

# COLUMBIA COUNTY BUILDING DEPARTMENT RESIDENTIAL CHECK LIST

MINIMUM PLAN REQUIREMENTS: FLORIDA BUILDING CODE RESIDENTIAL 2017 EFFECTIVE 1 JANUARY 2018

AND THE NATIONAL ELECTRICAL 2014 EFFECTIVE 1 JANUARY 2018

#### **ALL REQUIREMENTS ARE SUBJECT TO CHANGE**

ALL BUILDING PLANS MUST INDICATE COMPLIANCE WITH THE CURRENT FLORIDA BUILDING CODES RESIDENTIAL AND THE NATIONAL ELECTRICAL CODE. ALL PLANS OR DRAWINGS SHALL PROVIDE CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS, FBC 1609.3.1 THRU 1609.3.3.

FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEEDS ARE PER FLORIDA BUILDING CODE FIGURE 1609-A
THROUGH 1609-C ULTIMATE DESIGN WIND SPEEDS FOR RISK CATEGORY AND BUILDINGS AND OTHER STRUCTURES
Revised 7/1/18

	Website: http://www.columbiacountyfla.com/ GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BO	XES BEFORE SUBMITTAL		Each I Ci Ap	to Inclu Box shai rcled as plicable	l be
			Selec	ct From	n Drop	down
1	Two (2) complete sets of plans containing the following:		1			
2	All drawings must be clear, concise, drawn to scale, details that	are not used shall be marked void	7			
3		Ft.) under roof 2,442	Ye	es	No	NA

Designers name and signature shall be on all documents and a licensed architect or engineer, signature and official embossed seal shall be affixed to the plans and documents as per the FLORIDA BUILDING CODES RESIDENTIAL 107.1.

## Site Plan information including:

4	Dimensions of lot or parcel of land	Yes	
5	Dimensions of all building set backs	Yes	
6	Location of all other structures (include square footage of structures) on parcel, existing or proposed well and septic tank and all utility easements.	Yes	
7	Provide a full legal description of property.	Yes	

## Wind-load Engineering Summary, calculations and any details are required.

	GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		to Inclu Box shal ircled as licable	hall be	
8	Plans or specifications must show compliance with FBCR Chapter 3	Yes	No	NA	
		Select Fro	m Drop	down	
9	Basic wind speed (3-second gust), miles per hour	-405			
10	(Wind exposure – if more than one wind exposure				
	is used, the wind exposure and applicable wind direction shall be indicated)	yes			
11	Wind importance factor and nature of occupancy	-Yes			
12	The applicable internal pressure coefficient, Components and Cladding	-yes			
13	The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component, cladding materials not specifally designed by the registered design professional.	-yes			
Ele	evations Drawing including:	-			
14	All side views of the structure	Yes			
15	Roofpitch	Yes	1		
16	Overhang dimensions and detail with attic ventilation	Yes	1		
17	Location, size and height above roof of chimneys	NA	-		
18	Location and size of skylights with Florida Product Approval	NA	1		
19	Number of stories	Yes			
20	Building height from the established grade to the roofs highest peak	Yes			

Floor Plan Including:

Annualis			
21	Dimensioned area plan showing rooms, attached garage, breeze ways, covered porches, deck, balconies	Yes	
22	Raised floor surfaces located more than 30 inches above the floor or grade	NA	
23	All exterior and interior shear walls indicated	Yes	
24	Shear wall opening shown (Windows, Doors and Garage doors)	Yes	
25	Show compliance with Section FBCR 310 Emergency escape and rescue opening shown in each		
	bedroom (net clear opening shown) and Show compliance with Section FBC 1405.13.2 where the opening of an operable window is located more than 72 inches above the finished grade or surface below, the lowest part of the clear opening of the window shall be a minimum of 24 inches above the finished floor of the room in which the window is located. Glazing between the floor and 24 inches shall be fixed or have openings through which a 4-inch-diameter sphere cannot pass.	Yes	
26	Safety glazing of glass where needed	NA	
27	Fireplaces types (gas appliance) (vented or non-vented) or wood burning with Hearth (see chapter 10 and chapter 24 of FBCR)	NA	
28	Show stairs with dimensions (width, tread and riser and total run) details of guardrails, Handrails	NA	
29	Identify accessibility of bathroom (see FBCR SECTION 320)	Yes	

All materials placed within opening or onto/into exterior walls, soffits or roofs shall have Florida product approval number and mfg. installation information submitted with the plans (see Florida product approval form)

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be
	Circled as Applicable

### FBCR 403: Foundation Plans

		Select 1	From Drop down
	Location of all load-bearing walls footings indicated as standard, monolithic, dimensions, size and type of reinforcing.	Yes	
31	All posts and/or column footing including size and reinforcing	NA	
32	Any special support required by soil analysis such as piling.	NA	
33		Yes	
34	Location of horizontal and vertical steel, for foundation or walls (include # size and type) For structures with foundation which establish new electrical utility companies service connection a Concrete Encased Electrode will be required within the foundation to serve as an grounding electrode system. Per the National Electrical Code article 250.52.3	Yes	

#### FBCR 506: CONCRETE SLAB ON GRADE

3	5 Show Vapor retarder (6mil. Polyethylene with 'pints la ph 6 inches and sealed)	Yes		]
3	6 Show control j oints, synthetic fiber reinforcement or welded fire fabric reinforcement and Sports	Yes		Ī

#### FBCR 318: PROTECTION AGAINST TERMITES

	Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or		
37	Submit other approved termite protection methods. Protection shall be provided by registered termiticides	Yes	

### FBCR 606: Masonry Walls and Stem walls (load bearing & shear Walls)

F -	18 St 11	1.4	
L •	8 Show all materials making up walls, wall height, and Block size, mortar type	Yes	1 1
	Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement	NA	

Metal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof. Engineer or Architect

Floor Framing System: First and/or second story

40	Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer	NA	
41	Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, stem walls and/or priers	NA	
42	Girder type, size and spacing to load bearing walls, stem wall and/or priers	Yes	
43	Attachment of joist to girder	Yes	
44	Wind load requirements where applicable	Yes	
45	Show required under-floor crawl space	NA	
46	Show required amount of ventilation opening for under-floor spaces	NA	
47	Show required covering of ventilation opening	NA	
48	Show the required access opening to access to under-floor spaces	NA	
49	Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges & intermediate of the areas structural panel sheathing	NA	
50	Show Draftstopping, Fire caulking and Fire blocking	NA	
51	Show fireproofing requirements for garages attached to living spaces, per FBCR section 302.6	NA	
52	Provide live and dead load rating of floor framing systems (psf).	NA	

## FBCR CHAPTER 6 WOOD WALL FRAMING CONSTRUCTION

	GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Each I	to Include- Box shall be ircled as oplicable	
		Select from	n Drop do	WI
53	Stud type, grade, size, wall height and oc spacing for all load bearing or shear walls	Yes		- 55
	Contained askedula Constructional and asked CDC DCD2 2.2			_

	<u>-                                    </u>		
53	Stud type, grade, size, wall height and oc spacing for all load bearing or shear walls	Yes	
54	Fastener schedule for structural members per table FBC-R602.3.2 are to be shown	Yes	
55	Show wood structural panel's sheathing attachment to studs, joist, trusses, rafters and structural members, showing fastener schedule attachment on the edges & intermediate of the areas structural panel sheathing	Yes	
56	Show all required connectors with a max uplift rating and required number of connectors and oc spacing for continuous connection of structural walls to foundation and roof trusses or rafter systems	Yes	
57	Show sizes, type, span lengths and required number of support jack studs, king studs for shear wall opening and girder or header per FBC-R602.7.	Yes	
58	Indicate where pressure treated wood will be placed	Yes	
59	Show all wall structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing edges & intermediate areas	Yes	
60	A detail showing gable truss bracing, wall balloon framing details or/ and wall hinge bracing detail	Yes	

## **FBCR:ROOF SYSTEMS:**

61	Truss design drawing shall meet section FBC-R 802.10. 1 Wood trusses	Yes	
62	Include a layout and truss details, signed and sealed by Florida Professional Engineer	Yes	
	Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters	Yes	
64	Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details	Yes	
65	Provide dead load rating of trusses	Yes	

# FBCR 802:Conventional Roof Framing Layout

66	Rafter and ridge beams sizes, span, species and spacing	NA	
67	Connectors to wall assemblies' include assemblies' resistance to uplift rating	NA	
68	Valley framing and support details	NA	
69	Provide dead load rating of rafter system	NA	

## **FBCR 803 ROOF SHEATHING**

70	Include all materials which will make up the roof decking, identification of structural panel sheathing, grade, thickness	Yes	
71	Show fastener Size and schedule for structural panel sheathing on the edges & intermediate areas	Yes	

**ROOF ASSEMBLIES FRC Chapter 9** 

<u> </u>	Include all materials which will make up the roof assembles covering	Yes		
73	Submit Florida Product Approval numbers for each component of the roof assembles covering	Yes		

### FBCR Chapter 11 Energy Efficiency Code for Residential Building

Residential construction shall comply with this code by using the following compliance methods in the FBCR Chapter 11 Residential buildings compliance methods. Two of the required forms are to be submitted, N1100.1.1.1 As an alternative to the computerized Compliance Method A, the Alternate Residential Point System Method hand calculation, Alternate Form 600A, may be used. All requirements specific to this calculation are located in Sub appendix C to Appendix G. Buildings complying by this alternative shall meet all mandatory requirements of this chapter. Computerized versions of the Alternate Residential Point System Method shall not be acceptable for code compliance.

	GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to In Each Box Circle Applic	shall be d as able
	Science	elect from Di	rop Dow
74	Show the insulation R value for the following areas of the structure	Yes	T
75	Attic space	Yes	
76	Exterior wall cavity	Yes	
77	Crawl space	NA	
Н	AC information		
78	Submit two copies of a Manual J sizing equipment or equivalent computation study	Yes	T
79	Exhaust fans shown in bathrooms Mechanical exhaust capacity of 50 cfm intermittent or		
	20 cfm continuous required	Yes	
80	Show clothes dryer route and total run of exhaust duct	No	
PI:	ımbing Fixture layout shown		1
	All fixtures waste water lines shall be shown on the foundation lan	Yes	
82	Show the location of water heater	Yes	_
02	Show the location of water heater	res	
	vate Potable Water		
	Pump motor horse power	NA	
	Reservoir pressure tank gallon capacity	NA	
85	Rating of cycle stop valve if used	NA	
Ele	ectrical layout shown including		
86	Show Switches, receptacles outlets, lighting fixtures and Ceiling fans	Yes	T
87	Show all 120-volt, single phase, 15- and 20-ampere branch circuits outlets required to be protected		
	by Ground-Fault Circuit Interrupter (GFCI) Article 210.8 A	Yes	
88	Show the location of smoke detectors & Carbon monoxide detectors	Yes	
89	Show service panel, sub-panel, location(s) and total ampere ratings	Yes	
90	On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Indicate if the utility company service entrance cable will be of the overhead or underground type.	Yes	
24	For structures with foundation which establish new electrical utility companies service connection a Concrete Encased Electrode will be required within the foundation to serve as an Grounding electrode system. Per the National Electrical Code article 250.52.3		
91	Appliances and HVAC equipment and disconnects	Yes	
92	Show all 120-volt, single phase, 15- and 20-ampere branch circuits supplying outlets installed in dwelling unit family rooms, dining rooms, living rooms, parlors, libraries, dens, bedrooms, sunrooms, recreation rooms, closets, hallways, or similar rooms or areas shall be protected by a listed Combination arc-fault circuit interrupter, Protection device.	Yes	

## Notice Of Commencement:

A notice of commencement form RECORDED in the Columbia County Clerk Office is required to be filed with the Building Department BEFORE ANY INSPECTIONS can be performed.

	Items to Include-
GENERAL REQUIREMENTS:	Each Box shall be
APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Circled as
	Applicable

## \*\*ITEMS 95, 96, & 98 Are Required After APPROVAL from the ZONING DEPT.\*\*

Select from Drop down

	3	егесі згот	Drop !	aow
93	Building Permit Application A current Building Permit Application is to be completed, by following the Checklist all supporting documents must be submitted.  There is a \$15.00 application fee. The completed application with attached documents and application fee can be mailed.	Yes		
94	Parcel Number The parcel number (Tax ID number) from the Property Appraisers Office (386) 758-1083 is required. A copy of property deed is also required. www.columbiacountyfla.com	Yes		
95	Environmental Health Permit or Sewer Tap Approval A copy of a approved Columbia County Environmental Health (386) 758-1058	Yes		
96	City of Lake City A City Water and/or Sewer letter. Call 386-752-2031	Yes		
97	Toilet facilities shall be provided for all construction sites	Yes		
98	Town of Fort White (386) 497-2321 If the parcel in the application for building permit is within the Corporate city limits of Fort White, an approval land use development letter issued by the Town of Fort is required to be submitted with the application for a building permit.	NA		
99	Flood Information: All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting a application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.5.2 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.5.3 of the Columbia County Land Development Regulations (Municode.com)	NA		
100	CERTIFIED FINISHED FLOOR ELEVATIONS will be required on any project where the approved FIRM Flood Maps show the property is in a AE, Floodway, and AH flood zones. Additionally One Foot Rise letters are required for AE and AH zones. In the Floodway Flood zones a Zero Rise letter is required.	NA		
101	A Flood development permit is also required for AE, Floodway & AH. Development permit cost is \$50.00	NA		
102	<b>Driveway Connection:</b> If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. County Public Works Dept. determines the size and length of every culvert before instillation and completes a final inspection before permanent power is granted. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00) Separate Check when issued. If the project is to be located on an F.D.O.T. maintained road, then an F.D.O.T. access permit is required.	Yes		
103	911 Address: An application for a 911 address must be applied for and received through the Columbia County Emergency Management Office of 911 Addressing Department (386) 758-1125.	Yes		

Ordinance Sec. 90-75. - Construction debris. (e) It shall be unlawful for any person to dispose of or discard solid waste, including construction or demolition debris at any place within the county other than on an authorized disposal site or at the county's solid waste facilities. The temporary storage, not to exceed seven days of solid waste (excluding construction and demolition debris) on the premises where generated or vegetative trash pending disposition as authorized by law or ordinance, shall not be deemed a violation of this section. The temporary storage of construction and demolition debris on the premises where generated or vegetative trash pending disposition as authorized by law or ordinance shall not be deemed in violation of this section; provided, however, such construction and demolition debris must be disposed of in accordance with this article prior to the county's issuance of a certificate of occupancy for the premises. The burning of lumber from a construction or demolition project or vegetative trash when done so with legal and proper permits from the authorized agencies and in accordance with such agencies' rules and regulations, shall not be deemed a violation of this section. No person shall bury, throw, place, or deposit, or cause to be buried, thrown, placed, or deposited, any solid waste, special waste, or debris of any kind into or on any of the public streets, road right-of-way, highways, bridges, alleys, lanes, thoroughfares, waters, canals, or vacant lots or lands within the county. No person shall bury any vegetative trash on any of the public streets, road right-of-way, highways, bridges, lanes, thoroughfares, waters, canals, or lots less than ten acres in size within the county.

As required by Florida Statute 553.842 and Florida Administrative Code 98-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed online @ www.floridabuilding.org

Catagory/Subcatagory	Manufacturer	Product Description	Approval Number(s)
1. EXTERIOR DOORS			
A. SWINGING	Mesonite Int	tibergless doors	FL8228-1
8. SLIDING		<del>ا</del>	40
C. SECTIONAL/ROLL UP		•	
D. OTHER		84	
T <sub>i</sub>			Name of the last o
Z. WINDOWS		ų.	
A. SINGLE/DOUBLE HUNG	Atrium	S/H windows	FL 20123-1
B. HORIZONTAL SLIDER	77.2	40 g 50	-9
C. CASEMENT			
D. FIXED	Atrium	Fixed windsous	FL30471-1
E. MULLION			
F. SKYLIGHTS			
G. OTHER			i lie
	·		
3. PANEL WALL			
A. SIDING	James Hardie	Fiber Coment Siline	FL 13192-2
B. SOFFITS	Jenes Herdie	Fiber Cement Siding	FL 13265 -1
C. STOREFRONTS	T-	2 2 4 H	
D. GLASS BLOCK			
E. OTHER	Jemes Hardie	Harlie Shehes	EL 13192-4
4. ROOFING PRODUCTS			
A. ASPHALT SHINGLES	GAF	timberline HD Shingles	FL10124-1
B. NON-STRUCT METAL		3	
C. ROOFING TILES			
D. SINGLE PLY ROOF			
E. OTHER	junderlay ment Faf	· truer Pour	FL 10626-1.
5. STRUCT COMPONENTS	No.		
A. WOOD CONNECTORS	Simpson	word Connectors	FL 10007-R7
B. WOOD ANCHORS		1	
C. TRUSS PLATES			
D. INSULATION FORMS	T.	0	
E. LINTELS		2	
F. OTHERS			
	A		
6. NEW EXTERIOR			
ENVELOPE PRODUCTS			
2171201211000010	9		

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements.

Surther, understand these products may have to be removed if approval cannot be demonstrated during inspection.

NOTES:	45		
			_
	(3)	0 *	

# FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Business and Professional Regulation - Residential Performance Method

Project Name: Lot 19' Tur her Creels Street: City, State, Zip: Lake City, FL, 32055 Owner: Design Location: FL, Gainesville	Builder Name: Lipscomb & Eagle Permit Office: Columbia County Permit Number: Jurisdiction: County: Columbia (Florida Climate Zone 2)
1. New construction or existing 2. Single family or multiple family 3. Number of units, if multiple family 4. Number of Bedrooms 5. Is this a worst case? 6. Conditioned floor area above grade (ft²) Conditioned floor area below grade (ft²) 7. Windows (240.0 sqft.) Bescription Bescri	9. Wall Types (1602.0 sqft.)  a. Frame - Wood, Exterior  b. Frame - Wood, Adjacent  c. N/A  d. N/A  R=  ft²  d. N/A  R=  ft²  10. Ceiling Types (1762.0 sqft.)  a. Under Attic (Vented)  b. N/A  R=  ft²  c. N/A  R=  ft²  11. Ducts  a. Sup: Attic, Ret: Attic, AH: Garage  12. Cooling systems  a. Central Unit  13. Heating systems  a. Electric Heat Pump  Compliance  14. Hot water systems  a. Electric  Conservation features  None
Glass/Floor Area: 0.143 Total Proposed Modified Total Baseline	PASS
I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.  PREPARED BY:  DATE:  I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.  OWNER/AGENT:  DATE:	Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.  BUILDING OFFICIAL:  DATE:

- Compliance requires certification by the air handler unit manufacturer that the air handler enclosure qualifies as certified factory-sealed in accordance with R403.3.2.1.
- Compliance requires an Air Barrier and Insulation Inspection Checklist in accordance with R402.4.1.1 and this project requires an envelope leakage test report with envelope leakage no greater than 5.00 ACH50 (R402.4.1.2).

