



Lumber design values are in accordance with ANSI/TPI 1 section 6.3
These truss designs rely on lumber values established by others.

RE: 2369239 - RHETT KLEMANS

MiTek, Inc.
16023 Swingley Ridge Rd.
Chesterfield, MO 63017
314.434.1200

Site Information:

Customer Info: RHETT KLEMANS Project Name: Garage Model: Custom
Lot/Block: N/A Subdivision: N/A
Address: 101 Ginne Springs Rd., N/A
City: Gilchrist Cty State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: License #:
Address:
City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2023/TPI2014 Design Program: MiTek 20/20 8.7
Wind Code: N/A Wind Speed: N/A mph
Roof Load: N/A psf Floor Load: N/A psf

This package includes 2 individual, Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

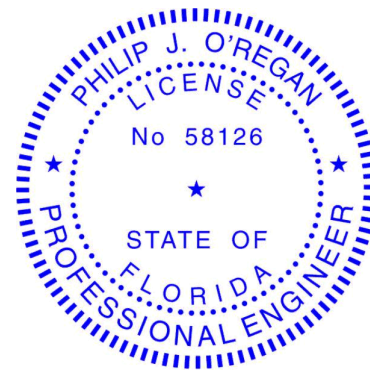
No.	Seal#	Truss Name	Date
1	T34355054	T01	7/5/24
2	T34355055	T01G	7/5/24

This item has been digitally signed and sealed by ORegan, Philip, PE on the date adjacent to the seal.
Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies

The truss drawing(s) referenced above have been prepared by
MiTek USA, Inc. under my direct supervision based on the parameters
provided by Builders FirstSource-Lake City, FL.

Truss Design Engineer's Name: ORegan, Philip
My license renewal date for the state of Florida is February 28, 2025.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Philip J. O'Regan PE No.58126
MiTek Inc. DBA MiTek USA FL Cert 6634
16023 Swingley Ridge Rd. Chesterfield, MO 63017
Date:

July 5, 2024

ORegan, Philip

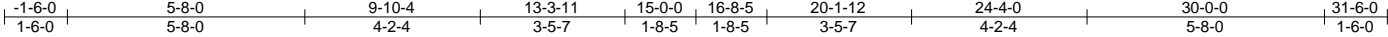
1 of 1

Job	Truss	Truss Type	Qty	Ply	RHETT KLEMANS	T34355054
2369239	T01	ATTIC	17	1	Job Reference (optional)	

