Width (ft) = 50Eave Height (ft) Length (ft) = 100Roof Slope (Rise/12) = 4.0:12

BUILDING LOADS

- A) THIS IS TO CERTIFY THAT THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY FBC 23 / 8TH EDITION
- B) THIS CERTIFICATION IS LIMITED TO THE STRUCTURAL DESIGN OF THE FRAMING AND COVERING PARTS MANUFACTURED BY THE BUILDING MANUFACTURER AND AS SPECIFIED IN THE CONTRACT. ACCESSORY ITEMS SUCH AS DOORS, WINDOWS, LOUVERS, TRANSLUCENT PANELS, VENTILATORS ARE NOT INCLUDED. ALSO EXCLUDED ARE OTHER PARTS OF THE PROJECT NOT PROVIDED BY THE BUILDING MANUFACTURER SUCH AS FOUNDATIONS, MASONRY WALLS, MECHANICAL EQUIPMENT AND THE ERECTION AND INSPECTION OF THE BUILDING. THE BUILDING SHOULD BE ERECTED ON A PROPERLY DESIGNED FOUNDATION IN ACCORDANCE WITH THE BUILDING MANUFACTURER'S DESIGN MANUAL, THE ATTACHED DRAWINGS, AND GOOD ERECTION PRACTICES. THE END USER AND/OR ENGINEER OF RECORD IS TO CONFIRM THAT THESE LOADS COMPLY WITH REQUIREMENTS OF THE

	H 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
OCCUPANCY/RISK CATEGORY	<u>II - Normal</u> <u>Is 1.0000 (e 1.00</u>
WIND LOAD ULTIMATE	119 MPH NOMINAL 92.18 MPH WIND EXPOSURE B
CLOSURE TYPE	Enclosed INTERNAL WIND COEF0.18 / 0.18
GROUND SNOW LOAD	0.00 PSF ROOF SNOW LOAD 0.00 PSF Ce 1.0000 Ct 1.2000
SNOW BANKING LOADS	PER CODE
COLLATERAL DEAD LOAD	0.50 PSF
ROOF LIVE LOAD	20.00 PSF (REDUCIBLE Yes)
DEAD LOAD	2.00 PSF (FOR ROOF PANELS AND PURLINS)
SEISMIC	
SPECTRAL RESPONSE Ss 0.12	00 \$1_0.0580 \$ds_0.1067 \$d1_0.0800
SITE CLASS D	DESIGN RISK CATEGORY B Cs 0.0356
RESPONSE MODIFICATION FACTOR, R	3.000* FRAMES 3.000* BRACING
BASIC SEISMIC FORCE RESISTING SYS	STEM (LATERAL DIRECTIONS) = ORDINARY STEEL MOMENT FRAMES
BASIC SEISMIC FORCE RESISTING SYS	STEM (ENDWALLS) = ORDINARY STEEL MOMENT FRAMES
BASIC SEISMIC FORCE RESISTING SYS	STEM (LONGITUDINAL DIRECTIONS) = ORDINARY STEEL CONC. BRACED FRAMES

= EQUIVALENT LATERAL FORCE PROCEDURE

ANALYSIS PROCEDURE SERVICEABILITY CRITERIA

STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR

	MINIMUM DESI	GN DEFLECTIONS		
Endwall Column	= 180	Roof Panel (Live)	=	60
Endwall Rafter (Live)	= 360	Roof Panel (Wind)	=	60
Endwall Rafter (Wind)	= 180	Rigid Frame (Horz)	=	60
Wall Girt	= 90	Rigid Frame (Vert)	=	360
Roof Purlin (Live)	= 150	Rîgid Frame (Seismîc)	=	50
Roof Purlin (Wind)	= 150			
Wall Panel	= 60			

GENERAL NOTES

- A) THE STRUCTURE UNDER THIS CONTRACT HAS BEEN DESIGNED AND DETAILED FOR THE LOADS AND CONDITIONS STIPULATED IN THE CONTRACT AND SHOWN ON THESE DRAWINGS. ANY ALTERATIONS TO THE STRUCTURAL SYSTEM OR REMOVAL OF ANY COMPONENT PARTS, OR THE ADDITION OF OTHER CONSTRUCTION MATERIALS OR LOADS MUST BE DONE UNDER THE ADVICE AND DIRECTION OF A REGISTERED ARCHITECT, CIVIL OR STRUCTURAL ENGINEER. THE BUILDING MANUFACTURER WILL ASSUME NO RESPONSIBILITY FOR ANY LOADS NOT INDICATED.

 B) THIS METAL BUILDING IS DESIGNED WITH THE BUILDING MANUFACTURER'S STANDARD PRACTICES WHICH ARE BASED
- ON PERTINENT PROCEDURES AND RECOMMENDATIONS OF THE FOLLOWING ORGANIZATIONS AND CODES.
- 1. AMERICAN INSTITUTE OF STEEL CONSTRUCTION: "AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS—ALLOWABLE STRESS DESIGN" AS ADOPTED BY THE BUILDING CODE REFERENCED IN "BUILDING LOADS" SECTION "A" ABOVE.
- AMERICAN IRON AND STEEL INSTITUTE: "SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS" AS ADOPTED BY THE BUILDING CODE REFERENCED IN "BUILDING LOADS" SECTION "A" ABOVE.
- 3. AMERICAN WELDING SOCIETY: "STRUCTURAL WELDING CODE" AWS D1.1. AS ADOPTED BY THE BUIDLING CODE REFERENCED IN "BUILDING LOADS" SECTION "A" ABOVE.
- 4. METAL BUILDING MANUFACTURER'S ASSOCIATION: "LOW RISE BUILDING SYSTEMS MANUAL" AS ADOPTED BY THE BUILDING CODE REFERENCED IN "BUILDING LOADS" SECTION "A" ABOVE. C) 1) MATERIAL PROPERTIES OF STEEL PLATE USED IN THE FABRICATION OF PRIMARY RIGID FRAMES, AND OTHER
- PRIMARY STRUCTURAL EXCLUSIVE OF COLD—FORMED SECTIONS, CONFORM TO ASTM—A529 OR A572 . FLANGES AND WEB MATERIAL CONFORMS TO ASTM—A529 OR A572 GRADE 55 WITH A MINIMUM YIELD POINT OF 55,000 pb..
- 2) MATERIAL PROPERTIES OF HSS ROUND SECTIONS CONFORM TO ASTM-A500, GRADE B OR C WITH A MINIMUM YIELD POINT OF 42,000 psi.

 3) MATERIAL PROPERTIES OF HSS RECT. OR SQUARE SECTIONS CONFORM TO ASTM—A500, GRADE B OR C WITH A MINIMUM
- YIELD POINT OF 46,000 psi.

 4) MATERIAL PROPERTIES OF HOT ROLLED CHANNEL AND ANGLE MEMBERS CONFORM TO THE REQUIREMENTS OF ASTM-A992 WITHMINIMUM YIELD POINT OF 50,000 PSI, HOT ROLLED W-SHAPED MEMBERS CONFORM TO THE REQUIREMENTS OF ASTM-A992WITH MINIMUM YIELD POINT OF 50,000 PSI.
- 5) MATERIAL PROPERTIES OF COLD FORMED LIGHT CAGE STEEL MEMBERS CONFORM TO EITHER ASTM A653-06 GR 55 OR A1011-04 HSLAS GRADE 55 WITH YIELD OF 55,000 psi.
- 6) MATERIAL PROPERTIES OF ROOF/WALL SHEETING, BASE METAL CONFORM TO ASTM-A792 GRADES 80 CLASS 1, 2 OR 3 WITH A MINIMUM YIELD STRENGTH OF 80,000 PSI. COATING OF BASE MATERIAL IS 55% ALUMINUM-ZINC ALLOY IN ACCORDANCE WITH AZ55 SPECIFICATIONS.
- 7) CABLE UTILIZED FOR BRACING CONFORMS TO ASTM A475. CABLE BRACING IS TO BE INSTALLED TO A TAUT
- B) ROU UTILIZED FOR BRACING MEMBERS CONFORM TO ASTM—A36 WITH MINIMUM YIELD POINT OF 36,000 PSI.

 9) IT IS THE RESPONSIBILITY OF ERECTOR TO ENSURE PROPER BOLT TIGHTNESS IN ACCORDANCE WITH APPLICABLE "RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING A-325 OR A-490 BOLTS". ALL A-325 BOLTS IN PRIMARY FRAMING MUST BE "SNUG—TIGHT". EXCEPT AS FOLLOWS:

 "FULLY—PRETENSION". A-325 BOLTS IF:
- a) BUILDING LOCATED IN A HIGH SEISMIC AREA. FOR IBC-BASED CODE, "HIGH SEISMIC AREA" IS DEFINED AS "SEISMIC DESIGN CATEGORY" OF "D", "E" OR "F".
- b) BUILDING SUPPORTS A CRANE SYSTEM WITH A CAPACITY GREATER THAN 5.00 TONS.
- c) BUILDING SUPPORTS MACHINERY THAT CREATES VIBRATION, IMPACT OR STRESS REVERSALS ON THE CONNECTIONS.
- d) ANY CONNECTION DESIGNATED IN THESE DRAWINGS AS "A-325 SC".

- 10) SECONDARY MEMBERS AND FLANGE BRACE CONNECTIONS SHALL ALWAYS BE SNUG TIGHT, UNO.
- ANCHOR BOLTS 3/4" IN DIAMETER THRU 1 1/4" IN DIAMETER CONFORM TO A.S.T.M. F1554 GR. 36. ANCHOR BOLTS 1/2" IN DIAMETER CONFORM TO A.S.T.M. A-307.
- D) UNLESS NOTED OTHERWISE ON FRAMING COLOR CHART: ALL STEEL MEMBERS EXCEPT BOLTS, FASTENERS, CABLE AND RODS SHALL RECEIVE ONE COAT OF STANDARD RED OXIDE SHOP PRIMER.
- E) SHOP AND FIELD INSPECTIONS AND ASSOCIATED FEES ARE THE RESPONSIBILITY OF THE CONTRACTOR, UNLESS STIPULATED OTHERWISE IN THE CONTRACT.

APPROVAL NOTES

THE FOLLOWING CONDITIONS APPLY IN THE EVENT THAT THESE DRAWINGS ARE USED AS APPROVAL DRAWINGS: A) IT IS IMPERATIVE THAT ANY CHANGES TO THESE DRAWINGS:

- 1) BE MADE IN CONTRASTING INK.
- 2) HAVE ALL INSTANCES OF CHANGE CLEARLY INDICATED.
- 3) BE LEGIBLE AND UNAMBIGUOUS.
- B) DATED SIGNATURE IS REQUIRED ON ALL PAGES.
- C) MANUFACTURER RESERVES THE RIGHT TO RESUBMIT DRAWINGS WITH EXTENSIVE OR COMPLEX CHANGES REQUIRED TO AVOID MISEABRICATION, THIS MAY IMPACT THE DELIVERY SCHEDULE
- D) APPROVAL OF THESE DRAWINGS INDICATES CONCLUSIVELY THAT THE MANUFACTURER HAS CORRECTLY INTERPRETED THE CONTRACT REQUIREMENTS, AND FURTHER CONSTITUTES AGREEMENT THAT THE BUILDING AS DRAWN, OR AS DRAWN WITH INDICATED CHANGES REPRESENTS THE TOTAL OF THE MATERIALS TO BE SUPPLIED
- E) ANY CHANGES NOTED ON THE DRAWINGS NOT IN CONFORMANCE WITH THE TERMS AND REQUIREMENTS OF THE CONTRACT BETWEEN MANUFACTURER AND ITS CUSTOMER ARE NOT BINDING ON MANUFACTURER UNLESS SUBSEQUENTLY SPECIFICALLY ACKNOWLEDGED AND AGREED TO IN WRITING BY CHANGE ORDER OR SEPARATE DOCUMENTATION, MANUFACTURER RECOGNIZES THAT RUBBER STAMPS ARE ROUTINELY USED FOR INDICATING APPROVAL, DISAPPROVAL, REJECTION, OR MERE REVIEW OF THE DRAWINGS SUBMITTED. HOWEVER, MANUFACTURER DOES NOT ACCEPT CHANGES OR ADDITIONS TO CONTRACTUAL TERMS AND CONDITIONS THAT MAY APPEAR WITH USE OF A STAMP OR SIMILAR INDICATION OF APPROVAL, DISAPPROVAL, ETC., SUCH LANGUAGE APPLIED TO MANUFACTURER'S DRAWINGS BY THE CUSTOMER, ARCHITECT, ENGINEER, OR ANY OTHER PARTY WILL BE CONSIDERED AS UNACCEPTABLE ALTERATIONS TO THESE DRAWING NOTES, AND WILL NOT ALTER THE CONTRACTUAL RIGHTS AND OBLIGATIONS EXISTING BETWEEN MANUFACTURER AND ITS CUSTOMER.