**INPUT SUMMARY CHECKLIST REPORT** 

	,			PROJE	ECT							
Title: Building T Owner Na # of Units Builder Na Permit Of Jurisdictic Family Ty New/Exis Comment	ame:  1 tipscomb & Ea ffice: Columbia Coulon:  pe: Single-family ting: New (From Pla	nty	Bedrooms: Conditioned Total Storie Worst Case Rotate Angl Cross Venti Whole Hous	s: e: e: ilation:	3 1678 1 No 0 Yes No		Lot# Block PlatB Stree Coun	k/Subdivis look: et:	19 sion: Co c La	ot Informat olumbia ake City ,		
				CLIMA	TE							
<b>√</b>	Design Location	TMY Site		97	esign Temp .5 % 2.5 %	Winte	esign Tem er Summ	ier Deg	eating ree Days		re Ra	Temp
	FL, Gainesville	FL_GAINESVILLE	:_REGI		32 92	70	75	1	305.5	51	M	edium
				BLOC	KS 	4						
Number	r Name	Area	Volume							<del></del>		····
1	Block1	1678	15102					-				
				SPAC	ES							
Number	r Name	Area	Volume K	itchen	Occupants	Bedroo	ms li	nfil ID	Finished	Coc	oled	Heate
1	Main	1678	15102	Yes	6	3	1		Yes	Yes	:	Yes
				FLOO	RS			,				
$\checkmark$	# Floor Type	Space	Perin	neter	R-Value	Area				Tile W	ood Ca	rpet
	1 Slab-On-Grade Edge	Insulation M	ain 181	ft	0	1678 ft²				0	0	1
				ROO	F					No.		
<b>/</b>	# Туре	Materials	Roof Area	Gable Area		Rad Barr	Solar Absor.	SA Tested	Emitt	Emitt Tested	Deck Insul.	Pitcl (deg
	1 Hip	Composition shing	les 1943 ft²	0 ft²	Medium	Υ	0.96	No	0.9	No	0	30.3
				ATTI	С		7					
$\checkmark$	# Туре	Ventil	ation	Vent Rati	o (1 in)	Area	RBS	IRO	СС			
	1 Full attic	Ven	ted	300	) 1	678 ft²	Υ	١	٧			
	· · · · · · · · · · · · · · · · · · ·			CEILI	NG		_					-
$\sqrt{}$	# Ceiling Type		Space	R-Value	e Ins Ty	pe	Area	Fran	ning Frac	Truss	Туре	
	1 Under Attic (Ve	ented)	Main	38	Double E	Batt	1762 ft²	-	0.11	W	ood	15

EODM DAGE 2047	INPUT SUMMARY CHECKLIST REPORT
FORM R405-2017	INPUT SUMMART CHECKLIST REPORT

RM F	₹405-	2017			INPUT	<u>SUMMA</u>			<u> </u>	EP	UKI					
							WA	LLS								
V #	Ornt		Adjace To	ent —Wall	Туре	Space	Cavity R-Value	Wid ——Ft	th _In	He Ft	ight In	Area	Sheathing R-Value	Framing Fraction	Solar Absor.	Belov Grade
1	S	ε	xterior	Frai	me - Wood	Main	13	17		9		153.0 ft <sup>2</sup>		0.23	0.75	(
2	W	E	xterior	Frai	me - Wood	Main	13	9		9		81.0 ft²		0.23	0.75	C
3	S	E	xterior	Fran	me - Wood	Main	13	2	8	9		24.0 ft <sup>2</sup>		0.23	0.75	C
4	S	G	arage	Fran	me - Wood	Main	13	21	8	9		195.0 ft <sup>2</sup>		0.23	0.75	C
5	Ε	E	xterior	Fran	me - Wood	Main	13	45		9		405.0 ft <sup>2</sup>		0.23	0.75	(
6	N	E	xterior	Fran	me - Wood	Main	13	24	4	9		219.0 ft <sup>2</sup>		0.23	0.75	(
7	W	E	xterior	Fran	me - Wood	Main	13	8		9		72.0 ft <sup>2</sup>		0.23	0.75	(
8	N	E	xterior	Fran	me - Wood	Main	13	17		9		153.0 ft²		0.23	0.75	C
9	W	E	xterior	Fran	me - Wood	Main	13	33	4	9		300.0 ft <sup>2</sup>		0.23	0.75	C
							DO	ORS								
$\checkmark$	#		Ornt	Ì	Door Type	Space			Storms		U-Valu	e F	Width t In	Height Ft I	ln	Area
	1		s		Insulated	Main			None		.46	3		6	8	20 ft²
	2		S		Insulated	Main			None		.46	3	l .	6	8	20 ft²
					(	Orientation sh	WIND own is the er	OWS tered, F		orie	ntation.					
$\sqrt{}$	#	Ornt	Wall ID	Frame	Panes	NFRC	U-Factor	SHGC	lmp	)	Area		rhang Separation	Int Sha	de	Screeni
	1	S	1	Vinyl	Low-E Double	Yes	0.36	0.25	N	3	0.0 ft²	10 ft 6 in	1 ft 0 in	None	•	None
	2	Ε	5	Vinyl	Low-E Double	Yes	0.36	0.25	N	3	0.0 ft²	1 ft 6 in	1 ft 0 in	None	•	None
	3	Ν	6	Vinyl	Low-E Double	Yes	0.36	0.25	N	4	5.0 ft²	1 ft 6 in	1 ft 0 in	None	•	None
	4	Ν	8	Metal	Low-E Double	Yes	0.36	0.25	N	6	0.0 ft²	9 ft 6 in	1 ft 0 in	None	•	None
	5	W	9	Vinyl	Low-E Double	Yes	0.36	0.25	N	7	5.0 ft²	1 ft 6 in	1 ft 0 in	None	•	None
							GAR	AGE								
$\sqrt{}$	#	•	Floo	r Area	Ceilin	g Area	Exposed V	Vall Peri	imeter	/	Avg. Wa	ll Height	Expose	ed Wall Ins	ulation	
	1		498.3	341 ft²	498.3	341 ft²	65	.67 ft			9	ft		1		
							INFILTI	RATIC	N							
: 5	Scope			/lethod		SLA	CFM 50	ELA		EqLA		ACH	ACH	1 50	S(f	
	olehou	se		osed AC	H(50) .00		1258.5	69.09		29.9		.1128		<del></del>		
•							HEATING	SYS	TEM							
$\sqrt{}$	#	Sy	stem T	уре	Sı	ubtype	Speed		Efficien	су	C	apacity		E	Block	Ducts
				leat Pum		one	Single		HSPF:8			4 kBtu/hr			1	sys#1

					COOL	LING SYS	TEM						
V	#	System Type		Subtype	Sub	otype	Efficiency	Capacity	Air	Flow	SHR	Block	Ducts
	1	Central Unit/		None	Sin	gle	SEER: 14	20.76 kBtu/	hr 630	cfm	0.7	1	sys#1
					HOT W	ATER SY	STEM					•	
	#	System Type	SubType	Locatio	n EF	Ca	ар	Use	SetPnt		Co	nservatio	n
	1	Electric	None	Garage	0.92	40 (	gal	40 gal	120 deg			None	
_				S	DLAR HO	T WATER	RSYSTI	EM					
$\checkmark$	FSEC Cert #	CompanyN	lame		System	Model#	C	ollector Model		ollector Area		age ume	FEF
	None	None								ft²			
						DUCTS						***	
<b>/</b>	#		oply R-Value Area	F Locatio	Return on Area	Leaka	geType	Air Handler	CFM 25 TOT	CFM2 OUT		RLF	HVAC #
	1	Attic	6 419.5 f	t² Attic	83.9 ft²	Default	Leakage	Garage	(Default)	c(Defau	ılt) c		1 1
_					TEM	PERATU	RES						
Program	ableThe	rmostat: Y			Ceiling Fans	s:							
Cooling Heating Venting	X) 1: X) 1: [	an [] Feb an [] Feb an [] Feb	[ ] Mar  X  Mar  X  Mar	Apr Apr X Apr	May May May	(X) Jun   ] Jun   ] Jun	(X) Jul     Jul     Jul	[X] Aug     Aug     Aug	[X] Se [ ] Se [ ] Se		Oct Oct Oct	X Nov X Nov X Nov	Dec  X Dec  Dec
Thermosta Schedule T		ıle: HERS 20	06 Reference	2		<b>E</b>	H-	ours 7	0		10	44	10
Cooling (W		ΔM	1 78	78 7					78	9 80	10	11 80	12
	·	AM PM	78 80	78 78 80 78		78 78	78 78	78 78	78 78	80 78	80 78	80 78	80 78
Cooling (W	EH)	AM PM	78 78	78 78 78 78	8 78 8 78	78 78	78 78	78 78	78 78	78 78	78 78	78 78	78 78
Heating (W	D)	AM PM	66 68	66 66 68 66	6 66 8 68	66 68	68 68	68 68	68 68	68 68	68 68	68 66	68 66
Heating (W	EH)	AM PM	66 68	66 66 68 66	6 66 8 68	66 68	68 68	68 68	68 68	68 68	68 68	68 66	68 66
	_					MASS	_		_				
Ma	ss Type	}		Area		Thickness		Furniture Fra	ction		Space		-
De	fault(8 II	os/sq.ft.		0 ft²		0 ft		0.3			Main		

# **ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD**

## **ESTIMATED ENERGY PERFORMANCE INDEX\* = 100**

The lower the Energy Performance Index, the more efficient the home.

1. New home or, addition	1. New (From Plans)	12. Ducts, location & insulation level
2. Single-family or multiple-family	2. Single-family	a) Supply ducts R 6.0 b) Return ducts R 6.0 c) AHU location Garage
3. No. of units (if multiple-family)	31	o, Arto location
4. Number of bedrooms	43	13. Cooling system: Capacity 20.8 a) Split system SEER
5. Is this a worst case? (yes/no)	5. <u>No</u>	b) Single package SEER c) Ground/water source SEER/COP
6. Conditioned floor area (sq. ft.)	6. <u>1678</u>	d) Room unit/PTAC EER e) Other 14.0
<ul><li>7. Windows, type and area</li><li>a) U-factor:(weighted average)</li><li>b) Solar Heat Gain Coefficient (SHGC)</li><li>c) Area</li></ul>	7a. 0.360 7b. 0.250 7c. 240.0	14. Heating system: Capacity 26.4 a) Split system heat pump HSPF b) Single package heat pump HSPF
8. Skylights		c) Electric resistance COP
<ul><li>a) U-factor:(weighted average)</li><li>b) Solar Heat Gain Coefficient (SHGC)</li></ul>	8a. <u>NA</u> 8b. <u>NA</u>	d) Gas furnace, natural gas AFUE e) Gas furnace, LPG AFUE f) Other 8.20
9. Floor type, insulation level:		
a) Slab-on-grade (R-value)	9a0.0	45 Metallandian austria
b) Wood, raised (R-value)	9b	15. Water heating system
c) Concrete, raised (R-value)	9c	a) Electric resistance EF 0.92 b) Gas fired, natural gas EF E
10. Wall type and insulation:		c) Gas fired, LPG EF
A. Exterior:		d) Solar system with tank EF
1. Wood frame (Insulation R-value)	10A1. 13.0	e) Dedicated heat pump with tank EF
Masonry (Insulation R-value)     B. Adjacent:	10A2	f) Heat recovery unit HeatRec% g) Other
Wood frame (Insulation R-value)	10B1. 13.0	3,
2. Masonry (Insulation R-value)	10B2	
,		16. HVAC credits claimed (Performance Method)
11. Ceiling type and insulation level		a) Ceiling fans
a) Under attic	11a. <u>38.0</u>	b) Cross ventilation Yes
b) Single assembly	11b	c) Whole house fan No
c) Knee walls/skylight walls	11c	d) Multizone cooling credit
d) Radiant barrier installed	11d. <u>Yes</u>	e) Multizone heating credit
		f) Programmable thermostat Yes
*Label required by Section R303.1.3 of the Fl	orida Building Code, Ene	ergy Conservation, if not DEFAULT.
I certify that this home has complied with the saving features which will be installed (or exc display card will be completed based on installed)	eeded) in this home befo	
Builder Signature:		Date:
Address of New Home:		City/FL Zip:Lake City, FL 32055
VARIESS OF 14EM FIGHTS:		City/i L Zip. Lake City, FL 32000

# **Envelope Leakage Test Report (Blower Door Test)**

Residential Prescriptive, Performance or ERI Method Compliance 2017 Florida Building Code, Energy Conservation, 6th Edition

	Jurisdiction:	Permit #:										
Jok	b Information											
Bui	llder: Lipscomb & Eagle Community:	Lot: 19										
Adc	dress:											
City	y: Lake City State	e: FL Zip: 32055										
Air	r Leakage Test Results Passing results must meet	t either the Performance, Prescriptive, or ERI Method										
C	PRESCRIPTIVE METHOD-The building or dwelling unit shall be tes changes per hour at a pressure of 0.2 inch w.g. (50 Pascals) in Clim	sted and verified as having an air leakage rate of not exceeding 7 air nate Zones 1 and 2.										
the	PERFORMANCE or ERI METHOD-The building or dwelling unit shall be tested and verified as having an air leakage rate of not exceeding the selected ACH(50) value, as shown on Form R405-2017 (Performance) or R406-2017 (ERI), section labeled as infiltration, sub-section ACH50.  ACH(50) specified on Form R405-2017-Energy Calc (Performance) or R406-2017 (ERI):  5.000											
:	x 60 ÷ 15102 = ACH(50)  PASS  When ACH(50) is less than 3, Mechanical Ventilation in must be verified by building department.	Method for calculating building volume:  Retrieved from architectural plans Code software calculated installation Field measured and calculated										
Duri 1. E con: 2. D mea 3. Ir 4. E 5. H	<b>02.4.1.2 Testing.</b> Testing shall be conducted in accordance with ANSI/R sting shall be conducted by either individuals as defined in Section 553.9 9.105(3)(f), (g), or (i) or an approved third party. A written report of the revided to theode official. Testing shall be performed at any time after creating testing:  Exterior windows and doors, fireplace and stove doors shall be closed, but not measures.  Dampers including exhaust, intake, makeup air, back draft and flue dampasures.  Interior doors, if installed at the time of the test, shall be open.  Exterior doors for continuous ventilation systems and heat recovery ventileating and cooling systems, if installed at the time of the test, shall be to Supply and return registers, if installed at the time of the test, shall be full	esuits of the test shall be signed by the party conducting the test and ation of all penetrations of the intended weatherstripping or other infiltration pers shall be closed, but not sealed beyond intended infiltration control tilators shall be closed and sealed.										
Te	esting Company											
Ιh	ompany Name:	nce with the 2017 6th Edition Florida Building Code										
Si	ignature of Tester:	Date of Test:										
Pr	rinted Name of Tester:											
Lic	cense/Certification #:	Issuing Authority:										

# **Residential System Sizing Calculation**

Summary
Project Title:
Lot 191 Justey Creek

Lake City, FL 32055

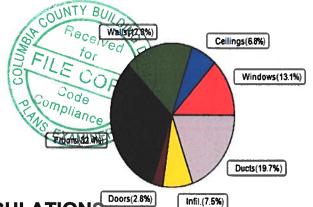
1/13/2020

Location for weather data: Gainesville, FL - Defaults: Latitude(29.7) Altitude(152 ft.) Temp Range(M)											
Humidity data: Interior RH (50%	6) Outdooi	r wet bulb (	77F) Humidity difference(51gr.)								
Winter design temperature(TMY3	99%) 30	F	Summer design temperature(TMY3 99%) 94 F								
Winter setpoint	70	F	Summer setpoint	75	F						
Winter temperature difference	40	F	Summer temperature difference	19	F						
Total heating load calculation	26404	Btuh	Total cooling load calculation	20763	Btuh						
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh						
Total (Electric Heat Pump)	100.0	26404	Sensible (SHR = 0.70)	84.6	14534						
Heat Pump + Auxiliary(0.0kW)	100.0	26404	Latent	174.1	6229						
			Total (Electric Heat Pump)	100.0	20763						

# **WINTER CALCULATIONS**

Winter Heating Load (for 1678 sqft)

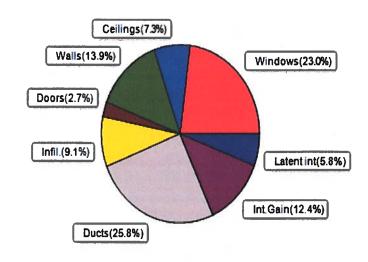
Load component			Load	
Window total	240	sqft	3456	Btuh
Wall total	1322	sqft	4694	Btuh
Door total	40	sqft	736	<sup>*</sup> Btuh
Ceiling total	1762	sqft	1789	Btuh
Floor total	1678	sqft	8543	Btuh
Infiltration	45	cfm	1989	Btuh
Duct loss			5198	Btuh
Subtotal			26404	Btuh
Ventilation	0	cfm	0	Btuh
TOTAL HEAT LOSS			26404	Btuh



# SUMMER CALCULATIONS

Summer Cooling Load (for 1678 sqft)

Load component			Load	
Window total	240	sqft	4785	Btuh
Wall total	1322	sqft	2880	Btuh
Door total	40	sqft	552	Btuh
Ceiling total	1762	sqft	1521	Btuh
Floor total			0	Btuh
Infiltration	34	cfm	708	Btuh
Internal gain			2580	Btuh
Duct gain			4158	Btuh
Sens. Ventilation	0	cfm	0	Btuh
Blower Load			0	Btuh
Total sensible gain			17184	Btuh
Latent gain(ducts)			1203	Btuh
Latent gain(infiltration)			1176	Btuh
Latent gain(ventilation)			0	Btuh
Latent gain(internal/occu	pants/othe	er)	1200	Btuh
Total latent gain			3579	Btuh
TOTAL HEAT GAIN			20763	Btuh



8th Edition

EnergyGauge® System Sizing PREPARED BY; DATE:

# **System Sizing Calculations - Winter**

# Residential Load - Whole House Component Details Project Title:

of 10 '

Lake City, FL 32055

Ot 19 Building Type: User

1/13/2020

Reference City: Gainesville, FL (Defaults) Winter Temperature Difference: 40.0 F (TMY3 99%)

### **Component Loads for Whole House**

Window	Panes/Type	Frame	. U	Orientation A	Area(sqft) X	HTM=	Load
1	2, NFRC 0.25	Vinyl	0.36	S	30.0	14.4	432 Btuh
2	2, NFRC 0.25	Vinyl	0.36	E	30.0	14.4	432 Btuh
3	2, NFRC 0.25	Vinyl	0.36	N	45.0	14.4	648 Btuh
4	2, NFRC 0.25	Metal	0.36	N	60.0	14.4	864 Btuh
5	2, NFRC 0.25	Vinyl	0.36	W	75.0	14.4	1080 Btuh
	Window Total	•			240.0(sqft)		3456 Btuh
Walls	Туре	Ornt. U	leff.	R-Value	Area X	HTM=	Load
				(Cav/Sh)			
1	Frame - Wood	- Ext (0	0.089)	13.0/0.0	103	3.55	366 Btuh
2	Frame - Wood	- Ext (0	0.089)	13.0/0.0	81	3.55	288 Btuh
3	Frame - Wood	- Ext (0	0.089)	13.0/0.0	4	3.55	14 Btuh
4	Frame - Wood		0.089)	13.0/0.0	195	3.55	692 Btuh
5	Frame - Wood	- Ext (0	0.089)	13.0/0.0	375	3.55	1331 Btuh
6	Frame - Wood	- Ext (0	0.089)	13.0/0.0	174	3.55	618 Btuh
7	Frame - Wood	- Ext (0	0.089)	13.0/0.0	72	3.55	256 Btuh
8	Frame - Wood	- Ext (0	,	13.0/0.0	93	3.55	330 Btuh
9	Frame - Wood	- Ext (0	0.089)	13.0/0.0	225	3.55	799 Btuh
	Wall Total				1322(sqft)		4694 Btuh
Doors	Туре	Storm	Ueff.		Area X	HTM=	Load
1	Insulated - Exter	rior, n ((	0.460)		20	18.4	368 Btuh
2	Insulated - Exter	rior, n ((	0.460)		20	18.4	368 Btuh
	Door Total				40(sqft)		736Btuh
Ceilings	Type/Color/Surfa		leff.	R-Value	Area X	HTM=	Load
1	Vented Attic/L/S	hing (0.	025)	38.0/0.0	1762	1.0	1789 Btuh
	Ceiling Total				1762(sqft)		1789Btuh
Floors	Type		Ueff.	R-Value	Size X	HTM=	Load
1	Slab On Grade		(1.180)	0.0	181.0 ft(peri	m.) 47.2	8543 Btuh
	Floor Total				1678 sqft		8543 Btuh
				E	Envelope Subto	tal:	19218 Btuh
Infiltration	Туре	Whole	house A	CH Volume(	cuft) Wall Rati	o CFM=	<u> </u>
3	Natural			18 15102	•		1989 Btuh
Duct load	Average sealed,	R6.0, Su	ıpply(Att)	), Return(Att)	(DLM	of 0.245)	5198 Btuh
All Zones				Sensible	Subtotal All Z	ones	26404 Btuh

# **Manual J Winter Calculations**

# Residential Load - Component Details (continued) Project Title:

Lake City, FL 32055

Lot 19 \ Building Type: User

1/13/2020

WHO			A I A
			$\Lambda$

## **EQUIPMENT**

1. Electric Heat Pump #	26404 Btuh
-------------------------	------------

Key: Window types - NFRC (Requires U-Factor and Shading coefficient(SHGC) of glass as numerical values) or - Glass as 'Clear' or 'Tint' (Uses U-Factor and SHGC defaults) U - (Window U-Factor) HTM - (ManualJ Heat Transfer Multiplier)



Version 8

# **System Sizing Calculations - Summer**

# Residential Load - Whole House Component Details Project Title:

Lot 19

Lake City, FL 32055

1/13/2020

Reference City: Gainesville, FL

Temperature Difference: 19.0F(TMY3 99%) Humidity difference: 51gr.

