DATE 02/16/2006

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT
000024145

APPLICANT GREG TALLEY PHONE 352.214.1055

ADDRESS 292 SW ARROWBEND DRIVE LAKE CITY FL 32024

OWNER GREG TALLEY PHONE 352.214.1055

ADDRESS 292 SW ARROWBEND DRIVE LAKE CITY FL 32024

CONTRACTOR G&J BLDRS,GEORGE ROHNER PHONE 352.214.1055

LOCATION OF PROPERTY 47-S TO C-242,TR TO CANNON CREEK DR., TL TO GERALD CONNER DR
ARROWBEND DR,TL, 3RD LOT BACK FROM CUL-DE-SAC.

TYPE DEVELOPMENT SFD/UTILITY ESTIMATED COST OF CONSTRUCTION 92800.00

HEATED FLOOR AREA 1856.00 TOTAL AREA 2419.00 HEIGHT 18.40 STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 6'12 FLOOR CONC

LAND USE & ZONING RSF-2 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 0 FLOOD ZONE XPP DEVELOPMENT PERMIT NO.

PARCEL ID 24-4S-16-03114-133 SUBDIVISION CANNON CREEK PLACE

LOT 33 BLOCK PHASE UNIT TOTAL ACRES 0.50

000000975		CGC021619	Greg Talley	
Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor	
18"X32'MITERED	06-0088-N	BLK	JTH	N
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: PLAT REQUIRES 1ST.FLOOR ELEVATION OF 97'. ELEVATION LETTER REQUIRED
NOC ON FILE.

Check # or Cash 1063

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by | Foundation _____ date/app. by | Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by | Slab _____ date/app. by | Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by | Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by | Heat & Air Duct _____ date/app. by | Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by | C.O. Final _____ date/app. by | Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by | Pool _____ date/app. by

Reconnection _____ date/app. by | Pump pole _____ date/app. by | Utility Pole _____ date/app. by

M/H Pole _____ date/app. by | Travel Trailer _____ date/app. by | Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 465.00 CERTIFICATION FEE $ 12.10 SURCHARGE FEE $ 12.10

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ _____ FLOOD ZONE FEE $ MC CULVERT FEE $ 25.00 TOTAL FEE 564.20

INSPECTORS OFFICE _____ CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

For Office Use Only Application # 0602-14 Date Received 2-6-06 By CH Permit # 24145/975

Application Approved by - Zoning Official BLK Date 15.02.06 Plans Examiner OK JTH Date 2-13-06

Flood Zone X Per PLAT Development Permit N/A Zoning RSF-2 Land Use Plan Map Category RES. Low Dens.

Comments Plat Requires 1st Floor Elevation of 97' Elevation Letter Required.

~~site plan~~, NOC

Applicants Name Greg Talley Phone 352-214-1055

Address

Owners Name Greg Talley Phone

911 Address 292 SW. ARROWBEND DR.

Contractors Name G & J Builders, Inc Phone ~~386~~ 352 214-1055

Address 21221 NW 238 Ave Highsprings 32643

Fee Simple Owner Name & Address NONE

Bonding Co. Name & Address NONE

Architect/Engineer Name & Address William Myers P.O. Box 1513 Lake city Fl.

Mortgage Lenders Name & Address NONE

Circle the correct power company – FL Power & Light – Clay Elec. (circled) – Suwannee Valley Elec. – Progressive Energy

Property ID Number R24-4S-16-03114-133 Estimated Cost of Construction 80,000

Subdivision Name Cannon Creek place Lot 33 Block ____ Unit ____ Phase ____

Driving Directions 47 S, (R) 242, (R) Cannon Creek Dr., (L) SW Gerald Conner Dr. (L) Arrowhead Dr., 3rd lot Back from Cul-de-sac.

Type of Construction New SFD Number of Existing Dwellings on Property NONE

Total Acreage .05 Lot Size ____ Do you need a Culvert Permit (circled) or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front ~~30~~ 30 Side ~~[illegible]~~ 32.8 Side ~~[illegible]~~ 32.8 Rear 120

Total Building Height ~~[illegible]~~ 18'4" Number of Stories 1 Heated Floor Area 1856 Roof Pitch 6/12

Porch 69 Garage 494 TOTAL 2419

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

(CK# 1063)

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

<u>WARNING TO OWNER:</u> YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

[signature]
Owner Builder or Agent (Including Contractor)

[signature]
Contractor Signature

Contractors License Number CGC021619

Competency Card Number ____

NOTARY STAMP/SEAL

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me

this 10 day of January 2006.

Personally known ✓ or Produced Identification ____

[signature]
Notary Signature

ROBERTA SABELLA
MY COMMISSION # DD267351
EXPIRES: January 13, 2008
1-800-3-NOTARY FL Notary Discount Assoc. Co.

~ JW left message . . 2.16.06 ~

Columbia County Property Appraiser

DB Last Updated: 9/16/2005

2005 Proposed Values

Parcel: 24-4S-16-03114-133 Tax Record Property Card Interactive GIS Map Print

Owner & Property Info

Search Result: 1 of 1

Owner's Name	G & J BUILDERS, INC.
Site Address	ARROWBEND
Mailing Address	21221 NW 238TH AVE. HIGH SPRINGS, FL 32643
Brief Legal	LOT 33 CANNON CREEK PLACE S/D. WD 1055-1339.

Use Desc. (code)	VACANT (000000)
Neighborhood	24416.00
Tax District	2
UD Codes	MKTA06
Market Area	06
Total Land Area	0.550 ACRES

Property & Assessment Values

Mkt Land Value	cnt: (1)	$36,000.00
Ag Land Value	cnt: (0)	$0.00
Building Value	cnt: (0)	$0.00
XFOB Value	cnt: (0)	$0.00
Total Appraised Value		$36,000.00

Just Value	$36,000.00
Class Value	$0.00
Assessed Value	$36,000.00
Exempt Value	$0.00
Total Taxable Value	$36,000.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale Vimp	Sale Qual	Sale RCode	Sale Price
8/15/2005	1055/1339	WD	V	Q		$79,800.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

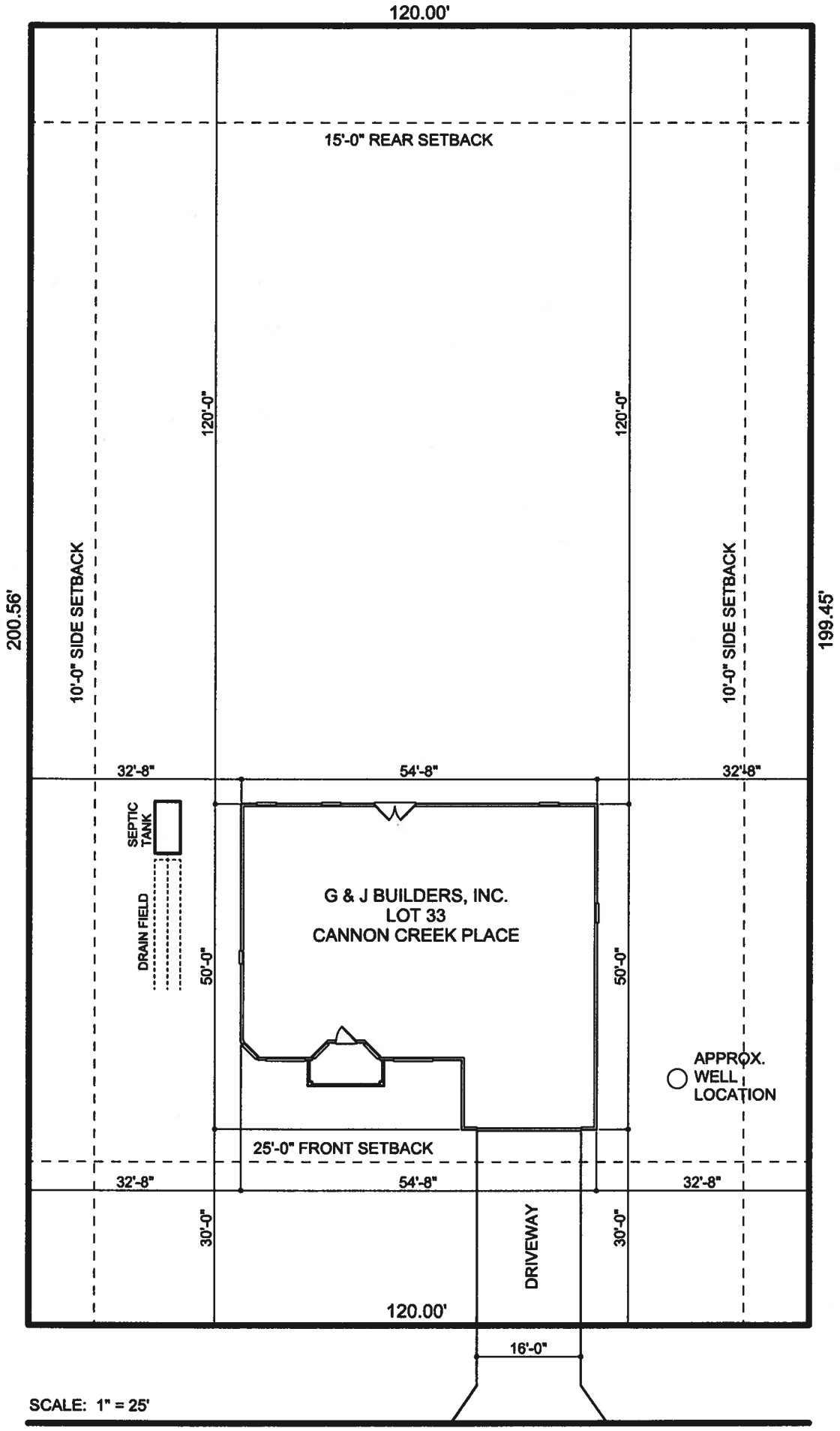
Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000000	VAC RES (MKT)	1.000 LT - (.550AC)	1.00/1.00/1.00/1.00	$36,000.00	$36,000.00

Columbia County Property Appraiser DB Last Updated: 9/16/2005

1 of 1

Disclaimer


120.00'
15'-0" REAR SETBACK
200.56'
199.45'
10'-0" SIDE SETBACK
10'-0" SIDE SETBACK
120'-0"
120'-0"
32'-8"
54'-8"
32'-8"
SEPTIC TANK
DRAIN FIELD
G & J BUILDERS, INC.
LOT 33
CANNON CREEK PLACE
50'-0"
50'-0"
APPROX. WELL LOCATION
25'-0" FRONT SETBACK
32'-8"
54'-8"
32'-8"
30'-0"
DRIVEWAY
30'-0"
120.00'
16'-0"
SCALE: 1" = 25'
SW ARROW DRIVE

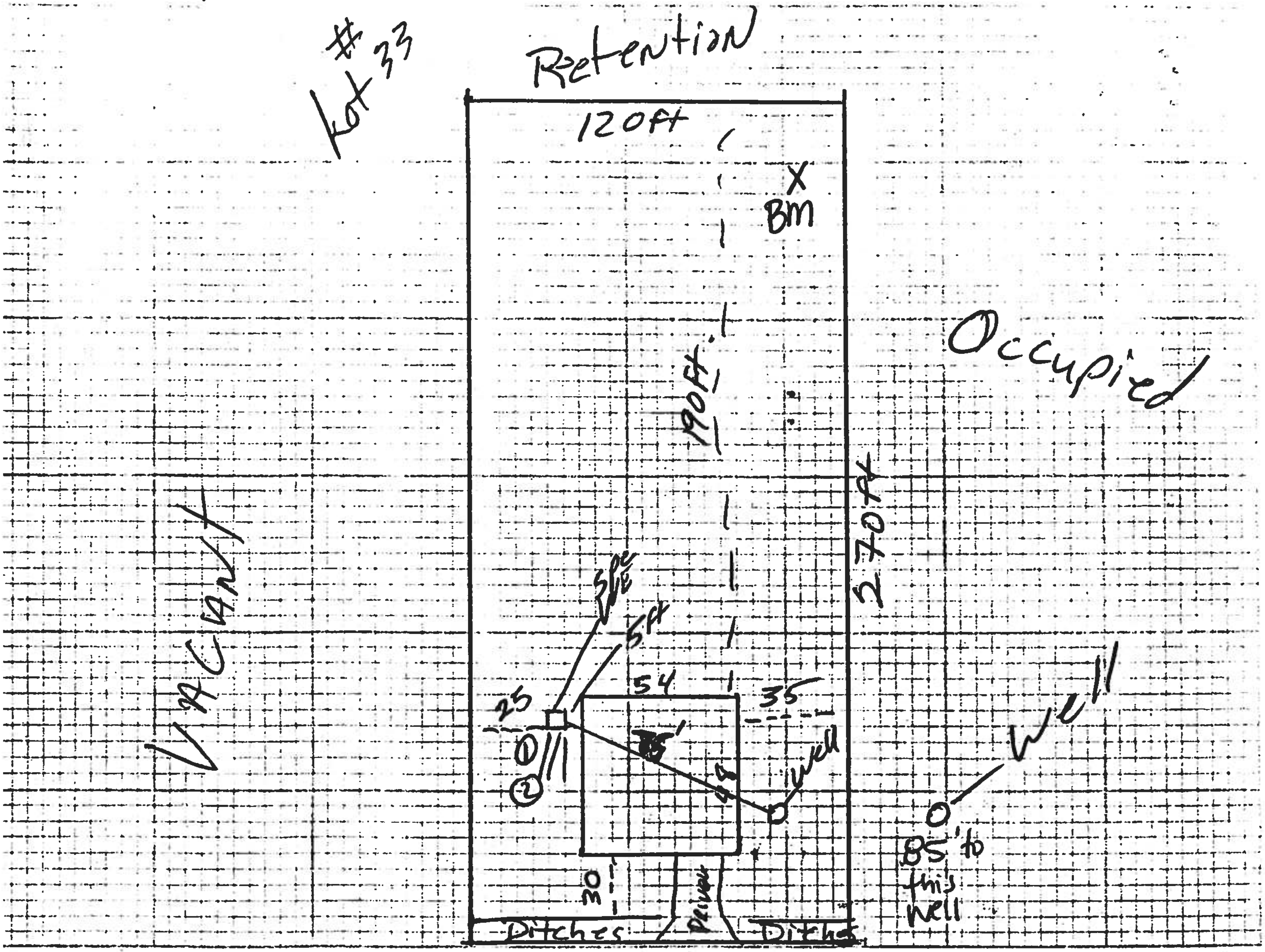
DEPARTMENT OF HEALTH

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 06-0088N

PART II - SITE PLAN

Scale: Each block represents 5 feet and 1 inch = 50 feet.



Notes:

Site Plan submitted by: J. Tall [signature] OWER

Signature Title

Plan Approved X Not Approved ______ Date 2-2-06

By Sallie Gaddy. ESI- COLUMBIA County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

Columbia County Building Department
Culvert Permit

Culvert Permit No.
000000975

DATE 02/16/2006 PARCEL ID # 24-4S-16-03114-133

APPLICANT GREG TALLEY PHONE 352-214-1055

ADDRESS 292 SW ARR0WBEND DRIVE LAKE CITY FL 32024

OWNER GREG TALLEY PHONE 352.214-1055

ADDRESS 292 SW ARROWBEND DRIVE LAKE CITY FL 32024

CONTRACTOR GREG ROHNER. G&J BLDRS,INC. PHONE 352.214.1055

LOCATION OF PROPERTY 47-S TO C-242,TR TO CANNON CREEK DR,TL TO GERALD CONNER TO ARROWBEND DR,TL AMD T'S 3RD LOT BACK FROM CUL-DE-SAC

SUBDIVISION/LOT/BLOCK/PHASE/UNIT CANNON CREEK PLACE 33

SIGNATURE

INSTALLATION REQUIREMENTS

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

[ ] Department of Transportation Permit installation approved standards.

[ ] Other

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00

COLUMBIA COUNTY GATEWAY TO FLORIDA

NOTICE OF COMMENCEMENT FORM
COLUMBIA COUNTY, FLORIDA

THIS DOCUMENT MUST BE RECORDED AT THE COUNTY CLERKS OFFICE BEFORE YOUR FIRST INSPECTION.

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

Tax Parcel ID Number 24-4S-16-03164-133 PERMIT NUMBER 24145

1. Description of property: (legal description of the property and street address or 911 address)
292 SW ARROW BEND DR.
LAKE CITY 32024

2. General description of improvement: SFD

3. Owner Name & Address 292 SW ARROW BEND
Interest in Property 100%

4. Name & Address of Fee Simple Owner (if other than owner):

5. Contractor Name G & J Builder INC Phone Number 352-214-1055
Address 21221 NW 238 AVE Highspring S

6. Surety Holders Name Phone Number
Address
Amount of Bond

7. Lender Name Phone Number
Address

8. Persons within the State of Florida designated by the Owner upon whom notices or other documents may be served as provided by section 718.13 (1)(a) 7; Florida Statutes:
Name Phone Number
Address

9. In addition to himself/herself the owner designates of
to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) –
(a) 7. Phone Number of the designee

10. Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording, (Unless a different date is specified)

NOTICE AS PER CHAPTER 713, Florida Statutes:
The owner must sign the notice of commencement and no one else may be permitted to sign in his/her stead.

Signature of Owner

Sworn to (or affirmed) and subscribed before 16th February, 2006

GALE TEDDER
MY COMMISSION EXPIRES June 29, 2009
Bonded Thru Notary Public Underwriters

NOTARY STAMP/SEAL

Signature of Notary

Inst:2006003887 Date:02/16/2006 Time:13:43
D.C.,P.DeWitt Cason,Columbia County B:1074 P:999

Land Surveyors
and Mappers

BRITT SURVEYING
830 West Duval Street • Lake City, FL 32055
Phone (386) 752-7163 • Fax (386) 752-5573

24145

03/02/06

L-17069A

ORIGINAL
COPY

To Whom It May Concern:

C/o: Greg Talley

Re: Lot 33 of Cannon Creek Place

The elevation of the foundation is found to be 97.58 feet. The minimum floor elevation shown on the plat of record is 97.00 feet. The highest adjacent grade is 95.12 feet and the lowest adjacent grade is 94.69 feet. The elevations shown hereon are based on NGVD 29 datum.

L. Scott Britt
PLS #5757

FORM 600A-2004 Tested sealed ducts must be certified in this house. EnergyGauge® 4.1

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**Greg Talley**	Builder:	**Greg Talley**
Address:	**Lot: 33, Sub: Cannon Creek, Plat:**	Permitting Office:	**Columbia County**
City, State:	**Lake City, Fl 32025-**	Permit Number:	24145
Owner:	**Spec House**	Jurisdiction Number:	221000
Climate Zone:	**North**		

1. New construction or existing	New		
2. Single family or multi-family	Single family		
3. Number of units, if multi-family	1		
4. Number of Bedrooms	3		
5. Is this a worst case?	No		
6. Conditioned floor area (ft²)	1856 ft²		
7. Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)			
a. U-factor: (or Single or Double DEFAULT)	7a. (Dble Default) 211.0 ft²		
b. SHGC: (or Clear or Tint DEFAULT)	7b. (Clear) 211.0 ft²		
8. Floor types			
a. Slab-On-Grade Edge Insulation	R=0.0, 188.0(p) ft		
b. N/A			
c. N/A			
9. Wall types			
a. Frame, Wood, Exterior	R=13.0, 1033.0 ft²		
b. Frame, Wood, Adjacent	R=13.0, 220.0 ft²		
c. N/A			
d. N/A			
e. N/A			
10. Ceiling types			
a. Under Attic	R=30.0, 1856.0 ft²		
b. N/A			
c. N/A			
11. Ducts(Leak Free)			
a. Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 50.0 ft		
b. N/A			
12. Cooling systems			
a. Central Unit		Cap: 35.0 kBtu/hr	
		SEER: 11.00	
b. N/A			
c. N/A			
13. Heating systems			
a. Electric Heat Pump		Cap: 35.0 kBtu/hr	
		HSPF: 6.80	
b. N/A			
c. N/A			
14. Hot water systems			
a. Electric Resistance		Cap: 50.0 gallons	
		EF: 0.90	
b. N/A			
c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)			
15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)			

Glass/Floor Area: 0.11 Total as-built points: 24140 Total base points: 26632 **PASS**

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: William Myers

DATE: 1.31.08

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 33, Sub: Cannon Creek, Plat: , Lake City, Fl, 32025-** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	1856.0	20.04	6695.0	Double, Clear	W	1.5	8.0	45.0	38.52	0.96	1660.9	
				Double, Clear	W	1.5	8.0	40.0	38.52	0.96	1476.4	
				Double, Clear	N	1.5	8.0	15.0	19.20	0.97	278.6	
				Double, Clear	E	1.5	8.0	60.0	42.06	0.96	2416.6	
				Double, Clear	SE	3.5	8.0	30.0	42.75	0.72	917.2	
				Double, Clear	NE	3.5	8.0	15.0	29.56	0.80	352.7	
				Double, Clear	S	1.5	8.0	6.0	35.87	0.92	198.7	
				As-Built Total:				**211.0**			**7301.0**	
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Adjacent	220.0	0.70	154.0	Frame, Wood, Exterior			13.0	1033.0	1.50		1549.5	
Exterior	1033.0	1.70	1756.1	Frame, Wood, Adjacent			13.0	220.0	0.60		132.0	
Base Total:	**1253.0**		**1910.1**	**As-Built Total:**				**1253.0**			**1681.5**	
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points	
Adjacent	20.0	1.60	32.0	Exterior Insulated				20.0	4.10		82.0	
Exterior	20.0	4.10	82.0	Adjacent Insulated				20.0	1.60		32.0	
Base Total:	**40.0**		**114.0**	**As-Built Total:**				**40.0**			**114.0**	
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points	
Under Attic	1856.0	1.73	3210.9	Under Attic			30.0	1856.0	1.73 X 1.00		3210.9	
Base Total:	**1856.0**		**3210.9**	**As-Built Total:**				**1856.0**			**3210.9**	
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Slab	188.0(p)	-37.0	-6956.0	Slab-On-Grade Edge Insulation			0.0	188.0(p	-41.20		-7745.6	
Raised	0.0	0.00	0.0									
Base Total:			**-6956.0**	**As-Built Total:**				**188.0**			**-7745.6**	
INFILTRATION	Area X	BSPM =	Points					Area X	SPM =		Points	
	1856.0	10.21	18949.8					1856.0	10.21		18949.8	

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 33, Sub: Cannon Creek, Plat: , Lake City, Fl, 32025-** PERMIT #:

BASE			AS-BUILT					
Summer Base Points: 23923.7			**Summer As-Built Points: 23511.6**					
Total Summer Points	X System Multiplier	= Cooling Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points
			(sys 1: Central Unit 35000 btuh ,SEER/EFF(11.0) Ducts:Unc(S),Unc(R),Gar(AH),R6.0(INS)					
23923.7	0.4266	10205.9	23512 **23511.6**	1.00 **1.00**	(1.09 x 1.000 x 1.00) **1.090**	0.310 **0.310**	1.000 **1.000**	7951.6 **7951.6**

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 33, Sub: Cannon Creek, Plat: , Lake City, Fl, 32025-** PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points
.18	1856.0	12.74	4256.2	Double, Clear	W	1.5	8.0	45.0	20.73	1.01	943.2
				Double, Clear	W	1.5	8.0	40.0	20.73	1.01	838.4
				Double, Clear	N	1.5	8.0	15.0	24.58	1.00	369.0
				Double, Clear	E	1.5	8.0	60.0	18.79	1.02	1150.0
				Double, Clear	SE	3.5	8.0	30.0	14.71	1.29	570.7
				Double, Clear	NE	3.5	8.0	15.0	23.57	1.02	360.7
				Double, Clear	S	1.5	8.0	6.0	13.30	1.04	83.1
				As-Built Total:				**211.0**			**4314.9**
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points
Adjacent	220.0	3.60	792.0	Frame, Wood, Exterior			13.0	1033.0	3.40		3512.2
Exterior	1033.0	3.70	3822.1	Frame, Wood, Adjacent			13.0	220.0	3.30		726.0
Base Total:	**1253.0**		**4614.1**	**As-Built Total:**				**1253.0**			**4238.2**
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =		Points
Adjacent	20.0	8.00	160.0	Exterior Insulated				20.0	8.40		168.0
Exterior	20.0	8.40	168.0	Adjacent Insulated				20.0	8.00		160.0
Base Total:	**40.0**		**328.0**	**As-Built Total:**				**40.0**			**328.0**
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X	WCM =	Points
Under Attic	1856.0	2.05	3804.8	Under Attic			30.0	1856.0	2.05 X	1.00	3804.8
Base Total:	**1856.0**		**3804.8**	**As-Built Total:**				**1856.0**			**3804.8**
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points
Slab	188.0(p)	8.9	1673.2	Slab-On-Grade Edge Insulation			0.0	188.0(p	18.80		3534.4
Raised	0.0	0.00	0.0								
Base Total:			**1673.2**	**As-Built Total:**				**188.0**			**3534.4**
INFILTRATION	Area X	BWPM =	Points					Area X	WPM =		Points
	1856.0	-0.59	-1095.0					1856.0	-0.59		-1095.0

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 33, Sub: Cannon Creek, Plat: , Lake City, Fl, 32025-** PERMIT #:

BASE			AS-BUILT					
Winter Base Points:		**13581.2**	**Winter As-Built Points:**					**15125.3**
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
			(sys 1: Electric Heat Pump 35000 btuh ,EFF(6.8) Ducts:Unc(S),Unc(R),Gar(AH),R6.0					
13581.2	0.6274	8520.9	15125.3	1.00	1.069	0.501	1.000	8108.2

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 33, Sub: Cannon Creek, Plat: , Lake City, Fl, 32025-** PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X	Tank Ratio	X Multiplier	X Credit Multiplier	= Total
3		2635.00	7905.0	50.0	0.90	3		1.00	2693.56	1.00	8080.7
				As-Built Total:							**8080.7**

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
10206	**8521**	**7905**	**26632**	**7952**	**8108**	**8081**	**24140**

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 33, Sub: Cannon Creek, Plat: , Lake City, Fl, 32025-** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

Tested sealed ducts must be certified in this house.

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 84.9

The higher the score, the more efficient the home.

Spec House, Lot: 33, Sub: Cannon Creek, Plat: , Lake City, Fl, 32025-

1.	New construction or existing	New		12.	Cooling systems	
2.	Single family or multi-family	Single family			a. Central Unit	Cap: 35.0 kBtu/hr
3.	Number of units, if multi-family	1				SEER: 11.00
4.	Number of Bedrooms	3			b. N/A	
5.	Is this a worst case?	No				
6.	Conditioned floor area (ft²)	1856 ft²			c. N/A	
7.	Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)					
	a. U-factor:	Description	Area	13.	Heating systems	
	(or Single or Double DEFAULT)	7a. (Dble Default)	211.0 ft²		a. Electric Heat Pump	Cap: 35.0 kBtu/hr
	b. SHGC:					HSPF: 6.80
	(or Clear or Tint DEFAULT)	7b. (Clear)	211.0 ft²		b. N/A	
8.	Floor types					
	a. Slab-On-Grade Edge Insulation	R=0.0, 188.0(p) ft			c. N/A	
	b. N/A					
	c. N/A			14.	Hot water systems	
9.	Wall types				a. Electric Resistance	Cap: 50.0 gallons
	a. Frame, Wood, Exterior	R=13.0, 1033.0 ft²				EF: 0.90
	b. Frame, Wood, Adjacent	R=13.0, 220.0 ft²			b. N/A	
	c. N/A					
	d. N/A				c. Conservation credits	
	e. N/A				(HR-Heat recovery, Solar	
10.	Ceiling types				DHP-Dedicated heat pump)	
	a. Under Attic	R=30.0, 1856.0 ft²		15.	HVAC credits	
	b. N/A				(CF-Ceiling fan, CV-Cross ventilation,	
	c. N/A				HF-Whole house fan,	
11.	Ducts(Leak Free)				PT-Programmable Thermostat,	
	a. Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 50.0 ft			MZ-C-Multizone cooling,	
	b. N/A				MZ-H-Multizone heating)	

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

Energy Code Compliance

Duct System Performance Report

Project Name:	Greg Talley	Builder:	Greg Talley
Address:		Permitting Office:	Columbia County
City, State:	Lake City, Fl 32025-	Permit Number:	
Owner:	Spec House	Jurisdiction Number:	
Climate Zone:	North		

Total Duct System Leakage Test Results

CFM25 Total Duct Leakage Test Values			
Line	System	Duct Leakage Total	Duct Leakage to Outdoors
1	System1	_____ cfm25(tot)	_____ cfm25(out)
2	System2	_____ cfm25(tot)	_____ cfm25(out)
3	System3	_____ cfm25(tot)	_____ cfm25(out)
4	System4	_____ cfm25(tot)	_____ cfm25(out)
5	**Total House Duct System Leakage**	Sum lines 1-4 _____ Divide by _____ (Total Conditioned Floor Area) = _____ (Q_n,tot) ☐ Receive credit if Q_n,tot≤ 0.03	Sum lines 1-4 _____ Divide by _____ (Total Conditioned Floor Area) = _____ (Q_n,out) ☐ Receive credit if Q_n,out≤ 0.03 AND Q_n,tot≤ 0.09

I hereby certify that the above duct testing performance results demonstrate compliance with the Florida Energy Code requirements in accordance with Section 610.1.A.1, Florida Building Code, Building Volume, Chapter 13 for leak free duct system credit.

Signature: _____

Printed Name: _____

Florida Rater Certification #: _____

DATE: _____

Florida Building Code requires that testing to confirm leak free duct systems be performed by a Class 1 Florida Energy Gauge Certified Energy Rater. Certified Florida Class 1 raters can be found at: http://energygauge.com/search.htp

BUILDING OFFICIAL: _____

DATE: _____

Geoengineering & Testing, Inc. 2445

Geotechnical • Environmental • Construction Materials Testing

IN-PLACE DENSITY TEST RESULTS

CLIENT: Greg Talley

PROJECT: Cannon Creek Place Subdivision Lot 33
Lake City Florida

AREA TESTED: Fill Below Foundation

COURSE: Final Grade DEPTH OF TEST: 0-12"

TYPE OF TEST: ASTM DATE TESTED: 2-23-6

NOTE: The below tests DO/DO NOT meet the minimum 95 % compaction requirements of maximum density.

REMARKS: Good Compaction under Slab

SOIL DESCRIPTION: Arches Gold Sand

LOCATION OF TESTS	DRY DEN.	MAX. DEN.	% MAX. DEN.	% MOIST.	OPT. MOIST.
SW corner Under Slab	102.7	106.2	96.8	9.3	10.8
NW "	108.1		101.8	8.6	
NE "	109.1		102.8	8.3	
SE	102.5		96.6	6.8	

4404

TECH. Mike Maier

M. Fred Rwebyogo, PE
Florida Registration No. 46694

3402 NE 2nd Street, Suite A
Gainesville, Florida 32609

Phone (352) 375-7108
FAX (352) 336-7630

Notice of Treatment 1183Ø

Applicator: Florida Pest Control & Chemical Co. (www.flapest .com)
Address: BAYA AVE
City Lake City Phone 752-1703

Site Location: Subdivision Cannon Creek Place
Lot # 33 Block# Permit # 24145
Address 292 SW Arrow Bend DR

Product used	**Active Ingredient**	**% Concentration**
☑ Premise	Imidacloprid	0.1%
☐ Termidor	Fipronil	0.12%
☐ Bora-Care	Disodium Octaborate Tetrahydrate	23.0%

Type treatment: ☑ Soil ☐ Wood

Area Treated	**Square feet**	**Linear feet**	**Gallons Applied**
Dwelling (MONO)	2419	214	200

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line ________.

2-23-06	0830	F254 Gunny
Date	Time	Print Technician's Name

Remarks: ____________________

Applicator - White   Permit File - Canary   Permit Holder - Pink

10/05 ©

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File a Complaint
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User Services
Renew a License
Change License Status
Maintain Account
Change My Address
View Messages
Change My PIN
View Continuing Ed

Term Glossary
Online Help

Licensee Details

Licensee Information

Name:	**ROHNER, GEORGE JOSEPH (Primary Name)**
	R B K BUILDERS INC (DBA Name)
Main Address:	**3031 SW 108 WAY** **PO BOX 290023** **DAVIE Florida 33329-0023**
County:	**BROWARD**
License Mailing:	
LicenseLocation:	**3031 SW 108 WAY** **PO BOX 290023** **DAVIE FL 33329-0023**
County:	**BROWARD**

License Information

License Type:	**Certified General Contractor**
Rank:	**Cert General**
License Number:	**CGC021619**
Status:	**Current,Active**
Licensure Date:	**03/14/1982**
Expires:	**08/31/2006**
Special Qualifications	**Qualification Effective**
Bldg Code Core Course Credit	
Qualified Business License Required	**02/20/2004**

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23642

From: The Columbia County Building Department
Plans Review
135 NE Hernando Av.
P. O Box 1529
Lake City Florida, 32056-1529

Reference to: Build permit application Number: 0602-14

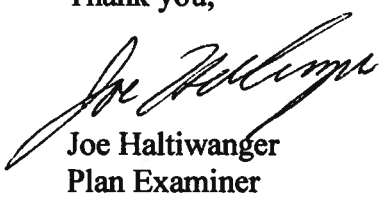
G & J Builders Owner Greg Talley lot 33 Cannon Creek Place

On the date of February 13, 2006 application 0602-14 and plans for construction of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0602-14 when making reference to this application.

1. Please have Mr. Nicholas Geisler supply the following information, show all required connectors with uplift rating for the truss system and required number and size of fasteners for continuous tie from the roof to foundation. These connection points shall be designed by an architect or engineer using the engineered roof truss plans.
2. Please submit a recorded (with the Columbia County Clerk Office) a notice of commencement before any inspections can be preformed by the Columbia County Building Department.

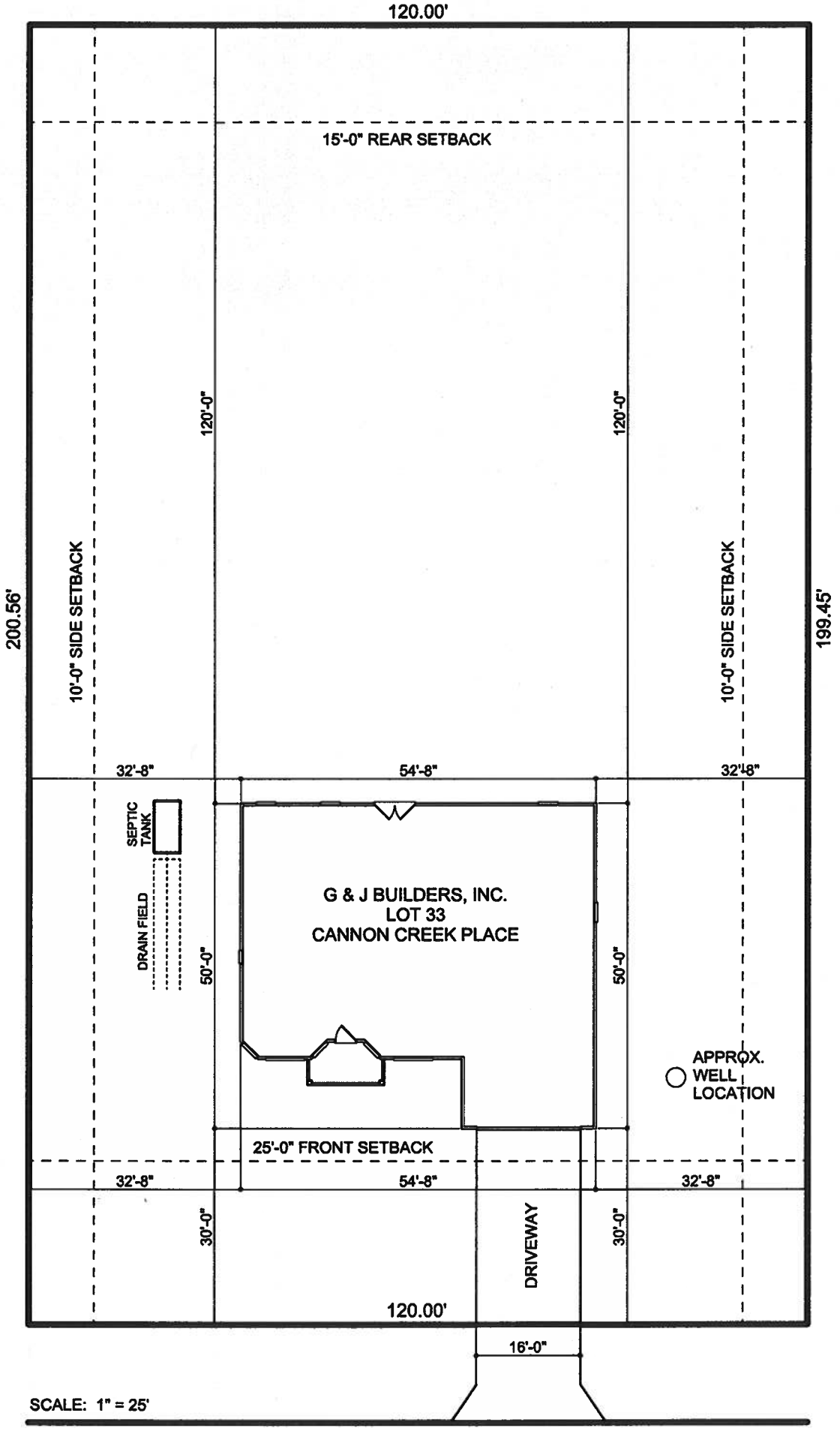
Thank you,



Joe Haltiwanger
Plan Examiner
Columbia County Building Department

Thank you,

Joe Haltiwanger
Plan Examiner
Columbia County Building Department


120.00'
15'-0" REAR SETBACK
200.56'
199.45'
10'-0" SIDE SETBACK
10'-0" SIDE SETBACK
120'-0"
120'-0"
32'-8"
54'-8"
32'-8"
SEPTIC TANK
DRAIN FIELD
G & J BUILDERS, INC.
LOT 33
CANNON CREEK PLACE
50'-0"
50'-0"
APPROX. WELL LOCATION
25'-0" FRONT SETBACK
32'-8"
54'-8"
32'-8"
30'-0"
30'-0"
DRIVEWAY
120.00'
16'-0"
SCALE: 1" = 25'
SW ARROW DRIVE

02-7-06

Randy, Harry, Joe, Janice, and Laurie

Please review and advise.

Select Year: 2005 Go

thanks John Kerce

The 2005 Florida Statutes

Title XXXII REGULATION OF PROFESSIONS AND OCCUPATIONS | Chapter 489 CONTRACTING | View Entire Chapter

489.111 Licensure by examination.--

(1) Any person who desires to be certified shall apply to the department in writing.

(2) A person shall be eligible for licensure by examination if the person:

(a) Is 18 years of age;

(b) Is of good moral character; and

(c) Meets eligibility requirements according to one of the following criteria:

1. Has received a baccalaureate degree from an accredited 4-year college in the appropriate field of engineering, architecture, or building construction and has 1 year of proven experience in the category in which the person seeks to qualify. For the purpose of this part, a minimum of 2,000 person-hours shall be used in determining full-time equivalency.

2. Has a total of at least 4 years of active experience as a worker who has learned the trade by serving an apprenticeship as a skilled worker who is able to command the rate of a mechanic in the particular trade or as a foreman who is in charge of a group of workers and usually is responsible to a superintendent or a contractor or his or her equivalent, provided, however, that at least 1 year of active experience shall be as a foreman.

3. Has a combination of not less than 1 year of experience as a foreman and not less than 3 years of credits for any accredited college-level courses; has a combination of not less than 1 year of experience as a skilled worker, 1 year of experience as a foreman, and not less than 2 years of credits for any accredited college-level courses; or has a combination of not less than 2 years of experience as a skilled worker, 1 year of experience as a foreman, and not less than 1 year of credits for any accredited college-level courses. All junior college or community college-level courses shall be considered accredited college-level courses.

4.a. An active certified residential contractor is eligible to take the building contractors' examination if he or she possesses a minimum of 3 years of proven experience in the classification in which he or she is certified.

b. An active certified residential contractor is eligible to take the general contractors' examination if he or she possesses a minimum of 4 years of proven experience in the classification in which he or she is certified.

c. An active certified building contractor is eligible to take the general contractors' examination if he or she possesses a minimum of 4 years of proven experience in the classification in which he or she is certified.

5.a. An active certified air-conditioning Class C contractor is eligible to take the air-conditioning Class B contractors' examination if he or she possesses a minimum of 3 years of proven experience in the classification in which he or she is certified.

b. An active certified air-conditioning Class C contractor is eligible to take the air-conditioning Class A contractors' examination if he or she possesses a minimum of 4 years of proven experience in the classification in which he or she is certified.

c. An active certified air-conditioning Class B contractor is eligible to take the air-conditioning Class A contractors' examination if he or she possesses a minimum of 1 year of proven experience in the classification in which he or she is certified.

6.a. An active certified swimming pool servicing contractor is eligible to take the residential swimming pool contractors' examination if he or she possesses a minimum of 3 years of proven experience in the classification in which he or she is certified.

b. An active certified swimming pool servicing contractor is eligible to take the swimming pool commercial contractors' examination if he or she possesses a minimum of 4 years of proven experience in the classification in which he or she is certified.

c. An active certified residential swimming pool contractor is eligible to take the commercial swimming pool contractors' examination if he or she possesses a minimum of 1 year of proven experience in the classification in which he or she is certified.

d. An applicant is eligible to take the swimming pool/spa servicing contractors' examination if he or she has satisfactorily completed 60 hours of instruction in courses related to the scope of work covered by that license and approved by the Construction Industry Licensing Board by rule and has at least 1 year of proven experience related to the scope of work of such a contractor.

(3)(a) The board may refuse to certify an applicant for failure to satisfy the requirement of good moral character only if:

1. There is a substantial connection between the lack of good moral character of the applicant and the professional responsibilities of a certified contractor; and

2. The finding by the board of lack of good moral character is supported by clear and convincing

evidence.

(b) When an applicant is found to be unqualified for a certificate because of a lack of good moral character, the board shall furnish the applicant a statement containing the findings of the board, a complete record of the evidence upon which the determination was based, and a notice of the rights of the applicant to a rehearing and appeal.

(4) The department shall ensure that a sensitivity review committee has been established including representatives of various ethnic/minority groups. No question found by this committee to be discriminatory against any ethnic/minority group shall be included in the examination.

History.--ss. 5, 17, ch. 79-200; s. 369, ch. 81-259; ss. 2, 3, ch. 81-318; ss. 6, 20, 21, ch. 88-156; s. 12, ch. 89-162; s. 4, ch. 91-429; s. 480, ch. 97-103; s. 5, ch. 97-228; s. 1, ch. 2001-117; s. 7, ch. 2002-392.

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 24-4S-16-03114-133 **Building permit No.** 000024145

Use Classification SFD/UTILITY **Fire:** 11.84

Permit Holder G&J BLDRS,GEORGE ROHNER **Waste:** 24.50

Owner of Building GREG TALLEY **Total:** 36.34

Location: 292 SW ARROWBEND DR (CANNON CREEK PLACE 33)

Date: 09/14/2006

Harry Dicks

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

Residential System Sizing Calculation

Summary

Spec House

Lake City, Fl 32025-

Project Title:
Greg Talley

Code Only
Professional Version
Climate: North

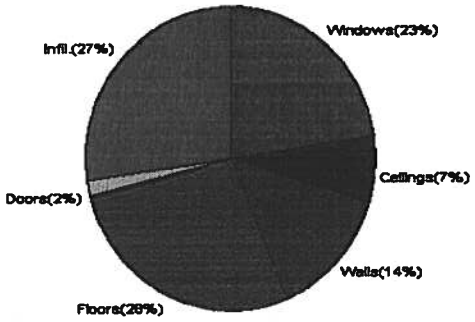
1/31/2006

Location for weather data: Gainesville - Defaults: Latitude(29) Altitude(152 ft.) Temp Range(M) Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(54gr.)					
Winter design temperature	33	F	Summer design temperature	92	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	37	F	Summer temperature difference	17	F
Total heating load calculation	**29839**	**Btuh**	**Total cooling load calculation**	**35397**	**Btuh**
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh
Total (Electric Heat Pump)	117.3	35000	Sensible (SHR = 0.75)	94.2	26250
Heat Pump + Auxiliary(0.0kW)	117.3	35000	Latent	116.2	8750
			Total (Electric Heat Pump)	98.9	35000

WINTER CALCULATIONS

Winter Heating Load (for 1856 sqft)

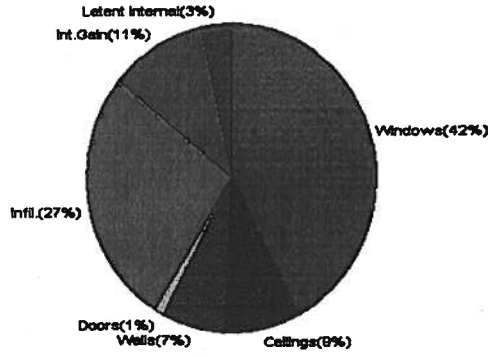
Load component			Load	
Window total	211	sqft	6792	Btuh
Wall total	1253	sqft	4115	Btuh
Door total	40	sqft	518	Btuh
Ceiling total	1856	sqft	2187	Btuh
Floor total	188	sqft	8208	Btuh
Infiltration	198	cfm	8019	Btuh
Duct loss			0	Btuh
Subtotal			**29839**	**Btuh**
Ventilation	0	cfm	0	Btuh
TOTAL HEAT LOSS			**29839**	**Btuh**



SUMMER CALCULATIONS

Summer Cooling Load (for 1856 sqft)

Load component			Load	
Window total	211	sqft	14910	Btuh
Wall total	1253	sqft	2487	Btuh
Door total	40	sqft	392	Btuh
Ceiling total	1856	sqft	3074	Btuh
Floor total			0	Btuh
Infiltration	173	cfm	3224	Btuh
Internal gain			3780	Btuh
Duct gain			0	Btuh
Sens. Ventilation	0	cfm	0	Btuh
Total sensible gain			**27866**	**Btuh**
Latent gain(ducts)			0	Btuh
Latent gain(infiltration)			6331	Btuh
Latent gain(ventilation)			0	Btuh
Latent gain(internal/occupants/other)			1200	Btuh
Total latent gain			**7531**	**Btuh**
TOTAL HEAT GAIN			**35397**	**Btuh**



For Florida residences only

EnergyGauge® System Sizing
PREPARED BY: _____
DATE: _____

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

Spec House
Lake City, Fl 32025-

Project Title:
Greg Talley

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

1/31/2006

Component Loads for Whole House

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	W	45.0	32.2	1449 Btuh
2	2, Clear, Metal, 0.87	W	40.0	32.2	1288 Btuh
3	2, Clear, Metal, 0.87	N	15.0	32.2	483 Btuh
4	2, Clear, Metal, 0.87	E	60.0	32.2	1931 Btuh
5	2, Clear, Metal, 0.87	SE	30.0	32.2	966 Btuh
6	2, Clear, Metal, 0.87	NE	15.0	32.2	483 Btuh
7	2, Clear, Metal, 0.87	S	6.0	32.2	193 Btuh
	Window Total		211(sqft)		6792 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1033	3.3	3392 Btuh
2	Frame - Wood - Adj(0.09)	13.0	220	3.3	722 Btuh
	Wall Total		1253		4115 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Adjacent		20	12.9	259 Btuh
2	Insulated - Exterior		20	12.9	259 Btuh
	Door Total		40		518Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin)	30.0	1856	1.2	2187 Btuh
	Ceiling Total		1856		2187Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab On Grade	0	188.0 ft(p)	43.7	8208 Btuh
	Floor Total		188		8208 Btuh
			Zone Envelope Subtotal:		21820 Btuh
Infiltration	Type	ACH X	Zone Volume	CFM=	
	Natural	0.80	14848	198.0	8019 Btuh
Ductload	Proposed leak free, R6.0, Supply(Attic), Return(Attic)			(DLM of 0.00)	0 Btuh
Zone #1			**Sensible Zone Subtotal**		**29839 Btuh**

WHOLE HOUSE TOTALS

	Subtotal Sensible	29839 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	29839 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Spec House

Lake City, Fl 32025-

Project Title:
Greg Talley

Code Only
Professional Version
Climate: North

1/24/2026

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

For Florida residences only

System Sizing Calculations - Winter

Residential Load - Room by Room Component Details

Spec House
Lake City, Fl 32025-

Project Title:
Greg Talley

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

1/31/2006

Component Loads for Zone #1: Main

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	W	45.0	32.2	1449 Btuh
2	2, Clear, Metal, 0.87	W	40.0	32.2	1288 Btuh
3	2, Clear, Metal, 0.87	N	15.0	32.2	483 Btuh
4	2, Clear, Metal, 0.87	E	60.0	32.2	1931 Btuh
5	2, Clear, Metal, 0.87	SE	30.0	32.2	966 Btuh
6	2, Clear, Metal, 0.87	NE	15.0	32.2	483 Btuh
7	2, Clear, Metal, 0.87	S	6.0	32.2	193 Btuh
	Window Total		211(sqft)		6792 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1033	3.3	3392 Btuh
2	Frame - Wood - Adj(0.09)	13.0	220	3.3	722 Btuh
	Wall Total		1253		4115 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Adjacent		20	12.9	259 Btuh
2	Insulated - Exterior		20	12.9	259 Btuh
	Door Total		40		518Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin)	30.0	1856	1.2	2187 Btuh
	Ceiling Total		1856		2187Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab On Grade	0	188.0 ft(p)	43.7	8208 Btuh
	Floor Total		188		8208 Btuh
			Zone Envelope Subtotal:		21820 Btuh
Infiltration	Type	ACH X	Zone Volume	CFM=	
	Natural	0.80	14848	198.0	8019 Btuh
Ductload	Proposed leak free, R6.0, Supply(Attic), Return(Attic)			(DLM of 0.00)	0 Btuh
Zone #1			**Sensible Zone Subtotal**		**29839 Btuh**

WHOLE HOUSE TOTALS

	Subtotal Sensible	29839 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	29839 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Spec House

Lake City, Fl 32025-

Project Title:
Greg Talley

Code Only
Professional Version
Climate: North

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

For Florida residences only

System Sizing Calculations - Summer

Residential Load - Whole House Component Details

Spec House

Lake City, Fl 32025-

Project Title:
Greg Talley

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F 1/31/2006

Component Loads for Whole House

Window	Type* Pn/SHGC/U/InSh/ExSh/IS	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load
1	2, Clear, 0.87, None,N,N	W	1.5ft	8ft.	45.0	0.0	45.0	29	80	3578 Btuh
2	2, Clear, 0.87, None,N,N	W	1.5ft	8ft.	40.0	0.0	40.0	29	80	3181 Btuh
3	2, Clear, 0.87, None,N,N	N	1.5ft	8ft.	15.0	0.0	15.0	29	29	434 Btuh
4	2, Clear, 0.87, None,N,N	E	1.5ft	8ft.	60.0	0.0	60.0	29	80	4771 Btuh
5	2, Clear, 0.87, None,N,N	SE	3.5ft	8ft.	30.0	17.3	12.7	29	63	1294 Btuh
6	2, Clear, 0.87, None,N,N	NE	3.5ft	8ft.	15.0	0.0	15.0	29	60	901 Btuh
7	2, Clear, 0.87, None,N,N	S	1.5ft	8ft.	6.0	6.0	0.0	29	34	174 Btuh
	Excursion									577 Btuh
	Window Total				211 (sqft)					14910 Btuh

Walls	Type	R-Value/U-Value	Area(sqft)	HTM	Load
1	Frame - Wood - Ext	13.0/0.09	1033.0	2.1	2155 Btuh
2	Frame - Wood - Adj	13.0/0.09	220.0	1.5	332 Btuh
	Wall Total		1253 (sqft)		2487 Btuh

Doors	Type	Area (sqft)	HTM	Load
1	Insulated - Adjacent	20.0	9.8	196 Btuh
2	Insulated - Exterior	20.0	9.8	196 Btuh
	Door Total	40 (sqft)		392 Btuh

Ceilings	Type/Color/Surface	R-Value	Area(sqft)	HTM	Load
1	Vented Attic/DarkShingle	30.0	1856.0	1.7	3074 Btuh
	Ceiling Total		1856 (sqft)		3074 Btuh

Floors	Type	R-Value	Size	HTM	Load
1	Slab On Grade	0.0	188 (ft(p))	0.0	0 Btuh
	Floor Total		188.0 (sqft)		0 Btuh

			Zone Envelope Subtotal:		20862 Btuh
Infiltration	Type	ACH	Volume(cuft)	CFM=	Load
	SensibleNatural	0.70	14848	173.2	3224 Btuh
Internal gain		Occupants	Btuh/occupant	Appliance	Load
		6	X 230 +	2400	3780 Btuh
Duct load	Proposed leak free, R6.0, Supply(Attic), Return(Attic)			DGM = 0.00	0.0 Btuh
			Sensible Zone Load		**27866 Btuh**

Manual J Summer Calculations

Residential Load - Component Details (continued)

Spec House

Lake City, Fl 32025-

Project Title:
Greg Talley

Code Only
Professional Version
Climate: North

1/31/2006

WHOLE HOUSE TOTALS

	Sensible Envelope Load All Zones	**27866 Btuh**
	Sensible Duct Load	0 Btuh
	Total Sensible Zone Loads	**27866 Btuh**
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
Whole House	**Total sensible gain**	**27866 Btuh**
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	6331 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	0 Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200 Btuh
	Latent other gain	0 Btuh
	Latent total gain	**7531 Btuh**
	TOTAL GAIN	**35397 Btuh**

*Key: Window types (Pn - Number of panes of glass)
(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))
(Ornt - compass orientation)

For Florida residences only

System Sizing Calculations - Summer

Residential Load - Room by Room Component Details

Spec House
Lake City, Fl 32025-

Project Title:
Greg Talley

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F 1/31/2006

Component Loads for Zone #1: Main

Window	Type* Pn/SHGC/U/InSh/ExSh/IS	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load
1	2, Clear, 0.87, None,N,N	W	1.5ft	8ft.	45.0	0.0	45.0	29	80	3578 Btuh
2	2, Clear, 0.87, None,N,N	W	1.5ft	8ft.	40.0	0.0	40.0	29	80	3181 Btuh
3	2, Clear, 0.87, None,N,N	N	1.5ft	8ft.	15.0	0.0	15.0	29	29	434 Btuh
4	2, Clear, 0.87, None,N,N	E	1.5ft	8ft.	60.0	0.0	60.0	29	80	4771 Btuh
5	2, Clear, 0.87, None,N,N	SE	3.5ft	8ft.	30.0	17.3	12.7	29	63	1294 Btuh
6	2, Clear, 0.87, None,N,N	NE	3.5ft	8ft.	15.0	0.0	15.0	29	60	901 Btuh
7	2, Clear, 0.87, None,N,N	S	1.5ft	8ft.	6.0	6.0	0.0	29	34	174 Btuh
	Excursion									577 Btuh
	Window Total				211 (sqft)					14910 Btuh

Walls	Type	R-Value/U-Value	Area(sqft)	HTM	Load
1	Frame - Wood - Ext	13.0/0.09	1033.0	2.1	2155 Btuh
2	Frame - Wood - Adj	13.0/0.09	220.0	1.5	332 Btuh
	Wall Total		1253 (sqft)		2487 Btuh

Doors	Type	Area (sqft)	HTM	Load
1	Insulated - Adjacent	20.0	9.8	196 Btuh
2	Insulated - Exterior	20.0	9.8	196 Btuh
	Door Total	40 (sqft)		392 Btuh

Ceilings	Type/Color/Surface	R-Value	Area(sqft)	HTM	Load
1	Vented Attic/DarkShingle	30.0	1856.0	1.7	3074 Btuh
	Ceiling Total		1856 (sqft)		3074 Btuh

Floors	Type	R-Value	Size	HTM	Load
1	Slab On Grade	0.0	188 (ft(p))	0.0	0 Btuh
	Floor Total		188.0 (sqft)		0 Btuh

			Zone Envelope Subtotal:		20862 Btuh
Infiltration	Type	ACH	Volume(cuft)	CFM=	Load
	SensibleNatural	0.70	14848	173.2	3224 Btuh
Internal gain		Occupants	Btuh/occupant	Appliance	Load
		6	X 230 +	2400	3780 Btuh
Duct load	Proposed leak free, R6.0, Supply(Attic), Return(Attic)			DGM = 0.00	0.0 Btuh
			Sensible Zone Load		**27866 Btuh**

Manual J Summer Calculations

Residential Load - Component Details (continued)

Spec House

Lake City, Fl 32025-

Project Title:
Greg Talley

Code Only
Professional Version
Climate: North

1/31/2006

WHOLE HOUSE TOTALS

	Sensible Envelope Load All Zones	**27866 Btuh**
	Sensible Duct Load	0 Btuh
	Total Sensible Zone Loads	**27866 Btuh**
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
Whole House	**Total sensible gain**	**27866 Btuh**
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	6331 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	0 Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200 Btuh
	Latent other gain	0 Btuh
	Latent total gain	**7531 Btuh**
	TOTAL GAIN	**35397 Btuh**

*Key: Window types (Pn - Number of panes of glass)
(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))
(Ornt - compass orientation)

For Florida residences only

Residential Window Diversity

MidSummer

Spec House

Lake City, Fl 32025-

Project Title:
Greg Talley

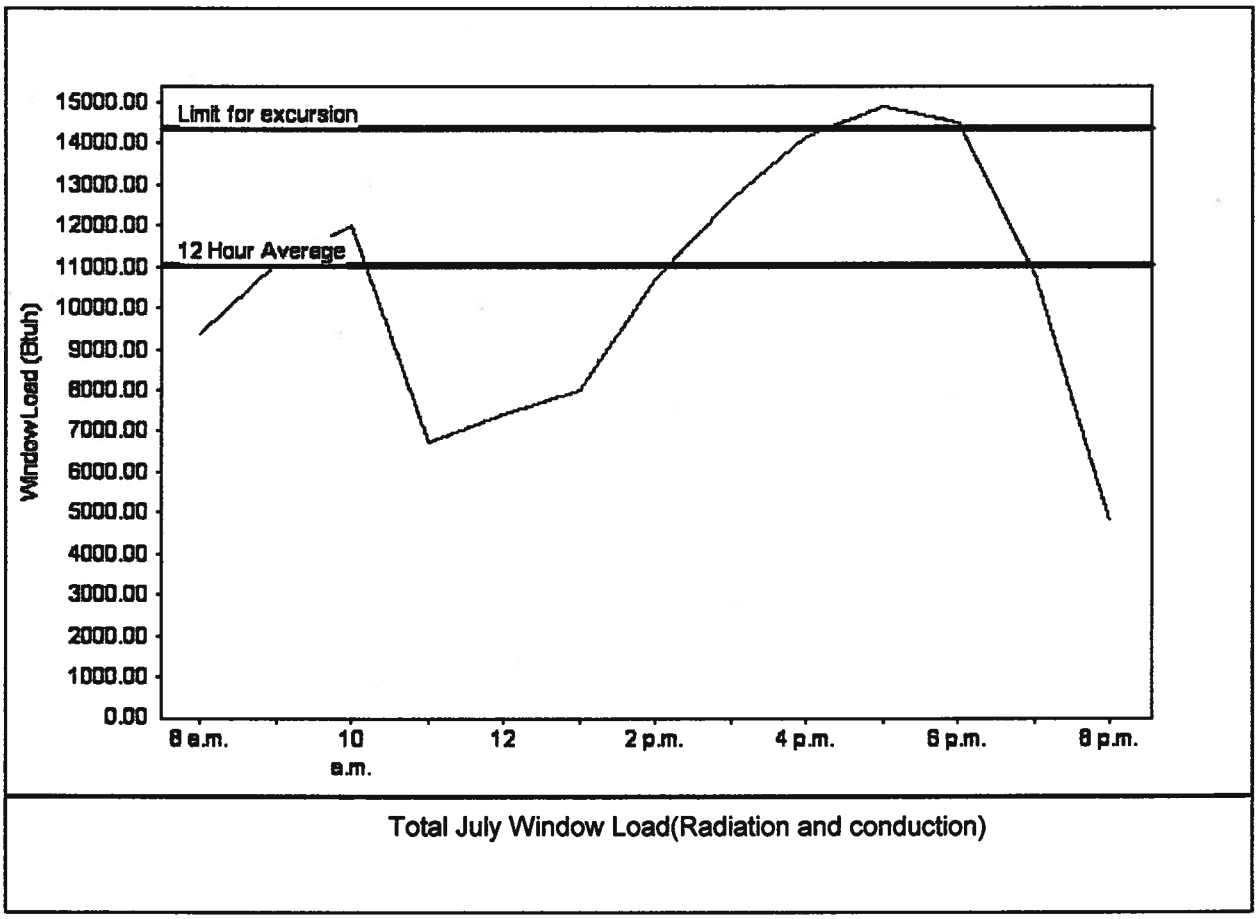
Code Only
Professional Version
Climate: North

1/31/2006

Weather data for: Gainesville - Defaults

Summer design temperature	92 F	Average window load for July	11033 Btu
Summer setpoint	75 F	Peak window load for July	14920 Btu
Summer temperature difference	17 F	Excusion limit(130% of Ave.)	14343 Btu
Latitude	29 North	Window excursion (July)	577 Btuh

WINDOW Average and Peak Loads



Total July Window Load(Radiation and conduction)

Warning: This application has glass areas that produce relatively large heat gains for part of the day. Variable air volume devices may be required to overcome spikes in solar gain for one or more rooms. A zoned system may be required or some rooms may require zone control.

EnergyGauge® System Sizing for Florida residences only

PREPARED BY: _____

DATE: _____

THE OVERHEAD DOOR
SINCE the original 1921

Overhead Door Company of Gainesville

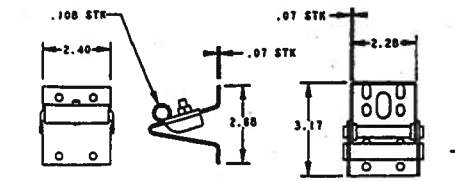
POST OFFICE BOX 568 • GAINESVILLE, FL 32602 • OFFICE (904) 468-2733 • ANS. SERVICE (904) 374-0802
A DIVISION OF FLORIDA OVERHEAD DOOR & SPECIALTIES, INC.

Please find enclosed approved 110mph Windload Drawings by Overhead Door Corp.for 8 & 9x 7's,16 x 7 and 18 x 7 281 Series Doors.

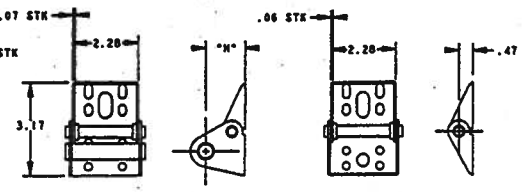
A copy of these drawings must be present on site as per Alachua County Code Enforcement.For drawings on other sizes and types of Doors,Please contact Overhead Door Company.