SAFETY COMMITMENT

- A) THE BUILDING MANUFACTURER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE ERECTOR ARE BEYOND THE CONTROL OF THE BUILDING MANUFACTURER.
- IT IS STRONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP PRIORITY OF ANY JOB SITE
- C) LOCAL, STATE AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE WORKER SAFETY
- MAKE CERTAIN ALL EMPLOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING A BUILDING. EMERGENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES.

 E) DAILY MEETINGS HIGHLIGHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HARD HATS, RUBBER
- SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFETY NETS WHERE APPLICABLE.

ERECTOR / CONTRACTOR RESPONSIBILITIES

- A) IT IS THE RESPONSIBILITY OF THE ERECTOR/CONTRACTOR TO INSURE THAT ALL PROJECT PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED Engineering data and drawings for the metal building system does not imply or constitute an agreement that the building manufacturer or its design engineer is acting as the engineer of record or design PROFESSIONAL FOR A CONSTRUCTION PROJECT.
- B) THE CONTRACTOR MUST SECURE ALL REQUIRED APPROVALS AND PERMITS FROM THE APPROPRIATE AGENCY AS
- C) APPROVAL OF THE MANUFACTURER'S DRAWINGS AND CALCULATIONS INDICATE THAT THE BUILDING MANUFACTURER CORRECTLY INTERPRETED AND APPLIED THE REQUIREMENTS OF THE CONTRACT DRAWINGS AND SPECIFICATIONS.

 (SECT. 4.4.1 AISC CODE OF STANDARD PRACTICES, LATEST ED.)

 D) WHERE DISCREPANCIES EXIST BETWEEN THE MANUFACTURER'S STRUCTURAL STEEL PLANS AND THE PLANS FOR OTHER TRADES, THE STRUCTURAL STEEL PLANS SHALL GOVERN. (SECT. 3.3 AISC CODE OF STANDARD PRACTICE LATEST ED.)

 E) DESIGN CONSIDERATIONS OF ANY MATERIALS IN THE STRUCTURE WHICH ARE NOT FURNISHED BY THE BUILDING
- MANUFACTURER ARE THE RESPONSIBILITY OF THE CONTRACTORS AND ENGINEERS OTHER THAN THE BUILDING MANUFACT-
- ANUFACTURER ARE THE RESPONDENCE.

 ANUFACTURER ARE THE RESPONDENCE.

 RER'S ENGINEERS UNLESS SPECIFICALLY INDICATED.

 THE ENECTOR/CONTRACTOR IS RESPONSIBLE FOR ALL ERECTION OF STEEL AND

 PRODUCTS SHIPPED TO ERECTOR/CONTRACTOR OR HIS CUSTOMER SHALL BE INSPECTED BY ERECTOR/CONTRACTOR

 PRODUCTS SHIPPED TO ERECTOR/CONTRACTOR OR HIS CUSTOMER SHALL BE INSPECTED BY ERECTOR/CONTRACTOR

 IMMEDIATELY UPON ARRIVAL CLAIMS FOR SHORTAGES OR DEFECTIVE MATERIAL IF NOT PACKAGED MUST BE SENT TO THE

 MANUFACTURER IN WRITING WITHIN FIVE (5) DAYS AFTER RECEIPT OF THE SHIPMENT. HOWEVER, IF A DEFECT IS OF SUCH A NATULE

 THAT REASONABLE VISUAL INSPECTION WOULD FAIL TO DISCLOSE IT, THEN THE CLAIM MUST BE MADE WITHIN FIVE (5) DAYS

 AFTER THE ERECTOR/CONTRACTOR LEARNS OF THE DEFECT. THE MANUFACTURER WILL NOT BE LIABLE FOR ANY DEFECT UNLESS

 AFTER THE ERECTOR/CONTRACTOR LEARNS OF THE DEFECT. THE MANUFACTURER WILL NOT BE LIABLE FOR ANY DEFECT UNLESS

 OR INCREMENTATION AT THE JOB SITE WITHOUT

 AT THE JOB SITE WITHOUT C) PRODUCTS SHIPPED TO ERECTOR/CONTRACTOR OR HIS CUSTOMER SHALL BE INSPECTED BY ERECTOR/CONTRACTOR IMMEDIATELY UPON ARRIVAL CLAIMS FOR SHORTAGES OR DEFECTIVE MATERIAL IF NOT PACKAGED MUST BE SENT TO THE
- WILL REIMBURSE THE CONTRACTOR FOR THE COST OF THE REPAIR IN ACCORDANCE WITH THE WRITTEN AUTHORIZATION.

THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE COMPONENTS IN TO LINE, MODERATE AMOUNTS OF REAMING, CHIPPING AND CUTTING, AND THE REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND ARE NOT SUBJECT TO CLAIM.

- H) ALL BRACING AS SHOWN AND PROVIDED BY THE MANUFACTURER FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE.
- TEMPORARY SUPPORTS, SUCH AS TEMPORARY GUYS, BRACES, FALSE WORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION WILL BE DETERMINED AND FURNISHED AND INSTALLED BY THE ERECTOR. THESE TEMPORARY SUPPORTS WILL SECURE THE STEEL FRAMING, OR ANY PARTLY ASSEMBLED STEEL FRAMING, AGAINST LOADS COMPARABLE IN INTENSITY TO THOSE FOR WHICH THE STRUCTURE WAS DESIGNED, RESULTING FROM WIND, SEISMIC FORCES AND EXECTION OPERATIONS, BUT NOT THE LOADS RESULTING FROM THE PERFORMANCE OF WORK BY OR THE ACTS OF OTHERS, NOR SUCH UNPREDICTABLE LOADS AS THOSE DUE TO TORNADO, EXPLOSION OR COLLISION.
- (SECT. 7.10.3 AISC CODE OF STANDARD PRACTICE, LATEST ED.)

 J) METAL BUILDING MANUFACTURER IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND WORKMANSHIP OF FOUNDATION. ANCHOR BOLT PLANS PREPARED BY MBM ARE INTENDED TO SHOW ONLY LOCATION, DIAMETER AND PROJECTION OF THE ANCHOR RODS REQUIRED TO ATTACH THE METAL BUILDING SYSTEM TO FOUNDATION. IT IS RESPONSIBILITY OF THE END CUSTOMER TO ENSURE THAT ADEQUATE PROVISIONS ARE MADE FOR SPECIFYING ROD EMBEDMENT, BEARING VALUES, TIE RODS AND OTHER ASSOCIATED ITEMS EMBEDDED IN THE CONCRETE FOUNDATION, AS WELL AS FOUNDATION DESIGN FOR THE LOADS IMPOSED BY MB SYSTEM, OTHER IMPOSED LOAD, AND THE BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE (MBMA 06 SECTIONS 3.2.2 AND A3)
- K) METAL BUILDING MANUFACTURER DOES NOT PROVIDE ANY FIELD SUPERVISION FOR THE ERECTION, NOR DOES MBM PERFORM ANY INSPECTIONS DURING OR AFTER ERECTION.

Wall Field Values = 22.520 psf / -24.429 psf Wall Edge Values = 22.520 psf / -30.154 psf

FLORIDA PRODUCT APPROVAL NUMBER

PBR ROOF PANEL 36875.1 PBR WALL PANEL 36876.1 3070 WALKDOORS +50/-50 PSF 22211.4

IT IS THE RESPONSIBILITY OF THE CUSTOMER TO PROVIDE ALL DOCUMENTATION REQUIRED FOR ANY ACCESSORIES NOT PROVIDED BY MBM TO THEIR LOCAL PERMITTING OFFICE. ALL ACCESSORIES MUST COMPLY AND MEET ALL DESIGN REQUIREMENTS PER LOCAL CODES.

ALL VEHICULAR FRAMED OPENINGS SUPPLIED ON THIS PROJECT HAVE BEEN DESIGNED TO SUPPORT WIND LOADS NORMAL TO A DOOR SYSTEM, BASED ON THE STANDARD BUILDING CODE CRITER THE VEHICULAR FRAMED OPENING HAS NOT BEEN DESIGNED FOR ANY ADDITIONAL MOMENT OR CATENARY FORCE FROM THE DOOR SYSTEM. ANY CHANGES TO THE INFORMATION SHOWN HERE WOULD REQUIRE AN ENGINEERING INVESTIGATION AND POSSIBLE BUILDING



BUILDING DESIGNED & MANUFACTURED BY AN IAS ACCREDITED FACILITY.

Rigid Frame: RO RO - Red Oxide

TRAMING COLORS

Flange brac Angle:	ec Ro		P – Gr Z – Go	ey Prim Ivanized	er I		
	Grt	Pur	EvSt	Jmb	BB	End Col	wali Raf
U SECTION:	RO	RO	RO	RO	RO	RO	RO
C SECTION:	RO	RO	RO	RO	RO	RO	RO
D SECTION:	RO	RO	RO	RO	RO	RO	RO
Z SECTION:	RO	RO	RO	RO	RO	RO	RO
E SECTION:	RO	RO	RO	RO	RO	RO	RO
R SECTION:	60	RO	RO	RO	RO	RO	RO

WHEN GALVANIZED PROVIDED: ALL FINISHED PRIMARY BUILT-UP AND HOT ROLL MEMBERS ARE HOT DIPPED GALVANIZED. ALL SECONDARY COLD FORMED MEMBERS ARE PRE-GALVANIZED.

copies.

W SECTION: RO RO RO RO RO RO



Pre-Engineered Metal Buildings

REV.	PAGE	DESCRIPTION						
	0	COVER PAGE						
	1	ANCHOR BOLT LAYOUT						
	1.1	ANCHOR BOLT DETAILS						
	1.2	ANCHOR BOLT REACTIONS						
	2	ROOF FRAMING LAYOUT						
	2.1-2.6	RIGID FRAME CROSS SECTION						
	3	SIDEWALL FRAMING LAYOUT						
	4	ENDWALL FRAMING LAYOUT						
	5-5.4	FRAMING DETAILS						
	6	ROOF PANELS & TRIM						
	6.1	ROOF PANEL DETAILS						
	7	SIDEWALL PANELS & TRIM						
	7.1	SIDEWALL PANEL DETAILS						
	8	ENDWALL PANELS & TRIM						
	8.1	ENDWALL PANEL DETAILS						
	9	SPECIAL DETAILS						
	10	WALLS INSULATION LAYOUT						
	11	ROOF INSULATION LAYOUT						
	12	INSULATION DETAILS						

DRAWING INDEX

THIS PROJECT IS DESIGNED AS AN ENCLOSED BUILDING. ACCESSORIES (DOORS, WINDOWS, ETC.) BY OTHERS MUST BE DESIGNED AS "COMPONENTS IND CLADDING" IN ACCORDANCE TO SPECIFIC WIND PROVISIONS OF REFERENCED BUILDING CODE.

FOR OCCUPANCY (RISK) CATEGORY I OR II, IBC PROVISIONS INDICATE THAT SINGLE—STORY BUILDINGS SHALL HAVE "NO DRIFT LIMIT" PROVIDED THAT INTERIOR WALLS, PARTITIONS, CEILINGS AND EXTERIOR WALL SYSTEMS HAVE BEEN DESIGNED TO ACCOMMODATE THE SEISMIC STORY DRIFTS. INTERIOR WALLS, PARTITIONS, CEILINGS OR EXTERIOR SYSTEMS NOT PROVIDED BY MEM SHALL BE DESIGNED AND DETAILED BY OTHERS TO ACCOMODATE THE SEISMIC

0.5 PSF COLL ONLY ALLOW LIGHTING TO HANG FROM ROOF SYSTEMS SUSPENSION OF ANY LOAD INDUCING SYSTEM IS EXPLICITLY PROHIBITED, UNLESS A CORRESPONDING REDUCTION IN CERTIFIED LIVE/SNOW LOADS CAN BE PERMITTED BY CODE.

*ADDITIONAL LOADING & SUPPORT PROVIDED *FOR CUSTOMERS 5 TON MONORAIL SYSTEM.

