	3'-10" / 1'-0"	3'-9" / 1'-0"	3'-7" / 11"	3'-5" / 10"
12"	7'-4" / 3'-7"	7'-0" / 3'-3"	6'-8" / 3'-0"	5'-6" / 2'-0"	5'-3" / 1'-10"	5'-0" / 1'-8"	4'-11" / 1'-7"	4'-8" / 1'-5"	4'-6" / 1'-4"
16"	8'-7" / 4'-10"	8'-2" / 4'-5"	7'-10" / 4'-1"	6'-7" / 2'-6"	6'-3" / 2'-6"	6'-0" / 2'-4"	5'-10" / 2'-3"	5'-7" / 2'-1"	5'-4" / 1'-11"
20"	9'-8" / 6'-3"	9'-3" / 5'-9"	8'-10" / 5'-3"	7'-5" / 3'-7"	7'-1" / 3'-3"	6'-9" / 3'-0"	6'-8" / 2'-11"	6'-4" / 2'-7"	6'-1" / 2'-5"
24"	10'-7" / 7'-8"	10'-1" / 6'-1"	9'-8" / 6'-3"	8'-1" / 4'-4"	7'-9" / 4'-0"	7'-5" / 3'-8"	7'-4" / 3'-6"	7'-0" / 3'-2"	6'-8" / 2'-11"



TYPICAL OPENING / LINTEL DETAIL

SCALE: 1/2" = 1'=0"

W49 - SINGLE STORY ICF
SCREEN-GRID WALL SECTION
SCALE: 1/2"=1'-0" REV-28-Jun-04

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R302.1.2 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 48 * DB (30" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

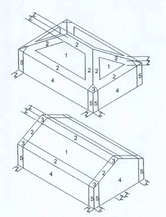
NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)
BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION
1.) BASIC WIND SPEED = 110 MPH
2.) WIND EXPOSURE = B
3.) WIND IMPORTANCE FACTOR = 1.0
4.) BUILDING CATEGORY = II
5.) ROOF ANGLE = 10-45 DEGREES
6.) MEAN ROOF HEIGHT = <30 FT
7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



Zone	Effective Wind Area (ft2)			
	10		100	
1	19.9	21.8	18.1	-18.1
2	19.9	25.5	18.1	-21.8
2 Ohg		40.6		-40.6
3	19.9	28.5	18.1	-21.8
3 Ohg		68.3		-42.4
4	21.8	23.6	18.5	-20.4
5	21.8	29.1	18.5	-22.6
Doors & Windows Worst Case (Zone 5, 10 ft2)		21.8		-29.1
8x7 Garage Door		19.5		-22.9
16x7 Garage Door		18.5		-21.0

DESIGN LOADS	
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)	
SOIL BEARING CAPACITY 1000PSF	
NOT IN FLOOD ZONE (BUILDER TO VERIFY)	

REVISIONS	

SOFTWAN
ARCHITECTURAL/DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Discoway, PE No.53915, POB 868 Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Discoway, P.E. fo resolution. Do not proceed withoutclarification.

CERTIFICATION: I herby certify that I have examined this plan, andhat the applicable portions of the plan, relding to wind engineering comply with section R31.2.1, florida building code residential 2004, t the best of my knowledge.

LIMITATION: This desin is valid for one building, at specified loation.

MARK DBOSWAY
P.E. 3915

SEAL

Crawford Stanley

Stoinoff Addition

ADDESS:
Lot 2 The Beadows of
Tuskenogee S/D
Columbia County, Florida

Mark Discsway P.E.
P.O. Bx 868
Lake City, Forida 32056
Phone: (386) 754 - 5419
Fax: (386)269 - 4871

PRINTED DATE:
May 08, 2008

DRAWN BY:

