Office copy

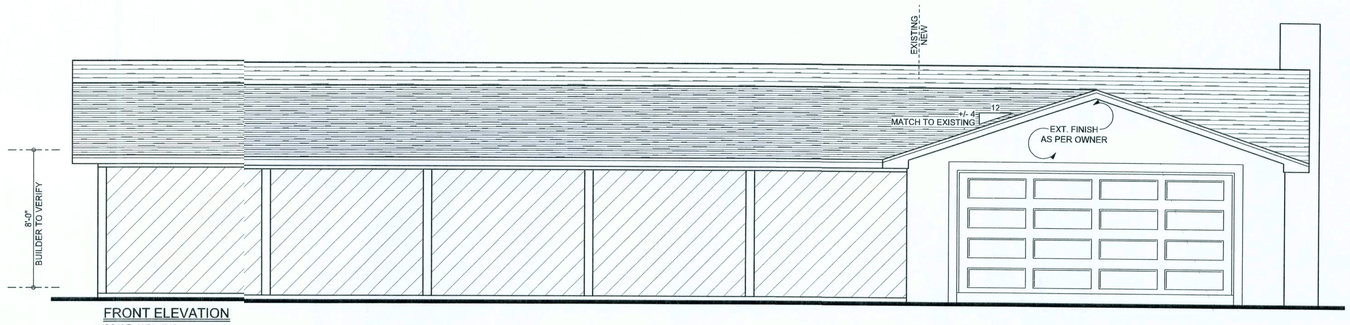
REQUIRED ROOF VENTILATION:
AS PER FLORIDA BUILDING CODE 2309.7

RIDGE VENT
MIN. 50% TOTAL VENT AREA
LOCATED IN THE UPPER PORTION OF ATTIC (MIN. 3' ABOVE EAVE)
3383 S.F. / 300 x 50% ÷ 5.7 S.F. RIDGE VENT AREA REQUIRED
52 FEET OF RIDGE VENT REQUIRED

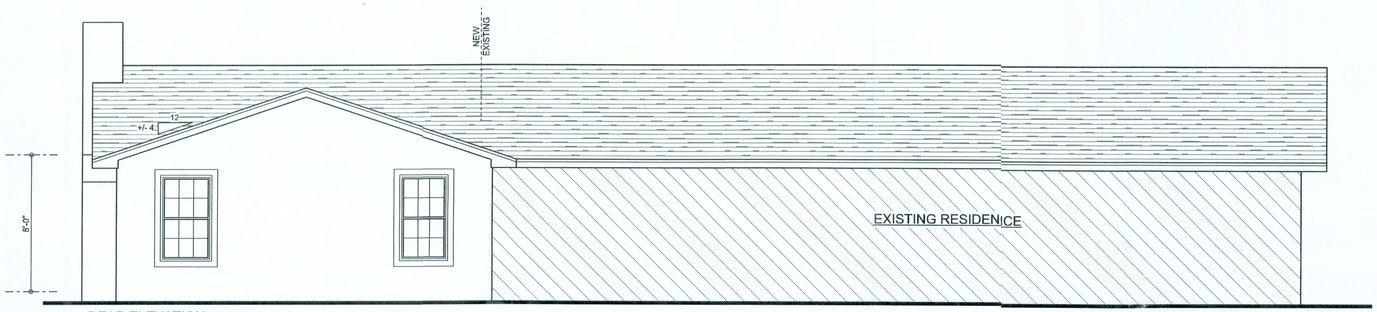
SOFFIT VENT
3383 S.F. / 300 x 50% ÷ 5.7 S.F. SOFFIT VENT AREA REQUIRED
190 FEET OF SOFFIT VENT REQUIRED

BUILDER MUST VERIFY THE FOLLOWING MINIMUM NET FREE VENT AREAS:

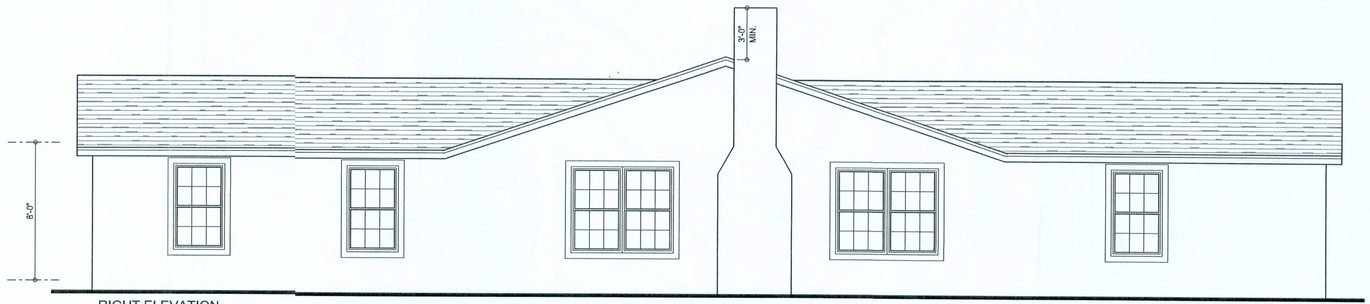
1. RIDGE VENTS = 16 N2/FT (.11 FT2/FT)
2. OFF-RIDGE VENTS = .70 FT2 PER 4' UNIT
3. SOFFIT VENTS = 4.:IN2/FT (.03 FT2/FT)



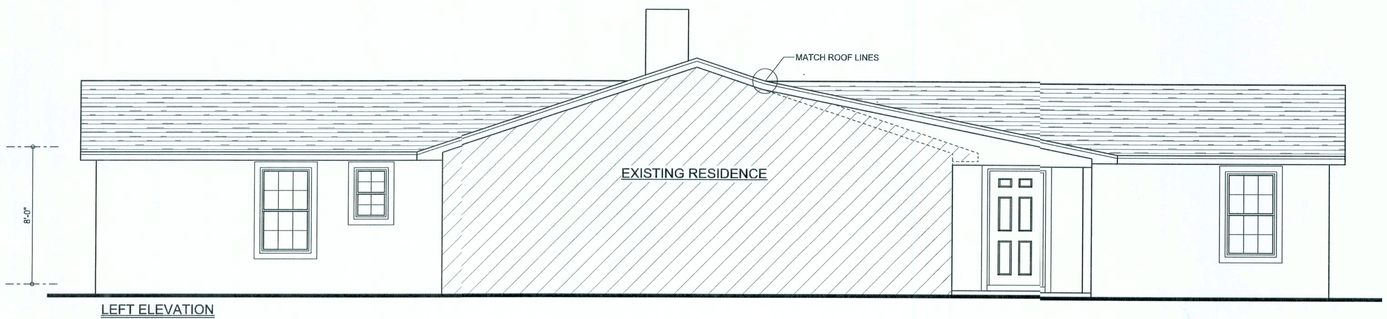
FRONT ELEVATION
SCALE: 1/4" = 1'-0"



REAR ELEVATION
SCALE: 1/4" = 1'-0"



RIGHT ELEVATION
SCALE: 1/4" = 1'-0"



LEFT ELEVATION
SCALE: 1/4" = 1'-0"

REVISIONS	

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY
P.E. 53915

SEAL

Damian Rivera

addition

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PRINTED DATE:
August 29, 2006