This item has been digitally signed and sealed by Richard FL-247 FOUNDATION T Smith on the date adjacent to the seal. Printed copies CITY, of this document are not considered signed and sealed 3309 LAKE

			ľ
CC	DLORS:	DRAWING STATUS	ţ.
ROOF:	GALVALUME	EOR APPROVAL: THESE DRAWINGS, BEING FOR APPROVAL, ARE BY DEFINITION NOT FINAL, AND ARE FOR CONCEPTUAL	Ī
WALLS:	COLOR	REPRESENTATION ONLY. THEIR PURPOSE IS TO	h
GABLE:	COLOR	CONFIRM PROPER INTERPRETATION OF THE PROJECT DOCUMENTS. ONLY DRAWINGS ISSUED "FOR CONSTRUCTION" CAN BE CONSIDERED AS COMPLETE.	L
EAVE:	COLOR	FOR PERMIT:	Ī
CORNER	COLOR	THESE DRAWINGS, BEING FOR PERMIT, ARE BY DEFINITION NOT FINAL IN THAT, AS A MINIMUM, PIECE MARKINGS ARE	ŀ
FRAMED OPENINGS:	COLOR	NOT IDENTIFIED, ONLY DRAWINGS ISSUED "FOR CONSTRUCTION" CAN BE CONSIDERED AS COMPLETE.	L
BASE:	COLOR	FOR CONSTRUCTION: THESE DRAWINGS ARE FINAL AND ISSUED FOR FIELD USE FOR BUILDING ERECTION	l

and the signature must be verified on any electronic

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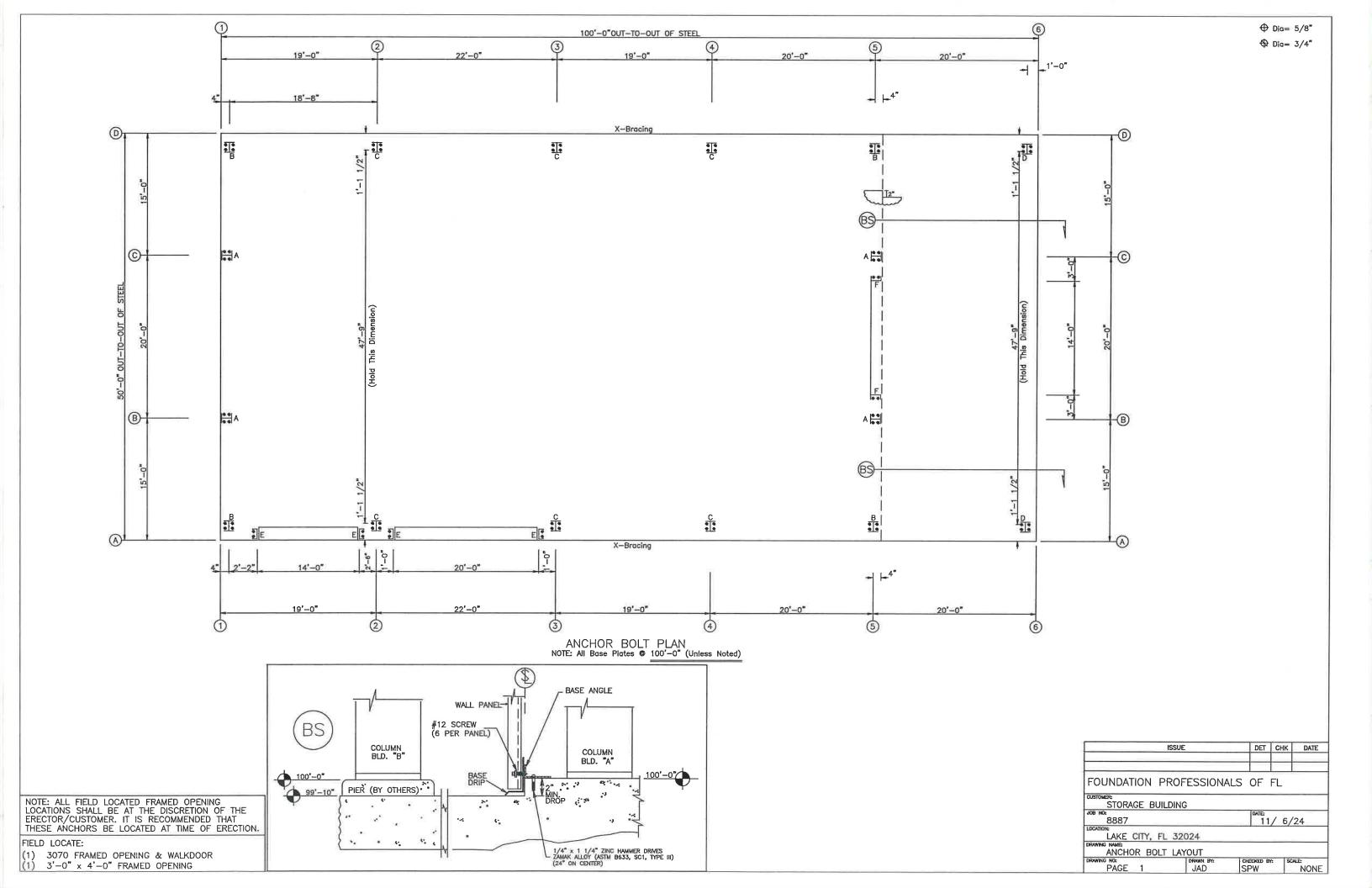
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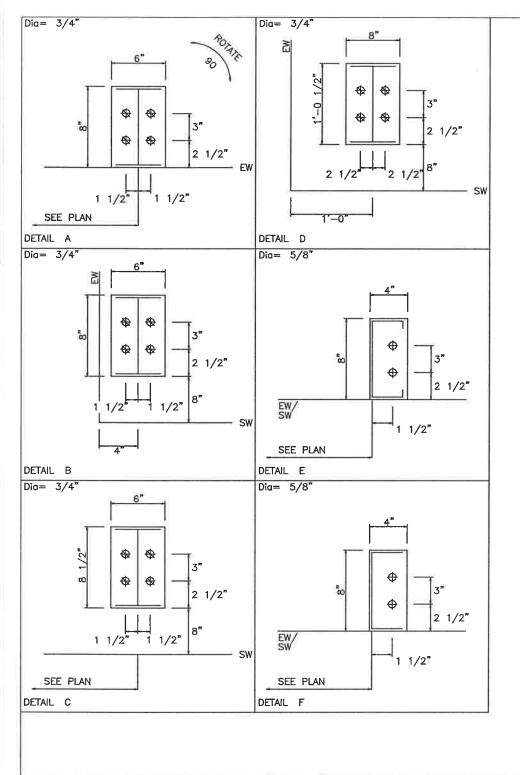
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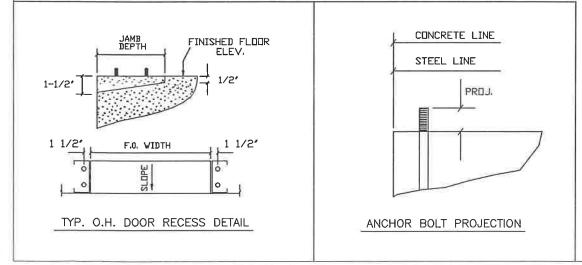
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COMPONENTS & CLADDING (unfactored)





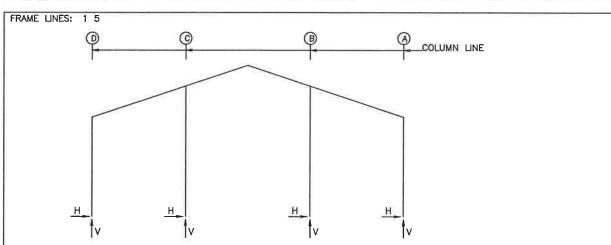


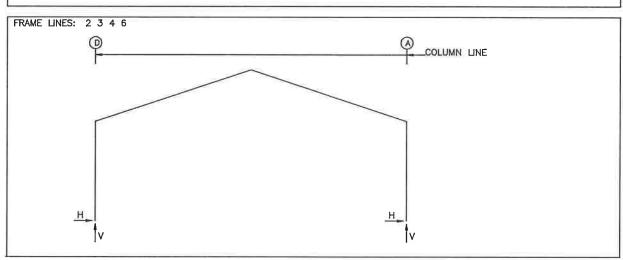
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	Column Line	Horz	-Dead Vert	Colla	CTIONS (teral— Vert		-Live Vert	Wind	Left1-	-Wind_ Horz	Right1- Vert	——Wind Horz	Left2- Vert
	D A C B	0.0 0.0 0.0 0.0	0.2 0.2 1.2 1.2	0.0 0.0 0.0 0.0	0.0 0.2 0.2	0.0 0.0 0.0	0.2 0.2 3.7 3.7	-1.7 -1.8 0.0 0.0	-2.5 0.9 -4.7 -6.3	1.8 1.7 0.0 0.0	-2.5 -6.3 -4.6	-2.1 -1.4 0.0 0.0	-1.8 1.5 -3.1 -4.8
rame ine	Column Line D A C B		Right2- Vert 1.5 -1.8 -4.8 -3.1	Horz 0.0 0.0	_Press- Vert 0.0 0.0 0.0 0.0	——Wind Horz 0.0 0.0 3.4a 3.4a	Vert 0,0 0.0	Horz 1.1 -0.4	Long1- Vert -0.5 -1.7 -4.5 -1.8	Wind Horz 0.4 -1.1 0.0 0.0	l_Long2- Vert -1.7 -0.5 -1.8 -4.5	-Seism Horz -0.1 -0.1 0.0 0.0	ic_Left Vert -0.3 0.3 -0.3
rame ine	Column Line D A C B		:_Right Vert 0.3 -0.3 -0.3 0.3		L_1- Vert -0.2 -0.8 3.8 2.2	0.0 0.0		Horz 0.0		Horz 0.0	U_4- Vert -0.8 -0.8 2.2 2.2	-F6CR4 Horz -1.2 -1.2 0.0 0.0	Vert −6.4 −0.7
ine	Column Line D A C B	Horz 1.2 1.2 0.0 0.0	Vert -0.7 -6.4 7.3 12.3										
rame ine	Column Line D A	Horz 0.6 -0.6	-Dead Vert 1.9 2.0	Colla Horz 0.1 0.1	teral— Vert 0.3 0.3	Horz 2.3 -2.3	-Live Vert 6.7 6.7	Wind Horz -5.6 -0.9	_Left1- Vert -10.7 -7.1	-Wind_ Horz 0.8 5.6	Right1- Vert -7.1 -10.7	Wind Horz -5.7 -0.7	Left2- Vert -6.7 -3.2
rame ine	Column Line D A		Right2- Vert -3.2 -6.8	Wind Horz 0.2 0.9	_Long1 Vert 8.2 7.3	Wind Horz -0.9 -0.2	Long2- Vert -7.3 -8.2	-Seism Horz -0.2 -0.2	ic_Left Vert -0.1 0.1	Seismic Horz 0.2 0.2	Right Vert 0.1 -0.1	-F2CR4 Horz 2.4 -5.1	
rame ine	Column Line D A	-F2CR/ Horz 5.0 -2.4											
rame ine	Column Line D A	Horz 0.6 -0.6	Vert	Colla Horz 0.1 0.1	Vert	Horz 2.3 -2.3	-Live Vert 6.8 6.8	Horz -5.6	_Left1- Vert -10.6 -7.1	Horz 0.9		Wind Horz -5.7 -0.8	
rame ine	Column Line D A		Right2- Vert -3.1 -6.7	——Wind. Horz 0.2 0.9	_Long1 Vert 8.2 7.4	Wind Horz -0.9 -0.2	_Long2- Vert -7.4 -8.2	-Seismi Horz -0.2 -0.2	ic_Left Vert -0.1 0.1	Seismic Horz 0.2 0.2	_Right Vert 0.1 -0.1	-F3CRA Horz 2.4 -5.0	
rame ine	Column Line D A	Horz											
rame ine	Line	Horz	-Dead Vert 1.9 1.9		Vert	Horz 2.2 -2.2	Vert	Horz	Vert	-Wind_ Horz 0.9 5.3	Right1- Vert -6.7 -10.1		
rame ine	Column Line D A	-Wind_ Horz 0.7 5.5	Right2- Vert -2.9 -6.4	——Wind_ Horz 0.2 0.9	_Long1 – Vert 7.8 7.0	Wind Horz -0.9 -0.2	_Long2- Vert -7.0 -7.8	-Seismi Horz -0.2 -0.2	ic_Left Vert -0.1 0.1	Seismic Horz 0.2 0.2	_Right Vert 0.1 _0.1	-F4CRA Horz 2.4 -5.0	NEA1 – Vert 5.2 7.8
rame ine	Column Line D A	-F4CR/ Horz 5.0 -2.4	NEA2- Vert 7.8 5.2										
rame ine	Column Line D A C B	Horz 0.0 0.0 0.0 0.0	-Dead Vert 0.2 0.2 1.7 1.7	Colla Horz 0.0 0.0 0.0 0.0	teral- Vert 0.0 0.0 0.3 0.3	Horz 0.0 0.0 0.0 0.0	-Live Vert 0.1 0.1 6.0 6,0	Wind Horz -2.9 -3.4 0.0 0.0	Left1- Vert -4.2 2.2 -5.5 -11.1	-Wind_ Horz 3.4 2.9 0.0 0.0	Right1- Vert 2.1 -4.2 -11.1 -5.5	Wind Horz -3.7 -2.6 0.0 0.0	_Left2- Vert -3.5 3.9 -2.8 -12.9
rame ine	Column Line D A C B	-Wind_ Horz 2.6 3.7 0.0 0.0	Right2- Vert 3.9 -3.5 -12.9 -2.8	Wind Horz 0.0 0.0 -3.1a -3.1a	Press Vert 0.0 0.0 0.0 0.0	Wind Horz 0.0 0.0 3.4a 3.4a	_Suct Vert 0.0 0.0 0.0 0.0	Wind Horz 1.0 -0.4 0.0 0.0	_Long1- Vert -0.7 -1.7 -8.4 -6.0	Wind Horz 0.4 -1.0 0.0 0.0	_Long2- Vert -1.7 -0.7 -6.0 -8.4	Seism Horz 0.2 0.2 0.0 0.0	c_Left Vert -0.4 0.4 -0.4
rame ine	Column Line D A C B	Seismic Horz 0.2 0.2 0.0 0.0	Right Vert 0.4 -0.4 -0.4 0.4	F5PAT_L Horz 0.0 0.0 0.0 0.0	Vert -0.3 -1.2 6.1 3.6	F5PAT_I Horz 0.0 0.0 0.0 0.0	Vert -1.2 -0.3 3.6 6.1	F5PAT_I Horz 0.1 -0.1 0.0 0.0	L_3- Vert 1.0 1.0 2.5 2.5	F5PAT_I Horz 0.0 0.0 0.0 0.0	Vert -1.2 -1.2 3.6 3.6	-F3CRA Horz -1.3 -1.3 0.0	NEA1 – Vert –6.7 –0.7 13.0 7.7
rame ine	Column Line D A C B	-F3CR/ Horz 1.3 1.3 0.0 0.0	NEA2- Vert -0.8 -6.7 7.7 13.0										

NOTE: THE FRAMING AT BOTH ENDWALLS IS NOT DESIGNED TO ACCOMMODATE FUTURE ADDITIONS. REACTIONS CORRESPONDING TO THESE FRAME LINES REFLECT LOADINGS FOR ACTUAL TRIBUTARY AREA AND ARE NOT INTENDED TO INCLUDE ANY FUTURE MODIFICATIONS UNLESS NOTED OTHERWISE.