352-468-2733

Thank You
Bob Hartman



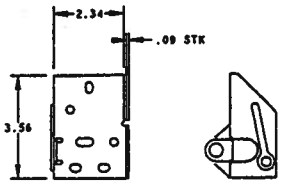
409808-0001 TOP FIXTURE



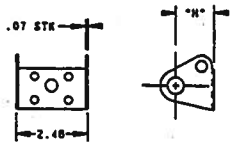
407605-XXXX END HINGE

P/N	"H"
407605-0002	.85
407605-0003	1.10
407605-0004	1.35
407605-0005	1.60

407603-0001 CTR HINGE

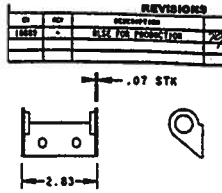


405771-XXXX BOTTOM FIXTURE

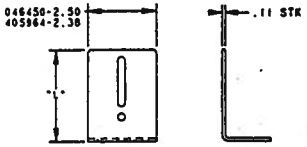


407604-XXXX HINGE LEAF, BTM

P/N	"H"
407604-0002	.85
407604-0003	1.10
407604-0004	1.35
407604-0005	1.60

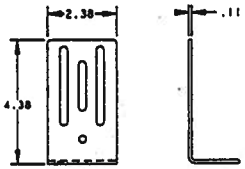


407532-1001 HINGE LEAF, BTM

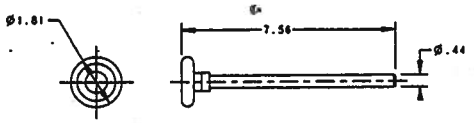


046450-0003 JAMB BRACKET
405964-0001 SCAB BRACKET

P/N	"L"
046450-0003	4.00
405964-0001	3.25

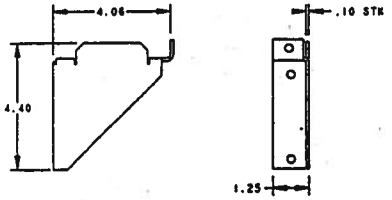


405964-0002 SCAB BRACKET

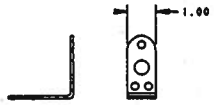


061166-0001 ROLLER ASSEMBLY, TR-3

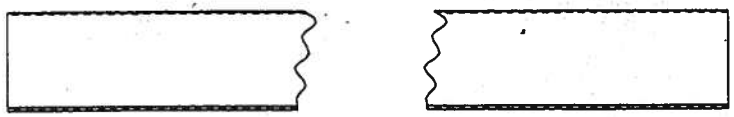
409898-XXXX DOUBLER, RAIL



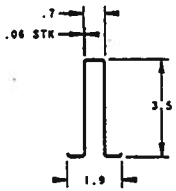
409863-XXXX CHANNEL SUPPORT

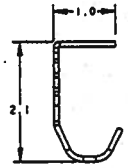


405742-0001 ROLLER BRKT



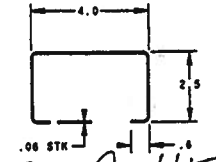
400650-XXX3 STRUT, HS-3





407743-XXX3
2" TRACK DETAIL
SCALE 1/1

409517-XXXX CHANNEL



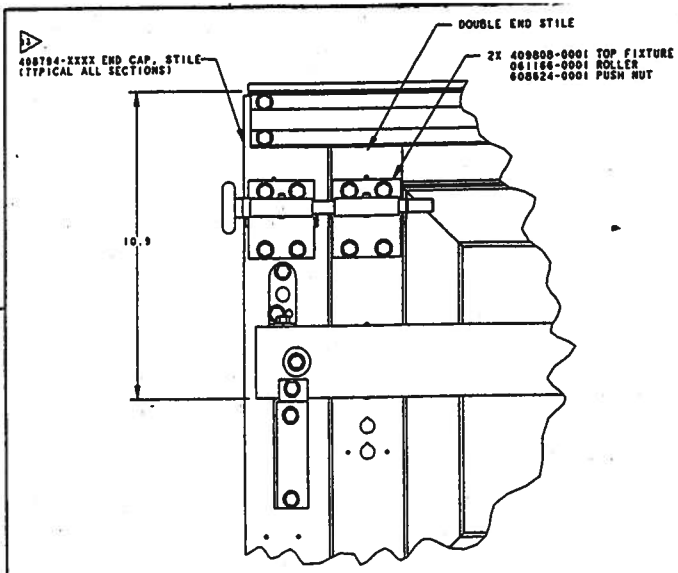
REF DWG	USED ON

WHOLE NUMBERS: ± .25
.X ± .1
.XX ± .03
.XXX ± .010
ANGLES: ± 1°
HOLES: ±

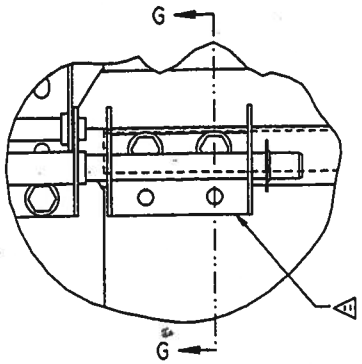
FINISH: N/A
UNIT OF MEASURE: EA
N/A

DRAWN BY: RK WOOFARD — DATE: 24 Sept 01
CHECKED: — DATE: 10/08/01
APPROVED: — DATE: 10/08/01

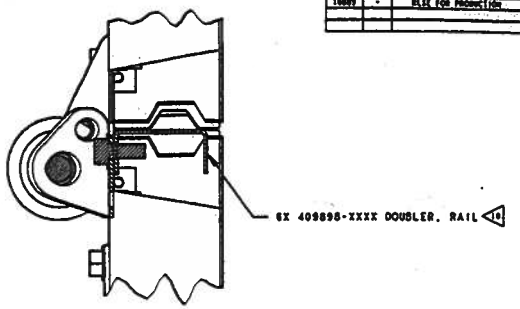
DRAWING TITLE: REST PAN STL DOOR, 31/46.5 PSF, 18" MAX
DRAWING NUMBER: D-409888
SCALE: 1/2 SHEET 4 OF 4



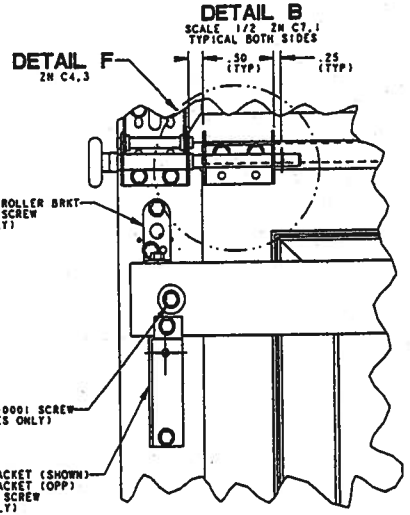
DETAIL B
SCALE 1/2 ZN C7.1
TYPICAL BOTH SIDES

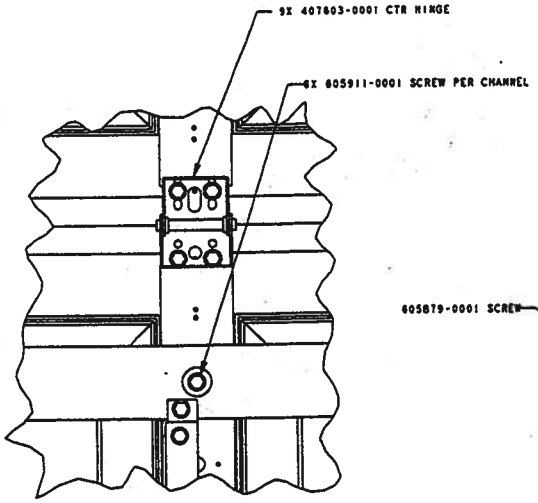


DETAIL F
SCALE 1/1 ZN B7.3

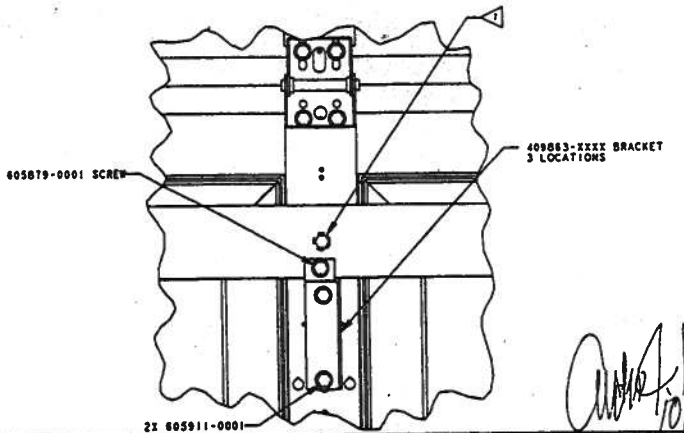


SECTION G-G





DETAIL C
TYPICAL CHANNEL INSTALLATION
SCALE 1/2 ZN C7.1



ID	REF	DESCRIPTION	DATE	APPROVED
10693	-	RLSE FOR PRODUCTION	10/10/01	

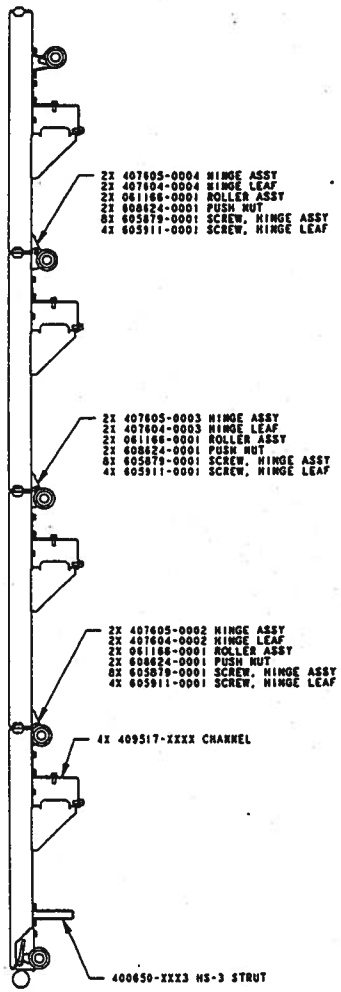
REF DWG	USED ON

UNLESS OTHERWISE SPECIFIED DIMENSIONS ARE IN INCHES
WHOLE NUMBERS: ± .25
.X ± .1
.XX ± .03
.XXX ± .010
ANGLES: ± 1°
HOLES: ±

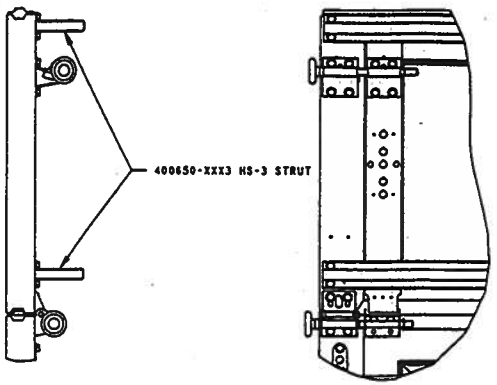
FINISH: N/A
UNIT OF MEASURE: EA
MATERIAL: N/A

DRAWN BY: BE WOOFARD — DATE: 24 Sept 01
CHECKED: 10/08/01
ENGINEER: 10/10/01

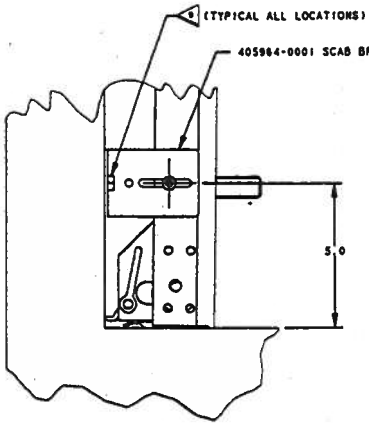
DRAWING TITLE: RES1 PAN STL DOOR, 31/40.5 PSF, 18" MAX
DRAWING NUMBER: D-409888
SCALE: NOTED — SHEET 3 OF 4



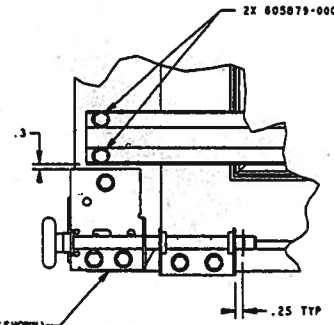
VIEW A-A
(TRACK ASSY REMOVED FOR CLARITY)
SCALE: 1/5 ZN A6.1



WINDOW BRACING DETAIL
SCALE 1/4



DETAIL E
TYPICAL BOTH SIDES
SCALE 1/2 ZN A7.1



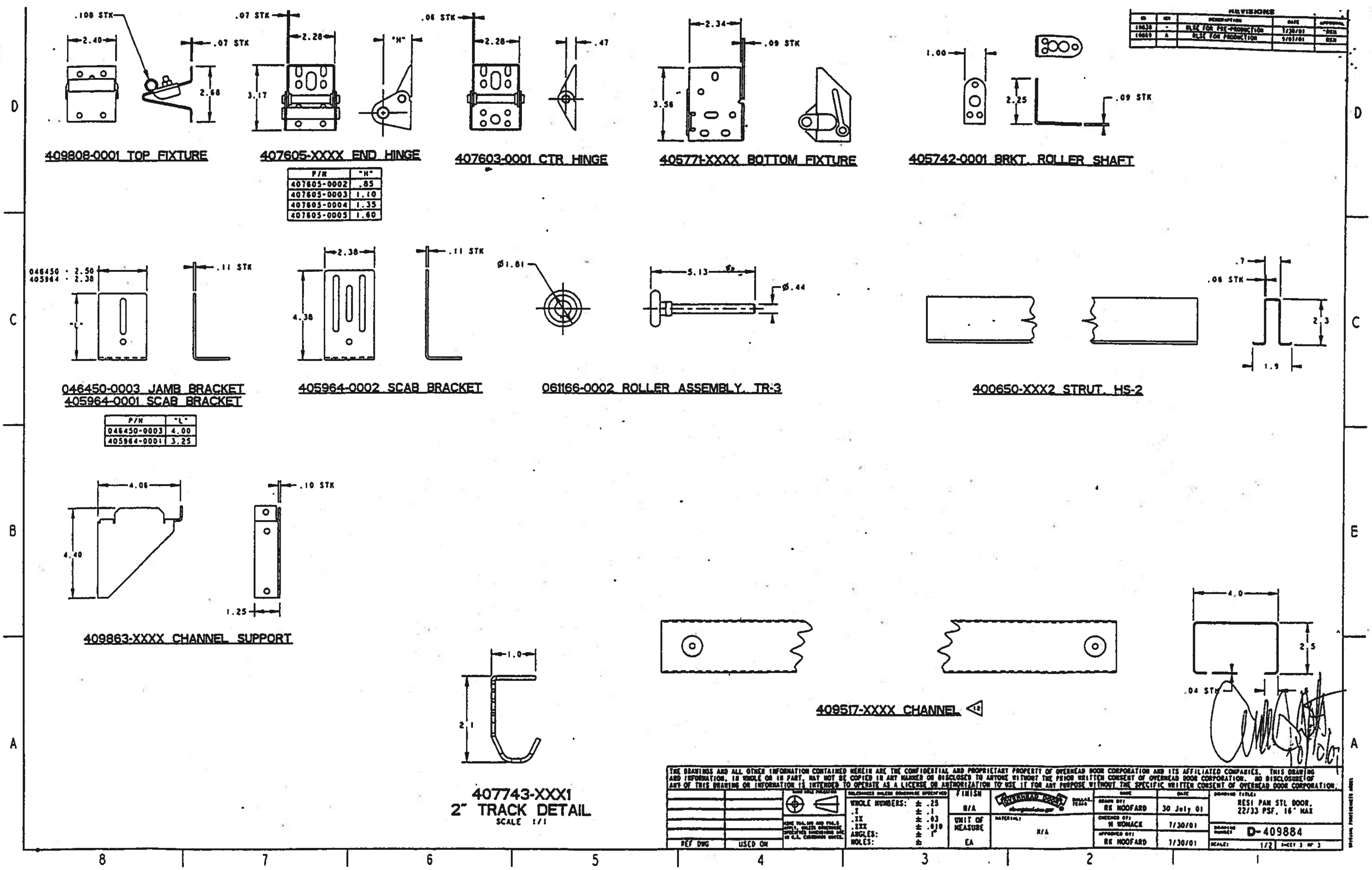
405771-0001 BOTTOM FIXTURE, LH (SHOWN)
405771-0002 BOTTOM FIXTURE, RH (OPP)
6X 605879-0001 SCREW, BOTTOM FIXTURE
2X 061166-0001 ROLLER ASSY
2X 407532-1001 HINGE LEAF, BOTTOM
4X 605911-0001 SCREW, HINGE LEAF, BOTTOM
2X 608624-0001 PUSH NUT

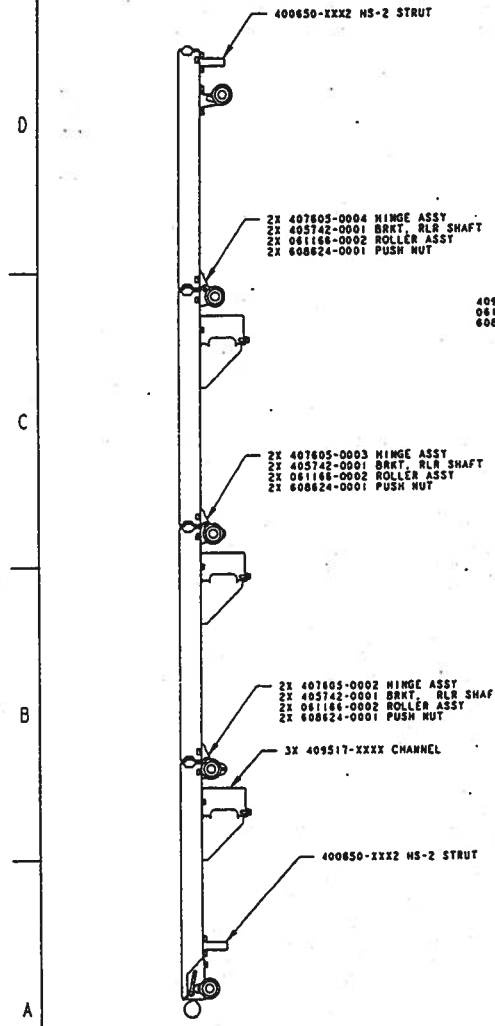
DETAIL D
SCALE 1/2 ZN A6.1

REVISIONS				
CO	ECO	DESCRIPTION	DATE	APPROVED
10889	-	RLSE FOR PRODUCTION	10/8/01	

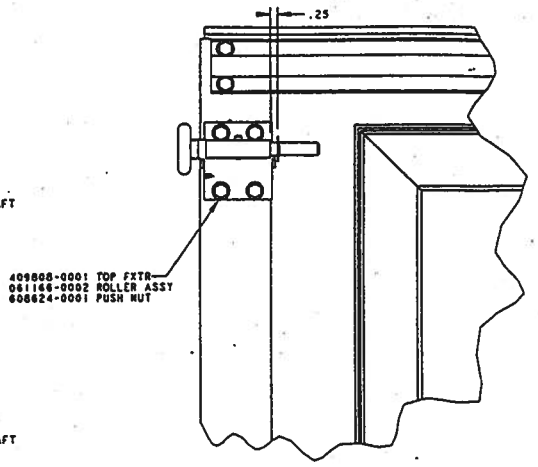
REF DRG	USED ON

UNLESS OTHERWISE SPECIFIED: WHOLE NUMBERS: ± .25; .X ± .1; .XX ± .03; .XXX ± .010; ANGLES: ± 1°; HOLES: ±
FINISH: N/A
UNIT OF MEASURE: EA
SCALE: N/A
DRAWN BY: RX WOOFARD — DATE: 24 Sept 01
APPROVED: 10/08/01
DATE: 10/8/01
DRAWING TITLE: RESI PAN STL DOOR, 31/46.5 PSF 18' MAX
DRAWING NUMBER: D-409888
SCALE: NOTED — SHEET 2 OF 4

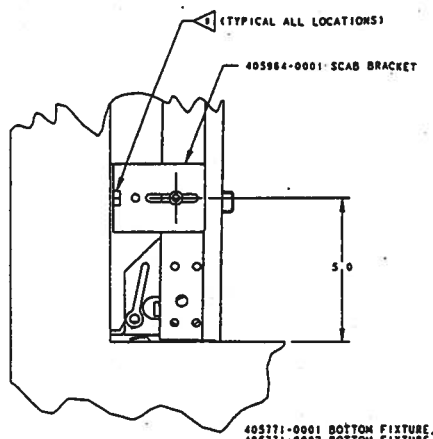

409808-0001 TOP FIXTURE
407605-XXXX END HINGE
P/N | "H"
407605-0002 | .85
407605-0003 | 1.10
407605-0004 | 1.35
407605-0005 | 1.60
407603-0001 CTR HINGE
405771-XXXX BOTTOM FIXTURE
405742-0001 BRKT. ROLLER SHAFT
046450-0003 JAMB BRACKET
405964-0001 SCAB BRACKET
P/N | "L"
046450-0003 | 4.00
405964-0001 | 3.25
405964-0002 SCAB BRACKET
061166-0002 ROLLER ASSEMBLY, TR-3
400650-XXX2 STRUT, HS-2
409863-XXXX CHANNEL SUPPORT
407743-XXX1
2" TRACK DETAIL
SCALE 1/1
409517-XXXX CHANNEL
THE DRAWINGS AND ALL OTHER INFORMATION CONTAINED HEREIN ARE THE CONFIDENTIAL AND PROPRIETARY PROPERTY OF OVERHEAD DOOR CORPORATION AND ITS AFFILIATED COMPANIES. THIS DRAWING AND INFORMATION, IN WHOLE OR IN PART, MAY NOT BE COPIED IN ANY MANNER OR DISCLOSED TO ANYONE WITHOUT THE PRIOR WRITTEN CONSENT OF OVERHEAD DOOR CORPORATION. NO DISCLOSURE OF ANY OF THIS DRAWING OR INFORMATION IS INTENDED TO OPERATE AS A LICENSE OR AUTHORIZATION TO USE IT FOR ANY PURPOSE WITHOUT THE SPECIFIC WRITTEN CONSENT OF OVERHEAD DOOR CORPORATION.
REF DWG
USED ON
WHOLE NUMBERS: ± .25
.X ± .1
.XX ± .03
.XXX ± .010
ANGLES: ± 1°
HOLES: ±
FINISH N/A
UNIT OF MEASURE EA
OVERHEAD DOOR
DALLAS, TEXAS
MATERIAL: N/A
DRAWN BY: RE HOOFARD 30 July 01
CHECKED BY: M WOMACK 7/30/01
APPROVED BY: RE HOOFARD 7/30/01
RESI PAN STL DOOR, 22/33 PSF, 16' MAX
D-409884
SCALE: 1/2
SHEET 3 OF 3



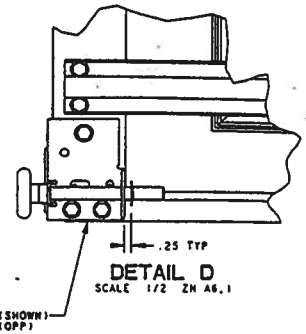
VIEW A-A
(TRACK ASSY REMOVED FOR CLARITY)
SCALE: 1/5 ZN A6.1



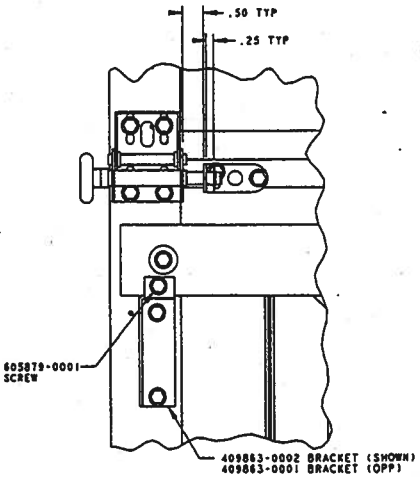
DETAIL B
SCALE 1/2 ZN C7.1
TYPICAL BOTH SIDES

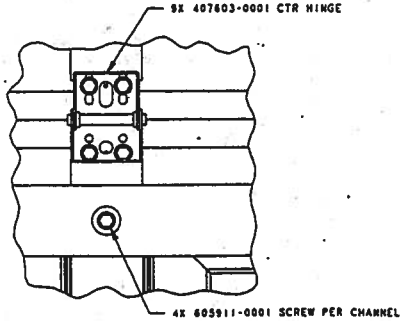


DETAIL E
SCALE 1/2 ZN A7.1
TYPICAL BOTH SIDES



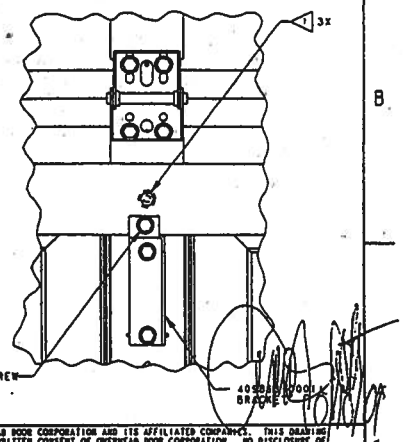
DETAIL D
SCALE 1/2 ZN A6.1





DETAIL C
SCALE 1/2 ZN C7.1

TYPICAL CHANNEL INSTALLATION



ECN	REV	DESCRIPTION	DATE	APPROVED
19836	-	RLSE FOR PRE-PRODUCTION	7/30/01	- BRR
19869	A	RLSE FOR PRODUCTION	9/07/01	BRR

REF DWG	USED ON

UNLESS OTHERWISE SPECIFIED: WHOLE NUMBERS: ± .25; .X ± .1; .XX ± .03; .XXX ± .010; ANGLES: ± 1°; HOLES: ± 1°
FINISH: N/A
UNIT OF MEASURE: EA
MATERIAL: N/A
DRAWN BY: RR NOOFARD 30 July 01
CHECKED BY: N MONACK 7/30/01
APPROVED BY: RR NOOFARD 7/30/01
DRAWING TITLE: RCS1 PAN STL DOOR, 22/33 PSF, 16' MAX
DRAWING NUMBER: D-409884
SCALE: NOTED SHEET 2 OF 3

NOTES:
1. TESTED IN ACCORDANCE WITH DASMA 108.
2. FOUR SECTION 7' TALL DOOR SHOWN. 8' DOORS HAVE 5 SECTIONS. SECTION HEIGHTS OF 20.88, 19.00 AND 16.75 ARE AVAILABLE AND MAY BE USED IN COMBINATION TO ACHIEVE VARIOUS DOOR HEIGHTS.
3. EMBOSSMENT PATTERN OF 14.5 X 20.4 SHOWN. ALTERNATE PATTERNS OF 12.5 X 43.4 AND 12.5 X 20.4 MAY BE USED.
4. ARMORTITE/REVOLUTION COUNTERBALANCE SHOWN. TORSION AND EXTENSION COUNTERBALANCE SYSTEMS AVAILABLE. 405964-0002 REQUIRED AT TRACK INTERFACE ON TORSION AND EXTENSION SYSTEMS.

[3] USE THIS BRACKET 405964-0002 ON 8' HIGH DOORS ONLY.

[6] LOCATE 409517 CHANNELS AS SHOWN. VERTICAL PLACEMENT CAN VARY PLUS/MINUS 3 INCHES.

[7] ON THE MIDDLE CENTER STILE THERE IS NO TOOL CLEARANCE HOLE FOR THE UPPER 605911-0001 SCREW. INSTALL FROM BELOW.

8. USE 605911-0001 AB POINT SCREWS FOR ALL COMPONENT INSTALLATION UNLESS OTHERWISE NOTED.

[9] JAMB DETAIL TO BE IN ACCORDANCE WITH DRAWING 409783. JAMB/SCAB BRACKET ATTACHING HARDWARE SHALL BE AS FOLLOWS: 605574-0001 SCREW, LAG 5/16 X 1-3/4 - WOOD JAMB RAWL LOCK BOLT, 5/16 X 1-1/2 EMBEDMENT - CONCRETE BLOCK

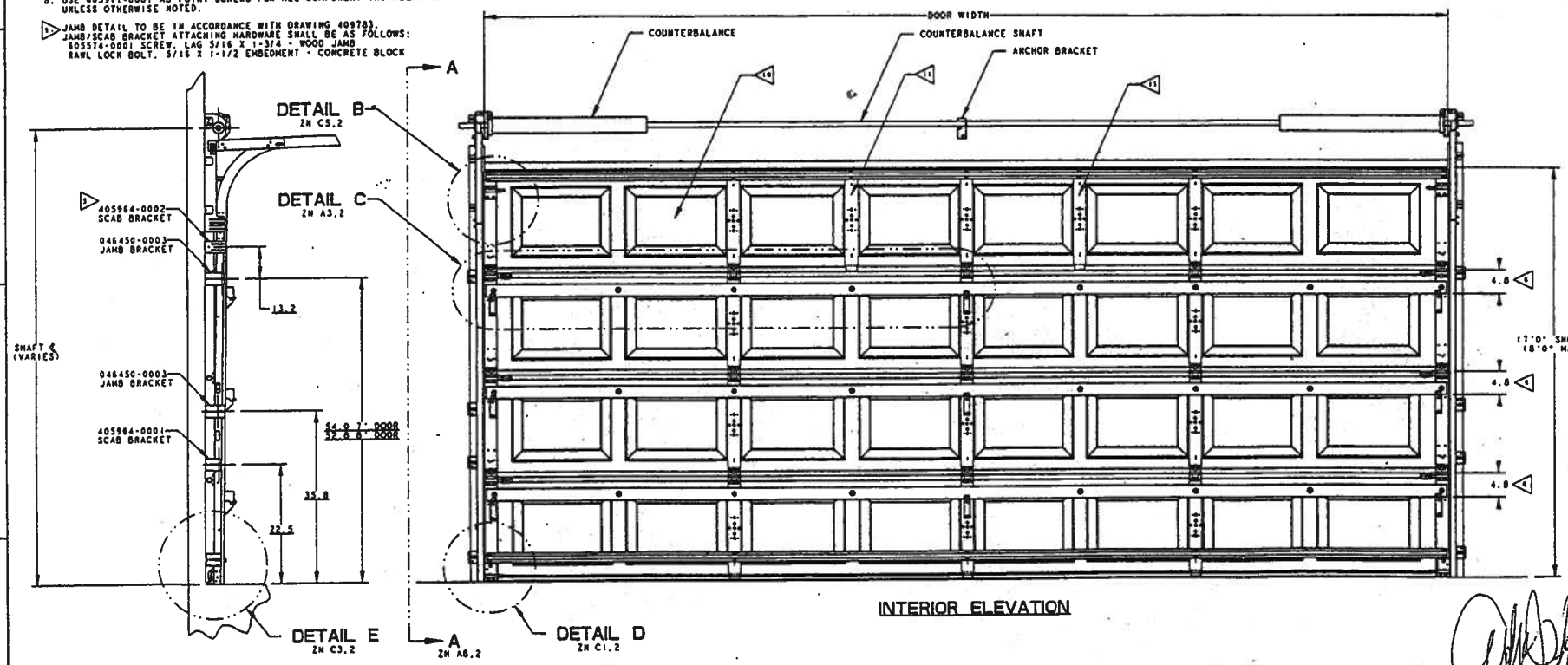
[10] WINDOW INSTALLATION ALLOWED. WINDOWS MUST BE INSTALLED IN TOP SECTION.

[11] EXTRA CENTER STILES ARE NOT REQUIRED ON DOORS WITHOUT A WINDOW SECTION.

[12] USE 18 GAUGE CHANNEL.

OHD/GENIE DOOR EQUIVALENCY CHART		
OHD DOOR SERIES No.	GENIE DOOR SERIES No.	PAN STEEL THICKNESS
SERIES 188	SERIES GDS300	24 GA
SERIES 189	SERIES GDS30R	24 GA
SERIES 281	SERIES GDS200	25 GA
SERIES 288	SERIES GDS500	25 GA

ch	qty	description	date	approved
10638	-	RLSE FOR PRE-PRODUCTION	7/30/01	RKH
10863	A	RLSE FOR PRODUCTION	9/07/01	RKH



INTERIOR ELEVATION

STD TRACK DETAIL
FOR 16' WIDE DOOR

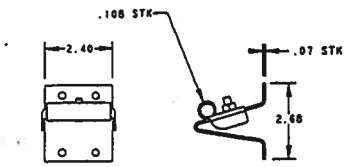
DOOR WIDTH	CENTER STILE	END STILE	BLR SHAFT BRACKET	STRUTS/SECT	ROLLER	VERT TRK GA	JAMB LOAD (#/FT-HT)
16'	3	SINGLE	YES	4" x 2.5" CH HS-2	STL 2"x7/16"	.049"	176

REF DWG | USED ON: RESIDENTIAL

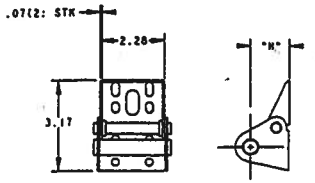
TOLERANCES UNLESS OTHERWISE SPECIFIED	
WHOLE NUMBERS:	± .25
.X	± .1
.XX	± .03
.XXX	± .010
ANGLES:	± 1°
HOLES:	± .005

FINISH: N/A
UNIT OF MEASURE: EA
MATERIAL: N/A
DRAWN BY: RK HOOFARD — DATE: 30 July 01
CHECKED BY: M WOMACK — 7/30/01
APPROVED BY: RK HOOFARD — 7/30/01
DRAWING TITLE: RESI PAN STL DOOR, 22/33 PSF, 16' MAX
DRAWING NUMBER: D-409884
SCALE: 1/10 SHEET 1 OF 3

ID	REV	DESCRIPTION	DATE	APPROVAL
19630	-	RLSE FOR PRE-PRODUCTION	7/30/01	DEH
19691	A	RLSE FOR PRODUCTION	8/07/01	DEH

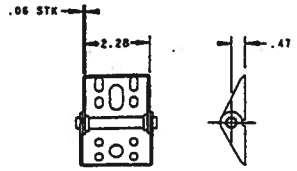


409808-0001 TOP FIXTURE

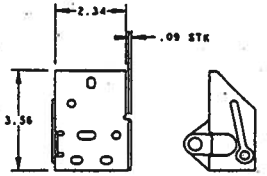


407605-XXXX END HINGE

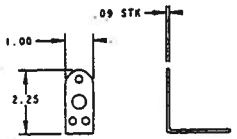
P/N	"H"
407605-0002	.85
407605-0003	1.10
407605-0004	1.35
407605-0005	1.60



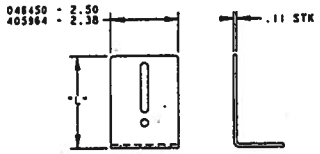
407603-0001 CTR HINGE



405771-XXXX BOTTOM FIXTURE

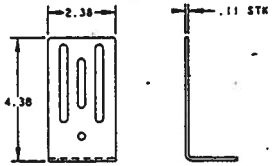


405742-0001 BRKT. RLR SHAFT

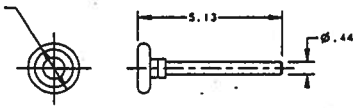


046450-0003 JAMB BRACKET
405964-0001 SCAB BRACKET

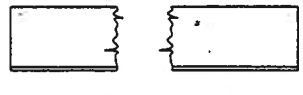
P/N	"L"
046450-0003	4.00
405964-0001	3.25

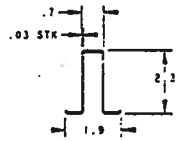


405964-0002 SCAB BRACKET

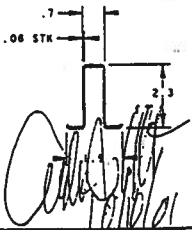


061166-0002 ROLLER ASSEMBLY, TR-3

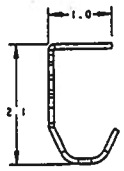




400650-XXX1 STRUT, HS-1



400650-XXX2 STRUT, HS-2



407743-XXX1
2" TRACK DETAIL
SCALE 1/1

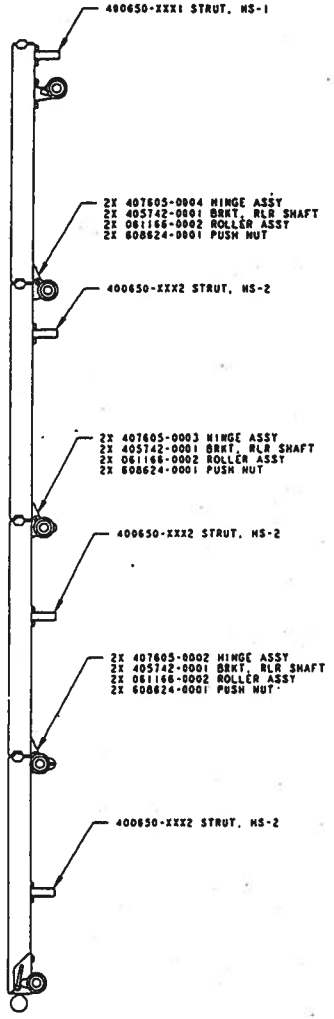
REF DWG	USED ON

UNLESS OTHERWISE SPECIFIED
WHOLE NUMBERS: ± .25
.X ± .1
.XX ± .03
.XXX ± .010
ANGLES: ± 1°
HOLES: ±

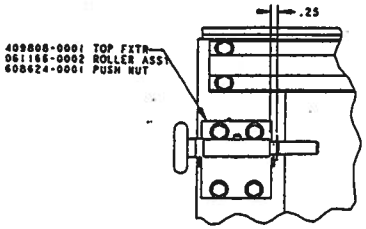
FINISH: N/A
UNIT OF MEASURE: EA
MATERIAL: N/A

	NAME	DATE
DRAWN BY	RE HOOFARD	30 July 01
CHECKED BY	M WONACK	7/30/01
APPROVED BY	RE HOOFARD	7/30/01

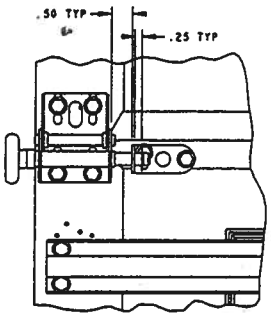
DRAWING TITLE: RESI PAN STL DOOR, 22/33 PSF, 9' MAX
DRAWING NUMBER: D-409887
SCALE: 1/2 SHEET 3 OF 3



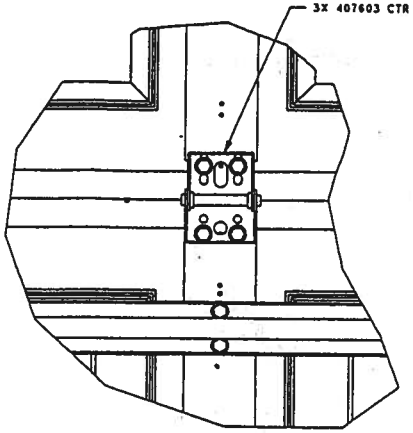
VIEW A-A
(TRACK ASSY REMOVED FOR CLARITY)
SCALE: 1/3 ZH A4.1



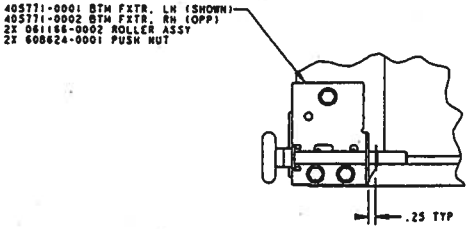
DETAIL B
SCALE1/2 ZH C5.1
TYPICAL BOTH SIDES



DETAIL C
SCALE1/2 ZH C5.1



DETAIL E
SCALE 1/2 C2.1



DETAIL D
SCALE1/2 ZH A4.1

REVISIONS				
ECN	REV	DESCRIPTION	DATE	APPROVED
19838	-	RLSE FOR ENGINEERING	7/30/01	DEER
19869	A	RLSE FOR PRODUCTION	9/07/01	DEER

REF DWG	USED ON

WHOLE NUMBERS:	± .25
.X	± .1
.XX	± .03
.XXX	± .010
ANGLES:	± 1°
HOLES:	±

FINISH: N/A
UNIT OF MEASURE: EA
MATERIAL: N/A

DRAWN BY: RK ROOFARD	30 July 01	DRAWING TITLE: RES1 PAN STL DOOR, 22/33 PSF, 9' MAX
CHECKED BY: N YONACK	7/30/01	DRAWING NUMBER: D-409887
APPROVED BY: RK ROOFARD	7/30/01	SCALE: NOTED SHEET 2 OF 3

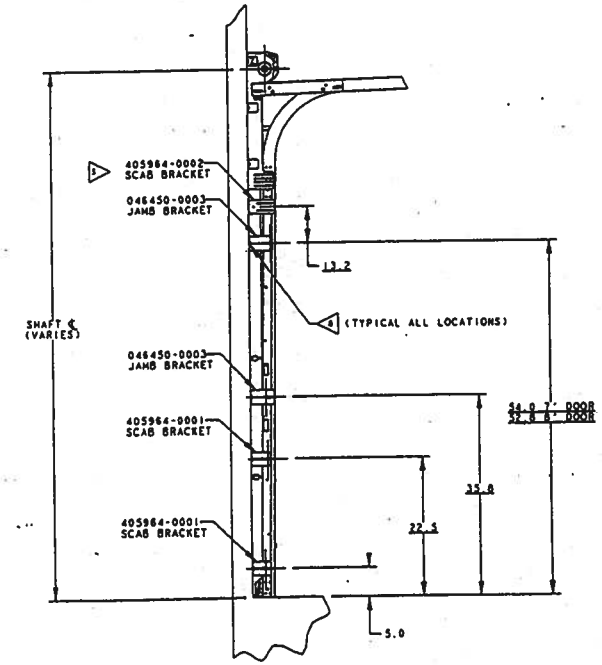
NOTES:
1. TESTED IN ACCORDANCE WITH DASMA 108.
2. FOUR SECTION 7' TALL DOOR SHOWN. 8' DOORS HAVE 5 SECTIONS. SECTION HEIGHTS OF 20.88, 19.00 AND 16.75 ARE AVAILABLE AND MAY BE USED IN COMBINATION TO ACHIEVE VARIOUS DOOR HEIGHTS.
3. EMBOSSMENT PATTERN OF 14.5 X 20.4 SHOWN. ALTERNATE PATTERNS OF 12.5 X 43.4 AND 12.5 X 20.4 MAY BE USED.
4. ARMORTITE/REVOLUTION COUNTERBALANCE SHOWN. TORSION AND EXTENSION COUNTERBALANCE SYSTEMS AVAILABLE. 405964-0002 REQUIRED AT TRACK INTERFACE ON TORSION AND EXTENSION SYSTEMS.
▷2 USE THIS BRACKET 405964-0002 ON 8' HIGH DOORS ONLY.
▷6 LOCATE 400650 STRUTS AS SHOWN. VERTICAL PLACEMENT CAN VARY PLUS/MINUS 3 INCHES.
7. USE 605911-0001 AB POINT SCREWS FOR ALL COMPONENT INSTALLATION UNLESS OTHERWISE NOTED.
▷9 JAMB DETAIL TO BE IN ACCORDANCE WITH DRAWING 409783. JAMB/SCAB BRACKET ATTACHING HARDWARE SHALL BE AS FOLLOWS: 605374-0001 SCREW, LAG 5/16 X 1-3/4 - WOOD JAMB RAWL LOCK BOLT, 5/16 X 1-1/2 EMBEDMENT - CONCRETE BLOCK

▷9 WINDOW INSTALLATION ALLOWED. WINDOWS MUST BE INSTALLED IN TOP SECTION.

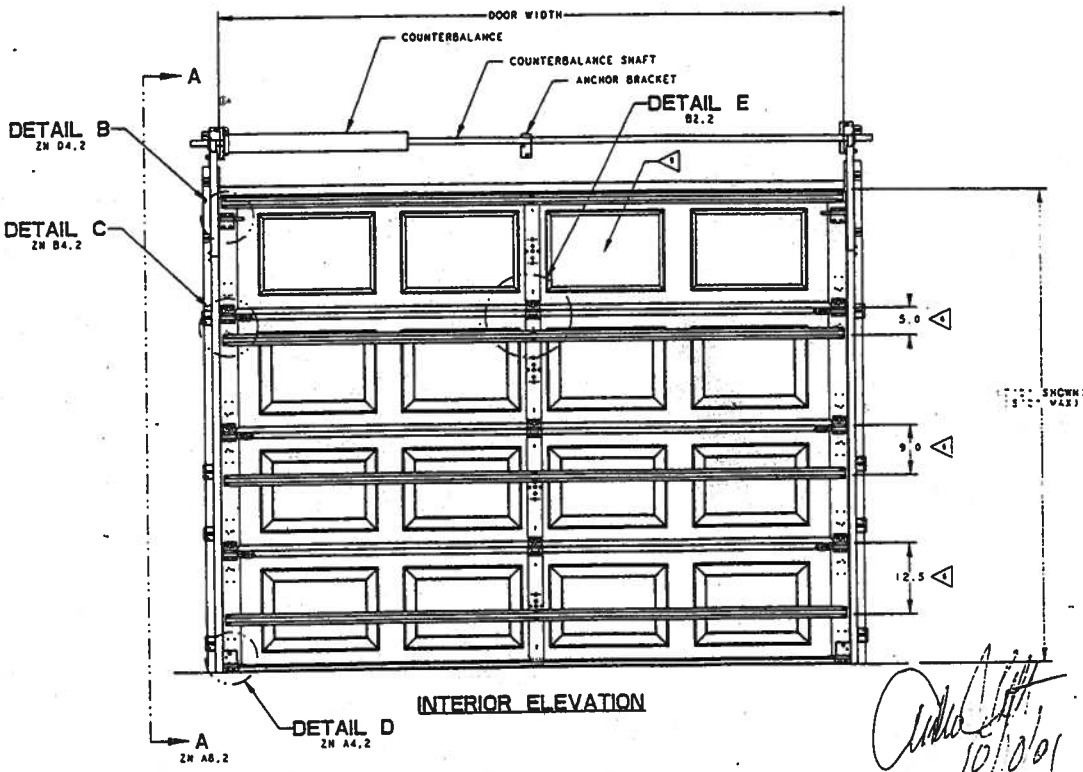
ID	QTY	DESCRIPTION	DATE	APPROVED
19638	-	RLSE FOR PRE-PRODUCTION	7/30/01	BER
19669	A	RLSE FOR PRODUCTION	9/07/01	BER

OHD/GENIE DOOR EQUIVALENCY CHART

OHD DOOR SERIES No.	GENIE DOOR SERIES No.	PAN STEEL THICKNESS
SERIES 188	SERIES GDS300	24 GA
SERIES 189	SERIES GDS30R	24 GA
SERIES 281	SERIES GDS200	25 GA
SERIES 288	SERIES GDS500	25 GA



STD TRACK DETAIL
FOR 9' WIDE DOOR



INTERIOR ELEVATION

10/8/01

DOOR WIDTH	CENTER STILE	END STILE	RLR SHAFT BRACKET	STRUTS/SECT	ROLLER	VERT TRK GA	JAMB LOAD (#/FT-HT)
9'	1	SINGLE	YES	HS-1 HS-2	STL 2"X7/16"	.049"	99

REF DWG | USED ON: RESIDENTIAL

UNLESS OTHERWISE SPECIFIED: WHOLE NUMBERS: ± .25; .X ± .1; .XX ± .03; .XXX ± .010; ANGLES: ± 1°; HOLES: ±

FINISH: N/A
UNIT OF MEASURE: EA
MATERIAL: N/A

	NAME	DATE
DRAWN BY:	RR WOOFARD	30 July 01
CHECKED BY:	M WOMACK	7/30/01
APPROVED BY:	RR WOOFARD	7/30/01

DRAWING TITLE: RESI PAN STL DOOR, 22/33 PSF, 9' MAX
DRAWING NUMBER: D-409687
SCALE: 1/12 | SHEET 1 OF 3

NOTES:
1. TESTED IN ACCORDANCE WITH DASMA 108.
2. FOUR SECTION 7' TALL DOOR SHOWN. 8' DOORS HAVE 5 SECTIONS. SECTION HEIGHTS OF 20.88, 19.00 AND 16.75 ARE AVAILABLE AND MAY BE USED IN COMBINATION TO ACHIEVE VARIOUS DOOR HEIGHTS.
3. EMBOSSMENT PATTERN OF 14.5 X 20.4 SHOWN. ALTERNATE PATTERNS OF 12.5 X 43.4 AND 12.5 X 20.4 MAY BE USED.
4. ARMORTITE/REVOLUTION COUNTERBALANCE SHOWN. TORSION AND EXTENSION COUNTERBALANCE SYSTEMS AVAILABLE. 405964-0002 REQUIRED AT TRACK INTERFACE ON TORSION AND EXTENSION SYSTEMS.

△ USE THIS BRACKET 405964-0002 ON 8' HIGH DOORS ONLY.

△ LOCATE 409517 CHANNEL AS SHOWN. VERTICAL PLACEMENT CAN VARY PLUS/MINUS 3 INCHES.

△ ON THE MIDDLE CENTER STILE THERE IS NO TOOL CLEARANCE HOLE FOR THE UPPER 605911-0001 SCREW. INSTALL FROM BELOW.

6. USE 605911-0001 AB POINT SCREWS FOR ALL COMPONENT INSTALLATION UNLESS OTHERWISE NOTED.

△ JAMB DETAIL TO BE IN ACCORDANCE WITH DRAWING 409783. JAMB/SCAB BRACKET ATTACHING HARDWARE SHALL BE AS FOLLOWS: 605574-0001 SCREW, LAG 5/16 X 1-3/4 - WOOD JAMB RAWL LOCK BOLT, 5/16 X 1-1/2 EMBEDMENT - CONCRETE BLOCK

△ 409898-XXXX RAIL DOUBLER TO BE LOCATED AT THE FACTORY; BOTTOM AND INTERMEDIATE SECTIONS ONLY.

△ INSTALL 407604-XXXX HINGE LEAF USING 605879-0001 SELF DRILLING FASTENER. ENSURE 409898-XXXX RAIL IS FLAT AGAINST INSIDE OF RAIL SURFACE PRIOR TO FASTENING.

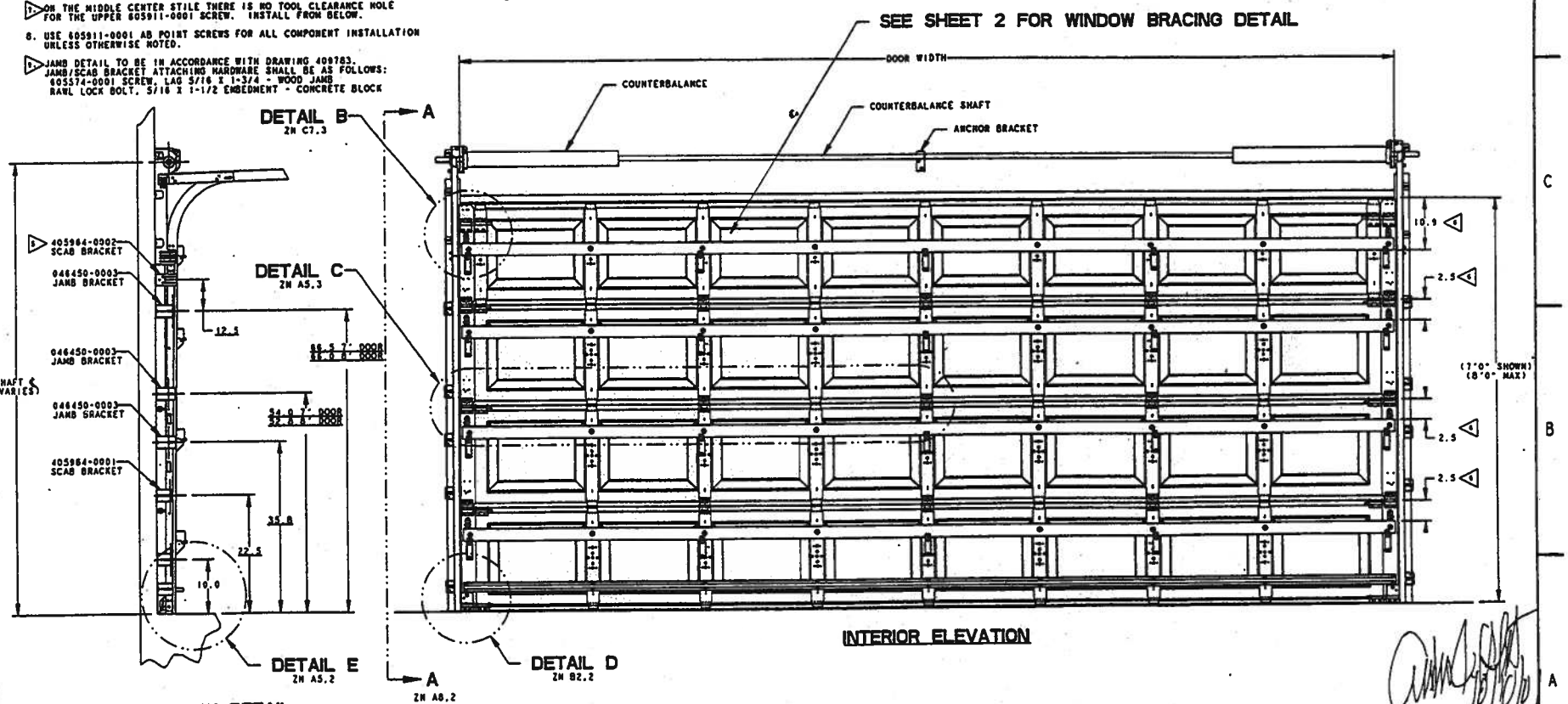
△ USE 16 GA CHANNEL

△ 408794 END CAP TO BE FACTORY INSTALLED.

REVISIONS				
ID	ECO	DESCRIPTION	DATE	APPROVED
19681	-	RLSE FOR PRODUCTION	10/08/01	

OHD/GENIE DOOR EQUIVALENCY CHART		
OHD DOOR SERIES No.	GENIE DOOR SERIES No.	PAN STEEL THICKNESS
SERIES 188	SERIES GDS300	24 GA
SERIES 189	SERIES GDS30R	24 GA
SERIES 281	SERIES GDS200	25 GA
SERIES 288	SERIES GDS500	25 GA

SEE SHEET 2 FOR WINDOW BRACING DETAIL



INTERIOR ELEVATION

STD TRACK DETAIL FOR 18' WIDE DOOR

DOOR WIDTH	CENTER STILE	END STILE	RLR SHAFT BRACKET	STRUTS/SECT	ROLLER	VERT TRK GA	JAMB LOAD (#/FT-HT)
18'	7	SGL W/CAP DBL-T SECT	YES	4" X 2.5" CH HS-3	STL 2"X7/16"	.068"	279

REF DWG | RESIDENTIAL USED ON

WHOLE NUMBERS: ± .25
.X ± .1
.XX ± .03
.XXX ± .010
ANGLES: ± 1°
HOLES: ±

FINISH: N/A
UNIT OF MEASURE: EA
MATERIAL: N/A

DRAWN BY: RE WOOFARD — DATE: 24 Sept 01
CHECKED: [signature] — DATE: 10/08/01
[signature] — DATE: 10/8/01

ORIGINAL TITLE: RESI PAN STL DOOR, 31/48.5 PSF, 18' MAX
DRAWING NUMBER: D-409888
SCALE: 1/10 SHEET 1 OF 4

AAMA/NWWDA 101/I.S.2-97
TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650 Fin
TYPE: Aluminum Single Hung Window

Title of Test	Results
Rating	H-R40 52 x 72
Overall Design Pressure	+45.0 psf -47.2 psf
Operating Force	11 lb max.
Air Infiltration	0.13 cfm/ft^2
Water Resistance	6.00 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-41134.01 dated 03/26/02 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess, Technician

MAH:nlb

ALLEN M. REEVES
CERTIFICATE
NO. 19354
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Allen M. Reeves
1 APRIL 2002

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.
650 West Market Street
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-41134.01
Test Date: 03/07/02
Report Date: 03/26/02
Expiration Date: 03/07/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on Series/Model 650 Fin, aluminum single hung window at their facility located in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for a H-R40 52 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 650 Fin

Type: Aluminum Single Hung Window

Overall Size: 4' 4-1/4" wide by 6' 0-3/8" high

Active Sash Size: 4' 1-3/4" wide by 3' 0-5/8" high

Daylight Opening Size: 3' 11-3/8" wide by 2' 9-1/2" high

Screen Size: 4' 0-1/4" wide by 2' 11-1/8" high

Finish: All aluminum was white.

Glazing Details: The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap-around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Allen N. Reeves
1 APRIL 2002

REEVES NO. 19354 STATE OF FLORIDA PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.230" high by 0.270" backed polypile with center fin	1 Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" x 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam-filled vinyl bulb seal	1 Row	Active sash, bottom rail

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. Meeting rail was secured to the frame utilizing two 1-1/4" screws.

Sash Construction: The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each jamb screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

Hardware:

Description	Quantity	Location
Metal cam lock with keeper		Midspan, active meeting rail with keeper adjacent on fixed meeting rail
Plastic tilt latch	2	Active sash, meeting rail ends
Metal tilt pin	2	Active sash, bottom rail ends
Balance assembly	2	One in each jamb
Screen plunger	2	4" from rail ends on top rail

Allen N. Reeves
1 APRIL 2002

ALLEN N. REEVES CERTIFICATE NO. 19[illegible] STATE OF FLORIDA PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood test buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	11 lbs	30 lbs max
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.13 cfm/ft^2	0.3 cfm/ft^2 max
	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 25.9 psf (positive)	0.42"*	0.26" max.
	@ 34.7 psf (negative)	0.43"*	0.26" max.
2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 38.9 psf (positive)	0.02"	0.18" max.
	@ 52.1 psf (negative)	0.02"	0.18" max.

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

**Exceeds L/175 for deflection, but passes all other test requirements.*

Allen N. Reeves
1 APRIL 2002

ALLEN N. REEVES
CERTIFICATE
NO. 18374
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.2	Deglazing Test (ASTM E 987)		
	In operating direction at 70 lbs		
	Meeting rail	0.12"/25%	0.50"/100%
	Bottom rail	0.12"/25%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.06"/12%	0.50"/100%
	Right stile	0.06"/12%	0.50"/100%
	Forced Entry Resistance (ASTM F 588-97)		
	Type: A Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Tests A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 6.00 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 45.0 psf (positive)	0.47"*	0.26" max.
	@ 47.2 psf (negative)	0.46"*	0.26" max.

**Exceeds L/175 for deflection, but passes all other test requirements.*

Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
@ 67.5 psf (positive)	0.05"	0.18" max.
@ 70.8 psf (negative)	0.05"	0.18" max.

Allen N. Reeves
1 APRIL 2002

LEIL N. REEVES
CERTIFICATE NO. 18554
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

Allen N. Reeves, P.E.
Director - Engineering Services
1 APRIL 2002

MAH:nlb
01-41134.01

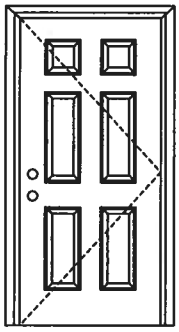
ALLEN N. REEVES
CERTIFICATE NO.
STATE OF FLORIDA
PROFESSIONAL ENGINEER

X
Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+66.0/-66.0
limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

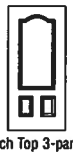
Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

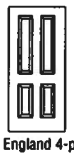
APPROVED DOOR STYLES:


Flush


Arch Top 3-panel


3-panel


6-panel


New England 4-panel


Eyebrow 4-panel


8-panel


9-panel


15-panel


5-panel


5-panel with scroll


Eyebrow 5-panel


Eyebrow 5-panel with scroll

Johnson™ EntrySystems

June 17, 2002
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PREMDOR Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

X
Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS

CERTIFIED TEST REPORTS:

NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA201, PA202 and PA203.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO
PA201, PA202 & PA203

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

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June 17, 2002
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PREMDOR Collection Premium Quality Doors

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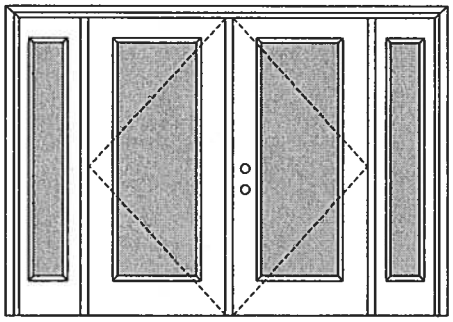
Masonite International Corporation

OXXO
Glazed Inswing Unit

COP-WL-JH4145-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Double Door with 2 Sidelites
Maximum unit size = 12'0" x 6'8"

Design Pressure
+40.5/-40.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0005-02 or MAD-WL-MA0008-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0005-02.

APPROVED DOOR STYLES:

1/4 GLASS:


100 Series

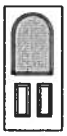

133, 135 Series

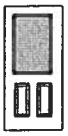

136 Series


680 Series

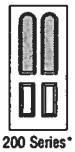

822 Series

1/2 GLASS:

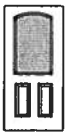

105 Series*


106, 160 Series*


129 Series*


200 Series*


12 R/L, 23 R/L, 24 R/L Series*


107 Series*


108 Series


304 Series

*This glass kit may also be used in the following door styles: 5-panel; 5-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.

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OXXO
Glazed Inswing Unit

COP-WL-JH4145-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

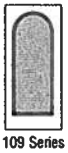
3/4 GLASS:

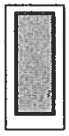

404 Series


410 Series


450 Series

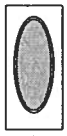
FULL GLASS:


109 Series


114, 120, 122 Series


152 Series


149 Series


300 Series

APPROVED SIDELITE STYLES:


680 Series


129 Series

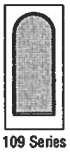

200 Series

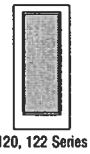

12R, 12L, 23R, 23L, 24R, 24L Series


450 Series


152 Series


149 Series


109 Series


120, 122 Series


300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab and sidelite panels glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

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June 17, 2002
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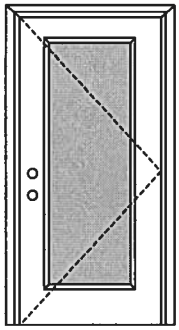
X
Glazed Inswing Unit

COP-WL-JH4141-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+40.5/-40.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:

1/4 GLASS:


100 Series


133, 135 Series

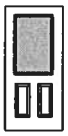

136 Series

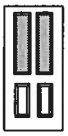

680 Series

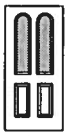

822 Series

1/2 GLASS:

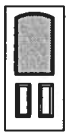

105 Series*


106, 160 Series*


129 Series*


200 Series*


12 R/L, 23 R/L, 24 R/L Series*


107 Series*


108 Series


304 Series

*This glass kit may also be used in the following door styles: 5-panel; 5-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.

Johnson™ EntrySystems

June 17, 2002
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PREMDOR Collection Premium Quality Doors

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Masonite International Corporation

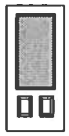
X
Glazed Inswing Unit

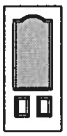
COP-WL-JH4141-02

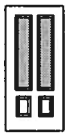
WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

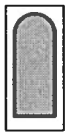
3/4 GLASS:


404 Series


410 Series

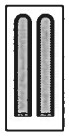

450 Series

FULL GLASS:


109 Series


114, 120, 122 Series


152 Series


149 Series


300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Johnson™ EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors

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website: www.nateval.org

INTERNATIONAL CODE COUNCIL

NATIONAL EVALUATION REPORT

Report No. NER-432
Reissued June 1, 2000

SIMPSON STRONG-TIE® COMPANY, INC., CONNECTORS

SIMPSON STRONG-TIE® COMPANY, INC.
4637 CHABOT DRIVE, SUITE 200
PLEASANTON, CALIFORNIA 94588

1.0 SUBJECT

Simpson Strong-Tie® Connectors:

1.1 ABE Adjustable Post Bases
1.2 CBA Adjustable Column Bases
1.3 EPB44T Elevated Post Base
1.4 H2.5, H10-2, H15, and H15-2, Hurricane Ties
1.5 HGT-2, HGT-3, and HGT-4 Heavy Girder Tiedowns
1.6 LSSU Field Slope and Skewable Hangers
1.7 LTHMA Light Multiple Truss Hanger
1.8 LTHJ Light Truss Hip/Jack Hangers
1.9 LTP4 Lateral Tie Plate
1.10 LTTI31 Tension Tie
1.11 MSC Multiple Seat Connectors
1.12 RSP4 Reversible Stud Plate Tie
1.13 SP Stud Plate Connectors
1.14 SS Stud Shoes
1.15 THG2A Skewed Truss Girder Hangers
1.16 TWB T-Type Wall Bracing

2.0 PROPERTY FOR WHICH EVALUATION IS SOUGHT

Structural

3.0 DESCRIPTION

3.1 ABE ADJUSTABLE POST BASES

The two-part bases are die-formed from No. 16 gage galvanized steel complying with ASTM A653 CS, with a minimum yield strength of 28,000 psi (193 MPa) and a minimum tensile strength of 38,000 psi (262 MPa). The base plate includes an adjustment slot for a 1/2 inch (12.7 mm) diameter anchor bolt, and a 1 inch (25.4 mm) pedestal is built into the four-sided standoff plate. The post bases are manufactured to fit nominal 3 1/2 inch by 3 1/2 inch (89 mm by 89 mm) wood posts. See Table 1 of this report for details and fastener schedules.

3.2 CBA ADJUSTABLE COLUMN BASES

The two piece bases attach a wood post to existing concrete footings and slabs and are adjustable to accommodate minimum 5 1/2 inch by 5 1/2 inch (140 mm by 140 mm) posts. The two pieces have two 3/4 inch (19 mm) diameter bolt holes in the base for attachment to the concrete elements. To connect to the posts, the projecting side flanges have 5/8 inch (15.9 mm) diameter bolt holes. Both pieces are formed from No. 10 gage galvanized steel that conforms to ASTM A653 SS Grade 33, with a minimum yield strength of 33,000 psi (227 MPa) and a minimum tensile strength of 45,000 psi (310 MPa). See Table 2 of this report for details, allowable loads, and fastener schedules.

3.3 EPB44T ELEVATED POST BASE

The two part base consists of a No. 12 gage galvanized steel channel. The channel includes a die drawn 5/8 inch (15.9 mm) diameter threaded hole trunnion at it's center, which accommodates a 5 inch long (127 mm), 5/8 inch (15.9 mm) diameter zinc plated threaded rod. The threaded rod and channel assembly adjusts to a maximum post elevation of 2 1/2 inches (63.4 mm) above the concrete and a minimum concrete embedment of 2 1/2 inches (63.4 mm). The post base is manufactured to accommodate nominal 4 inch by 4 inch (101.6 mm by 101.6 mm) wood posts. The steel meets ASTM A653 SS Grade 33 specifications with a minimum yield strength of 33,000 psi (227 MPa) and a minimum tensile strength of 45,000 psi (310 MPa). See Table 3 of this report for details, allowable loads, and fastener schedules.

3.4 H2.5, H10-2, H15, H15-2 HURRICANE TIES

Hurricane Ties are anchors designed to connect rafters or joists to wall plates or studs. The H2.5 and H10-2 are formed from No. 18 gage galvanized steel. The H15 and H15-2 are formed from No. 16 gage galvanized steel. The steel meets ASTM A653 FS with a minimum yield strength of 28,000 psi (193 MPa) and a minimum tensile strength of 38,000 psi (262 MPa). See Table 4 of this report for details and fastener schedules.

The H2.5 is a twisted strap tie used to attach a rafter or stud member to the side of a top plate or bottom sole plate. The lower end is fastened to the wall plate and is long enough to locate the nails into each of the two top plates.

The H10-2 is formed from a 5 1/2 inch (140 mm) plate, and has one 3 1/8 inch (79.4 mm) wide, 3 1/32 inch (80 mm) deep slot in the upper half of the plate, diagonal to the edge. The slot material is bent to form a 1 9/16 inch (39.7 mm) flange on each side. The H10-2 attaches double nominal 2 inch (50.8 mm) wide solid sawn rafters or joists to the top of a wall at the double plate.

The H15 and H15-2 are formed into an inverted U-shaped element with a 2 1/2 inch (63.5 mm) wide flange bent at right angles near the U-bend. The flanges are twisted again at 90 degree angles at the bottom of the connector. The H15 has a formed seat 1 5/8 inches (41.3 mm) wide and attaches over the heel of a single ply truss or rafter to the wall stud member below. The H15-2 has a formed seat 3 1/4 inches (82.6 mm) wide and attaches the heel of a double ply truss or rafter to a double nominal 2 inch (50.8 mm) thick wall stud member below. Both ties are capable of being field formed to accommodate rafter or truss pitches from 0:12 minimum to 7:12 maximum.

3.5 HGT-2, HGT-3, AND HGT-4 HEAVY GIRDER TIEDOWNS

The HGT-2, HGT-3, and HGT-4 are formed from No. 7 gage steel conforming to ASTM A570 Grade 33 with a minimum yield strength of 33,000 psi (227 MPa) and a minimum tensile strength of 52,000 psi (358 MPa), and a welded insert plate made from 1/2 inch (12.7 mm) thick A36 hot-rolled steel with a minimum yield strength of 33,000 psi (227 MPa) and a minimum ultimate strength of 46,000 psi (310 MPa). The HGT-2 attaches to the heel of a two-ply truss. The HGT-3 attaches to the heel of a three-ply truss. The HGT-4 attaches to the heel of a four-ply truss. The HGT tiedowns are anchored to concrete or wood construction with bolts, can accommodate top chord slopes from 3:12 minimum to 8:12 maximum and are provided with crescent washers for sloped top chord installations. See Table 5 of this report for details, allowable loads and fastener schedules.

3.6 LSSU FIELD SLOPE AND SKEWABLE HANGERS

The LSSU is fabricated from No.18 gage galvanized steel. It is designed to be sloped up or down 45 degrees and skewed right or left to 45 degrees. The steel complies with ASTM A653 FS having a minimum yield strength of 33,000 psi (227 MPa) and a minimum tensile strength of 45,000 psi (310 MPa). Use of the hangers to laterally support members is beyond the scope of this report. See Table 6 of this report for details, allowable loads, and fastener schedules.

3.7 LTHMA LIGHT MULTIPLE TRUSS HANGER

The LTHMA is fabricated from No. 16 gage galvanized steel conforming to ASTM A653 FS with a minimum yield strength of 33,000 psi (227 MPa) and a minimum tensile strength of 45,000 psi (310 MPa). The LTHMA is designed to carry up to three single-ply truss members intersecting at one point. The connector has three formed stirrups that are capable of being field sloped down from 0 to 45 degrees from the horizontal. See Table 7 of this report for details and fastener schedules.

3.8 LTHJ LIGHT TRUSS HIP/JACK HANGERS

The LTHJ hangers are formed from No.18 gage galvanized steel conforming to ASTM A653 FS having a minimum yield strength of 28,000 psi (193 MPa) and a minimum tensile strength of 38,000 psi (262 MPa). The hangers allow a 45 degree member and a 90 degree member to attach to the supporting member at the same location. Use of the hangers to laterally support members is beyond the scope of this report. See Table 8 of this report for hanger details, allowable loads, and fastener schedules.

3.9 LTP4 LATERAL TIE PLATE

The LTP4 is a 3 inch by 4 1/4 inch (76.2 mm by 108 mm), No. 20 gage, galvanized flat steel plate. The steel complies with ASTM A653 FS with a minimum yield strength of 33,000 psi (227 MPa) psi and a minimum tensile strength of 45,000 psi (310 MPa). See Table 9 of this report for details, allowable loads, and fastener schedules.

3.10 LTTI31 TENSION TIE

The LTTI31 is formed from No.18 gage steel strap that is 3 3/4 inches (95.2 mm) wide with a 90 degree bend at one end. The bend is 2 3/4 inches (69.8 mm) long, with an 11/16 inch (17.5 mm) diameter hole punched in the center to provide anchorage with a 5/8 inch (15.9 mm) diameter bolt. A No. 3 gage load transfer plate is installed in the bend in lieu of a washer. The No.18 gage steel complies with ASTM A653 SS and has a minimum yield strength of 33,000 psi (227 MPa) and a minimum tensile strength of 45,000 psi (310 MPa). The No. 3 gage steel complies with ASTM A570 with a minimum yield strength of 33,000 psi (227 MPa) and a minimum tensile strength of 52,000 psi (358 MPa). See Table 10 of this report for allowable loads and fastener schedules.

3.11 MSC MULTIPLE SEAT CONNECTORS

The MSC1.81 and MSC2 connectors are three No. 11 gage U-shaped hangers that are factory-welded to a single No. 3 gage steel angle with 1/8 inch (3.2 mm) fillet welds. The MSC4 connector consists of three No. 7 gage U-shaped stirrups that are factory-welded to a single No. 3 gage steel angle with 3/16 inch (4.8 mm) fillet welds. The MSC series connector is designed to carry up to three members intersecting at one point, with the center member perpendicular to the carried member. The two U-shaped stirrups for the side members can accommodate horizontal skews up to 45 degrees from the center member and down slopes from 0 to 45 degrees. The Nos. 11, 7 and 3 gage steel conform to ASTM A570 Grade 33 with a minimum yield strength of 33,000 psi (227 MPa) and a minimum tensile strength of 52,000 psi (358 MPa). See Table 11 of this report for details, allowable loads, and fastener schedules.

3.12 RSP4 REVERSIBLE STUD PLATE TIE

The RSP4 Reversible Stud Plate Tie is a No. 20 gage galvanized sheet metal plate cut in the shape of a "1/4". The RSP4 has two triangular and two rectangular tabs, bent at 90 degrees from the face of the "1/4", which act as placement guides. The RSP4 is a dual-purpose, reversible tie-plate used to secure vertical wood-stud framing members (typically 2 x 4, 2 x 6, and 2 x 8) to the bottom sole plate of a stud wall, or secure the top of the same stud member to the double top plate members.

In a stud-to-double-top-plate condition, the RSP4 rectangular tabs on the vertical leg of the "1/4" act as guides to position the nailing pattern to allow for an equal number of nails to be distributed to each top plate. In the stud-to-bottom sole-plate application, the RSP4 is inverted (the horizontal leg is on the bottom), and is installed such that the triangular tabs on the horizontal leg rest on top of the sole plate to position the tie plate for consistent nail placement. The steel complies with ASTM A653 FS, with a minimum yield strength of 28,000 psi (193 MPa) and a minimum tensile strength of 38,000 psi (262 MPa). See Table 12 of this report for details, allowable loads, and fastener schedules.

3.13 SP STUD PLATE CONNECTORS

SP connectors are die-formed from No. 20 gage galvanized steel conforming to ASTM A653 FS, with a minimum yield strength of 28,000 psi (193 MPa) and a minimum tensile strength of 38,000 psi (262 MPa). SP1 and SP2 connectors fasten to a single plate, or a double plate, respectively. See Table 13 of this report for allowable loads and fastener schedules.

3.14 SS STUD SHOES

SS stud shoes are formed from No.18 gage galvanized steel complying with ASTM A653 FS with a minimum yield strength of 28,000 psi (193 MPa) and a minimum tensile strength of 38,000 psi (262 MPa). The stud shoes reinforce notched wood studs. See Table 14 of this report for allowable loads and fastener schedules.

3.15 THG2A SKEWED TRUSS GIRDER HANGERS

THG2A hangers are formed from No. 10 gage galvanized steel, complying with ASTM A653 SS Grade 33, with a minimum yield strength of 33,000 psi (227 MPa) and a minimum tensile strength of 45,000 psi (310 MPa). The hangers allow the 45 degree attachment of a multiple girder truss to a carrying member that is attached to a vertical component with two 3/4 inch (19 mm) diameter machine bolts. Use of the hangers to laterally support members is beyond the scope of this report. See Table 15 of this report for details, allowable loads, and fastener schedules.

3.16 TWB T-Type Wall Bracing

The TWB10, TWB12 and TWB14 bracing are formed from No. 22 gage (0.030 inch) galvanized steel complying with ASTM A653 FS, having a minimum yield strength of 28,000 psi (193 MPa) and a minimum tensile strength of 38,000 psi (262 MPa). The flanges are 9/16 inch (143 mm) wide. The kerf leg is 9/16 inch (14.3 mm) deep with an overall width of 1/8 inch (3.2 mm). The wall bracings provide racking resistance for wood-framed walls during construction. See Table 16 of this report for details and fastener schedules.

4.0 DESIGN AND INSTALLATION

4.1 DESIGN CRITERIA

Load capacities shall be limited to lumber members with a minimum specific gravity of 0.50. Adjustments to allowable loads are permitted in accordance with the applicable code as referenced in this report. The maximum adjusted load shall not exceed the maximum design load shown in the tables.

The allowable loads for Simpson Strong-Tie Connectors are based on the lowest load obtained from comparing the following:

1. Test load, at which 1/8 inch (3.2 mm) deflection occurs at either end.
2. Lowest ultimate test load divided by 3.0.
3. Allowable loads on fasteners and wood, calculated in accordance with the applicable code as referenced in this report.

4.2 INSTALLATION

Connectors shall be installed in accordance with this report and the manufacturers installation instructions. Current copies of the installation instructions shall be available at all times on the job site during installation.

4.3 NAILS

Nails used with the Simpson Strong-Tie products described in this report shall comply with Federal Specification FF-N-105B and shall have the following minimum bending yield strengths, F_{yb}:

PENNYWEIGHT, COMMON TYPE	NAIL DIAMETER (in.)	F_{yb} (psi)
8d	0.131	100,000
10d	0.148	90,000
12d	0.148	90,000
16d	0.162	90,000

For SI: 1 in. = 25.4 mm, 1 psi = 6.89 kPa.

4.4 SHEET METAL COATING

Galvanized connectors conform to ASTM A653, G 60.

4.5 BOLTS

References to anchors, bolts or MB's (machine bolts) are for structural quality studs or through bolts equal to or better than ASTM Standard A307, Grade A.

5.0 IDENTIFICATION

Connectors described in this report shall be stamped with the words "Simpson Strong-Tie", the model number, and the National Evaluation Service report number, for field identification.

6.0 EVIDENCE SUBMITTED

6.1 Manufacturer's descriptive literature and published installation instructions dated January 1998.

6.2 Load tests performed in accordance with applicable provisions of ASTM D1761 witnessed by TEI Consulting Engineers, signed by Rostam Esfandiari, P.E.

6.3 Structural calculations prepared by Simpson Strong-Tie Co., Inc., signed and sealed by Karen W. Colonias, P.E., Daphne N. Schonert, P.E., and Evon M. C. Ballash, P.E.

6.4 LSU Series--torsional capacity tests performed in accordance with ASTM D1761, prepared by Testing Engineers Incorporated, File 01325, Lab MN087, dated November 30, 1981, signed by Dushyant Manmohan.

6.5 Simpson Strong-Tie Welding Operations and Procedures Manual, dated October 20, 1998, Revision 8. Signed by representatives of Simpson Strong-Tie and Professional Services Industries.

7.0 CONDITIONS OF USE

The National Evaluation Service Committee finds that the Simpson Strong-Tie® Connectors described in this report comply with the requirements of the 2000 *International Building Code*, the *BOCA National Building Code/1999*, the 1999 *Standard Building Code*, the 2000 *International Residential Code*, and the 1997 *Uniform Building Code*, subject to the following conditions:

7.1 Loads shall not exceed values shown in the tables of this report. These loads are based on the use of the tabulated fasteners, wood species with a specific gravity of 0.50 or higher, lumber moisture content less then 19 percent, and a maximum in-service temperature of 100 °F (37.8 °C).

7.2 Framing members are designed in accordance with the requirements of the applicable code, as referenced in this report.

7.3 Load capacities of connectors used under conditions different from those indicated in Section 7.1 of this report, are beyond the scope of this report and shall be verified by tests or calculations by a registered engineer.

