| REVISIONS | 11 / Nov. / 2008 |
|---|---|
| | |
| | |
| | |


BACK ELEVATION
scale 3/16-1'0"

ADDITIONS & RENOVATIONS FOR:

# TANNICHIONS RESIDENCE

PROJECT ADDRESS:
COLUMBIA COUNTY
FLORIDA

ISAAC CONSTRUCTION, L.L.C.
FINE CUSTOM HOMEBUILDING

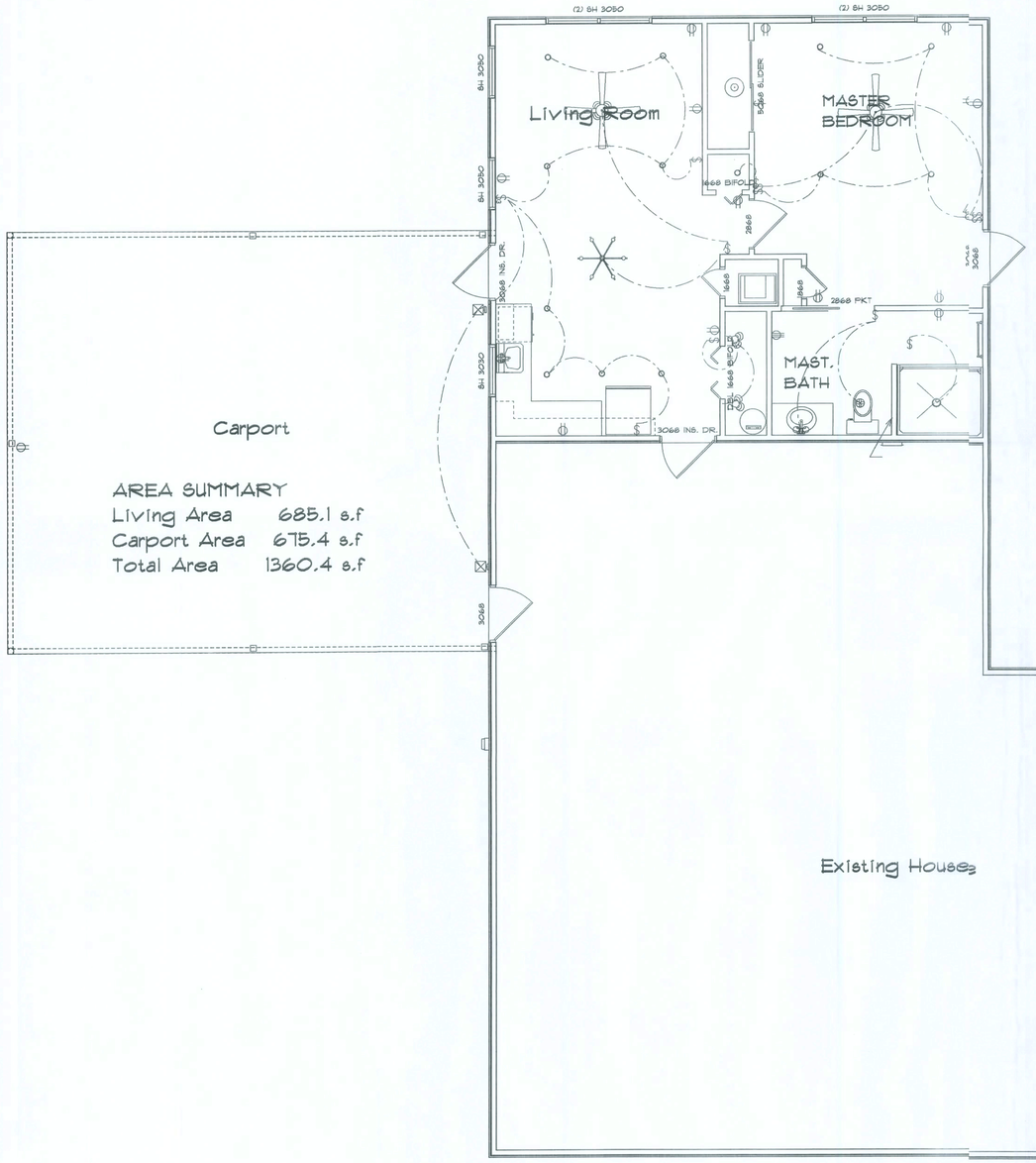
*Office: (386) 719 - 7143*
*Cell: (386) 867 - 0134*

DRAWING DATE:
November 19, 2008

SHEET NUMBER

OF 4 SHEETS

| REVISIONS | 11 / Nov. / 2008 |
|---|---|
| | |
| | |
| | |




ADDITIONS & RENOVATIONS FOR:

# TANNICHIONS RESIDENCE

PROJECT ADDRESS:

COLUMBIA COUNTY
FLORIDA

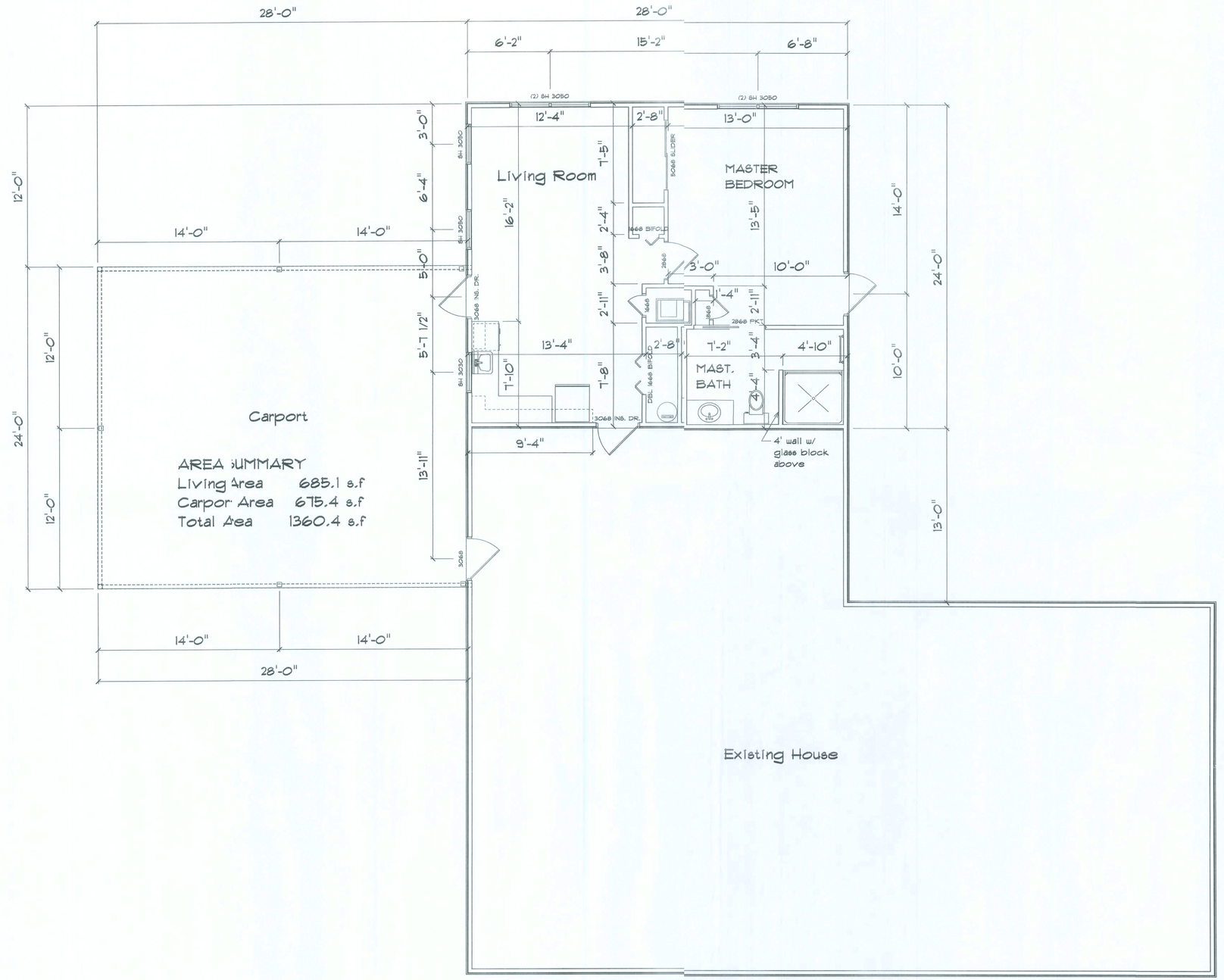

ISAAC CONSTRUCTION, L.L.C.
FINE CUSTOM HOME BUILDING

*Office: (386) 719 - 7143*
*Cell: (386) 867 - 0134*


DRAWING DATE:
November 20, 2008

SHEET NUMBER

OF 4 SHEETS




| REVISIONS | 11 / Nov. / 2008 |
|---|---|
| | |
| | |
| | |

ADDITIONS & RENOVATIONS FOR:

# TANNICHIONS RESIDENCE

PROJECT ADDRESS:

COLUMBIA COUNTY
FLORIDA


ISAAC CONSTRUCTION, L.L.C.
FINE CUSTOM HOMEBUILDING

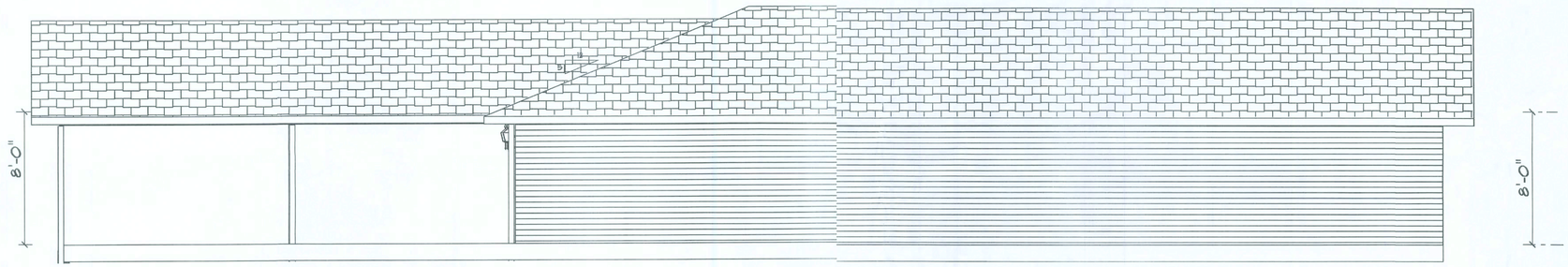
*Office: (386) 719 - 7143*
*Cell: (386) 867 - 0134*


DRAWING DATE:
November 18, 2008

SHEET NUMBER

OF 4 SHEETS

| REVISIONS | 11 / Nov. / 2008 |
|---|---|
| | |
| | |




FRONT ELAVATION
SCALE 3/16 1'-0"

ADDITIONS & RENOVATIONS FOR:

# TANNICHIONS RESIDENCE

PROJECT ADDRESS:

COLUMBIA COUNTY
FLORIDA

ISAAC CONSTRUCTION, L.L.C.
FINE CUSTOM HOMEBUILDING

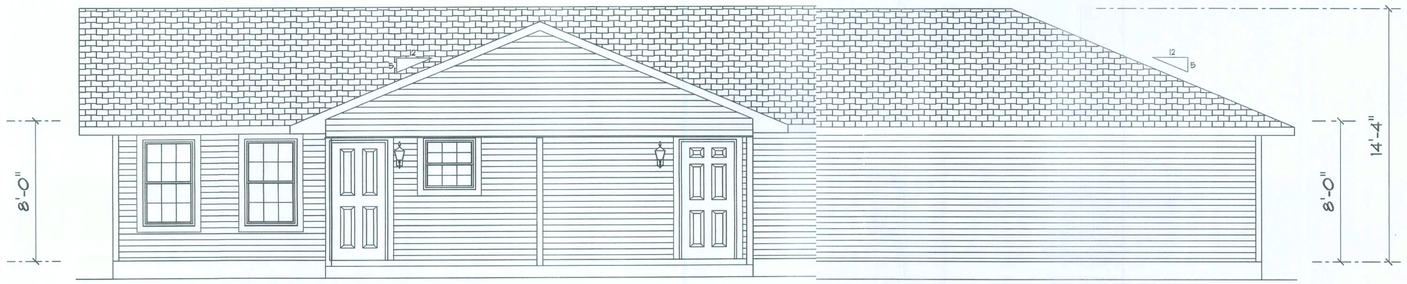
*Office: (386) 719 - 7143*
*Cell: (386) 867 - 0134*