Frame Line 6 6	Column Line D A	Horz 0.4 -0.4	-Dead Vert 1.5 1.5	Colla Horz 0.1 -0.1	teral- Vert 0.2 0.2	Horz 1.6 –1.6	Live Vert 4.2 4.2	Wind Horz -2.1 2.3	_Left1- Vert 6.3 6.2	-Wind_ Horz -2.3 2.1	Right1- Vert -6.2 -6.3	Wind Horz -3.2 1.5	Left2- Vert 5.9 7.2	
Frame Line 6 6	Column Line D A	-Wind_ Horz -1.5 3.2	Right2- Vert -7.2 -5.9	Wind. Horz -2.3 2.3	_Long1- Vert -6.6 -6.6	Wind Horz 1.0 -1.0	_Long2- Vert 2.8 2.8	-Seism Horz -0.1 -0.1	ic_Left Vert -0.1 0.1	Seismic Horz 0.1 0.1	_Right Vert 0.1 -0.1	-F6CRA Horz 2.2 -4.7	NEA1 – Vert 5.0 7.5	
Frame Line 6 6	Column Line D A	-F6CR4 Horz 4.7 -2.2	NEA2- Vert 7.5 5.0											
a - 0	ut-Of-Pl	ane Hori	izontal Lo	ad										





[DIO!								T							
RIGII	RIGID FRAME: ANCHOR BOLTS & BASE PLATE								D FF	RAME	: AN	ICHOR I	BOLTS &	BASE	PLATES
	Frm Col AncBolt Base_Plate (in) Grout Line Line Qty Dia Width Length Thick (in)							Frm Line	Col Line	Anc. Qty	_Boit Dia	Base. Width	_Plate (i Length	n) Thick	Grout (in)
1 1 1	D A C B		0.750	6.000	8.000	0.375 0.375 0.375	0.0 0.0 0.0	4 4	D A		0.750 0.750	6.000 6.000		0.375 0.375	0.0
1	В	4	0.750	6.000	8.000	0.375	0.0	RIGI	D FF	RAME	: AN	ICHOR I	BOLTS &	BASE	PI ATES

					1 (101)	J 111	,			30L13 Q.	DAGE	LAILO
RIGID FRAME	ANCHOR	BOLTS &	BASE F	PLATES	Frm Line	Col Line	Anc. Qtv	_Bolt		_Plate (i Lenath		Grout (in)
	_Bolt Base Dia Width	_Plate (ir Length	n) Thick	Grout (in)	5	D	4	0.750	6.000	8.000	0.375	0.0
2 D 4 2 A 4	0.750 6.000 0.750 6.000		0.375 0.375	0.0	555	A C B	4 4 4	0.750 0.750 0.750	6.000 6.000	8.000	0.375 0.375 0.375	0.0 0.0 0.0

RIGI	D FR	AME	: AN	CHOR E	BOLTS &	BASE	PLATES	RIGI	D FR	AME	: AN	CHOR I	BOLTS &	BASE	PLATES
Frm Line	Col Line	Anc. Qty	_Bolt Dia	Base_ Width	_Plate (i Length	n) Thick	Grout (in)	Frm Line	Col Line	Anc. Qty	_Bolt Dia	Base_ Width	_Plate(Length	in) Thick	Grout (in)
3	D A		0.750 0.750			0.375 0.375	0.0	6	D A		0.750 0.750		12.50 12.50	0.375 0.375	0.0

NOTES FOR REACTIONS Building reactions are based on the following building data: Width (ft) = 50.0 Length (ft) = 100.0 Eave Height (ft) = 16.0 / 16.0 Roof Slope (rise/12) = 4.00 / 4.00 Roof Dead Load (psf) = 2.0 Wall Dead Load Left Endwall (psf) = 2.0 Front Sidewall (psf) = 2.0 Back Sidewall (psf) = 2.0 Roof Live Load (psf) = 2.0 Frame Live Load (psf) = 20.0 Frame Live Load (psf) = 20.0 Frame Live Load (psf) = 12.5 Collateral Load (psf) = 0.5 Wind Speed (mph) = 119.0 Wind Code = FBC 23 (8th Edition) Exposure = B Closure = -0.18, +0.18 Risk Category = B Importance - Wind = 1.00 Importance - Seismic = 1.00 Seismic Design Category = B Seismic Coeff (Sms) = 0.16

L_EW 1 F_SW A 3,4 4.2 3.2 0.5 0.4 R_EW 5 B_SW D 4,3 4.2 3.2 0.5 0.4

(h)Rigid frame at endwall

Reactions for seismic represent shear force, Eh Reaction values shown are unfactored

GENERAL NOTES

- FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF METAL BUILDING MANUFACTURER.
- 2. ALL REACTIONS ARE UNFACTORED.
- 3 ULTIMATE WIND LOADS ARE USED TO DERIVE THE WIND REACTION.

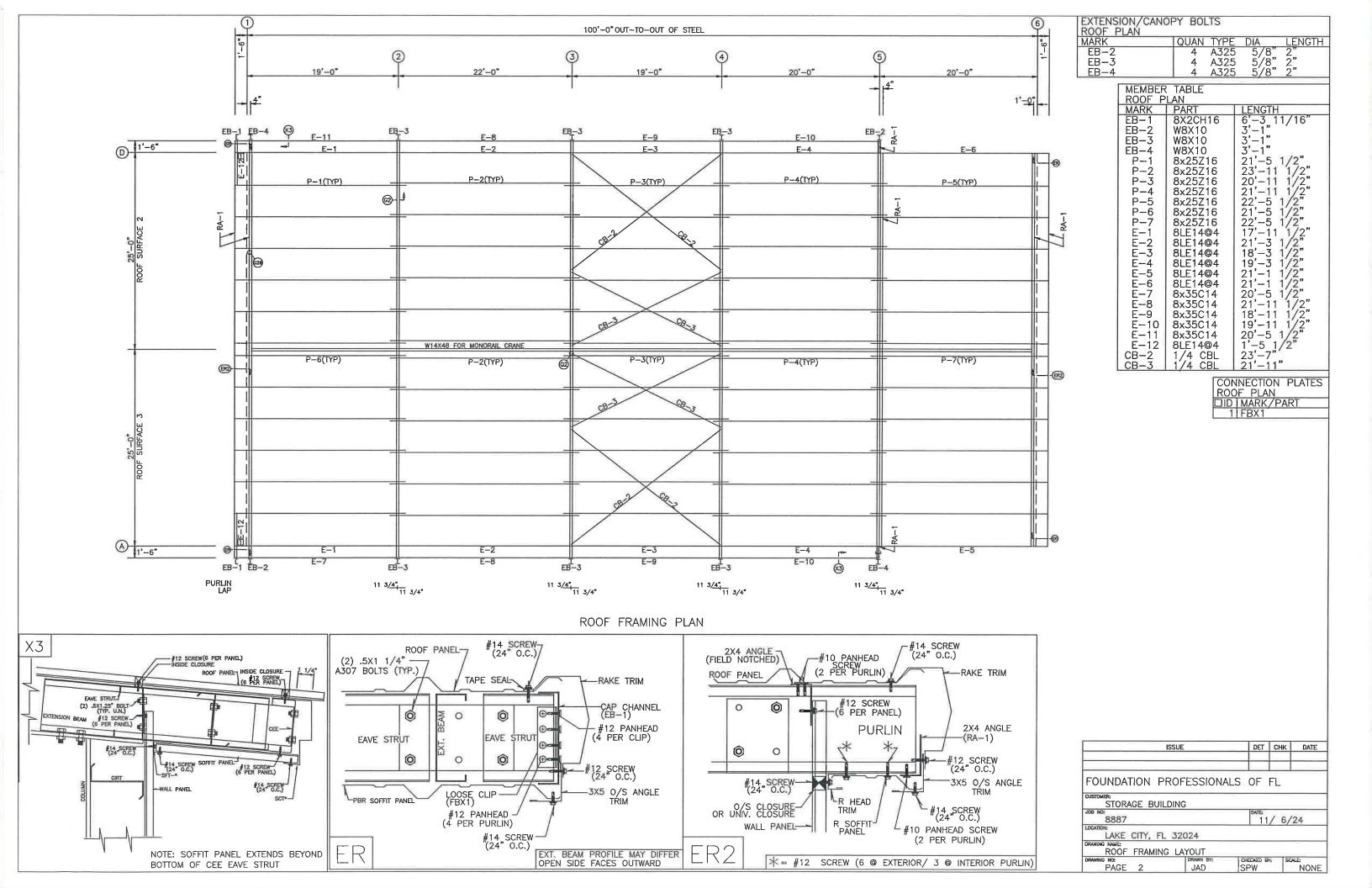
(h)

(h)

- 4. ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLLERANCE OF +/- 1/8" IN BOTH ELEVATION AND LOCATION.
- 5. COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1050 POUNDS PER SQUARE INCH.

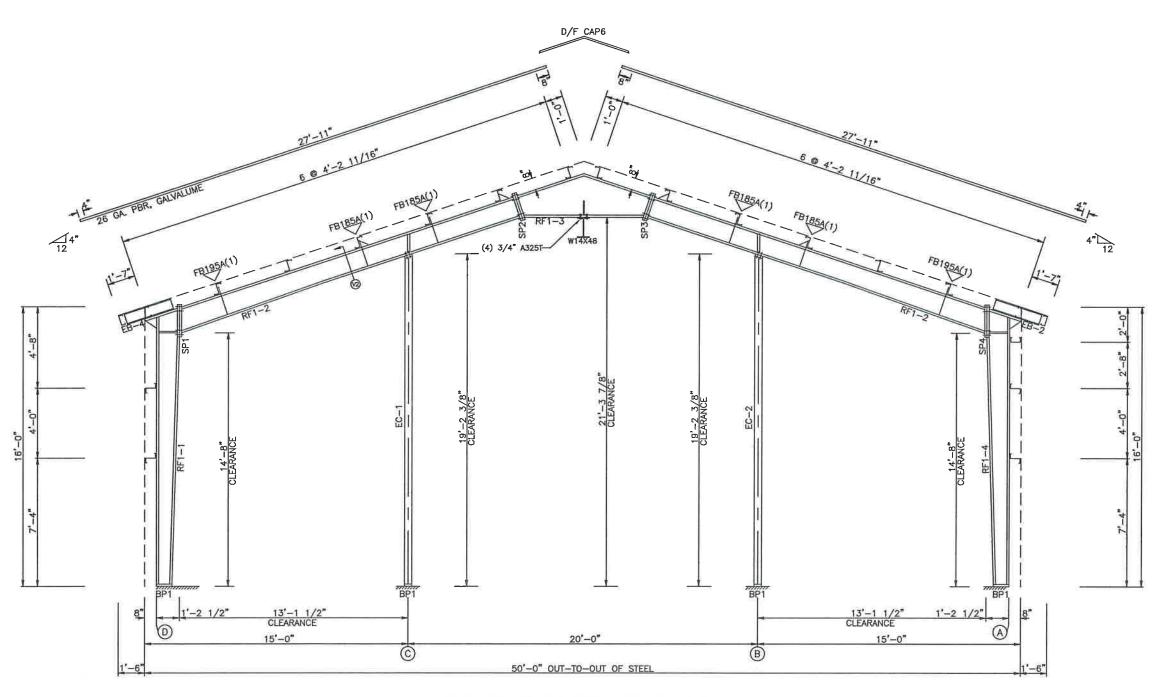
*ADDITIONAL LOADING & SUPPORT PROVIDED *FOR CUSTOMERS 5 TON MONORAIL SYSTEM.