7.4 Beams or headers shall have the following minimum widths, based on nail sizes attaching the hanger to the beams or headers:

NAIL SIZE	BEAM OR HEADER WIDTH (In.)
8d common	1.57
10d common	1.78
16d common	1.94

For SI: 1 in. = 25.4 mm.

7.5 Design calculations and details for specific applications shall be furnished to the code official verifying compliance with this report and the 2000 *International Building Code*, the *BOCA National Building Code/1999*, the 1999 *Standard Building Code*, the 2000 *International Residential Code*, and the 1997 *Uniform Building Code*, as applicable. The individual preparing such documents shall posses the necessary credentials regarding competency and qualifications as required by the applicable code and the professional registration laws of the state where the construction is undertaken.

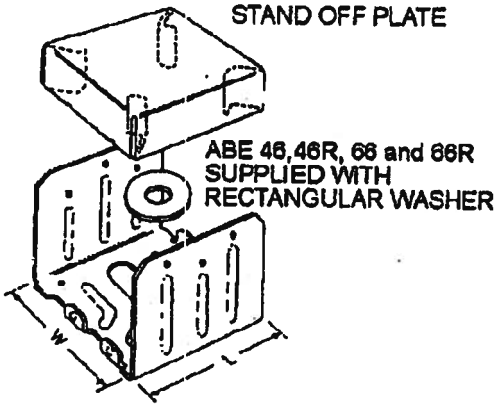
7.6 This report is subject to periodic re-examination. For information on the current status, consult the evaluation report listing or contact the NES.

Table 1 — ABE ADJUSTABLE POST BASES [1,2,3]

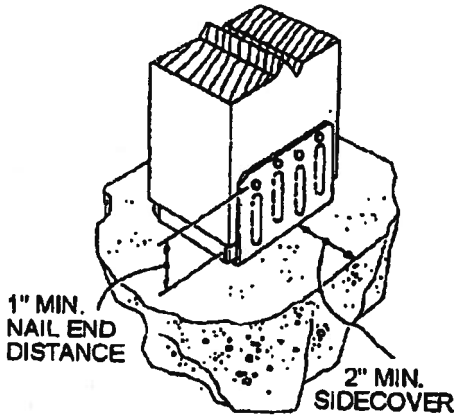
MODEL NO.	DIMENSIONS		FASTENERS [5]		ALLOWABLE LOADS [6]	
	W (in.)	L (in.)	Post (qty - size)	Anchor (in.)	Uplift [7,8] (133/160) (lbf)	Down (100) (lbf)
ABE44	3 9/16	3-1/2	6 - 10d	1/2 MB	520	6665

For SI: 1 in. = 25.4 mm; 1 psi = 6.89 kPa; 1 lbf = 4.45 N.

1. Use a 1/2 inch diameter anchor bolt embedded a minimum of 4 inches into the concrete in accordance with the applicable code.
2. Anchor bolt type, length and embedment to be specified by the designer, shall be designed to resist the uplift force, and shall be specified on the approved construction documents.
3. Minimum side cover shall be 2 inches.
4. Minimum concrete strength shall be 2,000 psi.
5. Nails shall be 0.148 inch in diameter by 3 inches long (10d common).
6. Loads shall not be increased for short-term load duration.
7. Both 133 % and 160 % load durations shall be limited to the loads listed.
8. A maximum value of 133 % load duration shall be used for wind and seismic loading in regions adopting the Uniform Building Code.



ABE 44



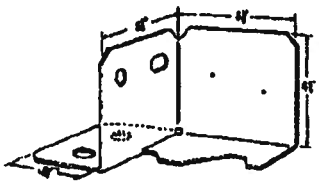
Typical ABE Installation

Table 2 — CBA ADJUSTABLE COLUMN BASES [1,2]

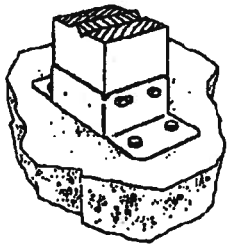
MODEL NO.	FASTENERS		ALLOWABLE UPLIFT LOAD [4,5,6,8,9] (133/160) (lbf)
	Anchor [3] qty-dia (in.)	Post qty-dia (in.)	
CBA66	4 - 3/4	2 - 5/8 MB	3,000

For SI: 1 in. = 25.4 mm; 1 psi = 6.89 kPa; 1 lbf = 4.45 N.

1. Minimum concrete strength shall be 2,000 psi.
2. Anchor bolt side cover shall be as specified on the approved construction documents.
3. Anchor bolt type, length and embedment to be specified by the designer, shall be designed to resist the uplift force, and shall be specified on the approved construction documents.
4. Loads shall not be increased for short-term load duration.
5. Loads apply only when the products are installed in pairs.
6. Loads are in pounds.
7. Allowable load has been increased 33 % and 60 % for wind or earthquake loading with no further increase allowed.
8. Both 133 % and 160 % load durations shall be limited to the loads listed.
9. A maximum value of 133 % load duration shall be used for wind and seismic loading in regions adopting the Uniform Building Code.



CBA66



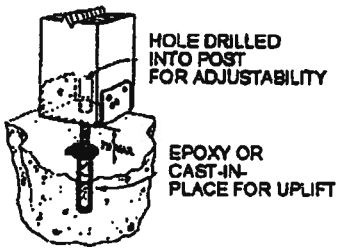
Typical CBA66 installation

Table 3 — EPB44T ELEVATED POST BASE

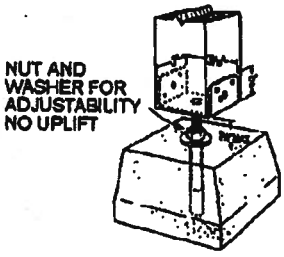
MODEL NO.	NAILS (qty - size)	ALLOWABLE LOADS (lbf)			
		Uplift		Lateral [4,5,6] (133/160)	Down (100)
		(133)	(160)		
EPB44T	6 - 10d	1130	1165	410	3275

For SI: 1 lbf = 4.45 N.

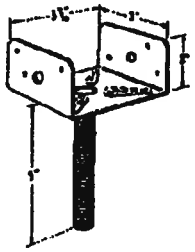
1. Load shall not be increased for short-term loading.
2. Uplift & lateral loads require the threaded rod to be set in wet concrete or attached to cured concrete with epoxy.
3. Lateral load is for both perpendicular and parallel to the connector directions.
4. Allowable load has been increased 33 % and 60 % for wind or earthquake loading with no further increases allowed.
5. Both 133 % and 160 % load durations shall be limited to the loads listed.
6. A maximum value of 133 % load duration shall be used for wind and seismic loading in regions adopting the Uniform Building Code.



EPB44T Installed with Epoxy



EPB44T Installed with Nut and Washer (not supplied)



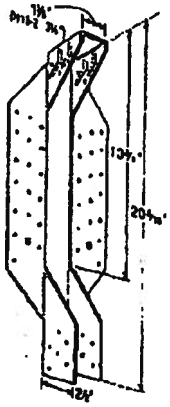
EPB44T

Table 4 — HURRICANE TIES

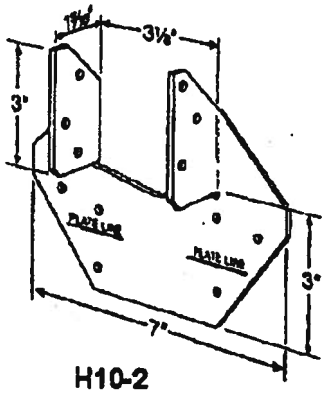
MODEL NO.	FASTENERS (qty - size)			ALLOWABLE LOADS [1,2,4,5] (lbf)			
	To Rafters	To Plates	To Studs	Uplift		Lateral (133/160)[4]	
				(133)	(160)	F1	F2
H2.5	—	4-8d×1-1/2	6-8d×1-1/2	400	400	—	—
H10-2	6-10d	6-10d	—	760	760	455	395
H15	4-10d×1-1/2	4-10d×1-1/2	12-10d×1-1/2	1300	1300	575	—
H15-2	4-10d×1-1/2	4-10d×1-1/2	12-10d×1-1/2	1300	1300	575	—

For SI: 1 in. = 25.4 mm; 1 lbf = 4.45 N.

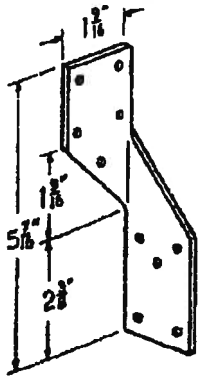
1. Loads have been increased 33 % and 60 % for wind or earthquake loading with no further increases allowed.
2. Allowable loads are for one anchor. A minimum rafter thickness of 2-1/2 inches shall be used when framing anchors are installed on each side of the joist and on the same side of the plate.
3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to all such forces shall be provided where required.
4. Both 133 % and 160 % load durations shall be limited to the loads listed.
5. A maximum value of 133 % load duration shall be used for wind and seismic loading in regions adopting the Uniform Building Code.



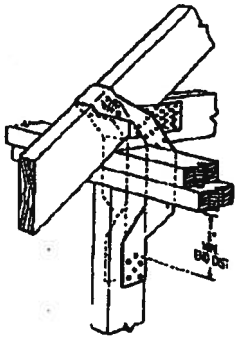
H15
(15-2 similar)



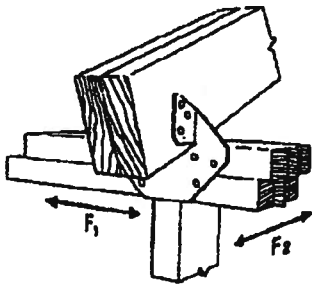
H10-2



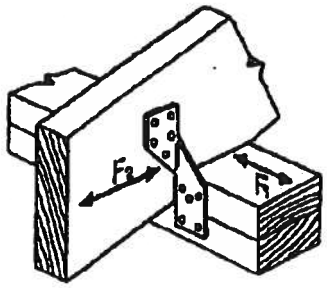
H2.5



Typical H15
Installation



H10-2 Installed



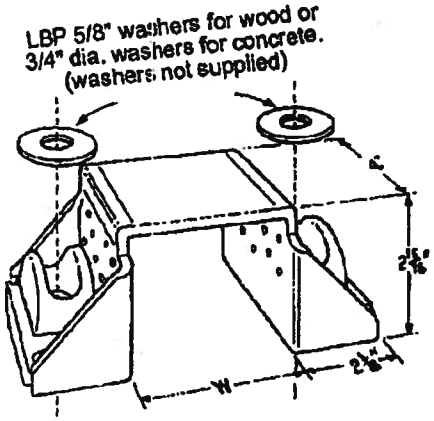
Typical H2.5
Installation

Table 5 — HGT HEAVY GIRDER TIEDOWNS

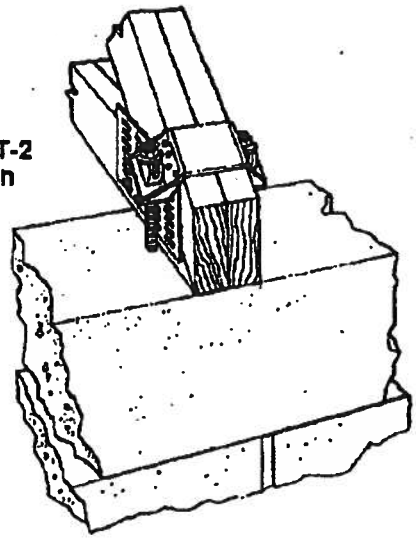
MODEL NO.	W (In.)	O.C. DIM BETWEEN ANCHORS (In.)	FASTENERS			ALLOWABLE UPLIFT (133/160)[5,6] (lbf)
			Anchor Dia. (In.)		Girder (qty - size)	
			Concrete	Wood		
HGT-2	3-7/16	5-3/4	3/4	LBP 5/8	16-10d	10980
HGT-3	5-1/8	7-1-2	3/4	LBP 5/8	16-10d	10530
HGT-4	6-3/4	9-3/6	3/4	LBP 5/8	16-10d	10530

For SI: 1 in. = 25.4 mm; 1 lbf = 4.45 N.

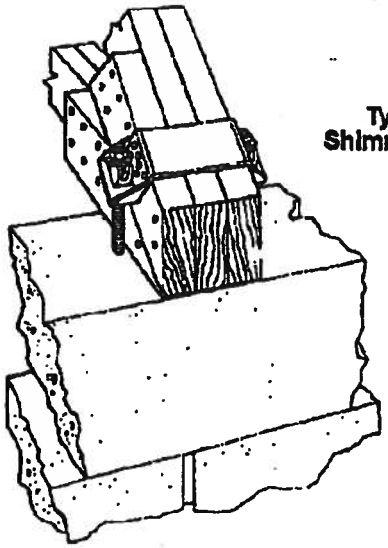
1. Attached members shall be designed to resist applied loads.
2. Allowable loads have been increased 33 % and 60 % for wind or earthquake loading with no further increase allowed.
3. When the HGT-3 is used with a 2-ply girder or beam, shimming shall be required. Shimming shall be a similar size and grade of lumber as the girder, and the entire assembly shall be fastened to act as one unit.
4. Anchor bolt type, length and embedment to be specified by the designer, shall be designed to resist the uplift force, and shall be specified on the approved construction documents.
5. Both 133 % and 160 % load durations shall be limited to the loads listed.
6. A maximum value of 133 % load duration shall be used for wind and seismic loading in regions adopting the Uniform Building Code.



HGT-2
(HGT-3 & 4 similar)



Typical HGT-2 Installation



Typical HGT-3 Shimmed Installation

Table 6 — LSSU FIELD SLOPE AND SKEWABLE HANGERS [1,2]

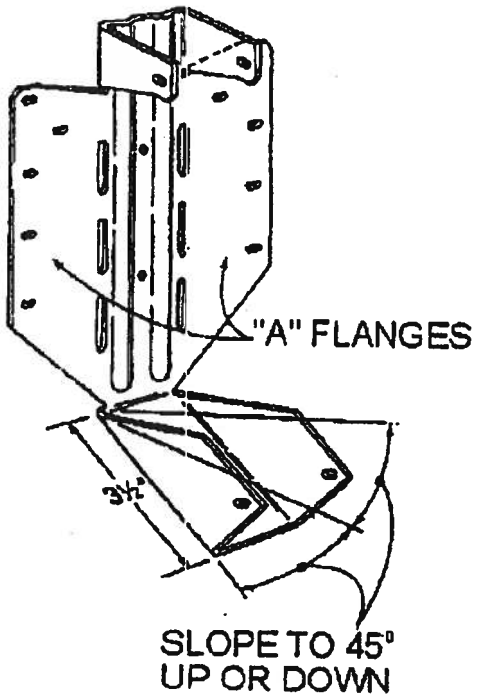
MODEL NO.	DIMENSIONS		FASTENERS [3]		ALLOWABLE LOADS					
					Uplift [4,5]		Slope Only		Skewed [3,6]	
	W (in.)	H (in.)	Header (qty-size)	Joist (qty-size)	(133)	(160)	Norm (100)	Max (125)	Norm (100)	Max (125)
LSSU28	1-9/16	7-1/8	10-10d	5-10d×1-1/2	450	450	1110	1390	990	990
LSSU210	1-9/16	8-1/2	10-10d	7-10d×1-1/2	730	785	1110	1390	1000	1205
LSSU125	1-13/16	8-1/2	10-10d	7-10d×1-1/2	730	785	1110	1390	1000	1205
LSSU135	5-5/16	8-1/2	10-10d	7-10d×1-1/2	730	785	1110	1390	1000	1205

For SI: 1 in. = 25.4 mm; 1 lbf = 4.45 N.

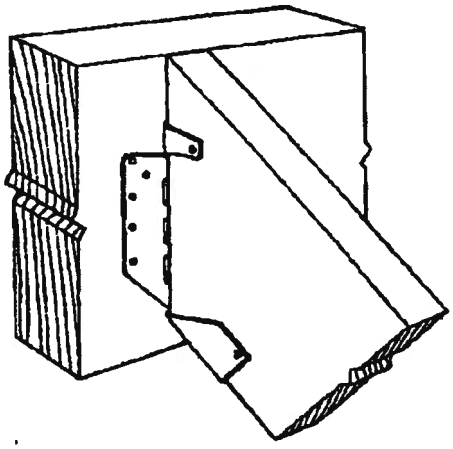
1. Loads are in pounds force.
2. Torsional capacity is 75 pounds times joist depth, summarized as follows:

JOIST DEPTH (in.)	TORSIONAL CAPACITY (lbf)
8	600
10	750

3. Nails shall be 0.148 inch in diameter by 3 inches long (10d common) and 0.148 inch in diameter by 1½ inches long for N10.
4. Uplift loads have been increased 33 % and 60 % for wind or earthquake loading with no further increase allowed.
5. Use nine (9) header nails for skewed LSSU28, LSSU210, LSSU125, LSSU135.
6. A maximum value of 133 % load duration shall be used for wind and seismic loading in regions adopting the Uniform Building Code.



LSSU28



Typical LSSU
Sloped Installation

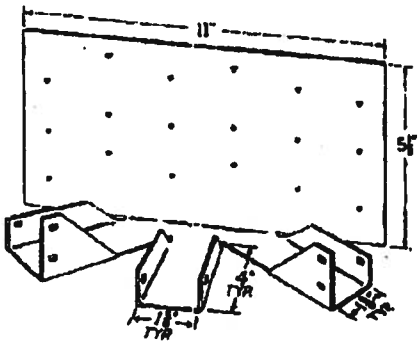
Table 7 — LTHMA LIGHT MULTIPLE TRUSS HANGER

MODEL NO.	HEADER NOMINAL SIZE (In.)	FASTENERS (qty - size)		
		Header	Hips	Jack
LTHMA	1 ply 2×4	12-10d×1-1/2	6-10d×1-1/2	2-10d×1-1/2
	2 ply 2×4	12-10d	6-10d×1-1/2	2-10d×1-1/2
	1 ply 2×6	18-10d×1-1/2	6-10d×1-1/2	2-10d×1-1/2
	2 ply 2×6	18-10d	6-10d×1-1/2	2-10d×1-1/2

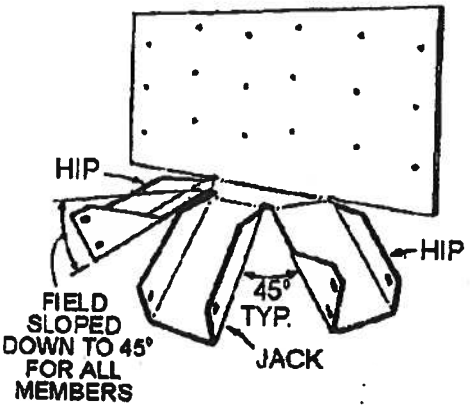
MODEL NO.	HEADER NOMINAL SIZE (In.)	WOOD SPECIES	ALLOWABLE LOADS (lbf)											
			Uplift (133/160)[6]			Floor (100)			Snow (115)			Roof (125/133/160)[7,8]		
			Hip	Jack	Total	Hip	Jack	Total	Hip	Jack	Total	Hip	Jack	Total
LTHMA	1 ply 2×4	Douglas Fir	55	20	130	485	110	1080	540	125	1205	540	125	1205
	2 ply 2×4		55	20	130	600	130	1330	675	150	1500	675	150	1500
	1 ply 2×6		55	20	130	635	140	1410	635	140	1410	635	140	1410
	2 ply 2×6		85	25	195	900	200	2000	1035	230	2300	1050	240	2340

For SI: 1 in. = 25.4 mm; 1 lbf = 4.45 N.

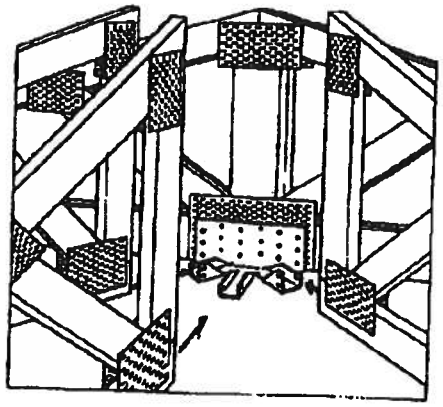
1. The total load is the sum of all three carried members.
2. Uplift loads include 133 % and 160 % increase for wind and earthquake loading with no further increases allowed.
3. Snow and roof loads are 115 % and 125 % of floor, respectively, unless limited by other criteria.
4. Combine two hips and one jack load for total capacity.
5. Total load shall be evenly distributed about the centerline to avoid eccentric loading.
6. Both 133 % and 160 % load durations shall be limited to the loads listed.
7. 125%, 133 % and 160 % load durations shall be limited to the loads listed.
8. A maximum value of 133 % load duration shall be used for wind and seismic loading in regions adopting the Uniform Building Code



LTHMA with jacks and hips non-sloped



LTHMA with jacks and hips sloped down 45°



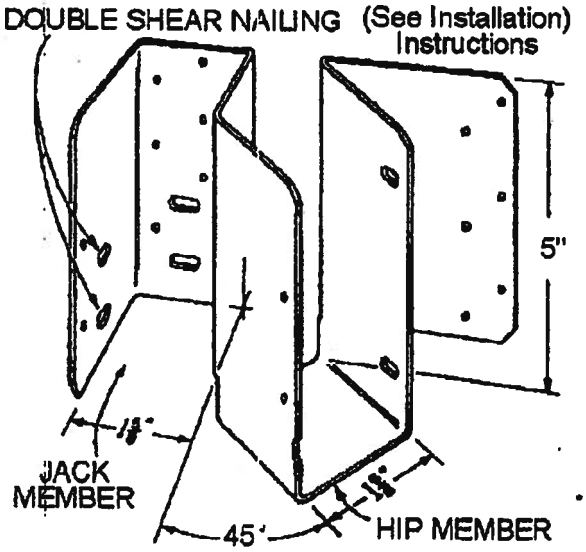
Typical LTHMA Installation

Table 8 — LTHJ HIP/JACK HANGERS [1]

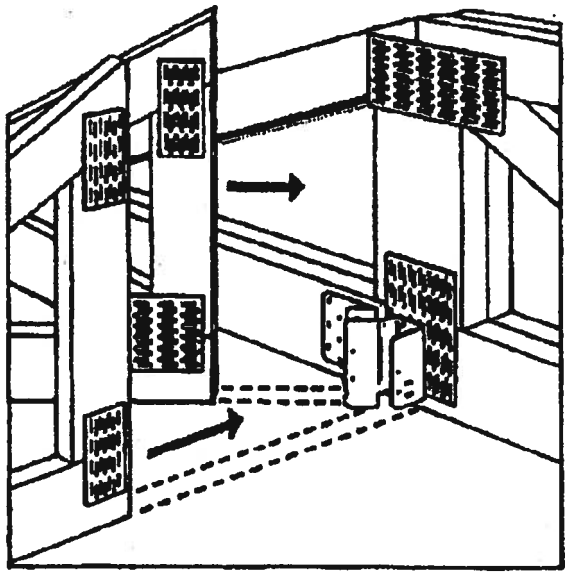
MODEL NO.	FASTENERS [2]			AVG ULT (lbf)	ALLOWABLE LOADS [3,4] (lbf)				
					Uplift			Roof	
	Header (qty-size)	Hip (qty-size)	Jack (qty-size)		(133)	(160)	Floor (100)	Snow (115)	Const (125)
LTHJR/L	12-10d	4-10d×1-1/2	2-10d×1-1/2 and 2-10d	7,317	HIP				
					490	590	1150	1320	1435
					JACK				
					250	250	385	440	480

For SI: 1 in. = 25.4 mm; 1 lbf = 4.45 N.

1. Use of the hanger to laterally support members is beyond the scope of this report.
2. Nails shall be 0.148 inch in diameter by 3 inches long (10d common) and 0.148 inch in diameter by 1½ inches long for N10. The 16d nails shall be 0.162 inch in diameter by 3½ inches long.
3. Distribute 75 % maximum of the total load to the hip member and 25 % maximum to the jack member.
4. Uplift loads have been increased 33 % and 60 % for wind or earthquake loading with no further increase allowed.



LTHJR
Hip Skewed 45° Right
(LTHJL similar)



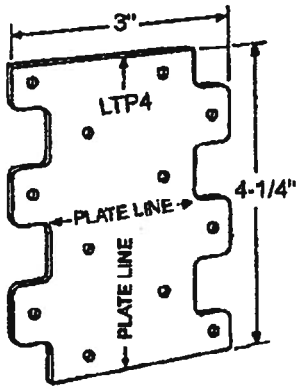
Typical LTHJL Installation

Table 9 — LTP LATERAL TIE PLATE

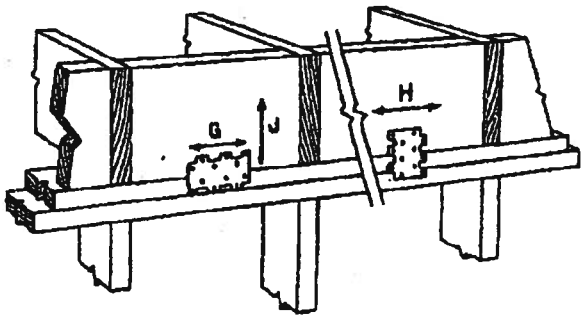
MODEL NO.	FASTENERS (qty-size)		DIRECTION OF LOAD	ALLOWABLE LOADS [1] (lbf)		
	Plates	Joist		(100)	(125)	(133)
LTP4	6-8d×1-1/2	6-8d×1-1/2	G	515	645	685
	6-8d×1-1/2	6-8d×1-1/2	J	515	645	685
	6-8d×1-1/2	6-8d×1-1/2	H	515	645	685

For SI: 1 in. = 25.4 mm; 1 lbf = 4.45 N.

1. 125 % and 133 % values are permitted to be used for roof and wind or earthquake loading, respectively.



LTP4
Lateral Tie Plate



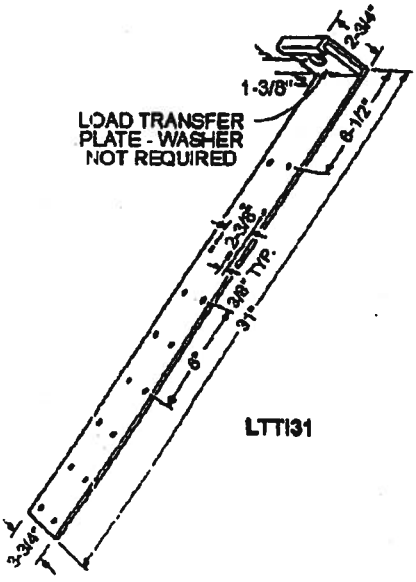
Typical LTP4 Installations to Transfer Shear Forces

Table 10 — LTTI TENSION TIE

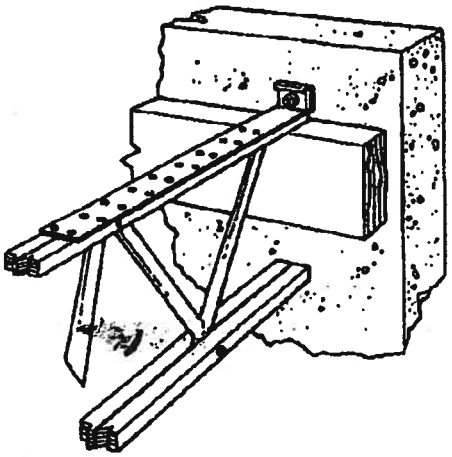
MODEL NO.	DIMENSIONS (In.)		FASTENERS		ALLOWABLE LOADS [2] (lbf)	
					Douglas Fir	
	W	L	Anchor Diameter (In.)	Nails (qty-size)	Tension (133)	Compression (133)
LTT131	3-3/4	31	5/8	18-10d×1-1/2	1805	305

For SI: 1 in. = 25.4 mm; 1 lbf = 4.45 N.

1. Anchor bolt type, length and embedment to be specified by the designer, shall be designed to resist the tension force, and shall be specified on the approved construction documents.
2. Allowable loads have been increased 33 % for wind or earthquake loads with no further increases allowed.



LTTI31



Typical LTTI31 Installation

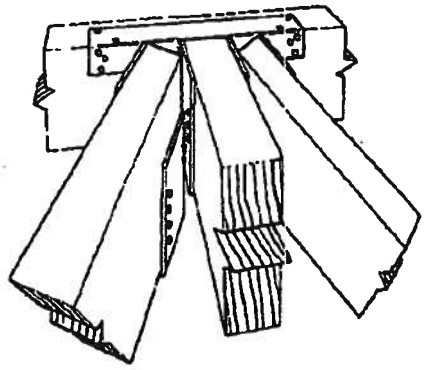
Table 11 — MSC MULTIPLE SEAT CONNECTORS

MODEL NO.	DIMENSIONS (in.)				HIPS		FASTENERS (qty-size)	
	W	H	TF	L	Max. Skew	Max. Slope	Header	Joists
MSC2	1-9/16	5-1/2 min.	2-7/8	12	45°	0°	10-16d	18-10d×1-1/2
						45°	10-16d	26-10d×1-1/2
MSC1.81	1-13/16	5-1/2 min.	2-7/8	12	45°	0°	10-16d	18-10d×1-1/2
						45°	10-16d	26-10d×1-1/2
MSC4	3-9/16	7-1/2 min.	2-7/8	18	45°	0°	10-16d	18-10d
						45°	10-16d	26-10d

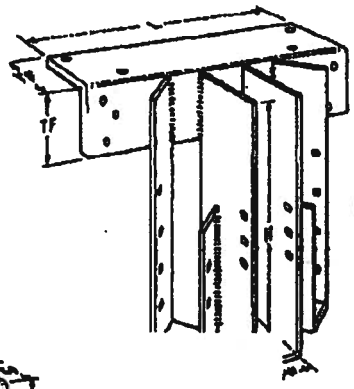
MODEL NO.	HIPS		ALLOWABLE LOADS (lbf)					
			Floor (100)			Snow (115) / Roof (125)[7]		
	Max. Skew	Max. Slope	Hip	Jack	Total	Hip	Jack	Total
MSC2	45°	0°	2535	1265	6335	2535	1265	6335
		45°	2010	1005	5025	2010	1005	5025
MSC1.81	45°	0°	2535	1265	6335	2535	1265	6335
		45°	2010	1005	5025	2010	1005	5025
MSC4	45°	0°	3335	1665	8335	3335	1665	8335
		45°	3335	1665	8335	3335	1665	8335

For SI: 1 in. = 25.4 mm, 1 lbf = 4.45 N.

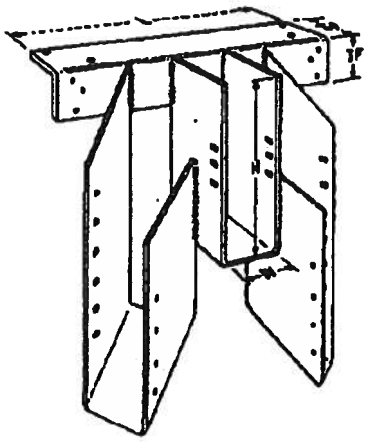
1. Allowable loads are per member.
2. Snow and roof loads are 115 % and 125 % of floor, respectively, unless limited by other criteria.
3. For hips with combined skew and slope angles > 0° and ≤ 45°, use load values for maximum skew = 45°, and maximum slope= 45°.
4. Use total load for cases when there is no center member.
5. W1 = W2 = W3 unless specified otherwise.
6. Total load shall be evenly distributed about the centerline to avoid eccentric loading.
7. Both 115 % and 125 % load durations shall be limited to the loads listed.



Typical MSC410 Installation



MSC1.81 Hips are skewed 45° and sloped 0°



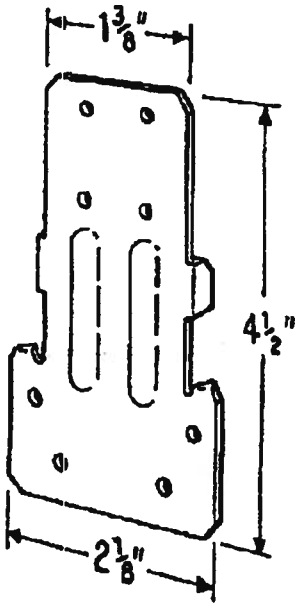
MSC410 Hips are sloped and skewed 45°

Table 12 — RSP4 REVERSIBLE STUD PLATE TIE

MODEL NO.	W (in.)	L (in.)	FASTENERS		ALLOWABLE LOADS (lbf)		
			Stud (qty-size)	Plate (qty-size)	Uplift[4,5] (133/160)	Lateral (133/160)[4,5]	
						F1[1]	F2[2]
RSP4(1)	2-1/8	4-1/2	4-8d×1-1/2	4-8d×1-1/2	315	210	280
RSP4(2)	2-1/8	4-1/2	4-8d×1-1/2	4-8d×1-1/2	450	210	305

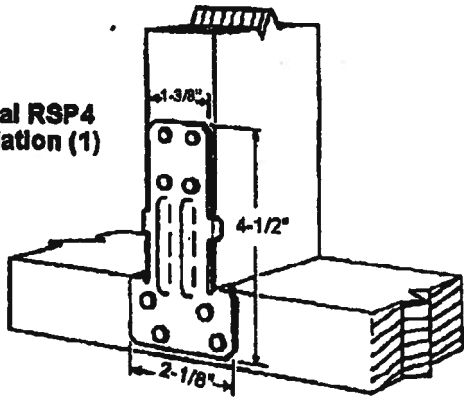
For SI: 1 in. = 25.4 mm; 1 lbf = 4.45 N

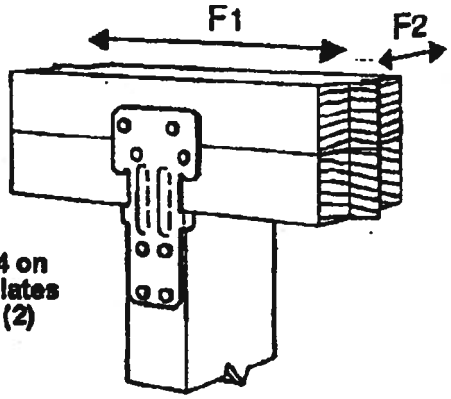
1. "F1" denotes direction parallel to plate load.
2. "F2" denotes direction perpendicular to plate load.
3. Allowable loads have been increased 33 % and 60 % for wind or earthquake load with no further increase allowed.
4. Both 133 % and 160 % load durations shall be limited to the loads listed.
5. A maximum value of 133 % load duration shall be used for wind and seismic loading in regions adopting the Uniform Building Code.



RSP4

Typical RSP4 Installation (1)





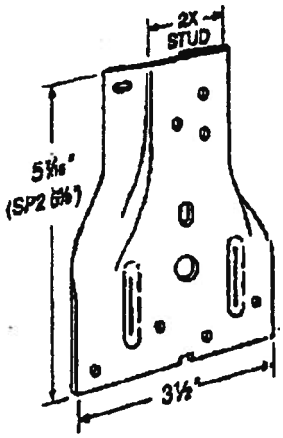
Typical RSP4 on Double Top Plates Installation (2)

TABLE 13 - SP STUD PLATE CONNECTORS

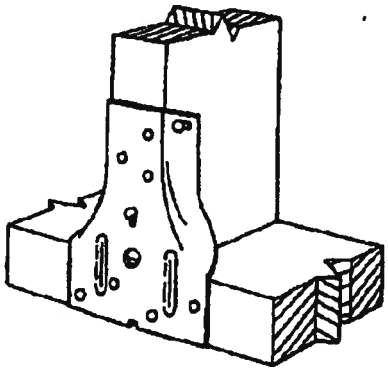
MODEL NO.	FASTENERS [2] (qty-size)		ALLOWABLE UPLIFT LOAD [3,4] (lbf)	
	Stud [1]	Plate	(133)	(160)
SP1	6-10d	4-10d	585	585
SP2	6-10d	6-10d	890	1065

For SI: 1 in. = 25.4 mm; 1 lbf = 4.45 N.

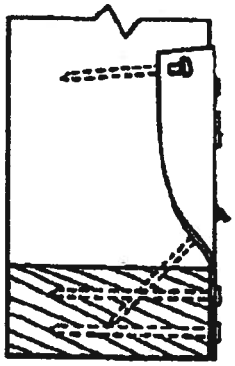
1. One stud nail shall be driven at an angle through the stud into the plate.
2. Nails shall be 0.148 inch in diameter by 3 inches long (10d common).
3. Allowable load includes a 33 % and 60 % increase for wind or earthquake loading with no further increase allowed.
4. A maximum value of 133 % load duration shall be used for wind and seismic loading in regions adopting the Uniform Building Code.



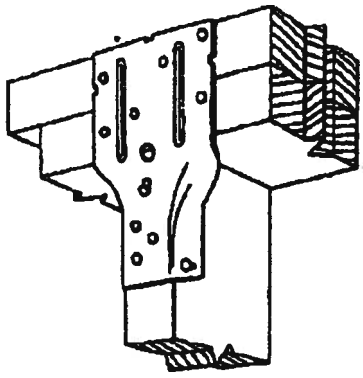
SP1



Typical SP1 Installation



SP1 Nailing Profile



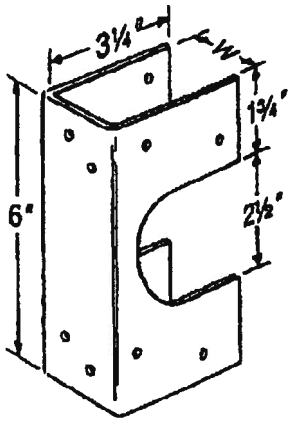
Typical SP2 Installation

Table 14 — SS STUD SHOES

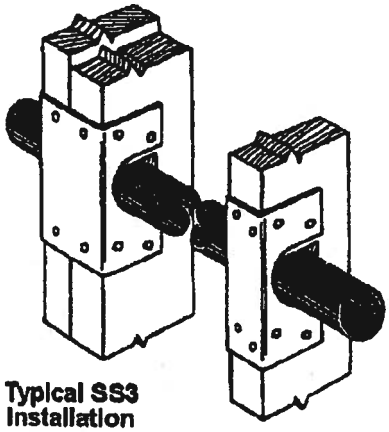
MODEL NO.	W (in.)	FASTENERS[1] (qty-size)	ALLOWABLE LOADS (lbf)		
			Floor (100)	Roof[2] (125)	Uplift (133)
SS1.5	1-9/16	12-10d×1-1/2	500	500	----
SS2.5	2-9/16	12-10d×1-1/2	500	500	----
SS3	3	12-10d	665	785	----
SS4.5	4-9/16	14-10d×1-1/2	665	785	----
HSS2	1-9/16	12-SDS 1/4×1-1/2	1215	1215	1025
HSS2-2	3	12-SDS 1/4×1-1/2	1215	1215	1025
HSS4	3-9/16	12-SDS 1/4×1-1/2	1215	1215	1025

For SI: 1 in. = 25.4 mm; 1 lb. = 4.45 N.

1. Nails shall be 0.148 inch in diameter by 1½ (N10) or 3 inches long (10d common).
2. Roof loads are 125 % of floor loads, unless limited by other criteria.

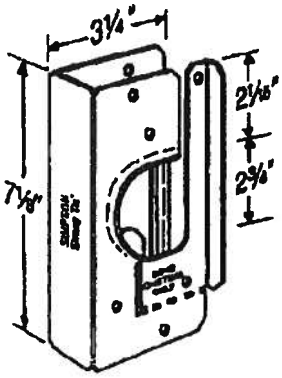


SS

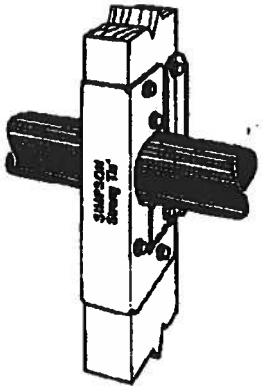


Typical SS3 Installation

Typical SS1.5 Installation



HSS



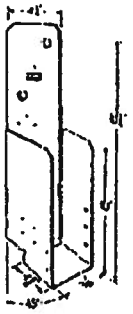
Typical HSS Installation

TABLE 15 — THG2A SKEWED TRUSS GIRDER HANGERS [1]

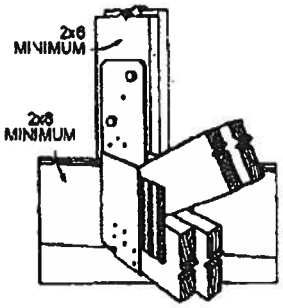
MODEL NO.	FASTENERS [2]			ALLOWABLE LOADS [4] (lbf)				
	Carrying Member		Carried Member (qty-size)	Uplift [3] (133)	Length of Bolt in Carrying Member	Floor (100)	Roof [5]	
	Bolts (in.)	Nails (qty-size)					Snow (115)	Const (125)
THG2AR/L	2-3/4 MB	4-10d	9-10d	1465	1-1/2	1385	1595	1735
					3	2750	3160	3435
					4-1/2	2985	3435	3730
					6	2985	3435	3730

For SI: 1 in. = 25.4 mm; 1 psi = 6.89 kPa; 1 lbf = 4.45 N.

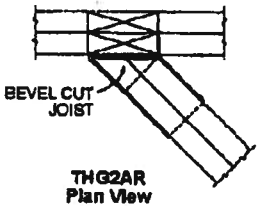
1. Use of the hanger to laterally support trusses is beyond the scope of this report.
2. Nails shall be 0.148 inch in diameter by 3 inches long (10d common).
3. Uplift loads have been increased by 33% for wind or earthquake, with no further increase allowed.
4. The allowable loads given are based on southern pine lumber and the lower of the following: test ultimate divided by three, the load producing 1/8 inch deflection, the bolt values and the seat bearing value at 565 psi plus the allowable joist nail rating.
5. Down loads given include a 25 % increase above the normal allowable load for a seven-day load duration. Load adjustments for other load durations in accordance with the applicable code, as referenced herein, are permitted but shall not exceed the table values.



THG2AR



Typical THG2AR Installation



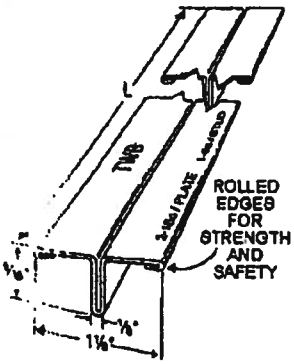
THG2AR
Plan View

TABLE 16 — TWB T-TYPE WALL BRACING [1,2]

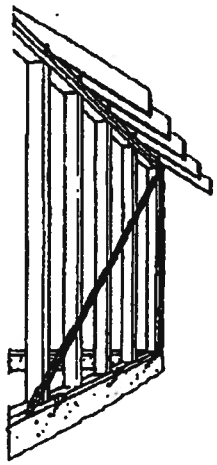
MODEL NO.	LENGTH	ANGLE FOR 8 ft WALL	FASTENERS [3] (qty-size)	
			Plates	Studs
TWB10	9'-3"	55°	2-16d	1-8d
TWB12	11'-4"	45°	2-16d	1-8d
TWB14	14'-2"	45°	2-16d	1-8d

For SI: 1 ft = 304.8 mm; 1 in. = 25.4 mm; 1 lbf = 4.45 N.

1. The TWB is intended to provide racking resistance to woodframe walls during construction. It is not designed to replace the shearwall load-carrying components.
2. The TWB10 shall be limited to a maximum load of 160 pounds. The TWB12 and TWB14 shall be limited to a maximum load of 190 pounds.
3. The 16d nails shall be 0.162 inch in diameter by 3½ inches long; the 8d nails shall be 0.131 inch in diameter by 2½ inches long.



TWB



Typical TWB
Exterior Wall Installation

THE DRAWINGS CONTAINED WITHIN THIS REPORT ARE FOR ILLUSTRATION PURPOSES ONLY. THEY ARE NOT INTENDED FOR USE AS CONSTRUCTION DOCUMENTS FOR THE PURPOSE OF DESIGN, FABRICATION OR ERECTION.

National Evaluation Service, Inc.

Participating Members:

BOCA Evaluation Services, Inc. | ICBO Evaluation Service, Inc. | SBCCI Public Safety Testing and Evaluation Services, Inc.

4051 West Flossmoor Road, Country Club Hills, Illinois 60478-5795, (708) 799-2305
5360 Workman Mill Road, Whittier, California 90601-2299, (310) 699-0543
900 Montclair Road, Suite A, Birmingham, Alabama 35213-1208, (205) 591-1853

NATIONAL EVALUATION REPORT

NER-443
Reissued December 1, 1996

SIMPSON STRONG-TIE® CONNECTORS

SIMPSON STRONG-TIE® COMPANY, INC.
4637 CHABOT DRIVE, SUITE 200
PLEASANTON, CALIFORNIA 94588

1.0 SUBJECT

Simpson Strong-Tie® Connectors:

1.1 CWB Compression Wall Brace
1.2 GH Girder Hangers
1.3 GLTV and HGLTV Beam Hangers
1.4 LPC4 Light Post Cap
1.5 LS Skewable Angle Series
1.6 LSTA/MSTA Light and Medium Strap Tie Series
1.7 LTB Light Tension Bridging
1.8 MA Series Mudsill Anchors
1.9 PB Series Post Bases
1.10 PC/EPC Series Post Caps
1.11 SP/SPA Stud Plate Ties
1.12 THM-2 Truss Multiple Hanger
1.13 WB/WBC Wall Bracing
1.14 DJT14 Deck Joist Tie
1.15 DBT1 Deck Board Tie
1.16 DPT Deck Post Ties
1.17 DRT 8 Deck Railing Tie

2.0 PROPERTY FOR WHICH EVALUATION IS SOUGHT

Structural Connections for Wood Construction.

3.0 DESCRIPTION

3.1 CWB COMPRESSION WALL BRACE

The CWB106 and CWB126 braces are cold-formed to a 90-degree angle from No. 18 gage galvanized steel complying with ASTM A 653-SQ Grade 33, with a minimum yield strength of 33,000 psi (230 MPa) and a minimum tensile strength of 45,000 psi (310 MPa). Angle legs are $^{15}/_{16}$ inch (23.8 mm) long. One leg provides a staggered pattern of 0.136-inch-diameter (3.45 mm) holes spaced at $^{1}/_{2}$ inch (12.7 mm) on center. The brace ends have angular cuts and a three-hole pattern of 0.171-inch-diameter (4.34 mm) holes. The CWB106 and the CWB126 braces are 9 feet $5^{3}/_{4}$ inches (2.89 m) and 11 feet $4^{3}/_{8}$ inches (3.46 m) long, respectively. The devices are used to brace wood-frame construction, with a single brace considered equivalent to one nominal 1-by-4 wood let-in brace. The braces have been evaluated for both tension and compression loads. The CWB106 and CWB126 braces shall be installed at angles of 60 and 45 degrees from the horizontal, respectively. The wall studs shall be spaced at 16 inches (406 mm) on center, maximum. A 1-inch-deep (25.4 mm) saw cut shall be provided in the studs and plates for installation of the brace. The brace shall be attached at both the top and bottom plates with two 16d common nails and at each intermediate stud with one 8d common nail. Dimension details and fastener schedules shall be in accordance with Table 1.

3.2 GH GIRDER HANGERS

The GH46 and GH48 girder hangers provide support for floor girders connected to concrete or grouted masonry foundation walls, complying with the applicable code. The devices are No. 12 gage painted steel. The U-shaped stirrups are welded to the face of a semi-U top which is 6 inches (153 mm) wide and not less than 6 inches (153 mm) in depth on one face with a 1-inch (25.4 mm) return leg. The device shall be mounted on top of the foundation wall and under a minimum nominal 2-inch-by-6-inch (51 mm by 153 mm) mudsill, which shall be installed in accordance with the applicable code. The steel complies with ASTM A 570 Grade 33, with a minimum yield of 33,000 psi (230 MPa) and a minimum tensile strength of 52,000 psi (360 MPa). Dimension details, allowable loads and fastener schedules shall be in accordance with Table 2.

3.3 GLTV AND HGLTV BEAM HANGERS

The GLTV stirrup is formed from a strip of sheet steel, No. 7 gage by 5 inches (127 mm) wide, bent into a "U" shape and welded to an angled top flange made of No. 3 gage steel. The hangers are designed for use with structural composite lumbers. They shall be installed on a wood header having a minimum allowable compres-

sion perpendicular-to-the-grain value of 500 psi (3447 kPa), or on a steel header. The HGLTV is similar, except that the stirrup is 6 inches (153 mm) wide and the top flange dimension, nailing schedule and welds are increased. The steel complies with ASTM A 570 Grade 33 specifications, with a minimum yield strength of 33,000 psi (230 MPa) and a minimum tensile strength of 52,000 psi (360 MPa). Dimension details, allowable loads and fastener schedules shall be in accordance with Table 3.

3.4 LPC4 LIGHT POST CAP

The light post cap is a two-piece connector formed from No. 18 gage galvanized steel. The connector is designed to join a nominal 4-by post to members $2^1/_2$ to $3^1/_2$ inches (63.5 mm to 88.9 mm) wide. The connectors shall be used in pairs. The steel complies with ASTM A 653-SQ Grade 33, with a minimum yield strength of 33,000 psi (230 MPa) and a minimum tensile strength of 45,000 psi (310 MPa). Allowable loads and the fastener schedule shall be in accordance with Table 4.

3.5 LS SKEWABLE ANGLE SERIES

The skewable angle is formed from No. 18 gage, galvanized, die-formed steel complying with ASTM A 653-LFQ, with a minimum yield strength of 33,000 psi (230 MPa) and a minimum tensile strength of 45,000 psi (310 MPa). The angle is designed with slots at the bend to allow field skewing from 0 to 135 degrees. Allowable loads and fastener schedules shall be in accordance with Table 5.

3.6 LSTA/MSTA LIGHT AND MEDIUM STRAP TIE SERIES

The light and medium straps are designed to act as tension ties between two butting wood members. The straps are formed from Nos. 16, 18 and 20 gage galvanized steel and punched to receive 10d or 16d common nails. The steel complies with ASTM A 653-SQ Grade 40 Special, with a minimum yield strength of 42,000 psi (290 MPa) and a minimum tensile strength of 56,000 psi (390 MPa). Allowable loads and fastener schedules shall be in accordance with Table 6.

3.7 LTB LIGHT TENSION BRIDGING

The LTB is die-formed from No. 22 gage galvanized steel complying with ASTM A 653-LFQ, with a minimum yield strength of 33,000 psi (230 MPa) and minimum tensile strength of 45,000 psi (310 MPa) for LTB21 and LTB42; and a 28,000 psi (195 MPa) yield strength and 38,000 psi (260 MPa) tensile strength for LTB20 and LTB40. The bridging is a tension-type utilizing either a single or an over/under application. The bridging shall be installed in pairs. Installation details and the fastener schedule shall be in accordance with Table 7.

3.8 MA SERIES MUDSILL ANCHORS

The mudsill anchors, formed from No. 16 gage galvanized steel, have a tapered, U-shaped body with the outboard flanges extending $4^1/_2$ inches (114.3 mm) upwards beyond the web section to bear against the sides and top of the mudsill. The web sections are $4^5/_8$ inches (117.5 mm) deep with a top width between outboard flanges of either $3^5/_8$ or $5^5/_8$ inches (92.1 mm or 142.9 mm) for nominal 4-by or 6-by sill plates, respectively. Adjacent to the flanges, at each side of the web, triangular tabs are bent horizontally to act as a gage and as a restraint to the flanges. The flanges are prepunched with $^5/_{32}$-inch-diameter (4 mm) holes at $^1/_4$ inch (6.4 mm) on center, staggered in two rows, for 10d common nails $1^1/_2$ inches (38 mm) long. The nails shall be installed at the edge and top of the mudsill. The steel complies with ASTM A 653-LFQ, with a minimum yield strength of 28,000 psi (195 MPa) and a minimum tensile strength of 38,000 psi (260 MPa). Allowable loads and fastener schedules shall be in accordance with Table 8.

3.9 PB SERIES POST BASES

The post bases consist of No. 12 gage, galvanized, die-formed, channel-shaped members, having deformed prongs protruding from the back of the web in line with each channel flange. The prongs shall be embedded into uncured concrete immediately after screeding to provide shear and uplift resistance for the supported post. The base-channel flanges shall be nailed to the post with 16d common nails. The post bases are manufactured to fit nominal 4-by-4, 4-by-6 and 6-by-6 surfaced and rough-sawn posts. The steel complies with ASTM A 653-CQ, with a minimum yield strength of 28,000 psi (195 MPa) and minimum tensile strength of 38,000 psi (260 MPa). Allowable loads and fastener schedules shall be in accordance with Table 9.

3.10 PC/EPC SERIES POST CAPS

The post caps are die-formed from No. 12 or No. 16 gage galvanized steel into a channel section to support beams on posts. Model numbers with a -16 suffix are formed from No. 16 gage material. The ends of the channel web are cut along each side of the beam seat and bent downward as tabs to engage opposite faces of the post. The post caps are manufactured to fit various combinations of post and beams and shall be attached to both the beam and post with 16d common nails. The EPC post caps are designed for end-post connections in lieu of continuous members. The steel complies with ASTM A 653-CQ and ASTM A 653-LFQ requirements for No. 12 gage and No. 16 gage, respectively, with a minimum yield strength of 28,000 psi (195 MPa) and a minimum tensile strength of 38,000 psi (260 MPa). Dimension details, allowable loads and fastener schedules shall be in accordance with Table 10.

3.11 SP/SPA STUD PLATE TIES

The SP/SPA 4, 6 and 8 stud plate ties are die-formed from No. 20 gage galvanized steel complying with ASTM A 653-LFQ, with a minimum yield strength of 28,000 psi (195 MPa) and a minimum tensile strength of 38,000 psi (260 MPa). The SP tie is attached to the edge of the wood member, while the SPA is twisted and attaches to the face of the member. They are designed to anchor double top plates to the stud. Dimension details, allowable loads and fastener schedules shall be in accordance with Table 11.

3.12 THM-2 TRUSS MULTIPLE HANGER

The THM-2 is designed to carry multiple truss members. The hanger is fabricated from No. 10 gage galvanized steel complying with ASTM A 653-SQ Grade 33, with a minimum yield strength of 33,000 psi (230 MPa) and a minimum tensile strength of 45,000 psi (310 MPa). The code-required minimum-7-bolt diameter distance from the end of the vertical member has been designed into the connector. The bottom cord of the carrying truss shall not exceed a nominal 2-by-8 member in order to maintain the required distance. Allowable loads and fastener schedules shall be in accordance with Table 12.

3.13 WB/WBC WALL BRACING

WB106 and WB126 wall braces are formed from $1^1/_4$-inch-wide (32 mm), No. 18 gage galvanized steel strips, complying with ASTM A 653-SQ Grade 33, with a minimum yield strength of 33,000 psi (230 MPa) and a minimum tensile strength of 45,000 psi (310 MPa). Wall bracing is also available in coiled form designated as WB106C, WB126C and WB143C straps. The braces are used in wood-frame construction, with a pair of braces equivalent to one nominal 1-by-4 let-in brace as prescribed by the applicable code. Braces shall be installed opposing each other, with each brace installed at an angle not more than 60 degrees nor less than 45 degrees from the horizontal. Four nail holes are provided at the ends of each brace for 16d nails, and shall be used to attach the top and bottom plates with two 16d common nails. Additionally, $^9/_{64}$-inch-diameter (3.6 mm) holes are punched at 1 inch (25.4 mm) on center for the entire length of the brace in two staggered rows spaced $^3/_4$ inch (19 mm) apart, to permit attachment to each intermediate stud with one 8d common nail. Dimension details and fastener schedules shall be in accordance with Table 13.

3.14 DJT14 DECK JOIST TIE

The tie is used to attach joists to posts and is die-formed from No. 14 gage galvanized steel. The connector shall be bolted or nailed to

a minimum nominal 4-by-4 post and a minimum nominal 2-by-4 joist. Steel complies with ASTM A 653-CQ, with a minimum yield strength of 28,000 psi (195 MPa) and a minimum tensile strength of 38,000 psi (260 MPa). Allowable loads and fastener schedules shall be in accordance with Table 14.

3.15 DBT1 DECK BOARD TIE

The tie is used to attach minimum $1^1/_4$-inch-thick (32 mm) board decking to joists in conjunction with toe-nailed 16d common nails and 10d by $1^1/_2$-inch (38 mm) common nails. Ties are die-formed from No. 18 gage galvanized steel complying with ASTM A 653-CQ, with a minimum yield strength of 28,000 psi (195 MPa) and a minimum tensile strength of 38,000 psi (260 MPa). Fastener installation shall be in accordance with Table 15.

3.16 DPT DECK POST TIES

The DPT 5, DPT 6 and DPT 7 are die-formed from No. 14 gage galvanized steel. The DPT 5 attaches 2-by-4 posts to the outside of a deck. The DPT 7 attaches 4-by-4 posts to the edges of a deck. Connectors shall be used in pairs spaced 5 inches (127 mm) on center. The DPT 6 attaches 4-by posts to the top surface of a deck. For the DPT 5 and DPT 7, the deck construction shall include a nominal 2-by-10 minimum fascia and a nominal 2-by-8 minimum rim joist. For the DPT 6, the deck construction shall include a nominal 2-by-8 minimum fascia and a nominal 2-by-6 minimum rim joist. Steel for the post ties complies with ASTM A 653-CQ, with a minimum yield strength of 28,000 psi (195 MPa) and a minimum tensile strength of 38,000 psi (260 MPa). Allowable loads and fastener schedules shall be in accordance with Table 16.

3.17 DRT 8 DECK RAILING TIE

The DRT 8 is die-formed from No. 18 gage galvanized steel, and connects the handrail to a post. The connector shall be attached to the post and handrail with wood screws. Steel complies with ASTM A 653-CQ, with a minimum yield strength of 28,000 psi (195 MPa) and a minimum tensile strength of 38,000 psi (260 MPa). Allowable loads and the fastener schedule shall be in accordance with Table 17.

3.18 MATERIALS

Galvanized connectors conform to ASTM A 653, G 60. Nongalvanized connectors have a painted coating.

Nails shall be common nails and have a diameter, length and bending yield strength complying with the values noted in the 1991 National Design Specification for Wood Construction, except for the length of special nails noted in Tables 8, 12, 15 and 16.

3.19 DESIGN

The design of the connected wood members shall be submitted to and approved by the building official. Tabulated design loads for the Simpson Strong-Tie connectors are based on the following criteria:

- Test load that causes $^1/_8$-inch (3 mm) deflection.
- Lowest ultimate test load with a safety factor of 3.
- Allowable fastener and compression perpendicular-to-grain values in accordance with the 1991 National Design Specification for Wood Construction, based on wood with a specific gravity of 0.50, such as Douglas fir-larch, except for allowable loads noted in Tables 16 and 17, which have also been evaluated for redwood.
- Torsional capacity is based on the ability of the joist hanger to resist 75 pounds (334 N) times the depth of the joist at 0.125 inch (3 mm) of movement.

4.0 INSTALLATION

Load capacities shown are based on wood with a minimum specific gravity of 0.50 and a moisture content of less than 19 percent. Tabulated allowable design loads are for normal duration of loading. Adjustments to these values are permitted for other durations of loading, i.e., plus 15 percent for two months duration (snow), or plus 33 percent for wind or earthquake. Tabulated allowable design loads shall be reduced by 10 percent for full design load applied longer than 10 years. The resulting allowable design load after duration-of-load adjustments shall not exceed the maximum design load indicated in the tables.

Connector installation shall comply with this report and the manufacturer's installation instructions. Connectors for Wood Construction, Product and Instruction Manual, dated January 1995. A copy of these instructions and this report shall be available at all times on the jobsite during installation.

For conditions of high temperature, a load reduction is required. See the National Design Specification for temperature effects.

5.0 IDENTIFICATION

Each of the connectors described in this report shall be stamped with the words "Simpson Strong-Tie," the model number and the evaluation report number (NER-443) for field identification.

6.0 EVIDENCE SUBMITTED

6.1 Manufacturer's descriptive literature and published installation instructions.

6.2 Load tests performed by TEI Consulting Engineers and signed by Rostam Esfandiari, P.E.:

Item	Work No.	Date
CWB	85328	April 22, 1986
CWB	85328-62	August 25, 1986
GH46	93085.120	November 29, 1993
GLTV	95001.100	July 26, 1995
HGLTV	95001.099	July 12, 1995
LPC4	89008.136	February 27, 1990
LPC4	89008.135	February 27, 1990
LS30	93085.118	November 24, 1993
LS50	93085.119	November 24, 1993
LS90	89008-46	August 2, 1989
LS90	89008-30	July 20, 1989
LS90	89008-47	August 2, 1989
MA4	OL16-290-424	February 22, 1974
PB66	86085	August 5, 1986
PB44	88018.190	March 31, 1989
PB44	88018.185	March 31, 1989
PB44	89008-25	June 5, 1989
PC-46	85328-139	February 23, 1987
EPC-46	85328-140	February 23, 1987
PC46	88018-20	April 29, 1988
EPC46	88-18-19	April 29, 1988
PC44-16	88018-116	October 19, 1988
EPC44-16	88018-115	October 19, 1988
PC48-16	88018-117	October 19, 1988
GLTI430	87005-16	May 18, 1987
EPC48-16	88018-118	October 19, 1988
PC44	88018-97	October 7, 1988
EPC44	88018-96	October 6, 1988
PC48	88018.103	October 12, 1988
EPC48	88018-102	October 12, 1988
LSTA	89008-48	August 8, 1989
THM-2	89008-95	November 15, 1989

Item	Work No.	Date
THM-2	88-18-57	July 29, 1988
DJT14	89008-101	November 15, 1989
DPT5 & DRT8	91006-38, 91006-44	July 31, 1991
DPT6 & DRT8	91006-41	July 31, 1991
DPT6 & DRT8	91006-98	October 23, 1991
DPT7 & DRT8	91006-39, 91006-45	July 31, 1991

6.3 Structural calculations prepared by Simpson Strong-Tie Company, Inc., and signed and sealed by Karen W. Colonias, P.E.:

Item	Date
CWB	April 12, 1990
GH	April 12, 1990, and February 28, 1996
GLTV/HGLTV	April 12, 1990, and February 22, 1996
LPC4	April 12, 1990
LS	April 12, 1990, and February 28, 1996
LSTA/MSTA	April 12, 1990, revised May 10, 1996
LTB	February 12, 1990
MA	June 23, 1987
PB	July 26, 1988, revised April 12, 1990
PC/EPC	April 12, 1990, revised May 10, 1996
SP4, 6, 8	August 10, 1989
LSTA	November 3, 1989
THM2	August 18, 1988, revised August 24, 1990
GTLV/HGLTV	August 24, 1990
WB/WBC	September 20, 1991
DJT14	October 25, 1991, revised May 10, 1996

6.4 Load tests performed by TEI Consultants and signed by Roger Tansley, P.E.:

Item	Work No.	Date
GLTV410	95001.100	July 26, 1995
HGLTV410	95001.099	July 12, 1995

6.5 Letter dated July 14, 1995, from TEI Consultants and signed by Roger Tansley, P.E., regarding verification of testing in accordance with ASTM D 1761.

6.6 Quality control manual, dated December 1994.

7.0 CONDITIONS OF USE

The National Evaluation Service Committee finds that the Simpson Strong-Tie® connectors described in this report comply with the BOCA National Building Code/1996, the 1994 Standard Building Code, and the 1994 Uniform Building Code, subject to the following conditions:

7.1 Connector loads are determined in accordance with the applicable code. Loads in the tables are predicated on the use of fasteners indicated in the tables, wood with a minimum specific gravity of 0.50 (except redwood) and a lumber moisture content less than 19 percent. Where redwood is referenced, specific gravity is assumed to be 0.37.

7.2 The scope of this evaluation report is limited to use of these connectors with lumber that has not been pressure-treated with chemicals such as those for fire-retardant treatment and preservative treatment.

7.3 Framing members shall be designed in accordance with the requirements referenced in the applicable code.

7.4 Loads for duration of load other than normal shall be adjusted in accordance with the 1991 National Design Specification for Wood Construction up to the "maximum" allowable tabulated load.

7.5 Beams or headers shall have the following minimum widths based on nail sizes attaching the hanger to the beams or headers, except that the GLTV and HGLTV connectors require a minimum 3-inch (76.2 mm) header:

Nail Size	Beam or Header Width	F_{yb}
6d	1.36 inches (34.5 mm)	100,000 psi (690 MPa)
8d	1.57 inches (39.9 mm)	100,000 psi (690 MPa)
10d	1.78 inches (45.2 mm)	90,000 psi (620 MPa)
16d	1.94 inches (49.3 mm)	90,000 psi (620 MPa)

7.6 Plans specifying connectors listed in this report shall be accompanied by calculations demonstrating that the allowable loads noted in this report are not exceeded. These calculations shall be signed and sealed by a registered design professional, as required by the applicable code.

7.7 The connectors shall be manufactured, identified and installed in accordance with this report and the manufacturer's installation instructions.

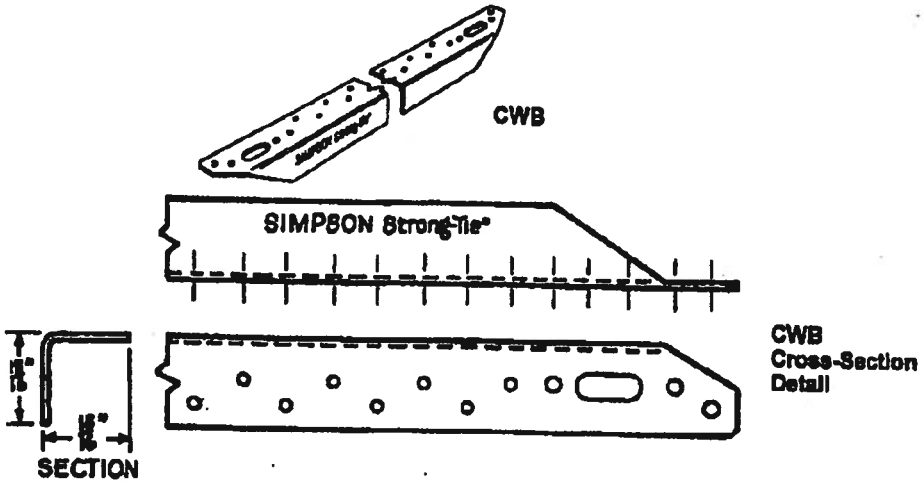
7.8 Connectors have not been evaluated for simultaneous loadings such as wind or seismic plus live and dead loads.

7.9 This report is subject to periodic re-examination. For information on the current status, consult the evaluation report listing or contact one of the participating members of the NES.

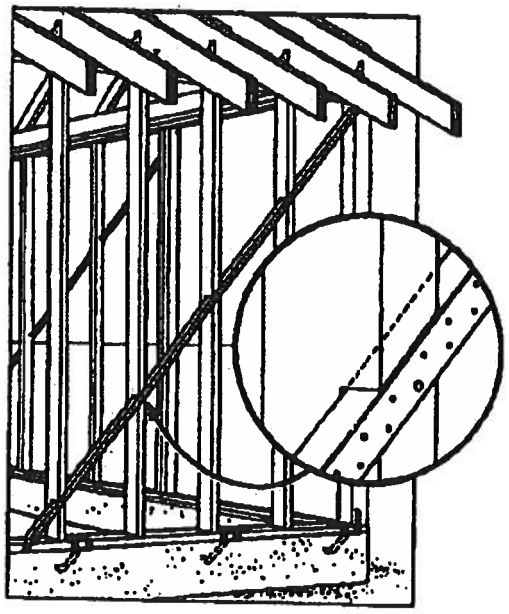
TABLE 1 - CWB[1,2]

MODEL NO.	DIMENSIONS (feet - x 304.8 for mm) (inches - x 25.4 for mm)		ANGLE FOR 8' WALL	FASTENERS[3]	
	LENGTH	SECTION		PLATES	STUDS
CWB106	9' - 5¾"	15/16 x 15/16	60°	2 - 16d	1 - 8d
CWB126	11' - 4⅜"	15/16 x 15/16	45°	2 - 16d	1 - 8d

1. The CWB is designed to provide racking resistance equivalent to a 1x4 let in during construction. It is not designed to replace the shearwall load carrying components.
2. The CWB is limited to a maximum load of 200 pounds (x 4.45 for N).
3. The 16d common nails are 0.162" x 3½" long. The 8d common nails are 0.131" x 2½" long (x 25.4 for mm).



CWB Cross-Section Detail

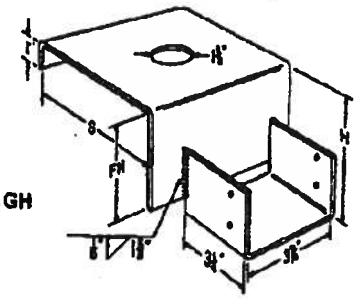


Typical CWB Installation

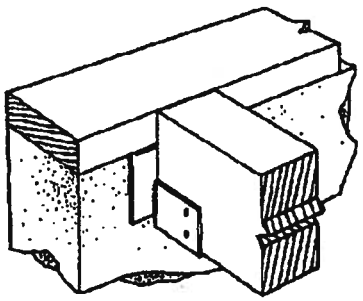
TABLE 2 - GH[1,2]

MODEL NO.	GIRDER	H (INCH)	S (INCH)	ALLOWABLE GRAVITY LOADS (LBS)[3,4] (x 4.45 for N)
		x 25.4 for mm		FLOOR/ROOF
GH46-S	4 x 6	4	6	2000
GH46-S	4 x 8	4	8	2000
GH48-S	4 x 8	6	8	2000
GH48-S	4 x 8	6	8	2000

1. Install 4 - 16d common nails into the girder.
2. Nails are 16d common 0.162" x 3½" long (x 25.4 for mm).
3. Loads are in pounds.
4. Allowable loads in the table are limited by test results. No load duration increase is allowed.



GH

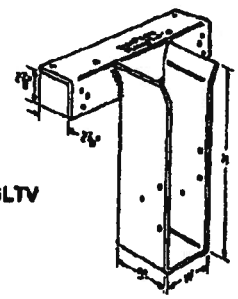


Typical GH Installation

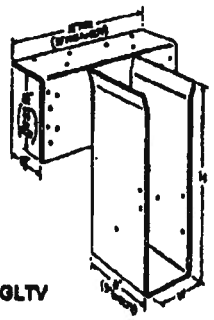
TABLE 3 - GLTV & HGLTV

MODEL NO.	DIMENSIONS (INCH) (x 25.4 for mm)				FASTENERS[1]			ALLOWABLE GRAVITY LOADS[2,3,4]	
	W	MIN H	B	L	CARRING MEMBER TOP	CARRING MEMBER FACE	CARRIED MEMBER	FLOOR (100)	ROOF (125)
GLTV3.5	3 7/16	9¼	5	10	4 - 16d	6 - 16d	6 - 16d	7000	7000
GLTV5.5	5½	9¼	5	10	4 - 16d	6 - 16d	6 - 16d	7000	7000
HGLTV3.5	3 7/16	9¼	8	12	6 - 16d	12 - 16d	6 - 16d	10500	10500
HGLTV5.5	5½	9¼	8	12	6 - 16d	12 - 16d	6 - 16d	10500	10500

1. Nails are 16d common 0.162 x 3½" long (x 25.4 for mm).
2. Loads are in pounds (x 4.45 for N).
3. Allowable loads in the table are limited by test results. No load duration increase is allowed.
4. The connectors provide torsional resistance up to a maximum joist depth of 32 inches (x 25.4 for mm).
5. Wood headers supporting the hangers shall have a minimum width of 3 inches and a minimum allowable compression perpendicular to the grain value of 600 psi (x 6.895 for kPa).