### **Component Loads for Whole House**

			Over	hang	Window Area(sqft)			НТМ		Load				
Window	Panes	SHGC L	InSh	IS	Ornt	Len	Hgt	Gross		Unshaded	Shaded	Unshaded		
1	2 NFRC	0.25, 0.3	6 No	No	S	10.5f	1.0ft.	30.0	30.0	0.0	12	14	363	Btuh
2	2 NFRC	0.25, 0.3	6 No	No	E	1.5ft.	1.0ft.	30.0	1.5	28.5	12	31	901	Btuh
3	2 NFRC	0.25, 0.3	6 No	No	N	1.5ft.	1.0ft.	45.0	0.0	45.0	12	12	544	Btuh
4	2 NFRC	0.25, 0.3	6 No	No	N	9.5ft.	1.0ft.	60.0	0.0	60.0	12	12	726	Btuh
5	2 NFRC	0.25, 0.3	6 No	No	W	1.5ft.	1.0ft.	75.0	3.7	71.3	12	31	2251	Btuh
	Windov	w Total						240 (s	sqft)				4785	<b>Btuh</b>
Walls	Туре				U	-Value	e R-\	/alue	Área	(sqft)	-	НТМ	Load	
	-						Cav/S	Sheath						
1	Frame -	Wood - Ex	t			0.09	13.0	0.0	10	3.0		2.3	233	Btuh
2	Frame -	Wood - Ex	t			0.09	13.0	0/0.0	81	1.0		2.3	183	Btuh
3	Frame -	Wood - Ex	t		. 1	0.09	13.0	0/0.0	4	.0		2.3	9	Btuh
4	Frame -	Wood - Ac	lj		0.09 13.0/			0/0.0	/0.0 195.0			1.7	329	Btuh
5		Wood - Ex				0.09		0/0.0		5.0		2.3	849	Btuh
6		Wood - Ex	•			0.09		0/0.0		4.0		2.3	394	
7		Wood - Ex				0.09 13.0/0				2.0		2.3	163	
-	8 Frame - Wood - Ext				0.09		0/0.0				2.3		Btuh	
9		Wood - Ex	t			0.09	13.0	0/0.0		5.0		2.3		Btuh
Wall Total		otal							132	22 (sqft)			2880	Btuh
Doors	Туре								Area	(sqft)		HTM	Load	
1	Insulated	d - Exterio	•						20	0.0		13.8	276	Btuh
2	Insulated	d - Exterio	•						20	0.0		13.8	276	Btuh
	Door T	otal							4	0 (sqft)			552	Btuh
Ceilings	Type/C	Color/Su	rface		U	-Value	9	R-Value				НТМ	Load	
1	Vented A	Attic/Light/S	Shinale/I	RB		0.025		38.0/0.0		32.0		0.86	1521	Btuh
·	Ceiling	-	9							2 (sqft)		0.00	1521	
Floors	Type						R-\	/alue		ze		НТМ	Load	
1	Slab On	Grade							 78 (ft-perir				Btuh	
•	Floor T							0.0			iletei)	0.0		Btuh
1	FIOOF	otai							1070	.0 (sqft)	-		U	Blun
									Е	nvelope	Subtota	l:	9737	Btuh
nfiltration	Туре	-			Δνρι	age A	CH	Valur	me/cuft	) Wall R	atio	CFM=	Load	
	Natura	4			77861	age r	0.14	Volui	15102		allo	34.1		Btuh
Internal	Taluia					Occup				cupant		Appliance	Load	Diul
						Joou	6		\ 23		,	1200		Dhil
gain							0	/	\ 23	<del>U +</del>		1200	2580	Dlur
	Sensible Envelope Load:									13026	Btuh			
Duct load	Average	sealed,Su	pply(R6	6.0-At	tic), Re	eturn(R	6.0-Attic	c)		(DGI	M of 0.3	19)	4158	Btuh
									Sen	sible Lo	oad All	Zones	17184	Btuh

# **Manual J Summer Calculations**

Residential Load - Component Details (continued)

Project Title: Climate:FL\_GAINESVILLE\_REGIONAL\_A

Lake City, FL 32055

1/13/2020

## WHOLE HOUSE TOTALS

			-
	Sensible Envelope Load All Zones	13026	Btuh
	Sensible Duct Load	4158	Btuh
146	Total Sensible Zone Loads	17184	Btuh
	Sensible ventilation	0	Btuh
	Blower	0	Btuh
Whole House	Total sensible gain	17184	Btuh
Totals for Cooling	Latent infiltration gain (for 51 gr. humidity difference)	1176	Btuh
	Latent ventilation gain	0	Btuh
	Latent duct gain	1203	Btuh
	Latent occupant gain (6.0 people @ 200 Btuh per person)	1200	Btuh
	Latent other gain	0	Btuh
8	Latent total gain	3579	Btuh
	TOTAL GAIN	20763	Btuh

EQUIPMENT		
1. Central Unit	#	20763 Btuh

\*Key: Window types (Panes - Number and type of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value)

(U - Window U-Factor)

(InSh - Interior shading device: none(No), Blinds(B), Draperies(D) or Roller Shades(R))

- For Blinds: Assume medium color, half closed For Draperies: Assume medium weave, half closed For Roller shades: Assume translucent, half closed

(IS - Insect screen: none(N), Full(F) or Half(1/2))

(Ornt - compass orientation)



Version 8

### AUGUST 1, 2016

### LATERAL BRACING RECOMMENDATIONS

#### MII-STRGBCK

MiTek USA, Inc.

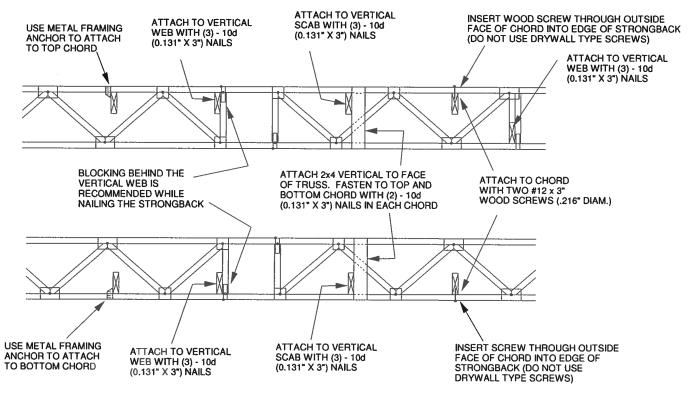
Page 1 of 1

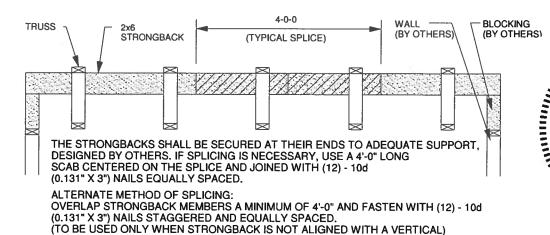


TO MINIMIZE VIBRATION COMMON TO ALL SHALLOW FRAMING SYSTEMS. 2x6 "STRONGBACK" IS RECOMMENDED, LOCATED EVERY 8 TO 10 FEET ALONG A FLOOR TRUSS.

NOTE 1: 2X6 STRONGBACK ORIENTED VERTICALLY MAY BE POSITIONED DIRECTLY UNDER THE TOP CHORD OR DIRECTLY ABOVE THE BOTTOM CHORD. SECURELY FASTENED TO THE TRUSS USING ANY OF THE METHODS ILLUSTRATED BELOW.

NOTE 2: STRONGBACK BRACING ALSO SATISFIES THE LATERAL BRACING REQUIREMENTS FOR THE BOTTOM CHORD OF THE TRUSS WHEN IT IS PLACED ON TOP OF THE BOTTOM CHORD, IS CONTINUOUS FROM END TO END, CONNECTED WITH A METHOD OTHER THAN METAL FRAMING ANCHOR, AND PROPERLY CONNECTED, BY OTHERS, AT THE ENDS.





No 39380

STATE OF ST SONAL ENGIN Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

HOMAS A. ALONA

February 12, 2018

Date:



Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

RE: 2228853 - LIPSCOMB EAGLE - LOT 19 . \_ . Juskey Greek

MiTek USA, Inc.

6904 Parke East Blvd. Tampa, FL 33610-4115

Site Information:

Customer Info: Lipscomb Eagle Project Name: Spec Hse Model: Custom

Subdivision: Tus 15 cm Creek

Lot/Block: 19 Address: N/A, N/A

City: Columbia Cty

State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name:

License #:

Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special

Loading Conditions):

Design Code: FBC2017/TPI2014

Wind Code: ASCE 7-10

Design Program: MiTek 20/20 8.2

Wind Speed: 130 mph

Truss Name

Roof Load: 37.0 psf

Floor Load: N/A psf

T12G T13

This package includes 27 individual, Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

Seal#

N 12345678911123456789	Seal# T19194297 T19194299 T19194300 T19194301 T19194302 T19194305 T19194307 T19194307 T19194307 T19194307 T19194311 T19194311 T19194311 T19194313 T19194314 T19194315	Truss Name CJ01 CJ03 CJ05 EJ01 EJ02 EJ03 HJ05 HJ10 PB01 PB02 T01 T01G T02 T03 T04 T05 T06 T07 T08	Date 1/21/20 1/21/20 1/21/20 1/21/20 1/21/20 1/21/20 1/21/20 1/21/20 1/21/20 1/21/20 1/21/20 1/21/20 1/21/20 1/21/20 1/21/20 1/21/20 1/21/20	



Date

1/21/20

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Jacksonville.

Truss Design Engineer's Name: Albani, Thomas

My license renewal date for the state of Florida is February 28, 2021.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



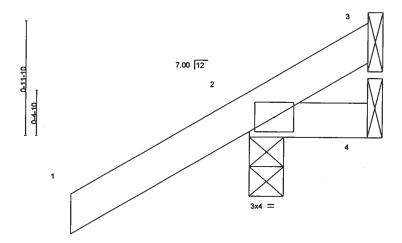
Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6834 6904 Parke East Blvd. Tampa FL 33610 Data:

January 21,2020

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE - LOT 19 WB N
2228853	CJ01	Jack-Open	8	1	T1919429
D 34 5 40					Job Reference (optional)
Builders FirstSource,	Jacksonville, FL - 32244,		(D-4b-V	3.240 s De	c 6 2019 MiTek Industries, Inc. Tue Jan 21 14:12:18 2020 Page 1

1D:1bYwwjYqtpHfiMFFcMVywFXb-2imXwW44FZRm5HY71iITMLQYi72woSGUmUfiwxzt6LR

Scale = 1:9



		<u></u>	1-0-0
CADING (psf)   TCLL   20.0   TCDL   7.0   BCLL   0.0   BCDL   10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2017/TPI2014	CSI. TC 0.17 BC 0.04 WB 0.00 Matrix-MP	DEFL. in (loc)   //defl   L/d   PLATES GRIP

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 BRACING-

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 1-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(lb/size) 3=-6/Mechanical, 2=179/0-3-8, 4=-19/Mechanical

Max Horz 2=64(LC 12)

Max Uplift 3=-6(LC 1), 2=-108(LC 12), 4=-25(LC 19) Max Grav 3=10(LC 16), 2=179(LC 1), 4=28(LC 16)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (t=lb) 2=108.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

January 21,2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI-THI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd Tampa, FL 36610

LIPSCOMB EAGLE - LOT 19 WB N Qty Job Truss Truss Type T19194298 2228853 CJ03 Jack-Open ob Reference (optional) 8.240 s Dec 6 2019 MiTek Industries, Inc. Tue Jan 21 14:12:20 2020 Page 1 ID:1bYwwjYqtpHfiMFFctmROVywFXb\_4tHLC5LmBhUKaiW87LxRmWuBxiBGMmnEo8p?qzt6LP Jacksonville, FL - 32244, **Builders FirstSource** 1-6-0 Scale = 1:15.3 7.00 12 0410 3-0-0 3-0-0 LOADING (psf) **PLATES** GRIP SPACING-CSI DEFL. 1/defl L/d 244/190 TCLL 20.0 Plate Grip DOL 1.25 TC 0.17 Vert(LL) 0.01 >999 240 MT20 TCDL 7.0 Lumber DOL 1.25 вс 0.11 Vert(CT) -0.01 >999 180 BCLL 0.0 Rep Stress Incr YES WB 0.00 Horz(CT) 0.00 3 n/a n/a

LUMBER-

BCDL

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

10.0

**BRACING-**

TOP CHORD **BOT CHORD**  Structural wood sheathing directly applied or 3-0-0 oc purlins.

Weight: 12 lb

FT = 20%

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(lb/size) 3=60/Mechanical, 2=210/0-3-8, 4=28/Mechanical

Max Horz 2=120(LC 12)

Max Uplift 3=-60(LC 12), 2=-91(LC 12), 4=-26(LC 9) Max Grav 3=65(LC 19), 2=210(LC 1), 4=50(LC 3)

Code FBC2017/TPI2014

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Wind; ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Matrix-MP

- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2, 4.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

January 21,2020

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7.00 12

5-0-0 5-0-0

Plate Of	fsets (X,Y)-	[2:0-1-12,0-1-8]										
LOADIN TCLL TCDL BCLL	IG (psf) 20.0 7.0 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.25 1.25 YES	CSI. TC BC WB	0.34 0.35 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.08 0.07 -0.00	(loc) 4-7 4-7	l/defl >721 >834 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 244/190
BCDL	10.0	Code FBC2017/TF			x-MP		-,,,,,	·			Weight: 19 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=113/Mechanical, 2=276/0-3-8, 4=58/Mechanical

Max Horz 2=177(LC 12)

Max Uplift 3=-111(LC 12), 2=-102(LC 12), 4=-46(LC 9) Max Grav 3=123(LC 19), 2=276(LC 1), 4=89(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 3=111, 2=102.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

January 21,2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-1473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual bullding component, not a truss system. Before use, the bullding designer must verify the applicability of design parameters and properly incorporate this design into the overall bullding design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/ITF11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qty LIPSCOMB EAGLE - LOT 19 WB N Ply T19194300 2228853 EJ01 lack-Partial 16 1 Job Reference (optional) Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2019 MiTek Industries, Inc. Tue Jan 21 14:12:23 2020 Page 1 ID:1bYwwjYqtpHfiMFFctmROVywFXb-P/ZQzE8D3643B2R4pFue3P7Fu8a4TjWDwmMTb8zt6LM 7-0-0 7-0-0 Scale = 1:26.6 7.00 12 949 3x4 / 7-0-0 Plate Offsets (X,Y)- [2:0-1-8,0-1-8] LOADING (psf) SPACING-2-0-0 CSI. DEFL. in (loc) l/defi **PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.25 TC 0.78 Vert(LL) 0.33 4-7 >253 240 MT20 244/190 TCDL 7.0 Lumber DOL 1.25 вс 0.74 Vert(CT) 0.28 >295 180 **BCLL** 0.0 Rep Stress Incr YES WB 0.00 Horz(CT) -0.013 n/a n/a **BCDL** 10.0 Code FBC2017/TPI2014 Matrix-MS Weight: 25 lb FT = 20% LUMBER-**BRACING-**TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 3-10-4 oc purlins.

BOT CHORD 2x4 SP No.2

**BOT CHORD** 

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(lb/size) 3=163/Mechanical, 2=346/0-3-8, 4=84/Mechanical

Max Horz 2=161(LC 12)

Max Uplift 3=-104(LC 12), 2=-98(LC 9), 4=-64(LC 9) Max Grav 3=171(LC 19), 2=346(LC 1), 4=126(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4 except (jt=lb)



MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

January 21,2020

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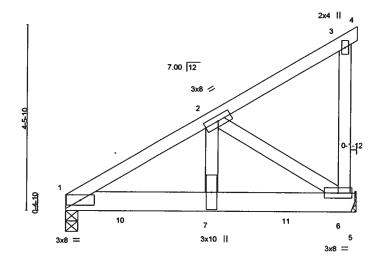
6904 Parke East Blvd. Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE - LOT 19 WB N
2228853	EJ02	Jack-Open Girder	1	1	T19194301
Builders FirstSource.	Jacksonville, FL - 32244.		Т		Job Reference (optional)
builders riistootice,	Jacksonville, FL - 32244,			3.240 s Dec	6 2019 MiTek Industries, Inc. Tue Jan 21 14:12:24 2020 Page 1

8.240 s Dec 6 2019 MiTek Industries, Inc. Tue Jan 21 14:12:24 2020 Page 1 ID:1bYwwjYqtpHfiMFFctmROVywFXb-tr7oBa8rqPCwpC0HNzPtccgaKYvVC2GN8Q618bzt6LL

3-6-0 7-0-0 3-6-0 3-6-0

Scale = 1:26.7



Matrix-MS

**PLATES** Ľ∕d TC BC 0.16 Vert(LL) -0.02 6-7 >999 240 MT20 0.79 Vert(CT) -0.04 6-7 >999 180 WB 0.54 Horz(CT) 0.01 6 n/a n/a

BRACING-

TOP CHORD

**BOT CHORD** 

GRIP

244/190

Weight: 42 lb FT = 20%

Structural wood sheathing directly applied or 4-7-2 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER-

REACTIONS.

TCDL

**BCLL** 

BCDL

TOP CHORD 2x4 SP No.2 BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3

7.0

10,0

0.0 \*

(lb/size) 1=1041/0-3-8, 6=1362/Mechanical

Lumber DOL

Rep Stress Incr

Code FBC2017/TPI2014

Max Horz 1=137(LC 8)

Max Uplift 1=-252(LC 8), 6=-367(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-1570/322

BOT CHORD 1-7=-376/1341, 6-7=-376/1341 WEBS 2-7=-316/1428, 2-6=-1587/445

#### NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

1.25

NO

- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=252. 6=367
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 339 lb down and 157 lb up at 1-4-12, and 779 lb down and 185 lb up at 3-4-12, and 779 lb down and 185 lb up at 5-4-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

#### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-4=-14, 1-5=-20

Concentrated Loads (lb)

Vert: 7=-779(F) 10=-339(F) 11=-779(F)



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

January 21,2020

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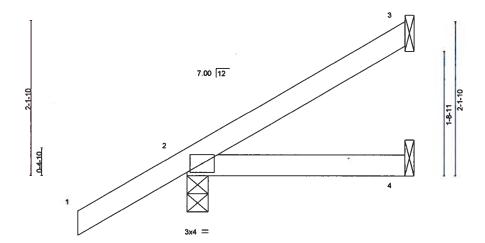
ANSUTPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd. Tampa, FL 36610 Job Truss Qty LIPSCOMB EAGLE - LOT 19 WB N Truss Type Ply T19194302 2228853 EJ03 Jack-Open 2 Job Reference (optional) Builders FirstSource Jacksonville, FL - 32244 8.240 s Dec 6 2019 MiTek Industries, Inc. Tue Jan 21 14:12:25 2020 Page 1 ID\_1bYwwjYqtpHfiMFFctmROVywFXb-L2hAOw9TbjKnRLbTxgw68qDlxyQLxd?WN4rag1zt6LK

| ID:1bYwy/YqtpHfiMFFctmROVywFXb-L2l -1-6-0 3-0-0 1-6-0 3-0-0

Scale = 1:15.3



LOADING (psf) SPACING-2-0-0 DEFL l/defl Ľ₫ **PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.25 TC 0.17 Vert(LL) 0.01 >999 240 MT20 244/190 TCDL. 7.0 BC Lumber DOL 1.25 0.11 Vert(CT) -0.01 >999 180 0.0 BCLL Rep Stress Incr YES WB 0.00 Horz(CT) 0.00 3 n/a n/a **BCDL** 10.0 Code FBC2017/TPI2014 Matrix-MP Weight: 12 lb FT = 20%

BRACING-

TOP CHORD

**BOT CHORD** 

3-0-0

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

P No.2

(lb/size) 3=60/Mechanical, 2=210/0-3-8, 4=28/Mechanical Max Horz 2=120(LC 12)

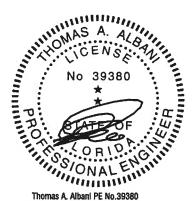
Max Uplift 3=-60(LC 12), 2=-91(LC 12), 4=-26(LC 9)

Max Grav 3=65(LC 19), 2=210(LC 1), 4=50(LC 3)

FORCES. (ib) - Max. Comp./Max. Ten. - All forces 250 (ib) or less except when shown,

#### NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2, 4.



Structural wood sheathing directly applied or 3-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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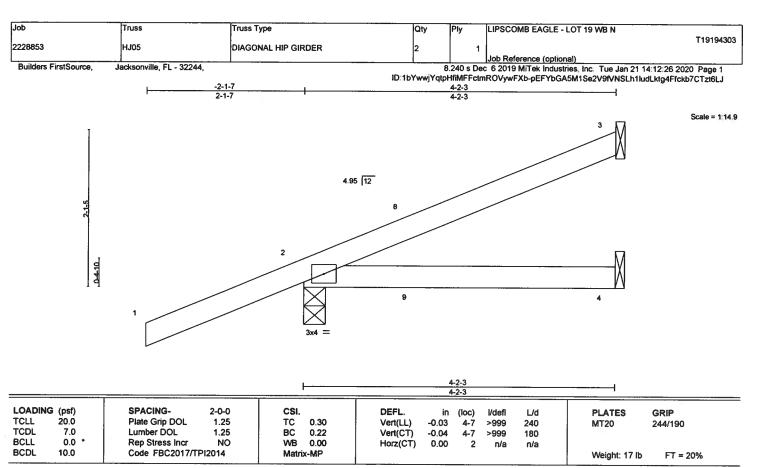
January 21,2020

WARNING - Verity design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-1473 rev. 10/03/2015 BEFORE USE.