DRAWN BY:	STRUCTURAL BY:
Evan Beamsley	Evan Beamsley

FINALS DATE:
Aug 29, 2006

JOB NUMBER:
605262

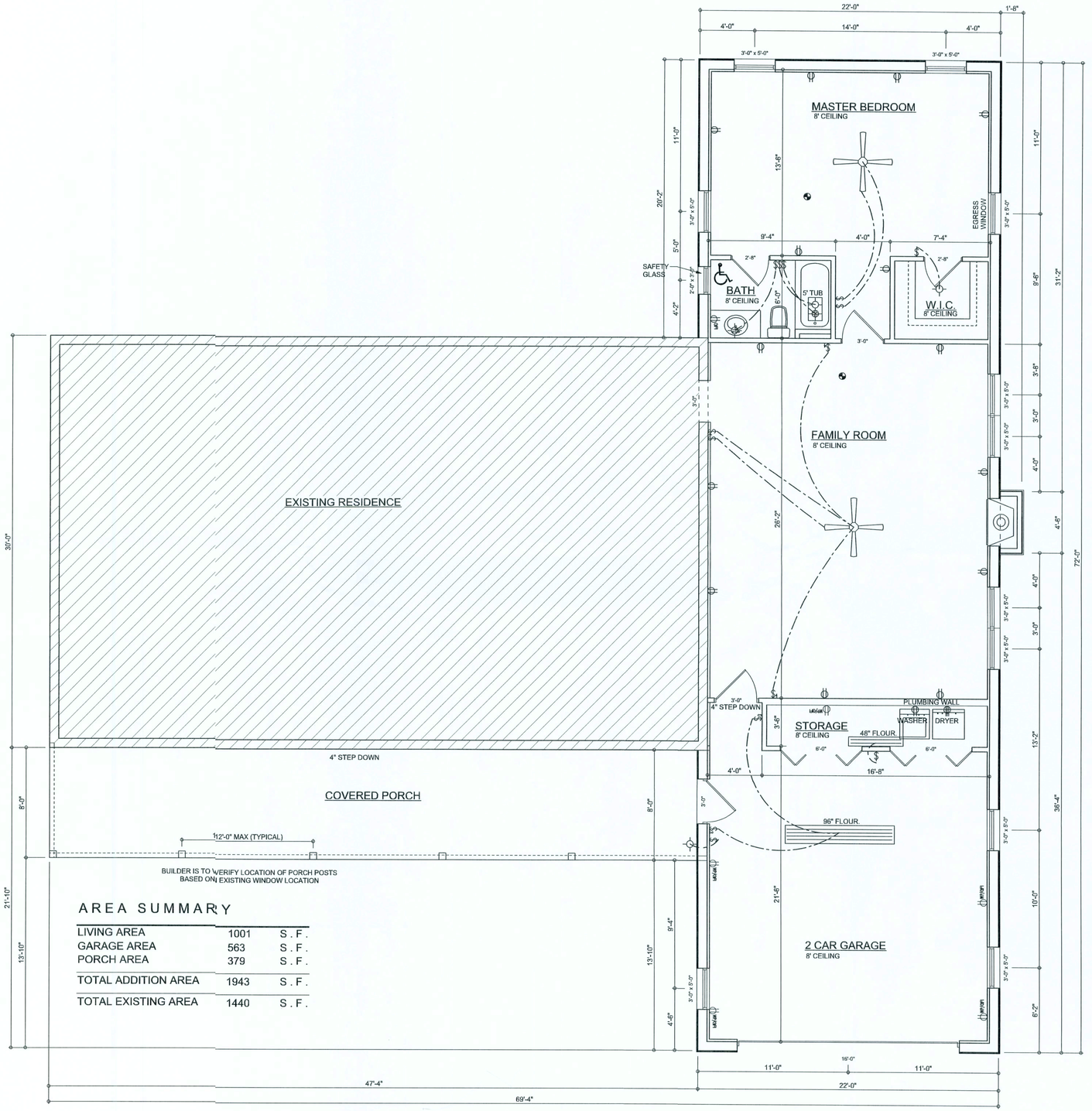
DRAWING NUMBER
A-1

OF 5 SHEETS

Z#137

REVISIONS	

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



FLOOR PLAN
SCALE: 1/4" = 1'-0"

AREA SUMMARY

LIVING AREA	1001	S . F .
GARAGE AREA	563	S . F .
PORCH AREA	379	S . F .
TOTAL ADDITION AREA	1943	S . F .
TOTAL EXISTING AREA	1440	S . F .

ELECTRICAL PLAN NOTES

E -1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.

E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.

E -3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.

E -4 ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.

E -5 TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.

E -6 ELECTRICAL CONT'R SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.

E -7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.

E -8 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)

E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION

E -10 A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC CONDUCTORS ENTER THE BUILDING. SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL APPROVAL OF THE BUILDING OFFICIAL

	ELECTRICAL LEGEND
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	2X4 FLUORESCENT LIGHT FIXTURE
	RECESSED CAN LIGHT
	BATH EXAUST FAN WITH LIGHT
	BATH EXAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET
	220v OUTLET
GFCI	GFI DUPLEX OUTLET
	SMOKE DETECTOR
$	WALL SWITCH
$3	3 WAY WALL SWITCH
$4	4 WAY WALL SWITCH
WP/GFI	WATER PROOF GFI OUTLET
	PHONE JACK
TV	TELEVISION JACK
GD	GARAGE DOOR OPENER
	WALL HEATER

WINDLOAD ENGINEER: Mark Disosway, RE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY
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SEAL

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STRUCTURAL BY: Evan Beamsley

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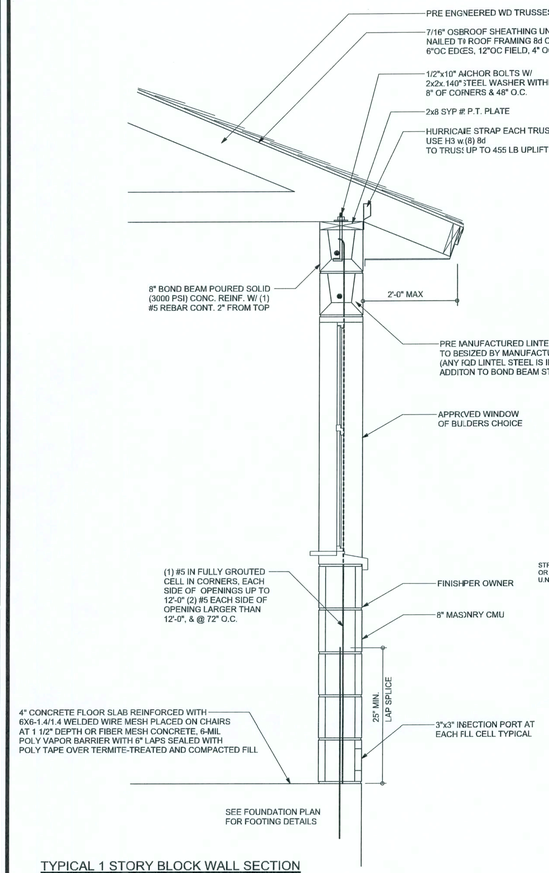
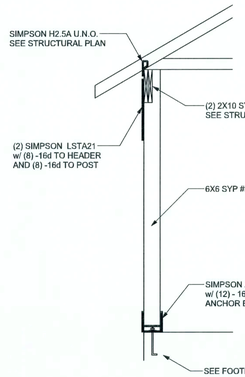
A2
OF 5 SHEETS



TYPICAL 1 STORY BLOCK WALL SECTION

SCALE: 1" = 1'-0"



TYPICAL PORCH POST DETAIL

SCALE: 1/2" = 1'-0"

MASONRY TRUSS ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

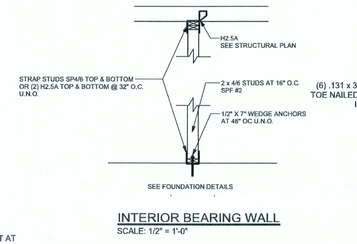
UPLIFT LBS.	TRUSS CONNECTOR MASONRY *
< 1205	TA22
< 1605	TA22
< 860	MTSM20
< 1175	HTSM20
< 1040	META20
< 1490	META20
< 1780	HETA20
< 1780	LGT2
< 2130	HHETA20
< 2310	HHETA24
< 3965	MGT
< 10980	HGT-2
< 10530	HGT-3