GLTV

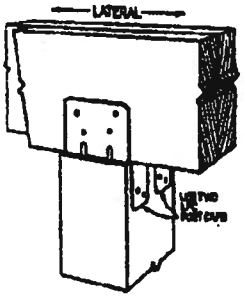


HGLTV

TABLE 4 - LPC4

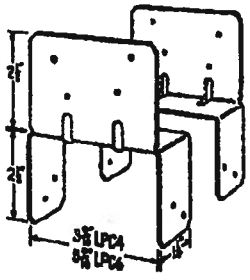
MODEL NO.	FASTENERS[1]		ALLOWABLE LOADS [2,3] (lbs) (x 4.45 for N)	
	BEAM	POST	UPLIFT	LATERAL[4]
LPC4	8 -10d	8 -10d	760	325

1. Nails are 10d common 0.148 x 3" long (x 25.4 for mm).
2. Loads are in pounds (x 4.45 for N).
3. Allowable loads in the table are limited by test results. No load duration increase is allowed.
4. Allowable loads are for hangers used in pairs.



Typical LPC4 Installation

Allows For Beam Widths 2½" to 3½"

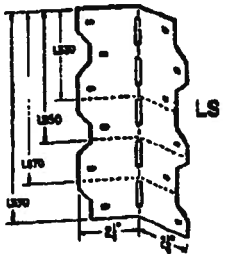


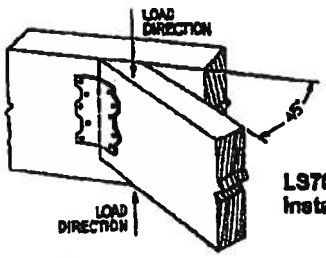
LPC4

TABLE 5 - LS

MODEL NO.	LENGTH	FASTENERS[1]	ALLOWABLE LOADS[2,3] (lbs.) (x 4.45 for N)	
			NORMAL	MAXIMUM
LS30	3⅜	6 - 10d	335	420
LS50	4⅞	8 - 10d	450	560
LS70	6⅜	10 - 10d	560	670
LS90	7⅞	12 - 10d	670	840

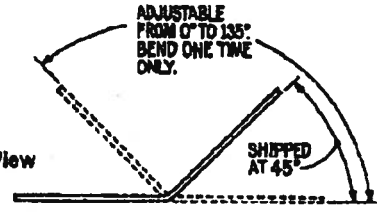
1. Nails are 10d common 0.148 x 3" long (x 25.4 for mm).
2. Loads are in pounds (x 4.45 for N).
3. Allowable loads are for vertical loads only, no lateral loads allowed.





LS70 Installed

LS Top View



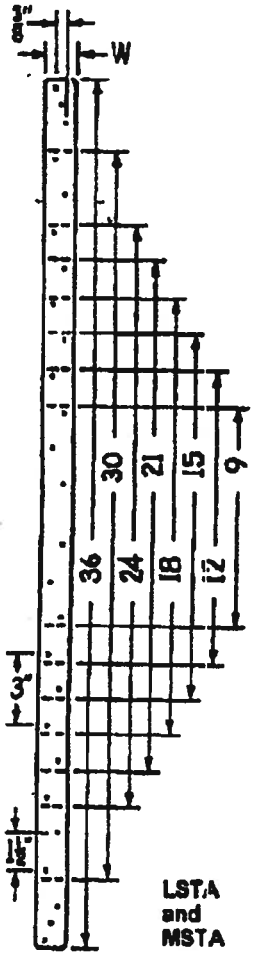


TABLE 6 - LSTA/MSTA

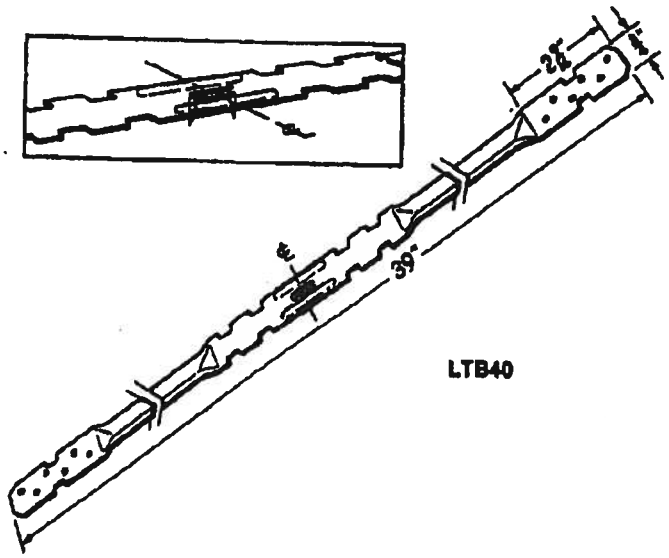
MODEL NO.	MATERIAL	DIMENSIONS(inch) (x 25.4 for mm)		FASTENERS (TOTAL)	MAXIMUM ALLOWABLE LOADS[2,3] (x 4.45 for N)	
		W	L		10d	16d
LSTA9	20 GA	1¼	9	8	605	720
LSTA12	20 GA	1¼	12	10	755	900
LSTA15	20 GA	1¼	15	12	905	1080
LSTA18	20 GA	1¼	18	14	1055	1260
LSTA21	20 GA	1¼	21	16	1205	1295
LSTA24	20 GA	1¼	24	18	1295	1295
LSTA30	18 GA	1¼	30	22	1670	1715
LSTA36	18 GA	1¼	36	26	1715	1715
MSTA9	18 GA	1¼	9	8	610	725
MSTA12	18 GA	1¼	12	10	760	905
MSTA15	18 GA	1¼	15	12	910	1090
MSTA18	18 GA	1¼	18	14	1065	1270
MSTA21	18 GA	1¼	21	16	1215	1450
MSTA24	18 GA	1¼	24	18	1370	1630
MSTA30	18 GA	1¼	30	22	1685	2010
MSTA36	18 GA	1¼	36	26	1995	2135

1. Nails are 16d common 0.162 x 3½" long or 10d common 0.148 x 3" long (x 25.4 for mm).
2. Loads are in pounds.
3. Maximum allowable loads have been increased 33% for wind or earthquake loading, no further increase allowed. Reduce the allowable loads by 33% for normal loading criteria.

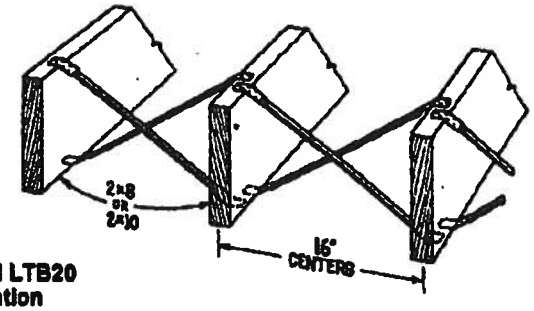
TABLE 7 - LTB

MODEL NO.	JOIST	SPACING(inch) (x 25.4 for mm)	LENGTH(inch) (x 25.4 for mm)
LTB20[1,2]	2x8, 2x10	16"	19.5"
LTB21[1,2]	2x8, 2x10, 2x12	16"	21"
LTB40[2,3]	2x8, 2x10	16"	39"
LTB42[2,3]	2x8, 2x10, 2x12	16"	42"

1. Install 2 - 6d nails each end for the LTB20 and LTB21.
2. Nails are 6d common 0.113 x 2" long (x 25.4 for mm).
3. Install prongs and 2 - 6d nails in the other end for the LTB40 and LTB42.



LTB40



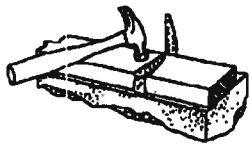
Typical LTB20 Installation (LTB40 Similar)

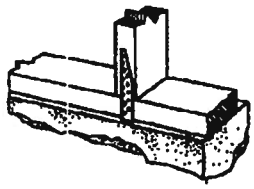
Space Bridging to Avoid Contact Noises

TABLE 8 - MA[1]

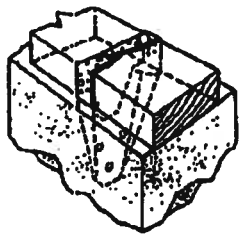
MODEL NO.	SILL SIZE	W	FASTENERS[2,3]		ALLOWABLE LOADS[4,5] (lbs.) (x 4.45 for N)		
			SIDES TOTAL	TOP	UPLIFT	PARALLEL TO SILL PLATE	PERPENDICULAR TO SILL PLATE
MA4	2 x 4	3⅝	2 - 10d x 1½	2 - 10d x 1½	830	480	1180
	3 x 4		4 - 10d x 1½	2 - 10d x 1½	1060	680	1180
MA6	2 x 6	5⅝	2 - 10d x 1½	4 - 10d x 1½	1060	680	1180
	3 x 6		4 - 10d x 1½	4 - 10d x 1½	1290	680	1180

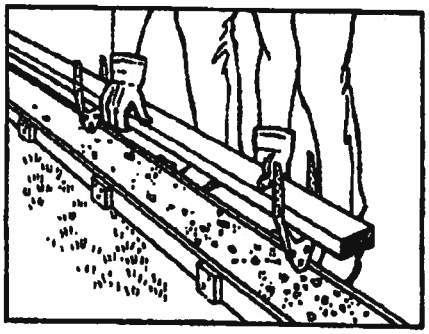
1. Minimum concrete strength shall be 2000 psi(x 6.895 for kPa) at 28 days and shall have a minimum thickness of 6 inches (x 25.4 for mm).
2. Nails are evenly divided between each side at the spacing and edge distance required by code.
3. Nails are 0.148 x 1½" long (x 25.4 for mm).
4. Loads shall not be increased for short-term load duration.
5. Loads are in pounds (x 4.45 for N).



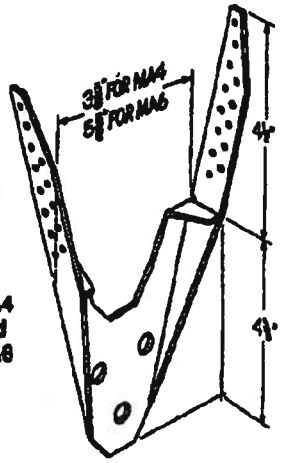


Typical MA4 and MA6 installed





Optional method with mudsill anchors in place for positioning into screeded concrete.

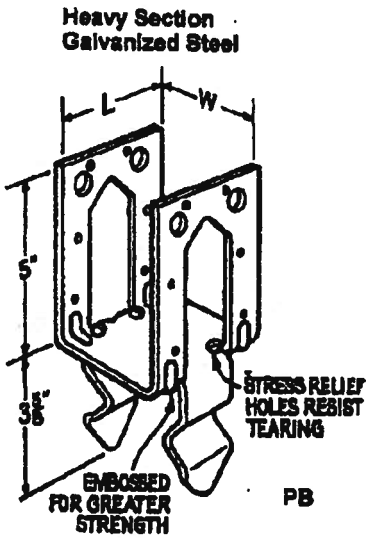


MA4 and MA6

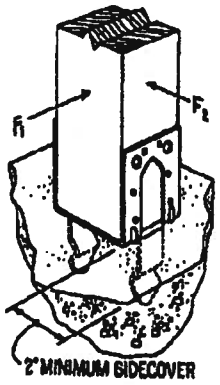
TABLE 9 - PB[1,2]

MODEL NO.	DIMENSIONS(inch) (x 25.4 for mm)		ALLOWABLE LOADS[3,4] (lbs.) (x 4.45 for N)			
	W	L	12 - 16d NAILS[5]			2 - ½ MB[6]
			UPLIFT	F_1	F_2	UPLIFT
PB44	3 9/16	3¼	1365	765	1325	—
PB44R	4	3¼	1365	765	1325	—
PB48	5½	3¼	1365	765	1325	—
PB88	5½	5⅜	1640	765	1325	1640
PB48R	8	3⅜	1365	765	1325	1640
PB66R	8	5⅜	1640	765	1325	1640

1. Minimum concrete strength is 2000 psi (x 6.895 for kPa).
2. Minimum side cover is 2" (x 25.4 for mm).
3. Loads are in pounds (x 4.45 for N).
4. Allowable loads in the table are limited by test results. No load duration increase is allowed.
5. Nails are 16d common 0.162 x 3½" long (x25.4 for mm).
6. The ½ - inch diameter (x 25.4 for mm) machine bolts shall be equal to or better than ASTM A307 quality. The length of the bolt shall be sufficient to allow proper installation of a nut and washer on the threaded end.



PB

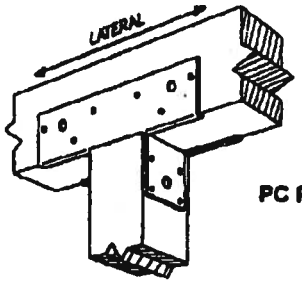


Typical PB Installed

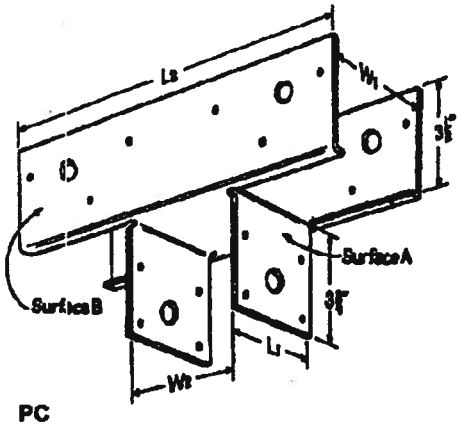
TABLE 10 - PC/EPC

MODEL NO.	POST SIZE	DIMENSIONS (inch) (x 25.4 for mm)					FASTENERS[1]			ALLOWABLE LOADS[2,3] (lbs.) (x 4.45 for N)			
		W1	W2	L1	L2	L3	SURFACE A	SURFACE B	SURFACE C	UPLIFT		LATERAL[4]	
										NORM	MAX	PC[5]	EPC[6]
PC44-16	4 x 4	3 1/16	3 9/16	2 5/8	11	7 3/8	4 - 16d	6 - 16d	4 - 16d	1000	1000	925	1000
PC44	4 x 4	3 1/16	3 9/16	2 5/8	11	7 3/8	4 - 16d	6 - 16d	4 - 16d	1105	1470	925	1070
PC46-16	4 x 6	3 1/16	5 1/2	2 5/8	13	9 1/4	4 - 16d	6 - 16d	4 - 16d	1000	1000	925	1000
PC46	4 x 6	3 1/16	5 1/2	2 5/8	13	9 1/4	4 - 16d	6 - 16d	4 - 16d	1105	1470	925	1070
PC48-16	4 x 8	3 1/16	7 1/2	2 5/8	15	11 1/4	4 - 16d	8 - 16d	6 - 16d	1000	1000	1475	1285
PC48	4 x 8	3 1/16	7 1/2	2 5/8	15	11 1/4	4 - 16d	8 - 16d	6 - 16d	1105	1470	2075	1610
PC64-16	4 x 6	5 1/2	3 9/16	4 9/16	11	7 3/8	4 - 16d	6 - 16d	4 - 16d	1000	1000	925	1000
PC64	4 x 6	5 1/2	3 9/16	4 9/16	11	7 3/8	4 - 16d	6 - 16d	4 - 16d	1105	1470	925	1070
PC66-16	6 x 6	5 1/2	5 1/2	4 9/16	13	9 1/4	4 - 16d	6 - 16d	6 - 16d	1000	1000	925	1285
PC66	6 x 6	5 1/2	5 1/2	4 9/16	13	9 1/4	4 - 16d	6 - 16d	6 - 16d	1105	1470	925	1610
PC68	6 x 8	5 1/2	7 1/2	4 9/16	15	11 1/4	4 - 16d	8 - 16d	6 - 16d	1105	1470	2075	1610
PC84	4 x 8	7 1/2	3 9/16	6 9/16	11	7 3/8	4 - 16d	6 - 16d	6 - 16d	1105	1470	925	1610
PC86	6 x 8	7 1/2	5 1/2	6 9/16	13	9 1/4	4 - 16d	6 - 16d	6 - 16d	1105	1470	925	1610
PC88	8 x 8	7 1/2	7 1/2	6 9/16	15	11 1/4	4 - 16d	8 - 16d	6 - 16d	1105	1470	2075	1610

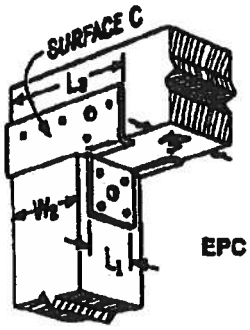
1. Nails are 16d common 0.162 x 3½" long (x 25.4 for mm).
2. Loads are in pounds.
3. Loads shall not be increased for short term load duration.
4. Allowable lateral loads are for loads applied parallel to the beam.
5. PC = post cap.
6. EPC = end post cap.



PC Post Cap



PC

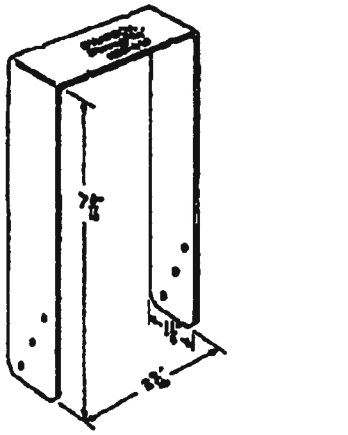


EPC End Post Cap

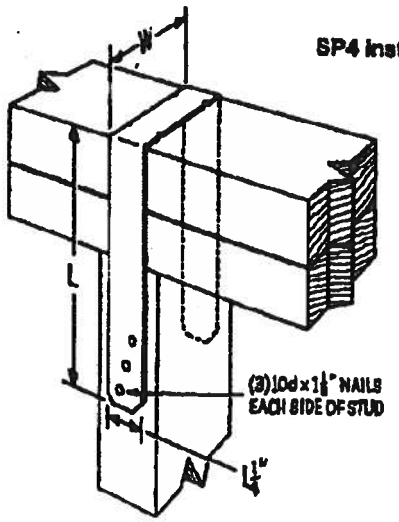
TABLE 11 - SP/SPA

MODEL NO.	MATERIAL	DIMENSIONS (Inch) (x 25.4 for mm)		NUMBER OF FASTENERS[1]	NORMAL ALLOWABLE LOADS[2,3] (lbs.) (x 4.45 for N)	
		W	L		10d x 1½	16d x 2½
SP4/SP4A	20 GA	3 9/16	7 3/16	6	550	600
SP6/SP6A	20 GA	5 9/16	7 3/4	6	550	600
SP8/SP8A	20 GA	7 5/16	8 5/16	6	550	600

1. Nails are 10d common, 0.148 x 1½" long or 16d common 0.162 x 2½" long (x 25.4 for mm).
2. Loads are in pounds.
3. Loads are permitted to be increased for short-term load duration.



SP4
(SP6 & SP7 Similar)

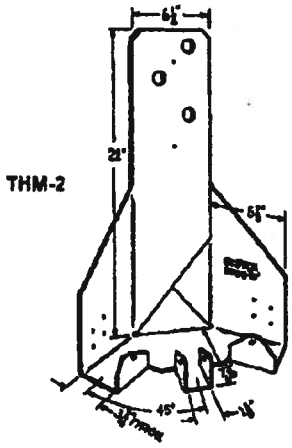


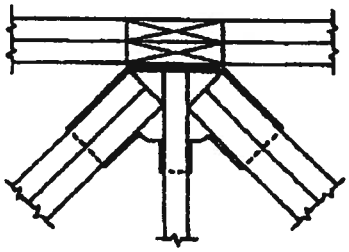
SP4 Installed

TABLE 12 - THM-2

MODEL NO.	FASTENERS[2]				ALLOWABLE LOADS[3,4,6] (x 4.45 for N)								
	CARRYING MEMBER		CARRIED MEMBER		UPLIFT				LENGTH OF BOLT IN WOOD MEMBER(inch) (x 25.4 for mm)	ROOF			
					HIP		JACK			(100)		(125)	
	BOLTS[5]	NAILS	HIP	JACK	NORM	MAX	NORM	MAX		HIP	JACK	HIP	JACK
THM-2	3 - 1	2 - 16d	6 - 10d x 1½	2 - 10d x 1½	490	520	195	260	1½	1105	555	1385	690
									3	2215	1110	2770	1385
									4½	3250	1625	3640	1820
									6	3250	1625	3640	1820

1. Use of the hanger to laterally support trusses where required by design is beyond the scope of this report.
2. Nails are 10d common 0.148 x 1½" long and 16d common 0.162 x 3½" long (x 25.4 for mm).
3. Loads are in pounds.
4. Loads shall not be increased for short-term load duration.
5. The total load shall be evenly distributed about the center line to avoid eccentric loading.
6. The 1-inch diameter machine bolts must be equal to or better than ASTM A 307 quality. Length of the bolt shall be sufficient to allow proper installation of a nut and washer on the threaded end.

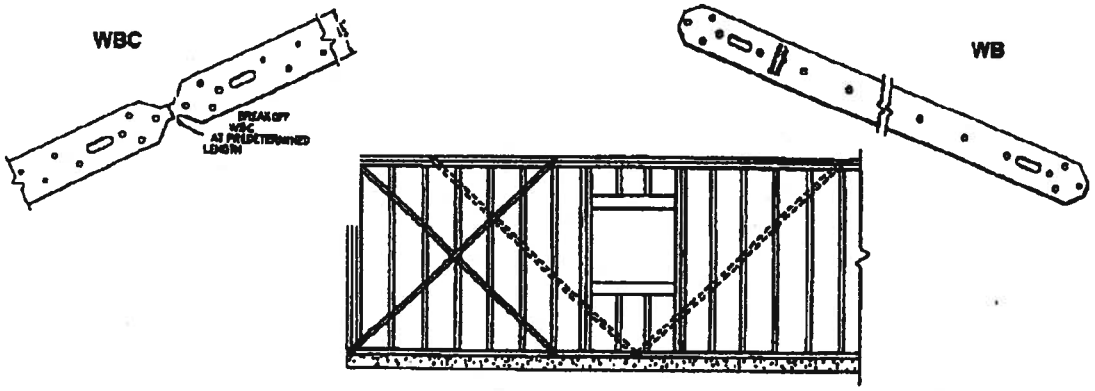




THM-2 Plan View

TABLE 13 - FASTENER SCHEDULE FOR WB AND WBC WALL BRACING

MODEL NO.	DIMENSIONS (feet/inches) (x 304.8 for mm)/(x 25.4 for mm)		ANGLE & WALL SIZE	FASTENERS	
	LENGTH	SECTION		PLATES	STUDS
WB106	9' - 5⅝"	1¼	8' at 60°	2 - 16d	1 - 8d
WB126	11' - 4⅜"	1¼	8' at 45°	2 - 16d	1 - 8d
WB106C	9' - 6"	1¼	8' at 60°	2 - 16d	1 - 8d
WB126C	11' - 4¾"	1¼	8' at 45°	2 - 16d	1 - 8d
WB143C	14' - 3"	1¼	10' at 45°	2 - 16d	1 - 8d

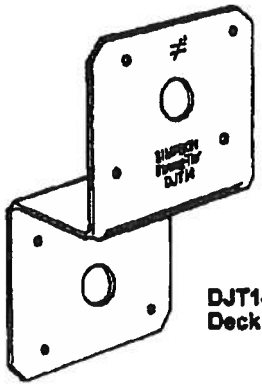


WB or WBC Wall Bracing "X" and "V" Applications

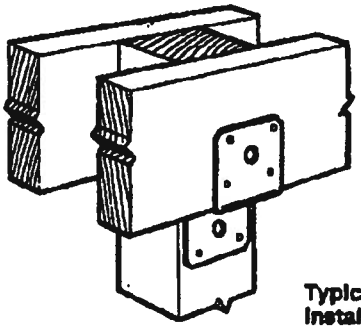
TABLE 14 - FASTENER SCHEDULE FOR THE DJT14

MODEL NO.	FASTENERS		ALLOWABLE GRAVITY LOADS[1,2] (lbs.) (x 4.45 for N)			
			NAILS		BOLTS	
	NAILS	BOLTS[3]	FLOOR (100)	ROOF (125)	FLOOR (100)	ROOF (125)
DJT14	8 - 16d	2 - ⅝ MB	1100	1375	1400	1400

1. Loads and fasteners noted in the table are for one DJT.
2. Roof loads are 125% of floor loads. When floor loads are adjusted for other load duration, in accordance with the applicable code, they shall not exceed those in the roof column.
3. The ⅝ - inch - diameter machine bolts shall be equal to or greater than ASTM A 307 quality, length shall be sufficient to allow proper installation of a nut and washer on the threaded end.



DJT14
Deck Joist Tie



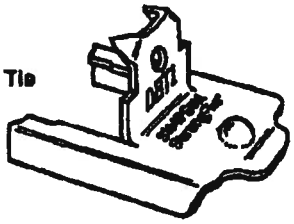
Typical DJT14 Installation

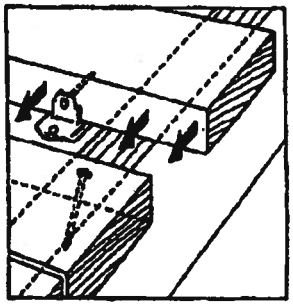
TABLE 15 - FASTENER SCHEDULE FOR THE DBT1

MODEL NO.	FASTENERS[1,2]
	DECK BOARD
DBT1	2 PRONGS & 1 - 10d X 1½"

1. 2-Prongs and 1-10d x 1½ nail are used to attach DBT1 to edge of deck board. Other edge of deck board shall be toe nailed using 1-16d common.
2. Minimum deck board thickness shall be 1¼" (¾).

DBT1
Deck Board Tie



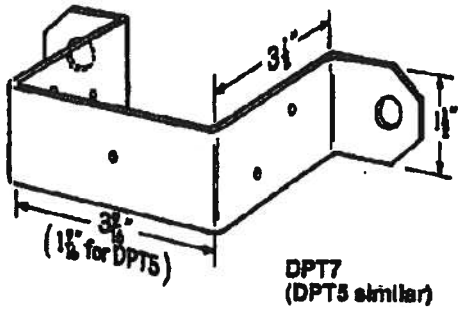


Typical DBT1 Installation.
Connectors slide under anchored deck board.

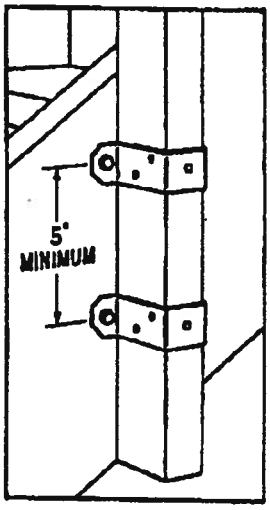
TABLE 16 - FASTENER SCHEDULE FOR THE DPT5, DPT6 AND DPT7[1,2]

MODEL NO.	FASTENERS		MINIMUM POST SPACING[3] (feet/inches) (x 304.8 for mm/ x 25.4 for mm)	
	FASCIA/RIM JOIST	POST	DOUGLAS FIR-LARCH	REDWOOD (Close Grain)
DPT5[4]	2 - ⅜ MB[5]	5 - 10d x 1½	2 - 1	1 - 2
DPT6	8 - 16d	8 - 16d	1 - 7	1 - 3
DPT7[4]	2 - ⅜ MB[5]	5 - 10d	2 - 5	1 - 9

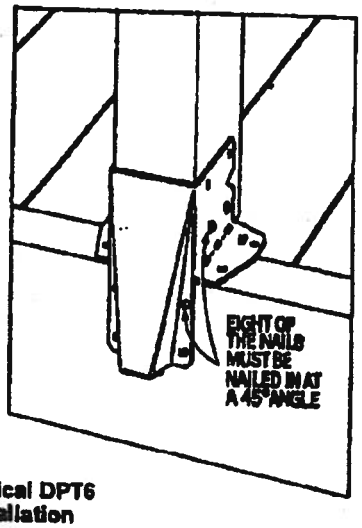
1. Limited to Group R residential dwellings and a maximum rail height of 36 inches.
2. For DPT5 and DPT7, minimum rim joist size is 2x8, nominal and minimum fascia size is 2x10, nominal. For DPT6, minimum rim joist size is 2x8, nominal, and minimum fascia size is 2x8, nominal.
3. Post spacing is based on an assumed handrail loading of 50 plf or a 300 pound point load.
4. Fastener quantities are for a single DPT5 or DPT7. Proper installation is in pairs 5 inches apart, center to center. A standard washer is required with each nut.
5. Bolts shall penetrate through a minimum nominal 2x rim joist (1½-inch minimum)(x 25.4 for mm) and a minimum nominal 2x fascia (1½-inch minimum)(x 25.4 for mm). Machine bolts shall be ASTM A 307 quality or better and have sufficient length to allow proper installation of a nut and washer on the threaded end.



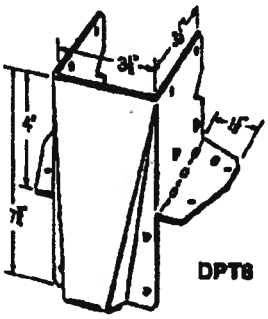
DPT7
(DPT5 similar)



Typical DPT5
Stairway Installation



Typical DPT6
Installation

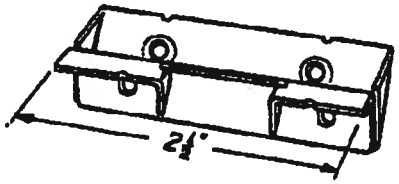


DPT8

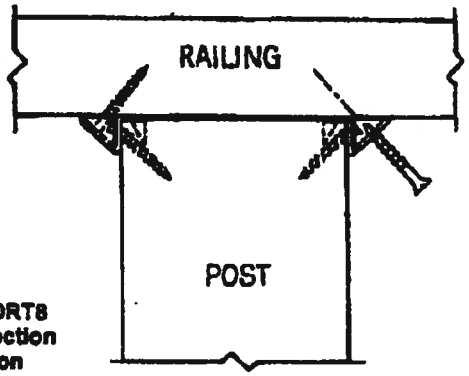
TABLE 17 - FASTENER SCHEDULE FOR THE DRT8

MODEL NO.	FASTENERS[1]		ALLOWABLE HORIZONTAL LOADS (lbs.) (x 4.45 for N)		MAXIMUM SPACING (feet/inches)[4] (x 304.8 for mm/x 25.4 for mm)	
	RAIL[2]	POST[3]	REDWOOD (Close Grain)	DOUG FIR-LARCH	REDWOOD (Close Grain)	DOUG FIR-LARCH
DRT8	2	2	80	113	4' 0"	5' 7"

1. Fastener quantities are for a single DRT8. Installation is in pairs with one DRT8 on either side of the post. Fasteners are No. 8 x 1¼ -inch wood screws.
2. Minimum deck rail thickness is 1½ inches (x 25.4 for mm).
3. Minimum post size is 2 x 4, nominal.
4. Post spacing is based on an assumed handrail loading of 50 plf (x .0146 for N/mm) or a 300 pound (x 4.45 for N) point load.



DRT8



Typical DRT8 Cross Section Installation

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

CONTRACTOR LICENSING SECTION
(305) 375-2527 FAX (305) 375-2558

CONTRACTOR ENFORCEMENT SECTION
(305) 375-2966 FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 372-6339

PRODUCT CONTROL NOTICE OF ACCEPTANCE

Simpson Strong Tie Company, Inc.
4637 Chabot Drive
Pledsanton CA 94588

Your application for Product Approval of:
Wood Connectors
under Chapter 8 of the Code of Miami-Dade County governing the use of Alternate Materials and Types of Construction, and completely described herein, has been recommended for acceptance by the Miami-Dade County Building Code Compliance Office (BCCO) under the conditions specified herein.

This approval shall not be valid after the expiration date stated below. BCCO reserves the right to secure this product or material at anytime from a jobsite or manufacturer's plant for quality control testing.
If this product or material fails to perform in the approved manner, BCCO may revoke, modify, or suspend the use of such product or material immediately. BCCO reserves the right to revoke this approval, if it is determined BCCO that this product or material fails to meet the requirements of the South Florida Building Code.

The expense of such testing will be incurred by the manufacturer.

Acceptance No.: 99-0713.05
Expires: 10/13/2002

Raul Rodriguez
Chief Product Control Division

THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL CONDITIONS

BUILDING CODE & PRODUCT REVIEW COMMITTEE

This application for Product Approval has been reviewed by the BCCO and approved by the Building Code and Product Review Committee to be used in Dade County, Florida under the conditions set forth above.

Francisco J. Quintana, R.A.
Director
Miami-Dade County
Building Code Compliance Office

Approved: 10/14/1999

Simpson Strong-Tie Co., Inc.

ACCEPTANCE NO: 99-0713.05

APPROVED : OCT 13 1999

EXPIRES : OCT 13 2002

NOTICE OF ACCEPTANCE: SPECIFIC CONDITIONS

1. SCOPE

1.1 This approves wood connectors; as described in Section 2 of this Notice of Acceptance, designed to comply with the South Florida Building Code (SFBC), 1994 Edition for Miami-Dade County. For the locations where the actual loads as determined by SFBC Chapter 23, do not exceed the design load indicated in the approved drawings.

2. PRODUCT DESCRIPTION

2.1 The Simpson Strong-Tie Wood Connectors shall be fabricated and used in strict compliance with the following documents: Drawing No. no number, titled "ABA Standoff Post Bases", "ABA Standoff Post Bases", "AC/ACE Post Caps", "BC Post Caps", "L/LS Reinforcing and Skeable Angles", "MTS Twist Straps", & "TA9 Staircase Angles", prepared by Simpson Strong-Tie Co., Inc., dated 03/04/99 sheet 1 through 7 of 7. The drawings shall bear the Miami-Dade County Product Control Approval stamp with the Notice of Acceptance number and approval date by the Miami-Dade Product Control Division. These documents shall hereinafter be referred to as the approved drawings.

3. LIMITATIONS

3.1 Allowable loads are for Douglas Fir-Larch or better with a specific gravity of 0.50 and moisture content of 19% or less.

3.2 Allowable loads are based on testing per ASTM D1761 and calculations per National Design Specifications for Wood Construction 1991 Edition & 1993 Errata.

4. INSTALLATION

4.1 The wood connectors shall be installed in strict compliance with the approved drawings.

5. LABELING

5.1 Each wood connector shall bear a permanent label with the manufacturer's name or logo, city, state and the following statement: "Miami-Dade County Product Control Approved".

6. BUILDING PERMIT

6.1 Application for Building Permit shall be accompanied by copies of the following:

6.1.1 This Notice of Acceptance

6.1.2 Duplicate copies of the approved drawings as identified in Section 2 of this Notice of Acceptance, clearly marked to show the hangers and angles selected for the proposed installation.

6.1.3 Any other document required by the Building Official or the SFBC in order to properly evaluate the installation of these products.

Candido Font, PE, Sr. Product Control Examiner
Product Control Division

Simpson Strong-Tie Co., Inc.

ACCEPTANCE NO.: 99-0713.05

APPROVED :OCT 13 1999

EXPIRES :OCT 13 2002

NOTICE OF ACCEPTANCE STANDARD CONDITIONS

1. Renewal of this Acceptance (approval) shall be considered after a renewal application has been filed and the original submitted documentation, including test supporting data, engineering documents, are no older than eight (8) years.

2. Any and all approved products shall be permanently labeled with the manufacturer's name, city, state, and the following statement: "Miami-Dade County Product Control Approved", or as specifically stated in the specific conditions of this Acceptance.

3. Renewals of Acceptance will not be considered if:
a) There has been a change in the South Florida Building Code affecting the evaluation of this product and the product is not in compliance with the code changes;
b) The product is no longer the same product (identical) as the one originally approved;
c) If the Acceptance holder has not complied with all the requirements of this acceptance, including the correct installation of the product;
d) The engineer who originally prepared, signed and sealed the required documentation initially submitted is no longer practicing the engineering profession.

4. Any revision or change in the materials, use, and/or manufacture of the product or process shall automatically be cause for termination of this Acceptance, unless prior written approval has been requested (through the filing of a revision application with appropriate fee) and granted by this office.

5. Any of the following shall also be grounds for removal of this Acceptance:
a) Unsatisfactory performance of this product or process.
b) Misuse of this Acceptance as an endorsement of any product, for sales, advertising or any other purpose.

6. The Notice of Acceptance number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the Notice of Acceptance is displayed, then it shall be done in its entirety.
7. A copy of this Acceptance as well as approved drawings and other documents, where it applies, shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at all time. The engineer need not reseal the copies.

8. Failure to comply with any section of this Acceptance shall be cause for termination and removal of Acceptance.

9. This Notice of Acceptance consists of pages 1, 2 and this last page 3.

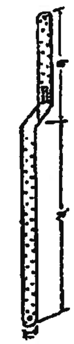
Candido Font, PE, Sr. Product Control Examiner
Product Control Division

END OF THIS ACCEPTANCE

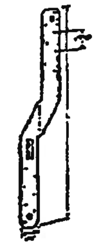
LTS/MTS TWIST STRAPS

Model No.	L	Fasteners[1]		Allowable Loads			
		10d	10dx1½"	Uplift		Parallel to Plate (F_1)	Perpendicular to Plate (F_2)
				10d	10dx1½"		
MTS12	12	14-10d	14-10dx1½"	815		75	125
MTS16	16	14-10d	14-10dx1½"	815		75	125
MTS18	18	14-10d	14-10dx1½"	815		75	125
MTS20	20	14-10d	14-10dx1½"	815		75	125
MTS30	30	14-10d	14-10dx1½"	815		–	–

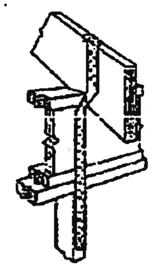
[1]Install half of the fasteners in each end of the strap to achieve full loads. When wrapped over a truss, install one nail on the side of the truss in the hole closet to the middle of the strap, three nails on top of the truss and three nails on the back of the truss. When attached to a double top plate, install one nail in the front of the upper plate closes to the middle of the strap, two nails in the front of the lower plate and three nails in the bottom of the lower plate.



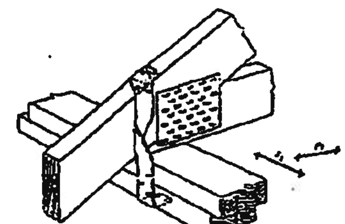
MTS30



MTS12 (Other Sizes Similar)



Typical MTS30 Installation



MTS Installation as a Truss-to-Top Plate Tie

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE Oct. 13, 1999
BY
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 99-0713.05

GENERAL NOTES:

1) Steel shall conform to 16 gauge ASTM A-653 FS with $F_{y,min}$ = 28 ksi and $F_{u,min}$ = 38 ksi and have a minimum galvanized coating of G60.
2) Fasteners diameter is 0.148".
3) Allowable loads have increased 33% for wind loading with no other duration increases allowed.
4) Allowable loads are based on the National Design Specification for Wood Construction 1991 Edition & 1993 Errata, for Douglas Fir-Larch (G=0.50 or better) and tests performed in accordance with ASTM D1761.

FOR OFFICE USE	SIMPSON STRONG-TIE CO.,INC. 4637 Chabot Drive, Suite 200 Pleasanton, CA 94588	
	TITLE MTS Twist Straps	
	Drawing No.	Sheet No. 6/7
	Drawing Date 3-4-99	Revision Date --
	Evon H.C. Ballosh, P.E.	Civil #PE0051752

9/15/99
Evon H.C. Ballosh

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
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CONTRACTOR LICENSING SECTION
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CONTRACTOR ENFORCEMENT DIVISION
(305) 375-2966 FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 372-6339

PRODUCT CONTROL NOTICE OF ACCEPTANCE

Simpson Strong-Tie Company, Inc.
4637 Chabot Drive Suite 200
Pleasanton ,CA 94588

Your application for Notice of Acceptance (NOA) of:
Varous Connectors: H, HH, FC, A
under Chapter 8 of the Code of Miami-Dade County governing the use of Alternate Materials and Types of Construction, and completely described herein, has been recommended for acceptance by the Miami-Dade County Building Code Compliance Office (BCCO) under the conditions specified herein.

This NOA shall not be valid after the expiration date stated below. BCCO reserves the right to secure this product or material at any time from a jobsite or manufacturer's plant for quality control testing. If this product or material fails to perform in the approved manner, BCCO may revoke, modify, or suspend the use of such product or material immediately. BCCO reserves the right to revoke this approval, if it is determined by BCCO that this product or material fails to meet the requirements of the South Florida Building Code.

The expense of such testing will be incurred by the manufacturer.

ACCEPTANCE NO.: 00-0926.01
EXPIRES: 01/11/2004

Raul Rodriguez
Chief Product Control Division

THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL CONDITIONS
BUILDING CODE & PRODUCT REVIEW COMMITTEE

This application for Product Approval has been reviewed by the BCCO and approved by the Building Code and Product Review Committee to be used in Miami-Dade County, Florida under the conditions set forth above.

Francisco J. Quintana, R.A.
Director
Miami-Dade County
Building Code Compliance Office

APPROVED: 01/11/2001

Simpson Strong-Tie Co., Inc.

ACCEPTANCE NO: 00-0926.01

APPROVED: JAN 1 1 2001

EXPIRES: JAN 1 1 2004

NOTICE OF ACCEPTANCE: SPECIFIC CONDITIONS

1. **SCOPE**

1.1 This renews the Notice of Acceptance No. 97-0107.05, which was issued on 08/14/97. It approves wood connectors; as described in Section 2 of this Notice of Acceptance, designed to comply with the South Florida Building Code (SFBC), 1994 Edition for Miami-Dade County. For the locations where the actual loads as determined by SFBC Chapter 23, do not exceed the design load indicated in the approved drawings.

2. **PRODUCT DESCRIPTION**

2.1 The **Simpson Strong-Tie Wood Connectors** shall be fabricated and used in strict compliance with the following documents: Drawing with No. SSTMD-001 and sheets 1 through 6 of 6, titled "Hurricane Ties, HH Header Hangers, FC Framing Clips, A34 Framing Anchors, A35 Framing Anchors and A35F Framing Anchors", prepared by Simpson Stong-Tie Co., Inc., dated 09/14/00 with no revisions. The drawings shall bear the Miami-Dade County Product Control Approval stamp with the Notice of Acceptance number and approval date by the Miami-Dade Product Control Division. These documents shall hereinafter be referred to as the approved drawings.

3. **LIMITATIONS**

3.1 Allowable loads are for Douglas Fir-Larch or better with a specific gravity of 0.50 and moisture content of 19% or less.

3.2 Allowable loads are based on testing per ASTM D1761 and calculations per National Design Specifications for Wood Construction 1991 Edition & 1993 Errata.

4. **INSTALLATION**

4.1 The wood connectors shall be installed in strict compliance with the approved drawings.

5. **LABELING**

5.1 Each wood connector shall bear a permanent label with the manufacturer's name or logo, city, state and the following statement: "Miami-Dade County Product Control Approved".

6. **BUILDING PERMIT**

6.1 Application for Building Permit shall be accompanied by copies of the following:

6.1.1 This Notice of Acceptance

6.1.2 Duplicate copies of the approved drawings as identified in Section 2 of this Notice of Acceptance, clearly marked to show the hangers and angles selected for the proposed installation.

6.1.3 Any other document required by the Building Official or the SFBC in order to properly evaluate the installation of these products.

Candido Font, PE, Sr. Product Control Examiner
Product Control Division

Simpson Strong-Tie Co., Inc.

ACCEPTANCE NO.: 00-0926.01

APPROVED: JAN 1 1 2001

EXPIRES: JAN 1 1 2004

NOTICE OF ACCEPTANCE STANDARD CONDITIONS

1. Renewal of this Acceptance (approval) shall be considered after a renewal application has been filed and the original submitted documentation, including test supporting data, engineering documents, are no older than eight (8) years.

2. Any and all approved products shall be permanently labeled with the manufacturer's name, city, state, and the following statement: "Miami-Dade County Product Control Approved", or as specifically stated in the specific conditions of this Acceptance.

3. Renewals of Acceptance will not be considered if:
a) There has been a change in the South Florida Building Code affecting the evaluation of this product and the product is not in compliance with the code changes;
b) The product is no longer the same product (identical) as the one originally approved;
c) If the Acceptance holder has not complied with all the requirements of this acceptance, including the correct installation of the product;
d) The engineer who originally prepared, signed and sealed the required documentation initially submitted is no longer practicing the engineering profession.

4. Any revision or change in the materials, use, and/or manufacture of the product or process shall automatically be cause for termination of this Acceptance, unless prior written approval has been requested (through the filing of a revision application with appropriate fee) and granted by this office.

5. Any of the following shall also be grounds for removal of this Acceptance:
a) Unsatisfactory performance of this product or process.
b) Misuse of this Acceptance as an endorsement of any product, for sales, advertising or any other purpose.

6. The Notice of Acceptance number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the Notice of Acceptance is displayed, then it shall be done in its entirety.
7. A copy of this Acceptance as well as approved drawings and other documents, where it applies, shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at all time. The engineer need not reseal the copies.

8. Failure to comply with any section of this Acceptance shall be cause for termination and removal of Acceptance.

9. This Notice of Acceptance consists of pages 1, 2 and this last page 3.

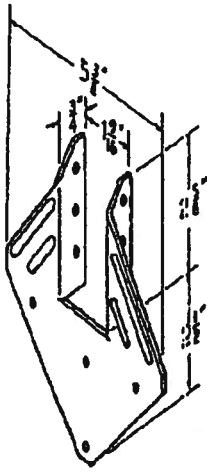
Candido Font, PE, Sr. Product Control Examiner
Product Control Division

END OF THIS ACCEPTANCE

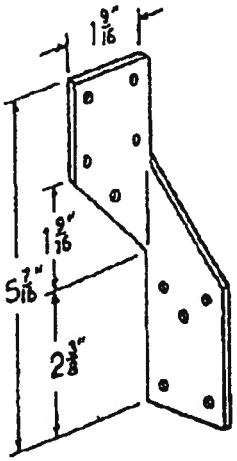
HURRICANE TIES

Model No.	GA.	Fasteners		Allowable Loads		
		Rafter	Plate or Stud	Uplift	F1	F2
H1	18	6 - 8d x 1½	4 - 8d	490	445	165
H2.5	18	5 - 8d x 1½	5 - 8d	410	150	150
H3	18	4 - 8d x 1½	4 - 8d	435	110	125
H4	20	4 - 8d x 1½	4 - 8d	—	150	150
H5	18	4 - 8d x 1½	4 - 8d	—	115	180

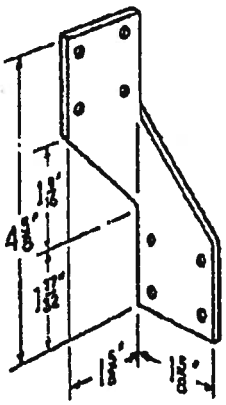
1. In order to comply with the South Florida Building Code Section 2908.05, two (2) H2, H2.5, or H3 connectors shall be installed for a minimum uplift of 700#.
2. Allowable loads are for one tie only.
3. A minimum rafter thickness of 2 ½" must be used when framing anchors are installed on each side of the rafter and on the same side of the plate.



H1



H2.5



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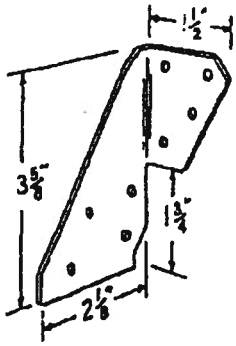
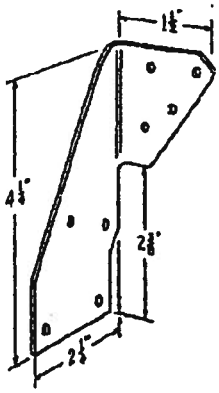


H4

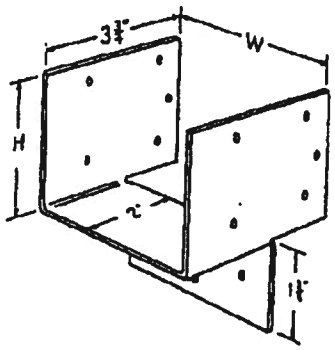


H5

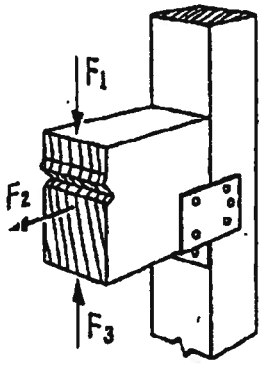
HEADER HANGERS

MODEL NO.	GA	Dimensions		Fasteners 16d		Allowable Loads		
		W	H	Stud	Header	F1	F2	F3
HH4	16	3½	2 - 13/16	8	4	1195	530	530
HH6	16	5½	5 - ⅛	12	4 or 6	1595	800	800

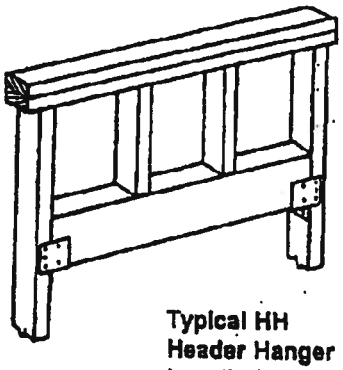
1. The above allowable loads require a minimum 2½" lumber thickness.



HH4



HH Load Directions



Typical HH Header Hanger Installation

GENERAL NOTES

1) Steel for the HH4 and HH6 shall conform to ASTM A-653 FS with $F_{y,min}$ = 28 ksi and $F_{u,min}$ = 38 ksi and have a minimum galvanized coating of G90.

2) Fasteners are common wire nails unless otherwise noted.

3) Allowable uplift loads have been increased 33% for wind loading with no other duration increases allowed.

4) Allowable down loads have not been increased by any duration factor.

5) Allowable loads are based on the National Design Specification for Wood Construction 1991 Edition & 1993 Errata, for Douglas Fir-Larch (G=.50 or better) and tests performed in accordance with ASTM D1761.

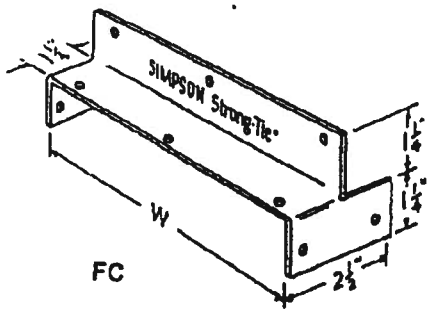
APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE January 11, 2001
BY
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0926.01

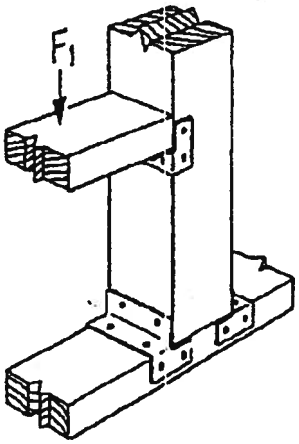
ICE USE	SIMPSON STRONG-TIE CO., INC. 4637 Chabot Drive, Suite 200 Pleasanton, CA 94588	12/18/00 Evon M.C. Ballash
	TITLE: HH HEADER HANGERS	
	Drawing No: SSTMD-001 Sheet No: 2/6	
	Drawing Date: 9-14-00 Revision Date: — —	
	Evon M.C. Ballash, P.E. Civil #PE0051762	

FC FRAMING CLIPS

MODEL NO.	GA	W	FASTENERS		ALLOWABLE LOAD
		In	Post	Header	
FC4	16	3⅜	6 - 16d	2 - 16d	800
FC6	16	5½	7 - 16d	3 - 16d	920

1. The above allowable loads require a minimum 2½ " lumber thickness.
2. Loads may not be increased for short-term loading.





Typical FC Load Direction

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE January 11, 2001
BY
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0926.01

GENERAL NOTES

1) Steel for the FC4 and FC6 shall conform to ASTM A-653 FS with $F_{y,min}$ = 28 ksi and $F_{u,min}$ = 38 ksi and have a minimum galvanized coating of G90.

2) Fasteners are common wire nails unless otherwise noted.

3) Allowable uplift loads have been increased 33% for wind loading with no other duration increases allowed.

4) Allowable down loads have not been increased by any duration factor.

5) Allowable loads are based on the National Design Specification for Wood Construction 1991 Edition &1993 Errata, for Douglas Fir-Larch (G=.50 or better) and tests performed in accordance with ASTM D1761.

FFICE USE

SIMPSON STRONG-TIE CO., INC.
4637 Chabot Drive, Suite 200
Pleasanton, CA 94588

TITLE: FC FRAMING CLIPS

Drawing No.: 9-14-00 | Sheet No.: 3/6

Drawing Date: SSTMD-001 | Revision Date: — —

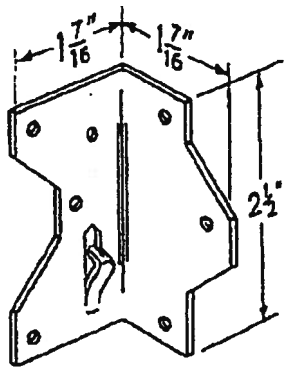
Evon M C Ballesh P E Civil #PE0051762

12/18/00
Evon M C Ballesh

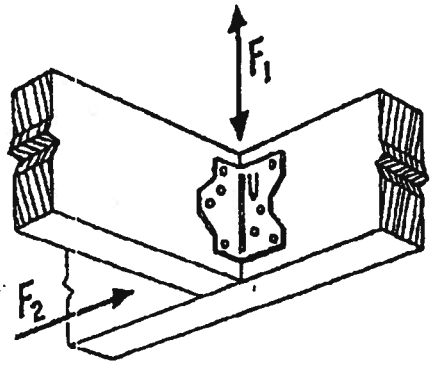
FRAMING ANCHORS

Model No.	GA	Load Direction	Fasteners 8d x 1½		Allowable Load
			Header	Joist	
A34	18	F1	4	4	345
A34	18	F2	4	4	280

1. Allowable loads are for one anchor. When anchors are installed on each side of the joist, the minimum joist thickness is 3".



A34



A34

GENERAL NOTES

1) Steel for the A34, A35and A35F shall conform to ASTM A-653 FS with $F_{y,min}$ = 33 ksi and $F_{u,min}$ = 45 ksi and have a minimum galvanized coating of G90.

2) Fasteners are common wire nails unless otherwise noted.

3) Allowable uplift loads have been increased 33% for wind loading with no other duration increases allowed.

4) Allowable down loads have not been increased by any duration factor.

5) Allowable loads are based on the National Design Specification for Wood Construction 1991 Edition &1993 Errata, for Douglas Fir-Larch (G=.50 or better) and tests performed in accordance with ASTM D1761.

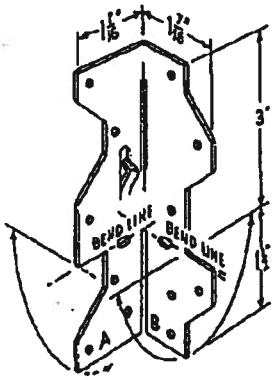
APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE January 11, 2001
BY
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0926.01

FICE USE	SIMPSON STRONG-TIE CO., INC. 4637 Chabot Drive, Suite 200 Pleasanton, CA 94588	
	TITLE: A34 FRAMING ANCHORS	
	Drawing No.: SSTMD-001	Sheet No.: 4/6
	Drawing Date: 9-14-00	Revision Date: --
	Evon M.C. Ballash, P.E. Civil #PE0051762	

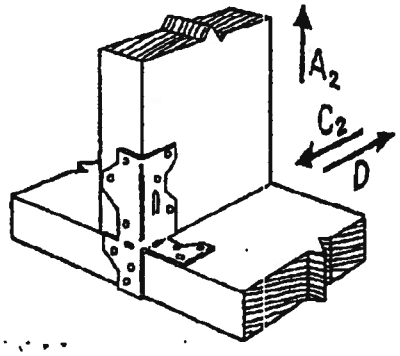
12/19/00
Evon C Ballash

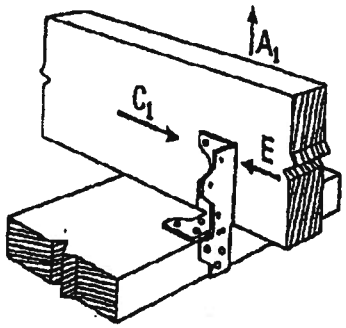
FRAMING ANCHORS

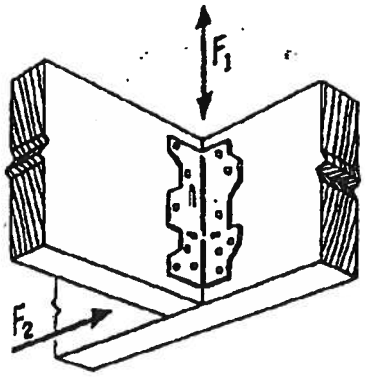
Model No.	GA	Load Direction	Fasteners 8d x 1½		Allowable Load
			Header	Joist	
A35	18	A1	6	3	260
A35	18	E	6	3	260
A35	18	C1	6	3	170
A35	18	A2	6	6	260
A35	18	C2	6	6	260
A35	18	D	6	6	150
A35	18	F1	6	6	450
A35	18	F2	6	6	430



A35







GENERAL NOTES

1) Steel for the A34, A35and A35F shall conform to ASTM A-653 FS with $F_{y,min}$ = 33 ksi and $F_{u,min}$ = 45 ksi and have a minimum galvanized coating of G90.

2) Fasteners are common wire nails unless otherwise noted.

3) Allowable uplift loads have been increased 33% for wind loading with no other duration increases allowed.

4) Allowable down loads have not been increased by any duration factor.

5) Allowable loads are based on National Design Specification for Wood Construction 1991 Edition &1993 Errata, for Douglas Fir-Larch (G=.50 or better) and tests performed in accordance with ASTM D1761.

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE January 11, 2001
BY
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0926.01

CE USE

SIMPSON STRONG-TIE CO., INC.
4637 Chabot Drive, Suite 200
Pleasanton, CA 94588

TITLE: A35 FRAMING ANCHORS

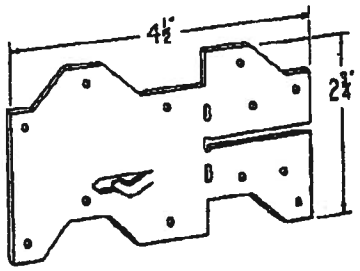
Drawing No.: SSTMD-001	Sheet No.: 5/6
Drawing Date: 9-14-00	Revision Date: — —

Evon M.C. Ballash, P.E. Civil #PE0051762

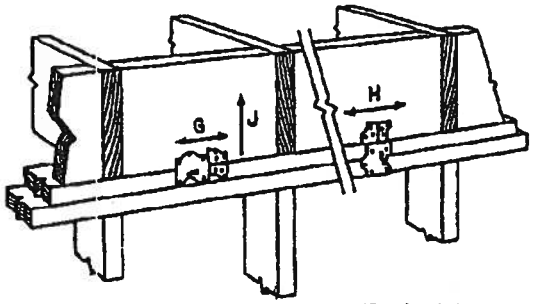
12/18/00
Evon M C Ballash

FRAMING ANCHORS

Model No.	GA	Load Direction	Fasteners 8 x 1½		Allowable Load
			Header	Joist	
A35F	18	G	6	6	500
A35F	18	J	6	6	200
A35F	18	H	6	6	440



A35F



Typical A35F Installation

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE January 11, 2001
BY
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0926.01

GENERAL NOTES

1) Steel for the A34, A35and A35F shall conform to ASTM A-653 FS with $F_{y,min}$ = 33 ksi and $F_{u,min}$ = 45 ksi and have a minimum galvanized coating of G90.

2) Fasteners are common wire nails unless otherwise noted.

3) Allowable uplift loads have been increased 33% for wind loading with no other duration increases allowed.

4) Allowable down loads have not been increased by any duration factor.

5) Allowable loads are based on the National Design Specification for Wood Construction 1991 Edition & 1993 Errata, for Douglas Fir-Larch (G=.50 or better) and tests performed in accordance with ASTM D1761.

OFFICE USE

SIMPSON STRONG-TIE CO., INC.
4637 Chabot Drive, Suite 200
Pleasanton, CA 94588

TITLE: A35F FRAMING ANCHORS	
Drawing No.: SSTMD-001	Sheet No.: 6/6
Drawing Date: 9-14-00	Revision Date: -
Evon M.C. Ballash, P.E.	Civil #PE0051762

12/6/00
Evon M C Ballash

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901
FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2902
FAX (305) 372-6339

Jeff Dunagan
Simpson Strong-Tie Company, Inc.
4120 Dublin Boulevard Suite 400
Dublin, CA 94568

NOTICE OF PROPOSED ACTION

To: ***Members of the Board of Rules and Appeals*** and
Simpson Strong-Tie Company, Inc., Applicant

In accordance with Dade County Administrative Order 10-3, which governs the product review process, the Product Control Division of the Office of Code Compliance, intends to issue a Product Control Notice of Acceptance to **Simpson Strong-Tie Company, Inc.** for **Wood Connectors**, ***No.*** **02-0402.01**, to allow its use in Dade County and its municipalities.

To: ***Members of the Board of Rules and Appeals*** ;

The documentation being provided to you represents the recommendation of the Product Control Division of the Office of Code Compliance in regards to the submittal of **Simpson Strong-Tie Company, Inc.** for **Wood Connectors**, ***No. 02-0402.01***. Under the provisions of Dade County Administrative Order 10-3, which governs the product review process. You must review this documentation. If within 20 days from the date of mailing, we do not receive any written objection stating the reason(s) for your disapproval, this product will be automatically approved.

To: **Simpson Strong-Tie Company, Inc., Applicant**

The Product Control Division of the Office of Code Compliance, in accordance with Dade County Administrative Order 10-3, which governs the product review process, has issued this notice of proposed action and intends to issue a Product Control Notice of Acceptance for your **Wood Connectors**, ***No. 02-0402.01***, to be used in Dade County and its municipalities, unless a member of the Board of Rules and Appeals or yourself has any objections. Should you not be in accord with this notice of proposed action and wish to appeal our recommendation, you must make a written request, stating the reasons for your objection(s), to our office within 20 days of the date of mailing. Upon receipt of your written request a hearing date will be set so that you can present your objection(s) to the Board of Rules and Appeals.

Sincerely,

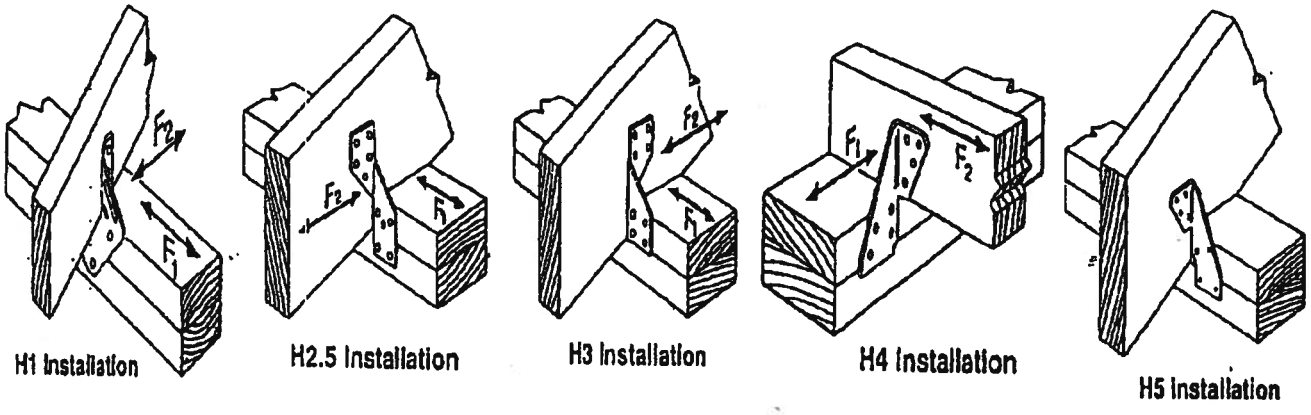
Raul Rodriguez
Chief Product Control Division

Francisco J. Quintana, R.A.
Director

DATE OF MAILING: 04/12/2002

Mailed by: P. Rojade

\\s0450001\pc2000\templates\notice proposed action.dot



GENERAL NOTES

1) Steel for the H1, H2.5, H3, and H5 shall conform to ASTM A-653 FS with $F_{y,min}$ = 28 ksi and $F_{u,min}$ = 38 ksi and have a minimum galvanized coating of G90. Steel for the H4 shall conform to ASTM A-653 SS GR 33 with Fy,min = 33 ksi and Fu,min = 45 ksi and have a minimum galvanized coating of G90.

2) Fasteners are common wire nails unless otherwise noted.

3) Allowable uplift loads have been increased 33% for wind loading with no other duration increases allowed.

4) Allowable loads are based on the National Design Specification for Wood Construction 1991 Edition &1993 Errata, for Douglas Fir-Larch (G=.50 or better) and tests performed in accordance with ASTM D1761.

APPROVED AS CONFORMING WITH THE SOUTH FLORIDA BUILDING CODE
DATE January 11, 2001
BY
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0926.01

FICE USE

SIMPSON STRONG-TIE CO., INC.
4637 Chabot Drive, Suite 200
Pleasanton, CA 94588

TITLE: HURRICANE TIES

Drawing No.: SSTMD-001 | Sheet No.: 1/6

Drawing Date: 9-14-00 | Revision Date: - -

Evon M.C. Ballash, P.E. Civil #PE0051762

12/18/2
Evon M. C Ballash

MIAMI-DADE

BUILDING CODE COMPLIANCE OFFICE (BCCO)
PRODUCT CONTROL DIVISION

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

Simpson Strong-Tie Co., Inc.
4120 Dublin Blvd., Suite 400
Dublin, CA 94568

SCOPE:
This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by Miami-Dade County Product Control Division and accepted by the Board of Rules and Appeals (BORA) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Division (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BORA reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Division that this product or material fails to meet the requirements of the applicable building code.

This product is approved as described herein, and has been designed to comply with the High Velocity Hurricane Zone of the Florida Building Code.

DESCRIPTION: Wood Connectors

APPROVAL DOCUMENT: Drawing No. no number, titled "A Angles", "CS Coiled Strap", LPC Post Caps", "LTHJ Truss Hip/Jack Hangers", & "SP Stud Plate Ties", sheets 1 through 5, prepared, signed and sealed by Evon Ballash, P.E., dated 7/9/99, bearing the Miami-Dade County Product Control Renewal stamp with the Notice of Acceptance number and expiration date by the Miami-Dade County Product Control Division.

MISSILE IMPACT RATING: None

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

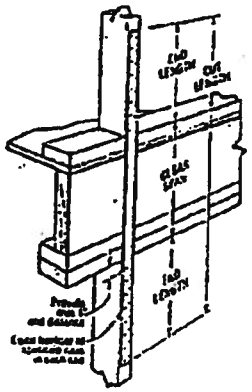
ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

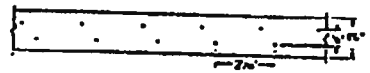
INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.
This NOA renews NOA # 99-0623.04 and, consists of this page 1 as well as approval document mentioned above.
The submitted documentation was reviewed by **Raul Rodriguez.**

CS Coiled Strap

Model No.	Total Length	End Length	Cut Length	Fasteners (Total) 1,	Allowable Loads
CS16S	100'	12"	clear span + 24"	22-6d	925
CS16S,	200'	10"	clear span + 20"	18-10d	
CS18-R	25'	10"	clear span + 20"	18-10d	

1) Minimum Steel Penetration shall be 1 3/8" for 8d & 1 7/8" for 10d.





CS Hole Pattern

APPROVED AS COMPLYING WITH THE
SOUTH FLORIDA BUILDING CODE
DATE ... 1999
BY ...
PRODUCT CONTROL DIVISION
REGIONAL CODE CLEARANCE OFFICE
ACCEPTANCE NO. 99-0423.04

GENERAL NOTES:

1) Steel shall conform to 18 gauge ASTM A-653 FS with F_{yield} = 33 ksi and F_{max} = 45 ksi and have a minimum galvanized coating of G60
2) Fasteners are common wire nails unless noted.
3) Allowable uplift loads have been increased 33% for wind loading with no other duration increases allowed.
4) Allowable loads are based on the National Design Specification for Wood Construction 1991 Edition & 1993 Errata, for Douglas Fir-Larch (G=0.50 or better) and tests performed in accordance with ASTM D1761.

FOR OFFICE USE

SIMPSON STRONG-TIE CO., INC.
4637 Chabot Drive, Suite 200
Pleasanton, CA 94588

Title: CS Coiled Strap

Drawing No. | Sheet No. 2/5

Drawing Date 2-2-98 | Revision Date --

Evon M.C. Ballesh, P.E. Civil #PE0051762

EMB
Evon M C Ballesh

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

CONTRACTOR LICENSING SECTION
(305) 375-2527 FAX (305) 375-2558

CONTRACTOR ENFORCEMENT DIVISION
(305) 375-2966 FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 372-6339

PRODUCT CONTROL NOTICE OF ACCEPTANCE

Simpson Strong-Tie Company, Inc.
4637 Chabot Drive Suite 200
Pleasanton ,CA 94588

Your application for Notice of Acceptance (NOA) of:
Wood Connectors
under Chapter 8 of the Code of Miami-Dade County governing the use of Alternate Materials and Types of Construction, and completely described herein, has been recommended for acceptance by the Miami-Dade County Building Code Compliance Office (BCCO) under the conditions specified herein.

This NOA shall not be valid after the expiration date stated below. BCCO reserves the right to secure this product or material at any time from a jobsite or manufacturer's plant for quality control testing. If this product or material fails to perform in the approved manner, BCCO may revoke, modify, or suspend the use of such product or material immediately. BCCO reserves the right to revoke this approval, if it is determined by BCCO that this product or material fails to meet the requirements of the South Florida Building Code.

The expense of such testing will be incurred by the manufacturer.

ACCEPTANCE NO.: 00-0512.11
EXPIRES: 05/10/2004

Raul Rodriguez
Chief Product Control Division

THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL CONDITIONS
BUILDING CODE & PRODUCT REVIEW COMMITTEE

This application for Product Approval has been reviewed by the BCCO and approved by the Building Code and Product Review Committee to be used in Miami-Dade County, Florida under the conditions set forth above.

Francisco J. Quintana, R.A.
Director
Miami-Dade County
Building Code Compliance Office

APPROVED: 05/10/2001

Simpson Strong-Tie Co., Inc.

ACCEPTANCE NO: 00-0512.11

APPROVED: MAY 10 2001

EXPIRES: MAY 10 2004

NOTICE OF ACCEPTANCE: SPECIFIC CONDITIONS

1. **SCOPE**

1.1 This approves wood connectors; as described in Section 2 of this Notice of Acceptance, designed to comply with the South Florida Building Code (SFBC), 1994 Edition for Miami-Dade County. For the locations where the actual loads as determined by SFBC Chapter 23; do not exceed the design load indicated in the approved drawings.

2. **PRODUCT DESCRIPTION**

2.1 The **Simpson Strong-Tie Wood Connectors** shall be fabricated and used in strict compliance with the following documents: Drawing with No. S-2068 and sheets 1 through 5 of 5, titled "Post Bases, Seismic & Hurricane Ties and Embedded Truss Anchors", prepared by Simpson Stong-Tie Co., Inc., dated 10/23/00 with latest revision on 03/08/01. The drawings shall bear the Miami-Dade County Product Control Approval stamp with the Notice of Acceptance number and approval date by the Miami-Dade Product Control Division. These documents shall hereinafter be referred to as the approved drawings.

3. **LIMITATIONS**

3.1 Allowable loads are for Douglas Fir-Larch or better with a specific gravity of 0.50 and moisture content of 19% or less.