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ANSI/THY Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 4-2-3 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(lb/size) 3=84/Mechanical, 2=296/0-3-8, 4=39/Mechanical

Max Horz 2=119(LC 8)

Max Uplift 3=-83(LC 8), 2=-227(LC 4), 4=-43(LC 5) Max Grav 3=84(LC 1), 2=296(LC 1), 4=71(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

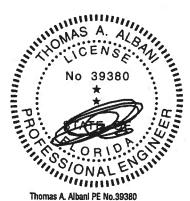
#### NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (jt=lb)
   2=227.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 83 lb down and 76 lb up at 1-6-1, and 83 lb down and 76 lb up at 1-6-1 on top chord, and 60 lb down and 52 lb up at 1-6-1, and 60 lb down and 52 lb up at 1-6-1 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (pff)

Vert: 1-3=-54, 4-5=-20



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

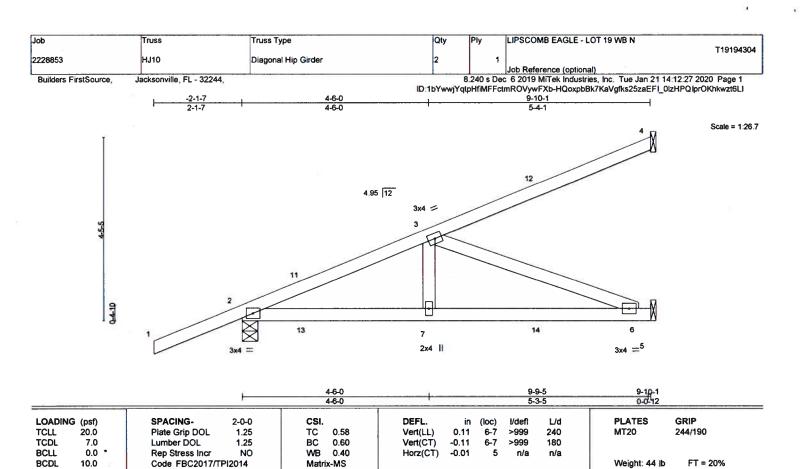
January 21,2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and perpet damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP1 Quality Criteria, DSB-99 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





**BRACING-**

TOP CHORD

**BOT CHORD** 

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

REACTIONS. (lb/si

(lb/size) 4=149/Mechanical, 2=527/0-4-9, 5=299/Mechanical

Max Horz 2=233(LC 8)

Max Uplift 4=-149(LC 8), 2=-403(LC 4), 5=-275(LC 5)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown,

TOP CHORD 2-3=-739/534

BOT CHORD 2-7=-611/629, 6-7=-611/629

WEBS 3-7=-144/283, 3-6=-674/655

#### NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=149, 2=403, 5=275.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 83 lb down and 76 lb up at 1-6-1, 83 lb down and 76 lb up at 1-6-1, 102 lb down and 50 lb up at 4-4-0, 102 lb down and 50 lb up at 4-4-0, and 134 lb down and 111 lb up at 7-1-15, and 134 lb down and 111 lb up at 7-1-15 on top chord, and 60 lb down and 52 lb up at 1-6-1, 60 lb down and 52 lb up at 1-6-1, 20 lb down and 34 lb up at 4-4-0, 20 lb down and 34 lb up at 4-4-0, and 42 lb down and 61 lb up at 7-1-15, and 42 lb down and 61 lb up at 7-1-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

#### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced); Lumber Increase=1,25, Plate Increase=1,25

Uniform Loads (plf)

Vert: 1-4=-54, 5-8=-20

Concentrated Loads (lb)

Vert: 7=-5(F=-3, B=-3) 12=-73(F=-36, B=-36) 14=-59(F=-29, B=-29)



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 6-11-11 oc bracing.

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Ozte:

January 21,2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

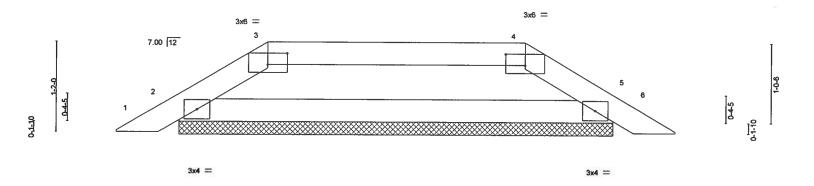
Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd. Tampa, FL 36610

Job	Truss	Truss Type	ļc	Qty	Ply	LIPSCOMB EAGLE - LOT 19 WB N
						T19194305
2228853	PB01	Piggyback	2	!	1	
						Job Reference (optional)
Builders FirstSource,	Jacksonville, FL - 32244,			8	.240 s Dec	c 6 2019 MiTek Industries, Inc. Tue Jan 21 14:12:28 2020 Page 1
			ID:1bYw	wjYqtpH	fiMFFctmR	ROVywFXb-ldMJ0xBMueiMlpJ2coUpmSrGQ9OY8zly324EHMzt6LH
L	2-0-0		5-4-0			7-4-0
•	2-0-0	1	3-4-0			2-0-0

Scale = 1:14.4



						7-4-0						
						7-4-0						4
late Offset	s (X,Y)— [3	3:0-3-0,0-1-12], [4:0-3-0,	0-1-12]								11	
OADING (	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
	20.0	Plate Grip DOL	1.25	TC	0.15	Vert(LL)	0.00	` <u>5</u>	n/r	120	MT20	244/190
DL	7.0	Lumber DOL	1.25	BC	0.27	Vert(CT)	0.00	6	n/r	120		
CLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	5	n/a	n/a		
CDL 1	10.0	Code FBC2017/TI	PI2014	Matri	x-R						Weight: 20 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 BRACING-

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 2=238/5-7-11, 5=238/5-7-11

Max Horz 2=-25(LC 10)

Max Uplift 2=-44(LC 9), 5=-44(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-294/266, 3-4=-251/244, 4-5=-294/267

BOT CHORD 2-5=-192/251

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 5.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

January 21,2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Qty Ply LIPSCOMB EAGLE - LOT 19 WB N Truss Type T19194306 2228853 PB02 Piggyback Job Reference (optional) Jacksonville, FL - 32244, Builders FirstSource 8.240 s Dec 6 2019 MiTek Industries, Inc. Tue Jan 21 14:12:29 2020 Page 1 ID:1bYwwjYqtpHfiMFFctmROVywFXb-DpwhEHC\_fyqCvzuEAW?2JgNRbZnptQe6lipopozt6LG Scale = 1:15.3 4×4 = 7.00 12 45 9 12 2x4 = 2x4 || 2x4 = LOADING (psf) SPACING-2-0-0 CSI. DEFL **PLATES** GRIP **TCLL** 20.0 Plate Grip DOL 1.25 TC 0.13 Vert(LL) 0.00 120 MT20 244/190 n/r TCDL 7.0 Lumber DOL 1.25 BC 0.08 Vert(CT) 0.00 5 n/r 120 0.0 \* **BCLL** Rep Stress Incr YES WB 0.02 Horz(CT) 0.00 BCDL 10.0 Code FBC2017/TPi2014 Matrix-P Weight: 23 lb FT = 20% BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 2x4 SP No 2 **BOT CHORD** 2x4 SP No.3 **OTHERS** 

(lb/size) 2=140/5-7-11, 4=140/5-7-11, 6=196/5-7-11

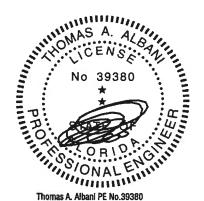
Max Horz 2=-49(LC 10)

Max Uplift 2=-45(LC 12), 4=-50(LC 13), 6=-17(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4, 6.
- 8) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

January 21,2020

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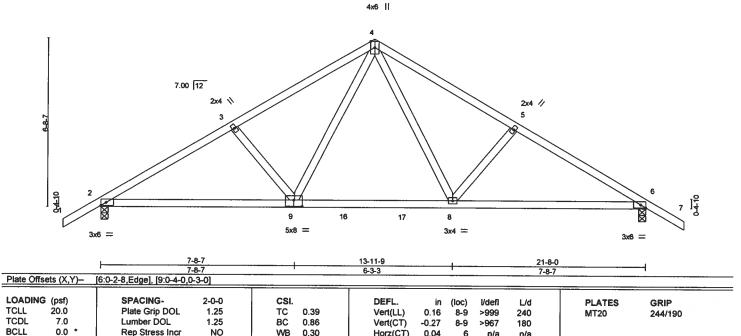
ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Composerity Information

Available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qty LIPSCOMB EAGLE - LOT 19 WB N T19194307 2228853 T01 10 Common Job Reference (optional) Builders FirstSource Jacksonville FL - 32244 8.240 s Dec 6 2019 MiTek Industries, Inc. Tue Jan 21 14:12:30 2020 Page 1 ID:1bYwwjYqtpHfiMFFctmROVywFXb-h?U3RdDcQFy3X7TRkDWHrtwXIzxocpWFXMZLLEzt6LF 10-10-0 21-8-0

Scale = 1:44.1



LUMBER-

**BCDL** 

WEBS

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

10.0

2x4 SP No.3

BRACING-

Horz(CT)

0.04

TOP CHORD **BOT CHORD** 

Structural wood sheathing directly applied or 4-5-0 oc purlins. Rigid ceiling directly applied or 7-7-14 oc bracing.

Weight: 107 lb

FT = 20%

6

n/a

n/a

REACTIONS.

(lb/size) 2=1072/0-3-8, 6=1071/0-3-8

Max Horz 2=-224(LC 10)

Max Uplift 2=-433(LC 12), 6=-433(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

Code FBC2017/TPI2014

TOP CHORD 2-3=-1640/716, 3-4=-1507/703, 4-5=-1503/700, 5-6=-1638/716

BOT CHORD 2-9=-580/1498, 8-9=-267/988, 6-8=-504/1365

WEBS

4-8=-309/713, 5-8=-339/282, 4-9=-311/715, 3-9=-338/281

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Matrix-MS

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

NO

- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=433, 6=433
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

#### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-7=-54, 9-10=-20, 8-9=-80(F=-60), 8-13=-20



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

January 21,2020

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ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE - LOT 19 WB N			
				'		T19194308		
2228853	T01G	Common Supported Gable	1	1		- 1		
					Job Reference (optional)	i		
Builders FirstSource,	Jacksonville, FL - 32244,	100.0000 1000 0		.240 s Dec	6 2019 MiTek Industries, Inc. Tue Jan 21 14:12:31 2020	Page 1		
Builders FirstSource, Jacksonville, FL - 32244,			ID:1bYwwjYqtpHfiMFFctmROVywFXb-AC2RfzEEBZ4w9G2dHx1W05TmpMSNLJHPi0luthzt6LE					
	ı	10-10-0			21-8-0 23-2-	0_1		
¹ 1-6-0		10-10-0			10-10-0 1-6-0			

Scale = 1:43.5

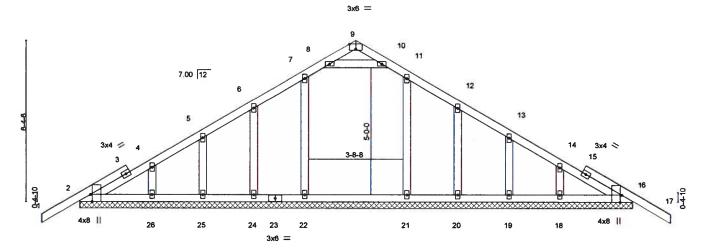


Plate Offsets (X,Y)-[2:0-3-8,Edge], [9:0-3-0,Edge], [16:0-3-8,Edge] LOADING (psf) SPACING-2-0-0 DEFL. **PLATES** I/defl L/d GRIP (loc) TCLL 20.0 Plate Grip DOL 1.25 TC 0.14 -0.01 17 120 244/190 Vert(LL) n/r MT20 TCDL 7.0 Lumber DOL 1.25 BC 0.14 Vert(CT) -0.01 17 120 n/r 0.0 BCLL Rep Stress Incr YES WB 0.08 Horz(CT) 0.01 16 n/a n/a **BCDL** 10.0 Code FBC2017/TPI2014 Matrix-S Weight: 118 lb FT = 20%

**BRACING-**

TOP CHORD

**BOT CHORD** 

LUMBER-TOP CHORD

2x4 SP No.2 BOT CHORD 2x4 SP No.2 WERS

2x4 SP No.3 OTHERS 2x4 SP No.3

REACTIONS. All bearings 21-8-0

(lb) - Max Horz 2=214(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 2, 16, 22, 26, 21, 18 except 24=-111(LC 12), 25=-102(LC 12),

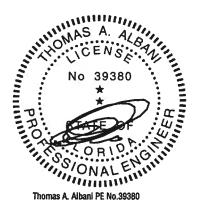
20=-117(LC 13), 19=-101(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 2, 16, 24, 25, 26, 20, 19, 18 except 22=317(LC 19), 21=285(LC

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 16, 22, 26, 21, 18 except (jt=lb) 24=111, 25=102, 20=117, 19=101.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

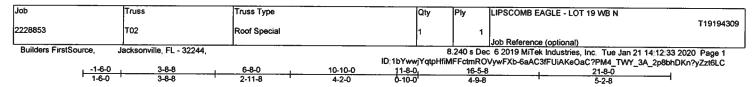
Rigid ceiling directly applied or 10-0-0 oc bracing.

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

January 21,2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MT ek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design, Bracing individual dis to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





Scale = 1:43.7

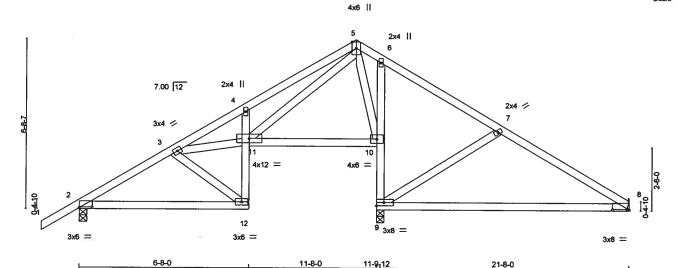


Plate Offsets (X,Y)	6-8-0 [2:0-6-0,0-0-3], [8:0-8-0,0-0-4]	5-0-0	0-1-12	9-10-4	
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2017/TPI2014	CSI. TC 0.67 BC 0.77 WB 0.41 Matrix-MS	DEFL. in (lo Vert(LL) -0.24 9- Vert(CT) -0.50 9- Horz(CT) 0.01	15 >485 240	PLATES GRIP MT20 244/190 Weight: 117 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2 \*Except\*

4-12,6-9: 2x4 SP No.3

WEBS 2x4 SP No.3

REACTIONS. (lb/size) 8=347/Mechanical, 2=508/0-3-8, 9=829/0-3-8

Max Horz 2=172(LC 11)

Max Uplift 8=-137(LC 13), 2=-139(LC 12), 9=-122(LC 12) Max Grav 8=377(LC 20), 2=512(LC 23), 9=829(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-644/352, 3-4=-1123/561, 4-5=-1274/689, 5-6=-233/258, 7-8=-411/302 BOT CHORD 2-12=-241/523, 11-12=-155/420, 9-10=-575/133, 8-9=-184/312

WEBS 3-12=-597/287, 3-11=-339/878, 5-11=-563/1192, 7-9=-383/257, 5-10=-455/141

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=137, 2=139, 9=122.



Structural wood sheathing directly applied or 4-11-6 oc purlins.

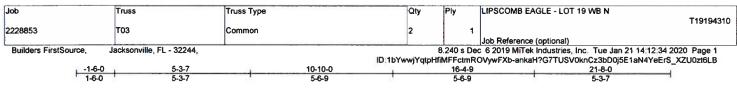
Rigid ceiling directly applied or 10-0-0 oc bracing, Except:

6-0-0 oc bracing: 9-10.

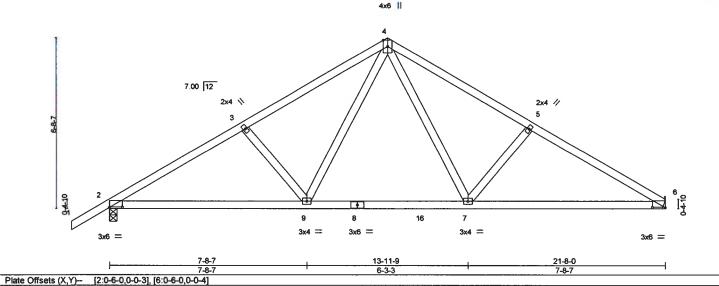
Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

January 21,2020





Scale = 1:43.2



----

TCLL

TCDL

BCLL

BCDI

LOADING (psf)

20.0

10.0

7.0

0.0 \*

BRACING-

DEFL.

Vert(LL)

Vert(CT)

Horz(CT)

-0.08 7-12

-0.17

0.03

7-12

6

TOP CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins.

**PLATES** 

Weight: 104 lb

MT20

GRIP

244/190

FT = 20%

Rigid ceiling directly applied or 9-1-1 oc bracing.

L/d

240

180

n/a

**V**defi

>999

>999

n/a

REACTIONS. (lb/size) 6=799/Mechanical, 2=885/0-3-8

Max Horz 2=172(LC 11)

Max Uplift 6=-165(LC 13), 2=-194(LC 12)

SPACING-

Plate Grip DOL

Rep Stress Incr

Code FBC2017/TPI2014

Lumber DOL

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1239/557, 3-4=-1125/541, 4-5=-1120/547, 5-6=-1249/564

BOT CHORD 2-9=-404/1118, 7-9=-156/713, 6-7=-413/1050

WEBS 4-7=-197/491, 5-7=-352/271, 4-9=-184/475, 3-9=-356/264

#### NOTES:

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

2-0-0

1.25

1.25

YES

CSI.