10-10d x 1 1/2"
11-10d
4 - 1/4"x2 1/4" TITEN IN BLOCK 7 - 10d IN TRUSS
4 - 1/4"x2 1/4" TITEN IN BLOCK 10 - 10d IN TRUSS
7-10d, 1 1/2"
10-10d, 1 1/2"
7-16d
7 - 1/4"x2 1/4" TITEN IN BLOCK 14 - 16d SIMKER IN GIRDER
17-10d, 1 1/2"
21-10d, 1 1/2"
22-10d TO TRUSS 5/8 AB TO WALL 15" EMBEDMENT
16-10d TO TRUSS (2) 3/4 AB TO WALL 15" EMBEDMENT
16-10d TO TRUSS (2) 3/4 AB TO WALL 15" EMBEDMENT

ANCHOR TABLE

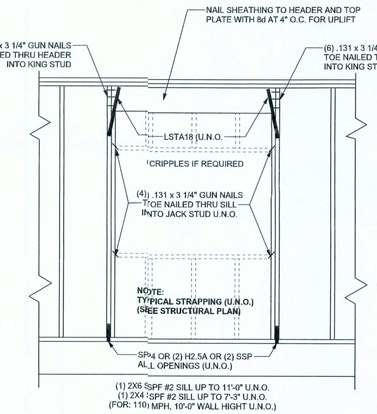
OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			**TO FOUNDATION**
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
		STUD STRAP CONNECTOR*			**TO STUDS**
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	**TO STUDS**		**TO FOUNDATION**
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB



INTERIOR BEARING WALL

SCALE: 1/2" = 1'-0"



TYPICAL HEADER STRAPING DETAIL

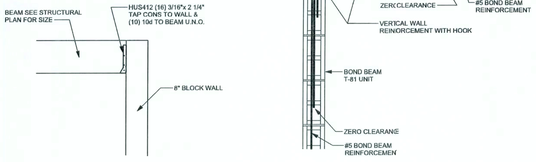
SCALE: 1/2" = 1'-0"

GRADE & SPECIES TABLE

		Fb (psi)	E (10^6 psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 10'-9" WALL HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" WALL HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10" WALL HEIGHT
(1) 2x6 @ 12" OC	TO 20.0' WALL HEIGHT

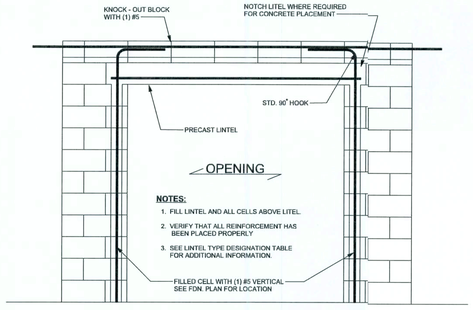


BEAM TO BLOCK CONNECTION DETAIL

SCALE: 1/2" = 1'-0"

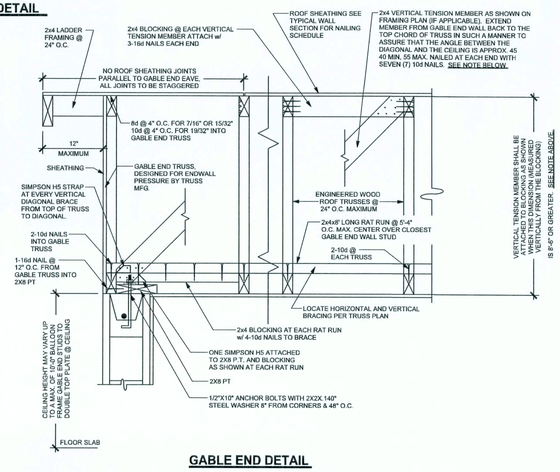
TYPICAL BOND BEAM CORNER DETAIL

SCALE: 1/2" = 1'=0"



TYPICAL FILLED LINTEL ASSEMBLY

SCALE: 1/2" = 1'=0"



GABLE END DETAIL

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, f'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI. WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-99, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3 SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 6d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

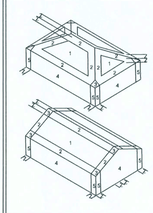
THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)
BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION
1.) BASIC WIND SPEED = 110 MPH
2.) WIND EXPOSURE = B
3.) WIND IMPORTANCE FACTOR = 1.0
4.) BUILDING CATEGORY = II
5.) ROOF ANGLE = 10-45 DEGREES
6.) MEAN ROOF HEIGHT = <30 FT
7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



Zone	Effective Wind Area (ft2)	
	10	100
1	19.9 -21.8	18.1 -18.1
2	19.8 -25.5	18.1 -21.8
2 Orig	-40.6	-40.6
3	19.8 -25.5	18.1 -21.8
3 Orig	-68.3	-42.4
4	21.8 -23.6	18.5 -20.4
5	21.8 -29.1	18.5 -22.6

Doors & Windows Worst Case (Zone 5, 10 ft2)	21.8	-29.1
8x7 Garage Door	19.5	-22.9
16x7 Garage Door	18.5	-21.0

DESIGN LOADS	
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)	
SOIL BEARING CAPACITY 1000PSF	
NOT IN FLOOD ZONE (BUILDER TO VERIFY)	

REVISIONS	

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915

29NOV06

SEAL

Damian Rivera

addition

ADDRESS: 835 SW Sherlock Terrace Lake City, FL, 32024

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Fax: (386) 269 - 4871

PRINTED DATE: August 29, 2006

DRAWN BY: Evan Beamsley

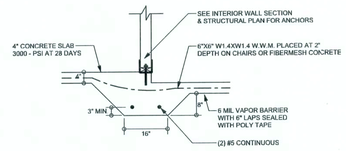
STRUCTURAL BY: Evan Beamsley

FINALS DATE: Aug 29, 2006

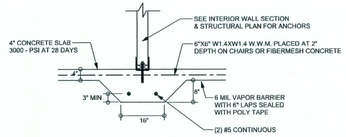
JOB NUMBER: 605262

DRAWING NUMBER S1

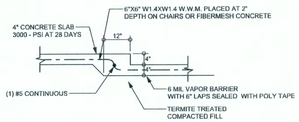
OF 5 SHEETS



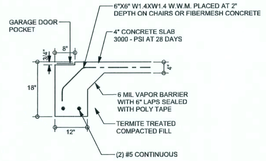
F3 S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



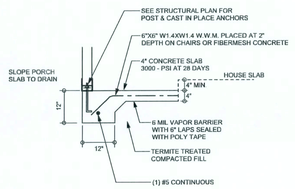
F2 S-2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



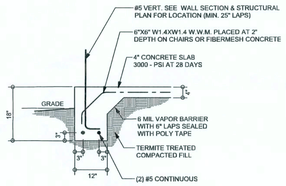
F6 S-2 TYPICAL NON - BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



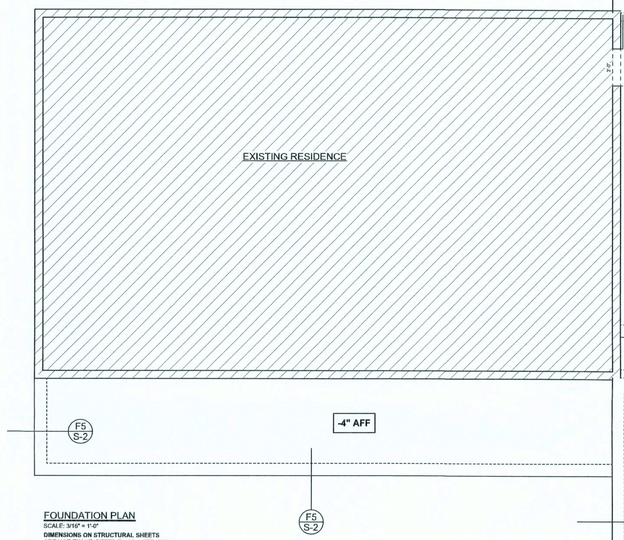
F4 S-2 GARAGE DOOR FOOTING
SCALE: 1/2" = 1'-0"