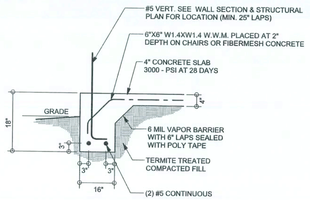
STRUCTURAL BY:
Ben Sparks

FINALS DATE:
7 / May / 08

JOB NUMBER:
80-281

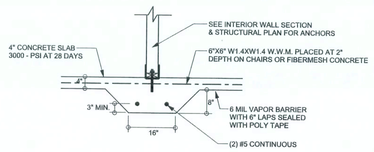
DRAWINGNUMBER
S-1

OF 3 SHEETS



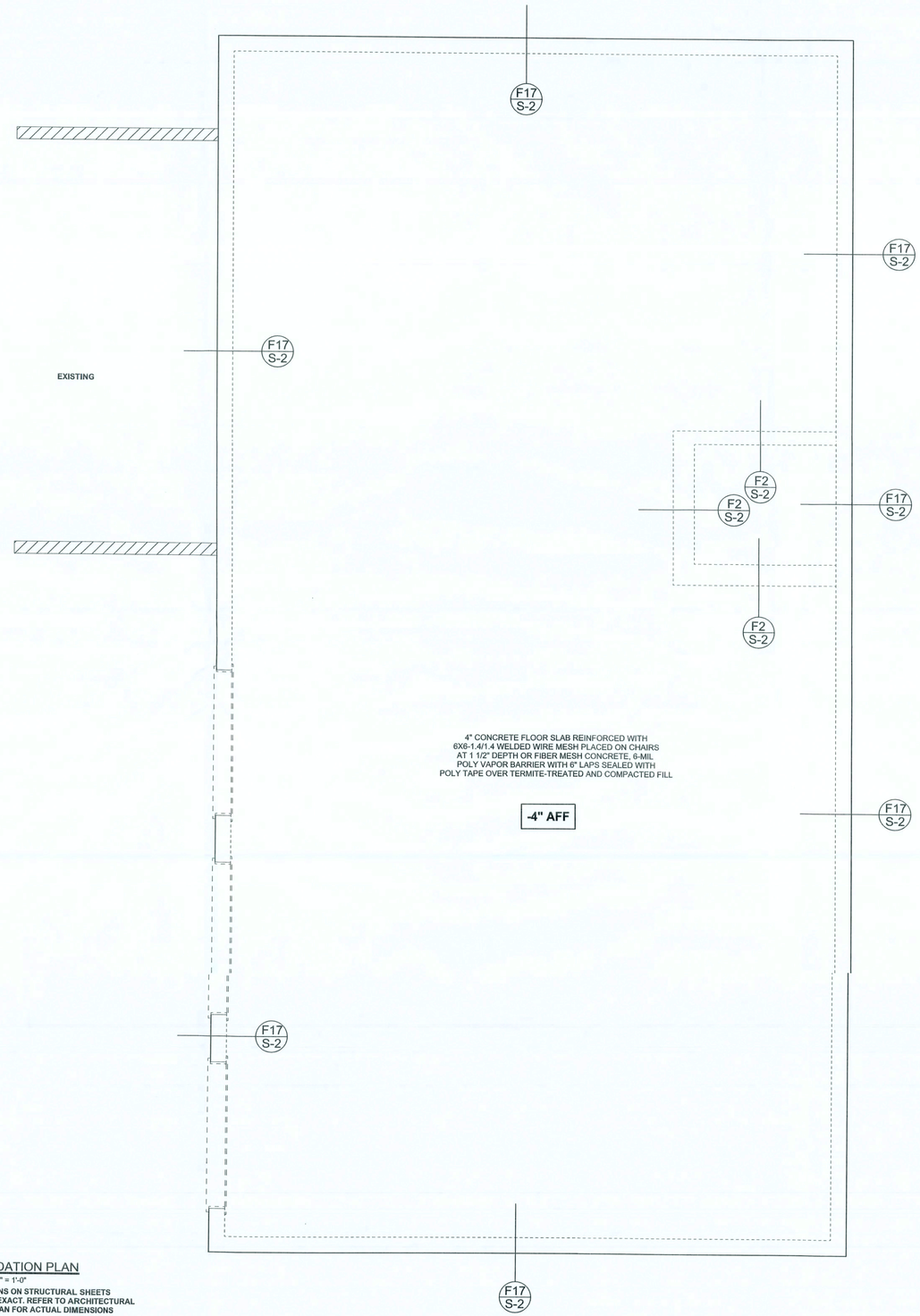
F17 S-2 MONOLITHIC FOOTING

SCALE: NTS



F2 S-2 INTERIOR BEARING FOOTING

SCALE: 1/2" = 1'-0"



FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

REVISIONS	

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY
P.E. 53915

SEAL

Crawford Stanley

Stoinoff Addition

ADDRESS:
Lot 2 The Meadows of Tuskenoogee S/D
Columbia County, Florida

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Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE: May 08, 2008