ISSL	JE	DE	т сн	K DATE
FOUNDATION PRO	FESSIONA	LS OF	FL	
CUSTOMER: STORAGE BUILDIN				
JOB NO: 8887		DATE	11/ 6	5/24
LAKE CITY, FL 3:	2024			
DRAWING NAME: ANCHOR BOLT RI	EACTIONS			
DRAWING NO: PAGE 1.2	JAD	SPW	BY:	SCALE: NONE



FLANGE BRACES: (1) One Side; (2) Two Sides FBxxA(1): xx=length(in) A - L2x2x14

MARK	Weight	Web Depth		PLATE	Outside Flange	Inside_Flange
IAIL ZI ZI Z	-	Start/End	THICK	Length	W x Thk x Length	W x Thk x Length
RF1-1	271	7.5/ 8.4	0.135	2"-0"	6 x 1/4" x 15'-5 5/8"	6 x 1/4" x 14'-3 3/16"
		8.4/14.0	0.135	13'-10 3/8"	6 x 1/4" x 1'-11 3/8"	
RF1-2	371	14.0/12.0	0.135	8'-7 3/16"	6 x 1/4" x 18'-1 15/16"	6 x 1/4" x 8'-7 3/16"
		12.0/12.0	0.135	11'-11 1/2"	6 x 1/4" x 2'-0"	6 x 1/4" x 11'-11 1/2"
RF1-3	238	12.2/27.0	0.188	7'-8 15/16"	6 x 5/16" x 4'-1"	6 x 3/8" x 7'-1"
		,		,	6 x 5/16" x 4'-1"	
RF1-4	273	14.0/ 8.4	0.135	13'-10 3/8"	6 x 1/4" x 1'-11 3/8"	6 x 1/4" x 14"-3 3/16"
		8.4/ 7.5	0.135	2'-0"	6 x 1/4" x 15'-5 5/8"	
EC-1	210	W8X10	100		., 10 0 0,0	
EC-2	210	W8X10	T 8			1
EB-2	33	W8X10				
EB-4	33	WBX10				1



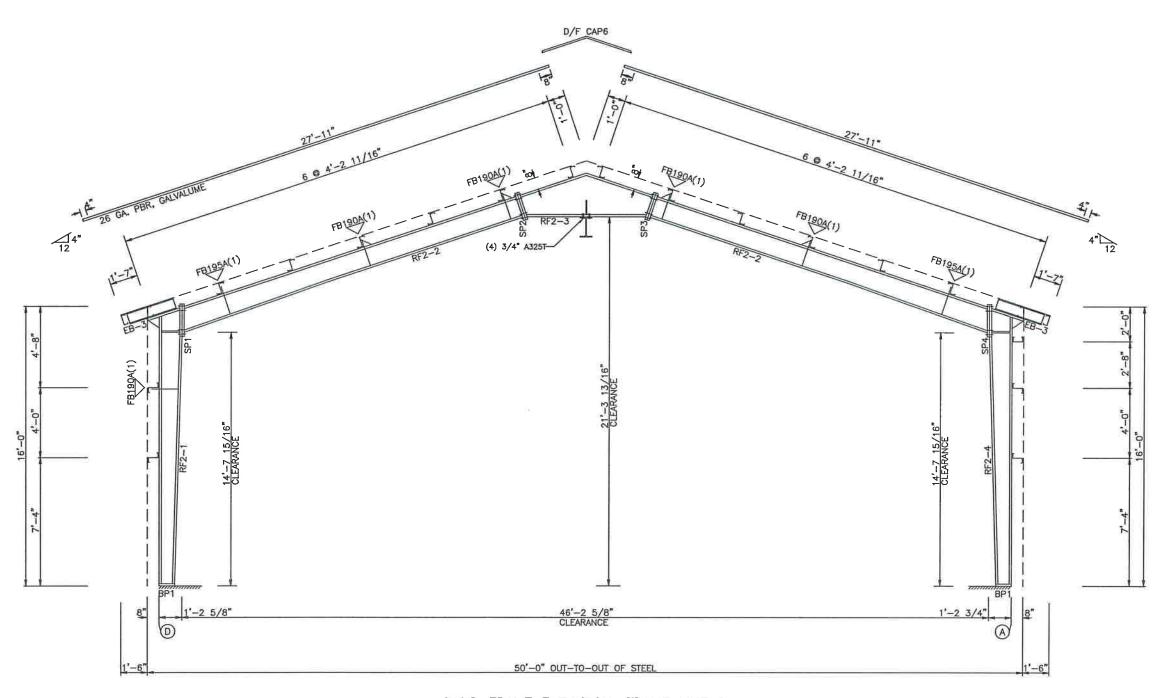
RIGID FRAME ELEVATION: FRAME LINE 1

ISSU	E	DET	CHK	DATE
UNDATION PRO	FESSIONA	ALS OF I	FL	
STORAGE BUILDIN	G			
8887		DATE 11	/ 6/	24
LAKE CITY, FL 32	2024			
RIGID FRAME CRO	SS SECTIO	N		
NG NO: PAGE 2.1	JAD	SPW SPW	: SC/	NONE

NOTE: THE FRAMING AS DEPICTED ABOVE IS NOT DESIGNED TO ACCOMMODATE ANY FUTURE EXPANSION.

FLANGE BRACES: (1) One Side; (2) Two Sides FBxxA(1): xx=length(in) A - L2x2x14

	We		Web	PLATE	Outside Flange	Inside Flange		
MARK	Weight	Start/End		Length	W x Thk x Length	W x Thk x Lenath		
RF2-1	355	7.5/14.0	0.188	14'-3 1/16"	6 x 1/4" x 15"-5 1/2"	6 x 3/8" x 14"-3 3/16"		
		14.0/14.0	0.250	1'-7 3/16"	6 x 5/16" x 1'-11 3/8"			
RF2-2	439	14.0/12.0	0.188	8'-6 11/16"	6 x 1/4" x 7'-7 15/16"	6 x 3/8" x 8'-6 11/16"		
		12.0/12.0	0.135	11'-11 3/8"	6 x 5/16" x 12'-5 3/8"	6 x 1/4" x 11'-11 3/8"		
RF2-3	247	12.2/27.0	0.188	7'-8 5/8"	6 x 5/16" x 4'-0 13/16"	6 x 3/8" x 7'-0 11/16"		
	1		1		6 x 5/16" x 4"-0 13/16"			
RF2-4	392	14.0/14.0	0.250	1'-7 3/16"	6 x 5/16" x 1"-11 3/8"	6 x 1/2" x 14'-3 3/16"		
		14.0/ 7.5	0.188	14'-3 1/16"	6 x 1/4" x 15'-5 1/2"			
CO 7	9-	11101111						



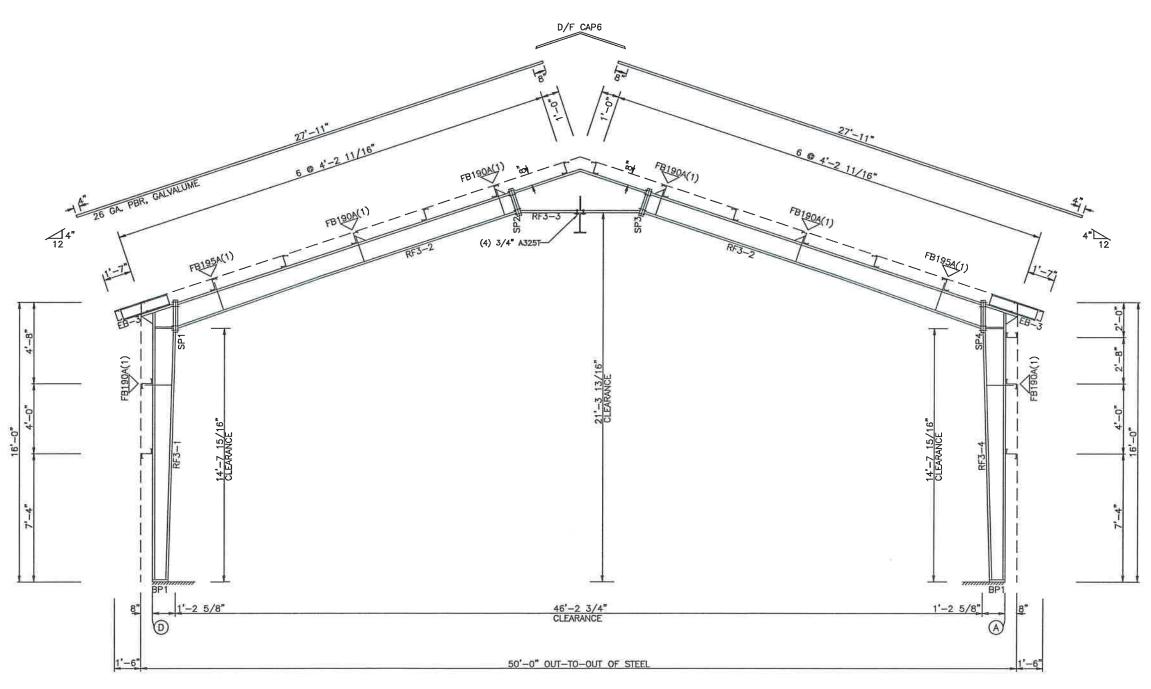
RIGID FRAME ELEVATION: FRAME LINE 2

IS	SUE	DET	СНК	DATE	
FOUNDATION PR	OFESSIONA	LS OF	FL		
STORAGE BUILD	ING				
JOB NO:		DATE			
8887		11/ 6/24			
LAKE CITY, FL DRAWING NAME: RIGID FRAME CI		N			
DRAWING NO:	DRAWN BY:	CHECKED BY	r: SC	ALE:	
PAGE 2.2	JAD	ISPW		NONE	

MARK	Qty Top	Bot	Int	TYPE	DIA	Le	ngth
SP4	4	4	0	A325	3/4"	2	1/2"

FLANGE BRACES: (1) One Side; (2) Two Sides FBxxA(1): xx=length(in) A - L2x2x14

MADIC W		Web Depth	Web PLATE		Outside Flange	Inside Flange	
MARK	Weight	Start/End	THICK	Length	W x Thk x Length	W x Thk x Length	
RF3-1	355	7.5/14.0	0.188	14'-3 1/16"	6 x 1/4" x 15'-5 1/2"	6 x 3/8" x 14'-3 3/16"	
	1	14.0/14.0	0.250	1'-7 3/16"	6 x 5/16" x 1'-11 3/8"		
RF3-2	439	14.0/12.0	0.188	8'-6 11/16"	6 x 1/4" x 7'-7 15/16"	6 x 3/8" x 6'-6 11/16"	
		12.0/12.0	0.135	11'-11 3/8"	6 x 5/16" x 12'-5 3/8"	6 x 1/4" x 11'-11 3/8"	
RF3-3	247	12.2/27.0	0.188	7'-8 5/8"	6 x 5/16" x 4'-0 13/16"	6 x 3/8" x 7'-0 11/16"	
				,	6 x 5/16" x 4'-0 13/16"		
RF3-4	357	14.0/14.0	0.250	1'-7 3/16"	6 x 5/16" x 1'-11 3/8"	6 x 3/8" x 14'-3 3/16"	
		14.0/ 7.5	0.188	14'-3 1/16"	6 x 1/4" x 15'-5 1/2"		
	I			,			



RIGID FRAME ELEVATION: FRAME LINE 3

	SSUE	DET	CHK	DATE
FOUNDATION PE	ROFESSIONA	LS OF	FL	
CUSTOMER STORAGE BUILL	DING			
лов но: 8887		DATE:	/ 6/	24
LAKE CITY, FL	32024			
RIGID FRAME (ROSS SECTIO	N		
DRAWING NO: PAGE 2.3	JAD	SPW	r: SC	NONE