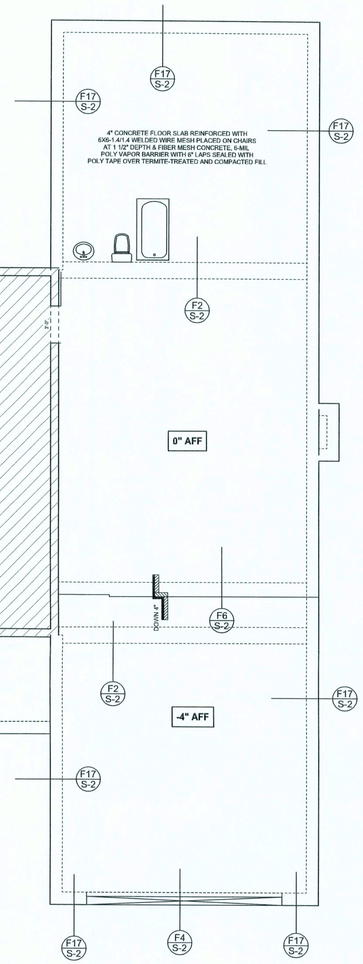
F5 S-2 PORCH FOOTING
SCALE: 1/2" = 1'-0"



F17 S-2 MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"



FOUNDATION PLAN
SCALE: 3/16" = 1'-0"
DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

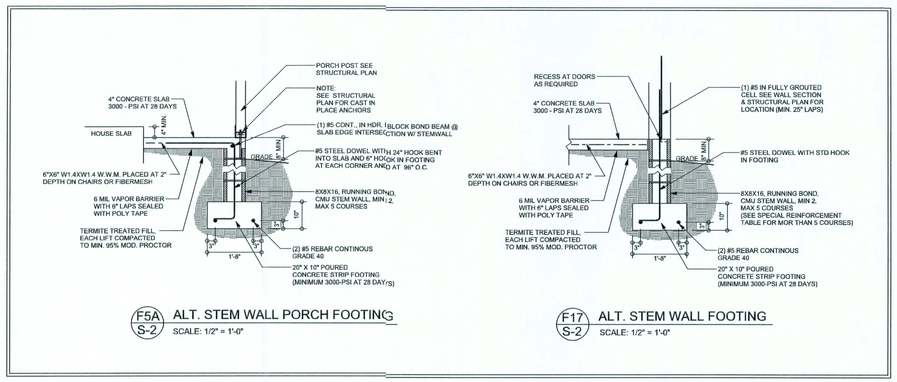


MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

ALT. STEM WALL FOUNDATION DETAILS



F5A S-2 ALT. STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"

F17 S-2 ALT. STEM WALL FOOTING
SCALE: 1/2" = 1'-0"

REVISIONS	

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, PCB 668, Lake City, FL 32056, 386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY
P.E. 53915

2/20/06

SEAL

Damian Rivera

addition

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PRINTED DATE:
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DRAWN BY: Evan Beamsley
STRUCTURAL BY: Evan Beamsley

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Aug 29, 2006

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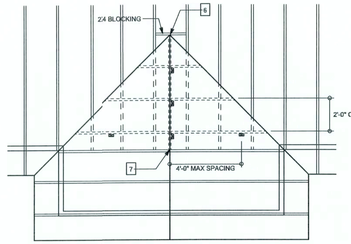
605262

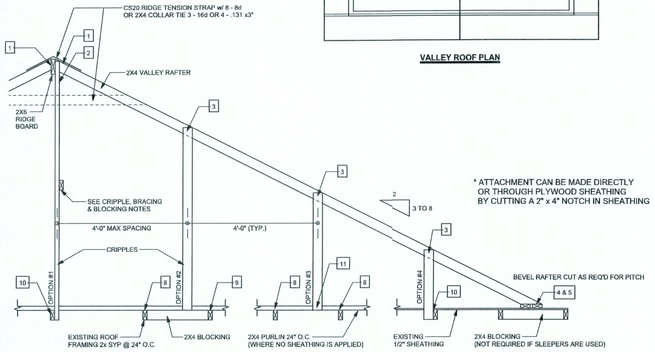
DRAWING NUMBER
S2
OF 5 SHEETS

LUMBER SIZE & GRADE MINUMUM REQUIREMENTS

RIDGE BOARD	2X6 SYP #2
RAFTER SPANS 20'-0" OR LESS	2X4 SYP #2
PURLINS / LATERAL BRACING	2X4 SPF #2
SLEEPERS	2X (WIDTH OF RAFTER SEAT CUT) SPF #3 OR 2 PARALLEL 2X4 SPF #3
CRIPPLES & BLOCKING	2X4 SPF #2 OR BETTER
TRUSS BELOW	SEE TRUSS DESIGN - SOUTHEREN PINE MATERIAL



VALLEY ROOF PLAN



* ATTACHMENT CAN BE MADE DIRECTLY OR THROUGH PLYWOOD SHEATHING BY CUTTING A 2" x 4" NOTCH IN SHEATHING

SECTION CUT PARALLEL TO VALLEY RAFTER

RETROFIT ROOFOVER FRAMING & BRACING DETAIL

SCALE: N.T.S

VALLEY ROOF PLAN MEMBER LEGEND

- TRUSS
- TRUSS UNDER VALLEY FRAMING
- VALLEY RAFTER OR RIDGE
- CRIPPLE

CRIPPLES 4'-0" O.C. FOR 29 psf (TL) AND 10 psf (TD) (TYP. SHINGLE ROOF) MAX

CONNECTION REQUIREMENT NOTES

1	2X4 RAFTERS TO RIDGE	3 - 16d OR 6 - 131 x 3" TOE NAILS
2	CRIPPLE TO RIDGE	3 - 16d OR 6 - 131 x 3" FACE NAILS
3	CRIPPLE TO RAFTERS	3 - 16d OR 6 - 131 x 3" FACE NAILS
4	RAFTER TO SLEEPER OR BLOCKING	6 -16d OR 12 - 131 x 3" TOE NAILS
5	SLEEPER TO TRUSS	4 - 16d OR 8 - 131 x 3" FACE NAILS EACH TRUSS
6	RIDGE BOARD TO ROOF BLOCK	3 -16d OR 6 - 131 x 3" TOE NAILS
7	RIDGE BOARD TO TRUSS	3 -16d OR 6 - 131 x 3" TOE NAILS
8	PURLIN TO TRUSS (TYP.)	3 -16d OR 6 - 131 x 3" NAILS
8	PURLIN TO TRUSS (IF CRIPPLE IS ATTACHED TO PURLIN)	4 -16d OR 8 - 131 x 3" NAILS
9	TRUSS TO BLOCKING	3 -16d OR 6 - 131 x 3" END NAILS
10	CRIPPLE TO TRUSS	3 -16d OR 6 - 131 x 3" FACE NAILS
11	CRIPPLE TO PURLIN	3 -16d OR 6 - 131 x 3" FACE NAILS

GENERAL NOTES

MAXIMUM RAFTER SPANS
6'-0" FOR 2X4, 9'-0" FOR 2X6 SPF #2 OR SYP #2.
MAXIMUM ROOF AREA PER SUPPORT
1682 IN ZONES 2 & 3 , 2452 IN ZONE 1, (EXAMPLE: 4'-0" O.C. X 4'-0" SPAN = 1682 OR 2'-0" X 8'-0" SPAN = 1682)
PURLINS REQUIRED 2'-0" O.C. IF EXISTING SHEATHING IS REMOVED.
PURLINS SHOULD OVERLAP SHEATHING ONE TRUSS SPACING MINIMUM. IN CASES THAT THIS IS IMPRACTICAL, OVERLAP SHEATHING A MINIMUM OF 6", AND NAIL UPWARDS THROUGH SHEATHING INTO PURLIN WITH A MINIMUM OF 8 - 8d COMMON WIRE NAILS.
THIS DRAWING APPLIES TO VALLEYS WITH THE FOLLOWING CONDITIONS:
- SPANS (DISTANCE BETWEEN HEELS) 42'-0" OR LESS
- MAXIMUM VALLEY HEIGHT: 14'-0" OR LESS
- MAXIMUM WIND SPEED: 120 MPH
- MAXIMUM MEAN ROOF HEIGHT: 30 FEET
- MAXIMUM TOTAL LOADING: 40 psf.
- MEETS FBC 2001/ASCE 7-98 WIND REQUIREMENTS
- EXPOSURE CATEGORY "B", I = 1.0, Kzt = 1.0
- ENCLOSED BUILDING