DRAWING DATE:
November 19, 2008

SHEET NUMBER

OF 4 SHEETS

| REVISIONS | 11 / Nov. / 2008 |
|---|---|
| | |
| | |




LEFT ELEVATION

SCALE: 3/16" = 1'-0"

ADDITIONS & RENOVATIONS FOR:

# TANNICHIONS RESIDENCE

PROJECT ADDRESS:

COLUMBIA COUNTY
FLORIDA

ISAAC CONSTRUCTION, L.L.C.
FINE CUSTOM HOMEBUILDING

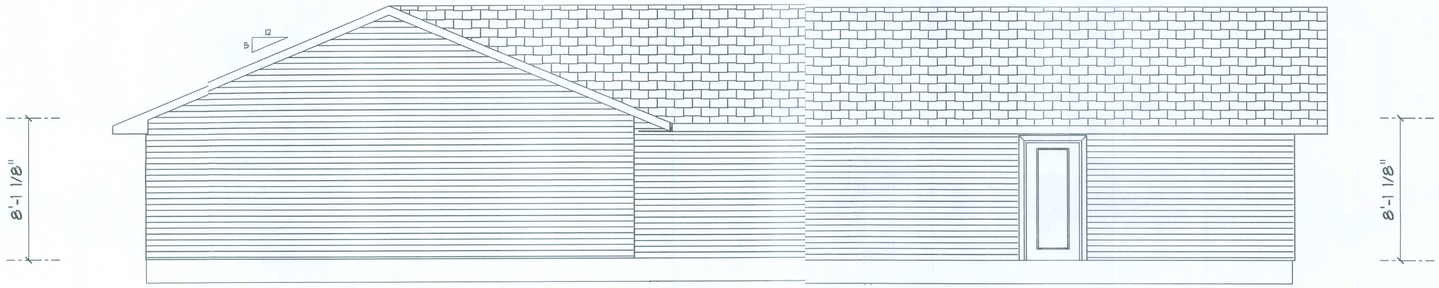
*Office: (386) 719 - 7143*
*Cell: (386) 867 - 0134*

DRAWING DATE:
November 18, 2008

SHEET NUMBER

OF 4 SHEETS

| REVISIONS | 11 / Nov. / 2008 |
|---|---|
| | |
| | |
| | |




Right Elevation

scale 1/16-1'0"

ADDITIONS & RENOVATIONS FOR:

# TANNICHIONS RESIDENCE

PROJECT ADDRESS:

COLUMBIA COUNTY
FLORIDA

ISAAC CONSTRUCTION, L.L.C.
FINE CUSTOM HOMEBUILDING

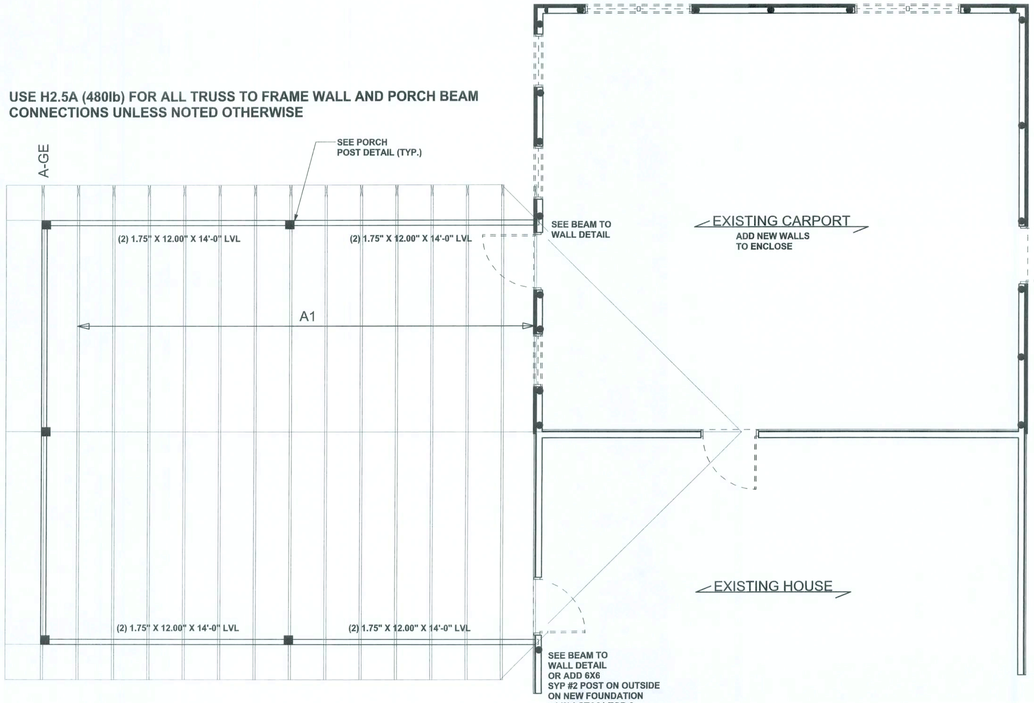
*Office: (386) 719 - 7143*
*Cell: (386) 867 - 0134*

DRAWING DATE:
November 19, 2008

SHEET NUMBER

OF 4 SHEETS

**USE H2.5A (480lb) FOR ALL TRUSS TO FRAME WALL AND PORCH BEAM CONNECTIONS UNLESS NOTED OTHERWISE**




## STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

## STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

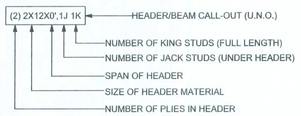
SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