BASE PLATE TABLE

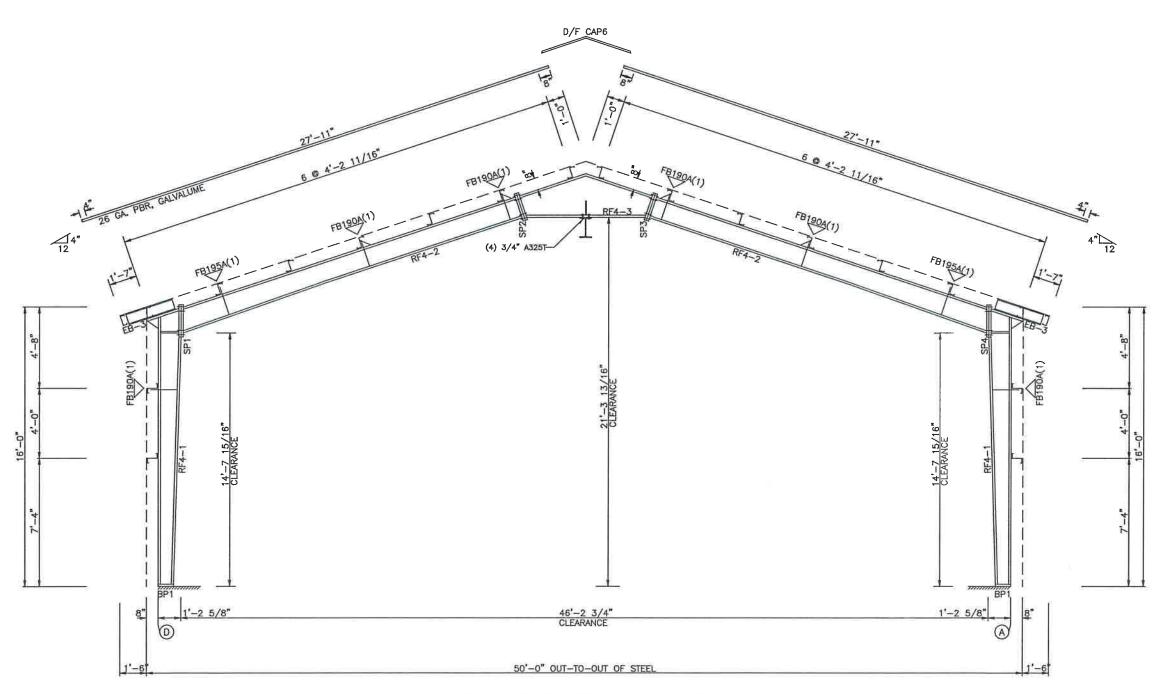
COL PLATE SIZE

MARK Width THICK Length

BP1 6" 3/8" 8 1/2"

FLANGE BRACES: (1) One Side; (2) Two Sides FBxxA(1): xx=length(in) A - L2x2x14

MEMBER	TABLE					HALL AND THE COLUMN
MARK	Weight	Web Depth Start/End	Web	PLATE Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF4-1	355	7.5/14.0	0.188	14'-3 1/16"	6 x 1/4" x 15'-5 1/2"	6 x 3/8" x 14"-3 3/16"
RF4-2	439	14.0/14.0 14.0/12.0	0.250 0.188	1'-7 3/16" 8'-6 11/16"	6 x 5/16" x 1'-11 3/8" 6 x 1/4" x 7'-7 15/16"	6 x 3/8" x 8'-6 11/16"
RF4-3	247	12.0/12.0	0.135 0.188	11'-11 3/8" 7'-8 5/8"	6 x 5/16" x 12'-5 3/8" 6 x 5/16" x 4'-0 13/16"	6 x 1/4" x 11'-11 3/8" 6 x 3/8" x 7'-0 11/16"
FD 7	7.5	WEY10		N 3400	6 x 5/16" x 4'-0 13/16"	



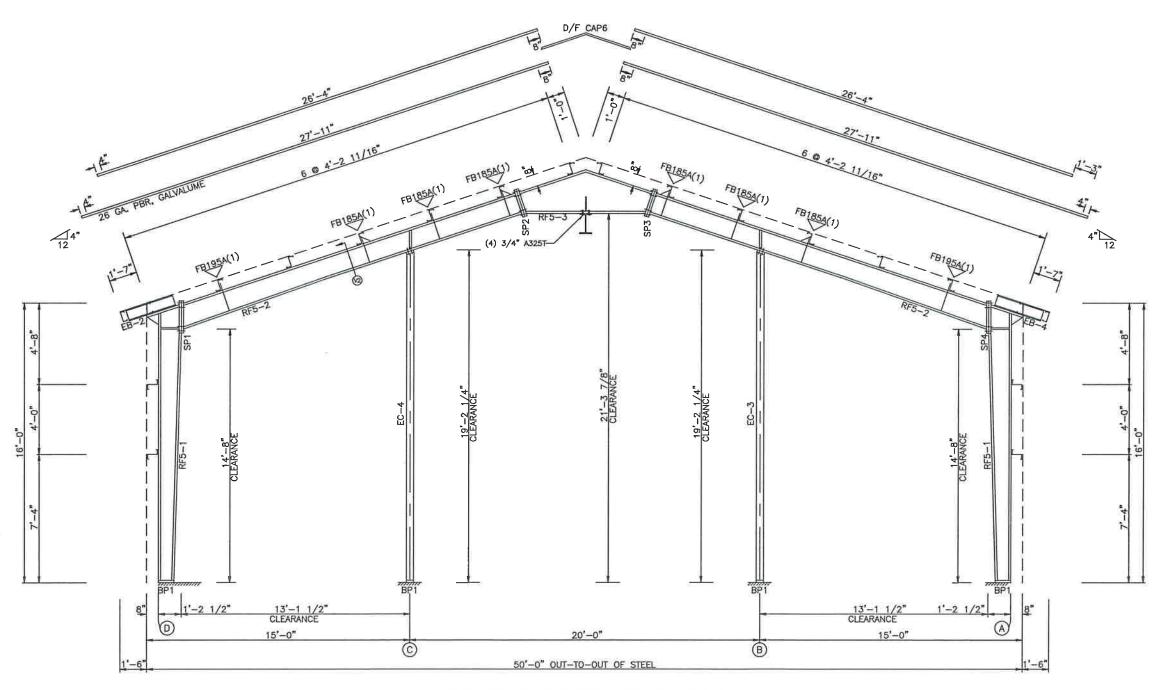
RIGID FRAME ELEVATION: FRAME LINE 4

	ISSUE	DET	СНК	DATE
FOUNDATION F	PROFESSIONALS	S OF	FL	
STORAGE BUI	LDING			
JOB NO: 8887		DATE:	/ 6,	/24
LAKE CITY, F	L 32024			
RIGID FRAME	CROSS SECTION			
PAGE 2.4	DRAWN BT: JAD	SPW	r: 5	NONE

SPLICE BOLT TABLE						CAP PLA	TE BO	LTS			
MARK	Qty Top	Bot	Int	TYPE	DIA	Length	MARK	Qty	TYPE	DIA	Length
SP4	4	4	0	A325	5/8"	2"	EC-4	4	A325T	5/8"	2"
BASE PL	ATE T	ABLE				1	EC-3	4	A325T	5/8"	2"

FLANGE BRACES: (1) One Side; (2) Two Sides FBxxA(1): xx=length(in) A - L2x2x14

MEMBER	TABLE			All the Control of th	- Contract Her	
MARK	Weight	Web Depth Start/End	Web	PLATE Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Lenath
RF5-1	271	7.5/ 8.4 8.4/14.0	0.135 0.135	2'-0" 13'-10 3/8"	6 x 1/4" x 15"-5 5/8" 6 x 1/4" x 1"-11 3/8"	6 x 1/4" x 14"-3 3/16"
RF5-2	375	14.0/12.0 12.0/12.0	0.135 0.135	8'-7 3/16" 11'-11 1/2"	6 x 1/4" x 18'-1 15/16" 6 x 1/4" x 2'-0"	6 x 1/4" x 8'-7 3/16" 6 x 1/4" x 4'-10 3/4" 6 x 1/4" x 6'-4 3/4"
RF5-3	238	12.2/27.0	0.188	7"-8 15/16"	6 x 5/16" x 4"-1" 6 x 5/16" x 4'-1"	6 x 3/8" x 7'-1"
EC-4 EC-3	211 211	W8X10 W8X10				
EB-4	33	W8X10				1

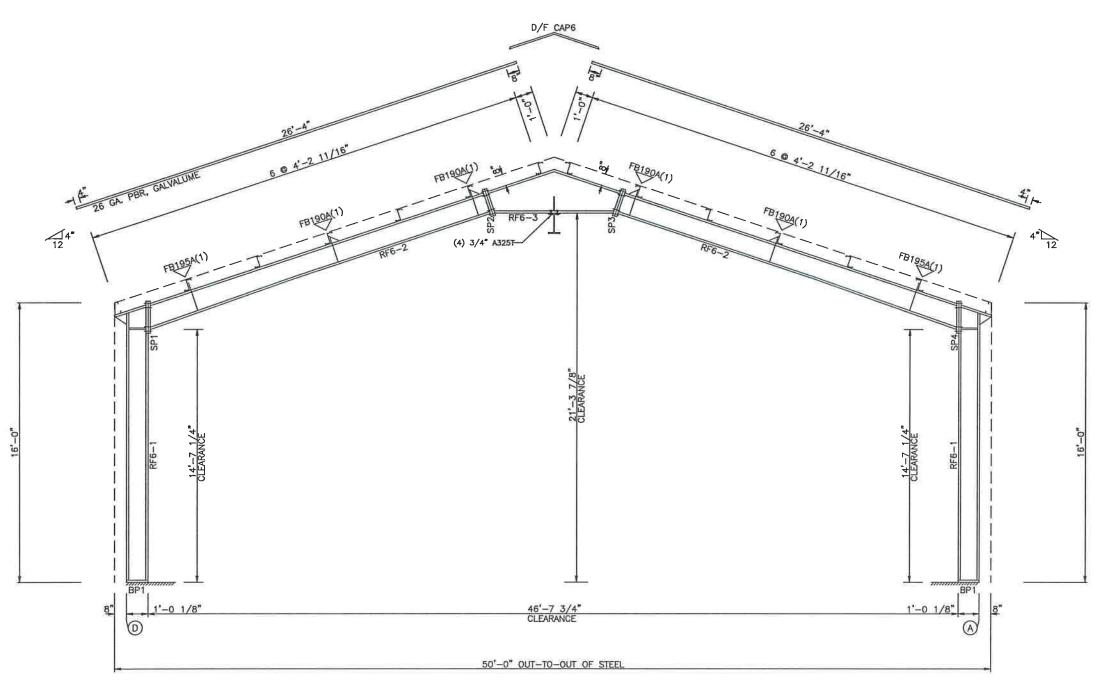


RIGID FRAME ELEVATION: FRAME LINE 5

ISSU	IE	DET	CHK	DATE	
FOUNDATION PRO	FESSIONA	LS OF	FL		
CUSTOMER: STORAGE BUILDIN	IG				
00 NO: 8887		DATE:	11/ 6/24		
LAKE CITY, FL 32	2024				
RIGID FRAME CRO	OSS SECTIO	N			
DRAWING NO: PAGE 2.5	JAD	SPW	r: S	NONE	

FLANGE BRACES: (1) One Side; (2) Two Sides FBxxA(1): xx=length(in) A - L2x2x14

MADIC	W	Web Depth	Web	PLATE	Outside Flange	Inside Flange
MARK W	Weight	Start/End	THICK	Length	W x Thk x Length	W x Thk x Length
RF6-1	441	11.5/11.5 11.5/11.5	0.188	14'-2 5/16" 1'-7 3/16"	8 x 1/4" x 15'-5 5/8" 8 x 1/4" x 1'-8 3/4"	8 x 3/8" x 14'-2 5/16"
RF6-2	459	14.0/12.0	0.18B 0.135	8'-9" ¹ 11'-11 3/8"	6 x 1/4" x 7'-10 1/4" 6 x 5/16" x 12"-5 3/8"	6 x 3/8" x 8'-9 1/16" 6 x 1/4" x 11'-11 3/8"
RF6-3	247	12.2/27.0	0.188	7'-8 11/16"	6 x 5/16" x 4'-0 7/8" 6 x 5/16" x 4'-0 7/8"	6 x 3/8" x 7'-0 3/4"