3.2 Allowable loads are based on testing per ASTM D1761 and calculations per National Design Specifications for Wood Construction 1991 Edition & 1993 Errata.

4. **INSTALLATION**

4.1 The wood connectors shall be installed in strict compliance with the approved drawings.

5. **LABELING**

5.1 Each wood connector shall bear a permanent label with the manufacturer's name or logo, city, state and the following statement: "Miami-Dade County Product Control Approved".

6. **BUILDING PERMIT**

6.1 Application for Building Permit shall be accompanied by copies of the following:

6.1.1 This Notice of Acceptance

6.1.2 Duplicate copies of the approved drawings as identified in Section 2 of this Notice of Acceptance, clearly marked to show the hangers and angles selected for the proposed installation.

6.1.3 Any other document required by the Building Official or the SFBC in order to properly evaluate the installation of these products.

Candido Font, PE, Sr. Product Control Examiner
Product Control Division

Simpson Strong-Tie Co., Inc.

ACCEPTANCE NO.: 00-0512.11

APPROVED: MAY 10 2001

EXPIRES: MAY 10 2004

NOTICE OF ACCEPTANCE STANDARD CONDITIONS

1. Renewal of this Acceptance (approval) shall be considered after a renewal application has been filed and the original submitted documentation, including test supporting data, engineering documents, are no older than eight (8) years.

2. Any and all approved products shall be permanently labeled with the manufacturer's name, city, state, and the following statement: "Miami-Dade County Product Control Approved", or as specifically stated in the specific conditions of this Acceptance.

3. Renewals of Acceptance will not be considered if:
a) There has been a change in the South Florida Building Code affecting the evaluation of this product and the product is not in compliance with the code changes;
b) The product is no longer the same product (identical) as the one originally approved;
c) If the Acceptance holder has not complied with all the requirements of this acceptance, including the correct installation of the product;
d) The engineer who originally prepared, signed and sealed the required documentation initially submitted is no longer practicing the engineering profession;

4. Any revision or change in the materials, use, and/or manufacture of the product or process shall automatically be cause for termination of this Acceptance, unless prior written approval has been requested (through the filing of a revision application with appropriate fee) and granted by this office.
5. Any of the following shall also be grounds for removal of this Acceptance:
a) Unsatisfactory performance of this product or process.
b) Misuse of this Acceptance as an endorsement of any product, for sales, advertising or any other purpose.

6. The Notice of Acceptance number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the Notice of Acceptance is displayed, then it shall be done in its entirety.

7. A copy of this Acceptance as well as approved drawings and other documents, where it applies, shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at all time. The engineer need not reseal the copies.

8. Failure to comply with any section of this Acceptance shall be cause for termination and removal of Acceptance.

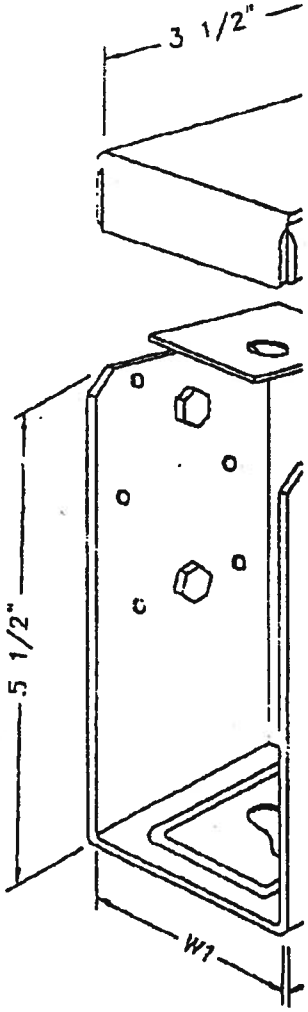
9. This Notice of Acceptance consists of pages 1, 2 and this last page 3.

Candido Font, PE, Sr. Product Control Examiner
Product Control Division

END OF THIS ACCEPTANCE

SIMPSON STRONG-TIE CO., INC.

ABU & EPB ELEVATED POST BASES, H10 SEISMIC & HURRICANE TIE HETA/HETAL EMBEDDED TRUSS ANCHORS & PB44 POST BASE



ABU44 ELEVATED

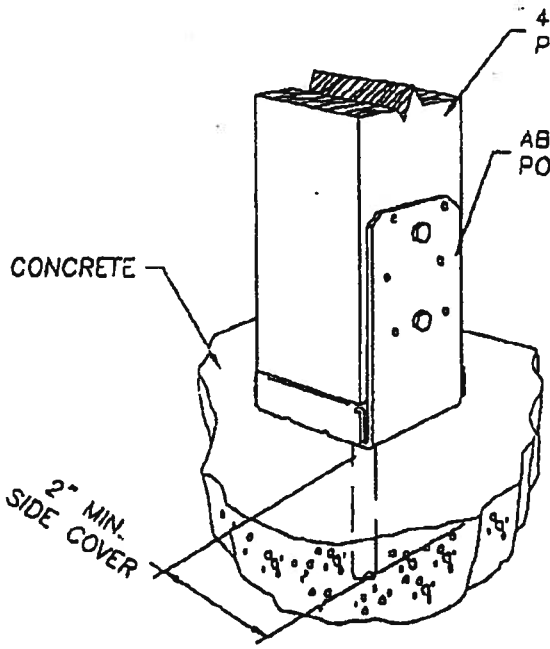
ABU ELEVATED POST BASES GENERAL NOTES

1. THIS PRODUCT IS DESIGNED TO MEET THE SOUTH FLORIDA BUILDING CODE 1994 EDITION FOR MIAMI-DADE COUNTY.
2. THE STEEL SHALL CONFORM TO ASTM A-653 CS WITH Fy. MIN. = 28ksi & Fu. MIN. = 38ksi FOR BASE & ASTM A-653 SS GRADE 33 WITH Fy MIN. = 33 ksi & Fu MIN. = 45ksi FOR THE STRAP, & HAVE A MINIMUM GALVANIZED COATING OF G60.
3. FASTENERS ARE COMMON WIRE NAILS UNLESS OTHERWISE NOTED.
4. ALLOWABLE DOWNLIFT LOADS HAVE NOT BEEN INCREASED BY ANY DURATION FACTOR.
5. ALLOWABLE UPLIFT LOADS HAVE BEEN INCREASED 33% FOR WIND LOADING. LOADS MAY NOT BE INCREASED FOR SHORT-TERM LOADING.
6. ALLOWABLE LOADS ARE BASED ON THE NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION 1991 EDITION & 1993 ERRATA, FOR DOUGLAS FIR-LARCH (G = 0.50 OR BETTER) & TESTS PERFORMED IN ACCORDANCE WITH ASTM D1761.
7. NOT RECOMMENDED FOR NON-TOP SUPPORTED INSTALLATIONS SUCH AS FENCES.
8. FOR PRE-POUR INSTALLED ANCHORS, EMBED 4" INTO CONCRETE. FOR EPOXY OR WEDGE ANCHORS, SELECT DESIGN & INSTALL ACCORDING TO MANUFACTURER'S RECOMMENDATIONS & SOUND ENGINEERING PRACTICES.

TABLE OF CONTENTS

MODEL NO.	NOMINAL POST SIZE	MATERIAL		DIMENSIONS					FASTENER	
		BASE	STRAP	W1	W2	D	L	HB	ANCHOR DIA.[2]	NAILS
ABU44	4 x 4	16GA.	12GA.	3 9/16"	3 1/2"	–	3"	1 3/4"	5/8"	12-16d
ABU46	4 x 6	12GA.	12GA.	3 9/16"	5 3/8"	–	5"	2 5/8"	5/8"	12-16d
ABU66	6 x 6	12GA.	10GA.	5 1/2"	5 3/8"	–	5"	1 3/4"	5/8"	12-16d

Mark
State Of Florida
Marc



ABU44 ELEVATED POST BASE
TYPICAL INSTALLATION

BASE

	ALLOWABLE LOADS		
	UPLIFT (133)		
TS DIA.	NAILS	BOLTS	DOWN (100)
1/2"	1340	1250	6400
1/2"	2300	1665	9635
1/2"	1980	1700	10000

thennan
ional Engineer #40116
01

MAY 10 2001

00-0512.11

SIMPSON STRONG-TIE CO., INC.
4637 CHABOT DRIVE, SUITE 200
PLEASANTON, CA. 94588
PH. 800.925.5099
FAX. 925.847.3068

PRODUCT: POST BASES, SEISMIC & HURRICANE TIES & EMBEDDED TRUSS ANCHORS

PART OR ASSEMBLY: ABU ELEVATED POST BASES NOTES & CHART

NO.	DATE	REVISIONS	BY
1	3/08/01	GENERAL REVISION	TJH

RW BUILDING CONSULTANTS, INC
813.684.3831

DATE: 10/23/00
SCALE: N.T.S.
DWG. BY: TJH
CHK. BY: RW
DRAWING NO.: S-2068
SHEET 1 OF 5

Project Information for:	L148527		
Builder:	Greg Talley	Date:	1/30/2006
Lot:	Lot 33 Cannon Creek Place	Start Number:	**1149**
Subdivision:	n/a		
County or City:	Columbia		
Truss Page Count:	31		

Truss Design Load Information (UNO) Design Program: MiTek 5.2 / 6.2

Gravity		**Wind**		**Building Code:**	**FBC2004**
Roof (psf):	**42**	Wind Standard:	**ASC 7-02**		
Floor (psf):	55	Wind Speed (mph):	**110**		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

ROHNER, GEORGE JOSEPH CGC021619
Address: 3031 SW 108 WAY
OCALA, FL. 34478 Designer: **111**

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	0130061149	1/30/2006				
2	CJ3	0130061150	1/30/2006				
3	CJ5	0130061151	1/30/2006				
4	EJ3	0130061152	1/30/2006				
5	EJ7	0130061153	1/30/2006				
6	HJ3	0130061154	1/30/2006				
7	HJ7	0130061155	1/30/2006				
8	HJ7A	0130061156	1/30/2006				
9	T01	0130061157	1/30/2006				
10	T02	0130061158	1/30/2006				
11	T03	0130061159	1/30/2006				
12	T04	0130061160	1/30/2006				
13	T05	0130061161	1/30/2006				
14	T06	0130061162	1/30/2006				
15	T07	0130061163	1/30/2006				
16	T08	0130061164	1/30/2006				
17	T09	0130061165	1/30/2006				
18	T10	0130061166	1/30/2006				
19	T11	0130061167	1/30/2006				
20	T12	0130061168	1/30/2006				
21	T13	0130061169	1/30/2006				
22	T14	0130061170	1/30/2006				
23	T15	0130061171	1/30/2006				
24	T16	0130061172	1/30/2006				
25	T17	0130061173	1/30/2006				
26	T18	0130061174	1/30/2006				
27	T19	0130061175	1/30/2006				
28	T20	0130061176	1/30/2006				
29	T21	0130061177	1/30/2006				
30	T22	0130061178	1/30/2006				
31	T23	0130061179	1/30/2006				

JAN 30 2006

Licensee Details

Licensee Information

Name:	**ROHNER, GEORGE JOSEPH** (Primary Name) **R B K BUILDERS INC** (DBA Name)
Main Address:	**3031 SW 108 WAY** **PO BOX 290023** **DAVIE Florida 33329-0023**
County:	**BROWARD**
License Mailing:	
LicenseLocation:	**3031 SW 108 WAY** **PO BOX 290023** **DAVIE FL 33329-0023**
County:	**BROWARD**

License Information

License Type:	**Certified General Contractor**
Rank:	**Cert General**
License Number:	**CGC021619**
Status:	**Current,Active**
Licensure Date:	**03/14/1982**
Expires:	**08/31/2006**
Special Qualifications	**Qualification Effective**
Bldg Code Core Course Credit	
Qualified Business License Required	**02/20/2004**

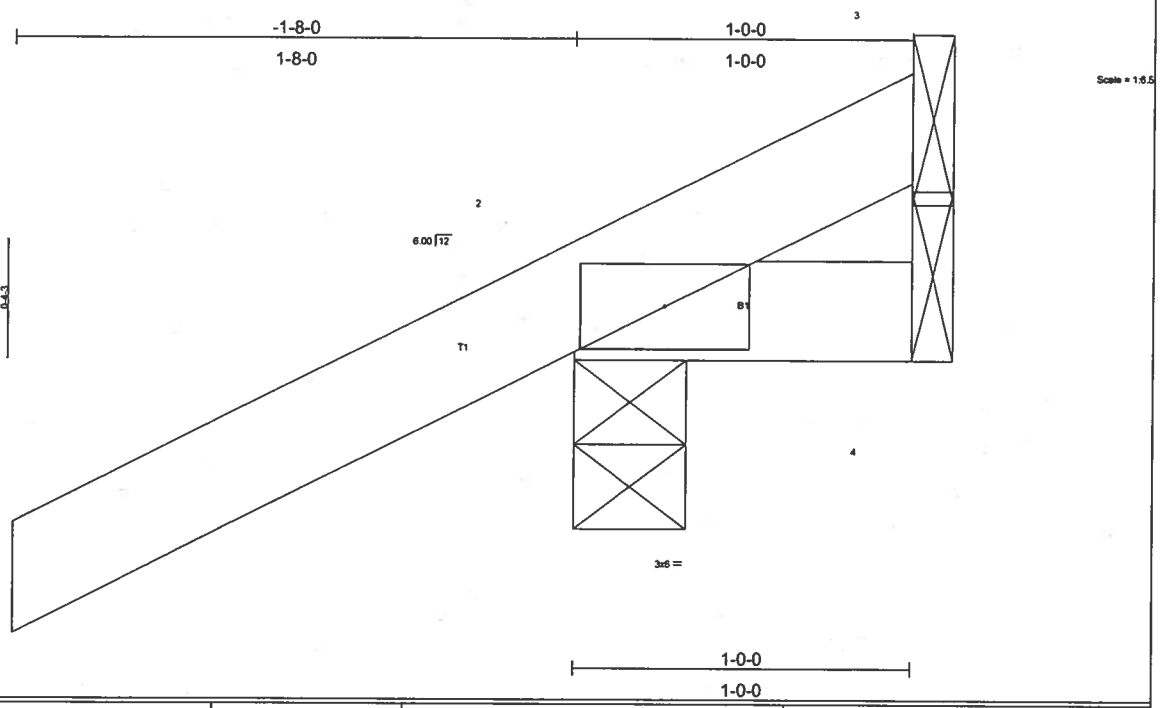
View Related License Information

View License Complaint

Dwg.#0130061149

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	CJ1	MONO TRUSS	14	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:26 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.19	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 6 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=214/0-4-0, 4=14/Mechanical, 3=-56/Mechanical
Max Horz 2=76(load case 5)
Max Uplift2=-222(load case 5), 4=-9(load case 3), 3=-56(load case 1)
Max Grav 2=214(load case 1), 4=14(load case 1), 3=81(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-53/47
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.11

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 222 lb uplift at joint 2, 9 lb uplift at joint 4 and 56 lb uplift at joint 3.

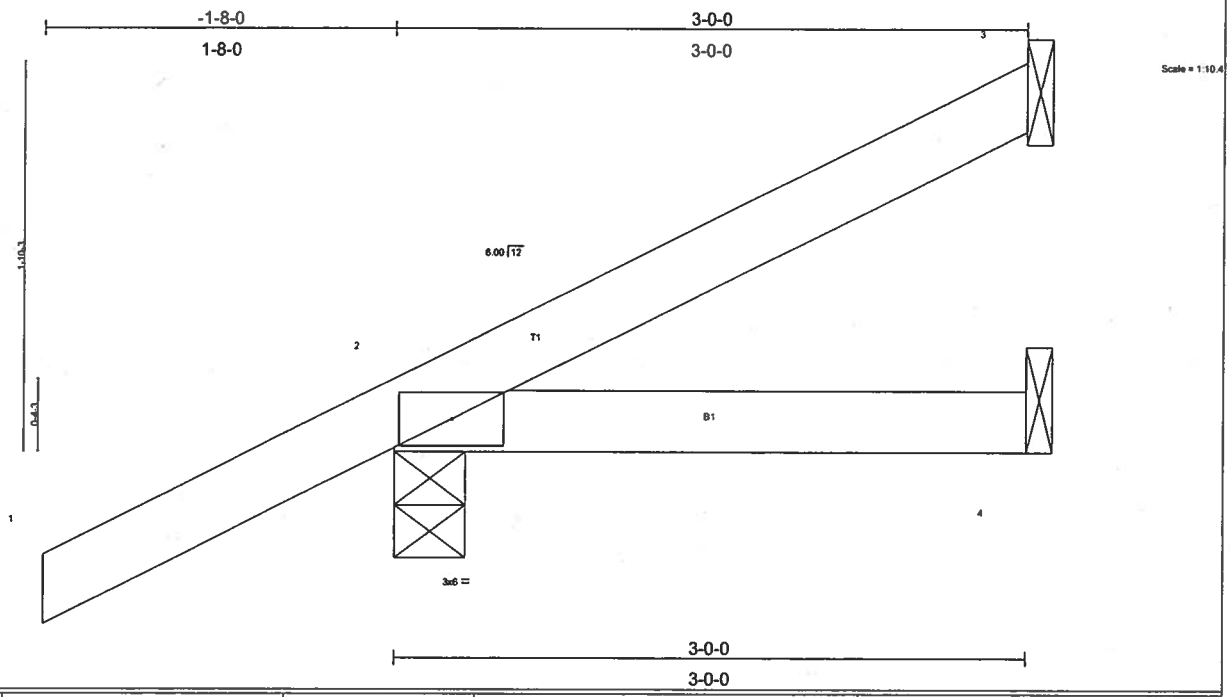
LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061150

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	CJ3	MONO TRUSS	10	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:27 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.00	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.05	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=42/Mechanical, 2=248/0-4-0, 4=42/Mechanical
Max Horz 2=121(load case 5)
Max Uplift3=-33(load case 6), 2=-169(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-51/13
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.11

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 33 lb uplift at joint 3 and 169 lb uplift at joint 2.

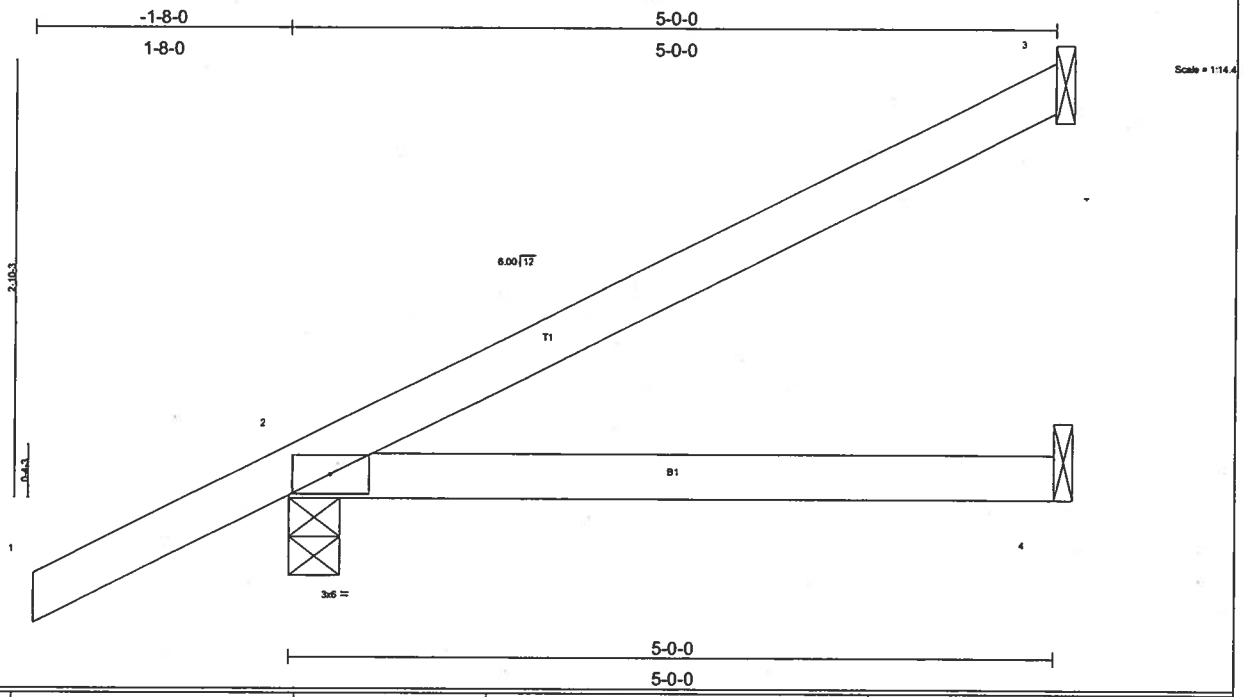
LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061151

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	CJ5	MONO TRUSS	10	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:28 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	-0.03	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=110/Mechanical, 2=318/0-4-0, 4=72/Mechanical
Max Horz 2=167(load case 5)
Max Uplift3=-96(load case 5), 2=-172(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-93/39
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.14

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 96 lb uplift at joint 3 and 172 lb uplift at joint 2.

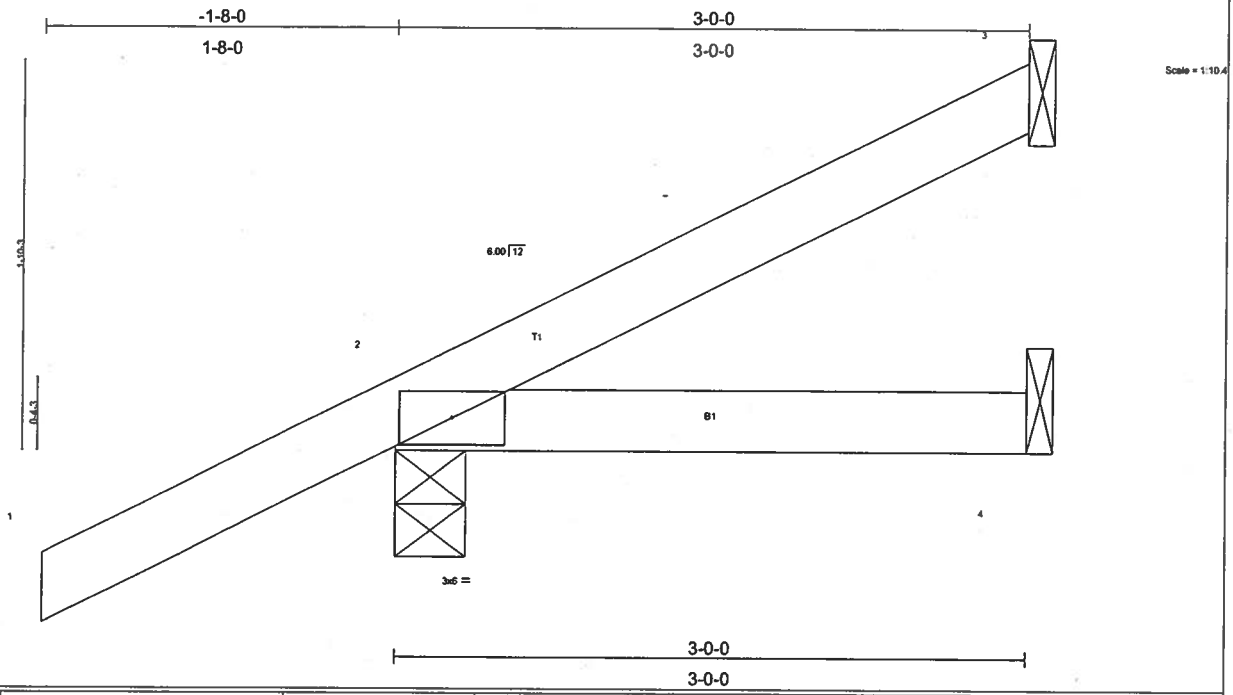
LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061152

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	EJ3	MONO TRUSS	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:28 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=42/Mechanical, 2=248/0-4-0, 4=42/Mechanical
Max Horz 2=121(load case 5)
Max Uplift3=-33(load case 6), 2=-204(load case 5), 4=-26(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-51/13
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.11

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 33 lb uplift at joint 3, 204 lb uplift at joint 2 and 26 lb uplift at joint 4.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061153

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	EJ7	MONO TRUSS	29	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:29 2006 Page 1

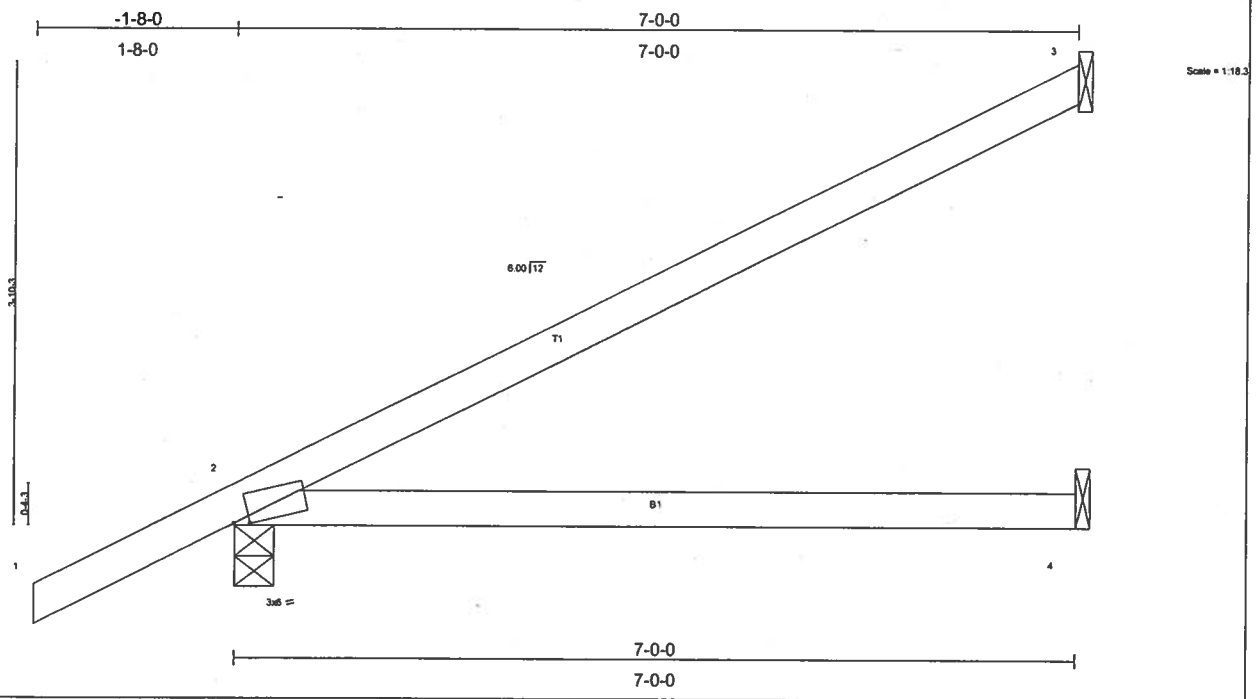


Plate Offsets (X,Y): [2:0-1-9,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.13	2-4	>631	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.36	Vert(TL)	-0.21	2-4	>379	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 25 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=164/Mechanical, 2=397/0-4-0, 4=107/Mechanical
Max Horz 2=213(load case 5)
Max Uplift3=-137(load case 5), 2=-185(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-121/59
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.85

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 137 lb uplift at joint 3 and 185 lb uplift at joint 2.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061154

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	HJ3	MONO TRUSS	2	1	Job Reference (optional)

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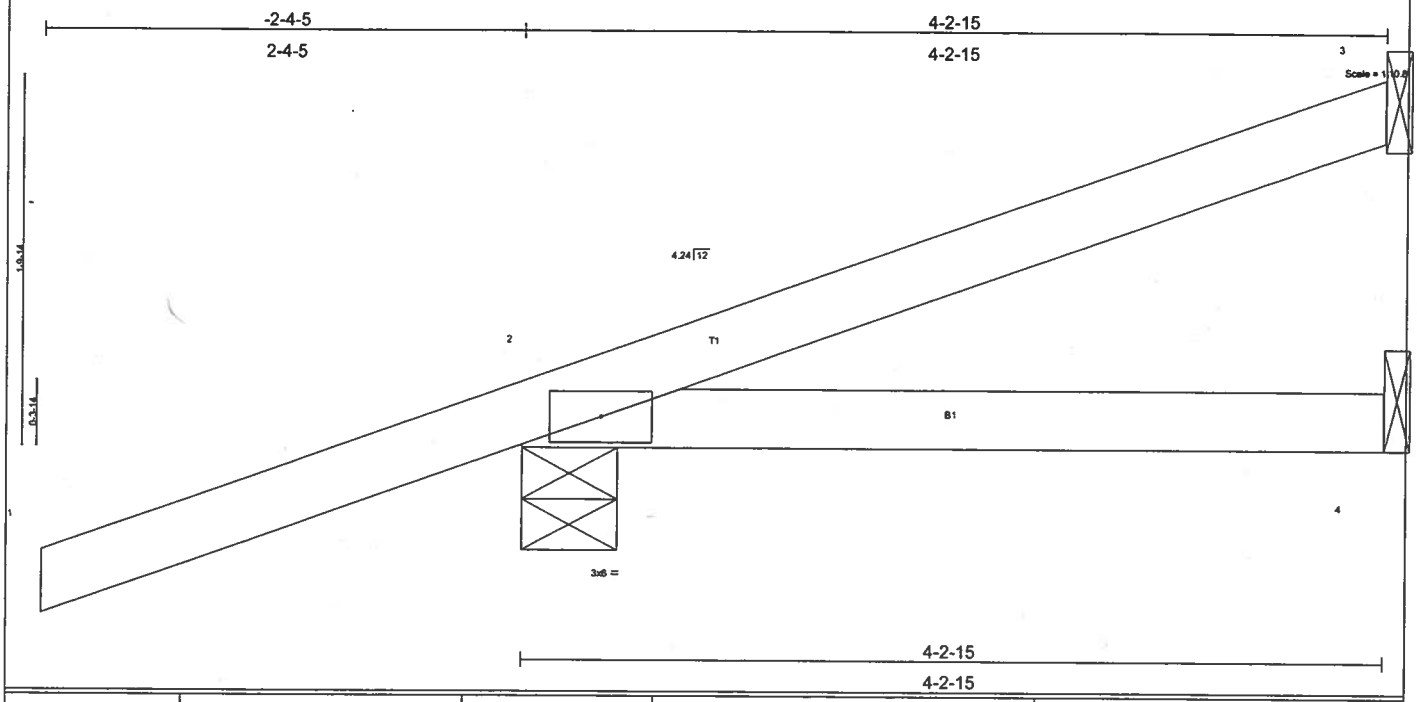

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	0.02	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 17 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=249/0-5-11, 4=42/Mechanical
Max Horz 2=87(load case 2)
Max Uplift3=-12(load case 5), 2=-257(load case 2), 4=-41(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/42, 2-3=-31/6
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.10

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 12 lb uplift at joint 3, 257 lb uplift at joint 2 and 41 lb uplift at joint 4.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-3=-57(F=-2, B=-2), 2=0(F=15, B=15)-to-4=-32(F=-1, B=-1)

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061155

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	HJ7	MONO TRUSS	4	1	Job Reference (optional)

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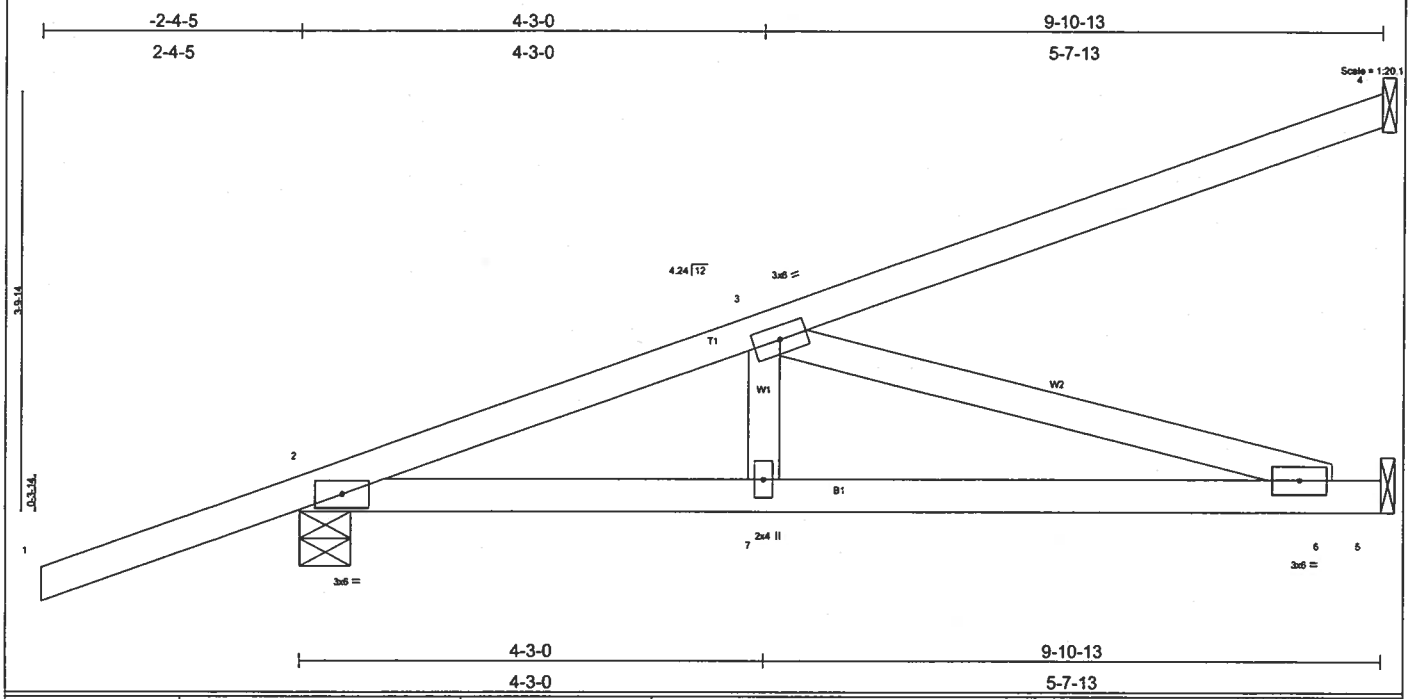

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.11	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.18	6-7	>625	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.49	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 44 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-11-1 oc bracing.

REACTIONS (lb/size) 4=269/Mechanical, 2=502/0-5-11, 5=383/Mechanical
Max Horz 2=258(load case 2)
Max Uplift4=-230(load case 2), 2=-290(load case 4), 5=-72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/42, 2-3=-919/162, 3-4=-104/65
BOT CHORD 2-7=-351/854, 6-7=-351/854, 5-6=0/0
WEBS 3-7=0/202, 3-6=-890/366

JOINT STRESS INDEX
2 = 0.71, 3 = 0.24, 6 = 0.25 and 7 = 0.15

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 230 lb uplift at joint 4, 290 lb uplift at joint 2 and 72 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=0(F=15, B=15)-to-5=-74(F=-22, B=-22)

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061156

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	HJ7A	MONO TRUSS	1	1	Job Reference (optional)

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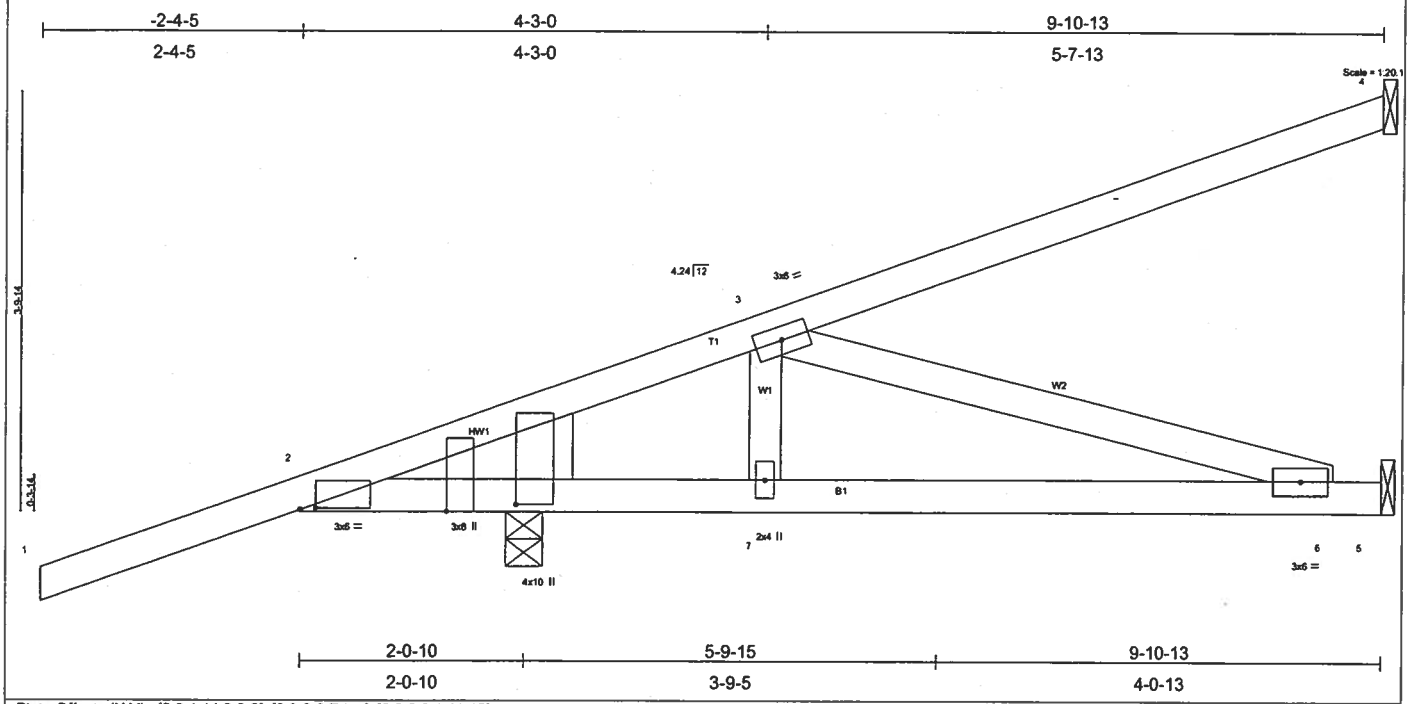

Plate Offsets (X,Y): [2:0-1-11,0-0-3], [2:0-0-3,Edge], [2:0-0-9,1-11-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.11	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.60	Vert(TL)	-0.19	6-7	>621	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.43	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 49 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 8 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-7-4 oc bracing.

REACTIONS (lb/size) 4=269/Mechanical, 5=361/Mechanical, 2=400/0-4-0
Max Horz 2=258(load case 2)
Max Uplift4=-230(load case 2), 5=-79(load case 2), 2=-306(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/42, 2-3=-818/191, 3-4=-104/66
BOT CHORD 2-7=-379/759, 6-7=-379/759, 5-6=0/0
WEBS 3-7=0/140, 3-6=-791/395

JOINT STRESS INDEX
2 = 0.67, 2 = 0.22, 2 = 0.27, 3 = 0.21, 6 = 0.22 and 7 = 0.10

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 230 lb uplift at joint 4, 79 lb uplift at joint 5 and 306 lb uplift at joint 2.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-2(F=26, B=26)-to-4=-134(F=-40, B=-40), 2=30(F=15, B=15)-to-7=-1(F=-1, B=-1), 7=-31(F=-1, B=-1)-to-5=-74(F=-22, B=-22)

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061157

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T01	HIP	1	1	Job Reference (optional)

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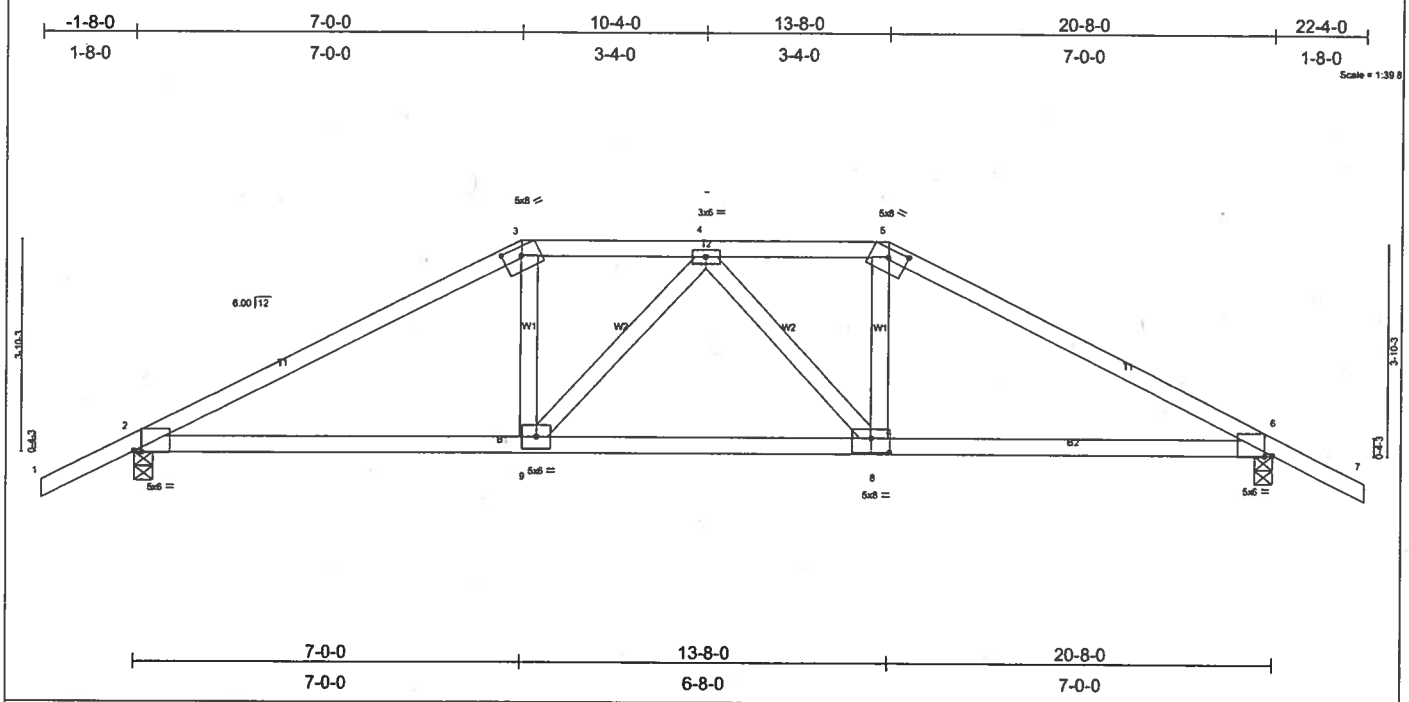

Plate Offsets (X,Y): [2:0-1-11,Edge], [3:0-4-0,0-1-15], [5:0-4-0,0-1-15], [6:0-1-11,Edge], [8:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	-0.20	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.91	Vert(TL)	-0.33	8-9	>749	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.35	Horz(TL)	0.10	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 93 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-1-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-8-12 oc bracing.

REACTIONS (lb/size) 2=1814/0-4-0, 6=1814/0-4-0
Max Horz 2=-81(load case 5)
Max Uplift2=-809(load case 4), 6=-809(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-3265/1315, 3-4=-2871/1249, 4-5=-2871/1249, 5-6=-3265/1315, 6-7=0/39
BOT CHORD 2-9=-1102/2828, 8-9=-1226/2993, 6-8=-1059/2828
WEBS 3-9=-398/1097, 4-9=-308/260, 4-8=-308/260, 5-8=-398/1097

JOINT STRESS INDEX
2 = 0.82, 3 = 0.75, 4 = 0.38, 5 = 0.75, 6 = 0.82, 8 = 0.77 and 9 = 0.39

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 809 lb uplift at joint 2 and 809 lb uplift at joint 6.
5) Girder carries hip end with 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 13-8-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-117(F=-63), 5-7=-54, 2-9=-30, 8-9=-65(F=-35), 6-8=-30
Concentrated Loads (lb)
Vert: 8=-539(F) 9=-539(F)

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061158

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T02	HIP	1	1	Job Reference (optional)

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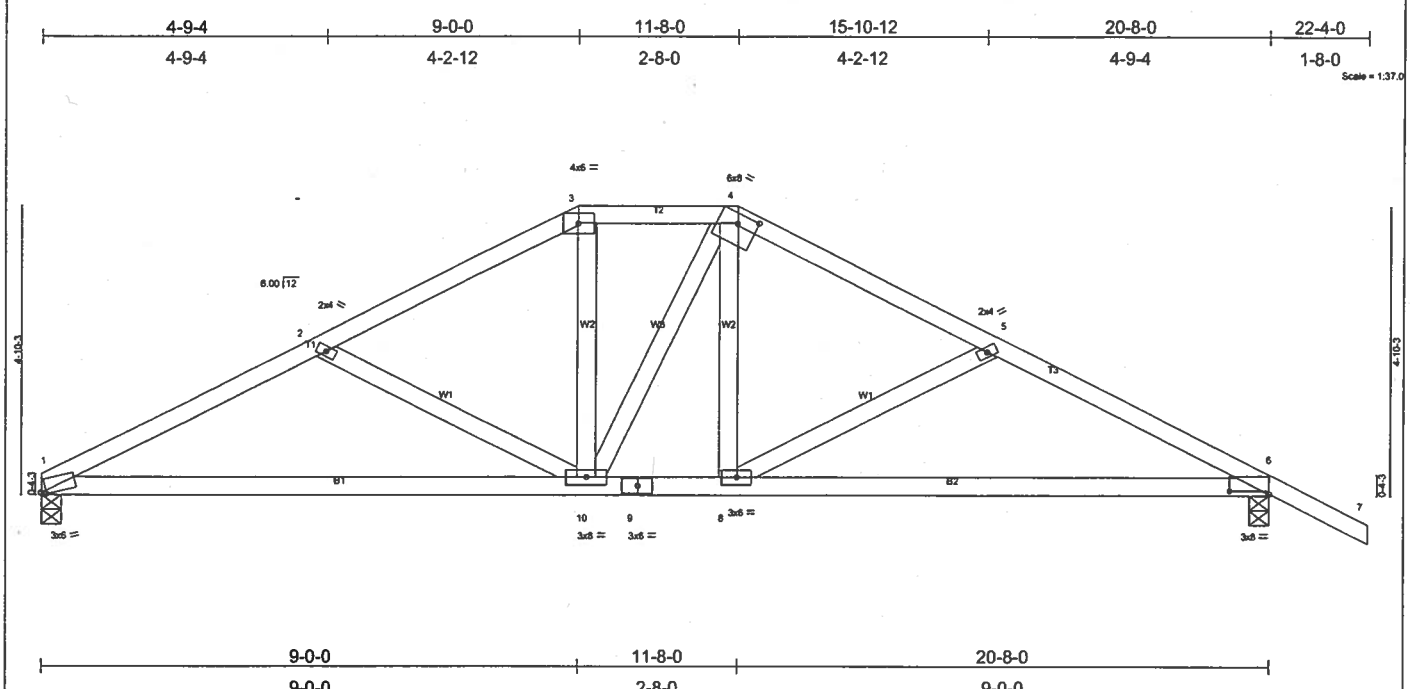

Plate Offsets (X,Y): [1:0-1-1,0-0-7], [4:0-4-0,0-1-15], [6:0-8-0,0-0-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.18	1-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.31	1-10	>798	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.14	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 102 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-3-4 oc bracing.

REACTIONS (lb/size) 1=850/0-4-0, 6=957/0-4-0
Max Horz 1=-115(load case 6)
Max Uplift1=-268(load case 5), 6=-378(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1425/656, 2-3=-1147/514, 3-4=-980/507, 4-5=-1139/502, 5-6=-1405/624, 6-7=0/39
BOT CHORD 1-10=-451/1242, 9-10=-203/974, 8-9=-203/974, 6-8=-412/1219
WEBS 2-10=-309/274, 3-10=-93/314, 4-10=-98/116, 4-8=-60/300, 5-8=-289/238

JOINT STRESS INDEX
1 = 0.88, 2 = 0.34, 3 = 0.33, 4 = 0.39, 5 = 0.34, 6 = 0.74, 8 = 0.35, 9 = 0.64 and 10 = 0.62

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 268 lb uplift at joint 1 and 378 lb uplift at joint 6.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061159

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T03	COMMON	4	1	Job Reference (optional)

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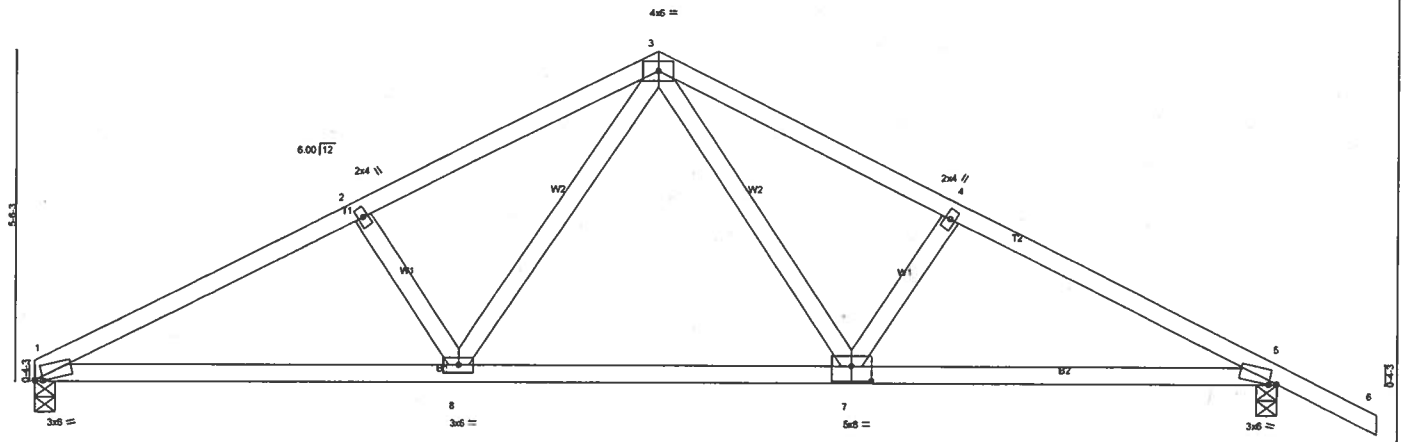
5-5-4	10-4-0	15-2-12	20-8-0	22-4-0
5-5-4	4-10-12	4-10-12	5-5-4	1-8-0

Scale = 1:36.6



7-0-13	13-7-3	20-8-0
7-0-13	6-6-5	7-0-13

Plate Offsets (X,Y): [1:0-1-9,0-0-7], [5:0-1-9,0-0-7], [7:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.17	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.78	Vert(TL)	-0.27	7-8	>888	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.23	Horz(TL)	0.05	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 95 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-5-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-1-12 oc bracing.

REACTIONS (lb/size) 1=1013/0-4-0, 5=1121/0-4-0
Max Horz 1=-124(load case 6)
Max Uplift1=-337(load case 5), 5=-448(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1822/803, 2-3=-1685/800, 3-4=-1668/772, 4-5=-1817/774, 5-6=0/39
BOT CHORD 1-8=-572/1576, 7-8=-279/1076, 5-7=-538/1555
WEBS 2-8=-248/244, 3-8=-306/724, 3-7=-262/699, 4-7=-235/225

JOINT STRESS INDEX
1 = 0.80, 2 = 0.34, 3 = 0.58, 4 = 0.34, 5 = 0.80, 7 = 0.67 and 8 = 0.55

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 337 lb uplift at joint 1 and 448 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-6=-54, 1-8=-30, 7-8=-80(F=-50), 5-7=-30

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061160

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T04	SPECIAL	1	1	Job Reference (optional)

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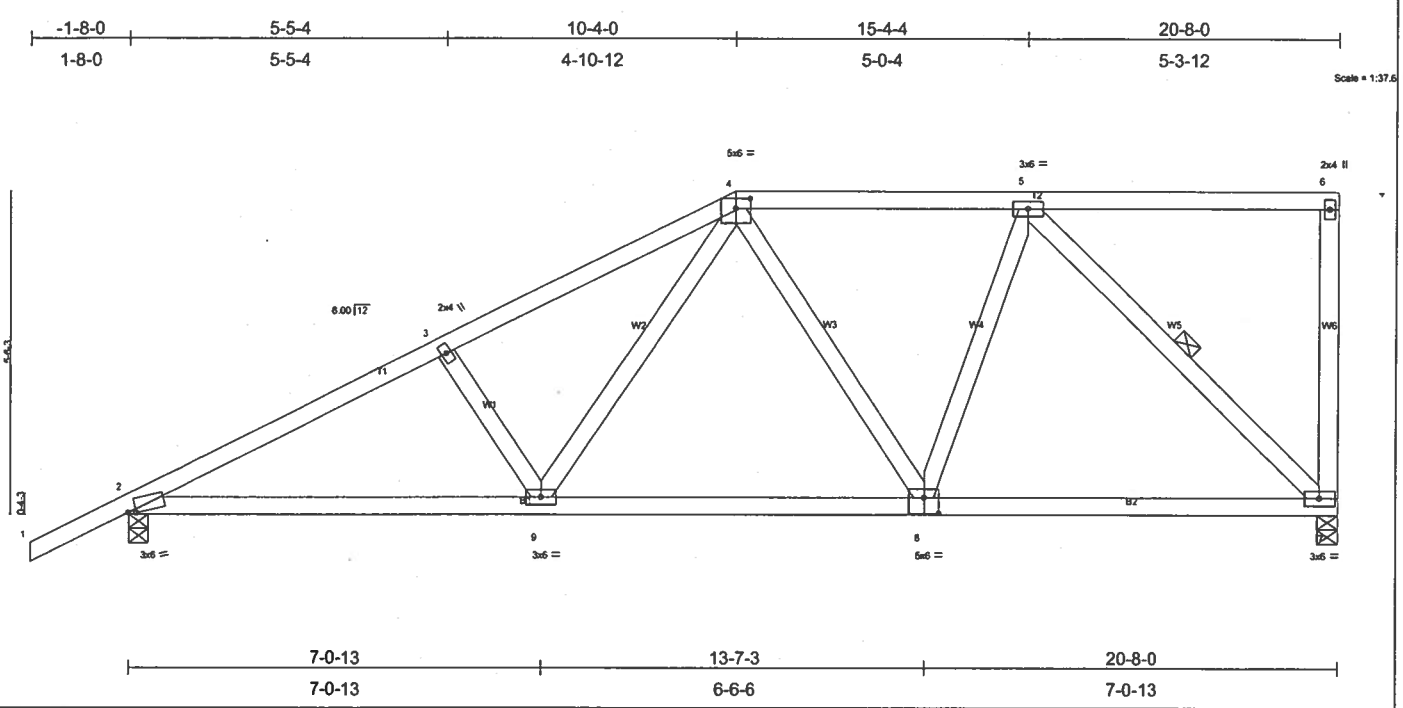

Plate Offsets (X,Y): [2:0-1-9,0-0-7], [4:0-3-0,0-2-0], [8:0-3-0,0-3-4]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.29	Vert(LL)	-0.16	8-9	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.75	Vert(TL)	-0.25	8-9	>979	180		
BCLL 10.0	Rep Stress Incr NO	WB 0.32	Horz(TL)	-0.04	2	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)						Weight: 114 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-6 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-2-11 oc bracing.
WEBS 1 Row at midpt 5-7

REACTIONS (lb/size) 7=1014/0-4-0, 2=1122/0-4-0
Max Horz 7=292(load case 5)
Max Uplift7=-356(load case 4), 2=-426(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-1820/684, 3-4=-1671/682, 4-5=-995/435, 5-6=-25/4, 6-7=-130/90
BOT CHORD 2-9=-459/1558, 8-9=-199/1075, 7-8=-153/821
WEBS 3-9=-222/233, 4-9=-274/687, 4-8=-151/159, 5-8=-181/547, 5-7=-1131/530

JOINT STRESS INDEX
2 = 0.80, 3 = 0.34, 4 = 0.53, 5 = 0.50, 6 = 0.76, 7 = 0.39, 8 = 0.84 and 9 = 0.52

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 356 lb uplift at joint 7 and 426 lb uplift at joint 2.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-6=-54, 2-9=-30, 8-9=-80(F=-50), 7-8=-30

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0130061161

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T05	SPECIAL	1	1	Job Reference (optional)

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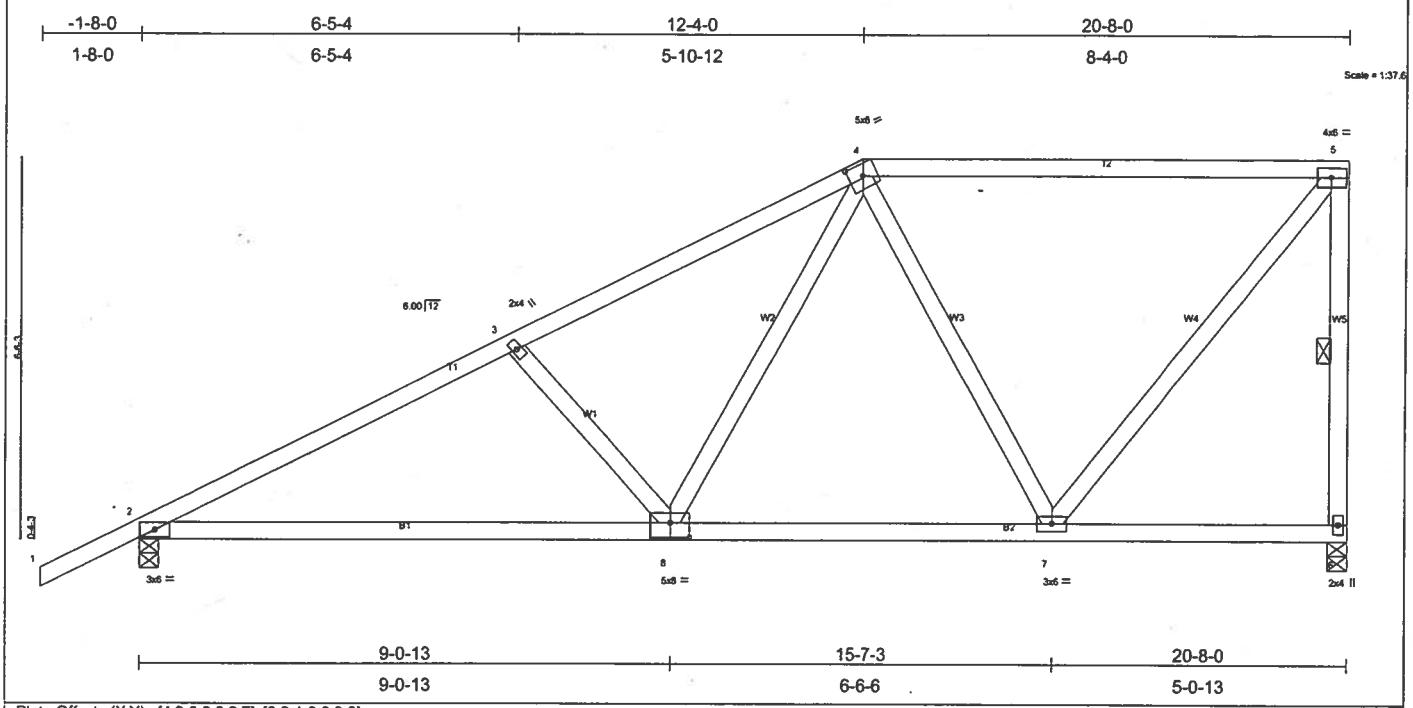

Plate Offsets (X,Y): [4:0-3-0,0-2-7], [8:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.72	Vert(LL)	-0.14	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.69	Vert(TL)	-0.24	2-8	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.48	Horz(TL)	-0.03	2	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 113 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 5-6

REACTIONS (lb/size) 6=1046/0-4-0, 2=1090/0-4-0
Max Horz 6=338(load case 5)
Max Uplift6=-357(load case 4), 2=-414(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-1664/612, 3-4=-1446/569, 4-5=-662/295, 5-6=-996/489
BOT CHORD 2-8=-382/1415, 7-8=-110/894, 6-7=0/358
WEBS 3-8=-278/281, 4-8=-274/734, 4-7=-500/341, 5-7=-437/1001

JOINT STRESS INDEX
2 = 0.73, 3 = 0.34, 4 = 0.75, 5 = 0.71, 6 = 0.70, 7 = 0.71 and 8 = 0.82

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 357 lb uplift at joint 6 and 414 lb uplift at joint 2.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-5=-54, 2-8=-30, 7-8=-80(F=-50), 6-7=-30

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061162

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T06	MONO HIP	1	1	Job Reference (optional)

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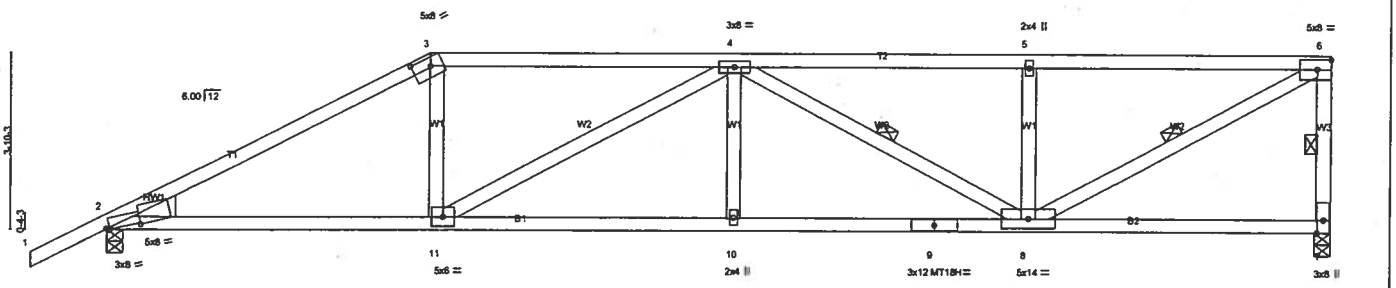

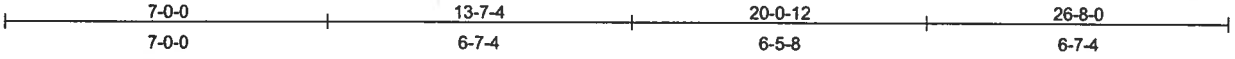

Plate Offsets (X,Y): [2:0-8-10,0-0-13], [2:0-0-10,Edge], [3:0-4-12,0-2-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.80	Vert(LL)	-0.32	10-11	>987	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.90	Vert(TL)	-0.52	10-11	>609	180	MT18H	244/190
BCLL	10.0	Rep Stress Incr	NO	WB	0.86	Horz(TL)	0.13	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 136 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.2 *Except*
W1 2 X 4 SYP No.3, W1 2 X 4 SYP No.3, W1 2 X 4 SYP No.3
WEDGE
Left: 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-4-11 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-6-7 oc bracing.
WEBS 1 Row at midpt 6-7, 4-8, 6-8

REACTIONS (lb/size) 7=2449/0-4-0, 2=2310/0-4-0
Max Horz 2=216(load case 4)
Max Uplift7=-1102(load case 3), 2=-982(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-4366/1844, 3-4=-3871/1705, 4-5=-3546/1591, 5-6=-3546/1591, 6-7=-2233/1125
BOT CHORD 2-11=-1687/3814, 10-11=-2159/4803, 9-10=-2159/4803, 8-9=-2159/4803, 7-8=-75/159
WEBS 3-11=-475/1383, 4-11=-1064/580, 4-10=0/429, 4-8=-1434/648, 5-8=-763/629, 6-8=-1731/3867

JOINT STRESS INDEX
2 = 0.80, 2 = 0.73, 3 = 0.92, 4 = 0.68, 5 = 0.36, 6 = 0.79, 7 = 0.61, 8 = 0.94, 9 = 0.99, 10 = 0.34 and 11 = 0.49

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) All plates are MT20 plates unless otherwise indicated.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1102 lb uplift at joint 7 and 982 lb uplift at joint 2.
5) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-6=-117(F=-63), 2-11=-30, 7-11=-65(F=-35)
Concentrated Loads (lb)
Vert: 11=-539(F)

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0130061163

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T07	MONO HIP	1	1	Job Reference (optional)

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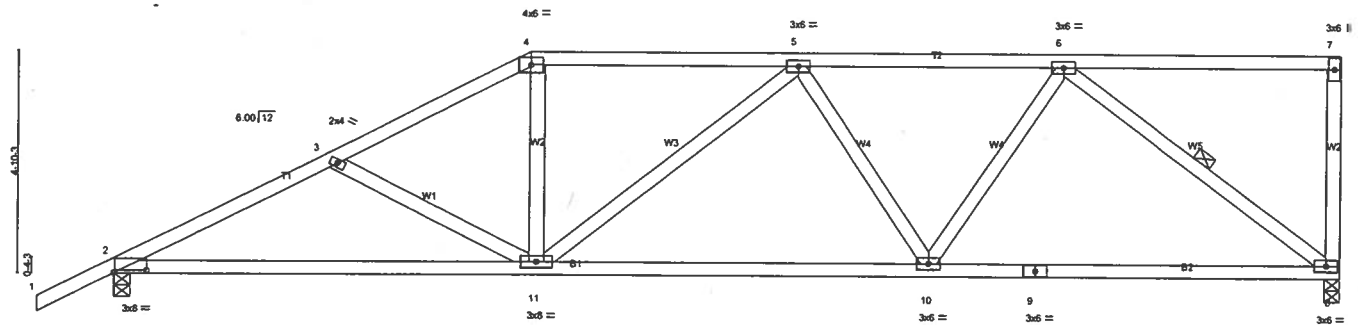
-1-8-0	4-9-4	9-0-0	14-9-8	20-7-0	26-8-0
1-8-0	4-9-4	4-2-12	5-9-8	5-9-8	6-1-0

Scale: 1/4"=1'



9-0-0	17-8-4	26-8-0
9-0-0	8-8-4	8-11-12

Plate Offsets (X,Y): [2:0-8-4,0-0-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.17	2-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.55	Vert(TL)	-0.29	2-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.40	Horz(TL)	0.06	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 140 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-10-14 oc bracing.
WEBS 1 Row at midpt 6-8

REACTIONS (lb/size) 8=1103/0-4-0, 2=1209/0-4-0
Max Horz 2=261(load case 5)
Max Uplift8=-401(load case 4), 2=-425(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-1941/771, 3-4=-1704/663, 4-5=-1493/648, 5-6=-1445/575, 6-7=-61/11, 7-8=-156/104
BOT CHORD 2-11=-837/1688, 10-11=-692/1593, 9-10=-494/1129, 8-9=-494/1129
WEBS 3-11=-236/214, 4-11=-77/462, 5-11=-128/168, 5-10=-277/218, 6-10=-152/588, 6-8=-1346/609

JOINT STRESS INDEX
2 = 0.80, 3 = 0.34, 4 = 0.61, 5 = 0.42, 6 = 0.45, 7 = 0.37, 8 = 0.74, 9 = 0.48, 10 = 0.45 and 11 = 0.57

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 401 lb uplift at joint 8 and 425 lb uplift at joint 2.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0130061164

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T08	MONO HIP	1	1	Job Reference (optional)

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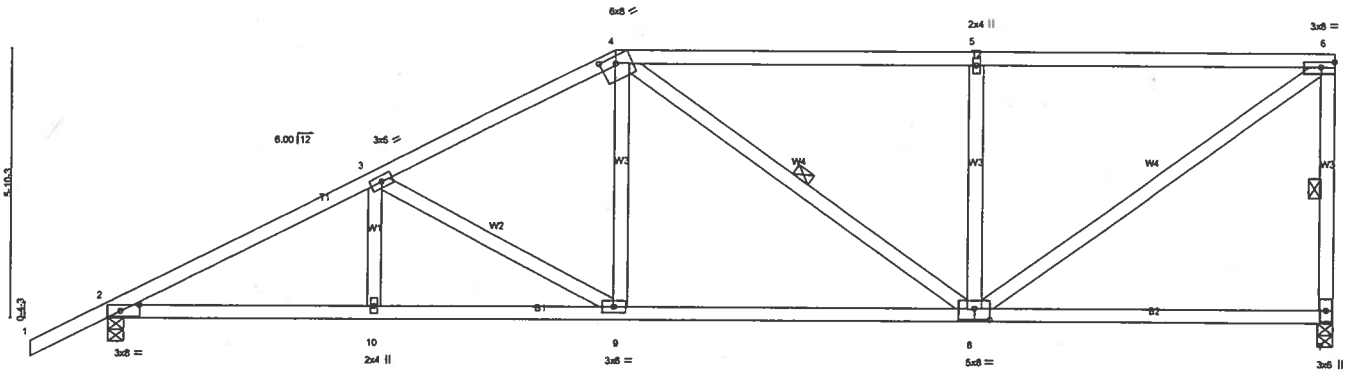
-1-8-0 | 5-9-4 | 11-0-0 | 18-10-0 | 26-8-0
1-8-0 | 5-9-4 | 5-2-12 | 7-10-0 | 7-10-0

Scale: 1/4"=1'



5-9-4 | 11-0-0 | 18-10-0 | 26-8-0
5-9-4 | 5-2-12 | 7-10-0 | 7-10-0

Plate Offsets (X,Y): [2:0-4-12,0-1-8], [4:0-4-0,0-1-15], [8:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.65	Vert(LL)	-0.13	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.22	8-9	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.92	Horz(TL)	0.05	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 147 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-11-4 oc bracing.
WEBS 1 Row at midpt 6-7, 4-8

REACTIONS (lb/size) 7=1103/0-4-0, 2=1209/0-4-0
Max Horz 2=307(load case 5)
Max Uplift7=-394(load case 4), 2=-434(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-1989/723, 3-4=-1523/614, 4-5=-1163/500, 5-6=-1163/500, 6-7=-992/480
BOT CHORD 2-10=-836/1703, 9-10=-836/1703, 8-9=-613/1323, 7-8=-21/53
WEBS 3-10=0/165, 3-9=-445/256, 4-9=-88/413, 4-8=-197/142, 5-8=-448/324, 6-8=-591/1370

JOINT STRESS INDEX
2 = 0.74, 3 = 0.41, 4 = 0.68, 5 = 0.34, 6 = 0.69, 7 = 0.40, 8 = 0.80, 9 = 0.35 and 10 = 0.34

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 394 lb uplift at joint 7 and 434 lb uplift at joint 2.