## WALL LEGEND

| | |
|---|---|
| | EXTERIOR WALL |
| | INTERIOR NON-LOAD BEARING WALL |
| | INTERIOR LOAD BEARING WALL w/ NO UPLIFT |
| | INTERIOR LOAD BEARING WALL w/ UPLIFT |

## HEADER LEGEND



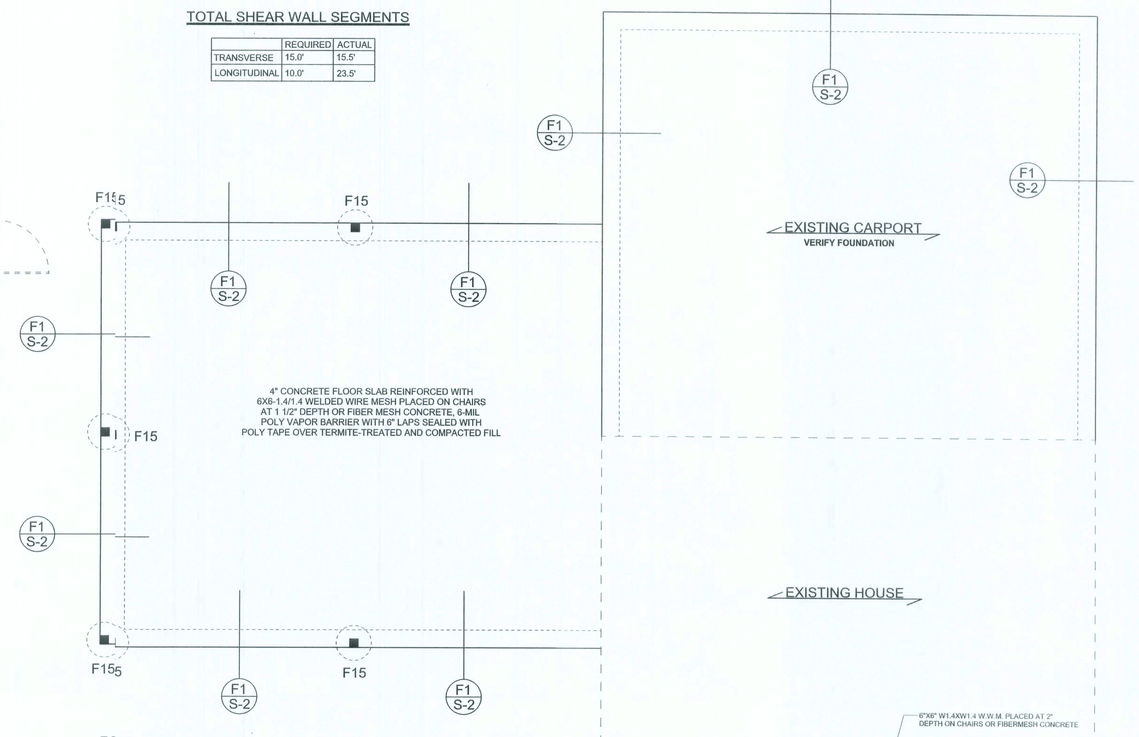

## THREADED ROD LEGEND

INDICATES LOCATION OF: 1ST FLOOR 1/2" A307 ALL THREADED ROD

INDICATES LOCATION OF: 2ND FLOOR 1/2" A307 ALL THREADED ROD

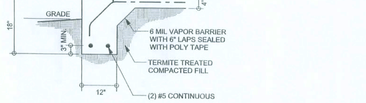
## TOTAL SHEAR WALL SEGMENTS

| | REQUIRED | ACTUAL |
|---|---|---|
| TRANSVERSE | 15.0' | 15.5' |
| LONGITUDINAL | 10.0' | 23.5' |




## FOUNDATION PLAN
SCALE: 1/4" = 1'-0"

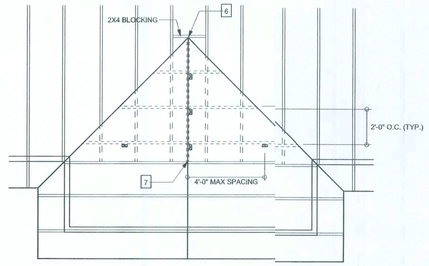
DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS



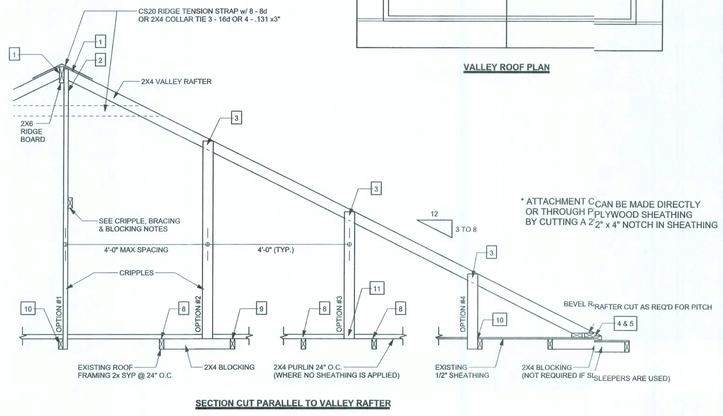

F1 S-2 MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"

**LUMBER SIZE & GRADE MINUMUM REQUIREMENTS**

| | |
|---|---|
| RIDGE BOARD | 2X8 SYP #2 |
| RAFTER SPANS 20'-0" OR LESS | 2X4 SYP #2 |
| PURLINS / LATERAL BRACING | 2X4 SPF #2 |
| SLEEPERS | 2X (WIDTH OF RAFTER SEAT CUT) SPF #3 OR 2 PARALLEL 2X4 SPF #3 |
| CRIPPLES & BLOCKING | 2X4 SPF #2 OR BETTER |
| TRUSS BELOW | SEE TRUSS DESIGN - SOUTHEREN PINE MATERIAL |