RIGID FRAME ELEVATION: FRAME LINE 6

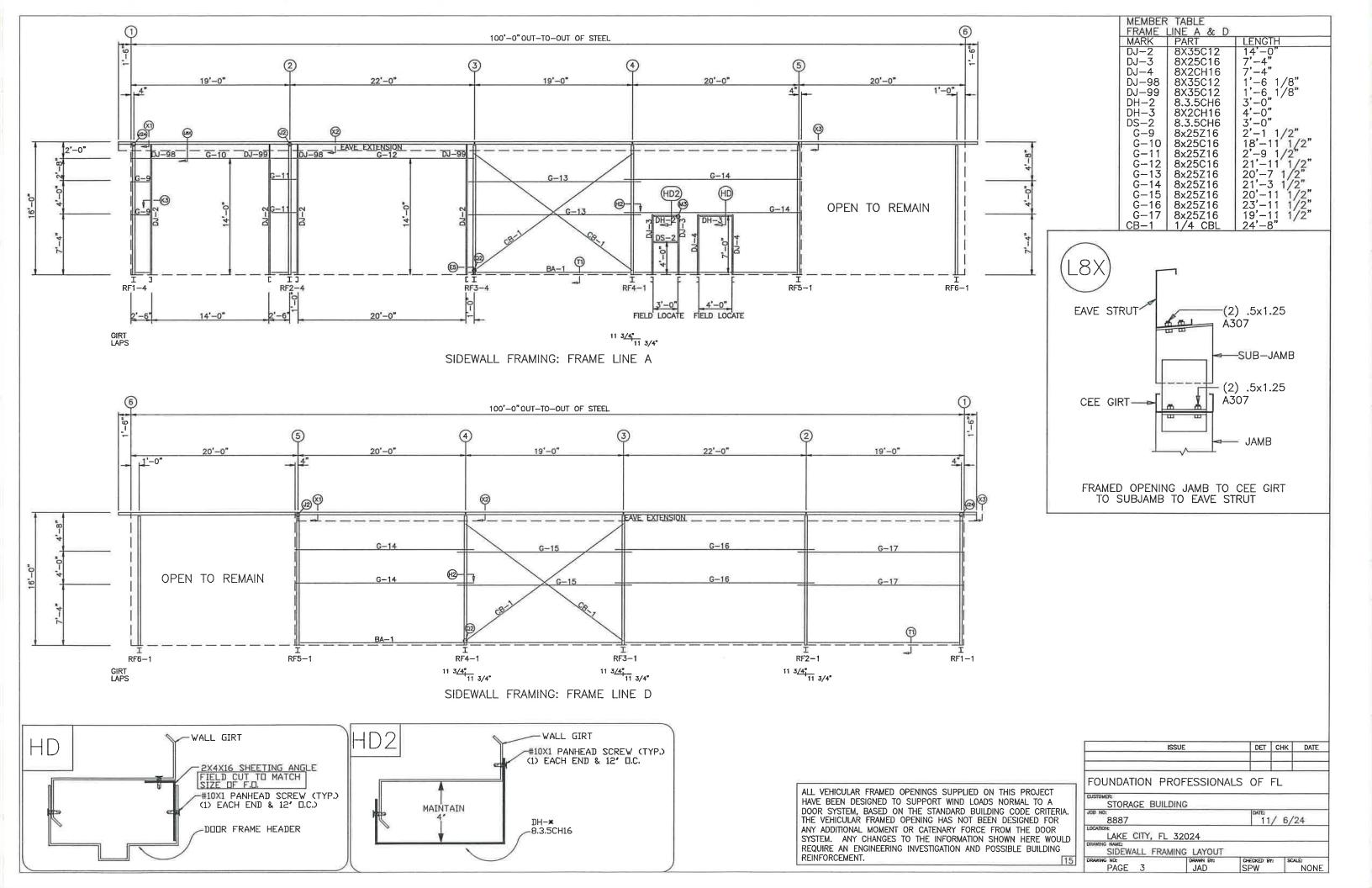
FOUNDATION PROFESSIONALS OF FL

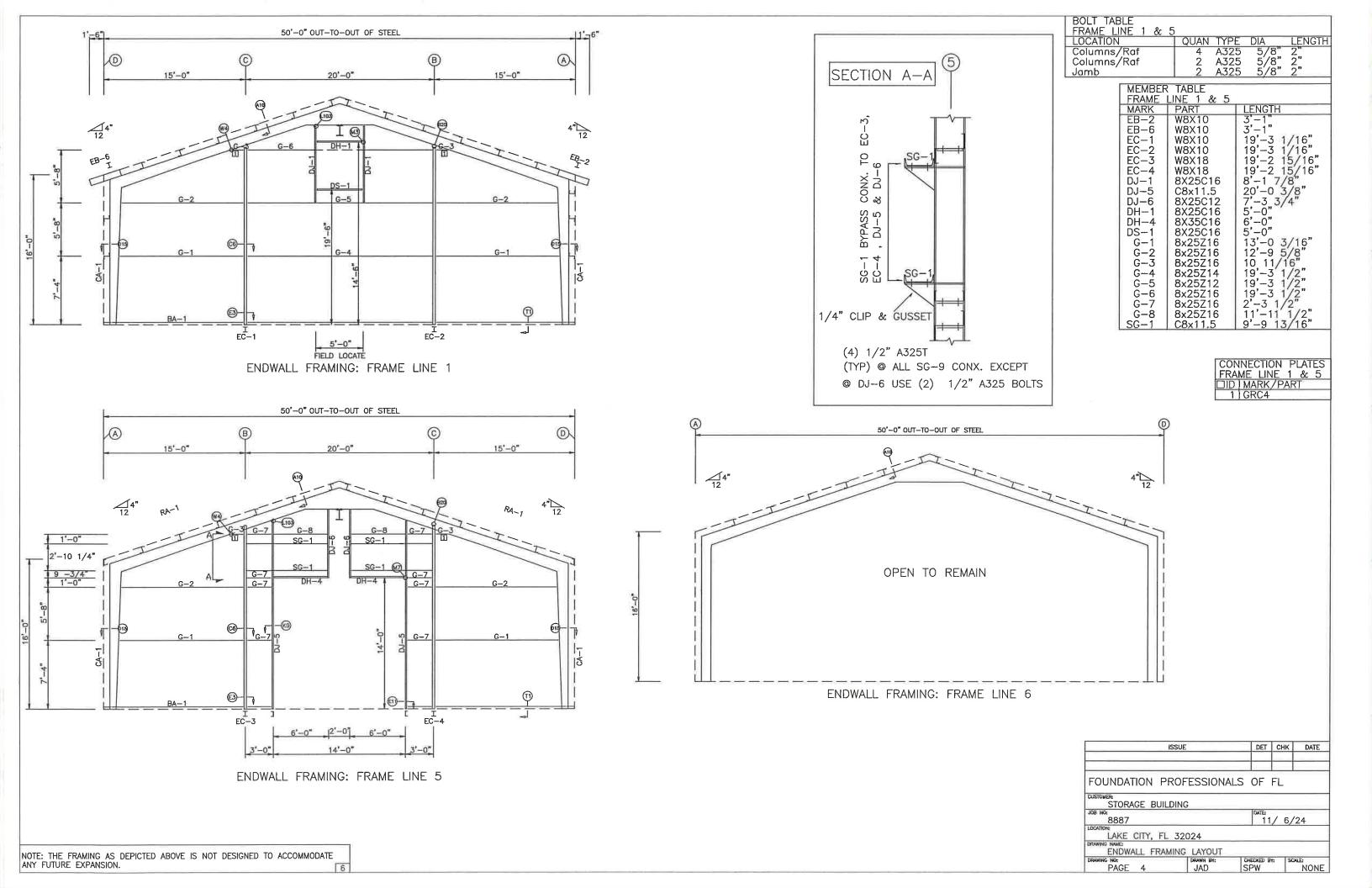
CUSTOMER:
STORAGE BUILDING
JOB NO:
B887
LOCATION:
LAKE CITY, FL 32024
DRAWING NAME:
RIGID FRAME CROSS SECTION
DRAWING NO:
PAGE 2.6 DRAWIN BT: CHECKED BT: SCALE:
NONE

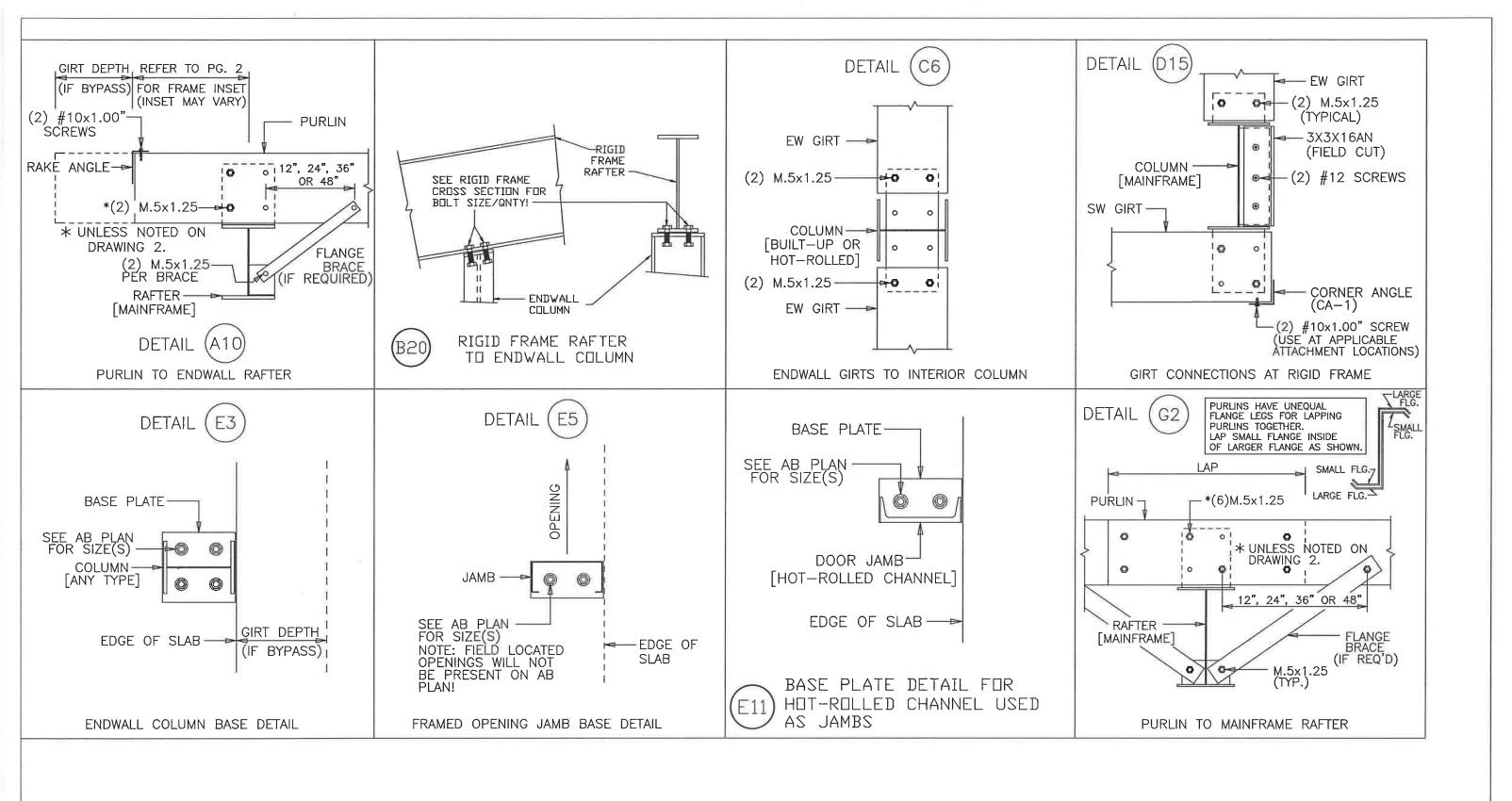
ISSUE

DET CHK DATE

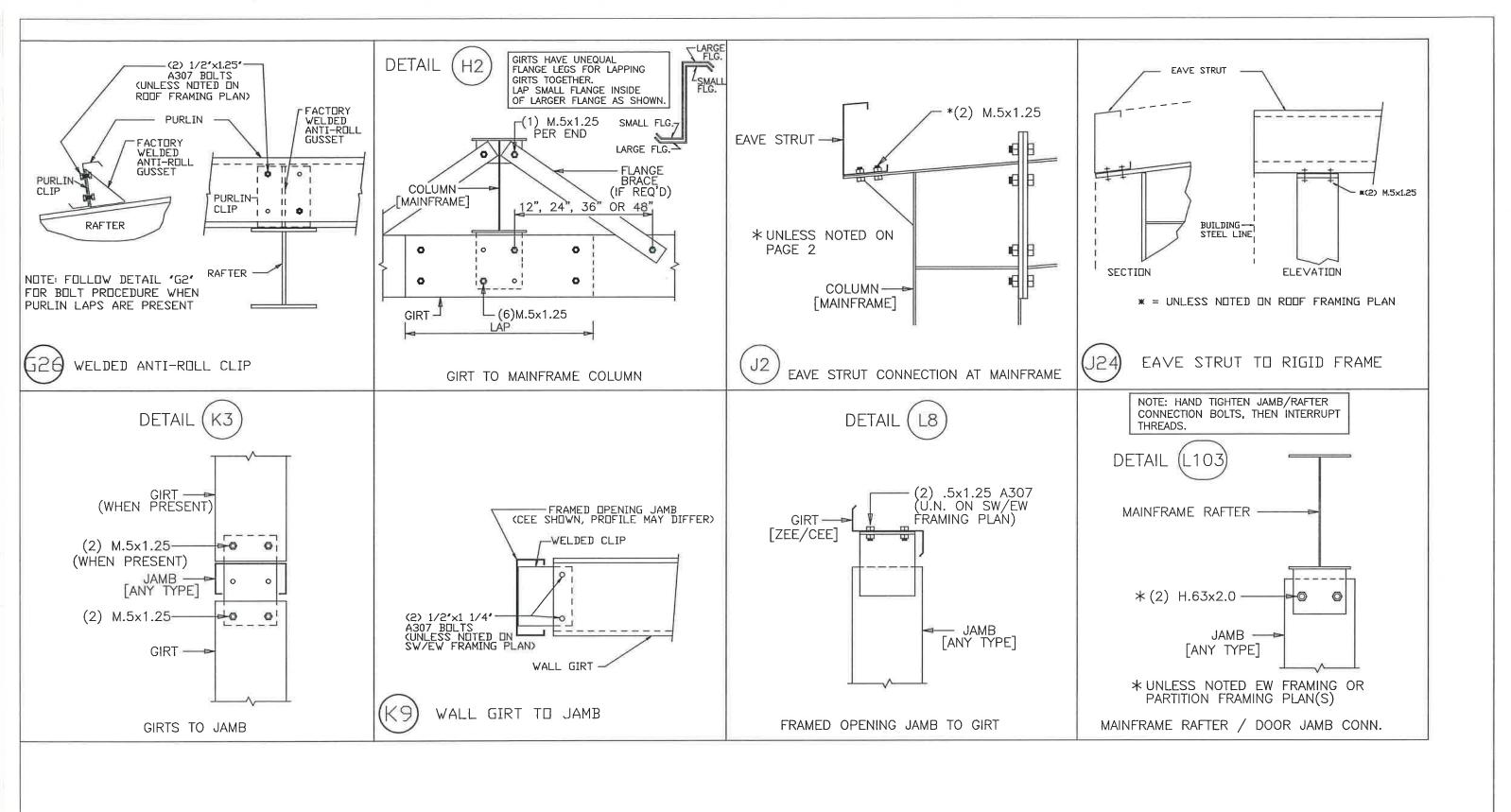
NOTE: THE FRAMING AS DEPICTED ABOVE IS NOT DESIGNED TO ACCOMMODATE ANY FUTURE EXPANSION.