TC 0.34

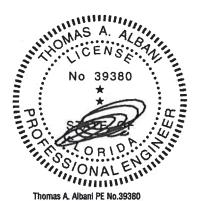
BC

WB 0.19

Matrix-MS

0.52

- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=165, 2=194.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Bivd. Tampa FL 33610 Date:

January 21,2020

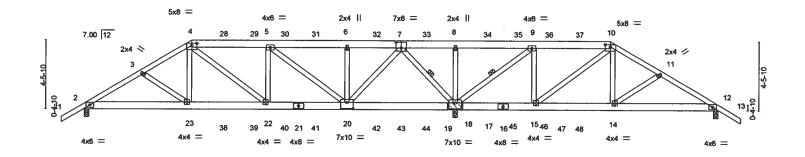
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-1473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses systems, see ANSUTH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314,



Job	Truss			Truss Type		Q	ty Ply	LIPSCOMB EAGLE - LOT	19 WB N		
2228853	T04			Hip Girder		1	1				T19194311
								Job Reference (optional)			
Builders FirstSource,	Jackson	rville, F	L - 32244,				8.240 s D	c 6 2019 MiTek Industries,	Inc. Tue Jan 21 1	4 12 36 20	020 Page 1
						ID:1bYv	wjYqtpHfiMFFc	mROVywFXb-W9rKigIN?5iD	F2wa4Udh58AW	N3h0NV8	vI0fZuzt6L9
<u>-1-6-0, 3-10-</u>			12-0-0	17-1-12	20-8-0	, 24-2-4	29-4-0	34-4-0	37-5-12	41-4-0	42-10-Q
1-6-0 3-10-	4 ' 3-1-	·12 '	5-0-0	5-1-12	3-6-4	3-6-4	5-1-12	5-0-0	3-1-12	3-10-4	1-6-0

Scale = 1:72.2



	3-10-4	7-0-0	12-0-0	17-1-12	24-2-4	29-4-0	34-4-0	37-5-12	41-4-0		
	3-10-4	' 3-1-12 '	5-0-0 '	5-1-12	7-0-8	5-1-12	5-0-0	3-1-12	3-10-4		
Plate Offsets (X,Y)— [4:0-4-0,0-1-11], [7:0-4-0,0-4-8], [10:0-4-0,0-1-11], [18:0-5-0,0-4-12]											
LOADING	(psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc) I/defl	L/d	PLATES	GRIP		
TCLL	20.0	Plate Grip DOL	1.25	TC 0.58	Vert(LL)	0.12 22-23 >999	240	MT20	244/190		
TCDL	7.0	Lumber DOL	1.25	BC 0.51	Vert(CT)	-0.14 22-23 >999	180				
BCLL	0.0 *	Rep Stress Inci	. NO	WB 0.91	Horz(CT)	0.05 12 n/a	n/a				
BCDL	10.0	Code FBC2017	7/TPI2014	Matrix-MS				Weight: 285 lb	FT = 20%		

LUMBER-

TOP CHORD 2x4 SP No.2 \*Except\*

4-7.7-10: 2x6 SP No.2 **BOT CHORD** 2x6 SP No.2

**WEBS** 2x4 SP No.3 BRACING-

TOP CHORD **BOT CHORD** WEBS

Structural wood sheathing directly applied or 3-8-3 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing. 1 Row at midpt 7-18, 9-18

REACTIONS. (lb/size) 2=1446/0-3-8, 18=4160/(0-3-8 + bearing block) (req. 0-4-15), 12=774/0-3-8

Max Horz 2=-124(LC 25)

Max Uplift 2=-751(LC 8), 18=-2573(LC 5), 12=-499(LC 4) Max Grav 2=1452(LC 19), 18=4160(LC 1), 12=787(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

2-3=-2428/1377, 3-4=-2279/1349, 4-5=-2109/1258, 5-6=-1185/697, 6-7=-1185/697, 7-8=-1096/1820, 8-9=-1096/1820, 10-11=-960/803, 11-12=-1109/823

BOT CHORD 2-23=-1213/2064, 22-23=-1156/1912, 20-22=-1249/2105, 14-15=-581/769,

12-14=-664/935 WEBS

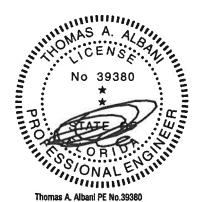
4-23=-424/684, 4-22=-194/310, 5-22=-93/307, 5-20=-1177/725, 6-20=-482/288,

7-20=-1216/2037, 7-18=-2474/1482, 8-18=-556/333, 9-18=-2279/1520, 9-15=-592/845,

10-15=-990/669, 10-14=-496/695

#### NOTES-

- 1) 2x6 SP No.2 bearing block 12" long at jt. 18 attached to front face with 3 rows of 10d (0.131"x3") nails spaced 3" o.c. 12 Total fasteners. Bearing is assumed to be SP No.2.
- 2) Unbalanced roof live loads have been considered for this design.
- 3) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope); porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (|t=lb) 2=751, 18=2573, 12=499.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

January 21,2020

#### Continued on page 2

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ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Compositely Information available from Truss Ptate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE - LOT 19 WB N
					T19194311
2228853	T04	Hip Girder	1	1	L
10000000			1	I	Job Reference (optional)

Builders FirstSource.

Jacksonville, FL - 32244.

8.240 s Dec 6 2019 MiTek Industries, Inc. Tue Jan 21 14:12:37 2020 Page 2 ID:1bYwwjYqtpHfiMFFctmROVywFXb-\_MPiv0I?mPq4tBVneB8wdLjhVnPwiqkH8yID5Lzt6L8

#### NOTES-

9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 128 lb down and 115 lb up at 7-0-0, 125 lb down and 111 lb up at 9-0-12, 125 lb down and 111 lb up at 11-0-12, 125 lb down and 111 lb up at 13-0-12, 125 lb down and 111 lb up at 15-0-12, 125 lb down and 111 lb up at 19-0-12, 125 lb down and 111 lb up at 19-0-12, 125 lb down and 111 lb up at 28-3-4, 125 lb down and 111 lb up at 28-3-4, 125 lb down and 111 lb up at 28-3-4, 125 lb down and 111 lb up at 28-3-4, 125 lb down and 111 lb up at 28-3-4, 125 lb down and 111 lb up at 28-3-4, 125 lb down and 111 lb up at 28-3-4, 125 lb down and 111 lb up at 28-3-4, 125 lb down and 111 lb up at 28-3-4, 125 lb down and 111 lb up at 28-3-4, 125 lb down and 111 lb up at 28-3-4, 125 lb down and 111 lb up at 28-3-4, 125 lb down and 280 lb up at 34-4-0 on top chord, and 336 lb down and 390 lb up at 7-0-0, 86 lb down and 84 lb up at 19-0-12, 86 lb down and 84 lb up at 11-0-12, 86 lb down and 84 lb up at 15-0-12, 86 lb down and 84 lb up at 15-0-12, 86 lb down and 84 lb up at 28-3-4, 86 lb down and 84 lb up at 28-3-4, and 86 lb down and 84 lb up at 32-3-4, and 336 lb down and 390 lb up at 34-3-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

#### LOAD CASE(S) Standard

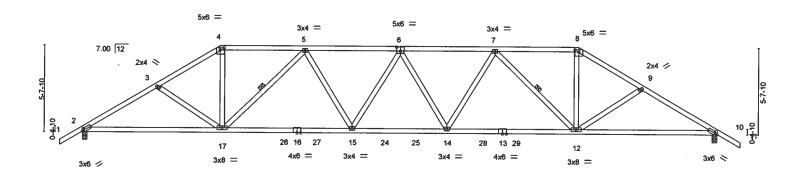
 Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-10=-54, 10-13=-54, 2-12=-20

Concentrated Loads (lb)

Vert: 4=-109(F) 7=-109(F) 10=-182(F) 23=-336(F) 20=-64(F) 6=-109(F) 8=-109(F) 14=-336(F) 28=-109(F) 29=-109(F) 30=-109(F) 31=-109(F) 32=-109(F) 33=-109(F) 35=-109(F) 35=-109(F)

Job		Truss	Truss Type		Qty	Ply	LIPSCOMB EAGL	E - LOT 19 WB N		
2228853		T05	Hip		2	1				T19194312
		<u> </u>					Job Reference (op			
Builders FirstSc	ource, .	Jacksonville, FL - 32244,					6 2019 MiTek Ind			
					ID:1bYwwjYqtpH	fiMFFctmR	OVywFXb-SYz57M	JdXjzxUL4zCvf9A	ZFvjBe9UQXRM	bVmdnzt6i_7
[1-6-0]	4-11-4	9-0-0	14-6-2	20-8-0	26-9-15		32-4-0	36-4-12	41-4-0	42-10-Q
'1-6-0 '	4-11-4	' 4-0-12 '	5-6-1	6-1-14	6-1-14		5-6-1	4-0-12	4-11-4	1-6-0



9-0-0 9-0-0 Plate Offsets (X,Y) [2:0-1-8,0-1-8], [4:0-3-0,0		17-7-0 8-7-0 9-1-12], [6:0-3-0,0-3-0], [8:0-3-0,0-1-		23-9-0 6-1-15 -12], [10:0-1-8,0-1-8]		2-4-0 3-7-0		41-4-0 9-0-0		
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017/T	2-0-0 1.25 1.25 YES	CSI. TC 0.42 BC 0.96 WB 0.36 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.28 12-14 -0.54 12-14 0.16 10	1/defl >999 >921 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 219 lb	GRIP 244/190 FT = 20%	
UMBER-				BRACING-						

TOP CHORD

**BOT CHORD** 

WEBS

REACTIONS.

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

2x4 SP No.3 **WEBS** 

(lb/size) 2=1610/0-3-8, 10=1610/0-3-8

Max Horz 2=153(LC 11)

Max Uplift 2=-291(LC 9), 10=-291(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD

2-3=-2645/1239, 3-4=-2419/1159, 4-5=-2060/1057, 5-6=-2877/1447, 6-7=-2877/1447,

7-8=-2060/1057, 8-9=-2419/1159, 9-10=-2645/1239

**BOT CHORD** 2-17=-942/2236, 15-17=-1119/2701, 14-15=-1244/2973, 12-14=-1121/2701,

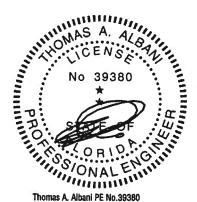
10-12=-955/2236 WEBS

3-17=-334/222, 4-17=-407/956, 5-17=-964/485, 5-15=-101/397, 7-14=-101/397,

7-12=-964/485, 8-12=-407/956, 9-12=-333/222

### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl.. GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except ([t=|b] 2=291, 10=291.



Structural wood sheathing directly applied or 3-2-2 oc purlins.

Rigid ceiling directly applied or 2-2-0 oc bracing.

1 Row at midpt

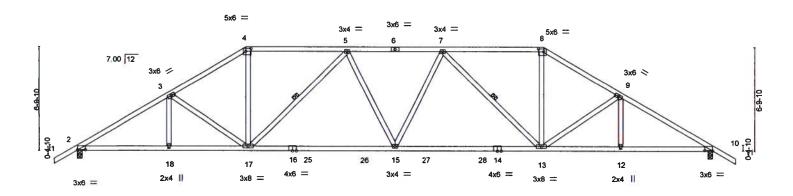
Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

January 21,2020

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Job		Truss		Truss Type				Qty	Ply	LIPSCOMB EAG	LE - LOT 19 V	WB N	
													T19194313
2228853		T06		Hip				2	1				
L										Job Reference (c	ptional)		
Builders FirstSou	rce, J	acksonville,	FL - 32244,						8.240 s De	c 6 2019 MiTek In	dustries, Inc.	Tue Jan 21 14:12:40	2020 Page 1
								ID:1bYwwj	YqtpHfiMFI	FctmROVywFXb-P	w5rX2Lu3KDf	MEMJKidF_LDI?MC	yK2jqv_tifzt6L5
<sub>c</sub> 1-6-0	5-11-7		11-0-0	17-	6-13		23-9-3	4	30-4-0	)	35-4-9	41-4-0	42-10-0
1-6-0	5-11-7		5-0-9	6-	-13	0.00	6-2-6		6-6-13	3	5-0-9	5-11-7	1-6-0



		11-7 11-0-0		20-8-0	1	30-4-0		35-4-9	41-4	-0
	5-	11-7 5-0-9	- 12	9-8-0	- 1	9-8-0		5-0-9	5-11-	-7
Plate Offse	ets (X,Y)-	[2:0-6-0,0-0-3], [4:0-4-0,0	-2-4], [8:0-4-0,	0-2-4], [10:0-6-0,0-0-3]						
LOADING	(psf)	SPACING-	2-0-0	CSI.	DEFL.	in (łoc)	l/defl	L∕d	PLATES	GRIP
CLL	20.0	Plate Grip DOL	1.25	TC 0.52	Vert(LL)	-0.26 15-17	>999	240	MT20	244/190
CDL	7.0	Lumber DOL	1.25	BC 0.80	Vert(CT)	-0.50 15-17	>983	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB 0.36	Horz(CT)	0.14 10	n/a	n/a		
BCDL	10.0	Code FBC2017/T	PI2014	Matrix-MS	, ,				Weight: 227 lb	FT = 20%

**BRACING-**TOP CHORD BOT CHORD

WEBS

LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD** 

2x4 SP No.2 \*Except\* 14-16: 2x4 SP M 31

2x4 SP No.3 WEBS

REACTIONS. (lb/size) 2=1610/0-3-8, 10=1610/0-3-8

Max Horz 2=-182(LC 10)

Max Uplift 2=-285(LC 12), 10=-285(LC 13)

FORCES. (ib) - Max. Comp./Max. Ten. - All forces 250 (ib) or less except when shown. TOP CHORD 2-3=-2646/1198, 3-4=-2288/1116, 4-5=-1929/1023, 5-7=-2434/1249, 7-8=-1929/1023,

8-9=-2288/1116, 9-10=-2646/1198

2-18=-895/2222, 17-18=-895/2222, 15-17=-933/2385, 13-15=-934/2385, 12-13=-908/2222, **BOT CHORD** 

10-12=-908/2222

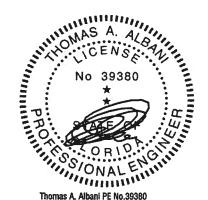
**WEBS** 3-17=-492/268, 4-17=-350/850, 5-17=-736/345, 7-13=-736/345, 8-13=-350/850,

9-13=-492/268

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=285, 10=285,



Structural wood sheathing directly applied or 3-3-11 oc purlins.

5-17, 7-13

Rigid ceiling directly applied or 6-1-8 oc bracing.

1 Row at midpt

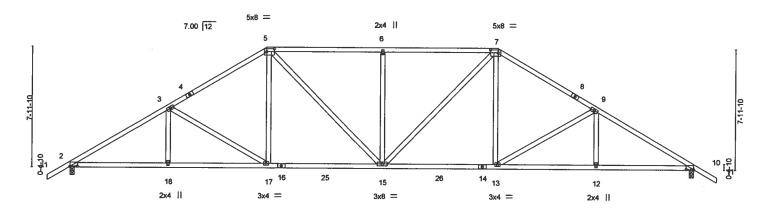
MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

January 21,2020

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Job	Truss	Truss Type		Qty	Ply	LIPSCOMB EAGLE - LOT 19 1	WB N	
2228853	то7	Hip		2	1			T19194314
						Job Reference (optional)		
Builders FirstSource,	Jacksonville, FL -	32244,				c 6 2019 MiTek Industries, Inc.		
				ID:1bYwwjY	qtpHfiMFFc	tmROVywFXb-t7fDIOLWqeLVL	.ppYt1DsoBtL1Okzhi7	7t3ZjQE6zt6L4
1-6-0	6-5-11	13-0-0	20-8-0	28-4	0	34-10-5	41-4-0	42-10-0
'1-6-0 '	6-5-11	6-6-5	7-8-0	7-8-	)	6-6-5	6-5-11	1-6-0



		6-5-11 6-5-11	13-0-0 6-6-5	20-8-0 7-8-0		28-4-0 7-8-0		34-10-5 6-6-5	41-4-( 6-5-1	
Plate Offse	ets (X,Y)	[2:0-6-0,0-0-3], [5:0-6-0,0	)-2-4], [7:0-6-0,0-	2-4], [10:0-6-0,0-0-3]						
LOADING TCLL TCDL BCLL BCDL	(psf) 20.0 7.0 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017/T	2-0-0 1.25 1.25 YES PI2014	CSI. TC 0.74 BC 0.70 WB 0.69 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.20 13-15 -0.38 13-15 0.13 10	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 231 lb	<b>GRIP</b> 244/190 FT = 20%

**BRACING-**

TOP CHORD

**BOT CHORD** 

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

2x4 SP No.3 WEBS

(lb/size) 2=1610/0-3-8, 10=1610/0-3-8

Max Horz 2=-211(LC 10)

Max Uplift 2=-301(LC 12), 10=-301(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-2645/1181, 3-5=-2168/1059, 5-6=-2105/1138, 6-7=-2105/1138, 7-9=-2168/1059,

9-10=-2645/1181

**BOT CHORD** 2-18=-879/2220, 17-18=-879/2220, 15-17=-600/1796, 13-15=-602/1796, 12-13=-890/2220,

10-12=-890/2220 **WEBS** 

3-18=0/258, 3-17=-622/336, 5-17=-125/505, 5-15=-227/560, 6-15=-475/355,

7-15=-227/560, 7-13=-125/505, 9-13=-622/336, 9-12=0/258

### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl. GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are 3x6 MT20 unless otherwise indicated.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (ft=lb) 2=301, 10=301.



Structural wood sheathing directly applied or 2-2-0 oc purlins.

Rigid ceiling directly applied or 6-2-10 oc bracing.

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

January 21,2020

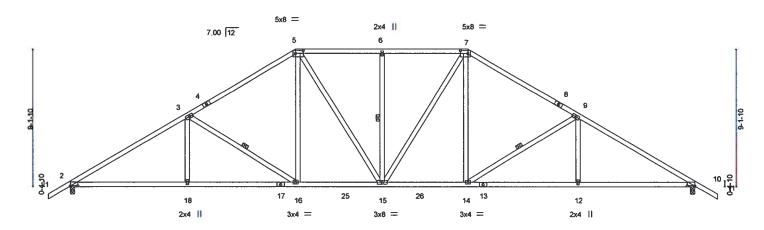
🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-1473 rev. 10/03/2016 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANS/TPH Quality Criteria, DSB-89 and BCSI Building Compo Safety Information

available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type		Qty	Ply	LIPSCOMB EAGLE - LOT	Γ 19 WB N	
	1			1				T19194315
2228853	T08	Hip		2	1			
						Job Reference (optional)		
Builders FirstSource,	Jacksonville, FL - 32244,				3.240 s Dec	6 2019 MiTek Industries	Inc. Tue Jan 21 14:12:4:	3 2020 Page 1
			ID:1b	YwwjYqtpHfiM	FFctmRO\	/ywFXb-pVm_A4NmMFbD	b6zw_SFKtcyhdCPk9gG/	AWtCXJ_zt6L2
<sub>c</sub> 1-6-0	7-9-1	15-0-0	20-8-0	26-4-0		33-6-15	41-4-0	42-10-0
1-6-0	7-9-1	7-2-15	5-8-0	5-8-0		7-2-15	7-9-1	1-6-0



	-	7-9-1	7-2-15	¹ 5-8-0	5-8-0	7-2-15	7-9-1
Plate Offset	ts (X,Y)-	[2:0-6-0,0-0-3], [5:0-6-	0,0-2-4], [7:0-6-0,0-	2-4], [10:0-6-0,0-0-3]			
LOADING	(psf)	SPACING-	2-0-0	CSI.	DEFL. in (lo	c) l/defl L/d	PLATES GRIP
	20.0	Plate Grip DOL	1.25	TC 0.73	Vert(LL) -0.15 15-1	16 >999 240	MT20 244/190
TCDL	7.0	Lumber DOL	1.25	BC 0.74	Vert(CT) -0,29 12-1	14 >999 180	
BCLL	0.0 *	Rep Stress Incr	YES	WB 0.40	Horz(CT) 0.13	10 n/a n/a	
BCDL	10.0	Code FBC2017	/TPI2014	Matrix-MS			Weight: 241 lb FT = 20%
			1				<u> </u>

20-8-0

LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 WEBS 2x4 SP No.3 **BRACING-**

TOP CHORD **BOT CHORD** WEBS

Structural wood sheathing directly applied or 2-2-0 oc purlins.

41-4-0

Rigid ceiling directly applied or 6-2-12 oc bracing. 1 Row at midpt 3-16, 6-15, 9-14

33-6-15

REACTIONS. (lb/size) 2=1610/0-3-8, 10=1610/0-3-8

Max Horz 2=-240(LC 10)

7-9-1

Max Uplift 2=-314(LC 12), 10=-314(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-2601/1147, 3-5=-2035/1008, 5-6=-1802/1013, 6-7=-1802/1013, 7-9=-2035/1008,

9-10=-2601/1147

**BOT CHORD** 2-18=-834/2170, 16-18=-834/2170, 15-16=-507/1666, 14-15=-508/1666, 12-14=-842/2170,

15-0-0

10-12=-842/2170

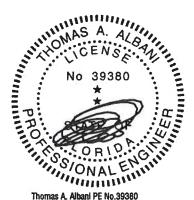
3-18=0/320, 3-16=-728/397, 5-16=-166/536, 5-15=-193/392, 6-15=-339/240,

7-15=-193/392, 7-14=-166/536, 9-14=-728/397, 9-12=0/320

### NOTES-

WEBS

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are 3x6 MT20 unless otherwise indicated.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=314, 10=314.



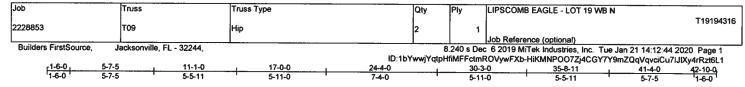
Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

January 21,2020

📤 WARNING - Verity design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer. Bracing individual temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/for chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see 

ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N Lee Street, Suite 312, Alexandria, VA 22314.





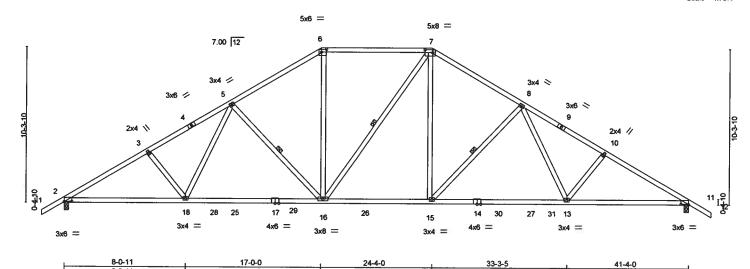


Plate Offsets (X,Y)	8-0-11 [2:0-6-0,0-0-3], [6:0-4-0,0-	8-11-5 2-4], [7:0-6-0,0-2-4], [1	1:0-6-0,0-0-3]	7-4-0		8-11-5	8-0-11	
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017/TP	1.25 TO 1.25 BO YES W	C 0.92	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.29 16-18 -0.53 13-15 0.13 11	Vdefi L/d >999 240 >938 180 n/a n/a	PLATES MT20 Weight: 241 lb	GRIP 244/190 FT = 20%

**BRACING-**

TOP CHORD

**BOT CHORD** 

**WEBS** 

33-3-5

1 Row at midpt

Structural wood sheathing directly applied.