CRIPPLE, BRACING, & BLOCKING NOTES

-2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 5'-0" TO 10'-0" LONG NAILED W/ 2 - 10d NAILS OR 2X4 "T" OR SCAB BRACE NAILD TO FLAT EDGE OF CRIPPLE WITH 8d NAILS @ 8" O.C. "T" OR SCAB MUST BE 90% OF CRIPPLE LENGTH. CRIPPLES OVER 10'-0" LONG REQUIRE TWO CLB'S OR BOTH FACES w/ "T" OR SCAB. USE STRESS GRADED LUMBER & BOX OR COMMON NAILS.
- NARROW EDGE OF CRIPPLE CAN FACE RIDGE OR RAFTER, AS LONG AS THE PROPER NUMBER OF NAILS ARE INSTALLED INTO RIDGE BOARD
- INSTALL BLOCKING UNDER RAFTER IF SLEEPERS ARE NOT USED.
- INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FALL BETWEEN LOWER TRUSS TOP CHORDS AND LATERAL BRACING IS NOT USED.
- APPLY ALL NAILING IN ACCORDANCE TO NDS-1997 SECTION 12. NAILS ARE COMMON WIRE NAILS UNLESS NOTED OTHERWISE.

GENERAL NOTES

1. Provide full mortar bed and head joints.
2. Shore filled lintels as required.
3. Installation of lintel must comply with the architectural and/or structural documents.
4. U-Lintels are manufactured with 5 1/2" long notches at the ends to accomodate vertical cell reinforcing and grouting.
5. All lintels meet or exceed L/360 deflection, except lintels 17'-4" and longer with a nominal height of 8" meet or exceed L/180 deflection.
6. Bottom field added rebar to be located at the bottom of the lintel cavity.
7. 7/32" diameter wire stirrups are welded to the bottom steel for mechanical anchorage.
8. Cast-in-place concrete may be provided in composite lintel in lieu of concrete masonry units.

9. Safe load rating based on rational design analysis per ACI 318 and ACI 530
10. Product Approvals: Miami-Dade County, Florida No. 03-0606.05
11. The exterior surface of lintels installed in exterior concrete masonry walls shall have a coating of stucco applied in accordance with ASTM C-280 or other approved coating.
12. Lintels loaded simultaneously with vertical (gravity or uplift) and horizontal (lateral) loads should be checked for the combined loading with the following equation:

$$\frac{\text{Applied vertical load}}{\text{Safe vertical load}} + \frac{\text{Applied horizontal load}}{\text{Safe horizontal load}} \leq 1.0$$

13. Additional lateral load capacity can be obtained by the designer by providing additional reinforced concrete masonry above the lintel. See detail at right:

SAFE LOAD TABLE NOTES

1. All values based on minimum 4 inch nominal bearing.
Exception: Safe loads for unfilled lintels must be reduced by 20% if bearing length is less than 6 1/2 inches
2. N.R. = Not Rated
3. Safe loads are supeimposed allowable loads.
4. Safe loads based on grade 40 or grade 60 field rebar.
5. One #7 rebar may be substituted for two #5 rebars in 8" lintels only
6. The designer may evaluate concentrated loads from the safe load tables by calculating the maximum resisting moment and shear at d-away form face of support.

7. For composite lintel heights not shown, use safe load from net lower height shown.
8. For lintels lengths nt shown, use safe load from next longest lengh shown
9. All safe loads in unit of pounds per linear foot
10. All safe loads based on simply supported span.
11. The number in the he parenthesis indicates the percent eduction for grade 40 field added rebar.
Example 7'-9" lintel tye 8F32-1B safe gravity load = 6472#\0.0469;(15)H0.0781; w/ 15% reduction 6472 ⇒ (.85) = 5501 plf

MATERIALS

1. f'c 8" precast lintel =3500 psi
2. f'c prestressed lintel= 6000 psi
3. Grout per ASTM C4?6 f'c = 3000 psi w/ maximum 3/8 inch aggregate & 8 to 11 inch slump
4. Concrete Masonry Units (CMU) per ASTM C90 w/minimum net area compressive strength= 1900 psi
5. Rebar per ASTM A615 grade 60
6. Prestressing strand per ASTM A416 grade 270 low relaxation
7. Mortar per ASTM C-70 type M or S

SAFE GRAVITY LOADS FOR 8" PRECAST w/ 2" RECESS DOOR U-LINTELS

LENGTH \ TYPE		8RU6	8RF6-0B / 8RF6-1B	8RF10-0B / 8RF10-1B	8RF14-0B / 8RF14-1B	8RF18-0B / 8RF18-1B	8RF22-0B / 8RF22-1B	8RF26-0B / 8RF26-1B	8RF30-0B / 8RF30-1B
4'-4" (52")	PRECAST	1635	1749	3355	3280	4349	5421	6453	7567
			1891	3609	5206	6639	8060	9479	10863
4'-6" (54")	PRECAST	1494	1596	3063	2992	3958	4946	5924	6904
			1756	3699	5206	6639	8060	9479	10893
5'-8" (68")	PRECAST	866	920	1770	1716	2277	2830	3402	3966
			1167	2481	4567	6388	8060 (4#)	7917 (5#)	8311 (6#)
5'-10" (70")	PRECAST	810	859	1653	1600	2124	2649	3174	3700
			1113	2342	4242	6539 (6#)	8060 (8#)	7402 (8#)	8706 (8#)
6'-8" (80")	PRECAST	797	901	1825	3120	5048	7747	9448	7360
			901	1825	3120	5048	7915	9479	10893 (4#)
7'-6" (90")	PRECAST	669	755	1490	2459	3776	5743	7239	5623
			755	1490	2459	3776	5743	8998 (3#)	10893 (4#)
9'-8" (116")	PRECAST	411	466	999	1568	2253	3129	4091	3146
			526	999	1568	2253	3129	4150	5891 (4#)

SAFE UPLIFT LOADS FOR 8" PRECAST w/ 2" RECESS DOOR U-LINTELS

LENGTH \ TYPE		8RF6-1T / 8RF6-2T	8RF10-1T / 8RF10-2T	8RF14-1T / 8RF14-2T	8RF18-1T / 8RF18-2T	8RF22-1T / 8RF22-2T	8RF26-1T / 8RF26-2T	8RF30-1T / 8RF30-2T
4'-4" (52")	PRECAST		905	1748	2635	3522	4409	5296	6183
			905	1748	2635	3522	4409	5296	6183
4'-6" (54")	PRECAST		867	1675	2525	3374	4224	5074	5924
			867	1675	2525	3374	4224	5074	5924
5'-8" (68")	PRECAST		675	1301	1960	2618	3277	3935	4594
			675	1301	1960	2618	3277	3935	4594
5'-10" (70")	PRECAST		655	1262	1900	2538	3176	3815	4453
			655	1262	1900	2538	3176	3815	4453
6'-8" (80")	PRECAST		579	1012	1651	2204	2758	3312	3865
			579	1097	1651	2204	2758	3312	3865
7'-6" (90")	PRECAST		506	797	1462 (2#)	1952 (2#)	2442 (2#)	2931 (2#)	3257
			506	967	1462	1952	2442	2931	3421
9'-8" (116")	PRECAST		395	491	931 (1#)	1301 (1#)	1640 (2#)	1980 (2#)	2322 (2#)
			395	589	1135	1514	1893	2272	2652