VALLEY ROOF PLAN




SECTION CUT PARALLEL TO VALLEY RAFTER

## RETROFIT ROOF OVER FRAMING & BRACING DETAIL
SCALE: N.T.S

**VALLEY ROOF PLAN MEMBER LEGEND**

TRUSS
TRUSS UNDER VALLEY FRAMING
VALLEY RAFTER OR RIDGE
CRIPPLE

CRIPPLES 4'-0" O.C. FOR 20 psf (TL) AND 10 psf (TD) (TYP. SHINGLE ROOF) MAX

**CONNECTION REQUIREMENT NOTES**

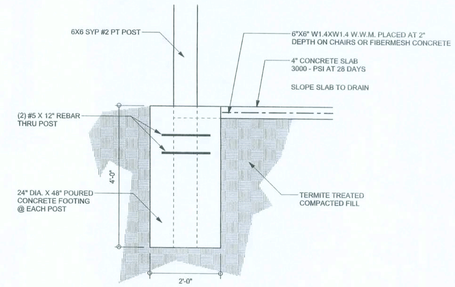
| | | |
|---|---|---|
| 1 | 2X4 RAFTERS TO RIDGE | 3 -16d OR 6 - .131 x 3" TOE NAILS |
| 2 | CRIPPLE TO RIDGE | 3 - 16d OR 6 - .131 x 3" FACE NAILS |
| 3 | CRIPPLE TO RAFTERS | 3 - 16d OR 6 - .131 x 3" FACE NAILS |
| 4 | RAFTER TO SLEEPER OR BLOCKING | 6 -16d OR 12 - .131 x 3" TOE NAILS |
| 5 | SLEEPER TO TRUSS | 4 - 16d OR 8 - .131 x 3" FACE NAILS EACH TRUSS |
| 6 | RIDGE BOARD TO ROOF BLOCK | 3 -16d OR 6 - .131 x 3" TOE NAILS |
| 7 | RIDGE BOARD TO TRUSS | 3 -16d OR 6 - .131 x 3" TOE NAILS |
| 8 | PURLIN TO TRUSS (TYP.) | 3 -16d OR 6 - .131 x 3" NAILS |
| 8 | PURLIN TO TRUSS (IF CRIPPLE IS ATTACHED TO PURLIN) | 4 -16d OR 8 - .131 x 3" NAILS |
| 9 | TRUSS TO BLOCKING | 3 -16d OR 6 - .131 x 3" END NAILS |
| 10 | CRIPPLE TO TRUSS | 3 -16d OR 6 - .131 x 3" FACE NAILS |
| 11 | CRIPPLE TO PURLIN | 3 -16d OR 6 - .131 x 3" FACE NAILS |

**GENERAL NOTES**

MAXIMUM RAFTER SPANS
6'-0" FOR 2X4, 9'-0" FOR 2X6 SPF #2 OR SYP #2.
MAXIMUM ROOF AREA PER SUPPORT
16ft2 IN ZONES 2 & 3 , 24ft2 IN ZONE 1. (EXAMPLE: 4'-0" O.C. X 4'-0" SPAN = 16ft2 OR 2'-0" X 8'-0" SPAN = 16ft2)
PURLINS REQUIRED 2'-0" O.C. IF EXISTING SHEATHING IS REMOVED.
PURLINS SHOULD OVERLAP SHEATHING ONE TRUSS SPACING MINIMUM.
IN CASES THAT THIS IS IMPRACTICAL, OVERLAP SHEATHING A MINIMUM OF 6", AND NAIL UPWARDS THROUGH SHEATHING INTO PURLIN WITH A MINIMUM OF 8 - 8d COMMON WIRE NAILS.
THIS DRAWING APPLIES TO VALLEYS WITH THE FOLLOWING CONDITIONS:
- SPANS (DISTANCES BETWEEN HEELS) 40'-0" OR LESS
- MAXIMUM VALLEY HEIGHT: 14'-0" OR LESS
-MAXIMUM WIND SPEED: 120 MPH
- MAXIMUM MEAN ROOF HEIGHT: 30 FEET
- MAXIMUM TOTAL LOADING: 40 psf
- MEETS FBC 2001/ASCE 7-98 WIND REQUIREMENTS
- EXPOSURE CATEGORY "B", I = 1.0, Kzt = 1.0
- ENCLOSED BUILDING

**CRIPPLE, BRACING, & BLOCKING NOTES**

-2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 6'-0" TO 10'-0" LONG NAILED w/ 2 - 10d NAILS OR 2X4 "T" OR SCAB BRACE NAILD TO FLAT EDGE OF CRIPPLE WITH 8d NAILS @ 8" O.C. "T" OR SCAB MUST BE 90% OF CRIPPLE LENGTH. CRIPPLES OVER 10'-0" LONG REQUIRE TWO CLB's OR BOTH FACES w/ "T" OR SCAB. USE STRESS GRADED LUMBER & BOX OR COMMON NAILS.
- NARROW EDGE OF CRIPPLE CAN FACE RIDGE OR RAFTER, AS LONG AS THE PROPER NUMBER OF NAILS ARE INSTALLED INTO RIDGE BOARD.
- INSTALL BLOCKING UNDER RAFTER IF SLEEPERS ARE NOT USED.
- INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FALL BETWEEN LOWER TRUSS TOP CHORDS AND LATERAL BRACING IS NOT USED.
- APPLY ALL NAILING IN ACCORDANCE TO NDS-1997 SECTION 12. NAILS ARE COMMON WIRE NAILS UNLESS NOTED OTHERWISE.




F15 S-2 CARPORT POST FOUNDATION
SCALE: 1/2" = 1'-0"

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. ANDERSON TRUSS JOB # 8-260

REVISIONS

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER:
Mark Disosway, PE
No.53915, POB 868, Lake City,FL 32056, 386-754-5419

DIMENSIONS:
Stated dimensions supercede saled dimensions. Refer all questionsto Mark Disosway, P.E. for resolutbn. Do not proceed without clarification.



CERTIFICATION: I hereby certify that I have examined this plan, and that theapplicable portions of the plan, relating to wind engineering comply with sction R301.2.1, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is vaif for one building, at specified location.