LOAD CASE(S) Standard

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061165

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T09	HIP	1	1	Job Reference (optional)

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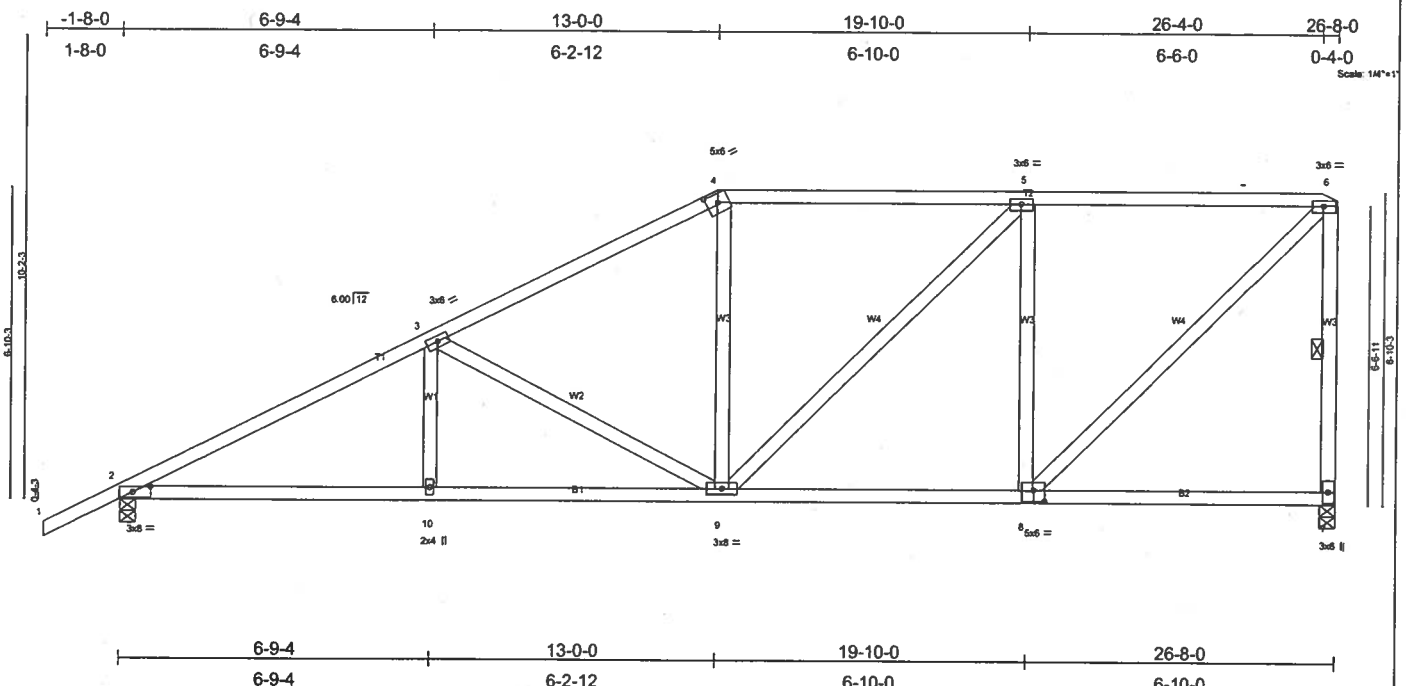

Plate Offsets (X,Y): [2:0-4-12,0-1-8], [4:0-3-0,0-2-7], [8:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	-0.11	2-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.17	2-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.81	Horz(TL)	0.05	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 154 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-9-14 oc bracing.
WEBS 1 Row at midpt 6-7

REACTIONS (lb/size) 2=1209/0-4-0, 7=1103/0-4-0
Max Horz 2=354(load case 5)
Max Uplift2=-439(load case 5), 7=-384(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-1956/696, 3-4=-1352/541, 4-5=-1146/550, 5-6=-891/397, 6-7=-1007/493
BOT CHORD 2-10=-854/1670, 9-10=-854/1670, 8-9=-397/891, 7-8=-13/32
WEBS 3-10=0/216, 3-9=-601/344, 4-9=0/226, 5-9=-215/356, 5-8=-636/426, 6-8=-538/1204

JOINT STRESS INDEX
2 = 0.73, 3 = 0.41, 4 = 0.58, 5 = 0.37, 6 = 0.76, 7 = 0.30, 8 = 0.53, 9 = 0.57 and 10 = 0.34

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 439 lb uplift at joint 2 and 384 lb uplift at joint 7.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061166

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T10	HIP	1	1	Job Reference (optional)

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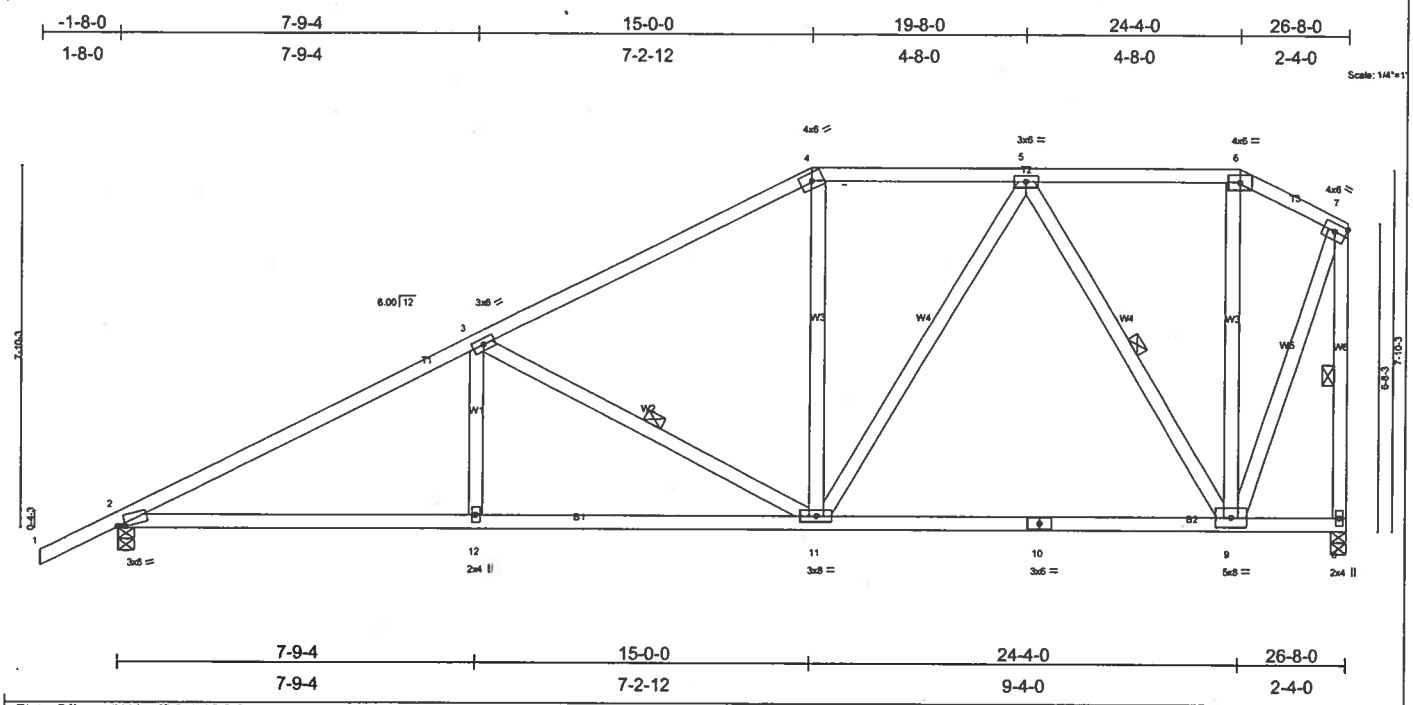

Plate Offsets (X,Y): [2:0-1-13,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.16	9-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.57	Vert(TL)	-0.28	9-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.32	Horz(TL)	0.05	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 169 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-9-8 oc bracing.
WEBS 1 Row at midpt 3-11, 5-9, 7-8

REACTIONS (lb/size) 2=1209/0-4-0, 8=1103/0-4-0
Max Horz 2=364(load case 5)
Max Uplift2=-450(load case 5), 8=-328(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-1908/694, 3-4=-1194/493, 4-5=-986/514, 5-6=-341/192, 6-7=-395/178, 7-8=-1138/474
BOT CHORD 2-12=-835/1622, 11-12=-835/1622, 10-11=-341/733, 9-10=-341/733, 8-9=-4/2
WEBS 3-12=0/243, 3-11=-724/426, 4-11=0/195, 5-11=-226/497, 5-9=-770/401, 6-9=-56/81, 7-9=-392/997

JOINT STRESS INDEX
2 = 0.81, 3 = 0.41, 4 = 0.72, 5 = 0.44, 6 = 0.33, 7 = 0.60, 8 = 0.42, 9 = 0.58, 10 = 0.53, 11 = 0.59 and 12 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 450 lb uplift at joint 2 and 328 lb uplift at joint 8.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061167

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T11	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:41 2006 Page 1

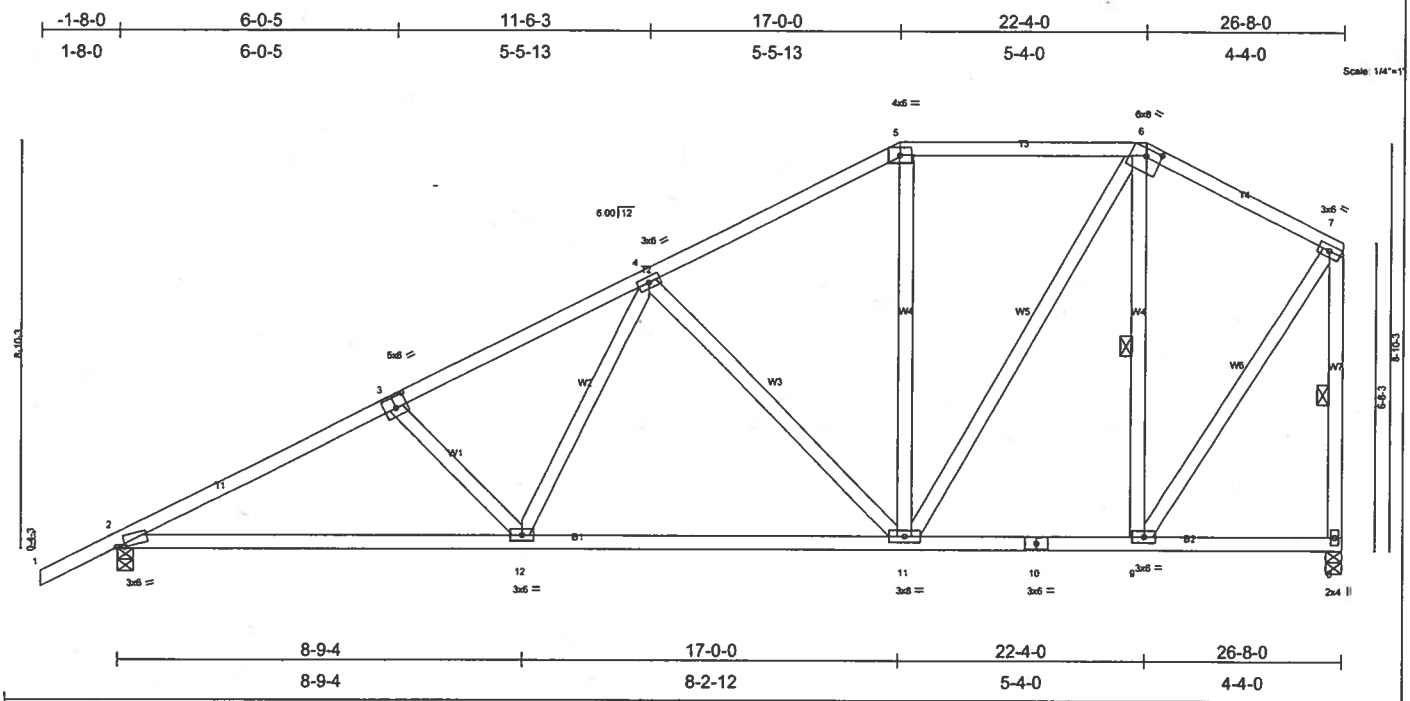

Plate Offsets (X,Y): [2:0-1-13,0-0-7], [3:0-3-0,0-3-0], [6:0-4-0,0-1-15]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	-0.16	2-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.27	2-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.72	Horz(TL)	0.04	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 172 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-8-6 oc bracing.
WEBS 1 Row at midpt 6-9, 7-8

REACTIONS (lb/size) 2=1209/0-4-0, 8=1103/0-4-0
Max Horz 2=378(load case 5)
Max Uplift2=-456(load case 5), 8=-349(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-1919/739, 3-4=-1712/685, 4-5=-965/461, 5-6=-806/470, 6-7=-584/296, 7-8=-1039/508
BOT CHORD 2-12=-894/1661, 11-12=-640/1238, 10-11=-204/473, 9-10=-204/473, 8-9=-7/13
WEBS 3-12=-284/273, 4-12=-159/541, 4-11=-621/397, 5-11=0/103, 6-11=-307/667, 6-9=-574/332, 7-9=-362/842

JOINT STRESS INDEX
2 = 0.82, 3 = 0.46, 4 = 0.41, 5 = 0.62, 6 = 0.49, 7 = 0.70, 8 = 0.51, 9 = 0.65, 10 = 0.19, 11 = 0.72 and 12 = 0.46

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 456 lb uplift at joint 2 and 349 lb uplift at joint 8.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ. FL 33549

Dwg.#0130061168

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T12	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:42 2006 Page 1

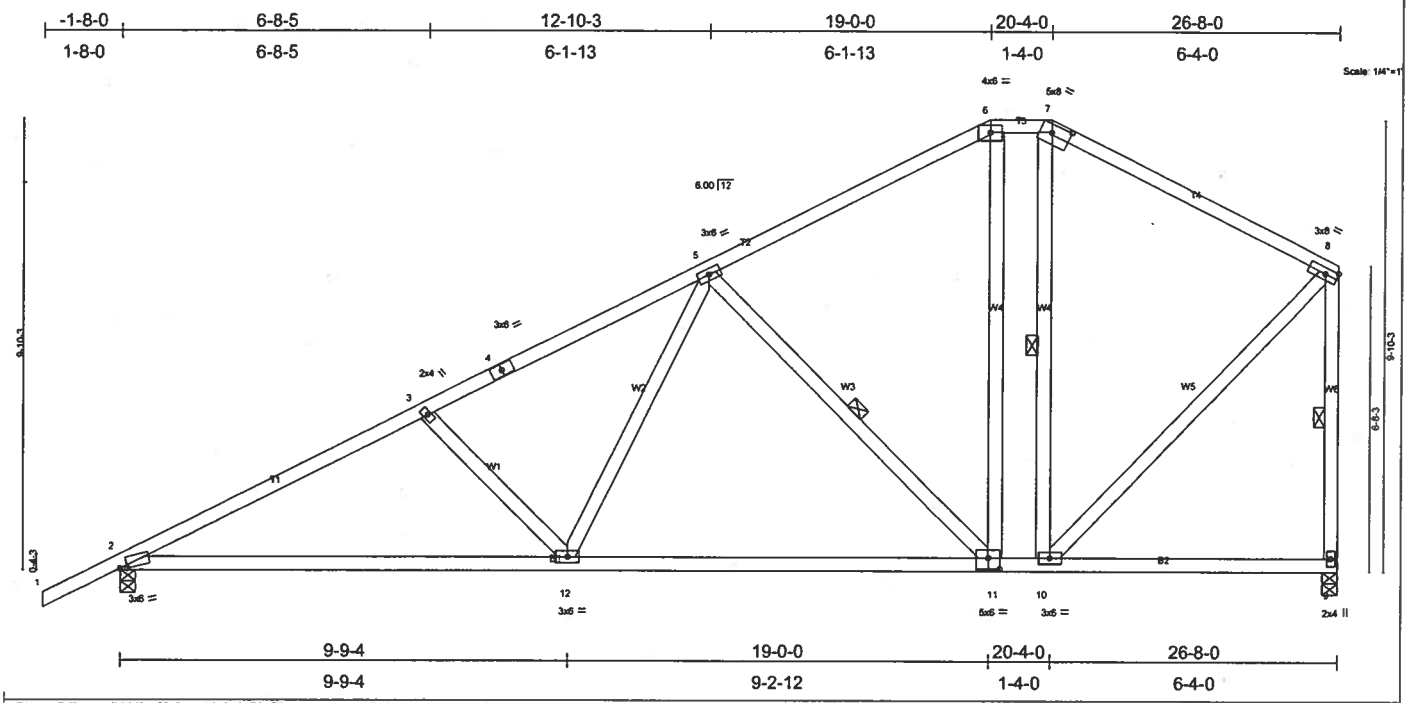

Plate Offsets (X,Y): [2:0-1-13,0-0-7], [7:0-4-12,0-2-4], [11:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.32	11-12	>989	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.78	Vert(TL)	-0.51	11-12	>622	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.52	Horz(TL)	0.04	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 166 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 5-11, 7-10, 8-9

REACTIONS (lb/size) 2=1209/0-4-0, 9=1103/0-4-0
Max Horz 2=392(load case 5)
Max Uplift2=-461(load case 5), 9=-373(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-1885/741, 3-4=-1651/655, 4-5=-1515/676, 5-6=-773/403, 6-7=-616/423, 7-8=-748/387, 8-9=-1053/540
BOT CHORD 2-12=-888/1628, 11-12=-592/1139, 10-11=-265/616, 9-10=-14/22
WEBS 3-12=-316/308, 5-12=-194/635, 5-11=-768/472, 6-11=-154/386, 7-10=-207/128, 8-10=-366/865

JOINT STRESS INDEX
2 = 0.80, 3 = 0.34, 4 = 0.29, 5 = 0.44, 6 = 0.41, 7 = 0.89, 8 = 0.96, 9 = 0.61, 10 = 0.56, 11 = 0.47 and 12 = 0.54

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 461 lb uplift at joint 2 and 373 lb uplift at joint 9.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061169

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T13	COMMON	1	1	Job Reference (optional)

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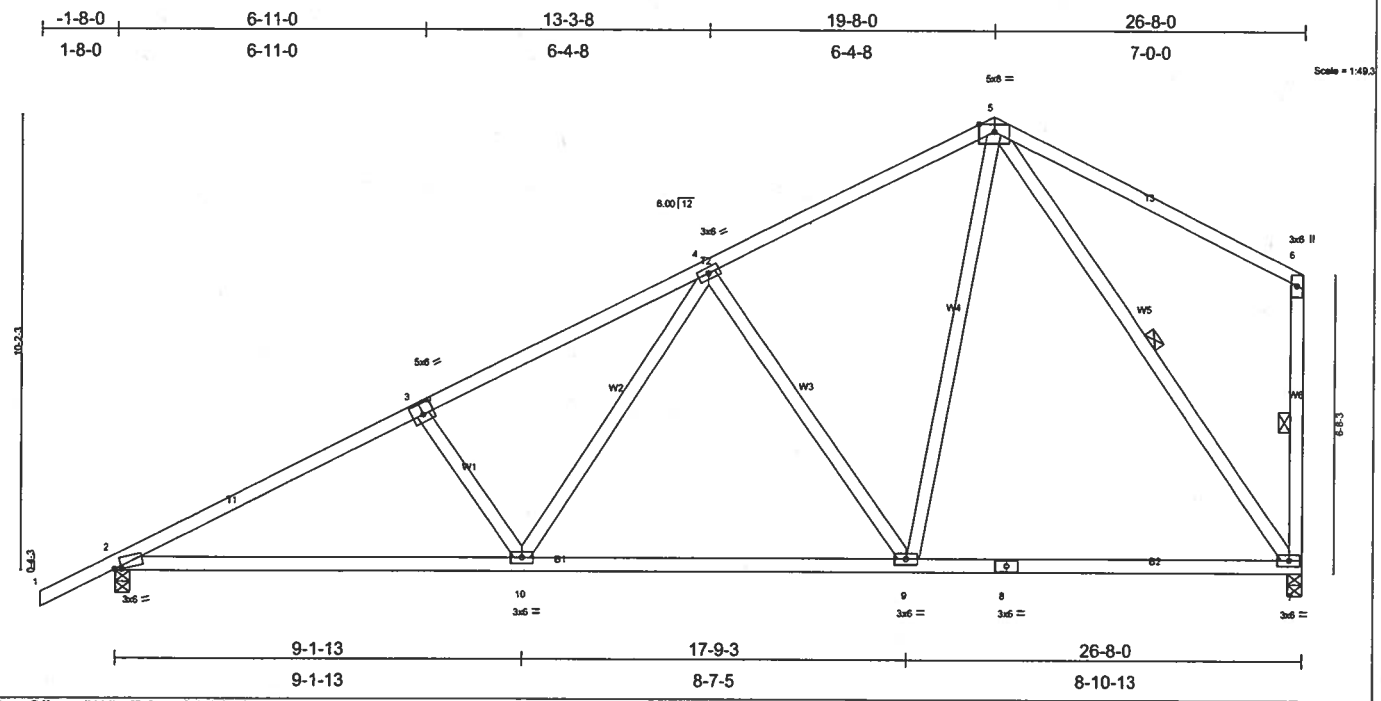


Plate Offsets (X,Y): [2:0-1-13,0-0-7], [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.60	Vert(LL)	-0.20	2-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.57	Vert(TL)	-0.34	2-10	>931	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.77	Horz(TL)	0.05	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 157 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-8-12 oc bracing.
WEBS 1 Row at midpt 6-7, 5-7

REACTIONS (lb/size) 2=1209/0-4-0, 7=1103/0-4-0
Max Horz 2=397(load case 5)
Max Uplift2=-462(load case 5), 7=-381(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-1882/736, 3-4=-1695/723, 4-5=-895/472, 5-6=-135/123, 6-7=-209/181
BOT CHORD 2-10=-882/1624, 9-10=-570/1096, 8-9=-264/587, 7-8=-264/587
WEBS 3-10=-328/321, 4-10=-255/657, 4-9=-663/443, 5-9=-311/830, 5-7=-952/431

JOINT STRESS INDEX
2 = 0.80, 3 = 0.64, 4 = 0.44, 5 = 0.57, 6 = 0.47, 7 = 0.70, 8 = 0.27, 9 = 0.59 and 10 = 0.51

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 462 lb uplift at joint 2 and 381 lb uplift at joint 7.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

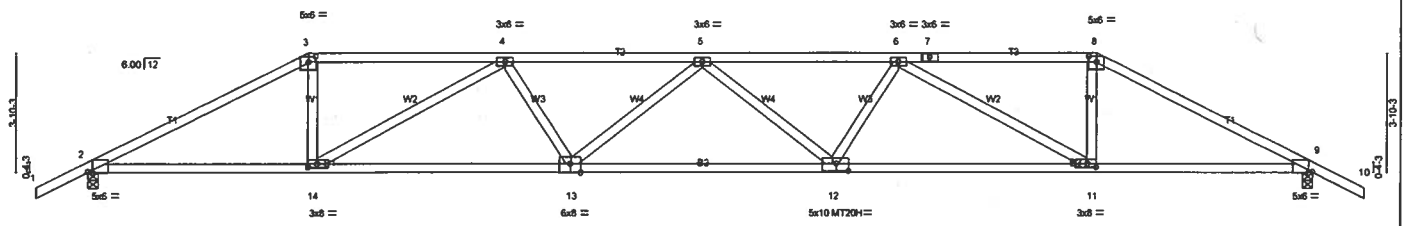
Dwg.#0130061170

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T14	HIP	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:44 2006 Page 1

-1-8-0	7-0-0	13-4-0	19-8-0	26-0-0	32-4-0	39-4-0	41-0-0
1-8-0	7-0-0	6-4-0	6-4-0	6-4-0	6-4-0	7-0-0	1-8-0

Scale = 1:70.8



7-0-0	15-5-5	23-10-11	32-4-0	39-4-0
7-0-0	8-5-5	8-5-5	8-5-5	7-0-0

Plate Offsets (X,Y): [2:0-1-11,Edge], [3:0-3-0,0-2-0], [8:0-3-0,0-2-0], [9:0-1-11,Edge], [11:0-3-8,0-1-8], [12:0-4-12,0-3-0], [13:0-4-0,Edge], [14:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.60	Vert(LL)	-0.59	12-13	>793	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.90	Vert(TL)	-0.95	12-13	>492	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.71	Horz(TL)	0.23	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 366 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-10-6 oc bracing.

REACTIONS (lb/size) 2=3512/0-4-0, 9=3512/0-4-0
Max Horz 2=-81(load case 5)
Max Uplift2=-1453(load case 4), 9=-1453(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-6991/2977, 3-4=-6248/2732, 4-5=-9708/4219, 5-6=-9707/4218, 6-7=-6248/2732, 7-8=-6248/2732, 8-9=-6990/2977, 9-10=0/39
BOT CHORD 2-14=-2616/6141, 13-14=-4042/9158, 12-13=-4480/10145, 11-12=-4004/9157, 9-11=-2573/6140
WEBS 3-14=-1017/2638, 4-14=-3444/1668, 4-13=-226/1100, 5-13=-597/472, 5-12=-598/472, 6-12=-226/1100, 6-11=-3443/1668, 8-11=-1017/2638

JOINT STRESS INDEX
2 = 0.85, 3 = 0.72, 4 = 0.52, 5 = 0.35, 6 = 0.52, 7 = 0.50, 8 = 0.72, 9 = 0.85, 11 = 0.85, 12 = 0.95, 13 = 0.99 and 14 = 0.85

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) All plates are MT20 plates unless otherwise indicated.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1453 lb uplift at joint 2 and 1453 lb uplift at joint 9.
8) Girder carries hip end with 7-0-0 end setback.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 32-4-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-8=-117(F=-63), 8-10=-54, 2-14=-30, 11-14=-65(F=-35), 9-11=-30
Concentrated Loads (lb)
Vert: 14=-539(F) 11=-539(F)

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ. FL 33549

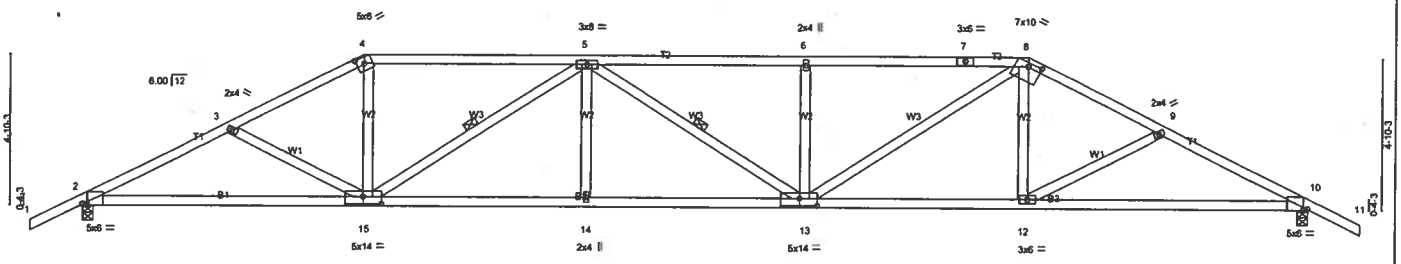
Dwg.#0130061171

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T15	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:45 2006 Page 1

-1-8-0	4-9-4	9-0-0	16-1-15	23-2-1	30-4-0	34-6-12	39-4-0	41-0-0
1-8-0	4-9-4	4-2-12	7-1-15	7-0-3	7-1-15	4-2-12	4-9-4	1-8-0

Scale = 1:70.8



9-0-0	16-1-15	23-2-1	30-4-0	39-4-0
9-0-0	7-1-15	7-0-3	7-1-15	9-0-0

Plate Offsets (X,Y): [2:0-1-11,Edge], [4:0-3-0,0-2-7], [8:0-5-0,0-1-7], [10:0-1-11,Edge], [13:0-7-0,0-3-0], [15:0-7-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.39	13-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.82	Vert(TL)	-0.63	13-14	>745	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.57	Horz(TL)	0.19	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 201 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-9-13 oc bracing.
WEBS 1 Row at midpt 5-15, 5-13

REACTIONS (lb/size) 2=1737/0-4-0, 10=1737/0-4-0
Max Horz 2=95(load case 5)
Max Uplift2=-556(load case 5), 10=-556(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-3059/1252, 3-4=-2860/1164, 4-5=-2520/1098, 5-6=-3441/1469, 6-7=-3441/1469, 7-8=-3442/1469, 8-9=-2851/1162, 9-10=-3058/1254, 10-11=0/39
BOT CHORD 2-15=-965/2673, 14-15=-1174/3443, 13-14=-1174/3443, 12-13=-801/2521, 10-12=-966/2673
WEBS 3-15=-177/188, 4-15=-269/927, 5-15=-1174/471, 5-14=0/203, 5-13=-71/66, 6-13=-391/275, 8-13=-471/1169, 8-12=-34/343, 9-12=-186/190

JOINT STRESS INDEX
2 = 0.76, 3 = 0.34, 4 = 0.71, 5 = 0.57, 6 = 0.34, 7 = 0.57, 8 = 0.90, 9 = 0.34, 10 = 0.76, 12 = 0.35, 13 = 0.75, 14 = 0.34 and 15 = 0.75

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 556 lb uplift at joint 2 and 556 lb uplift at joint 10.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

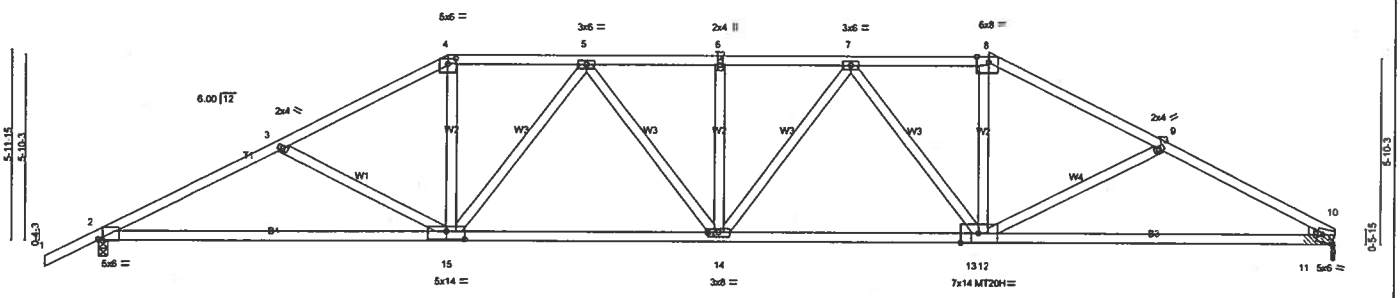
Dwg.#0130061172

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T16	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:47 2006 Page 1

-1-8-0	5-9-4	11-0-0	15-4-0	19-6-4	23-8-8	28-0-8	33-5-0	39-0-8
1-8-0	5-9-4	5-2-12	4-4-0	4-2-4	4-2-4	4-4-0	5-4-8	5-7-8

Scale = 1:69.3



11-1-12	19-6-4	27-10-12	39-0-8
11-1-12	8-4-8	8-4-8	11-1-12

Plate Offsets (X,Y): [2:0-1-10,Edge], [4:0-3-0,0-2-0], [8:0-4-8,Edge], [10:0-0-12,0-2-8], [13:0-6-12,Edge], [15:0-7-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.42	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.95	Vert(TL)	-0.71	10-12	>654	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.58	Horz(TL)	0.16	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 206 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3
WEDGE
Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

REACTIONS (lb/size) 10=1629/0-1-15 (0-1-8 + bearing block), 2=1731/0-3-8
Max Horz 2=129(load case 5)
Max Uplift10=-469(load case 6), 2=-574(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-3016/1279, 3-4=-2706/1130, 4-5=-2357/1070, 5-6=-2874/1286, 6-7=-2874/1286, 7-8=-2364/1080, 8-9=-2674/1132, 9-10=-2952/1280
BOT CHORD 2-15=-1047/2645, 14-15=-951/2702, 13-14=-960/2713, 12-13=-960/2713, 11-12=-1046/2574, 10-11=-1046/2574
WEBS 4-15=-285/914, 8-12=-295/914, 6-14=-229/171, 3-15=-330/294, 5-15=-658/316, 5-14=-106/355, 7-14=-103/340, 7-12=-679/322, 9-12=-272/290

JOINT STRESS INDEX
2 = 0.80, 3 = 0.34, 4 = 0.68, 5 = 0.41, 6 = 0.34, 7 = 0.41, 8 = 0.74, 9 = 0.34, 10 = 0.90, 10 = 0.00, 11 = 0.00, 11 = 0.00, 12 = 0.00, 13 = 0.80, 14 = 0.58 and 15 = 0.59

NOTES
1) 2 X 4 SYP No.1D bearing block 12" long at jt. 10 attached to front face with 2 rows of 0.131"x3" Nails spaced 3" o.c. 8 Total fasteners. Bearing is assumed to be SYP.
2) Unbalanced roof live loads have been considered for this design.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
4) Provide adequate drainage to prevent water ponding.
5) All plates are MT20 plates unless otherwise indicated.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 469 lb uplift at joint 10 and 574 lb uplift at joint 2.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B LUTZ, FL 33549

Dwg.#0130061173

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T17	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:48 2006 Page 1

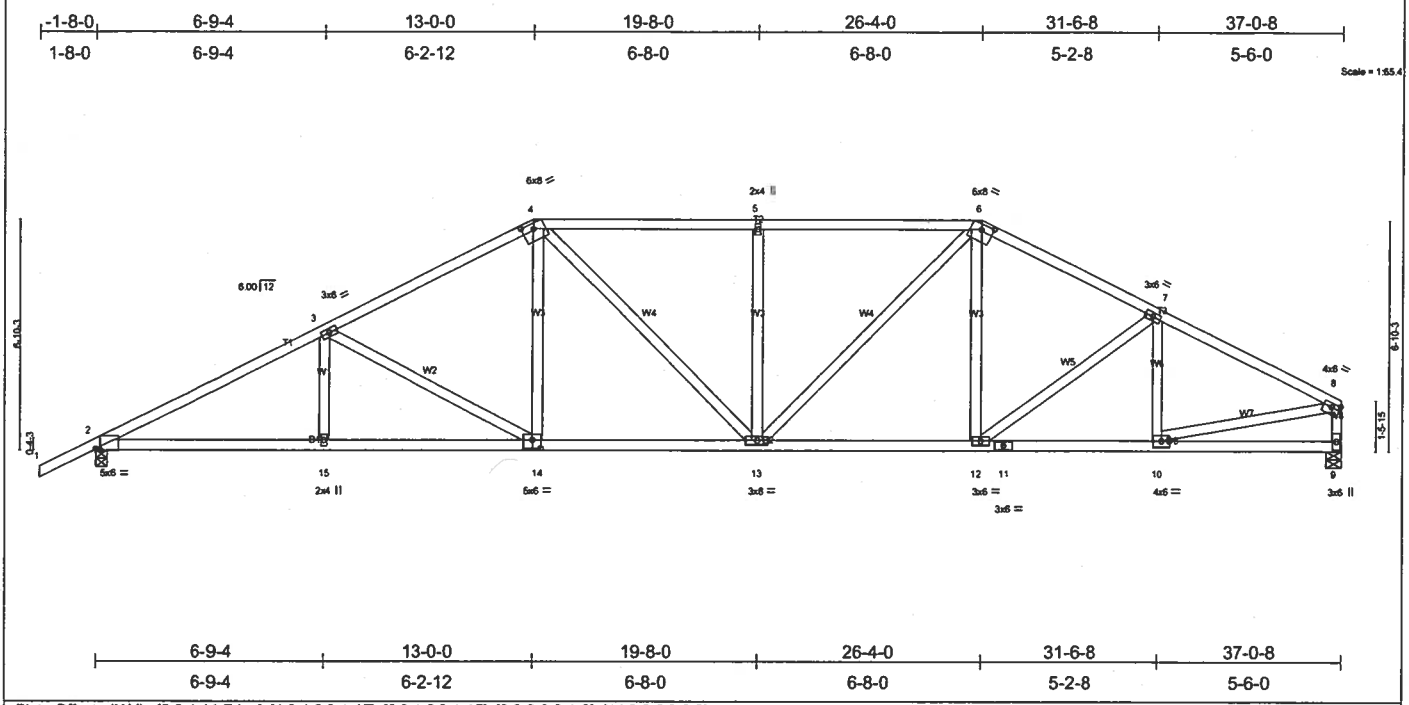


Plate Offsets (X,Y): [2:0-1-11,Edge], [4:0-4-0,0-1-15], [6:0-4-0,0-1-15], [8:0-3-0,0-1-8], [14:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.20	13-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.64	Vert(TL)	-0.32	13-14	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.53	Horz(TL)	0.11	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 211 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-3-10 oc bracing.

REACTIONS (lb/size) 2=1644/0-4-0, 9=1540/0-5-11
Max Horz 2=183(load case 5)
Max Uplift2=-571(load case 5), 9=-444(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-2896/1168, 3-4=-2305/1019, 4-5=-2193/1049, 5-6=-2193/1049, 6-7=-2078/943, 7-8=-2108/888, 8-9=-1447/649
BOT CHORD 2-15=-1003/2503, 14-15=-1003/2503, 13-14=-712/2003, 12-13=-633/1811, 11-12=-717/1824, 10-11=-717/1824, 9-10=-108/206
WEBS 3-15=0/213, 3-14=-583/333, 4-14=-122/467, 4-13=-192/408, 5-13=-378/270, 6-13=-240/633, 6-12=-49/217, 7-12=-85/138, 7-10=-242/186, 8-10=-626/1661

JOINT STRESS INDEX
2 = 0.73, 3 = 0.41, 4 = 0.62, 5 = 0.34, 6 = 0.57, 7 = 0.41, 8 = 0.73, 9 = 0.41, 10 = 0.76, 11 = 0.66, 12 = 0.35, 13 = 0.63, 14 = 0.70 and 15 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 571 lb uplift at joint 2 and 444 lb uplift at joint 9.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

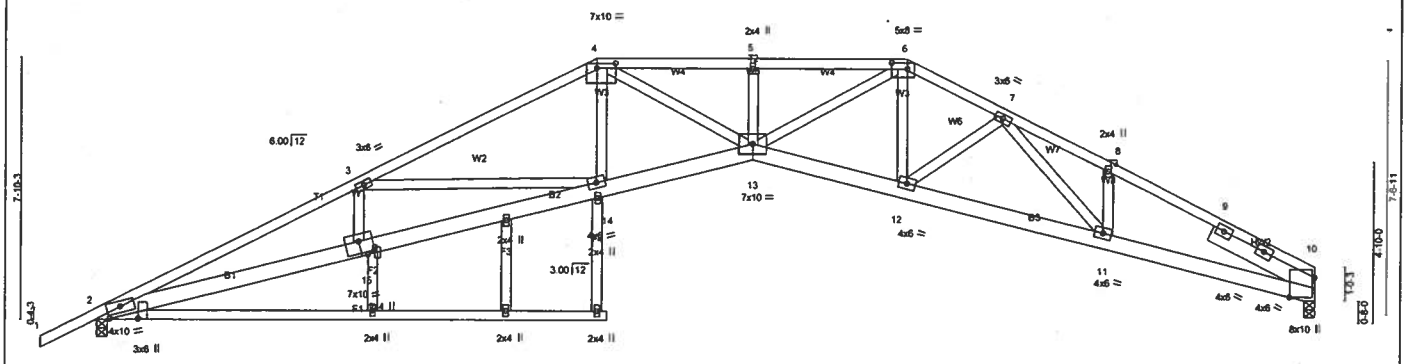
Dwg.#0130061174

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T18	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:49 2006 Page 1

-1-8-0	7-10-11	15-0-0	19-8-0	24-4-0	30-4-4	36-8-0
1-8-0	7-10-11	7-1-5	4-8-0	4-8-0	6-0-4	6-3-12

Scale = 1:66.0



7-10-11	15-0-0	15-4-0	19-8-0	24-4-0	30-4-4	36-8-0
7-10-11	7-1-5	0-4-0	4-4-0	4-8-0	6-0-4	6-3-12

Plate Offsets (X,Y): [4:0-6-8,0-1-12], [6:0-5-12,0-2-4], [10:0-7-0,Edge], [15:0-5-0,0-4-8], [16:0-0-0,0-10-1], [20:0-2-11,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.92	Vert(LL)	-0.63	13-14	>695	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.74	Vert(TL)	-1.01	13-14	>434	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.99	Horz(TL)	0.64	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 249 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except* T3 2 X 4 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D *Except* F1 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 6 SYP No.1D 3-7-0

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 7-4-14 oc bracing. Except: 1 Row at midpt 2-15, 14-15
JOINTS 1 Brace at Jt(s): 15

REACTIONS (lb/size) 10=1526/0-4-0, 2=1630/0-4-0
Max Horz 2=180(load case 5)
Max Uplift10=-453(load case 6), 2=-583(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/41, 2-3=-5309/2126, 3-4=-4126/1632, 4-5=-5124/2007, 5-6=-5124/2007, 6-7=-3768/1533, 7-8=-3576/1606, 8-9=-3732/1545, 9-10=-3819/1533
BOT CHORD 2-15=-1873/4812, 14-15=-1877/4830, 13-14=-1225/3736, 12-13=-1091/3445, 11-12=-1251/3576, 10-11=-1237/3293
WEBS 3-15=0/190, 3-14=-1061/626, 4-14=-193/608, 4-13=-534/1709, 5-13=-219/179, 6-13=-682/2031, 6-12=-177/455, 7-12=-156/202, 8-11=-21/267, 7-11=-457/161

JOINT STRESS INDEX
2 = 0.91, 3 = 0.41, 4 = 1.00, 5 = 0.34, 6 = 0.92, 7 = 0.41, 8 = 0.34, 9 = 0.00, 10 = 0.96, 10 = 0.69, 10 = 0.69, 11 = 0.27, 12 = 0.27, 13 = 0.85, 14 = 0.33, 14 = 0.34, 15 = 0.80, 16 = 0.16, 18 = 0.34, 19 = 0.34, 20 = 0.76, 21 = 0.34 and 22 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 10, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 453 lb uplift at joint 10 and 583 lb uplift at joint 2.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B LUTZ, FL 33549

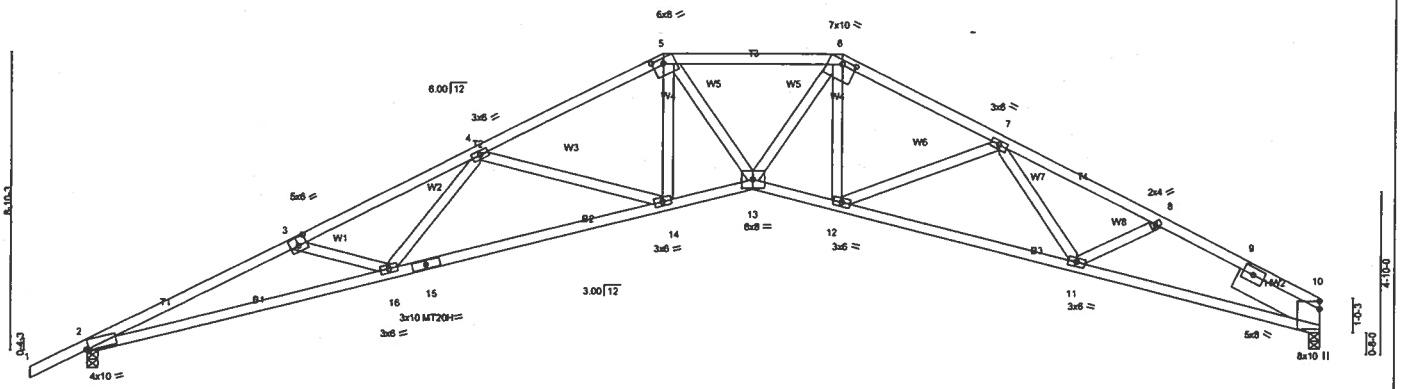
Dwg.#0130061175

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T19	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:50 2006 Page 1

-1-8-0	6-2-5	11-7-2	17-0-0	22-4-0	27-0-3	31-8-5	36-8-0
1-8-0	6-2-5	5-4-14	5-4-14	5-4-0	4-8-3	4-8-3	4-11-11

Scale = 1:65.4



8-10-11	17-0-0	19-8-0	22-4-0	29-4-4	36-8-0
8-10-11	8-1-5	2-8-0	2-8-0	7-0-4	7-3-12

Plate Offsets (X,Y): [2:0-0-14,Edge], [3:0-3-0,0-3-0], [5:0-4-0,0-1-15], [6:0-5-0,0-1-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.97	Vert(LL)	-0.66	14-16	>667	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.87	Vert(TL)	-1.06	14-16	>414	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.50	Horz(TL)	0.68	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 190 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T4 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.1D *Except*
B2 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 8 SYP No.1D 2-10-12

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 4-9-8 oc bracing.

REACTIONS (lb/size) 10=1528/0-4-0, 2=1632/0-4-0
Max Horz 2=190(load case 5)
Max Uplift10=-468(load case 6), 2=-596(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/38, 2-3=-5210/2188, 3-4=-4907/1993, 4-5=-3641/1473, 5-6=-3682/1520, 6-7=-3493/1424, 7-8=-3723/1555, 8-9=-3576/1545, 9-10=-3661/1537
BOT CHORD 2-16=-1936/4735, 15-16=-1528/4066, 14-15=-1526/4120, 13-14=-1012/3308, 12-13=-951/3162, 11-12=-1262/3530, 10-11=-1226/3103
WEBS 3-16=-259/312, 4-16=-175/632, 4-14=-809/526, 5-14=-243/658, 5-13=-235/792, 6-13=-335/1034, 6-12=-178/474, 7-12=-399/327, 7-11=-133/102, 8-11=-32/400

JOINT STRESS INDEX
2 = 0.81, 3 = 0.61, 4 = 0.42, 5 = 0.79, 6 = 0.92, 7 = 0.41, 8 = 0.34, 9 = 0.00, 10 = 0.90, 10 = 1.00, 11 = 0.39, 12 = 0.38, 13 = 0.78, 14 = 0.47, 15 = 0.88 and 16 = 0.41

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Bearing at joint(s) 10, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 468 lb uplift at joint 10 and 596 lb uplift at joint 2.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

Dwg.#0130061176

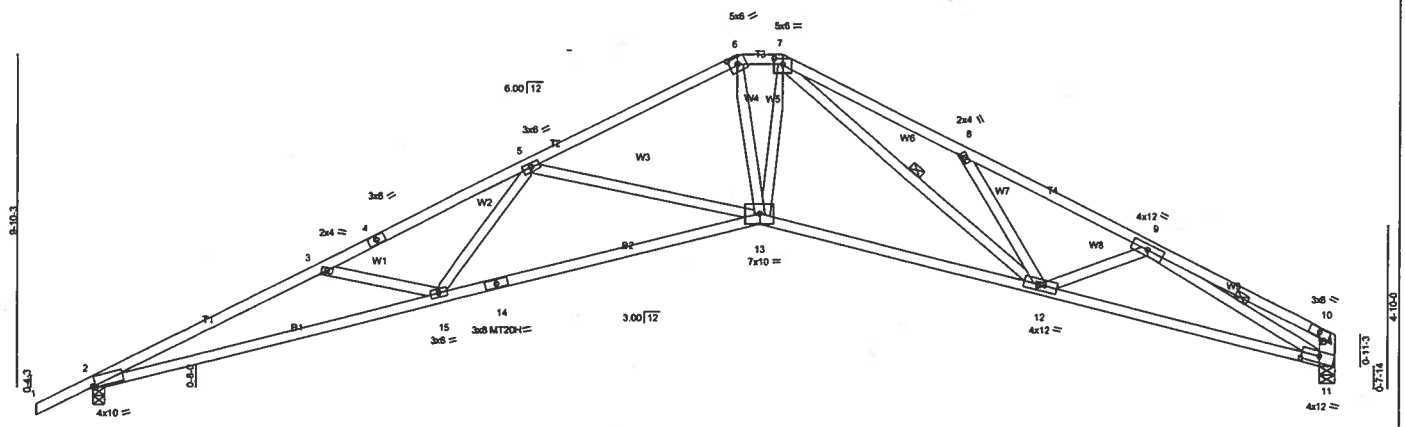
Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T20	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:51 2006 Page 1

-1-8-0 | 6-10-5 | 12-11-2 | 19-0-0 | 20-4-0 | 25-9-4 | 31-2-9 | 36-10-0

1-8-0 | 6-10-5 | 6-0-14 | 6-0-14 | 1-4-0 | 5-5-4 | 5-5-4 | 5-7-7

Scale = 1:65.2



10-2-11 | 19-8-0 | 28-0-3 | 36-4-5 | 36-10-0

10-2-11 | 9-5-5 | 8-4-3 | 8-4-3 | 0-5-11

Plate Offsets (X,Y): [2:0-0-14,Edge], [6:0-3-0,0-2-7], [7:0-3-0,0-2-0], [11:0-6-12,0-2-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.79	Vert(LL)	-0.66	13-15	>660	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.94	Vert(TL)	-1.07	13-15	>408	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.89	Horz(TL)	0.67	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 193 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 6 SYP No.1D, B1 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 7-12, 9-11

REACTIONS (lb/size) 11=1528/0-5-11, 2=1632/0-4-0
Max Horz 2=230(load case 5)
Max Uplift11=-482(load case 6), 2=-606(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/38, 2-3=-5170/2258, 3-4=-4757/1957, 4-5=-4681/1974, 5-6=-3244/1354, 6-7=-3005/1337, 7-8=-4154/2089, 8-9=-3861/1643, 9-10=-809/352
BOT CHORD 2-15=-2038/4709, 14-15=-1528/3891, 13-14=-1525/3953, 12-13=-921/2989, 11-12=-1423/3380, 10-11=-508/278
WEBS 3-15=-375/405, 5-15=-186/710, 5-13=-1009/633, 6-13=-450/1227, 7-13=-294/1038, 7-12=-831/1157, 8-12=-507/505, 9-12=0/256, 9-11=-3102/1366

JOINT STRESS INDEX
2 = 0.80, 3 = 0.34, 4 = 0.75, 5 = 0.48, 6 = 0.68, 7 = 0.80, 8 = 0.34, 9 = 0.56, 10 = 0.74, 11 = 0.65, 12 = 0.72, 13 = 0.75, 14 = 0.93 and 15 = 0.46

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Bearing at joint(s) 11, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 482 lb uplift at joint 11 and 606 lb uplift at joint 2.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

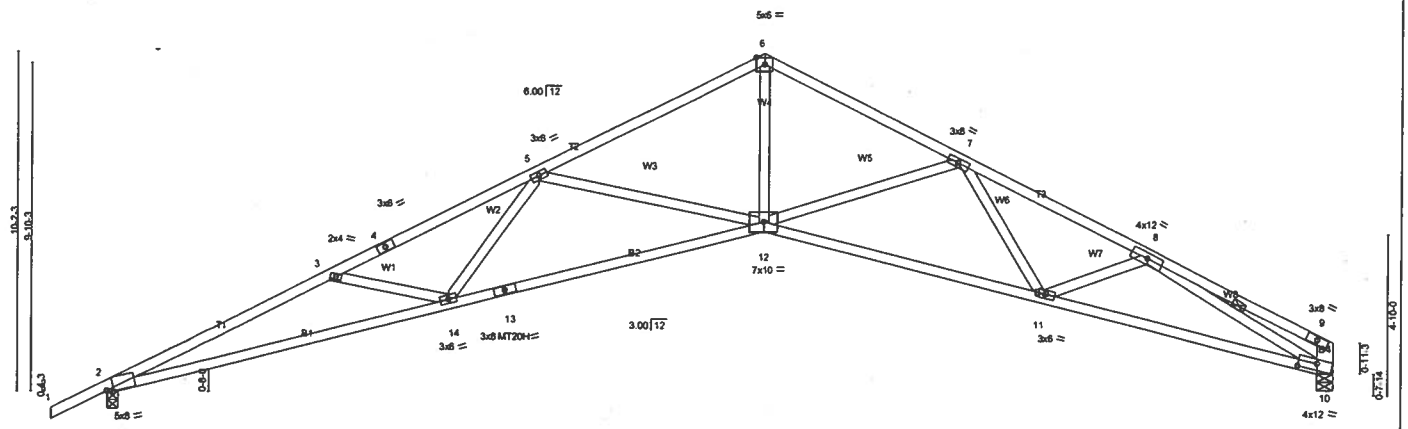
Dwg.#0130061177

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T21	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:52 2006 Page 1

-1-8-0	6-10-5	12-11-2	19-8-0	25-9-4	31-2-9	36-10-0
1-8-0	6-10-5	6-0-14	6-8-14	6-1-5	5-5-4	5-7-7

Scale = 1:66.1



10-2-11	19-8-0	28-0-3	36-4-5	36-10-0
10-2-11	9-5-5	8-4-3	8-4-3	0-5-11

Plate Offsets (X,Y): [2:0-2-7,Edge], [10:0-6-12,0-2-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.79	Vert(LL)	-0.68	12-14	>641	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.93	Vert(TL)	-1.10	12-14	>396	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.91	Horz(TL)	0.69	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 181 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 6 SYP No.1D, B1 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 8-10

REACTIONS (lb/size) 10=1528/0-5-11, 2=1632/0-4-0
Max Horz 2=234(load case 5)
Max Uplift10=-486(load case 6), 2=-609(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/38, 2-3=-5169/2265, 3-4=-4760/1969, 4-5=-4685/1986, 5-6=-3238/1359, 6-7=-3226/1364, 7-8=-3868/1656, 8-9=-809/353
BOT CHORD 2-14=-2045/4708, 13-14=-1540/3895, 12-13=-1538/3956, 11-12=-1300/3455, 10-11=-1428/3378, 9-10=-510/281
WEBS 3-14=-366/400, 5-14=-184/706, 5-12=-1040/647, 7-11=-54/245, 8-11=0/246, 8-10=-3101/1371, 6-12=-930/2468, 7-12=-610/418

JOINT STRESS INDEX
2 = 1.00, 3 = 0.34, 4 = 0.75, 5 = 0.47, 6 = 0.86, 7 = 0.41, 8 = 0.56, 9 = 0.75, 10 = 0.65, 11 = 0.47, 12 = 0.75, 13 = 0.93 and 14 = 0.46

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All plates are MT20 plates unless otherwise indicated.
4) Bearing at joint(s) 10, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 486 lb uplift at joint 10 and 609 lb uplift at joint 2.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

Dwg.#0130061178

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T22	SPECIAL	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:54 2006 Page 1

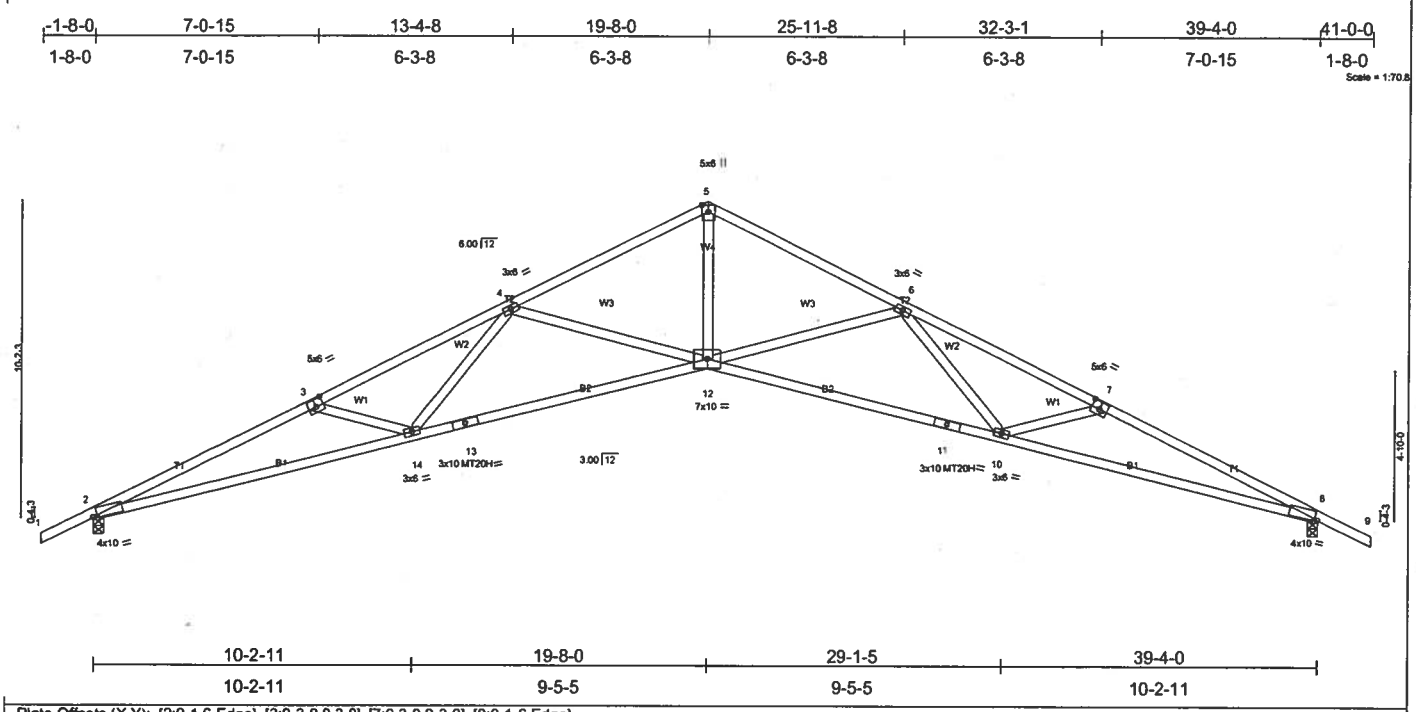


Plate Offsets (X,Y): [2:0-1-6,Edge], [3:0-3-0,0-3-0], [7:0-3-0,0-3-0], [8:0-1-6,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.74	Vert(LL)	-0.87	10-12	>541	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.99	Vert(TL)	-1.40	10-12	>335	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.92	Horz(TL)	0.94	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 183 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 4 SYP No.1D, T1 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D, B1 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

REACTIONS (lb/size) 2=1737/0-4-0, 8=1737/0-4-0
Max Horz 2=169(load case 5)
Max Uplift2=-637(load case 5), 8=-637(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/38, 2-3=-5590/2296, 3-4=-5198/2042, 4-5=-3686/1406, 5-6=-3686/1406, 6-7=-5198/2042, 7-8=-5590/2296, 8-9=0/38
BOT CHORD 2-14=-1931/5092, 13-14=-1398/4226, 12-13=-1395/4288, 11-12=-1395/4288, 10-11=-1398/4226, 8-10=-1931/5092
WEBS 3-14=-359/396, 4-14=-240/778, 4-12=-969/613, 5-12=-976/2879, 6-12=-969/613, 6-10=-240/778, 7-10=-359/396

JOINT STRESS INDEX
2 = 0.86, 3 = 0.72, 4 = 0.52, 5 = 0.71, 6 = 0.52, 7 = 0.72, 8 = 0.86, 10 = 0.50, 11 = 0.89, 12 = 0.81, 13 = 0.89 and 14 = 0.50

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All plates are MT20 plates unless otherwise indicated.
4) Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 637 lb uplift at joint 2 and 637 lb uplift at joint 8.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061179

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T23	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:55 2006 Page 1

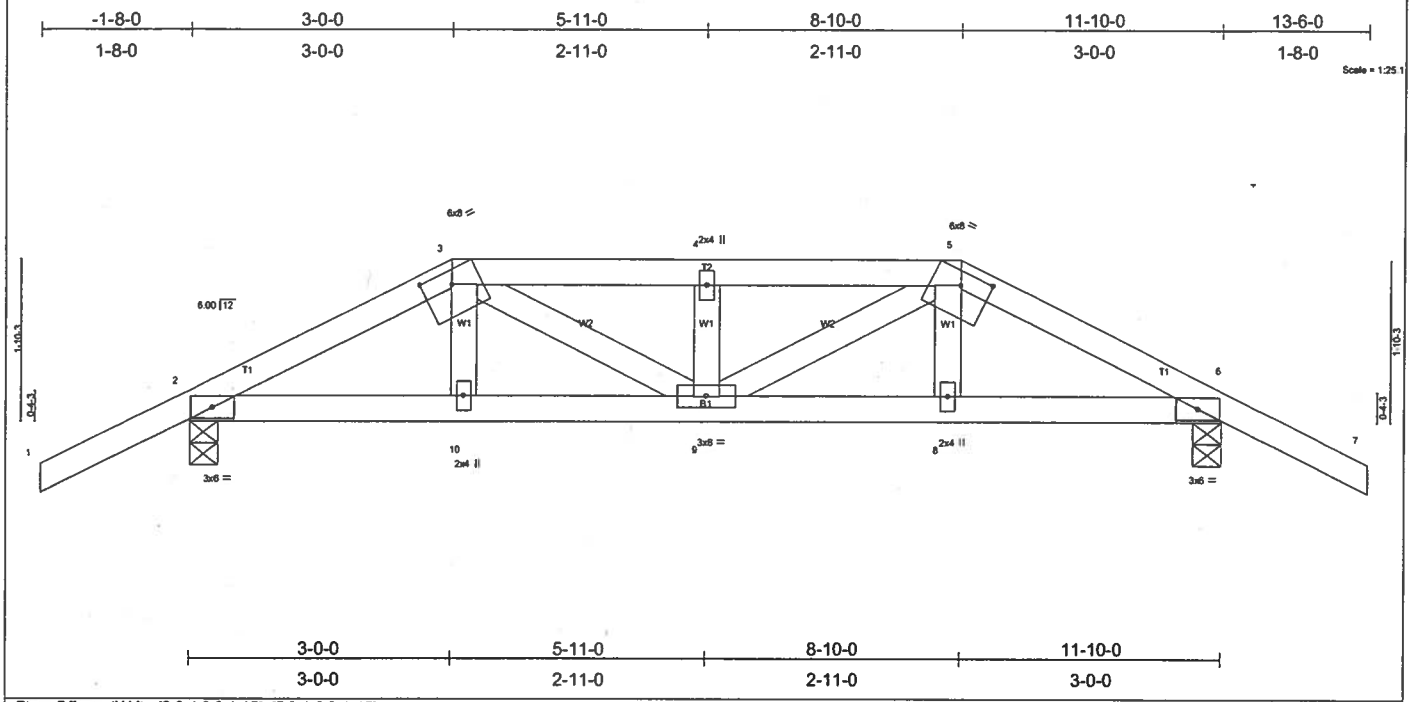


Plate Offsets (X,Y): [3:0-4-0,0-1-15], [5:0-4-0,0-1-15]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.20	Vert(LL)	-0.03	9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.21	Vert(TL)	-0.05	9	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.10	Horz(TL)	0.02	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 56 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=685/0-4-0, 6=685/0-4-0
Max Horz 2=-53(load case 5)
Max Uplift2=-298(load case 4), 6=-298(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-967/322, 3-4=-1094/399, 4-5=-1094/399, 5-6=-967/322, 6-7=0/39
BOT CHORD 2-10=-252/814, 9-10=-253/827, 8-9=-239/827, 6-8=-238/814
WEBS 3-10=-11/152, 3-9=-157/321, 4-9=-185/143, 5-9=-158/321, 5-8=-10/152

JOINT STRESS INDEX
2 = 0.48, 3 = 0.25, 4 = 0.08, 5 = 0.25, 6 = 0.48, 8 = 0.11, 9 = 0.30 and 10 = 0.11

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 298 lb uplift at joint 2 and 298 lb uplift at joint 6.
5) Girder carries hip end with 3-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 63 lb down and 32 lb up at 8-10-0, and 63 lb down and 32 lb up at 3-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

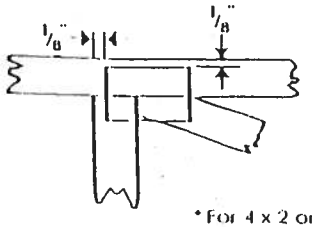
LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-63(F=-9), 5-7=-54, 2-10=-30, 8-10=-35(F=-5), 6-8=-30
Concentrated Loads (lb)
Vert: 10=-63(F) 8=-63(F)

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196

Symbols

PLATE LOCATION AND ORIENTATION

* Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seal.



* For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

* This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

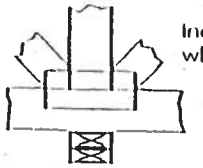
4 X 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

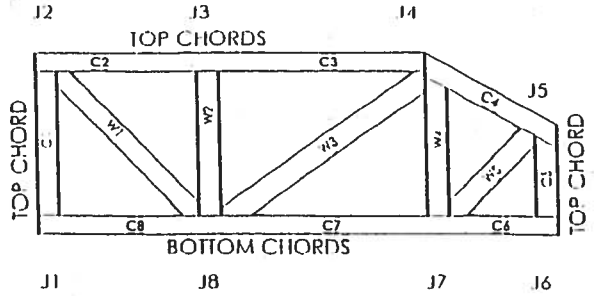
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-H
HER	561

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

Project Information for:	L148527		
Builder:	Greg Talley	Date:	1/30/2006
Lot:	Lot 33 Cannon Creek Place	Start Number:	**1149**
Subdivision:	n/a		
County or City:	Columbia		
Truss Page Count:	31		

Truss Design Load Information (UNO) Design Program: MiTek 5.2 / 6.2

Gravity		Wind		Building Code:	FBC2004
Roof (psf):	**42**	Wind Standard:	**ASCE 7-02**		
Floor (psf):	55	Wind Speed (mph):	**110**		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

ROHNER, GEORGE JOSEPH CGC021619
Address: 3031 SW 108 WAY
OCALA, FL. 34478 Designer: **111**

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	0130061149	1/30/2006				
2	CJ3	0130061150	1/30/2006				
3	CJ5	0130061151	1/30/2006				
4	EJ3	0130061152	1/30/2006				
5	EJ7	0130061153	1/30/2006				
6	HJ3	0130061154	1/30/2006				
7	HJ7	0130061155	1/30/2006				
8	HJ7A	0130061156	1/30/2006				
9	T01	0130061157	1/30/2006				
10	T02	0130061158	1/30/2006				
11	T03	0130061159	1/30/2006				
12	T04	0130061160	1/30/2006				
13	T05	0130061161	1/30/2006				
14	T06	0130061162	1/30/2006				
15	T07	0130061163	1/30/2006				
16	T08	0130061164	1/30/2006				
17	T09	0130061165	1/30/2006				
18	T10	0130061166	1/30/2006				
19	T11	0130061167	1/30/2006				
20	T12	0130061168	1/30/2006				
21	T13	0130061169	1/30/2006				
22	T14	0130061170	1/30/2006				
23	T15	0130061171	1/30/2006				
24	T16	0130061172	1/30/2006				
25	T17	0130061173	1/30/2006				
26	T18	0130061174	1/30/2006				
27	T19	0130061175	1/30/2006				
28	T20	0130061176	1/30/2006				
29	T21	0130061177	1/30/2006				
30	T22	0130061178	1/30/2006				
31	T23	0130061179	1/30/2006				

JAN 30 2006

Licensee Details

Licensee Information

Name:	**ROHNER, GEORGE JOSEPH (Primary Name)** **R B K BUILDERS INC (DBA Name)**
Main Address:	**3031 SW 108 WAY** **PO BOX 290023** **DAVIE Florida 33329-0023**
County:	**BROWARD**
License Mailing:	
LicenseLocation:	**3031 SW 108 WAY** **PO BOX 290023** **DAVIE FL 33329-0023**
County:	**BROWARD**

License Information

License Type:	**Certified General Contractor**
Rank:	**Cert General**
License Number:	**CGC021619**
Status:	**Current,Active**
Licensure Date:	**03/14/1982**
Expires:	**08/31/2006**
Special Qualifications	**Qualification Effective**
Bldg Code Core Course Credit	
Qualified Business License Required	**02/20/2004**

View Related License Information

View License Complaint

Dwg.#0130061149

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	CJ1	MONO TRUSS	14	1	Job Reference (optional)

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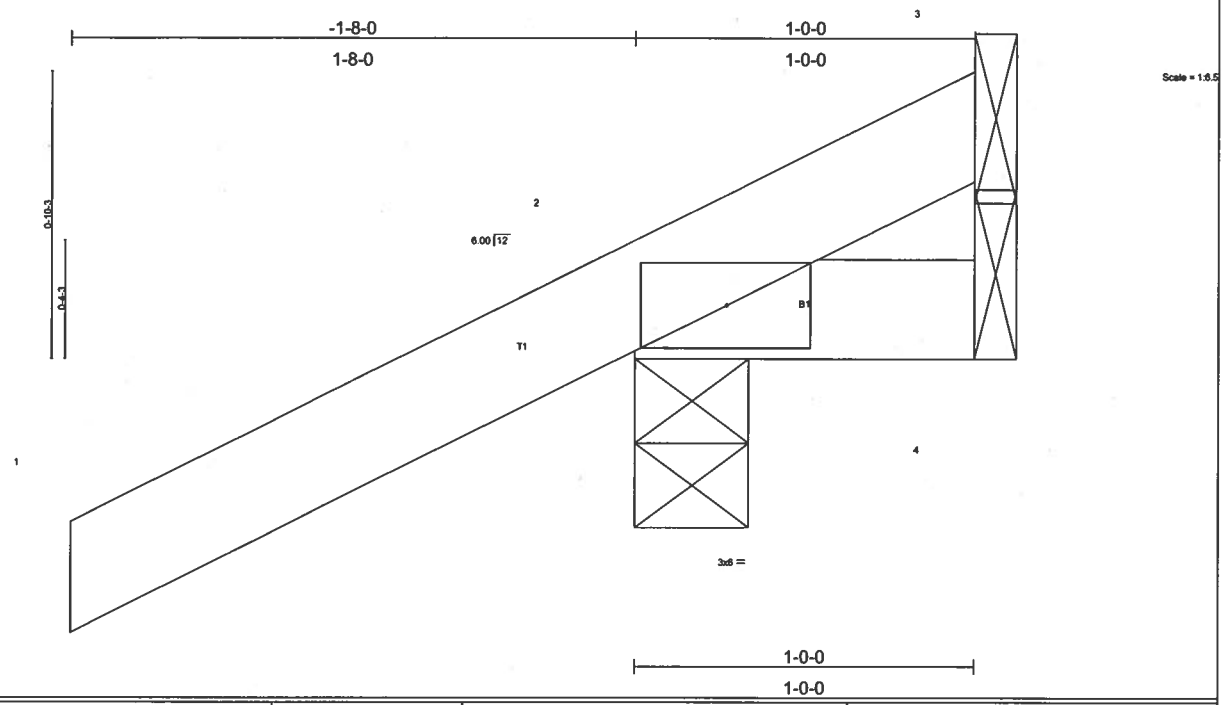


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.19	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 6 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=214/0-4-0, 4=14/Mechanical, 3=-56/Mechanical
Max Horz 2=76(load case 5)
Max Uplift2=-222(load case 5), 4=-9(load case 3), 3=-56(load case 1)
Max Grav 2=214(load case 1), 4=14(load case 1), 3=81(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-53/47
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.11

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 222 lb uplift at joint 2, 9 lb uplift at joint 4 and 56 lb uplift at joint 3.