Rigid ceiling directly applied or 2-2-0 oc bracing.

5-16, 7-16, 8-15

LUMBER-

REACTIONS.

WEBS

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2

2x4 SP No.3

(lb/size) 2=1610/0-3-8, 11=1610/0-3-8

Max Horz 2=-269(LC 10)

Max Uplift 2=-326(LC 12), 11=-326(LC 13)

Max Grav 2=1616(LC 19), 11=1625(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-2669/1163, 3-5=-2521/1155, 5-6=-1885/972, 6-7=-1577/906, 7-8=-1903/972, TOP CHORD

8-10=-2538/1155, 10-11=-2685/1163

**BOT CHORD** 2-18=-872/2435, 16-18=-667/2057, 15-16=-416/1593, 13-15=-670/1923, 11-13=-883/2247 3-18=-318/243, 5-18=-154/511, 5-16=-643/378, 6-16=-229/669, 7-15=-230/758, WEBS

8-15=-644/379, 8-13=-154/510, 10-13=-318/243

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb)



41-4-0

Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

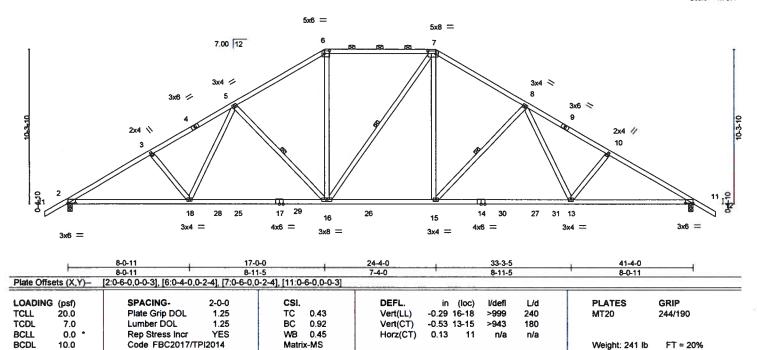
January 21,2020

🛦 WARNING - Verity design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Comp. Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss		Truss Type			Qty	Ply	LIPSCOMB EAGI	LE - LOT 19 WE	3 N	
											T19194317
2228853	T10		Piggyback Base			6	1				
								Job Reference (o	ptional)		
Builders FirstSource,	Jacksonville	e, FL - 32244,				8	.240 s Dec	6 2019 MiTek In	dustries, Inc. Ti	ue Jan 21 14:12:45	2020 Page 1
					ID:	1bYwwjY	qtpHfiMFF.	ctmROVywFXb-lu	ukblP0usrxqQ6.	J6tHoy125t02SdZ_5	SzBheNtzt6L0
<sub>c</sub> 1-6-0 ,	5-7-5	11-1-0	17-0-0		24-4-0		30-3	-0 ,	35-8-11	41-4-0	42-10-0
'1-6-0 <b>'</b>	5-7-5	5-5-11	5-11-0	ī	7-4-0		5-11-	-0	5-5-11	5-7-5	1-6-0



BRACING-

TOP CHORD

BOT CHORD

WEBS

except

1 Row at midpt

2-0-0 oc purlins (5-7-14 max.): 6-7.

Rigid ceiling directly applied or 2-2-0 oc bracing.

LUMBER-

TOP CHORD 2x4 SP No.2 \*Except\*

6-7: 2x4 SP M 31

2x4 SP No.2 **BOT CHORD** 

WEBS 2x4 SP No.3

REACTIONS. (lb/size) 2=1610/0-3-8, 11=1610/0-3-8

Max Horz 2=-269(LC 10) Max Uplift 2=-326(LC 12), 11=-326(LC 13) Max Grav 2=1616(LC 19), 11=1625(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2669/1163, 3-5=-2521/1155, 5-6=-1885/972, 6-7=-1577/906, 7-8=-1903/972,

8-10=-2538/1155, 10-11=-2685/1163

**BOT CHORD** 2-18=-872/2435, 16-18=-667/2057, 15-16=-415/1592, 13-15=-670/1923, 11-13=-883/2247 WEBS

3-18=-318/243, 5-18=-154/511, 5-16=-643/379, 6-16=-230/670, 7-15=-230/758,

8-15=-644/379, 8-13=-154/510, 10-13=-318/243

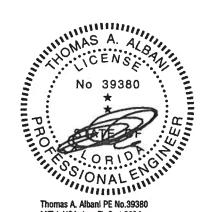
### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb)
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Structural wood sheathing directly applied or 3-4-12 oc purlins,

5-16, 7-16, 8-15

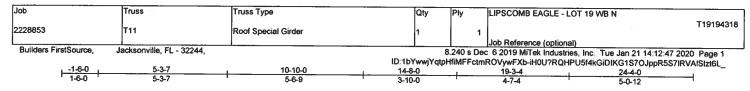
Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

January 21,2020

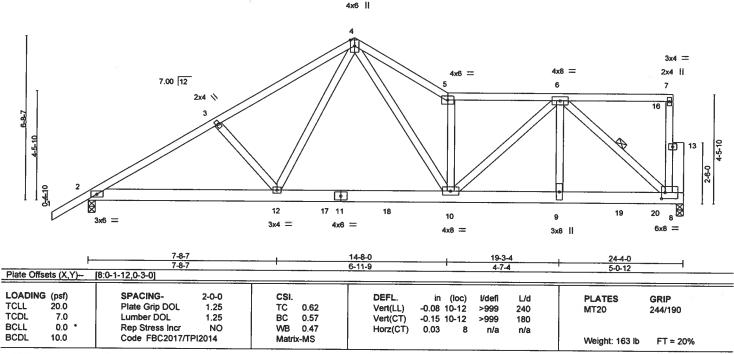
▲ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters and property incorporate this design in based only upon parameters and property incorporate this design into the overall building designer must verify the applicability of design parameters and property incorporate this design into the overall building design . Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP1 Quality Criteria, DSB-89 and BCSI Building Comp Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, NA 22314.





Scale = 1:45.3



LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x6 SP No.2 \*Except\*

8-11: 2x6 SP M 26 2x4 SP No.3 \*Except\*

**WEBS** 

8-13: 2x6 SP No.2

BRACING-

TOP CHORD

Structural wood sheathing directly applied or 4-0-7 oc purlins,

except end verticals

BOT CHORD WEBS

Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Row at midpt

REACTIONS. (lb/size) 2=1100/0-3-8, 8=2270/0-3-8

Max Horz 2=190(LC 27)

Max Uplift 2=-239(LC 8), 8=-686(LC 9)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1705/360, 3-4=-1538/353, 4-5=-2016/509, 5-6=-1665/386

**BOT CHORD** 2-12=-418/1435, 10-12=-244/1049, 9-10=-411/1577, 8-9=-411/1577 WEBS

3-12=-305/189, 4-12=-123/536, 4-10=-339/1233, 5-10=-1179/347, 6-10=-343/599,

6-9=-206/894, 6-8=-1921/489

### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=239, 8=686,
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 153 lb down and 109 lb up at 23-2-12 on top chord, and 1342 ib down and 387 ib up at 21-8-12, and 89 ib down and 84 ib up at 23-2-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

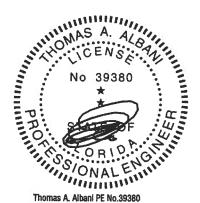
LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (ptf)

Vert: 1-4=54, 4-5=54, 5-7=54, 2-8=20

Concentrated Loads (lb)

Vert: 16=-123(B) 19=-1342(B) 20=-64(B)



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

January 21,2020

A WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design, Caracling Indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling designer and injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE - LOT 19 WB N
					T19194319
2228853	T12	Common	4	1	
1					Job Reference (optional)
Builders FirstSource	e, Jacksonville, FL - 32244,			3.240 s Dec	6 2019 MiTek Industries, Inc. Tue Jan 21 14:12:48 2020 Page 1
			ID:1bYwwjYqtp	HfMFFctm	ROVywFXb-ATatDnRvAnDWhurun?rVaggdjDDsq?Jvg9wlzCzt6Kz
	-1-6-0	5-2-0		987	10-4-0
	1-6-0	5-2-0	1		5-2-0

4x4 =

Scale = 1:22.7

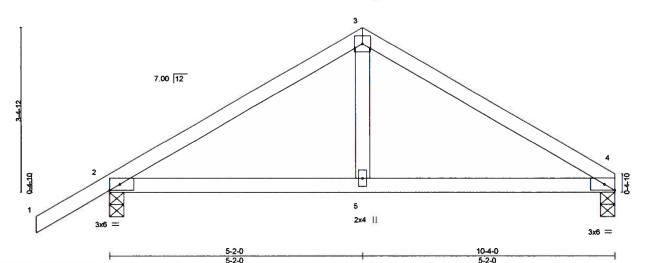


Plate Off	sets (X,Y)-	[4:0-2-8,Edge]										
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	Ľď	PLATES	GRIP
TCLL	20.Ó	Plate Grip DOL	1.25	тс	0.32	Vert(LL)	0.05	5-8	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.30	Vert(CT)	-0.05	5-8	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.09	Horz(CT)	0.00	4	n/a	n/a		
BCDL	10.0	Code FBC2017/T	PI2014	Matri	x-MS	1					Weight: 40 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 WEBS

**BRACING-**TOP CHORD

**BOT CHORD** 

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 8-4-8 oc bracing.

REACTIONS. (lb/size) 4=376/0-3-8, 2=469/0-3-8

Max Horz 2=112(LC 9)

Max Uplift 4=138(LC 13), 2=195(LC 12)

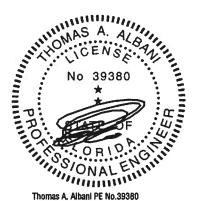
FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-3=-483/605, 3-4=-480/602

2-5=-440/362, 4-5=-440/362 BOT CHORD

WEBS 3-5=-377/235

### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 4=138, 2=195,



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

January 21,2020

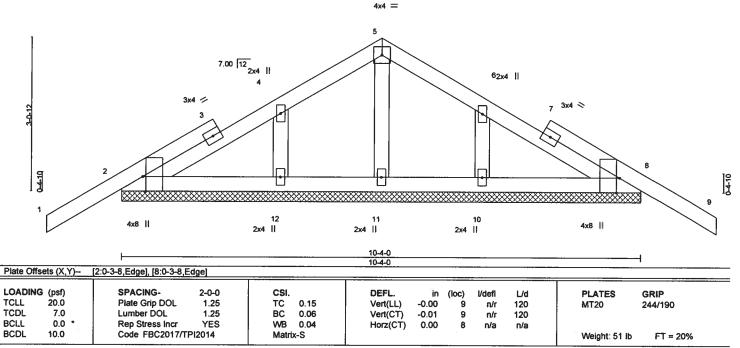
WARNING - Verity design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, crection and bracing of trusses and truss systems, see ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



ob	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE - LOT 19 WB N	
228853	T12G	Common Supported Gable		,		T19194320
	1120	Common Supported Gable		,	Job Reference (optional)	
Builders FirstSource, Ja	cksonville, FL - 32244,		8	.240 s Dec	6 2019 MiTek Industries, Inc. Tue Jan 21 14:	12:49 2020 Page 1
			bYwwjYqt	HfiMFFctr	mROVywFXb-ef8FQ7SXx5LNJ1Q4LjMk7tCr8d	clZTH2upgrVezt6Ky
					10-4-0	11-10-0
1-6-0	'	5-2-0			5-2-0	1-6-0
	228853	228853 T12G  Builders FirstSource, Jacksonville, FL - 32244,  -1-6-0	228853 T12G Common Supported Gable  Builders FirstSource, Jacksonville, FL - 32244,  ID:1	228853 T12G Common Supported Gable 1  Builders FirstSource, Jacksonville, FL - 32244, 8  ID:1bYwwjYqtp	228853 T12G Common Supported Gable 1 1  Builders FirstSource, Jacksonville, FL - 32244, ID:1bYwwjYqtpHfiMFFct	228853 T12G Common Supported Gable 1 1 Job Reference (optional)  Builders FirstSource, Jacksonville, FL - 32244,  Builders FirstSource,

Scale = 1:22.1



**BRACING-**

TOP CHORD

**BOT CHORD** 

LUMBER-

TOP CHORD 2x4 SP No.2

**BOT CHORD** 2x4 SP No.2

2x4 SP No.3 **OTHERS** 

REACTIONS. All bearings 10-4-0.

(lb) - Max Horz 2=-111(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2 except 8=-108(LC 13), 12=-123(LC 12), 10=-126(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 2, 8, 11, 12, 10

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 8=108, 12=123, 10=126.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

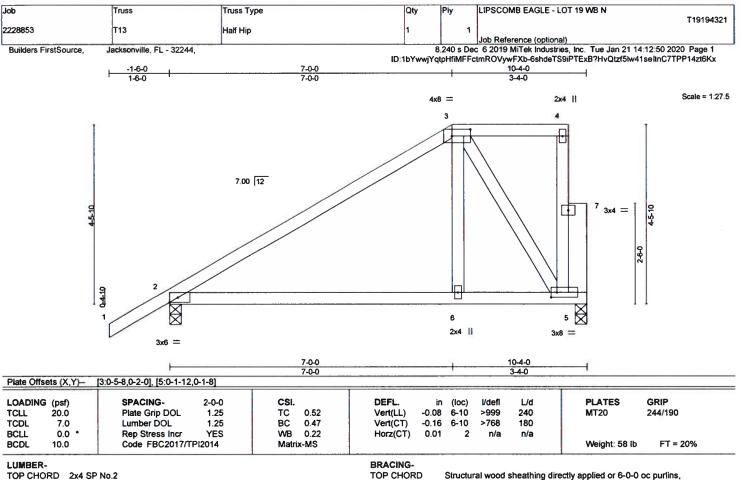
January 21,2020

🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev., 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property language. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANS/ITP11 Quality Criteria, DSB-89 and BCSI Building Comport Safety Information.

Available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





BOT CHORD

except end verticals

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3 \*Except\*

5-7: 2x6 SP No.2

REACTIONS. (lb/size) 2=455/0-3-8, 5=350/0-3-8

Max Horz 2=163(LC 12)
Max Uplift 2=-92(LC 12), 5=-89(LC 12)

Max Upiπ 2=-92(LC 12), 5=-89(LC 12

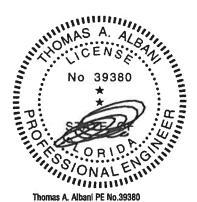
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-359/90

BOT CHORD 2-6=-158/264, 5-6=-159/272

WEBS 3-6=-19/279, 3-5=-487/276

### NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Provide adequate drainage to prevent water ponding.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 5.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

January 21,2020



6904 Parke East Blvd. Tampa, FL 36610

Job	Truss	Truss Type		Qty	Ply	LIPSCOMB EAGLE - LOT 19 WB N
2228853	T14	Hip Girder		1	1	T19194322
						Job Reference (optional)
Builders FirstSource, .	lacksonville, FL - 32244,			8	.240 s Dec	6 2019 MiTek Industries, Inc. Tue Jan 21 14:12:51 2020 Page 1
			ID:1b	YwwjYqtp	HfiMFFctn	nROVywFXb-a2F?rpTnTib5YLaTS7OCCIIBiQIP1NrLM79yaXzt6Kw
	1-6-0	3-0-0	4-3-8			7-3-8 , 8-9-8 ,
	1-6-0	3-0-0	1-3-8			3-0-0 1-6-0

Scale = 1:19.4

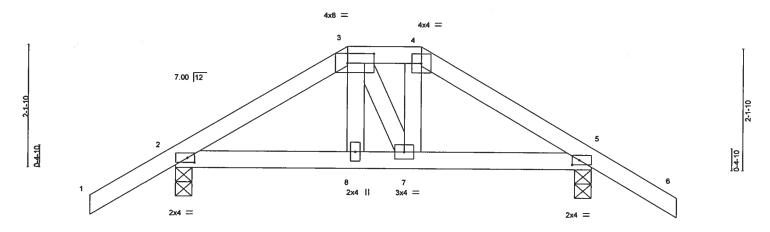


Plate Offse	ets (X,Y)	[2:0-1-8,0-1-0], [3:0-5-8,0	3-0- 3-0- 1-2-0], [5:0-1-8,	0	+-	4-3-8 1-3-8			7-3-8 3-0-0				
LOADING TCLL TCDL BCLL BCDL	(psf) 20,0 7,0 0,0 * 10,0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017/TI	2-0-0 1.25 1.25 NO Pl2014	BC 0	.15 .12 .04 IS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.01 -0.01 -0.00	(loc) 8 8-11 5	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 36 lb	GRIP 244/190 FT = 20%	

**BRACING-**

TOP CHORD

**BOT CHORD** 

LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2

2x4 SP No.3 **WEBS** 

REACTIONS. (lb/size) 2=378/0-3-8, 5=380/0-3-8

Max Horz 2=-66(LC 6)

Max Uplift 2=-227(LC 8), 5=-233(LC 9) Max Grav 2=379(LC 19), 5=380(LC 1)

FORCES. (ib) - Max. Comp./Max. Ten. - All forces 250 (ib) or less except when shown.

TOP CHORD 2-3=-363/318, 3-4=-286/305, 4-5=-365/338

**BOT CHORD** 2-8=-249/321, 7-8=-254/326, 5-7=-250/320

### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope); porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 2=227, 5=233
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 66 lb down and 66 lb up at 3-0-0, and 105 lb down and 152 lb up at 4-3-8 on top chord, and 112 lb down and 80 lb up at 3-0-0, and 112 lb down and 80 lb up at 4-2-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

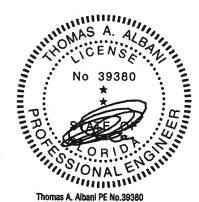
### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (ptf)

Vert: 1-3=-54, 3-4=-54, 4-6=-54, 9-12=-20

Concentrated Loads (lb)

Vert: 3=-6(F) 4=-14(F) 8=-19(F) 7=-19(F)



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd, Tampa FL 33610

January 21,2020

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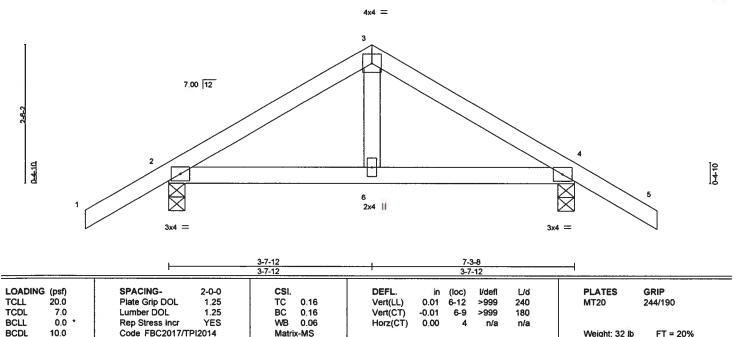
ANS/TPH Quality Criteria, DSB-89 and BCSI Building Comparately Information

available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE - LOT 19 WB N
1					T19194323
2228853	T15	Common	3	1	
					Job Reference (optional)
Builders FirstSource,	Jacksonville, FL - 32244,		8	3.240 s Dec	c 6 2019 MiTek Industries, Inc. Tue Jan 21 14:12:52 2020 Page 1
			ID:1bYwwjYqtpHfiN	IFFctmRO	VywFXb-2EpN39UPE0jyAV9f0rvRkWqMCqcymqmUbnuW6zzt6Kv
L	-1-6-0	3-7-12	1	7-3	-8 8-9-8
	1-6-0	3-7-12		3-7-	12 1-6-0

Scale = 1:20.0



LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 2x4 SP No.3 **BRACING-**

TOP CHORD **BOT CHORD**  Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 2=351/0-3-8, 4=351/0-3-8

Max Horz 2=-75(LC 10)

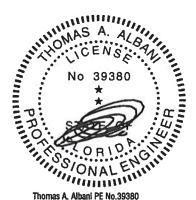
Max Uplift 2=-100(LC 9), 4=-100(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-302/394, 3-4=-302/394

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. It; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

  3) This trust has been designed for a 100 and better other lead accessed with the control of the cont
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20,0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=100, 4=100.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

January 21,2020

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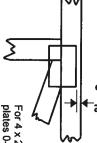


### **Symbols**

# PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y and tully embed teeth. Apply plates to both sides of truss Dimensions are in ft-in-sixteenths offsets are indicated



edge of truss. For 4 x 2 orientation, locate plates 0- 1/16' from outside

required direction of slots in This symbol indicates the

connector plates

\*Plate location detalls available in MiTek 20/20 software or upon request.