MARK DISOSWAY P.E. 53915

16Nov08

SEAL

Isaac Construction

Tannachion Addition / Renvation

ADDRESS: Columbia County, Forida

Mark DisoswayP.E.
P.O. Box 868
Lake City, Florida32056
Phone: (386) 754- 5419
Fax: (386) 269 -4871

PRINTED DATE: November 16, 208

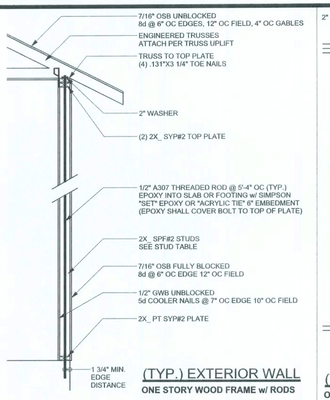
DRAWN BY: | STRUCTURAL BY: Davl Disosway

FINALS DATE: 16Nov08

JOB NUMBER: 811122

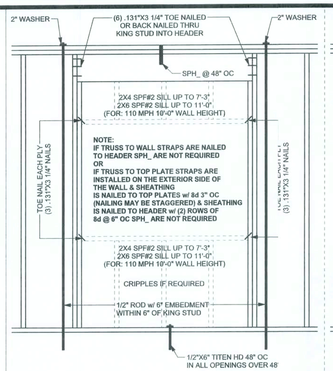
DRAWING NUMBER **S-2**

OF 2 SHEETS




## (TYP.) EXTERIOR WALL

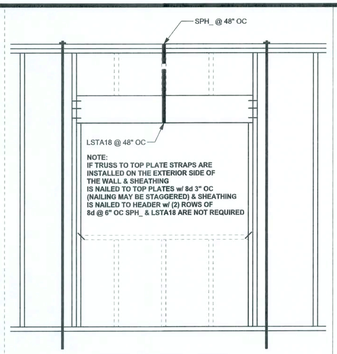
**ONE STORY WOOD FRAME w/ RODS**




## (TYP.) HEADER

**OPTION: 1 (FLUSH HEADER)**

**ONE STORY WOOD FRAME w/ RODS**




**OPTION: 2 (DROPPED HEADER)**

## ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

| TRUSS CONNECTOR | UPLIFT SYP | UPLIFT SPF | F1 SYP |
|---|---|---|---|
| H5 | 455 | 265 | 115 |
| H3 | 415 | 290 | 125 |
| H2.5 | 415 | 365 | 150 |
| H2.5A | 480 | 480 | 110 |
| H6 | 950 | 820 | |
| H8 | 745 | 565 | |
| H14-1 | 1465 | 1050 | 515 |
| H14-2 | 1465 | 1050 | 515 |
| H10 | 990 | 850 | 585 |
| H10-2 | 760 | 655 | 455 |
| H16 | 1470 | 1265 | |
| H16-2 | 1470 | 1265 | |
| LTS12 - LTS20 | 1000 | 620 | |
| MTS12 - MTS30 | 1000 | 860 | |
| HTS16 - HTS30 | 1450 | 1245 | |
| **HEAVY GIRDER TIEDOWNS** | | | |
| LGT2 | 2050 | 1785 | 700 |
| LGT3-SDS2.5 | 3685 | 2655 | 795 |
| LGT4-SDS3 | 4060 | 3860 | 2000 |
| MGT | 3965 | 3330 | |
| HGT-2 | 10980 | 6485 | |
| HGT-3 | 10530 | 9035 | |
| HGT-4 | 9250 | 9250 | |
| **STUD STRAP CONNECTOR** | | | |
| SSP DOUBLE TOP PLATE | 435 | 435 | |
| SSP SINGLE SILL PLATE | 455 | 420 | |
| DSP DOUBLE TOP PLATE | 825 | 825 | |
| DSP SINGLE SILL PLATE | 825 | 600 | |
| SP1 | 585 | 535 | |
| SP2 | 1065 | 605 | |
| SP4 | 885 | 760 | |
| SPH4 | 1240 | 1065 | |
| SP6 | 885 | 760 | |
| SPH6 | 1240 | 1065 | |
| LSTA18 | 1235 | 1110 | |
| LSTA21 | 1235 | 1235 | |
| CS20 | 1030 | 1030 | |
| CS16 | 1705 | 1705 | |
| **STUD ANCHORS** | | | |
| LTT19 | 1350 | 1305 | |
| LTT31 | 2310 | 2310 | |
| HD2A | 2775 | 2570 | |
| HTT16 | 4175 | 3695 | |
| HTT22 | 5260 | 5250 | |
| ABU44 | 2200 | 2200 | |
| ABU66 | 2300 | 2300 | |
| ABU88 | 2320 | 2320 | |

