

REVISIONS

SOFIPIAN

ELECTRICAL PLAN NOTES

E -1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.

E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.

E -3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.

ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY
BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL
BE INTERLOCKED TOGETHER. INSTALL INSIDE AND
NEAR ALL BEDROOMS.

TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE

DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S
DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE
SECTIONS OF NEC-LATEST EDITION.

 E -6 ELECTRICAL CONT'R SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.

E -7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.

E -8 ALL BEDROOM RECEPTACLES SHALL BE AFCI

E -C ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION

A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC

E -1(CONDUCTORS ENTER THE BUILDING.
SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED
INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL
APPROVAL OF THE BUILDING OFFICIAL

CARBON MONOXIDE ALARMS SHALL BE REQUIRED WITHIN 10'

E -1 OF ALL ROOMS FOR SLEEPING PURPOSES IN BUILDINGS HAVING A FOSSIL-FUEL-BURNING HEATER OR APPLIANCE, A FIREPLACE, OR ATTACHED GARAGE.

	ELECTRICAL LEGEND
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
₫₽	DOUBLE SECURITY LIGHT
	2X4 FLUORESCENT LIGHT FIXTURE
0	RECESSED CAN LIGHT
- → -	BATH EXAUST FAN WITH LIGHT
⊕	BATH EXAUST FAN
- 	LIGHT FIXTURE
Ф	DUPLEX OUTLET
•	220v OUTLET
d GFI	GFI DUPLEX OUTLET
•	SMOKE DETECTOR V
\$	WALL SWITCH
\$3	3 WAY WALL SWITCH
\$4	4 WAY WALL SWITCH
∯ _{WP/GFI}	WATER PROOF GFI OUTLET
∇	PHONE JACK
0	TELEVISION JACK
P	GARAGE DOOR OPENER

PHONE JACK

TELEVISION JACK

GARAGE DOOR OPENER

CARBON MONOXIDE ALARM

TELEVISION JACK

PCOLORES

THE COLORES

PORCH

BEDROOM

REG D

VERK GROWD

REG D

VERK GROW

PORCH

GREAT ROOM

BEDROOM #2

BATH

2008 N. E.C. (NEW!)

COMBO. ARC-FAMINS 15-20 AMP Cincuits

TAMPEN PROOF RECEPT.

* ELECTRICAL PLAN *

DINING

MASTER

BEDROOM

SCALE : 1/4" = 1'-0"

Bryan Zecher Construction

William # Beverly Wallace Residence

561 NW 3ison Court White Springs, FL 32096

PRINED DATE:
December 10, 2009

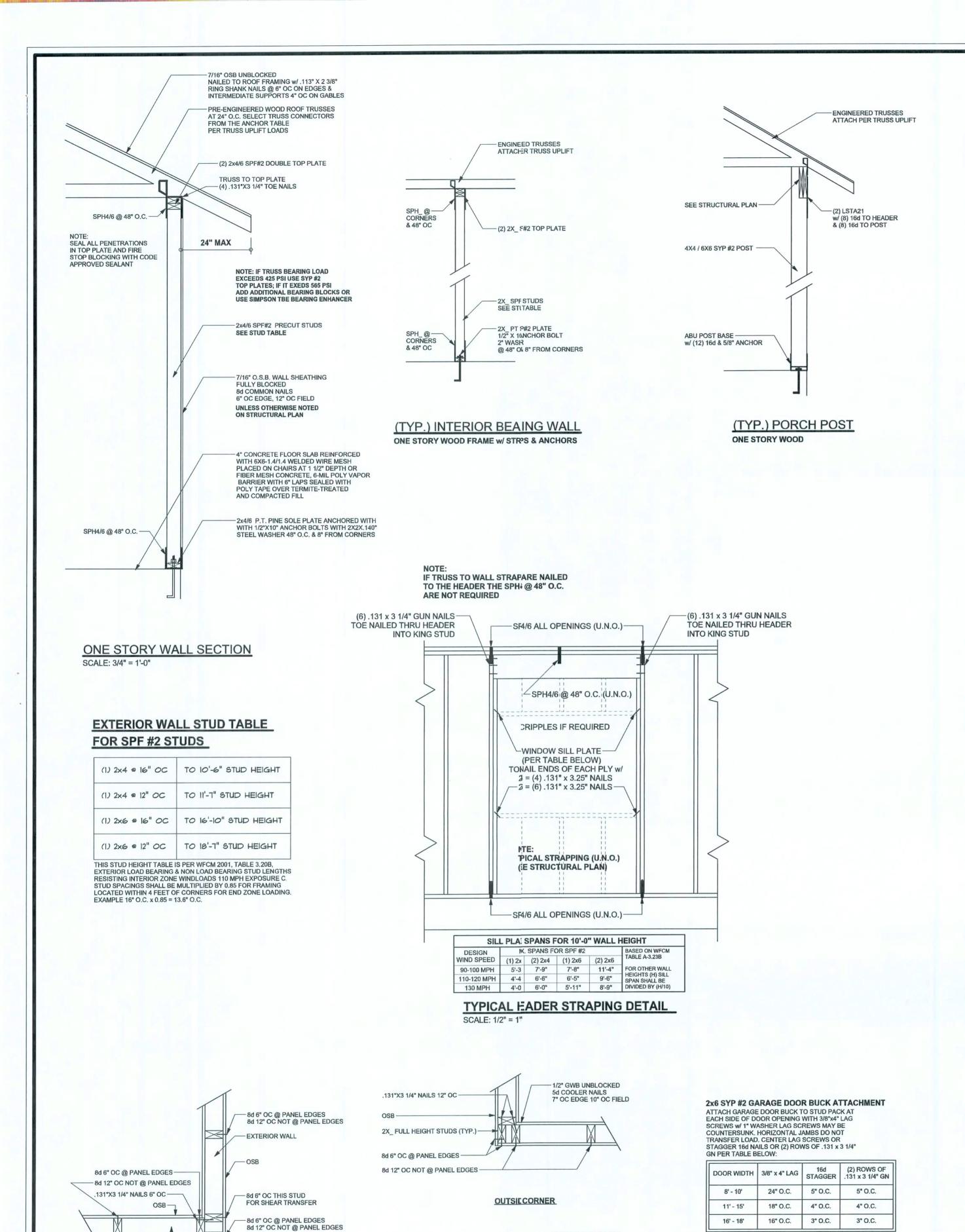
DRAWN BY: CHECKED BY:

DESGNED BY:

FINALS DATE:

DRAWING NUMBER

OF? SHEETS



- 1/2" GWB UNBLOCKED

7" OC EDGE 10" OC FIELD

2x6SYP #2 DOOR BUCK ----

BRACKET. -

GARAGE DOOR BUCK INSTALLATION DETAIL

5d COOLER NAILS

-8d 6" OC @ PANEL EDGES 8d 12" OC NOT @ PANEL EDGES -

INSIDEDRNER

(TYP.) CORNE FRAMING

2X_ FULL HEIGHT STUDS (TYP.)

WOOD FRAME

.131"X3 1/4" NAILS 12" OC -

-2X_FULL HEIGHT STUDS (TYP.)

--- 8d 6" OC @ PANEL EDGES 8d 12" OC NOT @ PANEL EDGES

INTERIOR SHEARWALL -.131"X3 1/4" NAILS 12" OC -

1/2" GWB UNBLOCKED -

7" OC EDGE 10" OC FIELD

(TYP.) INTERSECTING WALL FRAMING

5d COOLER NAILS

GENERAL NOTES:

IF TRUSS TO BEAM

ARE NOT REQUIRED

TO BEAM SPH

(DROPPED BEAM)

ALLOWABLE UPLIFT:

Fb (psi) E (10⁶ psi)

1200 1.6

1050 1.6

1.6

1.7

1.9

975

2400

1600

2900

STRAPS ARE NAILED

OPTION: 2 (POCKETED)

POCKETED

TOP PLATE

OPTION: 1 (BUCKET:T)

(2) 2X_SYP#_{P#2 TOP} PLATE —

18-16d TO 5 FACE 10-10d TO 5 JOIST 3" NOTCH —

BEAM 7, TO BEAR ON -(2) 2X_Y_SPF#2 JACKS

-2X_PT_{PT} SYP#2 PLATE -

(TYP.) BEAM 1 TO WALL

WOOD FRAME w/ STRIRAPS & ANCHORS

2" WASASHER WITHIN 3" OF STUD PACK

2x10

2x12

GRADE & SPECIES TABLE

SYP #2

SYP#2

SYP #2

LSL TIMBERSTRAND 1700

MICROLAM

PARALAM

PRE ENGINEERED ROOF TRUSS -

GLB 24F-V3 SP

DOUBIBLE 2x4 SPF TOP PLATE NAILED -

TOGE SETHER W/2-16d NAILS AT 16" O.C. 4' MIN IN. LAP w/ (12) - 16d OR 4" LAP w/ CS20 0 w/ (4) - 16d &(14) - 10d

ALLL STUDS TO BE 2x4
SPFPF NAILED TO TOP
ANIND BOTTOM PLATES
WIT/ITH 2-16d NAILS

SCALELE: N.T.S.