### PLATE SIZE

4 × 4

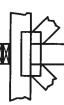
to slots. Second dimension is the length parallel to slots. width measured perpendicular The first dimension is the plate

## LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

### BEARING



Min size shown is for crushing only. number where bearings occur. reaction section indicates joint (supports) occur. Icons vary but Indicates location where bearings

### ANSI/TPI1: Industry Standards:

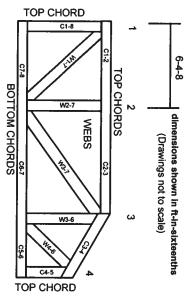
National Design Specification for Metal Plate Connected Wood Truss Construction Design Standard for Bracing

Guide to Good Practice for Handling, **Building Component Safety Information** Connected Wood Trusses. Installing & Bracing of Metal Plate

BCSI: **DSB-89** 

MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015

# **Numbering System**



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

NUMBERS/LETTERS. CHORDS AND WEBS ARE IDENTIFIED BY END JOINT

## PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

truss unless otherwise shown Trusses are designed for wind loads in the plane of the

section 6.3 These truss designs rely on lumber values established by others Lumber design values are in accordance with ANSI/TPI 1

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# **General Safety Notes**

## Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For bracing should be considered may require bracing, or alternative Tor I wide truss spacing, individual lateral braces themselves
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building all other interested parties. designer, erection supervisor, property owner and
- Cut members to bear tightly against each other

6

G

- Place plates on each face of truss at each locations are regulated by ANSI/TPI 1. oint and embed fully. Knots and wane at joint
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- shall not exceed 19% at time of fabrication Unless otherwise noted, moisture content of lumber
- use with fire retardant, preservative treated, or green lumber. Unless expressly noted, this design is not applicable for
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others
- 16. Do not cut or after truss member or plate without prior approval of an engineer
- 17. Install and load vertically unless indicated otherwise
- Use of green or treated lumber may pose unacceptable project engineer before use. environmental, health or performance risks. Consult with
- 19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

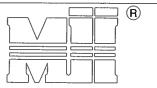
		8 * 0
w		

### T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

MII-T-BRACE 2



MiTek USA, Inc. Page 1 of 1



MiTek USA, Inc.

Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

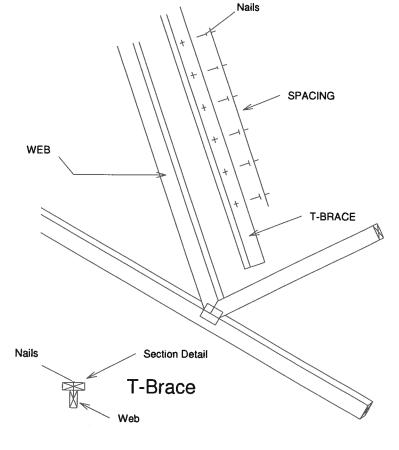
Nailing Pattern									
T-Brace size	Nail Size	Nail Spacing							
2x4 or 2x6 or 2x8	10d (0.131" X 3")	6" o.c.							

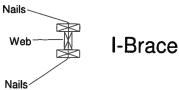
Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)

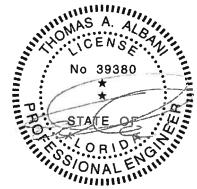
		Brace Size for One-Ply Truss				
		Continuous Iteral Bracing				
Web Size	1	2				
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace				
2x6	2x6 T-Brace	2x6 I-Brace				
2x8	2x8 T-Brace	2x8 I-Brace				

		Brace Size for Two-Ply Truss				
	Specified Continuous Rows of Lateral Bracir					
Web Size	1	2				
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace				
2x6	2x6 T-Brace	2x6 I-Brace				
2x8	2x8 T-Brace	2x8 T-Brace 2x8 I-Brace				

T-Brace / I-Brace must be same species and grade (or better) as web member.







Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

### SCAB-BRACE DETAIL

### MII-SCAB-BRACE

MiTek USA, Inc.

Page 1 of 1

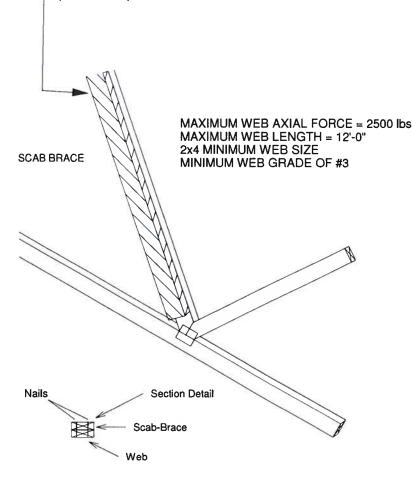


Note: Scab-Bracing to be used when continuous lateral bracing at midpoint (or T-Brace) is impractical.

Scab must cover full length of web +/- 6".

\*\*\* THIS DETAIL IS NOT APLICABLE WHEN BRACING IS \*\*\* REQUIRED AT 1/3 POINTS OR I-BRACE IS SPECIFIED.

APPLY 2x\_\_\_\_ SCAB TO ONE FACE OF WEB WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. SCAB MUST BE THE SAME GRADE, SIZE AND SPECIES (OR BETTER) AS THE WEB.



Scab-Brace must be same species grade (or better) as web member.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

### STANDARD REPAIR TO REMOVE END VERTICAL (RIBBON NOTCH VERTICAL)

MII-REP05

February 12, 2018

MiTek USA, Inc. Page 1 of 1



MiTek USA, Inc. ENGINEERED BY

1. THIS IS A SPECIFIC REPAIR DETAIL TO BE USED ONLY FOR ITS ORIGINAL INTENTION. THIS REPAIR DETAIL TO BE USED ONLY FOR ITS ORIGINAL
INTENTION. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION
OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO
VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED
REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING

HEPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING
THE LOADS INDICATED.

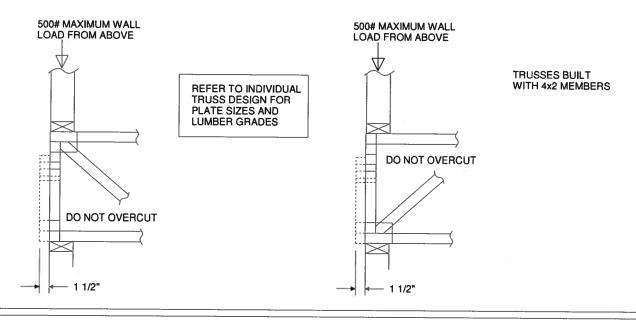
2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE
APPLYING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.

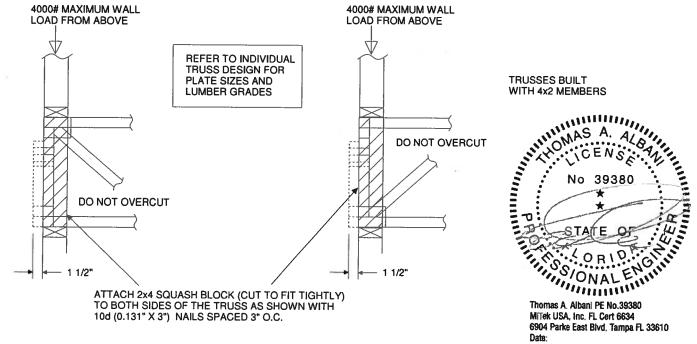
3. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE
SUCH AS TO AVOID SPLITTING OF THE WOOD.

4. LUMBER MUST BE CUT CLEANLY AND ACCURATELY AND THE REMAINING WOOD MUST BE UNDAMAGED.

5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 4X\_ ORIENTATION ONLY.

6. CONNECTOR PLATES MUST BE FULLY IMBEDDED AND UNDISTURBED.



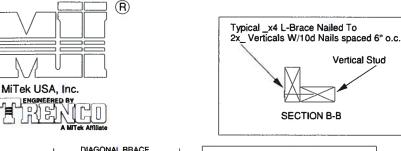


### Standard Gable End Detail

### MII-GE130-D-SP

Page 1 of 2

MiTek USA, Inc.



Vertical Stud DIAGONAL BRACE (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x\_ Verticals w/(4)-10d Nails SECTION A-A 2x4\Stud

DIAGONAL BRACE 4'-0" O.C. MAX TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY. Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA \*\* 3x4 =

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

- Diagonal Bracing Refer to Section A-A - L-Bracing Refer to Section B-B

### NOTE

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS. 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND

WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT 3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C. 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

DIAPHRAM AT 4-0" O.C.

6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)

GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.

THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.

DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE

06-01-13 BY SPIB/ALSC.

NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

f" Max	MI TO MAN
1'-3" Max.	(2) - 10d NAILS
	7russes @ 24" o.c.
Diag. Brace at 1/3 points if needed	2x6 DIAGONAL BRACE SPACED 48° O.C. ATTACHED TO VERTICAL WITH (4) -16d NAILS AND ATTACHED TO BLOCKING WITH (5) - 10d NAILS.
End Wall	HORIZONTAL BRACE (SEE SECTION A-A)

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD

SHEATHING TO 2x4 STD SPF BLOCK

Roof Sheathing

24

Minimum Stud Size Species	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS	
and Grade		Maximum Stud Length					
2x4 SP No. 3 / Stud	12" O.C.	3-9-13	4-1-1	5-9-6	7-1-3	11-5-7	
2x4 SP No. 3 / Stud	16" O.C.	3-5-4	3-6-8	5-0-2	6-10-8	10-3-13	
2x4 SP No. 3 / Stud	24" O.C.	2-9-11	2-10-11	4-1-1	5-7-6	8-5-1	

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING **EXPOSURE D** ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH ASCE 7-10 160 MPH **DURATION OF LOAD INCREASE: 1.60** 

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:



### Standard Gable End Detail

\*\*

### MII-GE130-SP

Page 1 of 2

(2) - 10d NAILS

∕Trusses @ 24" o.c.

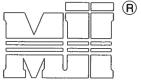
2x6 DIAGONAL BRACE SPACED 48" O.C.

ATTACHED TO VERTICAL WITH (4) -16d

TO BLOCKING WITH (5) - 10d NAILS.

NAILS AND ATTACHED

MiTek USA, Inc.



DIAGONAL BRACE

4'-0" O.C. MAX

MiTek USA, Inc.

Typical \_x4 L-Brace Nailed To 2x\_ Verticals W/10d Nails spaced 6" o.c. Vertical Stud SECTION B-B

TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY. Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING

DRAWINGS FOR DESIGN CRITERIA

24" Max

Diag. Brace

at 1/3 points

End Wall

if needed

3x4 =

Vertical Stud DIAGONAL BRACE (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x\_ Verticals w/(4)-10d Nails SECTION A-A

> PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X2.5") NAILS MINIMUM, PLYWOOD

(2) - 1Óg/

NAILS

SHEATHING TO 2x4 STD SPF BLOCK

Roof Sheathing

1'-3"

Max.

**Diagonal Bracing** Refer to Section A-A

L-Bracing Refer to Section B-B

### NOTE

- MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
   CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
- 3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT
- BRACING OF ROOF SYSTEM.

  4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB
  OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.

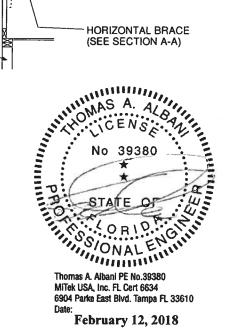
  5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF
- DIAPHRAM AT 4'-0" O.C.
- 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6\* O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
- GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
- DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES
- 10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
- 11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

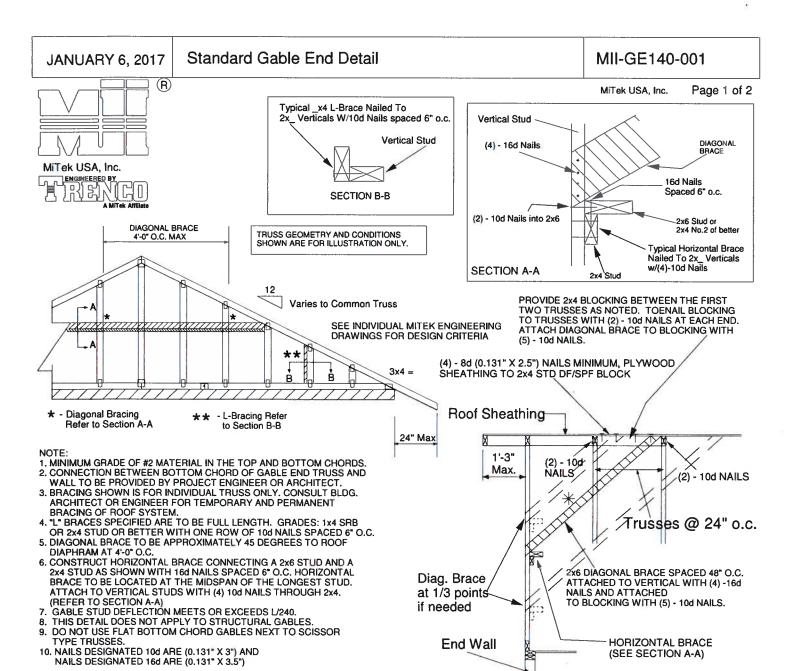
Minimum Stud Size Species	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS	
and Grade		Maximum Stud Length					
2x4 SP No. 3 / Stud	12" O.C.	4-0-7	4-5-6	6-3-8	8-0-15	12-1-6	
2x4 SP No. 3 / Stud		3-8-0	3-10-4	5-5-6	7-4-1	11-0-1	
2x4 SP No. 3 / Stud	24" O.C.	3-0-10	3-1-12	4-5-6	6-1-5	9-1-15	

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING **EXPOSURE B or C** ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH ASCE 7-10 160 MPH DURATION OF LOAD INCREASE: 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.





Minimum Stud Size Species	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS	
and Grade		Maximum Stud Length					
2x4 DF/SPF Std/Stud	12" O.C.	3-10-1	3-11-7	5-7-2	7-8-2	11-6-4	
2x4 DF/SPF Std/Stud	16" O.C.	3-3-14	3-5-1	4-10-2	6-7-13	9-11-11	
2x4 DF/SPF Std/Stud	24" O.C.	2-8-9	2-9-8	3-11-7	5-5-2	8-1-12	

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 1-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAXIMUM WIND SPEED = 140 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

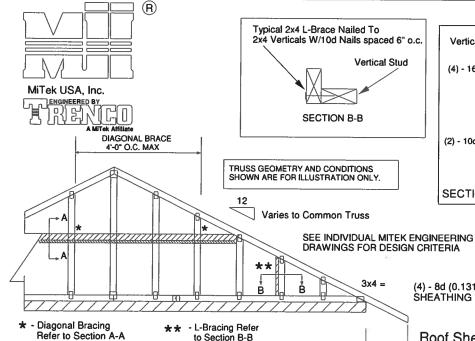
January 19, 2018

### Standard Gable End Detail

### MII-GE170-D-SP



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Vertical Stud 2X6 SP OR SPF No. 2 DIAGONAL BRACE (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2X6 SP OR SPF No. 2 Typical Horizontal Brace Nailed To 2x4 Verticals w/(4)-10d Nails SECTION A-A 2X4 SP OR SPF No. 2

> PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

NOTE

MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
 CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND

WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY, CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3 OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.

5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

DIAPHRAM AT 4'-0" O.C. DIAPHHAM AT 4-0" O.C.

6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A
2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL
BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD.
ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4.

(REFER TO SECTION A-A)
GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.

THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.

NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

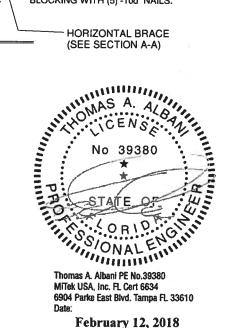
Roof Sheath	ning
1'-0" Max.	(2) - 10d NAILS
	Trusses @ 24" o.c.
Diag. Brace at 1/3 points if needed	2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d NAILS, AND ATTACHED TO BLOCKING WITH (5) -10d NAILS.
End Wall	HORIZONTAL BRACE (SEE SECTION A-A)

Minimum Stud Size	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS			
Species and Grade		Maximum Stud Length						
2x4 SP No. 3 / Stud	12" O.C.	3-9-7	5-8-8	6-11-1	11-4-4			
2x4 SP No. 3 / Stud	16" O.C.	3-4-12	4-11-15	6-9-8	10-2-3			
2x4 SP No. 3 / Stud	24" O.C.	2-9-4	4-0-7	5-6-8	8-3-13			
2x4 SP No. 2	12" O.C.	3-11-13	5-8-8	6-11-1	11-11-7			
2x4 SP No. 2	16" O.C.	3-7-7	4-11-5	6-11-1	10-10-5			
2x4 SP No. 2	24" O.C.	3-1-15	4-0-7	6-3-14	9-5-14			

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

MAX MEAN ROOF HEIGHT = 30 FEET EXPOSURE D ASCE 7-10 170 MPH **DURATION OF LOAD INCREASE: 1.60** 

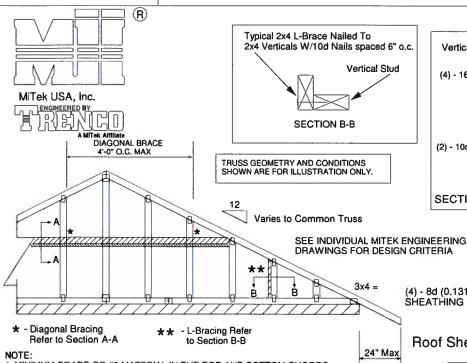
STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



### Standard Gable End Detail

### MII-GE180-D-SP

MiTek USA, Inc.



2X6 SP OR SPF No. 2 DIAGONAL BRACE

Page 1 of 2

16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2X6 SP OR SPF No. 2 Typical Horizontal Brace Nailed To 2x4 Verticals w/(4)-10d Nails **SECTION A-A** 2X4 SP OR SPF No. 2

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS. 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND

WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT

BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3

OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.

6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A

2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.

8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE

10. SOUTHERN FINE LOWINGER DESIGN VALUES AT 06-01-13 BY SPIB/ALSC. 11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

. 1	
Roof Sheath	ning
1'-0"	
Max.	(2) - 10d NAILS
	*** _ *
1	Trusses @ 24" o.c.
Diag. Brace	
at 1/3 points/ if needed	2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d NAILS, AND ATTACHED TO
	BLOCKING WITH (5) -10d NAILS.
End Wall	HORIZONTAL BRACE

Vertical Stud

(4) - 16d Nails

RIZONTAL BRACE (SEE SECTION A-A)

Minimum Stud Size Species	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS			
and Grade		Maximum Stud Length						
2x4 SP No. 3 / Stud	12" O.C.	3-7-12	5-4-11	6-2-1	10-11-3			
2x4 SP No. 3 / Stud	16" O.C.	3-2-8	4-8-1	6-2-1	9-7-7			
2x4 SP No. 3 / Stud	24" O.C.	2-7-7	3-9-12	5-2-13	7-10-4			
2x4 SP No. 2	12" O.C.	3-10-0	5-4-11	6-2-1	11-6-1			
2x4 SP No. 2	16" O.C.	3-5-13	4-8-1	6-2-1	10-5-7			
2x4 SP No. 2	24" O.C.	3-0-8	3-9-12	6-1-1	9-1-9			

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6in o.c., with 3in minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

MAX MEAN ROOF HEIGHT = 30 FEET EXPOSURE D ASCE 7-10 180 MPH

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS. **DURATION OF LOAD INCREASE: 1.60** 



Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date

MiTek USA, Inc. Page 1 of 1

(R)

MiTek USA, Inc.

ENGINEERED B 別以

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 ° O.C. CATEGORY II BUILDING EXPOSURE B or C ASCE 7-10

**DURATION OF LOAD INCREASE: 1.60** 

DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES).
ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.

A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
SHALL BE CONNECTED TO EACH PURLIN
WITH (2) (0.131\* X.3.5\*) TOE-NAILED.
B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24\* O.C.
UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING.
CONNECT TO BASE TRUSS WITH (2) (0.131\* X.3.5\*) NAILS EACH.
D - 2 X \_\_\_ X4\*0\* SCAB, SIZE TO MATCH TOP CHORD OF
PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED.
ON INTERSECTION, WITH (2) ROWS OF (0.131\* X.3\*) NAILS @ 4\* O.C.
SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING
IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH
DIRECTIONS AND:

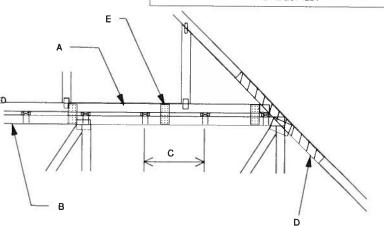
IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:

1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR

2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.