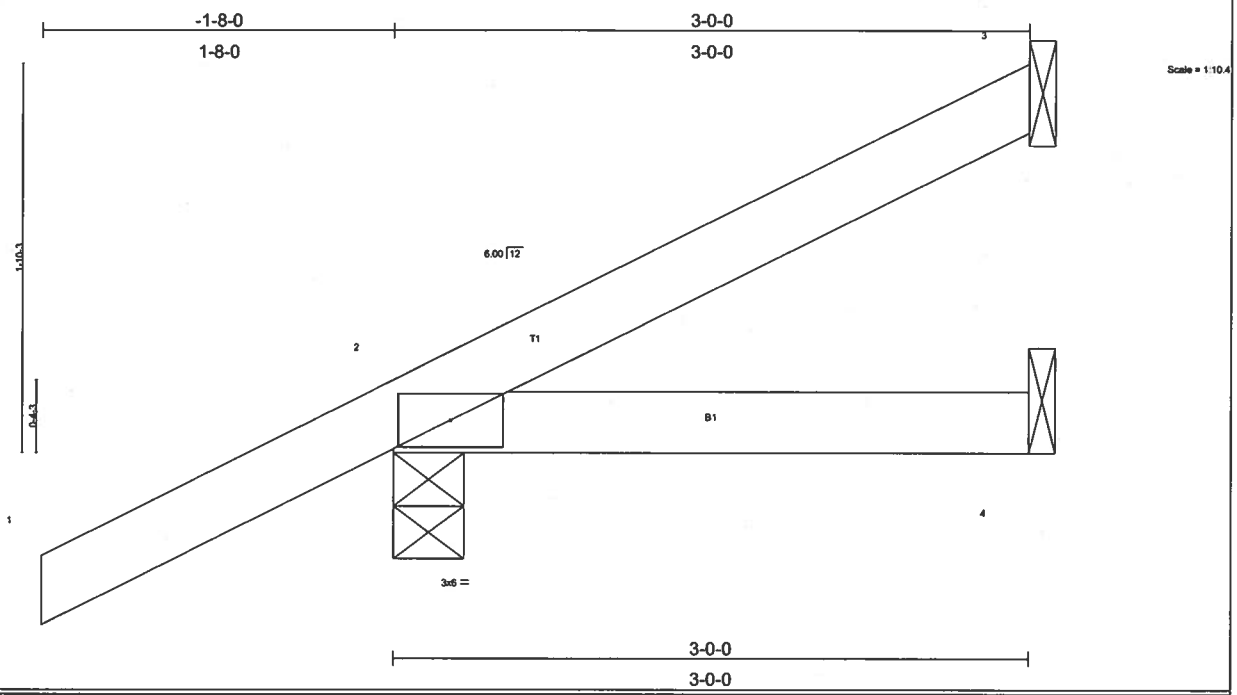
LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061150

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	CJ3	MONO TRUSS	10	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:27 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.00	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.05	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=42/Mechanical, 2=248/0-4-0, 4=42/Mechanical
Max Horz 2=121(load case 5)
Max Uplift3=-33(load case 6), 2=-169(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-51/13
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.11

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 33 lb uplift at joint 3 and 169 lb uplift at joint 2.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061151

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	CJ5	MONO TRUSS	10	1	Job Reference (optional)

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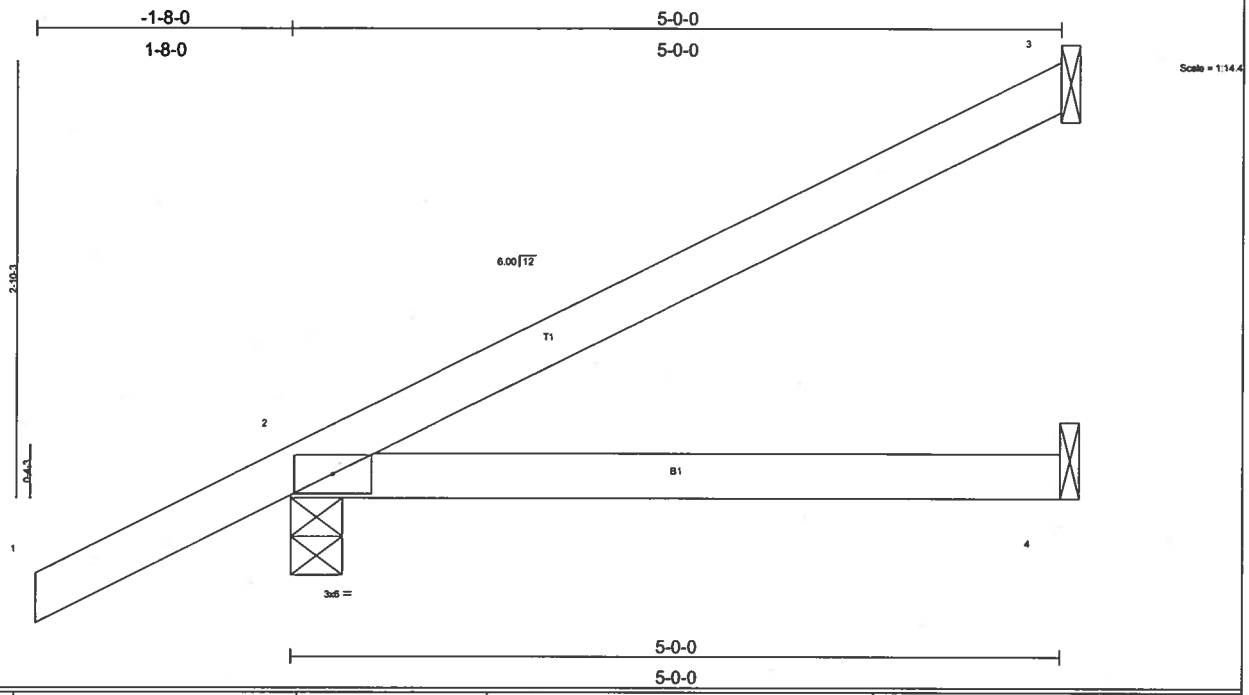


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	-0.03	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=110/Mechanical, 2=318/0-4-0, 4=72/Mechanical
Max Horz 2=167(load case 5)
Max Uplift3=-96(load case 5), 2=-172(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-93/39
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.14

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 96 lb uplift at joint 3 and 172 lb uplift at joint 2.

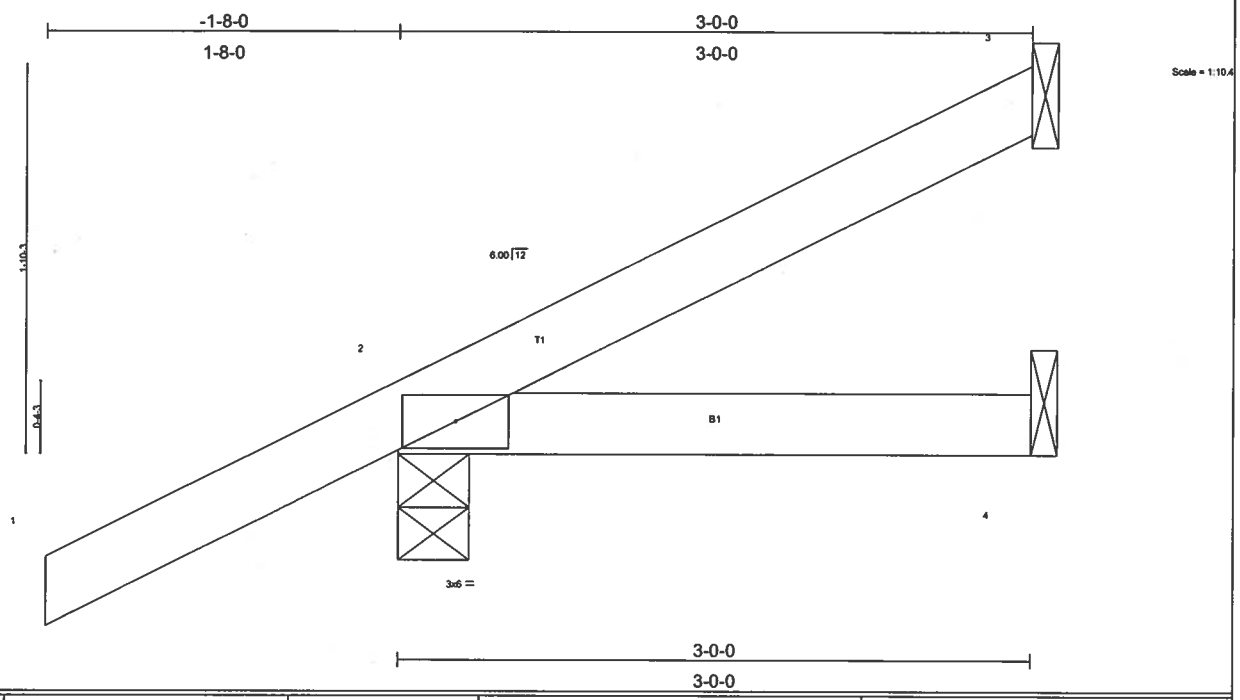
LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061152

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	EJ3	MONO TRUSS	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:28 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=42/Mechanical, 2=248/0-4-0, 4=42/Mechanical
Max Horz 2=121(load case 5)
Max Uplift3=-33(load case 6), 2=-204(load case 5), 4=-26(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-51/13
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.11

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 33 lb uplift at joint 3, 204 lb uplift at joint 2 and 26 lb uplift at joint 4.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061153

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	EJ7	MONO TRUSS	29	1	Job Reference (optional)

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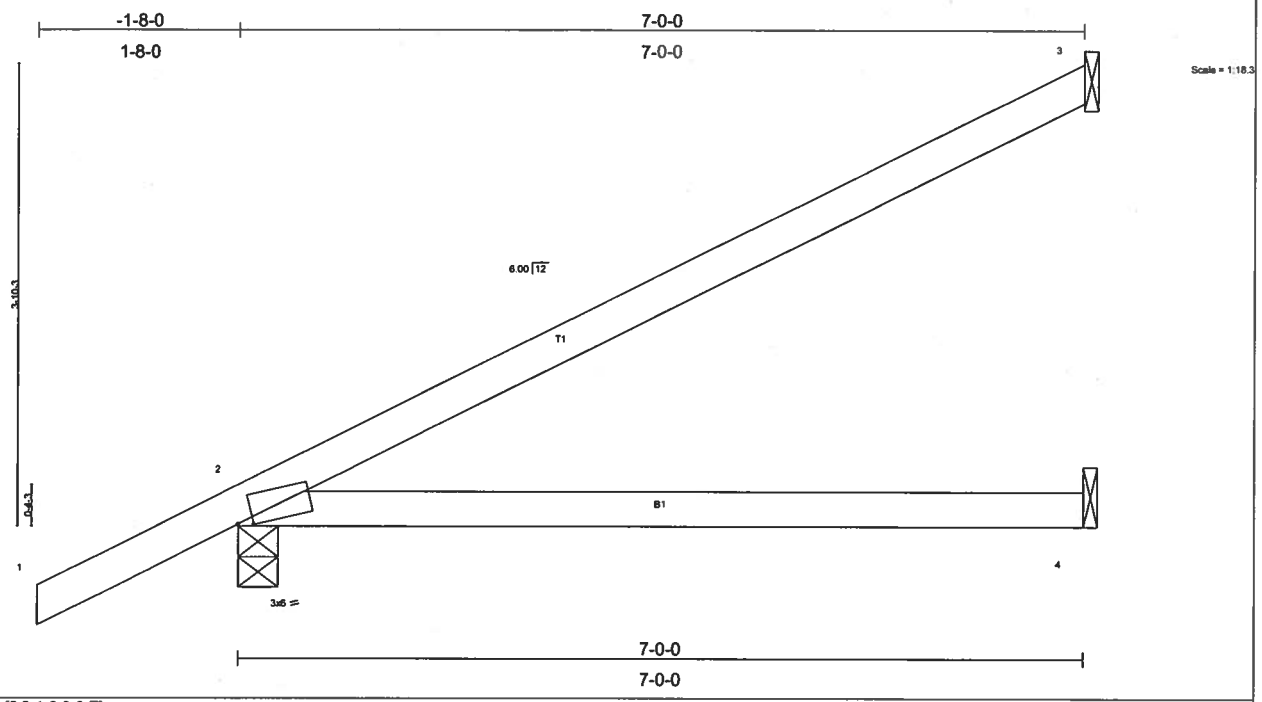

Plate Offsets (X,Y): [2:0-1-9,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.13	2-4	>631	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.36	Vert(TL)	-0.21	2-4	>379	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 25 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=164/Mechanical, 2=397/0-4-0, 4=107/Mechanical
Max Horz 2=213(load case 5)
Max Uplift3=-137(load case 5), 2=-185(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-121/59
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.85

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 137 lb uplift at joint 3 and 185 lb uplift at joint 2.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061154

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	HJ3	MONO TRUSS	2	1	Job Reference (optional)

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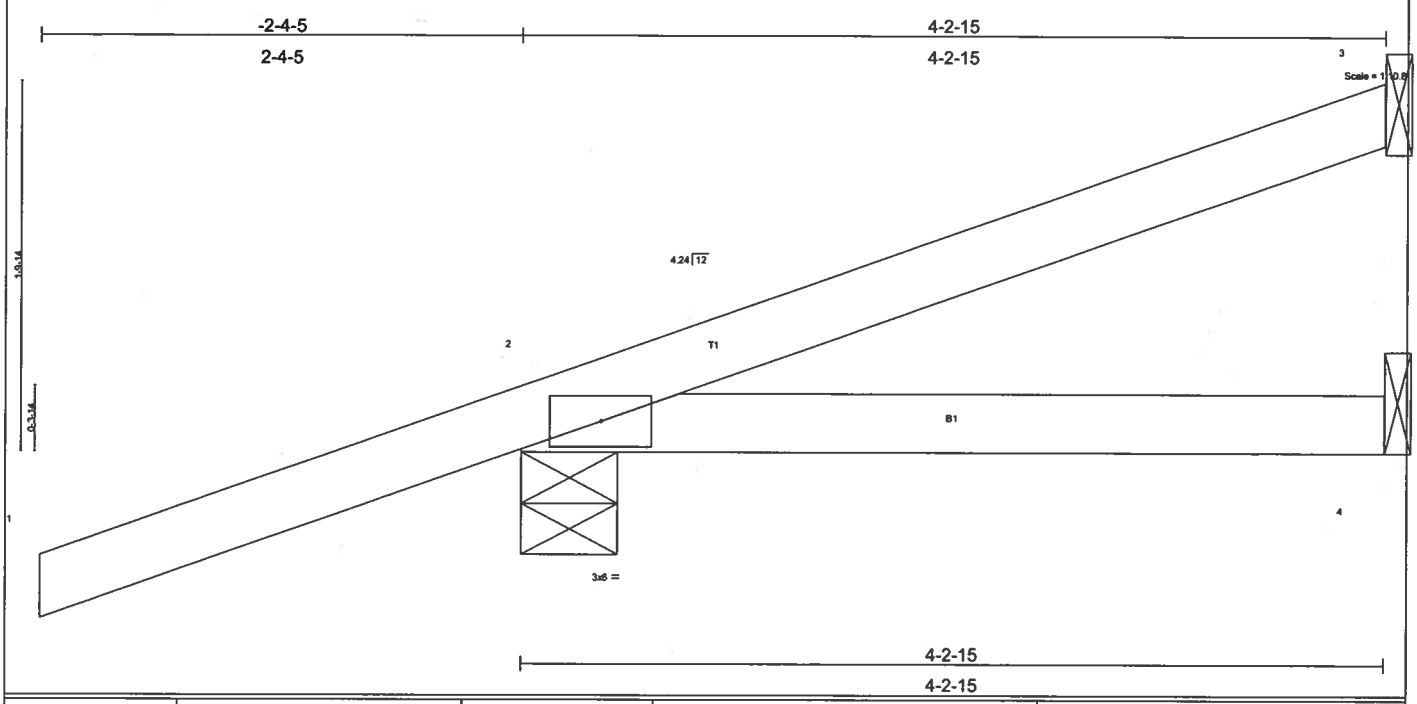

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	0.02	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 17 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=249/0-5-11, 4=42/Mechanical
Max Horz 2=87(load case 2)
Max Uplift3=-12(load case 5), 2=-257(load case 2), 4=-41(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/42, 2-3=-31/6
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.10

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 12 lb uplift at joint 3, 257 lb uplift at joint 2 and 41 lb uplift at joint 4.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-3=-57(F=-2, B=-2), 2=0(F=15, B=15)-to-4=-32(F=-1, B=-1)

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061155

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	HJ7	MONO TRUSS	4	1	Job Reference (optional)

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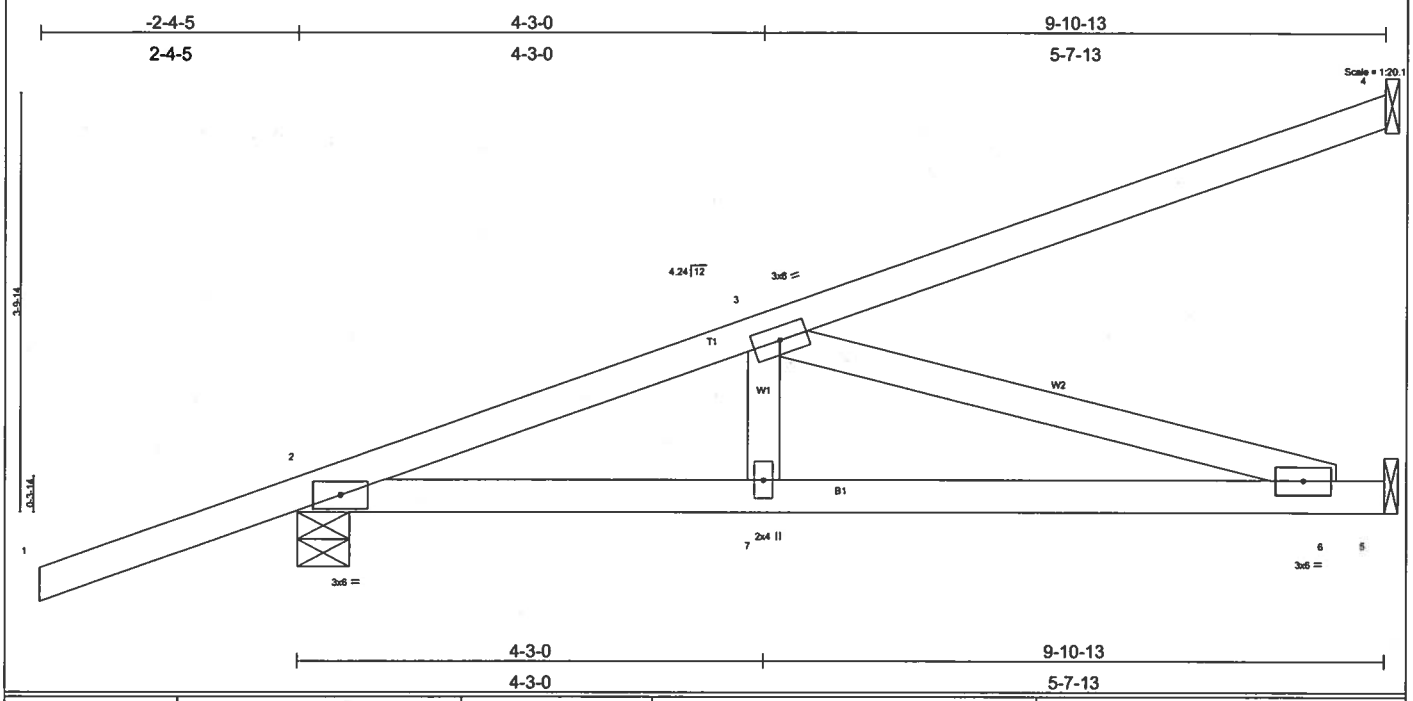

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.11	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.18	6-7	>625	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.49	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 44 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-11-1 oc bracing.

REACTIONS (lb/size) 4=269/Mechanical, 2=502/0-5-11, 5=383/Mechanical
Max Horz 2=258(load case 2)
Max Uplift4=-230(load case 2), 2=-290(load case 4), 5=-72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/42, 2-3=-919/162, 3-4=-104/65
BOT CHORD 2-7=-351/854, 6-7=-351/854, 5-6=0/0
WEBS 3-7=0/202, 3-6=-890/366

JOINT STRESS INDEX
2 = 0.71, 3 = 0.24, 6 = 0.25 and 7 = 0.15

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 230 lb uplift at joint 4, 290 lb uplift at joint 2 and 72 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=0(F=15, B=15)-to-5=-74(F=-22, B=-22)

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061156

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	HJ7A	MONO TRUSS	1	1	Job Reference (optional)

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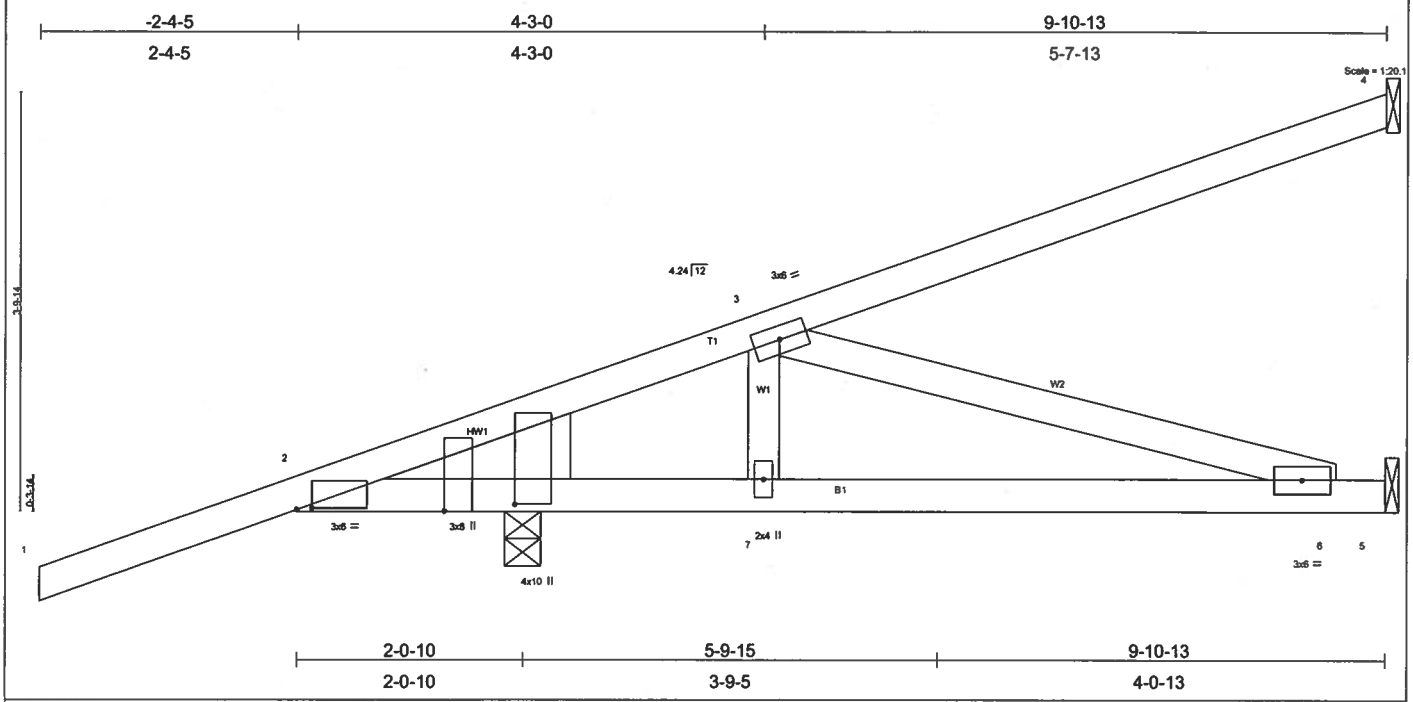

Plate Offsets (X,Y): [2:0-1-11,0-0-3], [2:0-0-3,Edge], [2:0-0-9,1-11-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.11	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.60	Vert(TL)	-0.19	6-7	>621	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.43	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 49 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 8 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-7-4 oc bracing.

REACTIONS (lb/size) 4=269/Mechanical, 5=361/Mechanical, 2=400/0-4-0
Max Horz 2=258(load case 2)
Max Uplift4=-230(load case 2), 5=-79(load case 2), 2=-306(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/42, 2-3=-818/191, 3-4=-104/66
BOT CHORD 2-7=-379/759, 6-7=-379/759, 5-6=0/0
WEBS 3-7=0/140, 3-6=-791/395

JOINT STRESS INDEX
2 = 0.67, 2 = 0.22, 2 = 0.27, 3 = 0.21, 6 = 0.22 and 7 = 0.10

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 230 lb uplift at joint 4, 79 lb uplift at joint 5 and 306 lb uplift at joint 2.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-2(F=26, B=26)-to-4=-134(F=-40, B=-40), 2=30(F=15, B=15)-to-7=-1(F=-1, B=-1), 7=-31(F=-1, B=-1)-to-5=-74(F=-22, B=-22)

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061157

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T01	HIP	1	1	Job Reference (optional)

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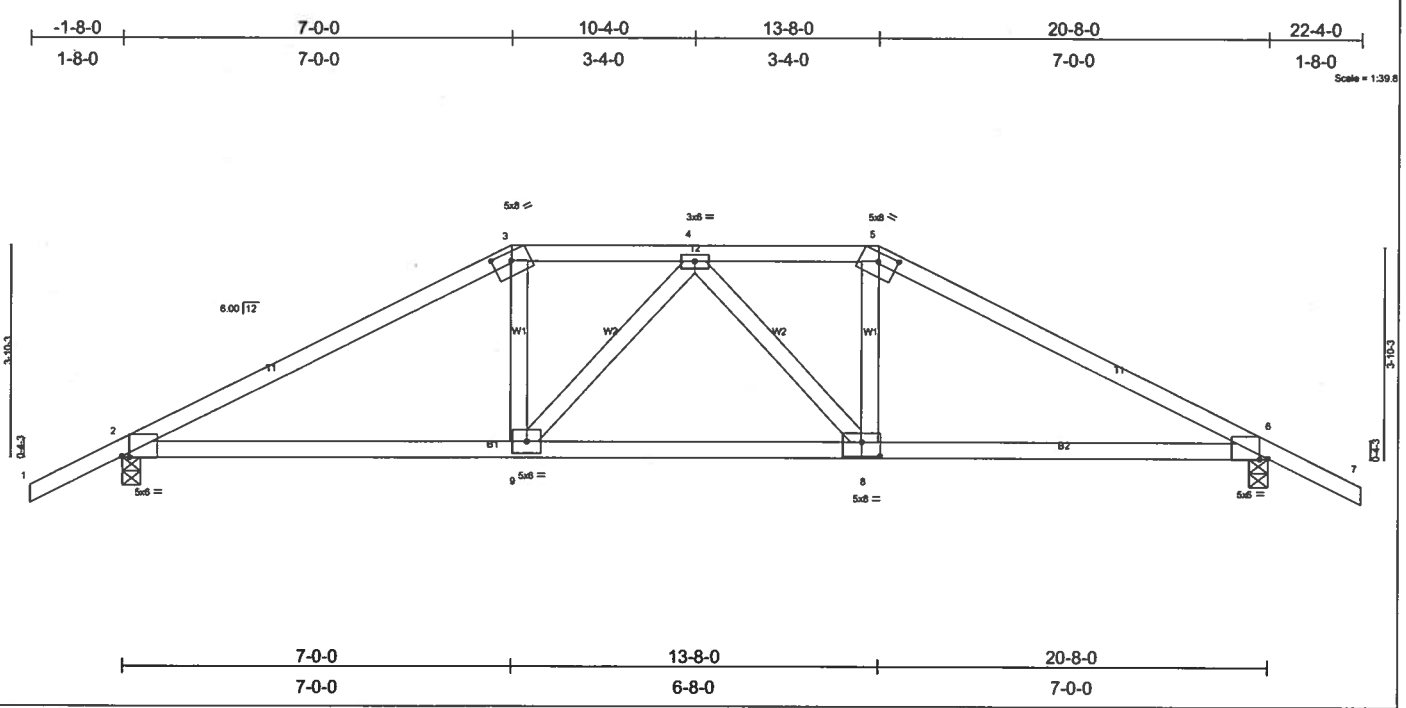

Plate Offsets (X,Y): [2:0-1-11,Edge], [3:0-4-0,0-1-15], [5:0-4-0,0-1-15], [6:0-1-11,Edge], [8:0-4-0,0-3-0]

LOADING (psf)		SPACING 2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	-0.20	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.91	Vert(TL)	-0.33	8-9	>749	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.35	Horz(TL)	0.10	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 93 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-1-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-8-12 oc bracing.

REACTIONS (lb/size) 2=1814/0-4-0, 6=1814/0-4-0
Max Horz 2=-81(load case 5)
Max Uplift2=-809(load case 4), 6=-809(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-3265/1315, 3-4=-2871/1249, 4-5=-2871/1249, 5-6=-3265/1315, 6-7=0/39
BOT CHORD 2-9=-1102/2828, 8-9=-1226/2993, 6-8=-1059/2828
WEBS 3-9=-398/1097, 4-9=-308/260, 4-8=-308/260, 5-8=-398/1097

JOINT STRESS INDEX
2 = 0.82, 3 = 0.75, 4 = 0.38, 5 = 0.75, 6 = 0.82, 8 = 0.77 and 9 = 0.39

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 809 lb uplift at joint 2 and 809 lb uplift at joint 6.
5) Girder carries hip end with 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 13-8-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-117(F=-63), 5-7=-54, 2-9=-30, 8-9=-65(F=-35), 6-8=-30
Concentrated Loads (lb)
Vert: 8=-539(F) 9=-539(F)

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061158

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T02	HIP	1	1	Job Reference (optional)

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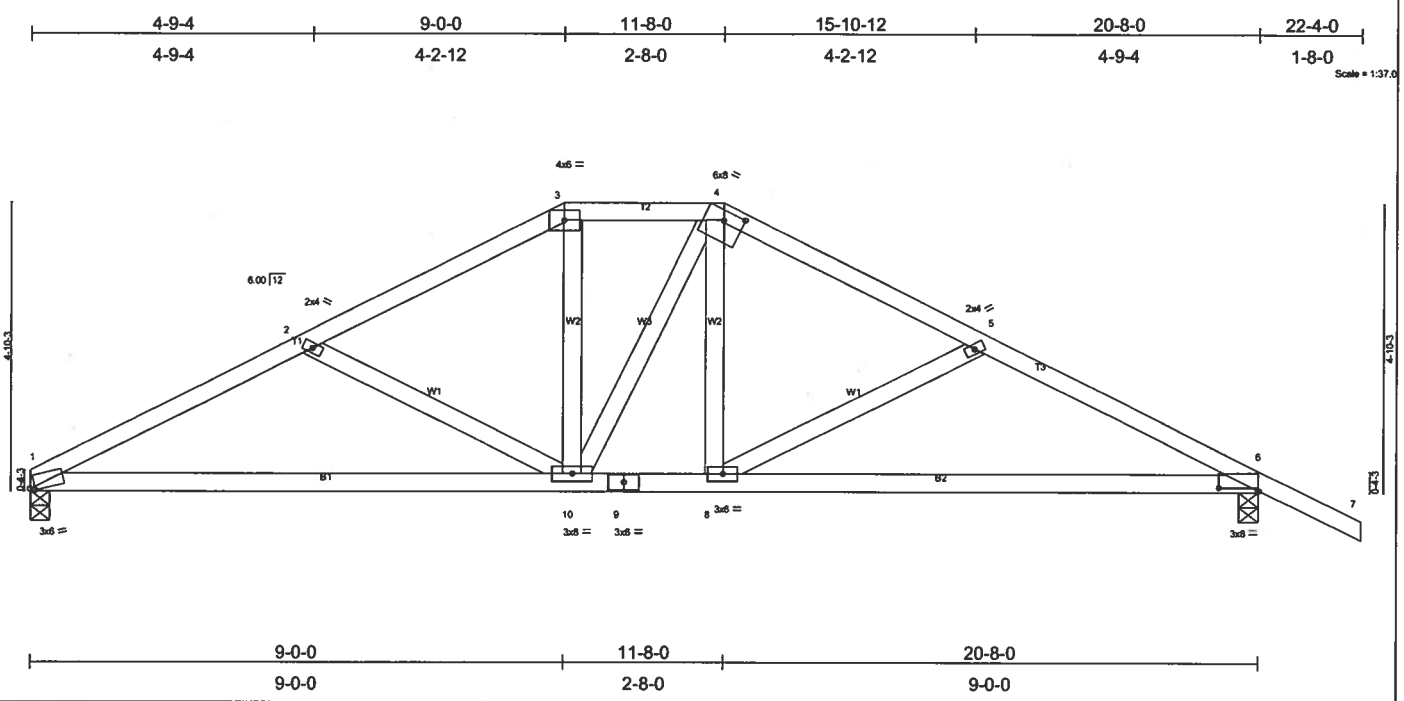

Plate Offsets (X,Y): [1:0-1-1,0-0-7], [4:0-4-0,0-1-15], [6:0-8-0,0-0-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.18	1-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.31	1-10	>798	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.14	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 102 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-3-4 oc bracing.

REACTIONS (lb/size) 1=850/0-4-0, 6=957/0-4-0
Max Horz 1=-115(load case 6)
Max Uplift1=-268(load case 5), 6=-378(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1425/656, 2-3=-1147/514, 3-4=-980/507, 4-5=-1139/502, 5-6=-1405/624, 6-7=0/39
BOT CHORD 1-10=-451/1242, 9-10=-203/974, 8-9=-203/974, 6-8=-412/1219
WEBS 2-10=-309/274, 3-10=-93/314, 4-10=-98/116, 4-8=-60/300, 5-8=-289/238

JOINT STRESS INDEX
1 = 0.88, 2 = 0.34, 3 = 0.33, 4 = 0.39, 5 = 0.34, 6 = 0.74, 8 = 0.35, 9 = 0.64 and 10 = 0.62

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 268 lb uplift at joint 1 and 378 lb uplift at joint 6.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061159

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T03	COMMON	4	1	Job Reference (optional)

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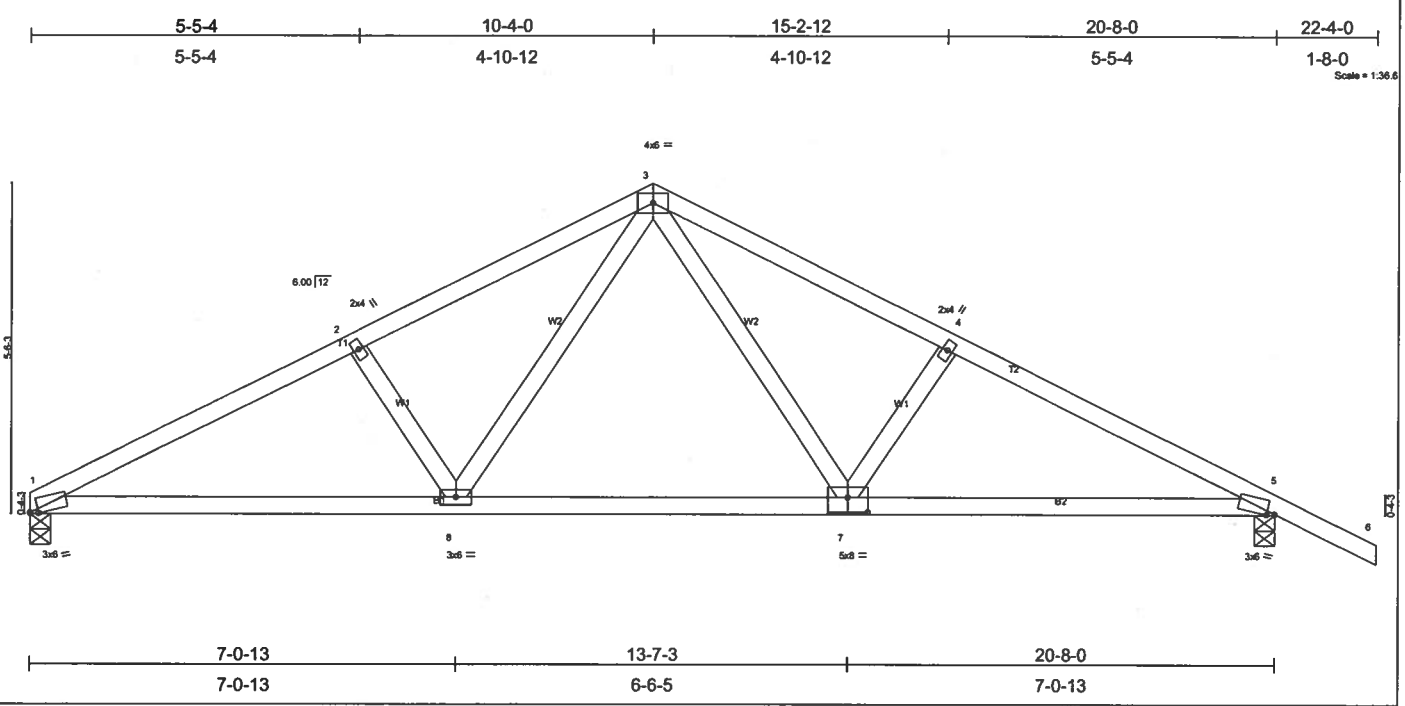

Plate Offsets (X,Y): [1:0-1-9,0-0-7], [5:0-1-9,0-0-7], [7:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.17	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.78	Vert(TL)	-0.27	7-8	>888	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.23	Horz(TL)	0.05	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 95 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-5-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-1-12 oc bracing.

REACTIONS (lb/size) 1=1013/0-4-0, 5=1121/0-4-0
Max Horz 1=-124(load case 6)
Max Uplift1=-337(load case 5), 5=-448(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1822/803, 2-3=-1685/800, 3-4=-1668/772, 4-5=-1817/774, 5-6=0/39
BOT CHORD 1-8=-572/1576, 7-8=-279/1076, 5-7=-538/1555
WEBS 2-8=-248/244, 3-8=-306/724, 3-7=-262/699, 4-7=-235/225

JOINT STRESS INDEX
1 = 0.80, 2 = 0.34, 3 = 0.58, 4 = 0.34, 5 = 0.80, 7 = 0.67 and 8 = 0.55

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 337 lb uplift at joint 1 and 448 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-6=-54, 1-8=-30, 7-8=-80(F=-50), 5-7=-30

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061160

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T04	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:35 2006 Page 1

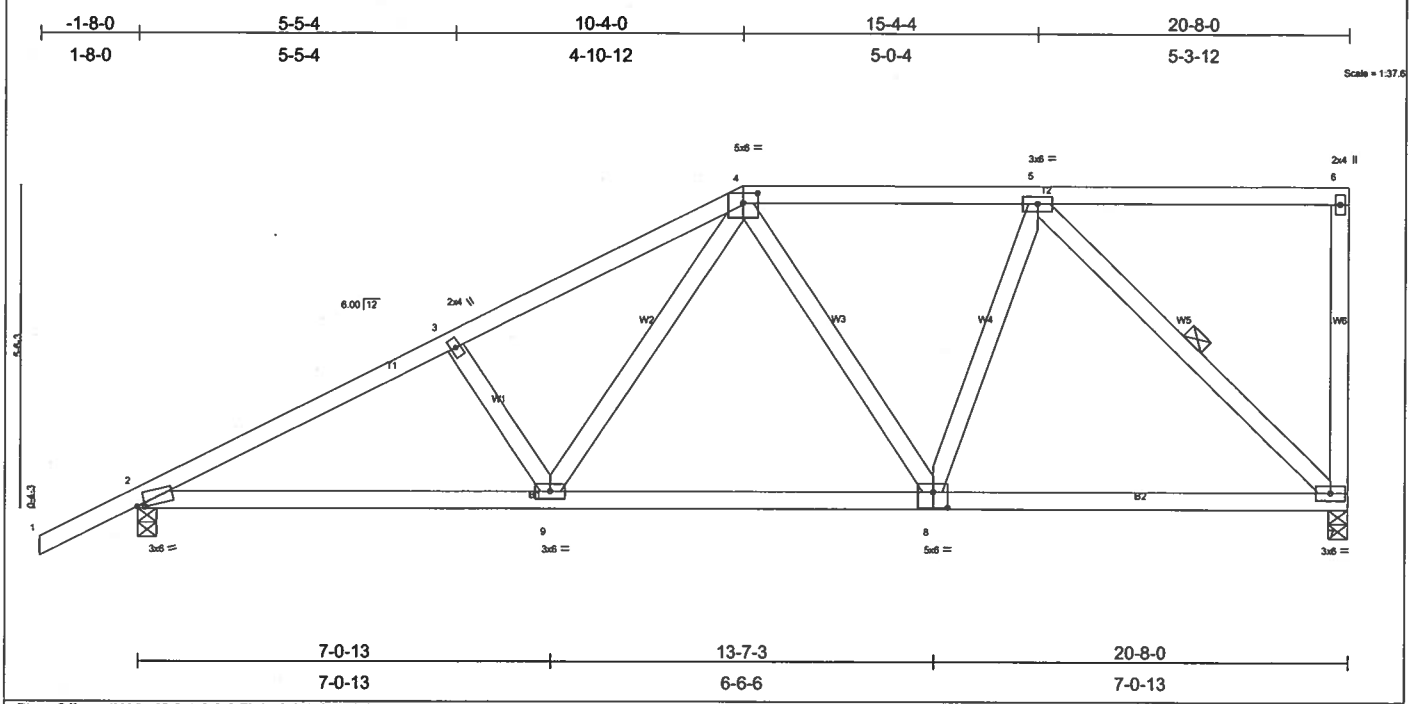

Plate Offsets (X,Y): [2:0-1-9,0-0-7], [4:0-3-0,0-2-0], [8:0-3-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.16	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.75	Vert(TL)	-0.25	8-9	>979	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.32	Horz(TL)	-0.04	2	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 114 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-6 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-2-11 oc bracing.
WEBS 1 Row at midpt 5-7

REACTIONS (lb/size) 7=1014/0-4-0, 2=1122/0-4-0
Max Horz 7=292(load case 5)
Max Uplift7=-356(load case 4), 2=-426(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-1820/684, 3-4=-1671/682, 4-5=-995/435, 5-6=-25/4, 6-7=-130/90
BOT CHORD 2-9=-459/1558, 8-9=-199/1075, 7-8=-153/821
WEBS 3-9=-222/233, 4-9=-274/687, 4-8=-151/159, 5-8=-181/547, 5-7=-1131/530

JOINT STRESS INDEX
2 = 0.80, 3 = 0.34, 4 = 0.53, 5 = 0.50, 6 = 0.76, 7 = 0.39, 8 = 0.84 and 9 = 0.52

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 356 lb uplift at joint 7 and 426 lb uplift at joint 2.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-6=-54, 2-9=-30, 8-9=-80(F=-50), 7-8=-30

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061161

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T05	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:36 2006 Page 1

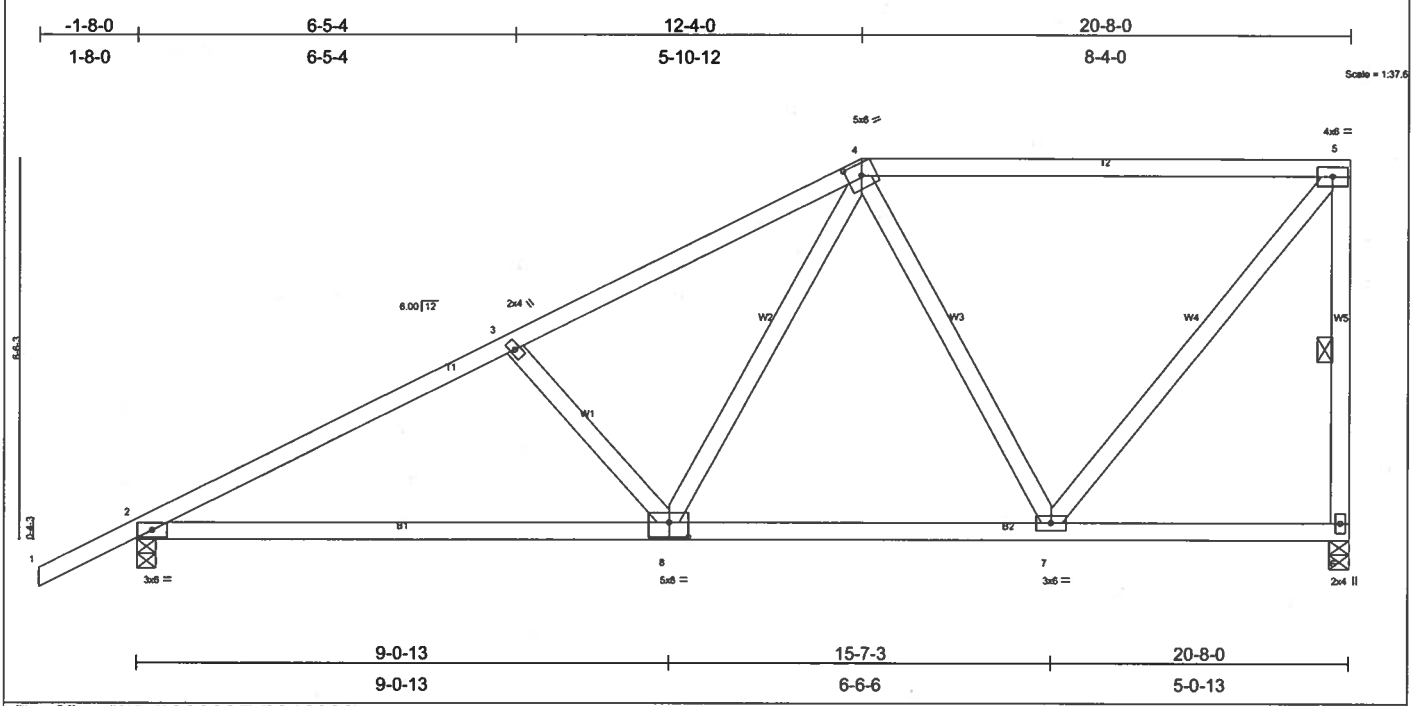


Plate Offsets (X,Y): [4:0-3-0,0-2-7], [8:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.72	Vert(LL)	-0.14	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.69	Vert(TL)	-0.24	2-8	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.48	Horz(TL)	-0.03	2	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 113 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 5-6

REACTIONS (lb/size) 6=1046/0-4-0, 2=1090/0-4-0
Max Horz 6=338(load case 5)
Max Uplift6=-357(load case 4), 2=-414(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-1664/612, 3-4=-1446/569, 4-5=-662/295, 5-6=-996/489
BOT CHORD 2-8=-382/1415, 7-8=-110/894, 6-7=0/358
WEBS 3-8=-278/281, 4-8=-274/734, 4-7=-500/341, 5-7=-437/1001

JOINT STRESS INDEX
2 = 0.73, 3 = 0.34, 4 = 0.75, 5 = 0.71, 6 = 0.70, 7 = 0.71 and 8 = 0.82

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 357 lb uplift at joint 6 and 414 lb uplift at joint 2.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-5=-54, 2-8=-30, 7-8=-80(F=-50), 6-7=-30

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

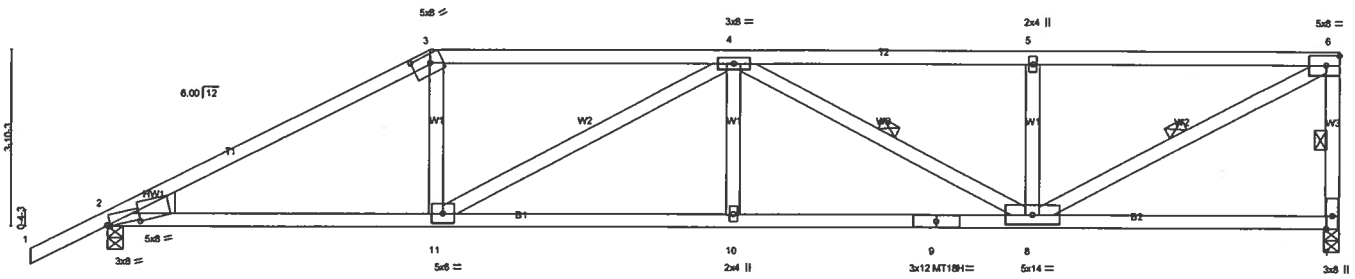
Dwg.#0130061162

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T06	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:37 2006 Page 1

-1-8-0	7-0-0	13-7-4	20-0-12	26-8-0
1-8-0	7-0-0	6-7-4	6-5-8	6-7-4

Scale: 1/4"=1'



7-0-0	13-7-4	20-0-12	26-8-0
7-0-0	6-7-4	6-5-8	6-7-4

Plate Offsets (X,Y): [2:0-8-10,0-0-13], [2:0-0-10,Edge], [3:0-4-12,0-2-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.80	Vert(LL)	-0.32	10-11	>987	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.90	Vert(TL)	-0.52	10-11	>609	180	MT18H	244/190
BCLL	10.0	Rep Stress Incr	NO	WB	0.86	Horz(TL)	0.13	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 136 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.2 *Except*
W1 2 X 4 SYP No.3, W1 2 X 4 SYP No.3, W1 2 X 4 SYP No.3
WEDGE
Left: 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-4-11 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-6-7 oc bracing.
WEBS 1 Row at midpt 6-7, 4-8, 6-8

REACTIONS (lb/size) 7=2449/0-4-0, 2=2310/0-4-0
Max Horz 2=216(load case 4)
Max Uplift7=-1102(load case 3), 2=-982(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-4366/1844, 3-4=-3871/1705, 4-5=-3546/1591, 5-6=-3546/1591, 6-7=-2233/1125
BOT CHORD 2-11=-1687/3814, 10-11=-2159/4803, 9-10=-2159/4803, 8-9=-2159/4803, 7-8=-75/159
WEBS 3-11=-475/1383, 4-11=-1064/580, 4-10=0/429, 4-8=-1434/648, 5-8=-763/629, 6-8=-1731/3867

JOINT STRESS INDEX
2 = 0.80, 2 = 0.73, 3 = 0.92, 4 = 0.68, 5 = 0.36, 6 = 0.79, 7 = 0.61, 8 = 0.94, 9 = 0.99, 10 = 0.34 and 11 = 0.49

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) All plates are MT20 plates unless otherwise indicated.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1102 lb uplift at joint 7 and 982 lb uplift at joint 2.
5) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-6=-117(F=-63), 2-11=-30, 7-11=-65(F=-35)
Concentrated Loads (lb)
Vert: 11=-539(F)

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

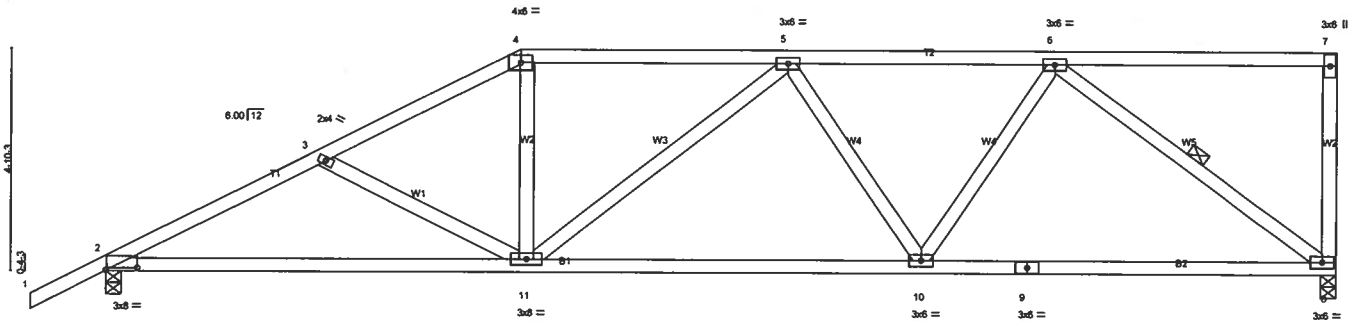
Dwg.#0130061163

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T07	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:37 2006 Page 1

-1-8-0	4-9-4	9-0-0	14-9-8	20-7-0	26-8-0
1-8-0	4-9-4	4-2-12	5-9-8	5-9-8	6-1-0

Scale: 1/4"=1'



9-0-0	17-8-4	26-8-0
9-0-0	8-8-4	8-11-12

Plate Offsets (X,Y): [2:0-8-4,0-0-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.17	2-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.55	Vert(TL)	-0.29	2-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.40	Horz(TL)	0.06	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 140 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-10-14 oc bracing.
WEBS 1 Row at midpt 6-8

REACTIONS (lb/size) 8=1103/0-4-0, 2=1209/0-4-0
Max Horz 2=261(load case 5)
Max Uplift8=-401(load case 4), 2=-425(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-1941/771, 3-4=-1704/663, 4-5=-1493/648, 5-6=-1445/575, 6-7=-61/11, 7-8=-156/104
BOT CHORD 2-11=-837/1688, 10-11=-692/1593, 9-10=-494/1129, 8-9=-494/1129
WEBS 3-11=-236/214, 4-11=-77/462, 5-11=-128/168, 5-10=-277/218, 6-10=-152/588, 6-8=-1346/609

JOINT STRESS INDEX
2 = 0.80, 3 = 0.34, 4 = 0.61, 5 = 0.42, 6 = 0.45, 7 = 0.37, 8 = 0.74, 9 = 0.48, 10 = 0.45 and 11 = 0.57

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 401 lb uplift at joint 8 and 425 lb uplift at joint 2.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061164

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T08	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:38 2006 Page 1

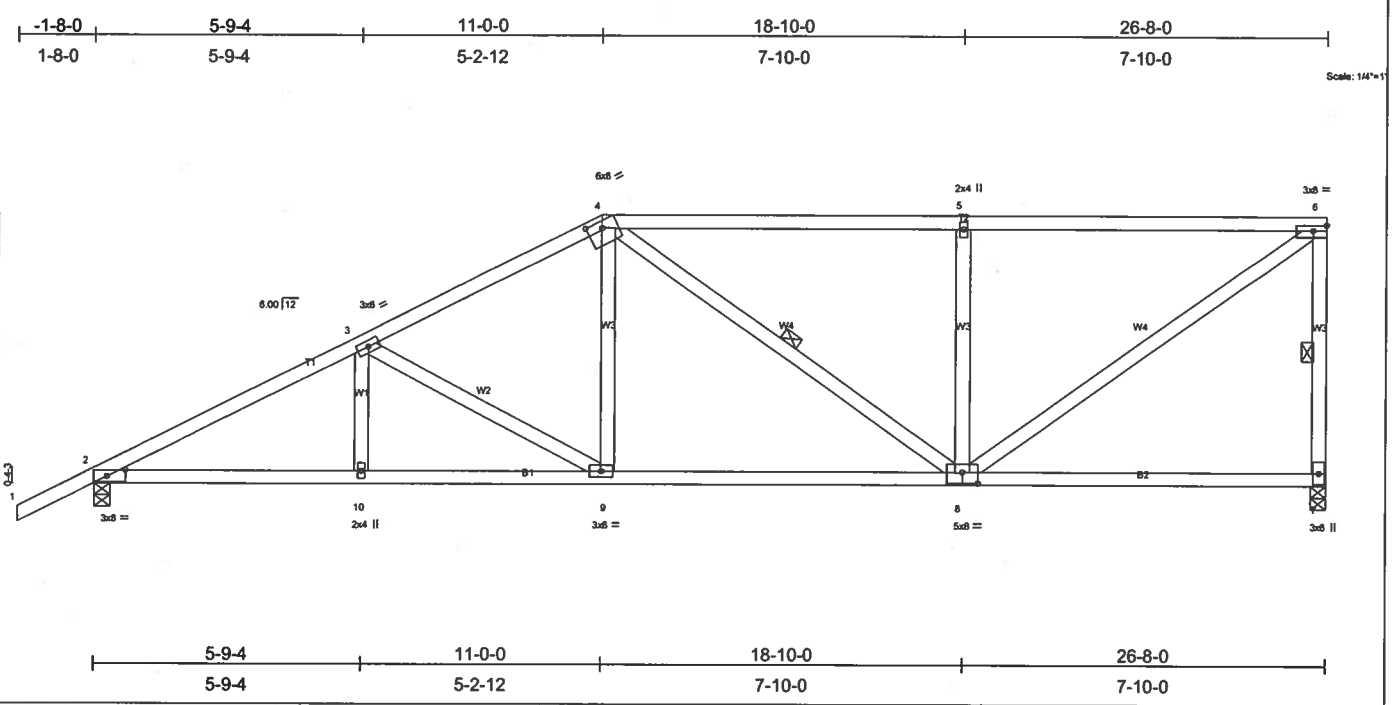


Plate Offsets (X,Y): [2:0-4-12,0-1-8], [4:0-4-0,0-1-15], [8:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.65	Vert(LL)	-0.13	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.22	8-9	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.92	Horz(TL)	0.05	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 147 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-11-4 oc bracing.
WEBS 1 Row at midpt 6-7, 4-8

REACTIONS (lb/size) 7=1103/0-4-0, 2=1209/0-4-0
Max Horz 2=307(load case 5)
Max Uplift7=-394(load case 4), 2=-434(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-1989/723, 3-4=-1523/614, 4-5=-1163/500, 5-6=-1163/500, 6-7=-992/480
BOT CHORD 2-10=-836/1703, 9-10=-836/1703, 8-9=-613/1323, 7-8=-21/53
WEBS 3-10=0/165, 3-9=-445/256, 4-9=-88/413, 4-8=-197/142, 5-8=-448/324, 6-8=-591/1370

JOINT STRESS INDEX
2 = 0.74, 3 = 0.41, 4 = 0.68, 5 = 0.34, 6 = 0.69, 7 = 0.40, 8 = 0.80, 9 = 0.35 and 10 = 0.34

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 394 lb uplift at joint 7 and 434 lb uplift at joint 2.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061165

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T09	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:39 2006 Page 1

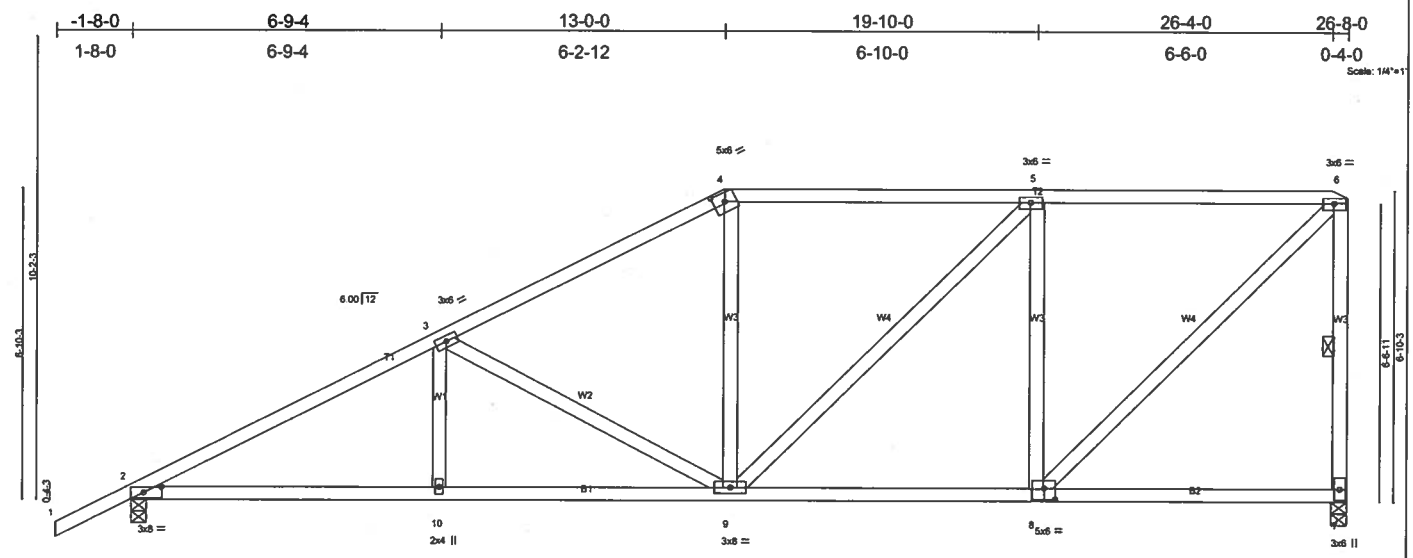


Plate Offsets (X,Y): [2:0-4-12,0-1-8], [4:0-3-0,0-2-7], [8:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	-0.11	2-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.17	2-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.81	Horz(TL)	0.05	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 154 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-9-14 oc bracing.
WEBS 1 Row at midpt 6-7

REACTIONS (lb/size) 2=1209/0-4-0, 7=1103/0-4-0
Max Horz 2=354(load case 5)
Max Uplift2=-439(load case 5), 7=-384(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-1956/696, 3-4=-1352/541, 4-5=-1146/550, 5-6=-891/397, 6-7=-1007/493
BOT CHORD 2-10=-854/1670, 9-10=-854/1670, 8-9=-397/891, 7-8=-13/32
WEBS 3-10=0/216, 3-9=-601/344, 4-9=0/226, 5-9=-215/356, 5-8=-636/426, 6-8=-538/1204

JOINT STRESS INDEX
2 = 0.73, 3 = 0.41, 4 = 0.58, 5 = 0.37, 6 = 0.76, 7 = 0.30, 8 = 0.53, 9 = 0.57 and 10 = 0.34

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 439 lb uplift at joint 2 and 384 lb uplift at joint 7.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061166

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T10	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:40 2006 Page 1

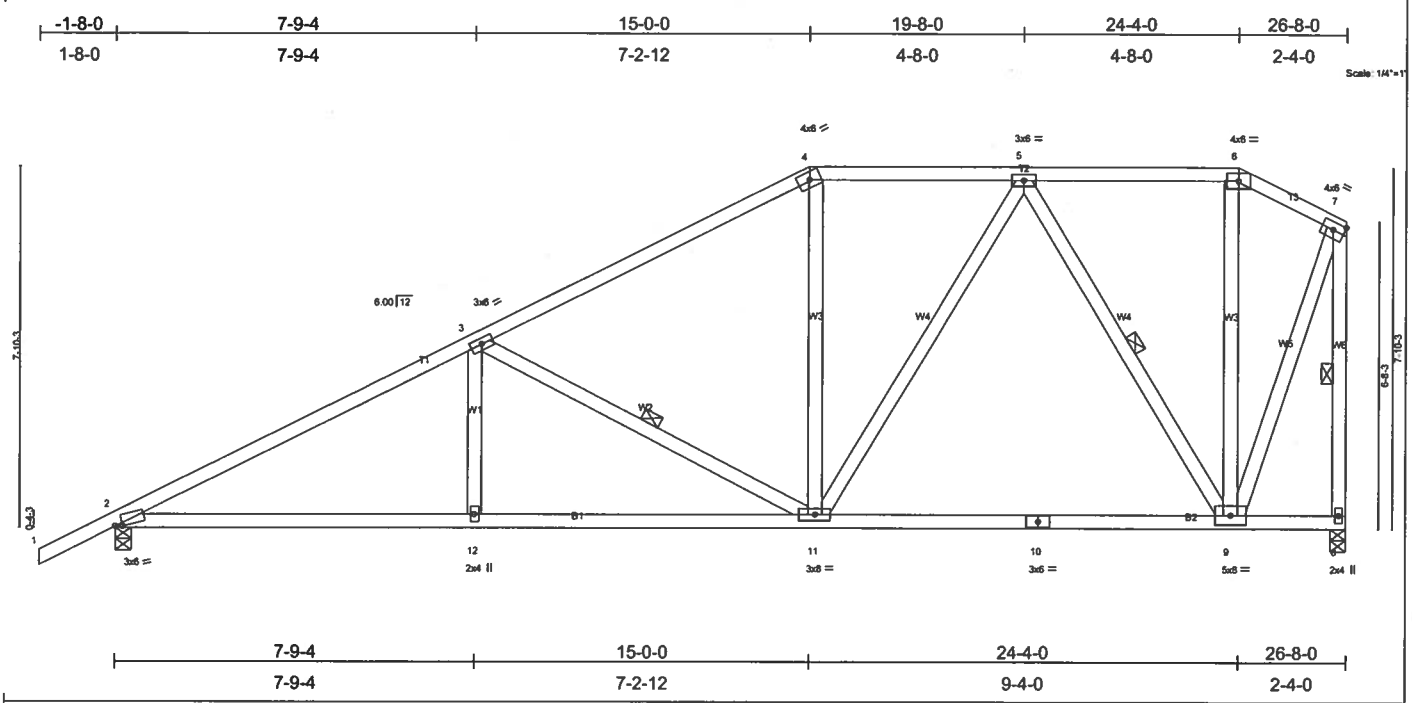


Plate Offsets (X,Y): [2:0-1-13,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.16	9-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.57	Vert(TL)	-0.28	9-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.32	Horz(TL)	0.05	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 169 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-9-8 oc bracing.
WEBS 1 Row at midpt 3-11, 5-9, 7-8

REACTIONS (lb/size) 2=1209/0-4-0, 8=1103/0-4-0
Max Horz 2=364(load case 5)
Max Uplift2=-450(load case 5), 8=-328(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-1908/694, 3-4=-1194/493, 4-5=-986/514, 5-6=-341/192, 6-7=-395/178, 7-8=-1138/474
BOT CHORD 2-12=-835/1622, 11-12=-835/1622, 10-11=-341/733, 9-10=-341/733, 8-9=-4/2
WEBS 3-12=0/243, 3-11=-724/426, 4-11=0/195, 5-11=-226/497, 5-9=-770/401, 6-9=-56/81, 7-9=-392/997

JOINT STRESS INDEX
2 = 0.81, 3 = 0.41, 4 = 0.72, 5 = 0.44, 6 = 0.33, 7 = 0.60, 8 = 0.42, 9 = 0.58, 10 = 0.53, 11 = 0.59 and 12 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 450 lb uplift at joint 2 and 328 lb uplift at joint 8.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

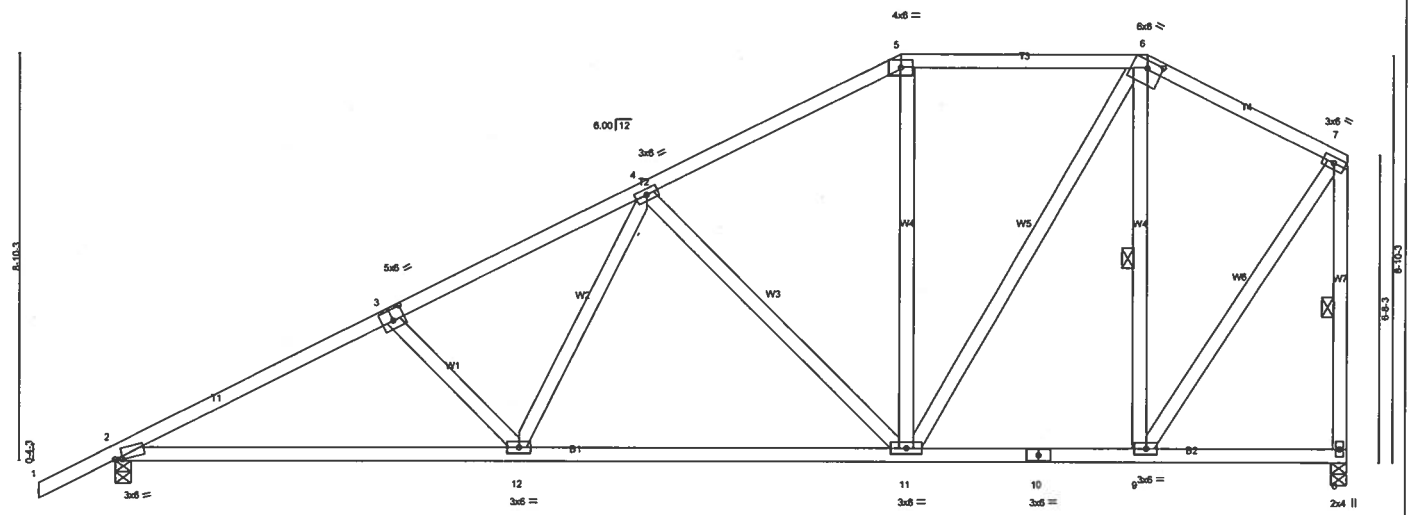
Dwg.#0130061167

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T11	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:41 2006 Page 1

-1-8-0	6-0-5	11-6-3	17-0-0	22-4-0	26-8-0
1-8-0	6-0-5	5-5-13	5-5-13	5-4-0	4-4-0

Scale: 1/4"=1'



8-9-4	17-0-0	22-4-0	26-8-0
8-9-4	8-2-12	5-4-0	4-4-0

Plate Offsets (X,Y): [2:0-1-13,0-0-7], [3:0-3-0,0-3-0], [6:0-4-0,0-1-15]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	-0.16	2-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.27	2-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.72	Horz(TL)	0.04	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 172 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-8-6 oc bracing.
WEBS 1 Row at midpt 6-9, 7-8

REACTIONS (lb/size) 2=1209/0-4-0, 8=1103/0-4-0
Max Horz 2=378(load case 5)
Max Uplift2=-456(load case 5), 8=-349(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-1919/739, 3-4=-1712/685, 4-5=-965/461, 5-6=-806/470, 6-7=-584/296, 7-8=-1039/508
BOT CHORD 2-12=-894/1661, 11-12=-640/1238, 10-11=-204/473, 9-10=-204/473, 8-9=-7/13
WEBS 3-12=-284/273, 4-12=-159/541, 4-11=-621/397, 5-11=0/103, 6-11=-307/667, 6-9=-574/332, 7-9=-362/842

JOINT STRESS INDEX
2 = 0.82, 3 = 0.46, 4 = 0.41, 5 = 0.62, 6 = 0.49, 7 = 0.70, 8 = 0.51, 9 = 0.65, 10 = 0.19, 11 = 0.72 and 12 = 0.46

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 456 lb uplift at joint 2 and 349 lb uplift at joint 8.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061168

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T12	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:42 2006 Page 1

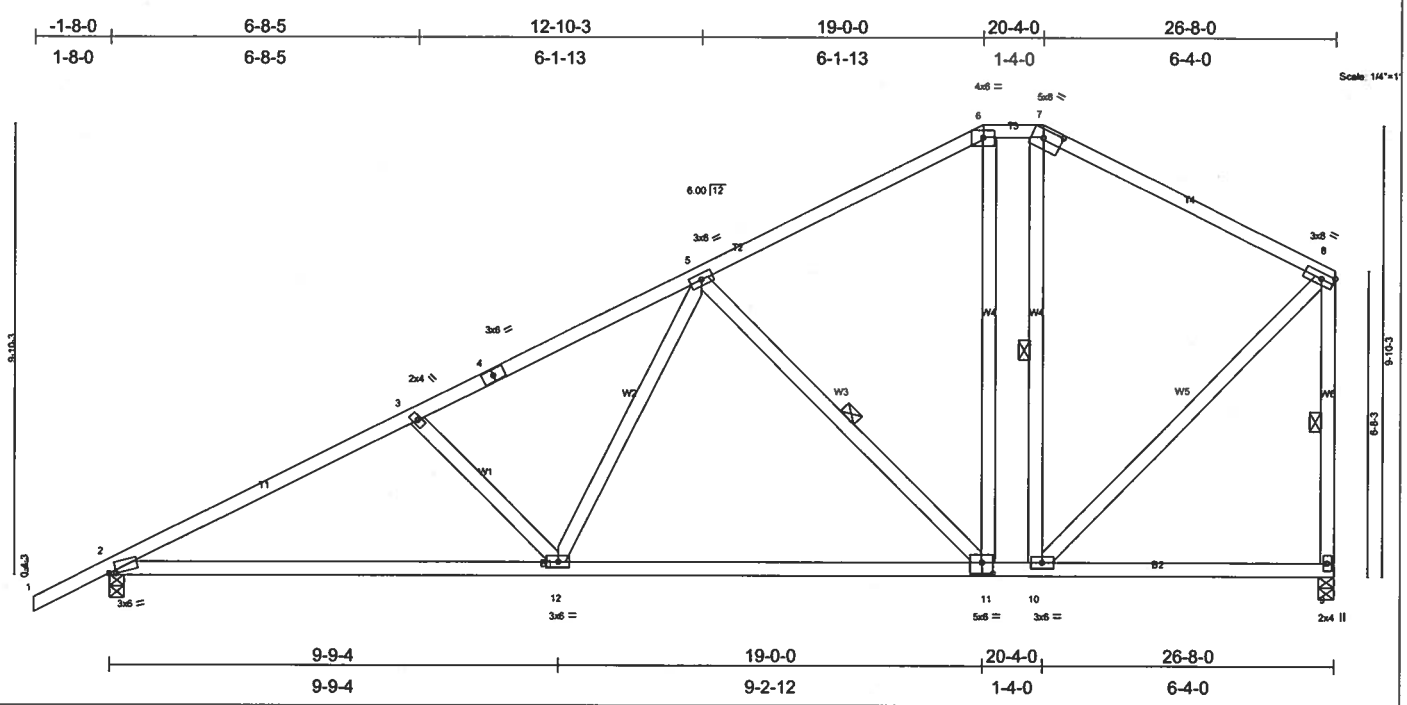


Plate Offsets (X,Y): [2:0-1-13,0-0-7], [7:0-4-12,0-2-4], [11:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.32	11-12	>989	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.78	Vert(TL)	-0.51	11-12	>622	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.52	Horz(TL)	0.04	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 166 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 5-11, 7-10, 8-9

REACTIONS (lb/size) 2=1209/0-4-0, 9=1103/0-4-0
Max Horz 2=392(load case 5)
Max Uplift2=-461(load case 5), 9=-373(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-1885/741, 3-4=-1651/655, 4-5=-1515/676, 5-6=-773/403, 6-7=-616/423, 7-8=-748/387, 8-9=-1053/540
BOT CHORD 2-12=-888/1628, 11-12=-592/1139, 10-11=-265/616, 9-10=-14/22
WEBS 3-12=-316/308, 5-12=-194/635, 5-11=-768/472, 6-11=-154/386, 7-10=-207/128, 8-10=-366/865

JOINT STRESS INDEX
2 = 0.80, 3 = 0.34, 4 = 0.29, 5 = 0.44, 6 = 0.41, 7 = 0.89, 8 = 0.96, 9 = 0.61, 10 = 0.56, 11 = 0.47 and 12 = 0.54

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 461 lb uplift at joint 2 and 373 lb uplift at joint 9.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0130061169

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T13	COMMON	1	1	Job Reference (optional)

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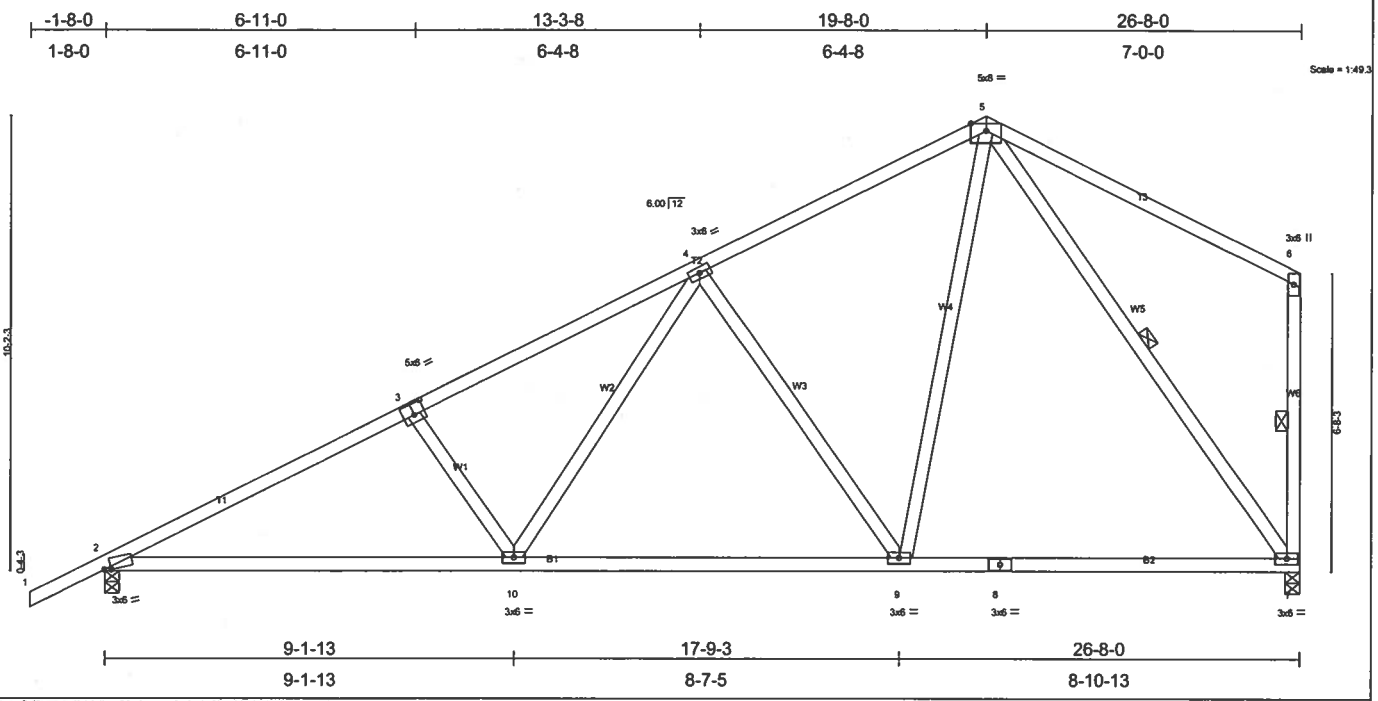


Plate Offsets (X,Y): [2:0-1-13,0-0-7], [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.60	Vert(LL)	-0.20	2-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.57	Vert(TL)	-0.34	2-10	>931	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.77	Horz(TL)	0.05	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 157 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-8-12 oc bracing.
WEBS 1 Row at midpt 6-7, 5-7

REACTIONS (lb/size) 2=1209/0-4-0, 7=1103/0-4-0
Max Horz 2=397(load case 5)
Max Uplift2=-462(load case 5), 7=-381(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-1882/736, 3-4=-1695/723, 4-5=-895/472, 5-6=-135/123, 6-7=-209/181
BOT CHORD 2-10=-882/1624, 9-10=-570/1096, 8-9=-264/587, 7-8=-264/587
WEBS 3-10=-328/321, 4-10=-255/657, 4-9=-663/443, 5-9=-311/830, 5-7=-952/431

JOINT STRESS INDEX
2 = 0.80, 3 = 0.64, 4 = 0.44, 5 = 0.57, 6 = 0.47, 7 = 0.70, 8 = 0.27, 9 = 0.59 and 10 = 0.51

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 462 lb uplift at joint 2 and 381 lb uplift at joint 7.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

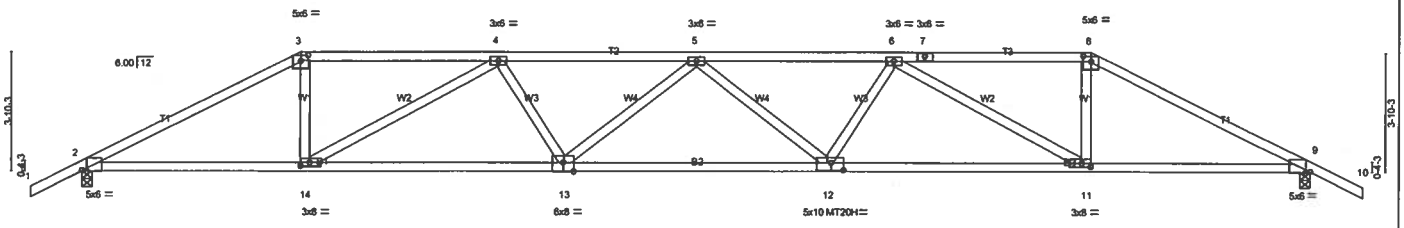
Dwg.#0130061170

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T14	HIP	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:44 2006 Page 1

-1-8-0	7-0-0	13-4-0	19-8-0	26-0-0	32-4-0	39-4-0	41-0-0
1-8-0	7-0-0	6-4-0	6-4-0	6-4-0	6-4-0	7-0-0	1-8-0

Scale = 1:70.8



7-0-0	15-5-5	23-10-11	32-4-0	39-4-0
7-0-0	8-5-5	8-5-5	8-5-5	7-0-0

Plate Offsets (X,Y): [2:0-1-11,Edge], [3:0-3-0,0-2-0], [8:0-3-0,0-2-0], [9:0-1-11,Edge], [11:0-3-8,0-1-8], [12:0-4-12,0-3-0], [13:0-4-0,Edge], [14:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.60	Vert(LL)	-0.59	12-13	>793	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.90	Vert(TL)	-0.95	12-13	>492	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.71	Horz(TL)	0.23	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 366 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-10-6 oc bracing.