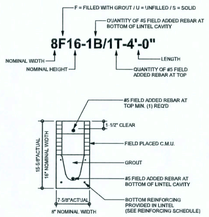
SAFE GRAVITY LOADS FOR 8" PRECAST & PRESTRESSED U-LINTELS

LENGTH \ TYPE		8UB	8F8-0B / 8F8-1B	8F12-0B / 8F12-1B	8F16-0B / 8F16-1B	8F20-0B / 8F20-1B	8F24-0B / 8F24-1B	8F28-0B / 8F28-1B	8F32-0B / 8F32-1B
2'-10" (34")	PRECAST	2231	3069	4605	6113	7547	8974	10394	11809
			3069	4605	6113	7547	8974	10394	11809
3'-6" (42")	PRECAST	2231	3069	4605	6113	7547	8974	10394	11809
			3069	3719	5163	6607	8054	9502	10951
4'-0" (48")	PRECAST	1966	3069	4605	6113	7547	8974	10394	11809
			2561	2751	3820	4890	5961	7034	8107
4'-6" (54")	PRECAST	1599	2693	4605	6113	7547	8974	10394	11809
			1969	2110	2931	3753	4576	5400	6224
5'-4" (64")	PRECAST	1217	2189	4375	6113	7547 (7)	8672	10294	11809
			1349	1438	1999	2560	3123	3686	4249
5'-10" (70")	PRECAST	1062	1663	3090	5365	7547 (9#)	7342 (1#)	8733 (1#)	10127 (1#)
			1105	1173	1631	2090	2549	3009	3470
6'-8" (78")	PRECAST	908	1451	2622	4360	7168 (4#)	6036 (1#)	7181 (1#)	8328 (2#)
			1238	2177	3480	3031	3707	4383	5061
7'-6" (90")	PRECAST	743	1238	2177	3480	5381	8360	10394 (2#)	8825 (1#)
			1011	1729	2632	2205	2698	3191	3685
9'-4" (112")	PRECAST	554	1011	1729	2661	3898	5681	8467 (4#)	6472 (1#)
			699	1160	1625	2564	3486	2818	3302
10'-6" (126")	PRECAST	475	752	1245	1843	2564	3486	4705 (1#)	6305 (1#)
			535	890	1247	2093	2777	2163	2539
11'-4" (136")	PRECAST	362	643	1052	1533	2093	2781	3643 (3#)	4754 (4#)
			582	945	1366	1846	2423	3127	4006
12'-0" (144")	PRECAST	337	582	945	1366	1846	2423	3127	4006
			540	873	1254	1684	2193	2805	3552
13'-4" (160")	PRECAST	296	540	873	1254	1684	2193	2805	3552
			471	755	1075	1428	1838	2316	2883
14'-0" (168")	PRECAST	279	471	755	1075	1428	1838	2316	2883
			424	706	1002	1326	1697	2127	2630
14'-8" (176")	PRESTRESSED	N.R.	442	706	1002	1326	1697	2127	2630
			NR	NR	NR	NR	NR	NR	NR
15'-4" (184")	PRESTRESSED	N.R.	458	783	1370	1902	2245	2517	2712
			NR	NR	NR	NR	NR	NR	NR
17'-4" (208")	PRESTRESSED	N.R.	412	710	1250	1733	2058	2320	2513
			NR	NR	NR	NR	NR	NR	NR
19'-4" (232")	PRESTRESSED	N.R.	300	536	956	1326	1609	1849	2047
			NR	NR	NR	NR	NR	NR	NR
21'-4" (256")	PRESTRESSED	N.R.	235	418	750	1037	1282	1515	1716
			NR	NR	NR	NR	NR	NR	NR
22'-0" (264")	PRESTRESSED	N.R.	180	340	598	845	1114	1359	1468
			NR	NR	NR	NR	NR	NR	NR
24'-0" (288")	PRESTRESSED	N.R.	165	315	550	784	1047	1285	1389
			NR	NR	NR	NR	NR	NR	NR
			129	250	450	654	884	1092	1222

SAFE UPLIFT LOADS FOR 8" PRECAST & PRESTRESSED U-LINTELS

LENGTH \ TYPE		8F8-1T / 8F8-2T	8F12-1T / 8F12-2T	8F16-1T / 8F16-2T	8F20-1T / 8F20-2T	8F24-1T / 8F24-2T	8F28-1T / 8F28-2T	8F32-1T / 8F32-2T
2'-10" (34")	PRECAST	1972	3173	4480	5747	7034	8321	9608
		1972	3173	4480	5747	7034	8321	9608
3'-6" (42")	PRECAST	1569	2524	3547	4569	5591	6613	7636
		1569	2524	3547	4569	5591	6613	7636
4'-0" (48")	PRECAST	1363	2192	3079	3966	4853	5740	6627
		1363	2192	3079	3966	4853	5740	6627
4'-6" (54")	PRECAST	1207	1940	2724	3508	4292	5077	5861
		1207	1940	2724	3508	4292	5077	5861
5'-4" (64")	PRECAST	1016	1632	2290	2949	3607	4265	4924
		1016	1632	2290	2949	3607	4265	4924
5'-10" (70")	PRECAST	909	1462	2053	2684	3295	3897	4498
		929	1492	2093	2684	3295	3897	4498
6'-6" (78")	PRECAST	835 (1#)	1340	1880	2419	2959	3498	4038
		835	1340	1880	2419	2959	3498	4038
7'-6" (90")	PRECAST	727 (2#)	1021	1834 (1#)	2102 (2#)	2571 (2#)	3039 (2#)	3508 (2#)
		727	1166	1634	2102	2571	3039	3508
9'-4" (112")	PRECAST	591	680	1133 (2#)	1471 (2#)	1811 (2#)	2152 (2#)	2494 (2#)
		591	851	1326	1705	2084	2463	2842
10'-6" (126")	PRECAST	530	552	914 (4#)	1185 (4#)	1458 (3#)	1732 (2#)	2007 (2#)
		530	686	1183	1526	1865	2204	2544
11'-4" (136")	PRECAST	474	485	798 (2#)	1034 (2#)	1272 (2#)	1510 (2#)	1749 (2#)
		494	599	1028	1422	1738	2053	2369
12'-0" (144")	PRECAST	470 (4#)	441	723 (4#)	936 (3#)	1151 (3#)	1366 (2#)	1582 (2#)
		470	543	928	1349	1649	1948	2247
13'-4" (160")	PRECAST	418 (3#)	373	606 (3#)	783 (3#)	962 (3#)	1141 (3#)	1321 (3#)
		428	455	770	1145	1444	1718	1993
14'-0" (168")	PRECAST	384 (4#)	346	559 (3#)	723 (3#)	887 (3#)	1052 (3#)	1218 (3#)
		410	420	709	1050	1434 (2#)	1694 (2#)	1954 (2#)
14'-8" (176")	PRESTRESSED	239	323	519 (2#)	671 (2#)	823 (2#)	976 (2#)	1129 (2#)
		246	390	655	968	1324 (2#)	1625 (2#)	1874 (2#)
15'-4" (184")	PRESTRESSED	224	302	485 (2#)	626 (1#)	767 (1#)	909 (1#)	1052 (1#)
		230	364	609	897	1224 (2#)	1502 (2#)	1801 (2#)
17'-4" (208")	PRESTRESSED	187	255	404 (2#)	520 (2#)	637 (2#)	754 (2#)	872 (1#)
		192	303	500	732	993 (2#)	1268 (2#)	1470 (2#)
19'-4" (232")	PRESTRESSED	162	222	347 (2#)	446 (1#)	546 (1#)	646 (1#)	746 (1#)
		166	261	424	616	831 (2#)	1057 (2#)	1225 (2#)
21'-4" (256")	PRESTRESSED	142	198	308 (2#)	393 (1#)	480 (1#)	567 (1#)	654 (1#)
		142	230	369	531	713 (2#)	903 (2#)	1046 (2#)
22'-0" (264")	PRESTRESSED	137	192	295 (2#)	378 (1#)	461 (1#)	545 (1#)	629 (1#)
		137	221	354	508	681 (2#)	861 (2#)	997 (1#)
24'-0" (288")	PRESTRESSED	124	175	267 (2#)	341 (2#)	416 (1#)	491 (1#)	566 (1#)
		124	200	316	450	600 (2#)	756 (2#)	875 (1#)

TYPE DESIGNATION



INDICATES LOCATION OF #5 VERTICAL DOWEL IN CMU CELL, FILLED SOLID W/ 3000 P.S.I. CONC.