CODNTINUOUS FRAME TO

CEI:ILING DIAPHRAGM DETAIL

CONTINUCJOUS FRAME -

TO TOP PLPLATE AT

(2) MTS20 -

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2007. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE, STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB:
6" × 6" W1.4 × W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

ACCORDANCE WITH ACI 302, JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25* FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO. NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

	AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH AR Y NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
	ONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND T, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
	IALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2007 FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
BELIEVE THE PL	FINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU AN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL ENGINEER IMMEDIATELY.
DESIGN, PLACE	SS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS IENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, S CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

777	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		-
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d	-1	5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d	-	
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

INDLOAD ENGINEER: Mark Disosway PE No.53915, POB 68, Lake City, FL 2056, 386-754-549 DIMENSIONS:

Stated dimensions spercede scaled nensions. Refer a questions to Mark Disosway, P.E for resolution. o not proceed withut clarification COPYRIGHTS ANDPROPERTY RIGHTS:

REVISIONS

SOFPIAN

Mark Disosway, P.Ehereby expressly reserve its common law coprights and property right in these instruments oservice. This document is not to be reproduce, altered or copied in any form or manner withut first the express writte permission and conent of Mark Disosway. CERTIFICATION: I lereby certify that I have

examined this plan, ind that the applicable

portions of the plan, elating to wind engineeric comply with section 301.2.1, florida building code residential 200, to the best of my

LIMITATION: This design is valid for one building, at specifiedocation.



Bryan Zecher

Construction

William & Beverly Wallace Residence

AIDRESS: 561 NV Bison Court White Spings, FL 32096

Mark Disosway P.E. P.O.Box 868 Lake City, Florida 32056 Phone: (316) 754 - 5419 Fax: (38) 269 - 4871

PRINED DATE: December 10, 2009 DRAWN BY: STRUCTURAL BY David Disosway

FINALS DATE: 10Dec09

> JOB NUMBER: 912022 DRAWING NUMBER

> > OF (SHEETS

ROOF SYSTEM DESIGN

BEARING LOCATIONS.

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2007, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER, IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2007 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

111/42/55	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

DESIGN DATA

MIND LOADS PER FLORIDA BUILDING CODE 2007 ENCLOSED SIMPLE DIAPHRAGM BUILDINGS W MEAN ROOF HEIGHT NOT EXCEEDING LEAST H DN UPPER HALF OF HILL OR ESCARPMENT 60F SLOPE AND UNOBSTRUCTED UPWIND FOR 50x	/ITH FLAT, HI HORIZONTAL FT IN EXP. B.	IPPED, DIMEN	OR GA	OR 60 I	T; NOT	
BUILDING IS NOT IN THE HIGH VELOCITY HURF						
BUILDING IS NOT IN THE WIND-BORNE DEBRIS	REGION					
1.) BASIC WIND SPEED = 110 MPH						
2.) WIND EXPOSURE = C						
3.) WIND IMPORTANCE FACTOR = 1.0						
4.) BUILDING CATEGORY = II						
5.) ROOF ANGLE = 10-45 DEGREES						
6.) MEAN ROOF HEIGHT = <30 FT						
7.) INTERNAL PRESSURE COEFFICIENT = N/A	(ENCLOSED	BUILD	ING)			
8.) COMPONENTS AND CLADDING DESIGN WI				301.20	2))	
	1					
XX.	Zone	Zone Effective Wind Area (ft2)				
		-	0		100	
2 2	1	-	-30.5		-25.3	
	2	27.8	-35.7	25.3	-30.5	
19 2 2	2 O'hg	07.0	-56.8	05.0	-56.8	
2 4 2 3 5	3	27.8	-35.7	25.3	-30.5	
3 4	3 O'hg	20.5	-95.6	05.0	-59.3	
55	5	30.5			-28.5	
	5	30.5	-40.7	25.9	-31.6	
The state of the s		0 14/	lows	30.5	-40.7	
	Doors 8	& vvino	10110	00.0	-40.7	
**************************************	Wors	st Cas	е	00.0	-40.7	
	Wors (Zone	st Cas 5, 10	e ft2)			
	Wors (Zone 8x7 Gara	st Cas 5, 10 age Do	e ft2) oor	27.3	-32.0	
2 2 3 4 5 T	Wors (Zone	st Cas 5, 10 age Do	e ft2) oor			
2 2 3 4 5 T	Wors (Zone 8x7 Gara	st Cas 5, 10 age Do	e ft2) oor	27.3	-32.0	
3 3 2 2 4 3 4 5 5 5 5	Wors (Zone 8x7 Gara	st Cas 5, 10 age Do	e ft2) oor	27.3	-32.0	
3 3 5 5 5 5 5 5 5 5	Wors (Zone 8x7 Gara	st Cas 5, 10 age Do	e ft2) oor	27.3	-32.0	