REACTIONS (lb/size) 2=3512/0-4-0, 9=3512/0-4-0
Max Horz 2=-81(load case 5)
Max Uplift2=-1453(load case 4), 9=-1453(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-6991/2977, 3-4=-6248/2732, 4-5=-9708/4219, 5-6=-9707/4218, 6-7=-6248/2732, 7-8=-6248/2732, 8-9=-6990/2977, 9-10=0/39
BOT CHORD 2-14=-2616/6141, 13-14=-4042/9158, 12-13=-4480/10145, 11-12=-4004/9157, 9-11=-2573/6140
WEBS 3-14=-1017/2638, 4-14=-3444/1668, 4-13=-226/1100, 5-13=-597/472, 5-12=-598/472, 6-12=-226/1100, 6-11=-3443/1668, 8-11=-1017/2638

JOINT STRESS INDEX
2 = 0.85, 3 = 0.72, 4 = 0.52, 5 = 0.35, 6 = 0.52, 7 = 0.50, 8 = 0.72, 9 = 0.85, 11 = 0.85, 12 = 0.95, 13 = 0.99 and 14 = 0.85

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) All plates are MT20 plates unless otherwise indicated.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1453 lb uplift at joint 2 and 1453 lb uplift at joint 9.
8) Girder carries hip end with 7-0-0 end setback.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 32-4-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-8=-117(F=-63), 8-10=-54, 2-14=-30, 11-14=-65(F=-35), 9-11=-30
Concentrated Loads (lb)
Vert: 14=-539(F) 11=-539(F)

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

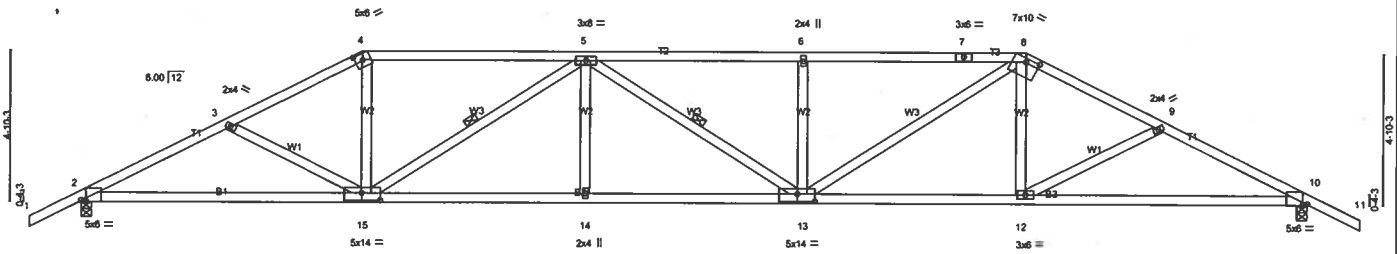
Dwg.#0130061171

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T15	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:45 2006 Page 1

-1-8-0	4-9-4	9-0-0	16-1-15	23-2-1	30-4-0	34-6-12	39-4-0	41-0-0
1-8-0	4-9-4	4-2-12	7-1-15	7-0-3	7-1-15	4-2-12	4-9-4	1-8-0

Scale = 1:70.6



9-0-0	16-1-15	23-2-1	30-4-0	39-4-0
9-0-0	7-1-15	7-0-3	7-1-15	9-0-0

Plate Offsets (X,Y): [2:0-1-11,Edge], [4:0-3-0,0-2-7], [8:0-5-0,0-1-7], [10:0-1-11,Edge], [13:0-7-0,0-3-0], [15:0-7-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.39	13-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.82	Vert(TL)	-0.63	13-14	>745	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.57	Horz(TL)	0.19	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 201 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-9-13 oc bracing.
WEBS 1 Row at midpt 5-15, 5-13

REACTIONS (lb/size) 2=1737/0-4-0, 10=1737/0-4-0
Max Horz 2=95(load case 5)
Max Uplift2=-556(load case 5), 10=-556(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-3059/1252, 3-4=-2860/1164, 4-5=-2520/1098, 5-6=-3441/1469, 6-7=-3441/1469, 7-8=-3442/1469, 8-9=-2851/1162, 9-10=-3058/1254, 10-11=0/39
BOT CHORD 2-15=-965/2673, 14-15=-1174/3443, 13-14=-1174/3443, 12-13=-801/2521, 10-12=-966/2673
WEBS 3-15=-177/188, 4-15=-269/927, 5-15=-1174/471, 5-14=0/203, 5-13=-71/66, 6-13=-391/275, 8-13=-471/1169, 8-12=-34/343, 9-12=-186/190

JOINT STRESS INDEX
2 = 0.76, 3 = 0.34, 4 = 0.71, 5 = 0.57, 6 = 0.34, 7 = 0.57, 8 = 0.90, 9 = 0.34, 10 = 0.76, 12 = 0.35, 13 = 0.75, 14 = 0.34 and 15 = 0.75

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 556 lb uplift at joint 2 and 556 lb uplift at joint 10.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

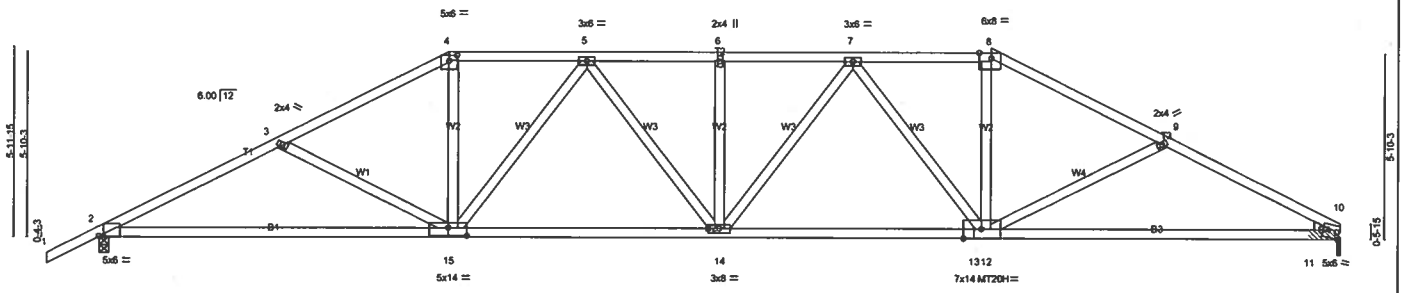
Dwg.#0130061172

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T16	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:47 2006 Page 1

-1-8-0	5-9-4	11-0-0	15-4-0	19-6-4	23-8-8	28-0-8	33-5-0	39-0-8
1-8-0	5-9-4	5-2-12	4-4-0	4-2-4	4-2-4	4-4-0	5-4-8	5-7-8

Scale = 1:69.3



11-1-12	19-6-4	27-10-12	39-0-8
11-1-12	8-4-8	8-4-8	11-1-12

Plate Offsets (X,Y): [2:0-1-10,Edge], [4:0-3-0,0-2-0], [8:0-4-8,Edge], [10:0-0-12,0-2-8], [13:0-6-12,Edge], [15:0-7-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.42	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.95	Vert(TL)	-0.71	10-12	>654	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.58	Horz(TL)	0.16	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 206 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3
WEDGE
Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

REACTIONS (lb/size) 10=1629/0-1-15 (0-1-8 + bearing block), 2=1731/0-3-8
Max Horz 2=129(load case 5)
Max Uplift10=-469(load case 6), 2=-574(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-3016/1279, 3-4=-2706/1130, 4-5=-2357/1070, 5-6=-2874/1286, 6-7=-2874/1286, 7-8=-2364/1080, 8-9=-2674/1132, 9-10=-2952/1280
BOT CHORD 2-15=-1047/2645, 14-15=-951/2702, 13-14=-960/2713, 12-13=-960/2713, 11-12=-1046/2574, 10-11=-1046/2574
WEBS 4-15=-285/914, 8-12=-295/914, 6-14=-229/171, 3-15=-330/294, 5-15=-658/316, 5-14=-106/355, 7-14=-103/340, 7-12=-679/322, 9-12=-272/290

JOINT STRESS INDEX
2 = 0.80, 3 = 0.34, 4 = 0.68, 5 = 0.41, 6 = 0.34, 7 = 0.41, 8 = 0.74, 9 = 0.34, 10 = 0.90, 10 = 0.00, 11 = 0.00, 11 = 0.00, 12 = 0.00, 13 = 0.80, 14 = 0.58 and 15 = 0.59

NOTES
1) 2 X 4 SYP No.1D bearing block 12" long at jt. 10 attached to front face with 2 rows of 0.131"x3" Nails spaced 3" o.c. 8 Total fasteners. Bearing is assumed to be SYP.
2) Unbalanced roof live loads have been considered for this design.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
4) Provide adequate drainage to prevent water ponding.
5) All plates are MT20 plates unless otherwise indicated.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 469 lb uplift at joint 10 and 574 lb uplift at joint 2.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

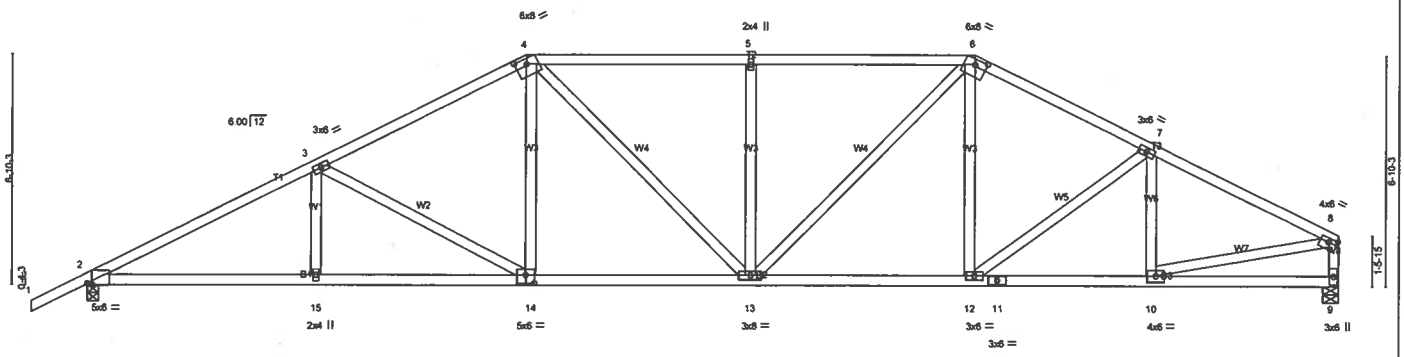
Dwg.#0130061173

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T17	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:48 2006 Page 1

-1-8-0	6-9-4	13-0-0	19-8-0	26-4-0	31-6-8	37-0-8
1-8-0	6-9-4	6-2-12	6-8-0	6-8-0	5-2-8	5-6-0

Scale = 1:65.4



6-9-4	13-0-0	19-8-0	26-4-0	31-6-8	37-0-8
6-9-4	6-2-12	6-8-0	6-8-0	5-2-8	5-6-0

Plate Offsets (X,Y): [2:0-1-11,Edge], [4:0-4-0,0-1-15], [6:0-4-0,0-1-15], [8:0-3-0,0-1-8], [14:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.20	13-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.64	Vert(TL)	-0.32	13-14	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.53	Horz(TL)	0.11	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 211 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-3-10 oc bracing.

REACTIONS (lb/size) 2=1644/0-4-0, 9=1540/0-5-11
Max Horz 2=183(load case 5)
Max Uplift2=-571(load case 5), 9=-444(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-2896/1168, 3-4=-2305/1019, 4-5=-2193/1049, 5-6=-2193/1049, 6-7=-2078/943, 7-8=-2108/888, 8-9=-1447/649
BOT CHORD 2-15=-1003/2503, 14-15=-1003/2503, 13-14=-712/2003, 12-13=-633/1811, 11-12=-717/1824, 10-11=-717/1824, 9-10=-108/206
WEBS 3-15=0/213, 3-14=-583/333, 4-14=-122/467, 4-13=-192/408, 5-13=-378/270, 6-13=-240/633, 6-12=-49/217, 7-12=-85/138, 7-10=-242/186, 8-10=-626/1661

JOINT STRESS INDEX
2 = 0.73, 3 = 0.41, 4 = 0.62, 5 = 0.34, 6 = 0.57, 7 = 0.41, 8 = 0.73, 9 = 0.41, 10 = 0.76, 11 = 0.66, 12 = 0.35, 13 = 0.63, 14 = 0.70 and 15 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 571 lb uplift at joint 2 and 444 lb uplift at joint 9.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

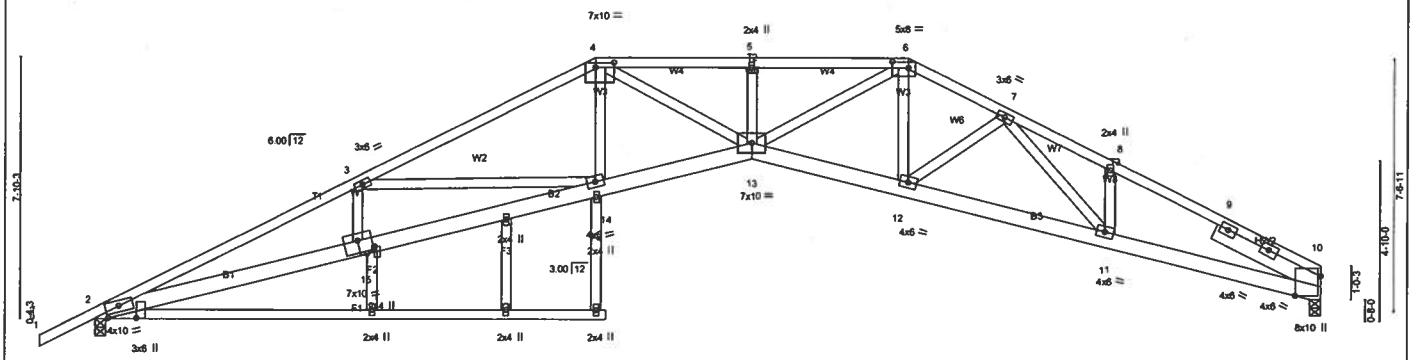
Dwg.#0130061174

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T18	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:49 2006 Page 1

-1-8-0	7-10-11	15-0-0	19-8-0	24-4-0	30-4-4	36-8-0
1-8-0	7-10-11	7-1-5	4-8-0	4-8-0	6-0-4	6-3-12

Scale = 1:66.0



7-10-11	15-0-0	15-4-0	19-8-0	24-4-0	30-4-4	36-8-0
7-10-11	7-1-5	0-4-0	4-4-0	4-8-0	6-0-4	6-3-12

Plate Offsets (X,Y): [4:0-6-8,0-1-12], [6:0-5-12,0-2-4], [10:0-7-0,Edge], [15:0-5-0,0-4-8], [16:0-0-0,0-10-1], [20:0-2-11,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.92	Vert(LL)	-0.63	13-14	>695	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.74	Vert(TL)	-1.01	13-14	>434	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.99	Horz(TL)	0.64	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 249 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T3 2 X 4 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D *Except*
F1 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 6 SYP No.1D 3-7-0

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 7-4-14 oc bracing. Except:
1 Row at midpt 2-15, 14-15
JOINTS 1 Brace at Jt(s): 15

REACTIONS (lb/size) 10=1526/0-4-0, 2=1630/0-4-0
Max Horz 2=180(load case 5)
Max Uplift10=-453(load case 6), 2=-583(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/41, 2-3=-5309/2126, 3-4=-4126/1632, 4-5=-5124/2007, 5-6=-5124/2007, 6-7=-3768/1533, 7-8=-3576/1606, 8-9=-3732/1545, 9-10=-3819/1533
BOT CHORD 2-15=-1873/4812, 14-15=-1877/4830, 13-14=-1225/3736, 12-13=-1091/3445, 11-12=-1251/3576, 10-11=-1237/3293
WEBS 3-15=0/190, 3-14=-1061/626, 4-14=-193/608, 4-13=-534/1709, 5-13=-219/179, 6-13=-682/2031, 6-12=-177/455, 7-12=-156/202, 8-11=-21/267, 7-11=-457/161

JOINT STRESS INDEX
2 = 0.91, 3 = 0.41, 4 = 1.00, 5 = 0.34, 6 = 0.92, 7 = 0.41, 8 = 0.34, 9 = 0.00, 10 = 0.96, 10 = 0.69, 10 = 0.69, 11 = 0.27, 12 = 0.27, 13 = 0.85, 14 = 0.33, 14 = 0.34, 15 = 0.80, 16 = 0.16, 18 = 0.34, 19 = 0.34, 20 = 0.76, 21 = 0.34 and 22 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 10, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 453 lb uplift at joint 10 and 583 lb uplift at joint 2.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

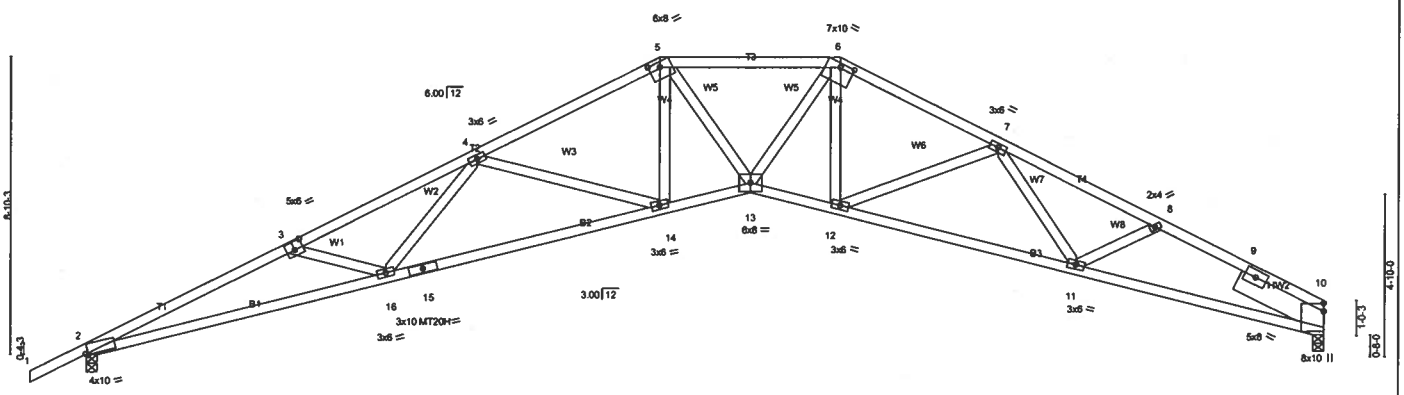
Dwg.#0130061175

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T19	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:50 2006 Page 1

-1-8-0	6-2-5	11-7-2	17-0-0	22-4-0	27-0-3	31-8-5	36-8-0
1-8-0	6-2-5	5-4-14	5-4-14	5-4-0	4-8-3	4-8-3	4-11-11

Scale = 1:65.4



8-10-11	17-0-0	19-8-0	22-4-0	29-4-4	36-8-0
8-10-11	8-1-5	2-8-0	2-8-0	7-0-4	7-3-12

Plate Offsets (X,Y): [2:0-0-14,Edge], [3:0-3-0,0-3-0], [5:0-4-0,0-1-15], [6:0-5-0,0-1-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.97	Vert(LL)	-0.66	14-16	>667	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.87	Vert(TL)	-1.06	14-16	>414	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.50	Horz(TL)	0.68	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 190 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T4 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.1D *Except*
B2 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 8 SYP No.1D 2-10-12

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 4-9-8 oc bracing.

REACTIONS (lb/size) 10=1528/0-4-0, 2=1632/0-4-0
Max Horz 2=190(load case 5)
Max Uplift10=-468(load case 6), 2=-596(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/38, 2-3=-5210/2188, 3-4=-4907/1993, 4-5=-3641/1473, 5-6=-3682/1520, 6-7=-3493/1424, 7-8=-3723/1555, 8-9=-3576/1545, 9-10=-3661/1537
BOT CHORD 2-16=-1936/4735, 15-16=-1528/4066, 14-15=-1526/4120, 13-14=-1012/3308, 12-13=-951/3162, 11-12=-1262/3530, 10-11=-1226/3103
WEBS 3-16=-259/312, 4-16=-175/632, 4-14=-809/526, 5-14=-243/658, 5-13=-235/792, 6-13=-335/1034, 6-12=-178/474, 7-12=-399/327, 7-11=-133/102, 8-11=-32/400

JOINT STRESS INDEX
2 = 0.81, 3 = 0.61, 4 = 0.42, 5 = 0.79, 6 = 0.92, 7 = 0.41, 8 = 0.34, 9 = 0.00, 10 = 0.90, 10 = 1.00, 11 = 0.39, 12 = 0.38, 13 = 0.78, 14 = 0.47, 15 = 0.88 and 16 = 0.41

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Bearing at joint(s) 10, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 468 lb uplift at joint 10 and 596 lb uplift at joint 2.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

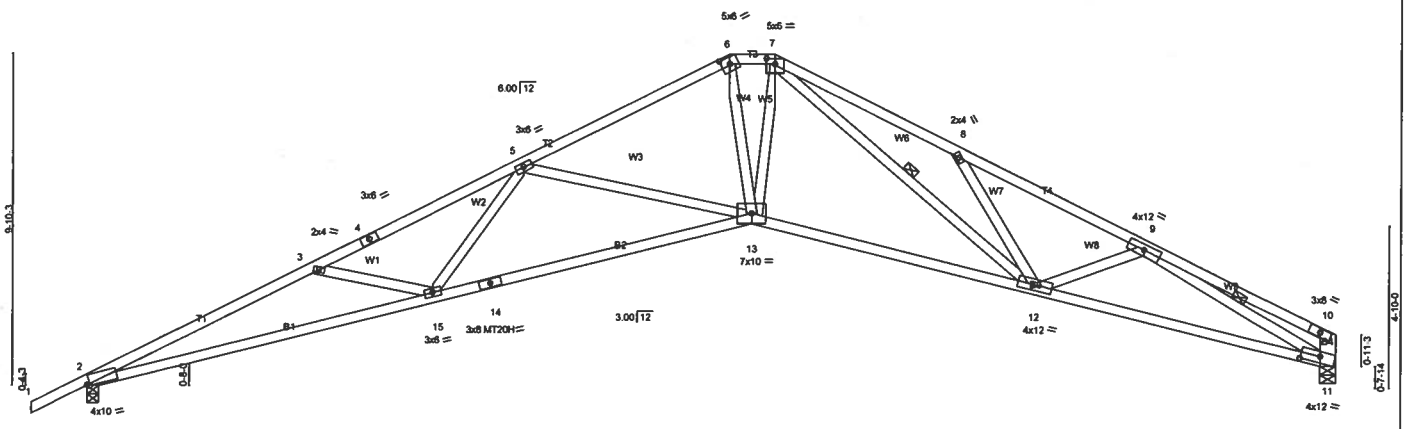
Dwg.#0130061176

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T20	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:51 2006 Page 1

-1-8-0	6-10-5	12-11-2	19-0-0	20-4-0	25-9-4	31-2-9	36-10-0
1-8-0	6-10-5	6-0-14	6-0-14	1-4-0	5-5-4	5-5-4	5-7-7

Scale = 1:65.2



10-2-11	19-8-0	28-0-3	36-4-5	36-10-0
10-2-11	9-5-5	8-4-3	8-4-3	0-5-11

Plate Offsets (X,Y): [2:0-0-14,Edge], [6:0-3-0,0-2-7], [7:0-3-0,0-2-0], [11:0-6-12,0-2-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.79	Vert(LL)	-0.66	13-15	>660	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.94	Vert(TL)	-1.07	13-15	>408	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.89	Horz(TL)	0.67	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 193 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 6 SYP No.1D, B1 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 7-12, 9-11

REACTIONS (lb/size) 11=1528/0-5-11, 2=1632/0-4-0
Max Horz 2=230(load case 5)
Max Uplift11=-482(load case 6), 2=-606(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/38, 2-3=-5170/2258, 3-4=-4757/1957, 4-5=-4681/1974, 5-6=-3244/1354, 6-7=-3005/1337, 7-8=-4154/2089, 8-9=-3861/1643, 9-10=-809/352
BOT CHORD 2-15=-2038/4709, 14-15=-1528/3891, 13-14=-1525/3953, 12-13=-921/2989, 11-12=-1423/3380, 10-11=-508/278
WEBS 3-15=-375/405, 5-15=-186/710, 5-13=-1009/633, 6-13=-450/1227, 7-13=-294/1038, 7-12=-831/1157, 8-12=-507/505, 9-12=0/256, 9-11=-3102/1366

JOINT STRESS INDEX
2 = 0.80, 3 = 0.34, 4 = 0.75, 5 = 0.48, 6 = 0.68, 7 = 0.80, 8 = 0.34, 9 = 0.56, 10 = 0.74, 11 = 0.65, 12 = 0.72, 13 = 0.75, 14 = 0.93 and 15 = 0.46

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Bearing at joint(s) 11, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 482 lb uplift at joint 11 and 606 lb uplift at joint 2.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

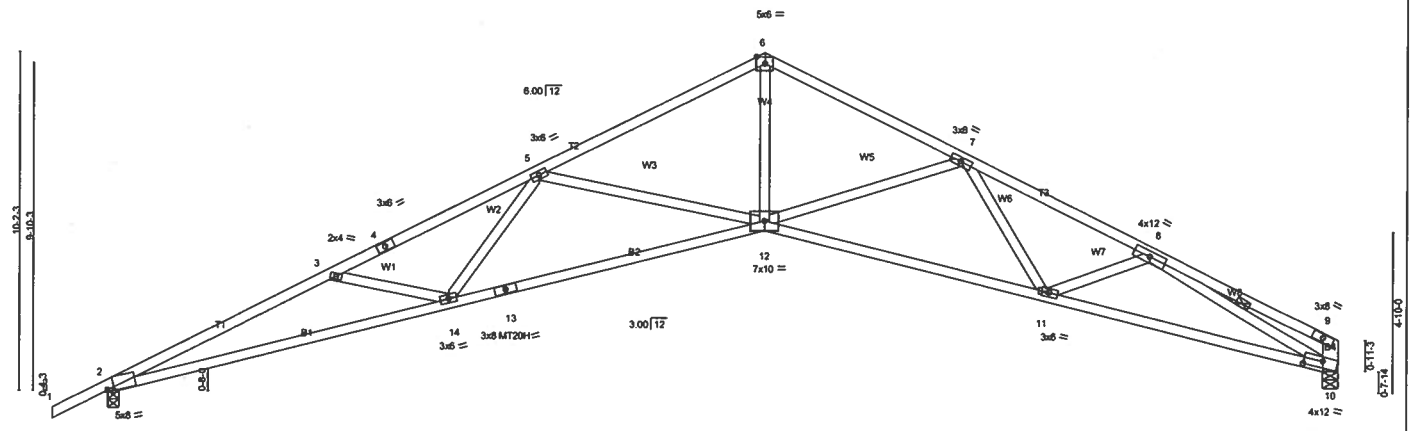
Dwg.#0130061177

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T21	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:52 2006 Page 1

-1-8-0	6-10-5	12-11-2	19-8-0	25-9-4	31-2-9	36-10-0
1-8-0	6-10-5	6-0-14	6-8-14	6-1-5	5-5-4	5-7-7

Scale = 1:66.1



10-2-11	19-8-0	28-0-3	36-4-5	36-10-0
10-2-11	9-5-5	8-4-3	8-4-3	0-5-11

Plate Offsets (X,Y): [2:0-2-7,Edge], [10:0-6-12,0-2-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.79	Vert(LL)	-0.68	12-14	>641	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.93	Vert(TL)	-1.10	12-14	>396	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.91	Horz(TL)	0.69	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 181 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 6 SYP No.1D, B1 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 8-10

REACTIONS (lb/size) 10=1528/0-5-11, 2=1632/0-4-0
Max Horz 2=234(load case 5)
Max Uplift10=-486(load case 6), 2=-609(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/38, 2-3=-5169/2265, 3-4=-4760/1969, 4-5=-4685/1986, 5-6=-3238/1359, 6-7=-3226/1364, 7-8=-3868/1656, 8-9=-809/353
BOT CHORD 2-14=-2045/4708, 13-14=-1540/3895, 12-13=-1538/3956, 11-12=-1300/3455, 10-11=-1428/3378, 9-10=-510/281
WEBS 3-14=-366/400, 5-14=-184/706, 5-12=-1040/647, 7-11=-54/245, 8-11=0/246, 8-10=-3101/1371, 6-12=-930/2468, 7-12=-610/418

JOINT STRESS INDEX
2 = 1.00, 3 = 0.34, 4 = 0.75, 5 = 0.47, 6 = 0.86, 7 = 0.41, 8 = 0.56, 9 = 0.75, 10 = 0.65, 11 = 0.47, 12 = 0.75, 13 = 0.93 and 14 = 0.46

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All plates are MT20 plates unless otherwise indicated.
4) Bearing at joint(s) 10, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 486 lb uplift at joint 10 and 609 lb uplift at joint 2.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

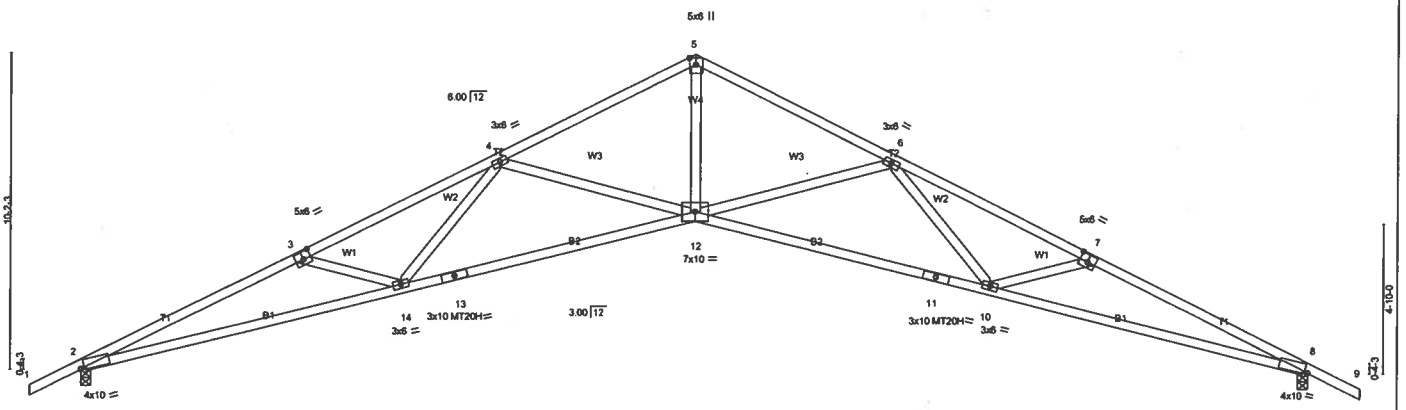
Dwg.#0130061178

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T22	SPECIAL	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:54 2006 Page 1

-1-8-0	7-0-15	13-4-8	19-8-0	25-11-8	32-3-1	39-4-0	41-0-0
1-8-0	7-0-15	6-3-8	6-3-8	6-3-8	6-3-8	7-0-15	1-8-0

Scale = 1:70.8



10-2-11	19-8-0	29-1-5	39-4-0
10-2-11	9-5-5	9-5-5	10-2-11

Plate Offsets (X,Y): [2:0-1-6,Edge], [3:0-3-0,0-3-0], [7:0-3-0,0-3-0], [8:0-1-6,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.74	Vert(LL)	-0.87	10-12	>541	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.99	Vert(TL)	-1.40	10-12	>335	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.92	Horz(TL)	0.94	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 183 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 4 SYP No.1D, T1 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D, B1 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

REACTIONS (lb/size) 2=1737/0-4-0, 8=1737/0-4-0
Max Horz 2=169(load case 5)
Max Uplift2=-637(load case 5), 8=-637(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/38, 2-3=-5590/2296, 3-4=-5198/2042, 4-5=-3686/1406, 5-6=-3686/1406, 6-7=-5198/2042, 7-8=-5590/2296, 8-9=0/38
BOT CHORD 2-14=-1931/5092, 13-14=-1398/4226, 12-13=-1395/4288, 11-12=-1395/4288, 10-11=-1398/4226, 8-10=-1931/5092
WEBS 3-14=-359/396, 4-14=-240/778, 4-12=-969/613, 5-12=-976/2879, 6-12=-969/613, 6-10=-240/778, 7-10=-359/396

JOINT STRESS INDEX
2 = 0.86, 3 = 0.72, 4 = 0.52, 5 = 0.71, 6 = 0.52, 7 = 0.72, 8 = 0.86, 10 = 0.50, 11 = 0.89, 12 = 0.81, 13 = 0.89 and 14 = 0.50

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All plates are MT20 plates unless otherwise indicated.
4) Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 637 lb uplift at joint 2 and 637 lb uplift at joint 8.

LOAD CASE(S) Standard

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

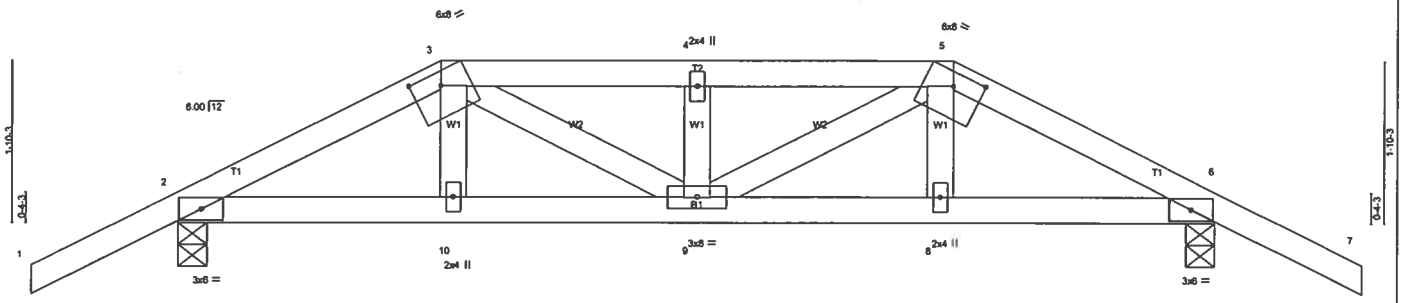
Dwg.#0130061179

Job	Truss	Truss Type	Qty	Ply	GREG TALLEY LOT 33 CANNON CREEK PLACE
L148527	T23	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 27 14:07:55 2006 Page 1

-1-8-0	3-0-0	5-11-0	8-10-0	11-10-0	13-6-0
1-8-0	3-0-0	2-11-0	2-11-0	3-0-0	1-8-0

Scale = 1:25.1



3-0-0	5-11-0	8-10-0	11-10-0
3-0-0	2-11-0	2-11-0	3-0-0

Plate Offsets (X,Y): [3:0-4-0,0-1-15], [5:0-4-0,0-1-15]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.20	Vert(LL)	-0.03	9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.21	Vert(TL)	-0.05	9	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.10	Horz(TL)	0.02	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 56 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=685/0-4-0, 6=685/0-4-0
Max Horz 2=-53(load case 5)
Max Uplift2=-298(load case 4), 6=-298(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-967/322, 3-4=-1094/399, 4-5=-1094/399, 5-6=-967/322, 6-7=0/39
BOT CHORD 2-10=-252/814, 9-10=-253/827, 8-9=-239/827, 6-8=-238/814
WEBS 3-10=-11/152, 3-9=-157/321, 4-9=-185/143, 5-9=-158/321, 5-8=-10/152

JOINT STRESS INDEX
2 = 0.48, 3 = 0.25, 4 = 0.08, 5 = 0.25, 6 = 0.48, 8 = 0.11, 9 = 0.30 and 10 = 0.11

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 298 lb uplift at joint 2 and 298 lb uplift at joint 6.
5) Girder carries hip end with 3-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 63 lb down and 32 lb up at 8-10-0, and 63 lb down and 32 lb up at 3-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-63(F=-9), 5-7=-54, 2-10=-30, 8-10=-35(F=-5), 6-8=-30
Concentrated Loads (lb)
Vert: 10=-63(F) 8=-63(F)

JANUARY 30, 2006 TRUSS DESIGN ENGINEER:
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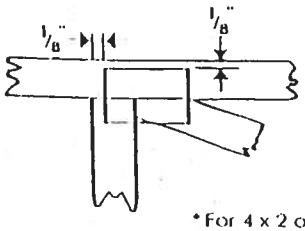
Symbols

PLATE LOCATION AND ORIENTATION

* Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seal.



* For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

* This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

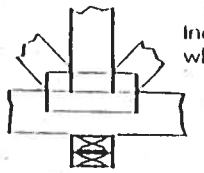
4 X 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

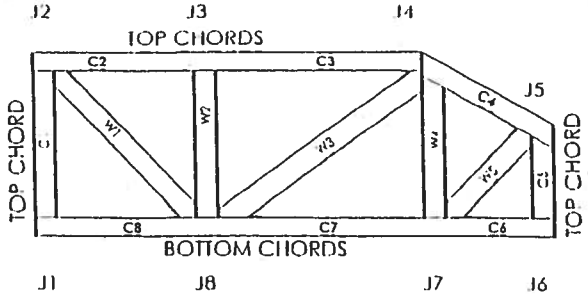
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

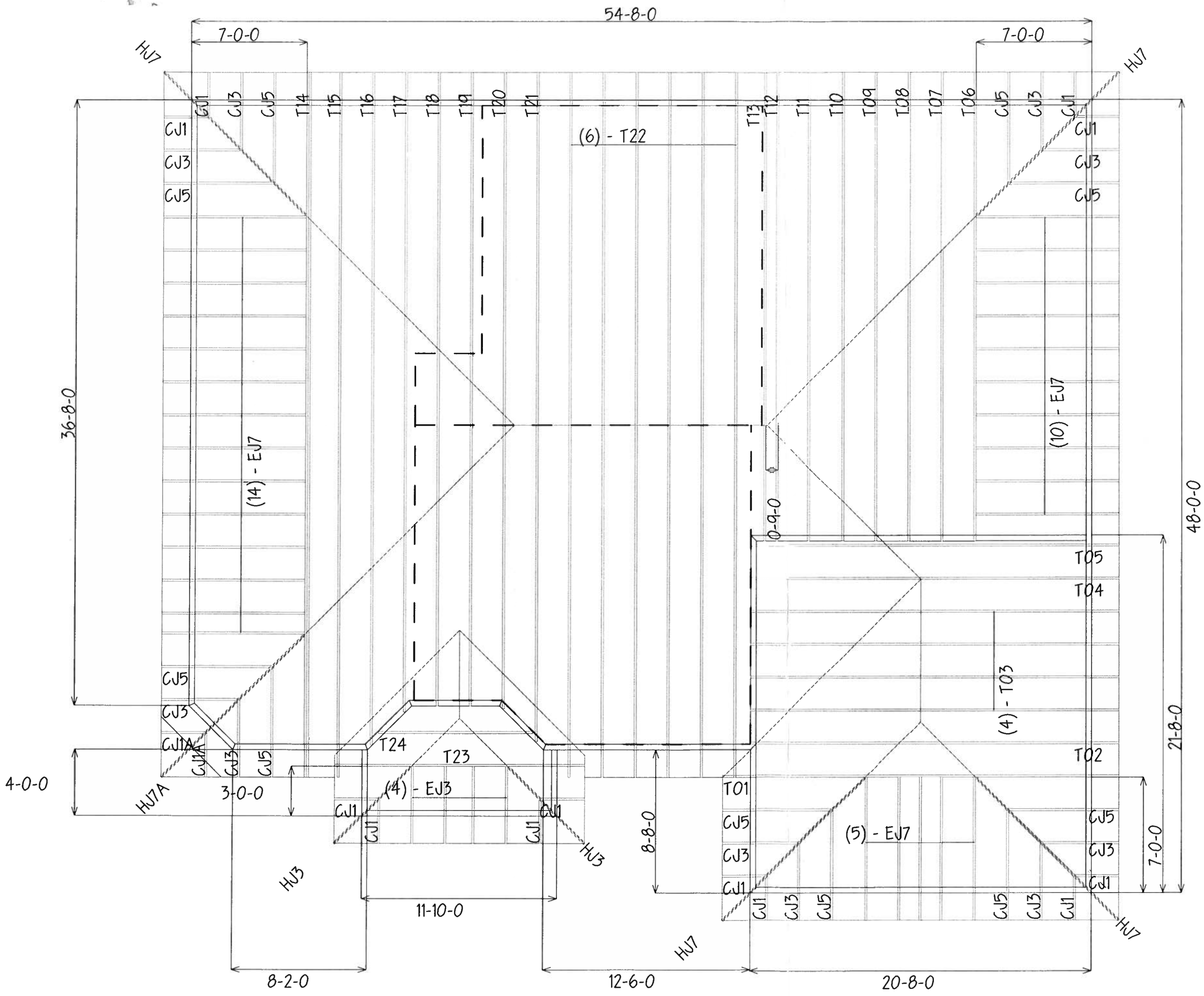
BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
HER	561

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transfering connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.



BEARING HEIGHT SCHEDULE

8'

1'-8" O/H
6/12

NOTES:

1.) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

2.) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DEGKED OR REFER TO DETAIL YIOS FOR ALTERNATE BRACING REQUIREMENTS.

3.) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4.) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.

5.) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6.) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7.) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8.) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date: _____

Approved by: _____ Date: _____

Builders FirstSource

Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994

Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973

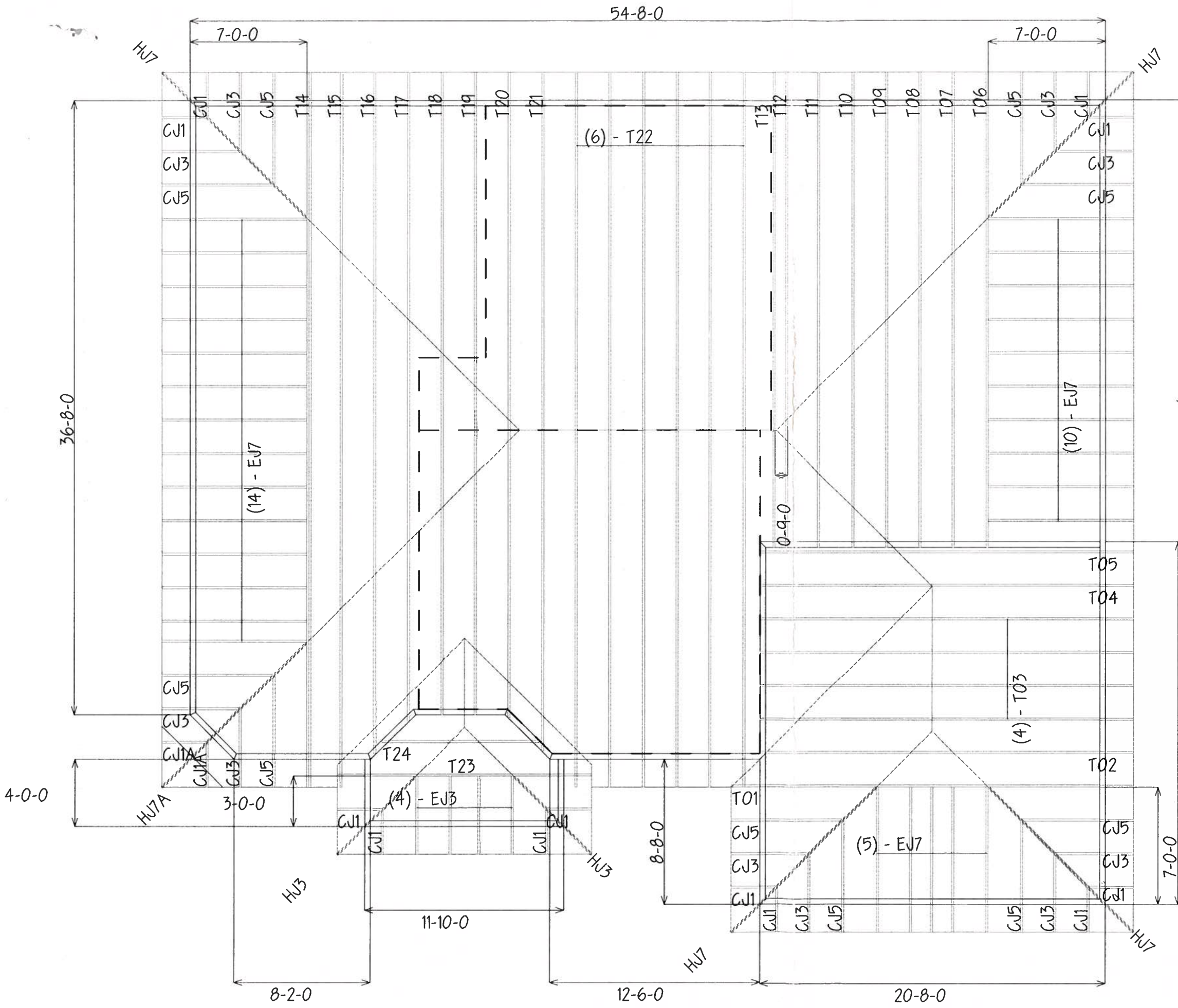
Lake City
PHONE: 904-755-6894 FAX: 904-755-7973

Sanford
PHONE: 407-322-0059 FAX: 407-322-5593

BUILDER: GREG TALLEY

LEGAL ADDRESS: LOT 33 CANNON CREEK PLACE

MODEL:	REVISION:
CUSTOM	SCALE: NTS
DATE: 2/1/06	DRAWN BY: WWL
	JOB #: L148527



Typical Truss Strap:
Simpson H2.5A
(THIS IS STANDARD FOR ALL LOCATIONS EXCEPT AS NOTED.)

TRUSS:
T14 = Simpson = LGT2
T06 = " "

DATED: 2.13.06

BEARING HEIGHT SCHEDULE

8'

1'-8" O/H
6/12

NOTES:

1) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.
2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL VIO5 FOR ALTERNATE BRACING REQUIREMENTS.
3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.
4) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.
5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.
6) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.
7) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.
8) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date _____

Approved by _____ Date _____

Builders FirstSource
Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994
Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973
Lake City
PHONE: 904-755-6894 FAX: 904-755-7973
Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

BUILDER: GREG TALLEY
LEGAL ADDRESS: LOT 33 CANNON CREEK PLACE
MODEL: CUSTOM
REVISION:
SCALE: NTS
DATE: 2/1/06
DRAWN BY: WWL
JOB #: L148527

ELK Roofing Products Specifications – Tuscaloosa, AL

PRESTIQUE®
HIGH DEFINITION®

RAISED PROFILE®

Prestique Plus *High Definition* and Prestique Gallery Collection™

Product size	13¼"x 39⅜"
Exposure	5⅝"
Pieces/Bundle	16
Bundles/Square	4/98.5 sq.ft.
Squares/Pallet	11

50-year limited warranty period: 5-7**years non-prorated coverage for shingles and application labor with prorated coverage for remainder of limited warranty period, plus an option for transferability*. 5-year limited wind warranty*. Wind Coverage: standard 80 mph, extended 110 mph***

Prestique I *High Definition*

Product size	13¼"x 39⅜"
Exposure	5⅝"
Pieces/Bundle	16
Bundles/Square	4/98.5 sq.ft.
Squares/Pallet	14

40-year limited warranty period: 5-7**years non-prorated coverage for shingles and application labor with prorated coverage for remainder of limited warranty period, plus an option for transferability*. 5-year limited wind warranty*. Wind Coverage: standard 80 mph, extended 90 mph***

Prestique *High Definition*

Product size	13¼"x 38¾"
Exposure	5⅝"
Pieces/Bundle	22
Bundles/Square	3/100 sq.ft.
Squares/Pallet	16

30-year limited warranty period: 5-7**years non-prorated coverage for shingles and application labor with prorated coverage for remainder of limited warranty period, plus an option for transferability*. 5-year limited wind warranty*. Wind Coverage: standard 80 mph.

Raised Profile

Product size	13¼"x 38¾"
Exposure	5⅝"
Pieces/Bundle	22
Bundles/Square	3/100 sq.ft.
Squares/Pallet	16

30-year limited warranty period: 5-7**years non-prorated coverage for shingles and application labor with prorated coverage for remainder of limited warranty period, plus an option for transferability*. 5-year limited wind warranty*. Wind Coverage: standard 70 mph.

HIP AND RIDGE SHINGLES

Seal-A-Ridge® w/*FLX*™

Size: 12"x 12"
Exposure: 6⅝"
Pieces/Bundle: 45
Coverage: 4 Bundles = 100 linear feet

Vented RidgeCrest™ w/*FLX*™

Size: 13"x13¼"
Exposure: 9¼"
Pieces/Box: 26
Coverage: 5 boxes = 100 linear feet

Elk Starter Strip

52 Bundles/Pallet
18 Pallets/Truck
936 Bundles/Truck
19 Pieces/Bundle
1 Bundle = 120.33 linear feet

Available Colors (Check Availability): Antique Slate, Weatheredwood, Shakewood, Sablewood, Hickory, Barkwood, Forest Green, Wedgewood, Birchwood, Sandalwood.
Gallery Collection: Balsam Forest™, Weathered Sage™, Sienna Sunset™.

All Prestique, Raised Profile and Seal-A-Ridge, and Prestique Starter Strip roofing products contain sealant which activates with the sun's heat, bonding shingles into a wind and weather resistant cover that resists blow-offs and leaks.

Check for availability with built-in StainGuard® treatment to inhibit the discoloration of roofing granules caused by the growth of certain types of algae.

All Prestique and Raised Profile shingles meet UL® Wind Resistant (UL 997) and Class "A" Fire Ratings (UL 790); and ASTM Specifications D 3018, Type-I; D 3161, Type-I; E 108 and the requirements of ASTM D 3462.

All Prestique and Raised Profile shingles have approval from the Florida Building Code Commission, Metro-Dade County, ICBO, and Texas Department of Insurance.

***See actual limited warranty for conditions and limitations.**
**** Effective January 1, 2004, the seven year non-prorated Umbrella Coverage Period applies only when a full Elk Roof System is installed with the original installation of the Elk shingles, all in accordance with Elk's application instructions for such products. A full Elk roof system includes Elk Hip and Ridge shingles on all hips and ridges, Elk Starter Strip along all rake and eave edges, an Elk ventilation system, and Elk All-Climate Self-Adhering Underlayment in all valleys. Additionally, Elk All-Climate Self-Adhering Underlayment is required along the rake and eave edges of the roof in and north of the states of VA, KY, MO, KS, CO, UT, NV, & OR.**
*****For a limited Wind Warranty up to 110 mph for Prestique Gallery Collection, Prestique Plus, or 90 mph for Prestique I or Grandé, at least six (6) properly placed NAILS and Elk Starter Strip shingles are required. See application instructions printed on the shingle wrapper for additional requirements.**

Specifications

Scope: Work includes furnishing all labor, materials and equipment necessary to complete installation of (name) shingles specified herein. Color shall be (name of color). Hip and ridge type to be Elk Seal-A-Ridge with formula *FLX*.

All exposed metal surfaces (flashing, vents, etc.) to be painted with matching Elk roof accessory paint.

Preparation of Roof Deck: Roof deck to be dry, well-seasoned 1" x 6" (25.4mm x 152.4mm) boards; exterior-grade plywood (exposure 1 rated sheathing) at least 3/8" (9.525mm) thick conforming to the specifications of the American Plywood Association; 7/16" (11.074mm) oriented strandboard; or chipboard. Most fire retardant plywood decks are NOT approved substrates for Elk shingles. Consult Elk Field Service for application specifications over other decks and other slopes.

Materials: Underlayment for standard roof slopes, 4" per foot (101.6/304.8mm) or greater: apply non-perforated No. 15 or 30 asphalt-saturated felt underlayment. For Low slopes[4" per foot (101.6/304.8mm) to a minimum of 2" per foot (50.8/304.8mm)], use two plies of underlayment overlapped a minimum of 19". Fasteners shall be of sufficient length and holding power for securing material as required by the application instructions printed on shingle wrapper.

For areas where algae is a problem, shingles shall be (name) with StainGuard treatment, as manufactured by the Elk Tuscaloosa plant. Hip and ridge type to be Seal-A-Ridge with formula *FLX* with StainGuard treatment.

Complete application instructions are published by Elk and printed on the back of every shingle bundle. All warranties are contingent upon the correct installation as shown on the instructions. These instructions are the minimum required to meet Elk application requirements. In some areas, building codes may require additional application techniques or methods beyond our instructions. In these cases, the local code must be followed. Under no circumstances will Elk accept application requirements less than those contained in its application instructions.

For specifications in CSI format, call 800.354.SPEC (7732) or e-mail specinfo@elkcorp.com.

Southeast & Atlantic Office:
800.945.5551

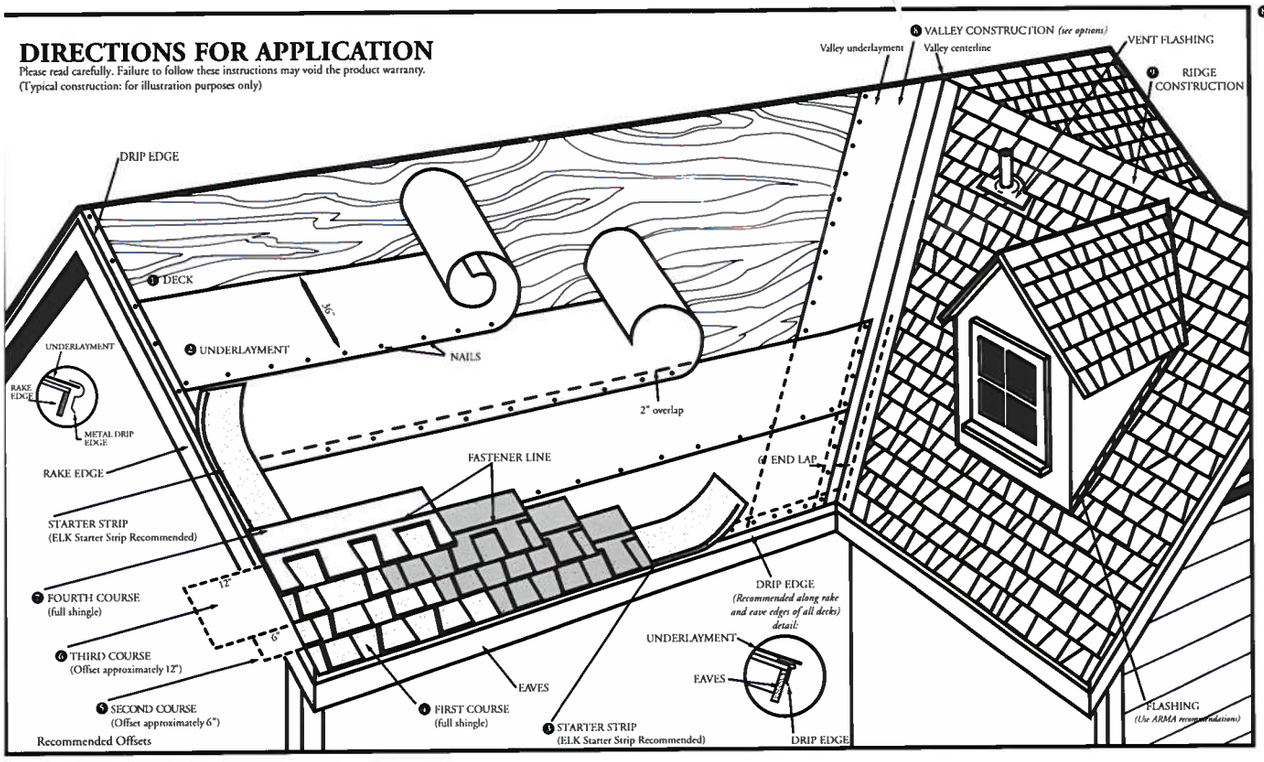
Corporate Headquarters:
800.354.7732

Plant Location:
800.945.5545

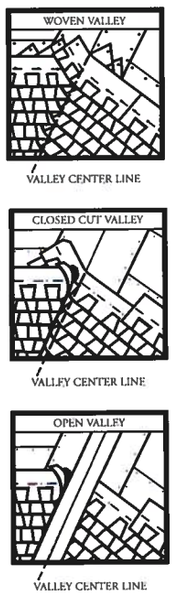
ELK The Premium Choice®
www.elkcorp.com

DIRECTIONS FOR APPLICATION

Please read carefully. Failure to follow these instructions may void the product warranty.
(Typical construction: for illustration purposes only)



8 VALLEY CONSTRUCTION OPTION
(California Open and California Closed are also acceptable valleys.)



NOTE: For complete ARMA valley installation details, see ARMA roofing installation guide.

DIRECTIONS FOR APPLICATION

These application instructions are the minimum required to meet Elk's application requirements. Your failure to follow these instructions may void the product warranty. In some areas, the building codes may require additional application techniques or methods beyond our instructions. In these cases, the local code must be followed. Under no circumstances will Elk accept application requirements that are less than those printed here. Shingles should not be jammed tightly together. All attics should be properly ventilated. Note: It is not necessary to remove tape on back of shingle.

1 DECK PREPARATION

Roof decks should be dry, well-seasoned 1" x 6" boards or exterior grade plywood minimum 3/8" thick and conform to the specifications of the American Plywood Association or 7/16" oriented strandboard, or 7/16" chipboard.

2 UNDERLAYMENT

Apply underlayment (Non-Perforated No. 15 or 30 asphalt saturated felt). Elk Versashield® or self adhering underlayment is also acceptable. Cover drip edge at eaves only.

For low slope(2/12 up to 4/12), completely cover the deck with two plies of underlayment overlapping a minimum of 19". Begin by fastening a 19" wide strip of underlayment placed along the eaves. Place a full 36" wide sheet over the starter, horizontally placed along the eaves and completely overlapping the starter strip.

EAVE FLASHING FOR ICE DAMS (ASK A ROOFING CONTRACTOR, REFER TO ARMA MANUAL OR CHECK LOCAL CODES)

For standard slope (4/12 to less than 21/12), use coated roll roofing of no less than 50 pounds over the felt underlayment extending from the eave edge to a point at least 24" beyond the inside wall of the living space below or one layer of a self-adhered eave and flashing membrane.

For low slope (2/12 up to 4/12), use a continuous layer of asphalt plastic cement between the two plies of underlayment from the eave edge up roof to a point at least 24" beyond the inside wall of the living space below or one layer of a self-adhered eave and flashing membrane.

Consult the Elk Technical Services Department for application specifications over other decks and other slopes.

3 STARTER SHINGLE COURSE

USE AN ELK STARTER STRIP OR THE HEADLAP OF A STRIP SHINGLE WITH THE ADHESIVE STRIP POSITIONED AT THE EAVE EDGE. With at least 3" trimmed from the end of the first shingle, start at the rake edge overhanging the eave and rake edges 1/2" to 3/4". Fasten 2" from the lower edge and 1" from each side.

4 FIRST COURSE

Start at rake and continue course with full shingles laid flush with the starter course. Shingles may be applied with a course alignment of 45° on the roof

5 SECOND COURSE

Offset the second course of shingles with respect to the first by approximately 6". Other offsets are approved if greater than 4".

6 THIRD COURSE

Offset the next course by 6" with respect to the second course, or consistent with the original offset.

7 FOURTH COURSE

Start at the rake and continue with full shingles across roof.

FIFTH AND SUCCEEDING COURSES.

Repeat application as shown for second, third, and fourth courses. Do not rack shingles straight up the roof. Offsets may be adjusted around valleys and penetrations.

8 VALLEY CONSTRUCTION

Open, woven and closed cut valleys are acceptable when applied by Asphalt Roofing Manufacturing Association (ARMA) recommended procedures. For metal valleys, use 36" wide vertical underlayment prior to applying metal flashing (secure edge with nails). No nails are to be within 6" of valley center.

9 RIDGE CONSTRUCTION

For ridge construction Elk recommends Class "A" Z®Ridge or Seal-A-Ridge® with formula *FLX*™ or RidgeCrest™ with *FLX* (See ridge package for installation instructions). Vented RidgeCrest or 3-tab shingles are also approved.

FASTENERS

While nailing is the preferred method for Elk shingles, Elk will accept fastening methods according to the following instructions.

Using the fastener line as a reference, nail or staple the shingle in the double thickness common bond area. For shingles without a fastener line, nails or staples must be placed between and/or in the sealant dots.

NAILS: Corrosive resistant, 3/8" head, minimum 12-gauge roofing nails. Elk recommends 1-1/4" for new roofs and 1-1/2" for roof-overs. In cases where you are applying shingles to a roof that has an exposed overhang, for new roofs only, 3/4" ring shank nails are allowed to be used from the eave's edge to a point up the roof that is past the outside wall line. 1" ring shank nails allowed for re-roof.

STAPLES: Corrosive resistant, 16-gauge minimum, crown width minimum of 15/16". Note: An improperly adjusted staple gun can result in raised staples that can cause a fish-mouthed appearance and can prevent sealing.

Fasteners should be long enough to obtain 3/4" deck penetration or penetration through deck, whichever is less. This product meets the requirements of the IRC 2003 code when fastened with 4 nails.

MANSARD APPLICATIONS

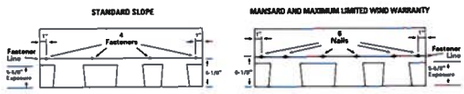
Correct fastening is critical to the performance of the roof. For slopes exceeding 60° (or 21/12) use six fasteners per shingle. Locate fasteners in the fastener area 1" from each side edge with the remaining four fasteners equally spaced along the length of the double thickness (laminated) area. Only fastening methods according to the above instructions are acceptable.

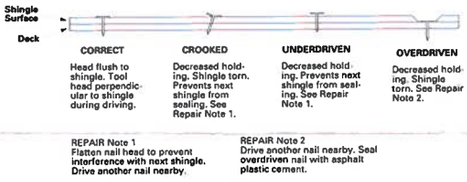
LIMITED WIND WARRANTY

- For a Limited Wind Warranty, all Prestique and Raised Profile™ shingles must be applied with 4 properly placed fasteners, or in the case of mansard applications, 6 properly placed fasteners per shingle.
- * For a Limited Wind Warranty up to 110 MPH for Prestique Gallery Collection or Prestique Plus or 90 MPH for Prestique I, shingles must be applied with 6 properly placed NAILS per shingle. SHINGLES APPLIED WITH STAPLES WILL NOT QUALIFY FOR THIS ENHANCED LIMITED WIND WARRANTY. Also, Elk Starter Strip shingles must be applied at the eaves and rake edges to qualify Prestique Plus, Prestique Gallery Collection and Prestique I shingles for this enhanced Limited Wind Warranty. Under no circumstances should the Elk Shingles or the Elk Starter Strip overhang the eaves or rake edge more than 3/4 of an inch.

HELP STOP BLOW-OFFS AND CALL-BACKS

A minimum of four fasteners must be driven into the DOUBLE THICKNESS (laminated) area of the shingle. Nails or staples must be placed along – and through – the "fastener line" or on products without fastener lines, nail or staple between and in line with sealant dots. CAUTION: Do not use fastener line for shingle alignment.





Refer to local codes which in some areas may require specific application techniques beyond those Elk has specified.
All Prestique and Raised Profile shingles have a U.L.® Wind Resistance Rating when applied in accordance with these instructions using nails or staples on re-roofs as well as new construction.

CAUTION TO WHOLESALER: Careless and improper storage or handling can harm fiberglass shingles. Keep these shingles completely covered, dry, reasonably cool, and protected from the weather. Do not store near various sources of heat. Do not store in direct sunlight until applied. DO NOT DOUBLE STACK. Systematically rotate all stock so that the material that has been stored the longest will be the first to be moved out.

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