| F2 SYP | F1 SPF | F2 SPF | TO RAFTER/TRUSS | TO PLATES | |
|---|---|---|---|---|---|
| 200 | 100 | 170 | 4-8d x 1 1/2" | 4-8d x 1 1/2" | |
| 160 | 105 | 140 | 4-8d x 1 1/2" | 4-8d x 1 1/2" | |
| 150 | 130 | 130 | 5-8d x 1 1/2" | 5-8d x 1 1/2" | |
| 110 | 110 | 110 | 5-8d x 1 1/2" | 5-8d x 1 1/2" | |
| | | | 8-8d | 8-8d | |
| | | | 5-10d x 1 1/2" | 5-10d x 1 1/2" | |
| 265 | 480 | 245 | 12-8d x 1 1/2" | 13-8d | |
| 265 | 480 | 245 | 12-8d x 1 1/2" | 15-8d | |
| 525 | 505 | 450 | 8-8d x 1 1/2" | 8-8d x 1 1/2" | |
| 395 | 390 | 340 | 6-10d | 6-10d | |
| | | | 2-10d x 1 1/2" | 10-10d x 1 1/2" | |
| | | | 2-10d x 1 1/2" | 10-10d x 1 1/2" | |
| | | | 6-10d x 1 1/2" | 6-10d x 1 1/2" | |
| | | | 7-10d x 1 1/2" | 7-10d x 1 1/2" | |
| | | | 12-10d x 1 1/2" | 12-10d x 1 1/2" | |
| | | | | | **TO FOUNDATION** |
| 170 | 700 | 170 | 14-16d | 14-16d | |
| 410 | 795 | 410 | 12-SDS 1/4" x 2 1/2" | 26-16dS | |
| 675 | 2000 | 675 | 12-SDS 1/4" x 3" | 36-16dS | |
| | | | 22 -10d | | 5/8" ANCHOR |
| | | | 16 -10d | | 2-5/8" ANCHOR |
| | | | 16 -10d | | 2-5/8" ANCHOR |
| | | | 16 -10d | | 2-5/8" ANCHOR |
| | | | | | **TO STUDS** |
| | | | | 3-10d | 4 -10d |
| | | | | 1-10d | 4 -10d |
| | | | | 6 -10d | 8 -10d |
| | | | | 2 -10d | 8 -10d |
| | | | | 4 -10d | 6 -10d |
| | | | | 6 -10d | 6 -10d |
| | | | | | 6-10d x 1 1/2" |
| | | | | | 10-10d x 1 1/2" |
| | | | | | 6-10d x 1 1/2" |
| | | | | | 10-10d x 1 1/2" |
| | | | | | 14-10d |
| | | | | | 16-10d |
| | | | | | 14-10d |
| | | | | | 22-10d |
| | | | **TO STUDS** | | **TO FOUNDATION** |
| | | | 8-16d | | 1/2" ANCHOR |
| | | | 18-10d x 1 1/2" | | 5/8" ANCHOR |
| | | | 2-5/8" BOLTS | | 5/8" ANCHOR |
| | | | 18-16d | | 5/8" ANCHOR |
| | | | 32-16d | | 5/8" ANCHOR |
| | | | 12-16d | | 5/8" ANCHOR |
| | | | 12-16d | | 5/8" ANCHOR |
| | | | 18-16d | | 2-5/8" ANCHOR |

(1) w/ INSTALLATION OF 4-16dS OPTIONAL NAIL HOLES
(2) FOR SYP GIRDER & SPF STUDS

## EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

| | |
|---|---|
| (1) 2x4 @ 16" OC | TO 11'-9" STUD HEIGHT |
| (1) 2x4 @ 12" OC | TO 13'-0" STUD HEIGHT |
| (1) 2x6 @ 16" OC | TO 18'-10" STUD HEIGHT |
| (1) 2x6 @ 12" OC | TO 20.0' STUD HEIGHT |

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

## GRADE & SPECIES TABLE

| | | Fb (psi) | E ($10^6$ psi) |
|---|---|---|---|
| 2x8 | SYP #2 | 1200 | 1.6 |
| 2x10 | SYP #2 | 1050 | 1.6 |
| 2x12 | SYP #2 | 975 | 1.6 |
| GLB | 24F-V3 SP | 2400 | 1.8 |
| LSL | TIMBERSTRAND | 1700 | 1.7 |
| LVL | MICROLAM | 2900 | 2.0 |
| PSL | PARALAM | 2900 | 2.0 |

## GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $F'c$ = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, $FY$ = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS) UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

## BUILDER'S RESPONSIBILITY

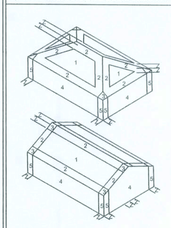
| |
|---|
| **THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.** |
| CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. |
| PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. |
| PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. |
| VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. |

## ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

## DESIGN DATA

| **WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1** |
|---|
| (ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.) |
| BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE |
| BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION |
| 1.) BASIC WIND SPEED = 110 MPH |
| 2.) WIND EXPOSURE = B |
| 3.) WIND IMPORTANCE FACTOR = 1.0 |
| 4.) BUILDING CATEGORY = II |
| 5.) ROOF ANGLE = 10-45 DEGREES |
| 6.) MEAN ROOF HEIGHT = <30 FT |
| 7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING) |
| 8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2)) |



| Zone | Effective Wind Area (ft2) | |
|---|---|---|
| | 10 | 100 |
| 1 | 19.9 / 21.8 | 18.1 / -18.1 |
| 2 | 19.9 / 25.5 | 18.1 / -21.8 |
| 2 Ohg | -40.6 | -40.6 |
| 3 | 19.9 / 25.5 | 18.1 / -21.8 |
| 3 Ohg | -68.3 | -42.4 |
| 4 | 21.8 / 23.6 | 18.5 / -20.4 |
| 5 | 21.8 / 29.1 | 18.5 / -22.6 |

| Doors & Windows Worst Case (Zone 5, 10 ft2) | 21.8 | -29.1 |
|---|---|---|
| | | |
| 8x7 Garage Door | 19.5 | -22.9 |
| 16x7 Garage Door | 18.5 | -21.0 |
| | | |
| | | |
| | | |

| **DESIGN LOADS** | |
|---|---|
| FLOOR | 40 PSF (ALL OTHER DWELLING ROOMS) |
| | 30 PSF (SLEEPING ROOMS) |
| | 30 PSF (ATTICS WITH STORAGE) |
| | 10 PSF (ATTICS WITHOUT STORAGE, <3:12) |
| ROOF | 20 PSF (FLAT OR <4:12) |
| | 16 PSF (4:12 TO <12:12) |
| | 12 PSF (12:12 AND GREATER) |
| STAIRS | 40 PSF (ONE & TWO FAMILY DWELLINGS) |
| SOIL BEARING CAPACITY | 1000PSF |
| NOT IN FLOOD ZONE (BUILDER TO VERIFY) | |

| REVISIONS | |
|---|---|
| | |
| | |
| | |
| | |

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER:
Mark Disosway, PE
No.53915, POB 868, Lake Cy, FL 32056,
386-754-5419

DIMENSIONS:
Stated dimensions supercedes scaled dimensions. Refer all questios to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

COPYRIGHTS AND PROPEITY RIGHTS:
Mark Disosway, P.E. hereby xpressly reserves its common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express written permissio and consent of Mark Disosway.