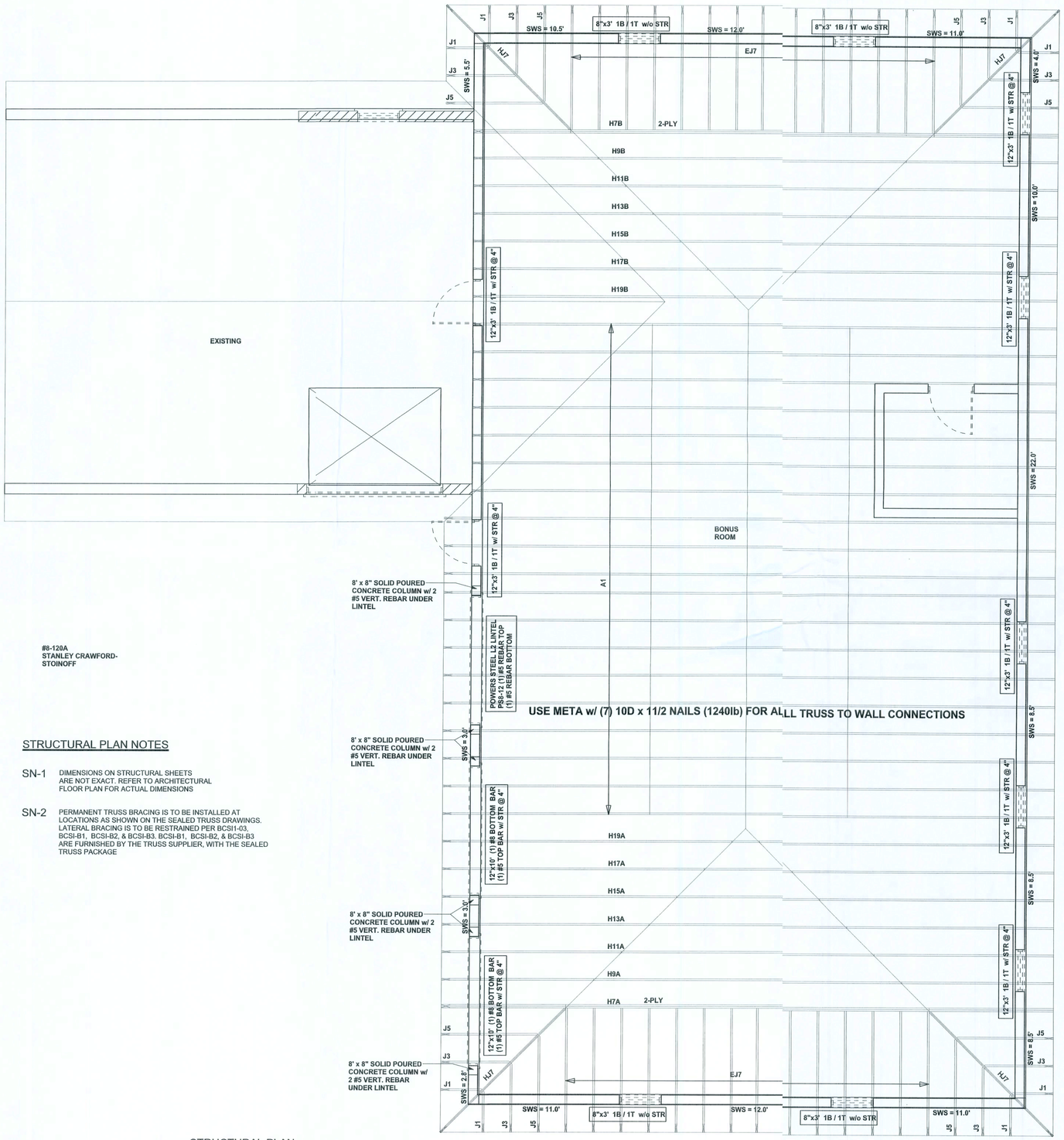
DRAWN BY: | STRUCTURAL BY: Ben Sparks

FINALS DATE: 7 / May / 08

JOB NUMBER: **804281**

DRAWING NUMBER **S-2**

OF 3 SHEETS



#8-120A
STANLEY CRAWFORD-
STOINOFF

STRUCTURAL PLAN NOTES

SN-1 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

SN-2 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

WALL LEGEND

SWS = 0.0'	1ST FLOOR ICF EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

LINTEL LEGEND

16"x6' 1B / 1T w/ STR @ 4"
- LINTEL CALL-OUT (U.N.O.)
- SPACING OF STIRRUPS IF STIRRUPS ARE REQUIRED
- w/ STR = #3 STIRRUPS REQUIRED w/o STR = NO STIRRUPS REQUIRED
- QUANTITY OF #5 HORZ. REBAR AT THE TOP OF LINTEL
- QUANTITY OF #5 HORZ. REBAR AT THE BOTTOM OF LINTEL
- SPAN OF LINTEL
- MIN. LINTEL DEPTH (INCHES)

TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	7.12'	67.5'
LONGITUDINAL	6.0'	75.8'

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY ANDERSON TRUSS CO. JOB 8-120A

REVISIONS

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R01.2.1, florida building code residential 2004, o the best of my knowledge.

LIMITATION: This design is valid for one building, at specified lcation.

MARK CSOSWAY
P.E.53915
08MAY08
SJAL

Crawford Stanley

Stoinoff Addition

ADRESS:
Lot 2 The Meadows of Tuskenogee S/D
Columbia County, Florida

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PRINTED DATE: May 08, 2008

DRAWN BY:

STRUCTURAL BY: Ben Sparks

FINALS DATE: 7 / May / 08

JOB NUMBER: 80-281

DRAWING NUMBER S-3

OF 3 SHEETS