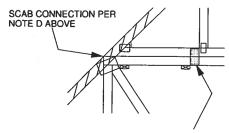
- FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72° O.C. W (4) (0.131° X 1.5°) NAILS PER MEMBER. STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5° EDGE DISTANCE.

(MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)

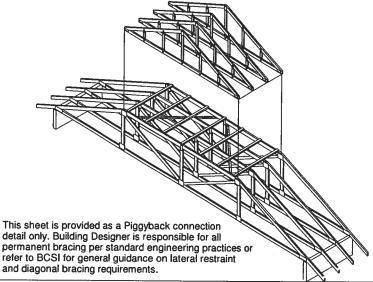


### WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

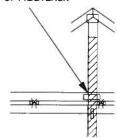
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



FOR ALL WIND SPEEDS, ATTACH MITEK 3X6 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) (0.131" X 1.5") PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE.



VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP

AS SHOWN IN DETAIL.

ATTACH 2 x \_\_\_ x 4-0\* SCAB TO EACH FACE OF

TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131\* X 3\*) NAILS

SPACED 4\* O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH

VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.)

THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS

GREATER THAN 400 LBS.
FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS,
NUMBER OF PIYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
CONCENTRATED LOAD MUST BE APPLIED TO BOTH
THE PIGGYBACK AND THE BASE TRUSS DESIGN.



Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

### STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

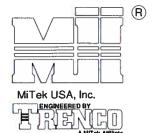
MII-PIGGY-ALT 7-10

MiTek USA, Inc. Page 1 of 1

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 \* O.C. CATEGORY II BUILDING EXPOSURE B or C ASCE 7-10

**DURATION OF LOAD INCREASE: 1.60** 

DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES).
ADDITIONAL CONSIDERATIONS BY BUILDING
ENGINEER/DESIGNER ARE REQUIRED.



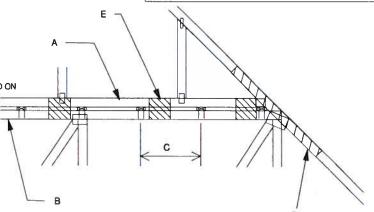
A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
SHALL BE CONNECTED TO EACH PURLIN
WITH (2) 0(0.131\* X 3.5") TOE-NAILED.
B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24\* O.C.
UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING.
CONNECT TO BASE TRUSS WITH (2) (0.131\* X 3.5") NAILS EACH.
D - 2 X \_\_ X 4\*-0" SCAB, SIZE TO MATCH TOP CHORD OF
PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED ON
INTERSECTION, WITH (2) ROWS OF (0.131\* X 3") NAILS @ 4\* O.C.
SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING
IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH
DIRECTIONS AND:

IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTTO
DIRECTIONS AND:

1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR

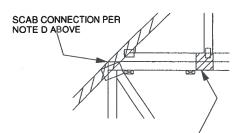
2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM
PIGGYBACK SPAN OF 12 ft.

E - FOR WIND SPEED IN THE RANGE 126 MPH - 160 MPH
ADD 9" x 9" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET
EACH SIDE AT 48" O.C. OR LESS. ATTACH WITH
3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM
EACH SIDE (TOTAL - 12 NAILS)

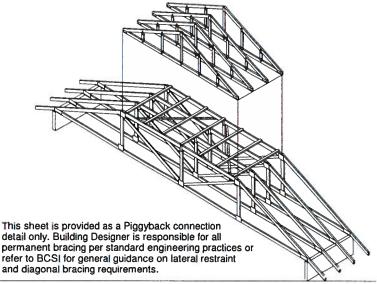


### WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

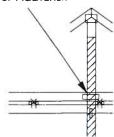
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH PLYWOOD GUSSETS AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



 $7^{\circ}$  x  $7^{\circ}$  x  $1/2^{\circ}$  PLYWOOD (or  $7/16^{\circ}$  OSB) GUSSET EACH SIDE AT 24° O.C. ATTACH WITH 3 - 6d (0.113° X 2°) NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)



VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP

AS SHOWN IN DETAIL.

ATTACH 2 x \_\_\_ x 4-0" SCAB TO EACH FACE OF
TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS
SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.)

VERTICAL WEBS OF PIGGYBACK AND BASE THUSS.)
(MINIMUM 2X4)
THIS CONNECTION IS ONLY VALID FOR A MAXIMUM
CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW
BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.

FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS. CONCENTRATED LOAD MUST BE APPLIED TO BOTH

5) THE PIGGYBACK AND THE BASE TRUSS DESIGN.



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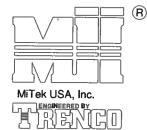
January 19, 2018

### STANDARD REPAIR DETAIL FOR BROKEN CHORDS, WEBS AND DAMAGED OR MISSING CHORD SPLICE PLATES

### MII-REP01A1

MiTek USA, Inc.

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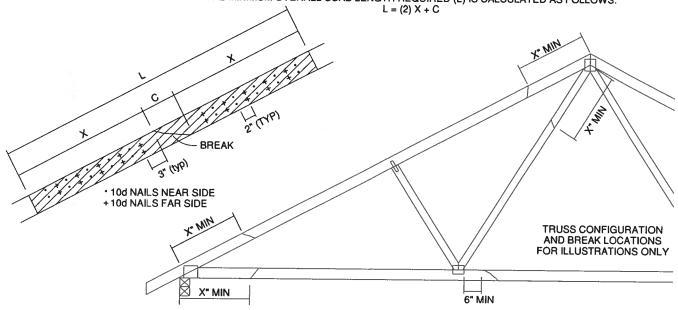


TOTAL NUMBER OF NAILS EACH SIDE OF BREAK *	MAXIMUM FORCE (Ibs) 15% LOAD DURATION									
	X INCHES	SP		DF		SPF		HF		
2x4	2x6		2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6
20	30	24"	1706	2559	1561	2342	1320	1980	1352	2028
26	39	30"	2194	3291	2007	3011	1697	2546	1738	2608
32	48	36"	2681	4022	2454	3681	2074	3111	2125	3187
38	57	42"	3169	4754	2900	4350	2451	3677	2511	3767
44	66	48"	3657	5485	3346	5019	2829	4243	2898	4347

\* DIVIDE EQUALLY FRONT AND BACK

ATTACH 2x\_SCAB OF THE SAME SIZE AND GRADE AS THE BROKEN MEMBER TO EACH FACE OF THE TRUSS (CENTER ON BREAK OR SPLICE) WITH 10d (0.131" X 3") NAILS (TWO ROWS FOR 2x4, THREE ROWS FOR 2x6) SPACED 4" O.C. AS SHOWN. STAGGER NAIL SPACING FROM FRONT FACE AND BACK FACE FOR A NET 0-2-0 O.C. SPACING IN THE MAIN MEMBER. USE A MIN. 0-3-0 MEMBER END DISTANCE.

THE LENGTH OF THE BREAK (C) SHALL NOT EXCEED 12". (C=PLATE LENGTH FOR SPLICE REPAIRS) THE MINIMUM OVERALL SCAB LENGTH REQUIRED (L) IS CALCULATED AS FOLLOWS:



THE LOCATION OF THE BREAK MUST BE GREATER THAN OR EQUAL TO THE REQUIRED X DIMENSION FROM ANY PERIMETER BREAK OR HEEL JOINT AND A MINIMUM OF 6" FROM ANY INTERIOR JOINT (SEE SKETCH ABOVE)

### DO NOT USE REPAIR FOR JOINT SPLICES

### NOTES:

- NOTES:

  1. THIS REPAIR DETAIL IS TO BE USED ONLY FOR THE APPLICATION SHOWN. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.

  2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.

  3. THE END DISTANCE, EDGE DISTANCE AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.

  4. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.

  5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 2x\_ ORIENTATION ONLY.

  6. THIS REPAIR IS LIMITED TO TRUSSES WITH NO MORE THAN THREE BROKEN MEMBERS.



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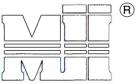
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### LATERAL TOE-NAIL DETAIL

MII-TOENAIL\_SP

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### NOTES:

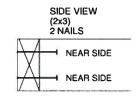
- 1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER 1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.

  2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.

  3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES
- FOR MEMBERS OF DIFFERENT SPECIES.

### THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY



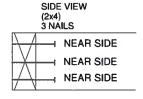
### OE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail) DIAM. SP SPF SPF-S .131 88.0 80.6 69.9 68.4 59.7 3.5" LONG .135 93.5 85.6 74.2 72.6 63.4 84.5 73.8 .162 108.8 99.6 86.4 LONG .128 74.2 57.6 67.9 58.9 50.3 75,9 69.5 60.3 59.0 51.1 .131 3.25 81.4 74.5 64.6 63.2 52.5 .148

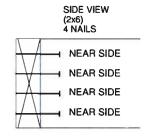
VALUES SHOWN ARE CAPACITY PER TOE-NAIL. APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

(3) - 16d (0.162" X 3.5") NAILS WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1.15:

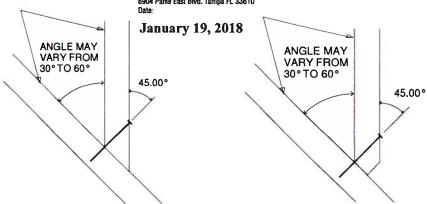
3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity

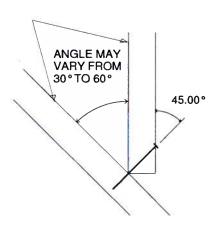






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### TRUSSED VALLEY SET DETAIL

MII-VALLEY HIGH WIND1

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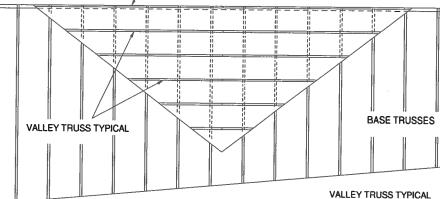


MiTek USA, Inc. ENGINEERED BY

GABLE END, COMMON TRUSS OR GIRDER TRUSS

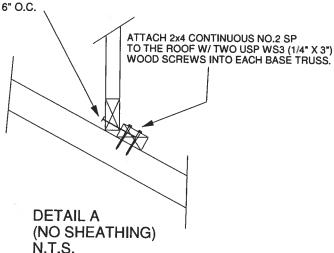
### **GENERAL SPECIFICATIONS**

- 1. NAIL SIZE 10d (0.131" X 3")
  2. WOOD SCREW = 3" WS3 USP OR EQUIVALENT DO NOT USE DRYWALL OR DECKING TYPE SCREW
- 3. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
- 4. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
- 5. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
- 6. NAILING DONE PER NDS 01
- 7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.



VALLEY TRUSS TYPICAL GABLE END, COMMON TRUSS 12 OR GIRDER TRUSS SEE DETAIL A BELOW (TYP.)

SECURE VALLEY TRUSS W/ ONE ROW OF 10d NAILS 6" O.C.



WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12 CATEGORY II BUILDING **EXPOSURE C** WIND DURATION OF LOAD INCREASE: 1.60 MAX TOP CHORD TOTAL LOAD = 50 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 6 PSF ON THE TRUSSES



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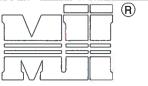
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### TRUSSED VALLEY SET DETAIL

MII-VALLEY HIGH WIND2

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GABLE END, COMMON TRUSS OR GIRDER TRUSS

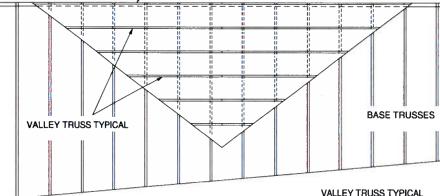
**GENERAL SPECIFICATIONS** 

1. NAIL SIZE 10d (0.131" X 3")
2. WOOD SCREW = 4.5" WS45 USP OR EQUILIVANT
3. INSTALL SHEATHING TO TOP CHORD OF BASE TRUSSES.
4. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE TO BASE TRUSSES AS PER DETAIL A
5. BRACE VALLEY WEBS IN ACCORDANCE WITH THE

INDIVIDUAL DESIGN DRAWINGS.

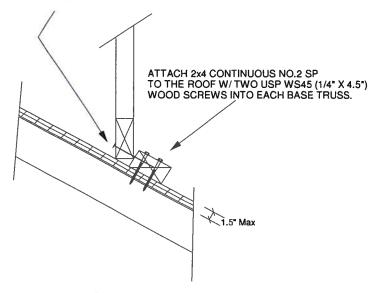
6. NAILING DONE PER NDS-01

7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.



GABLE END, COMMON TRUSS VALLEY TRUSS TYPICAL P 12 OR GIRDER TRUSS SEE DETAIL A BELOW (TYP.)

**SECURE VALLEY TRUSS** W/ ONE ROW OF 10d NAILS 6" O.C.



WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12 CATEGORY II BUILDING EXPOSURE C WIND DURATION OF LOAD INCREASE: 1.60
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24\* O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 6 PSF
ON THE TRUSSES

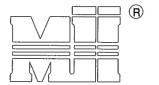
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STATE OF THE OF THE OWNER AND ADDRESS A Albani PE No.39380

Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

MiTek USA, Inc.

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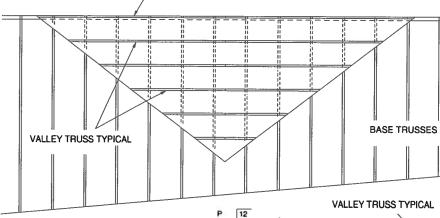
MiTek USA, Inc. ENGINEERED BY

GABLE END, COMMON TRUSS OR GIRDER TRUSS

### **GENERAL SPECIFICATIONS**

- 1. NAIL SIZE 16d (0.131" X 3.5") 2. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
- 3. BRACE VALLEY WEBS IN ACCORDANCE WITH THE
- INDIVIDUAL DESIGN DRAWINGS.

  4. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
- 5. NAILING DONE PER NDS 01 6. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.
- 7. ALL LUMBER SPECIES TO BE SP.



GABLE END, COMMON TRUSS OR GIRDER TRUSS P 12 SEE DETAIL A BELOW (TYP.)

SECURE VALLEY TRUSS W/ ONE ROW OF 16d NAILS 6" O.C. ATTACH 2x4 CONTINUOUS NO.2 SP TO THE ROOF W/TWO 16d NAILS INTO EACH BASE TRUSS.

DETAIL A (MAXIMUM 1" SHEATHING) N.T.S.

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 120 MPH WIND DESIGN PER ASCE 7-10, 150 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
ROOF PITCH = MINIMUM 3/12 MAXIMUM 10/12
CATEGORY II BUILDING EXPOSURE C OR B WIND DURATION OF LOAD INCREASE: 1.60 MAX TOP CHORD TOTAL LOAD = 60 PSF
MAX SPACING = 24° O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 4.2 PSF
ON THE TRUSSES



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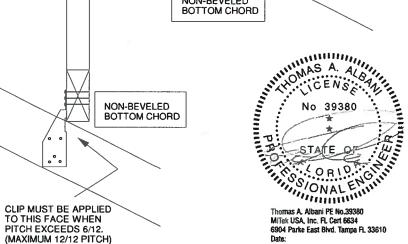
TRUSSED VALLEY SET DETAIL **MII-VALLEY** AUGUST 1, 2016 (HIGH WIND VELOCITY) R NOTE: VALLEY STUD SPACING NOT Page 1 of 1 MiTek USA, Inc. TO EXCEED 48" O.C. SPACING MiTek USA, Inc. ENGINEERED BY FOR BEVELED BOTTOM CHORD, CLIP MAY BE APPLIED TO EITHER FACE CLIP MAY BE APPLIED TO THIS FACE UP TO A MAXIMUM 6/12 PITCH ATTACH VALLEY TRUSSES TO LOWER TRUSSES WITH **USP RT7 OR EQUIVALENT** WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET **CATEGORY II BUILDING** NON-BEVELED **EXPOSURE B or C BOTTOM CHORD** WIND DURATION OF LOAD INCREASE: 1.6 MAX TOP CHORD TOTAL LOAD = 50 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY) SUPPORTING TRUSSES DIRECTLY UNDER VALLEY TRUSSES MUST BE DESIGNED WITH A MAXIMUM UNBRACED LENGTH OF

### NOTES:

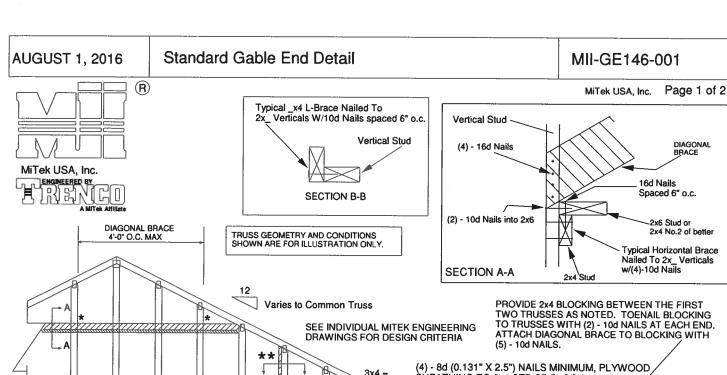
- SHEATHING APPLIED AFTER INSTALLATION OF VALLEY TRUSSES

2'-10" ON AFFECTED TOP CHORDS.

- THIS DETAIL IS NOT APPLICABLE FOR SPF-S SPECIES LUMBER.



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### NOTE

- Diagonal Bracing

Refer to Section A-A

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS. 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND

L-Bracing Refer

to Section B-B

- WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
- 3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY, CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT
  BRACING OF ROOF SYSTEM.

  4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES:
  2x4 No 3/STUD SP OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.

  5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

- DIAPHRAM AT 4'-0" O.C
- DIAPHHAM AT 4-0 O.C.

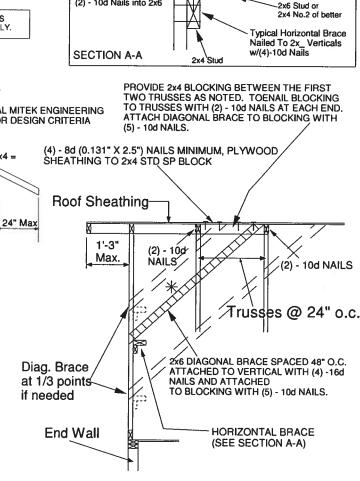
  6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 160 NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 100 NAILS THROUGH 2x4. (REFER TO SECTION A-A)
- GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
- THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
  DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR
- TYPE TRUSSES
- 10. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAI BRACES AT 1/3 POINTS		
		Maximum Stud Length					
2x4 SP No 3/Stud	12" O.C.	3-11-3	6-8-0	7-2-14	11-9-10		
2x4 SP No 3/Stud	16" O.C.	3-6-14	5-9-5	7-1-13	10-8-11		
2x4 SP No 3/Stud	24" O.C.	3-1-8	4-8-9	6-2-15	9-4-7		

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAXIMUM WIND SPEED = 146 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 **DURATION OF LOAD INCREASE: 1.60** 

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



DIAGONAL

16d Nails

Spaced 6" o.c.



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January 19, 2018

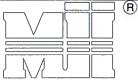
### **OCTOBER 5, 2016**

### REPLACE BROKEN OVERHANG

MII-REP13B

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ENGINEERED B

TRUSS CRITERIA:

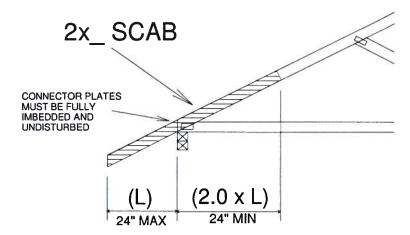
LOADING: 40-10-0-10 **DURATION FACTOR: 1.15** SPACING: 24" O.C. TOP CHORD: 2x4 OR 2x6 PITCH: 4/12 - 12/12 HEEL HEIGHT: STANDARD HEEL UP TO 12" ENERGY HEEL

**END BEARING CONDITION** 

NOTES:

ATTACH 2x\_ SCAB (MINIMUM NO.2 GRADE SPF, HF, SP, DF) TO ONE FACE OF TRUSS WITH TWO ROWS OF 10d (0.131" X 3") SPACED 6" O.C.
 THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
 WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED.

TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.



### **IMPORTANT**

This detail to be used only with trusses (spans less than 40') spaced 24" o.c. maximum and having pitches between 4/12 and 12/12 and total top chord loads not exceeding 50 psf. Trusses not fitting these criteria should be examined individually.

REFER TO INDIVIDUAL TRUSS DESIGN FOR PLATE SIZES AND LUMBER GRADES



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