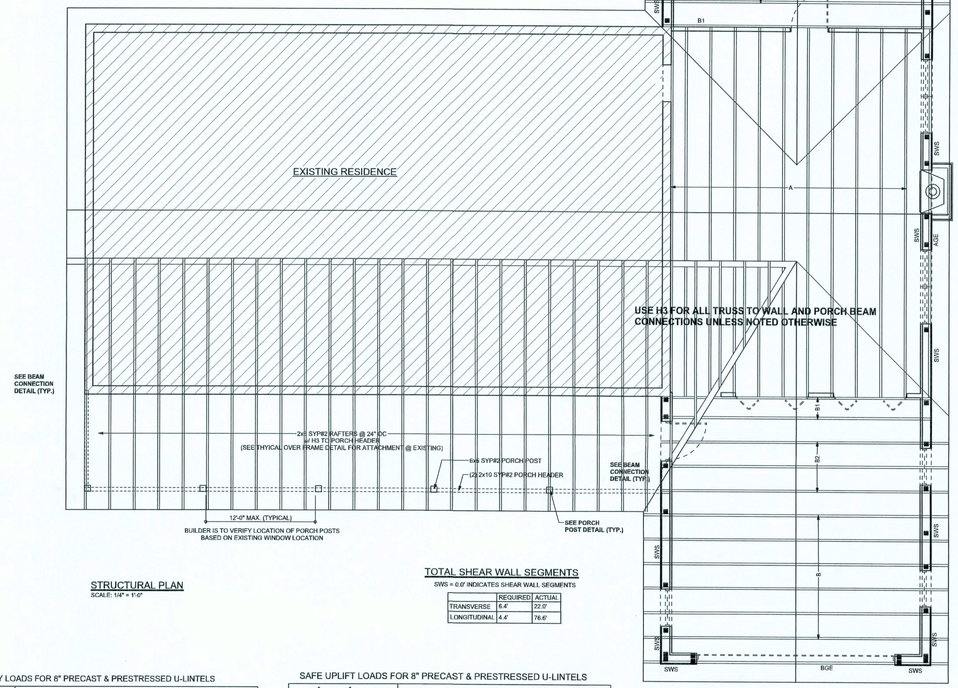
INDICATES LOCATION OF (2) #5 VERTICAL DOWEL IN CMU CELL, FILLED SOLID W/ 3000 P.S.I. CONC.

STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING WALLS & LINTELS SHALL BE A MINIMUM OF 8F16-1B/1T-0'-0"

SN-2 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

SN-3 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE



STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	6.4'	22.0'
LONGITUDINAL	4.4'	76.6'

DOOR & WINDOW BUCK ATTACHMENT

TAPCON IN FACE OF CMU
2 1/2"MIN. EDGE DISTANCE
1 1/4"MIN. EMBEDMENT
3" MIR. SPACING

WINDOWS & DOORS UP TO 6'X6'
3/16" "APCONS @ 2' O.C.
1/4" TAPCONS @ 3' O.C.

WINDOWS & DOORS UP TO 8'X12'
3/16" "APCONS @ 16" O.C.
1/4" TAPCONS @ 24" O.C.

SLIDERS UP TO 8'HX20'W
3/16" "APCONS @ 12" O.C.
1/4" TAPCONS @ 18" O.C.

GARAGE DOOR UP TO 10'W
(2) 3/8" TAPCONS & 16" O.C.
(2) 1/4" TAPCONS & 24" O.C.

GARAGE DOOR UP TO 18'W
(2) 3/8" TAPCONS & 8" O.C.
(2) 1/4" TAPCONS & 12" O.C.

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. ANDERSON TRUSS CO. (JOB #5295)

REVISIONS	

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disoway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY P.E. 53915

SEAL

Damian Rivera

addition

ADDRESS:
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PRINTED DATE: August 29, 2006

DRAWN BY: Evan Beamsley

STRUCTURAL BY: Evan Beamsley

FINALS DATE: Aug 29, 2006

JOB NUMBER: 605262

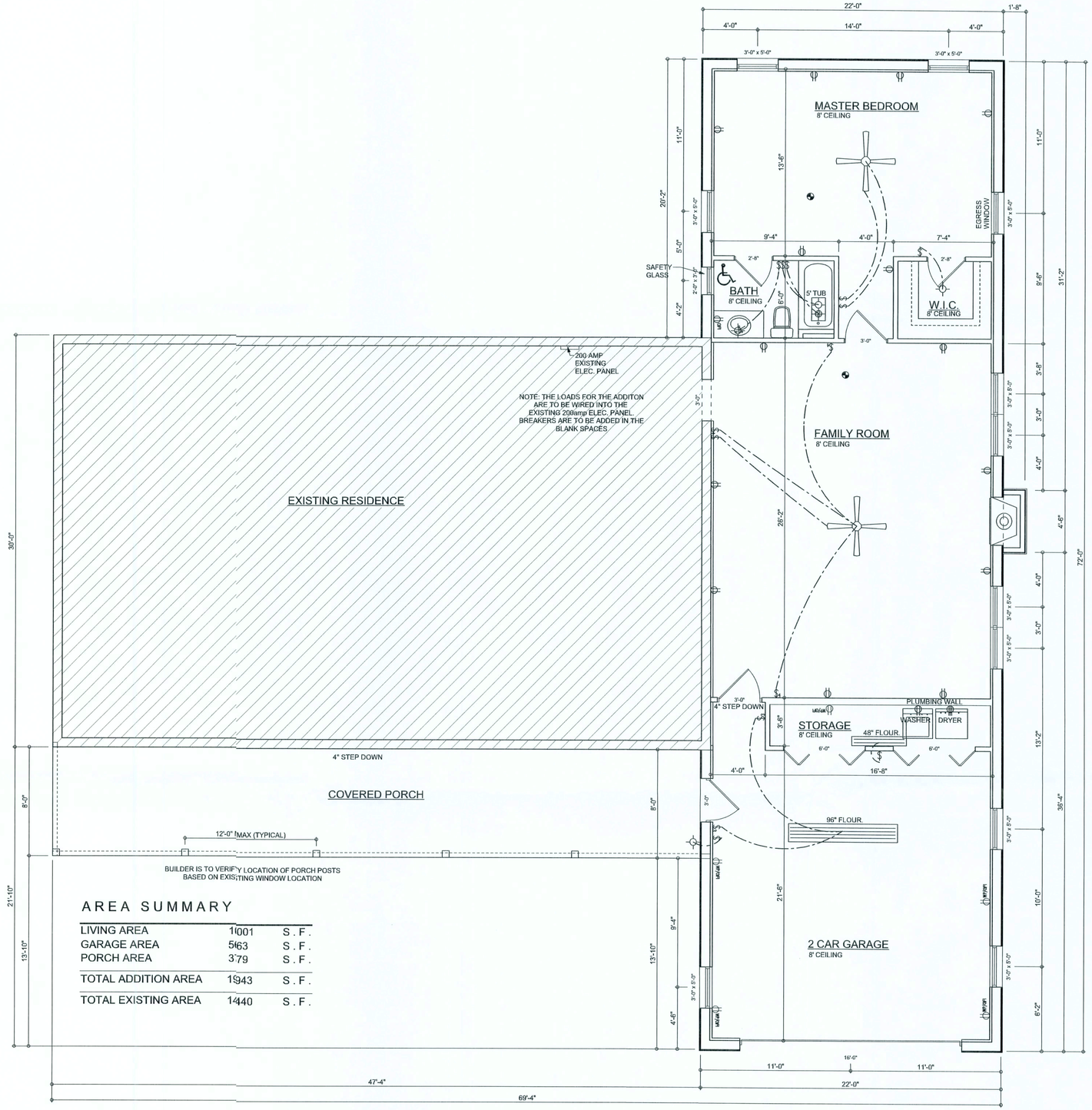
DRAWING NUMBER

S-3

OF 5 SHEETS

REVISIONS	

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE



FLOOR PLAN
SCALE: 1/4" = 1'-0"

ELECTRICAL PLAN NOTES

E -1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.