30 PSF (SLEEPING ROOMS) 30 PSF (ATTICS WITH STORAGE)

16 PSF (4:12 TO <12:12)

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

12 PSF (12:12 AND GREATER)

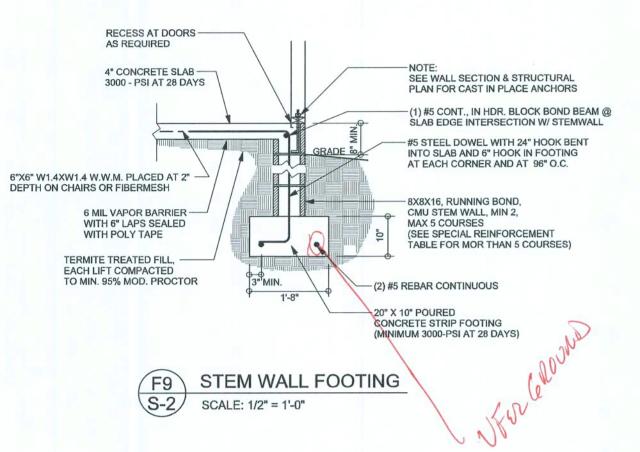
STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

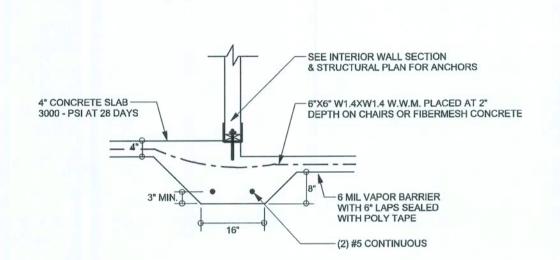
ROOF 20 PSF (FLAT OR <4:12)

SOIL BEARING CAPACITY 1000PSF

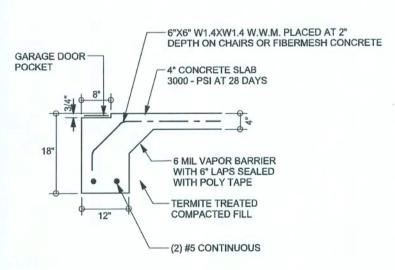
10 PSF (ATTICS WITHOUT STORAGE, <3:12)

SOTPLAN

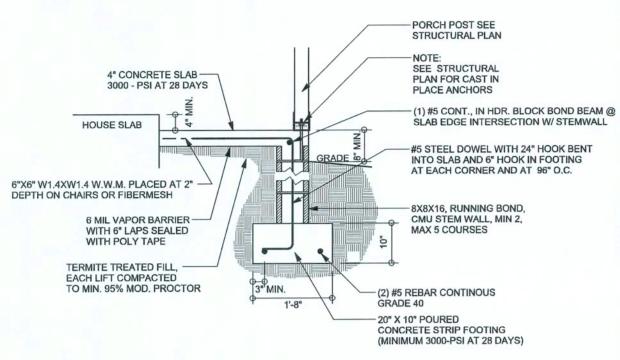




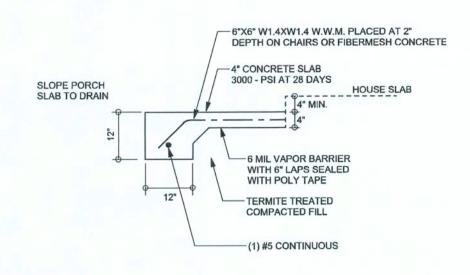
F3 INTERIOR BEARING STEP FOOTING S-2 SCALE: 1/2" = 1'-0"



F4 GARAGE DOOR FOOTING
S-2 SCALE: 1/2" = 1'-0"



F12 STEM WALL PORCH FOOTING S-2 SCALE: 1/2" = 1'-0"