CERTIFICATION: I hereby crtify that I have examined this plan, and that 'e applicable portions of the plan, relating t wind engineering comply wit section R301.2.1, florida building coo residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location

MARK DISOSWAY
P.E. 53915

SEAL

Isaac Construction

Tannachon
Addition / Rerovation

ADDRESS
Columbia County Florida


Mark Disoswy P.E.
P.O. Box 868
Lake City, Florica 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871


PRINTED DATE:
November 16,2008

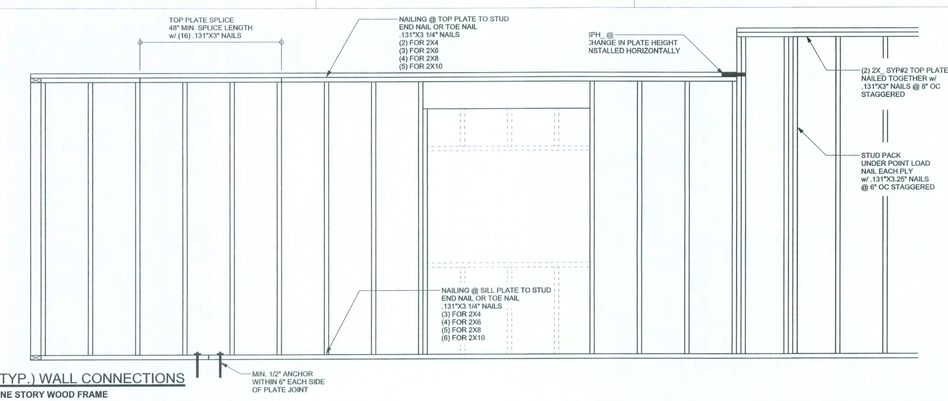
DRAWN BY:

STRUCTURAL BY:
David Disoswy

FINALS DATE:
16Nov08

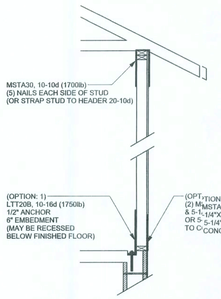
JOB NUMBER:
81112

DRAWING NUMBER
**S-1**
OF 2 SHEETS




## (TYP.) WALL CONNECTIONS

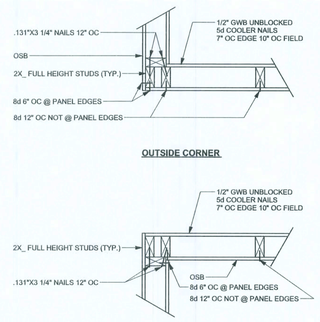
**ONE STORY WOOD FRAME**




## ALTERNATE CONNECTION WHERE ROD CANNOT BE PLACED IN WALL

**ONE STORY WOOD FRAME w/ RODS**

**ALLOWABLE UPLIFT: 1900 LB**



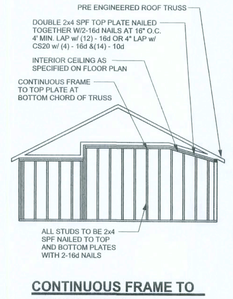

**OUTSIDE CORNER**




**INSIDE CORNER**

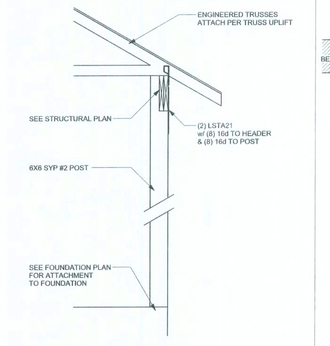
## (TYP.) CORNER FRAMING

**WOOD FRAME**



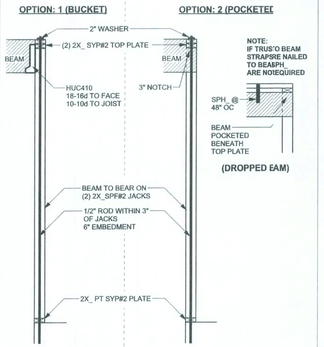

## CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL

SCALE: N.T.S.




## (TYP.) PORCH POST

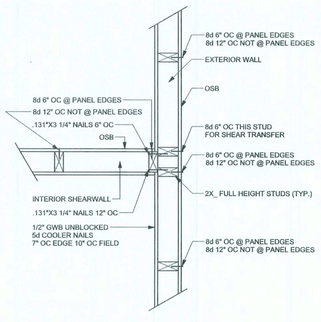
**ONE STORY WOOD**




## (TYP.) BEAM TO WALL

**WOOD FRAME w/ RODS**

**ALLOWLE UPLIFT: 1770 LE**



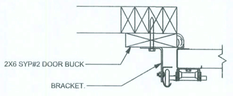

## (TYP.) INTERSECTING WALL FRAMING

**WOOD FRAME**

**2X6 SYP#2 GARAGE DOOR BUCK ATTACHMENT**

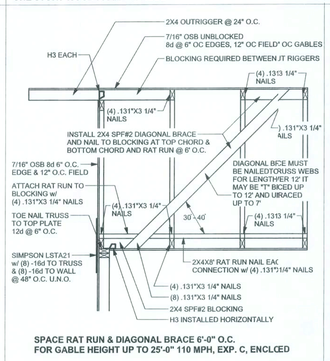
ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"X4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131X3 1/4" GN PER TABLE BELOW:

| DOOR WIDTH | 3/8"X4" LAG | 16d STAGGER | (2) ROWS OF .131"X3 1/4" NAILS |
|---|---|---|---|
| 8' - 10' | 24" OC | 5" OC | 5" OC |
| 11' - 15' | 18" OC | 4" OC | 4" OC |
| 16' - 18' | 16" OC | 3" OC | 3" OC |




## (TYP.) GARAGE DOOR BUCK INSTALLATION

**WOOD FRAME**




**SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C.**
**FOR GABLE HEIGHT UP TO 25'-0" 110 MPH, EXP. C, ENCLOED**

## (TYP.) GABLE BRACING DETAIL

**WOOD FRAME**