E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.

E -3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.

E -4 ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.

E -5 TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.

E -6 ELECTRICAL CONT'R SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.

E -7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.

E -8 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)

E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION

E -10 A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC CONDUCTORS ENTER THE BUILDING. SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL APPROVAL OF THE BUILDING OFFICIAL

	ELECTRICAL LEGEND
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	2X4 FLUORESCENT LIGHT FIXTURE
	RECESSED CAN LIGHT
	BATH EXAUST FAN WITH LIGHT
	BATH EXAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET
	220v OUTLET
WP/GFI	GFI DUPLEX OUTLET
	SMOKE DETECTOR
$	WALL SWITCH
$3	3 WAY WALL SWITCH
$4	4 WAY WALL SWITCH
WP/GFI	WATER PROOF GFI OUTLET
	PHONE JACK
TV	TELEVISION JACK
	GARAGE DOOR OPENER
	WALL HEATER

AREA SUMMARY

LIVING AREA	1001	S . F .
GARAGE AREA	563	S . F .
PORCH AREA	379	S . F .
TOTAL ADDITION AREA	1943	S . F .
TOTAL EXISTING AREA	1440	S . F .

WINDLOAD ENGINEER: Mark Disosway, PE No.59151, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building,at specified location.

MARK DISOSWAY P.E. 53915

10/06

SEAL

Damian Rivera

addition

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PRINTED DATE: October 12, 2006

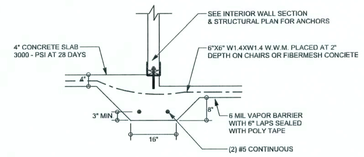
DRAWN BY: Evan Beamsley

STRUCTURAL BY: Evan Beamsley

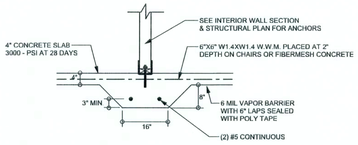
FINAIS DATE: Aug 29, 2006

JOB NUMBER: 605262

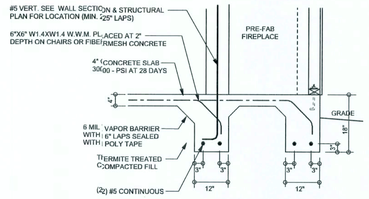
DRAWING NUMBER
A2
OF 5 SHEETS



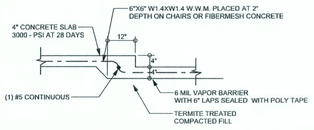
F3 S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



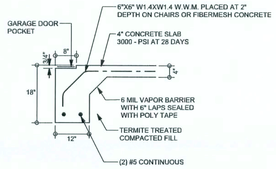
F2 S-2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



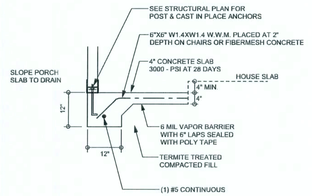
F19 S-2 TYP. FIREPLACE FOOTING
SCALE: 1/2" = 1'-0"



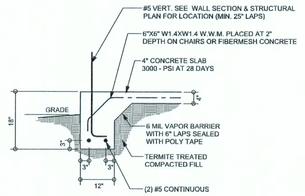
F6 S-2 TYPICAL NON - BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



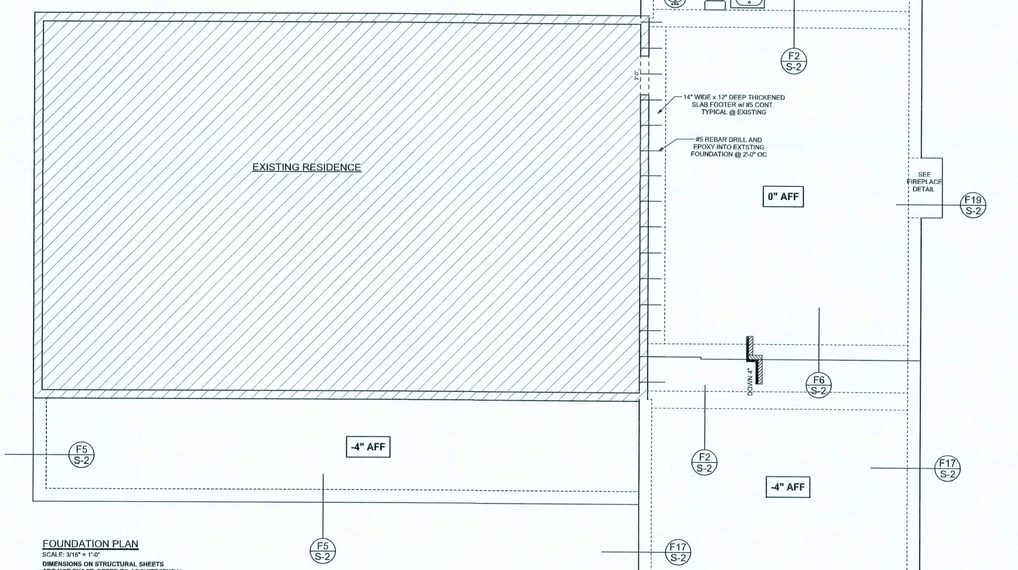
F4 S-2 GARAGE DOOR FOOTING
SCALE: 1/2" = 1'-0"



F5 S-2 PORCH FOOTING
SCALE: 1/2" = 1'-0"



F17 S-2 MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"

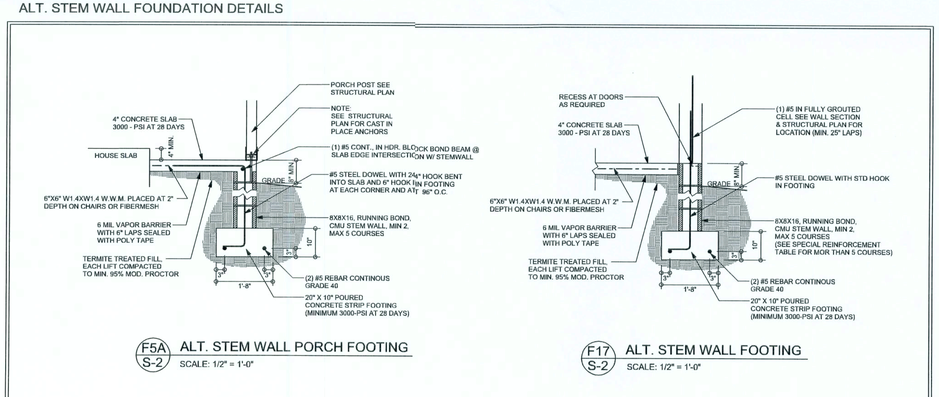


MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

ALT. STEM WALL FOUNDATION DETAILS



F5A S-2 ALT. STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"

F17 S-2 ALT. STEM WALL FOOTING
SCALE: 1/2" = 1'-0"

REVISIONS

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

VINDLOAD ENGINEER: Mark Disosway, FE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

IMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY
P.E. 53915
12 OCT 06
SEAL

Damian Rivera

addition

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