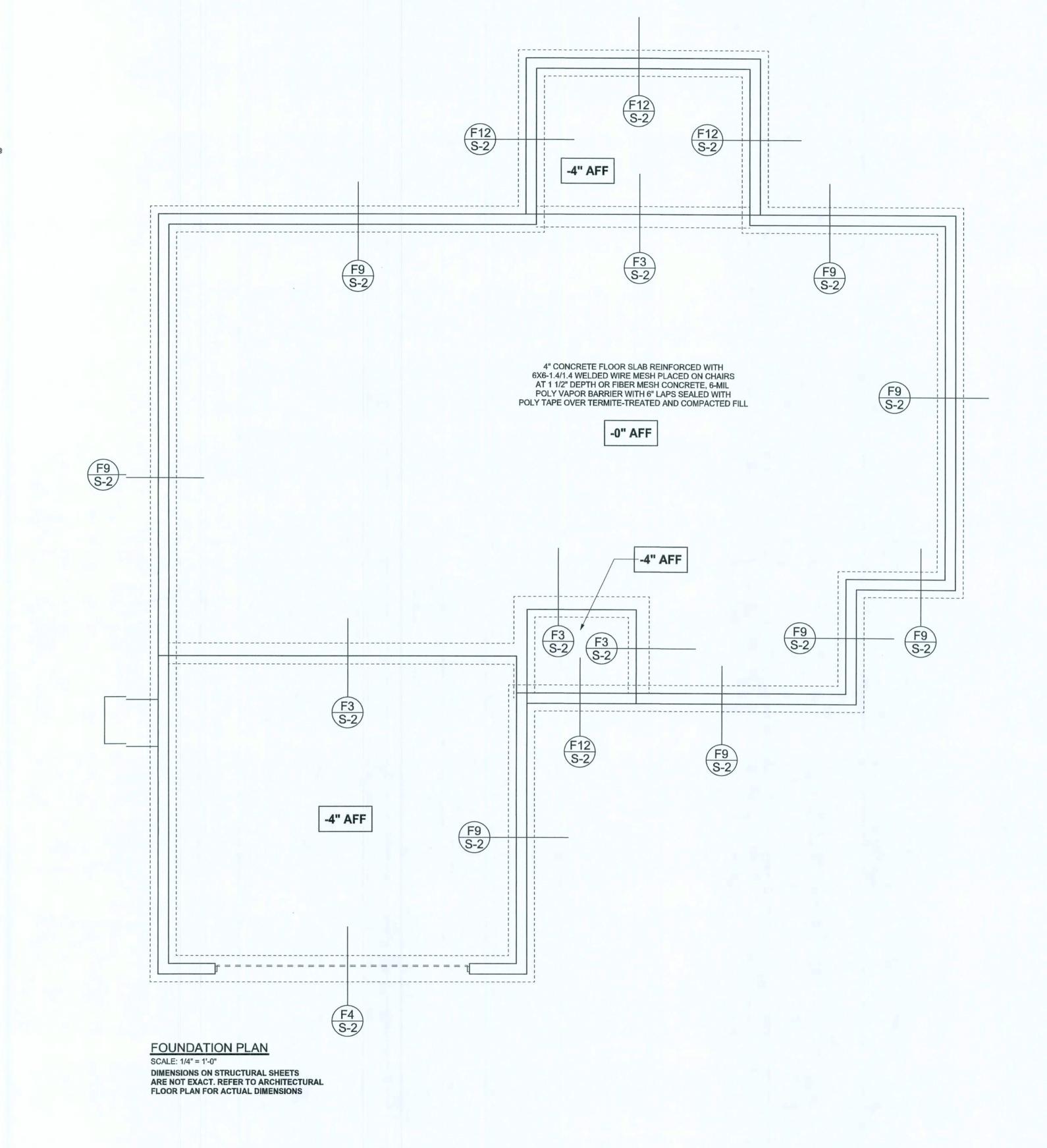


F5 PORCH FOOTING
S-2 SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid helght. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	FOR 8	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)		VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
	#5	#7	#8	#5	#7	#8	
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



WINDLOAD ENGNEER: Mark Disosway, PE No.53915, Pt 868, Lake City, FL 32056, 386-754-419

DIMENSIONS: Stated dimension supercede scaled dimensions. Refe all questions to Mark Disosway, I.E. for resolution, Do not proceed \(\formalfortambox{1}\)thout clarification.

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form or manner without first the express written
permission and onsent of Mark Disosway.

CERTIFICATION I hereby certify that I have examined this pla, and that the applicable portions of the pln, relating to wind engineering comply with sectin R301.2.1, florida building code residential 207, to the best of my knowledge.

LIMITATION: Thi design is valid for one building, at specied location.

MRK DISOSWAY
P.E. 53915

Bryan Zecher Construction

William & Beverly Wallace Residence

ADDRESS: 561 IW Bison Court White prings, FL 32096

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PIINTED DATE:
Decimber 10, 2009

DRAWN BY: STRUCTURAL BY:

David Disosway

FINALS DATI: 10Dec09

JOBNUMBER: 912022 DRAVING NUMBER

S-2O' 3 SHEETS