Builders FirstSource (Lake City,FL), Lake City, FL - 32055,

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Scale = 1:55.0

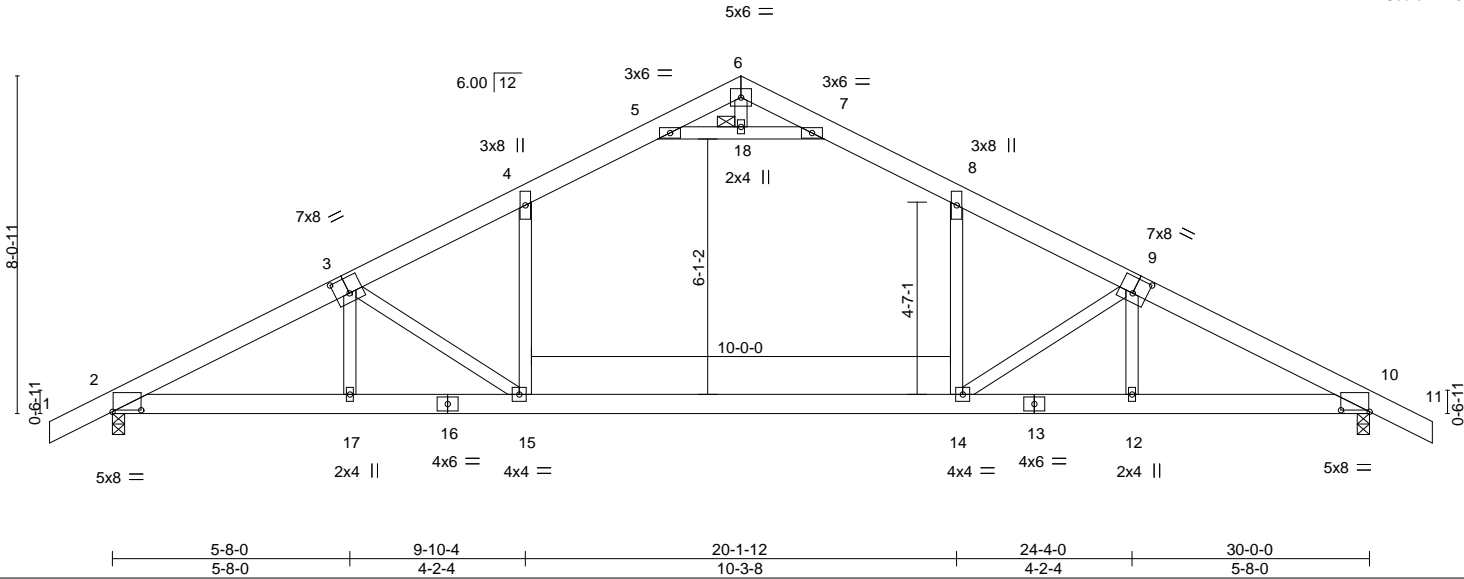


Plate Offsets (X,Y)--		[2:0-8-3,0-0-7], [3:0-4-0,0-4-8], [9:0-4-0,0-4-8], [10:0-8-3,0-0-7]							
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.48	Vert(LL)	-0.31 14-15	>999	240	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.58	Vert(CT)	-0.62 14-15	>581	180		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.66	Horz(CT)	0.06 10	n/a	n/a		
BCDL 10.0	Code	FBC2023/TPI2014	Matrix-MS	Attic	-0.15 14-15	808	360	Weight: 199 lb	FT = 20%

LUMBER-		BRACING-	
TOP CHORD	2x6 SP No.2 "Except" 3-6,6-9: 2x6 SP M 26	TOP CHORD	Structural wood sheathing directly applied or 4-3-13 oc purlins.
BOT CHORD	2x6 SP No.2 "Except" 13-16: 2x6 SP M 26	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2x4 SP No.3	JOINTS	1 Brace at Jt(s): 18

REACTIONS. (size) 2=0-3-8, 10=0-3-8
Max Horz 2=174(LC 17)
Max Uplift 2=324(LC 12), 10=324(LC 13)
Max Grav 2=1462(LC 2), 10=1462(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-2682/456, 3-4=-2412/273, 4-5=-1901/320, 5-6=-39/845, 6-7=-39/845,
7-8=-1901/320, 8-9=-2412/272, 9-10=-2682/458
BOT CHORD 2-17=-472/2390, 15-17=-475/2385, 14-15=-119/1986, 12-14=-303/2385, 10-12=-300/2390
WEBS 5-18=-2998/350, 7-18=-2998/350, 4-15=-5/903, 8-14=-6/903, 3-15=-684/427,
9-14=-684/429, 6-18=-11/257

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Ceiling dead load (5.0 psf) on member(s). 4-5, 7-8, 5-18, 7-18; Wall dead load (5.0psf) on member(s).4-15, 8-14
 - Bottom chord live load (30.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 14-15
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=324, 10=324.
 - ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.

This item has been digitally signed and sealed by ORegan, Philip, PE on the date indicated here. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regan PE No.58126
MiTek Inc. DBA MiTek USA FL Cert 6634
16023 Swingley Ridge Rd. Chesterfield, MO 63017
Date:

July 5,2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcsccomponents.com)

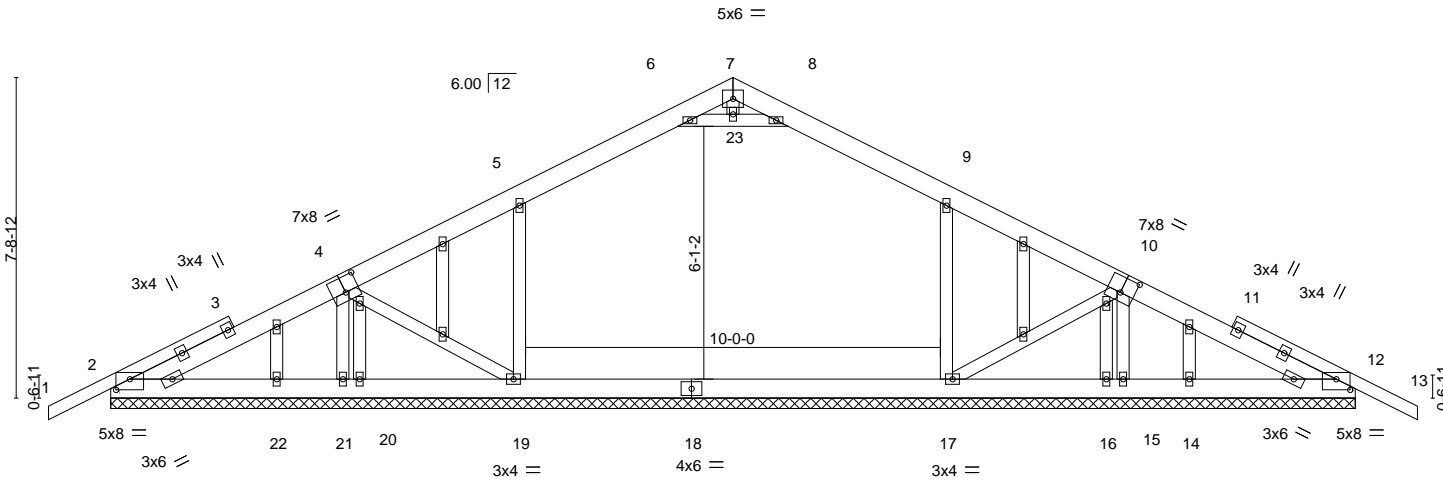
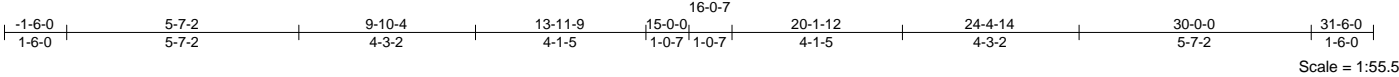
MiTek®
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Job	Truss	Truss Type	Qty	Ply	RHETT KLEMANS	T34355055
2369239	T01G	GABLE	2	1	Job Reference (optional)	

Builders FirstSource (Lake City, FL), Lake City, FL - 32055,

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		5-7-2		9-10-4		20-1-12		24-4-14		30-0-0	
		5-7-2		4-3-2		10-3-8		4-3-2		5-7-2	
Plate Offsets (X,Y)--		[2:0-4-0,0-3-1], [4:0-4-0,0-4-8], [10:0-4-0,0-4-8], [12:0-4-0,0-3-1]									
LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0		Plate Grip DOL	1.25	TC 0.13		Vert(LL)	0.00 12	n/r	120	MT20	244/190
TCDL 7.0		Lumber DOL	1.25	BC 0.46		Vert(CT)	0.00 13	n/r	120		
BCLL 0.0 *		Rep Stress Incr	YES	WB 0.14		Horz(CT)	0.01 12	n/a	n/a		
BCDL 10.0		Code	FBC2023/TPI2014	Matrix-S						Weight: 214 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x6 SP No.2 *Except* 1-3,11-13: 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2x6 SP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No.3	
OTHERS 2x4 SP No.3	

REACTIONS.	All bearings 30-0-0.
(lb) - Max Horz	2=-165(LC 13)
Max Uplift	All uplift 100 lb or less at joint(s) except 2=-109(LC 12), 19=-136(LC 12), 17=-156(LC 13), 21=-175(LC 12), 15=-130(LC 13), 16=-412(LC 18), 20=-412(LC 18), 12=-134(LC 13)
Max Grav	All reactions 250 lb or less at joint(s) 14, 22 except 2=336(LC 1), 19=672(LC 28), 17=672(LC 29), 21=649(LC 20), 15=648(LC 2), 12=336(LC 1)

FORCES.	(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD	2-4=-266/110, 4-5=-446/132, 5-6=-463/188, 8-9=-463/180, 9-10=-446/120, 10-12=-266/49
BOT CHORD	17-19=0/367
WEBS	5-19=-331/173, 9-17=-331/166, 4-21=-457/225, 10-15=-457/180

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
 - All plates are 2x4 MT20 unless otherwise indicated.
 - Gable requires continuous bottom chord bearing.
 - Gable studs spaced at 2-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Ceiling dead load (5.0 psf) on member(s). 5-6, 8-9, 6-23, 8-23; Wall dead load (5.0psf) on member(s).5-19, 9-17
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 109 lb uplift at joint 2, 136 lb uplift at joint 19, 156 lb uplift at joint 17, 175 lb uplift at joint 21, 130 lb uplift at joint 15, 412 lb uplift at joint 16, 412 lb uplift at joint 20 and 134 lb uplift at joint 12.
 - ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.

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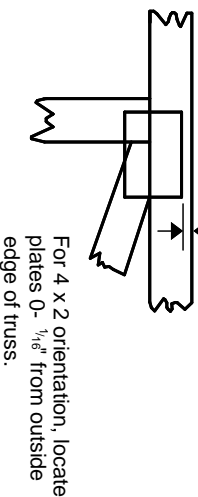
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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Symbols

PLATE LOCATION AND ORIENTATION



* Plate location details available in MITek software or upon request.

PLATE SIZE

4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

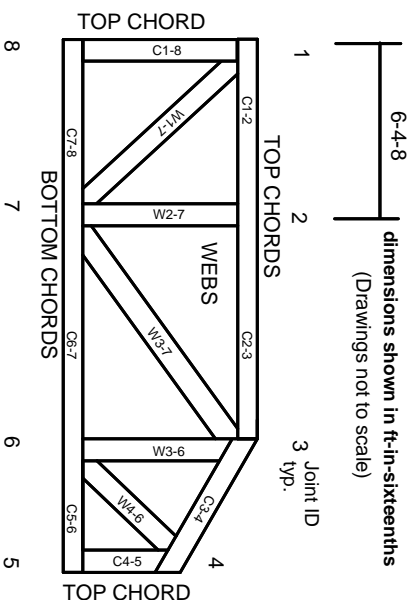


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number/letter where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TP1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-22: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

Product Code Approvals

ICC-ES Reports:

ESR-1988, ESR-2362, ESR-2685, ESR-3282
ESR-4722, ESL-1388

Design General Notes

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TP1 1 section 6.3. These truss designs rely on lumber values established by others.

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MITek Engineering Reference Sheet: MII-7473 rev. 1/2/2023

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TP1 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP1 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TP1 1 Quality Criteria.
21. The design does not take into account any dynamic or other loads other than those expressly stated.