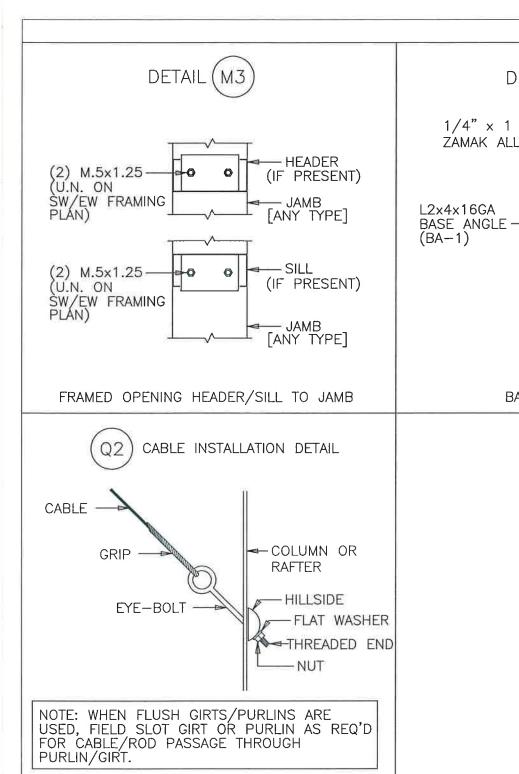




ù l	DET	СНК	DATE				
FOUNDATION PI	ROFESSIONA	ALS OF	FL				
STORAGE BUIL	DING	DATE					
8887			11/ 6/24				
LAKE CITY, FL	32024						
FRAMING DETA	ILS						
DRAWING NO: PAGE 5	JAD	SPW	r: SC	NONE			



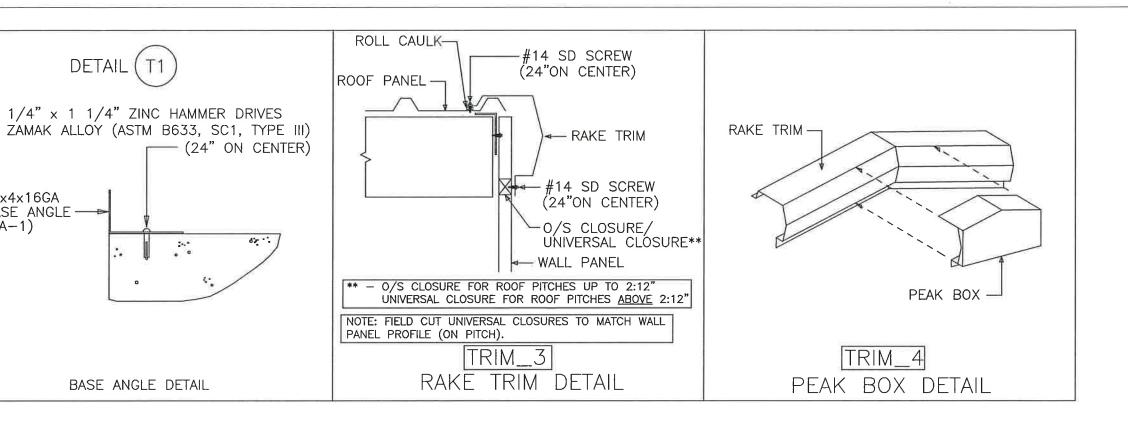
ISSUE		DET	СНК	DATE
FOUNDATION PROF	ESSIONALS	S OF	FL	
STORAGE BUILDING	;			
8887		DATE:	/ 6/	24
LAKE CITY, FL 320	024			
FRAMING DETAILS				
PAGE 5.1	JAD	SPW	r: 50/	NONE

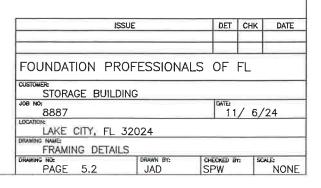


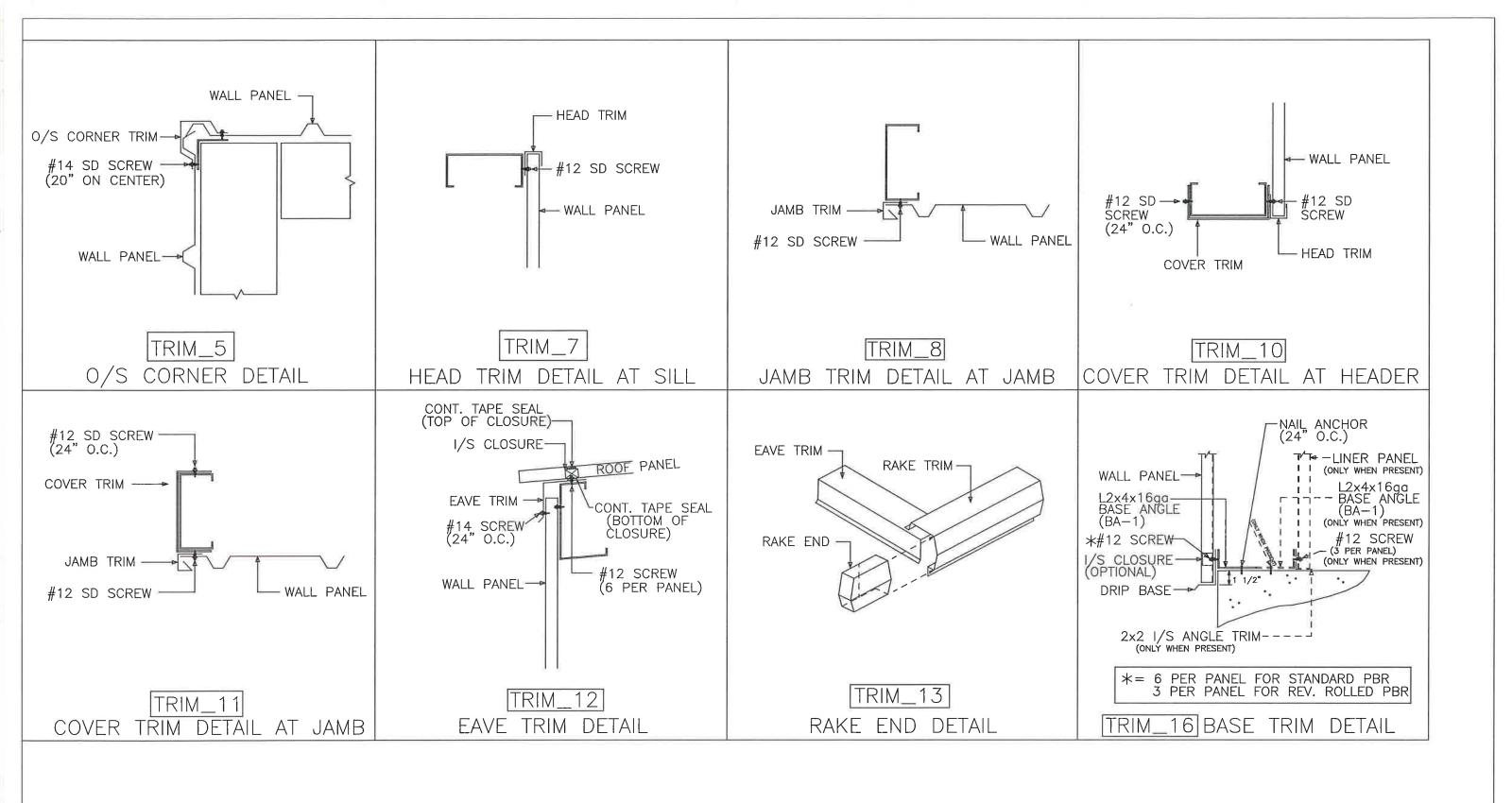
DETAIL (T1

BASE ANGLE DETAIL

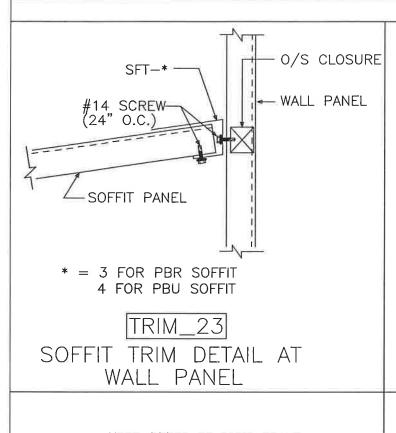
d.

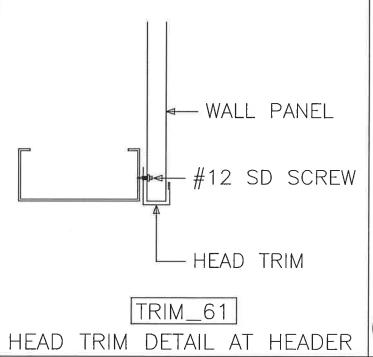


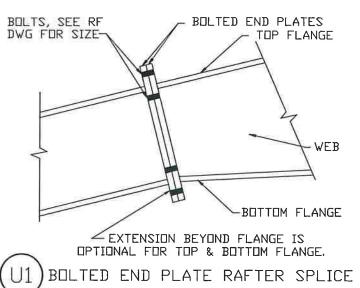




ls .	DET	CHK	DATE				
FOUNDATION PR	ROFESSIONA	ALS OF I	L FL				
STORAGE BUILD	DING						
JOB NO: 8887		DATE:	11/ 6/24				
LAKE CITY, FL	32024						
DRAWING NAME: FRAMING DETAIL	LS						
PAGE 5.3	JAD	SPW	: 50	NONE			







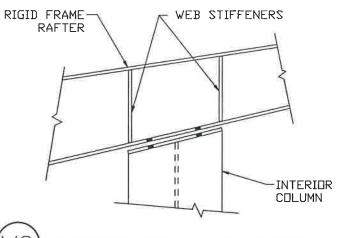
MORTISE PREPPED PERSONNEL DOORS

ALL MORTISE PREPPED PERSONNEL DOORS COME AS RIGHTHAND REVERSED SWING.

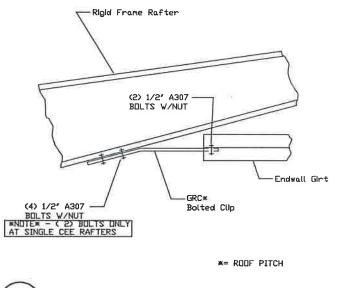
(i.e. STANDING ON THE OUTSIDE OF THE BUILDING FACING THE DOOR, THE LOCK WILL BE ON THE LEFTHAND SIDE OF THE DOOR AND THE DOOR WILL SWING OUTWARD FROM THE BUILDING.)

ANY FIELD MODIFICATIONS ARE THE RE— SPONSIBILITY OF THE ERECTOR AND MBM IS NOT LIABLE FOR LABOR CHARGES NOR DAMAGES DUE TO ERROR.





INTERIOR COLUMN TO RAFTER



SECTION OF ENDWALL GIRT TO RAFTER

STRUCTURAL BOLTED CONNNECTIONS

REFER TO COVER PAGE "GENERAL NOTES" PARAGRAPH "C", SECTION "9" FOR INSTRUCTIONS ON TIGHTENING ALL A325 AND A490 CONNECTION BOLTS.

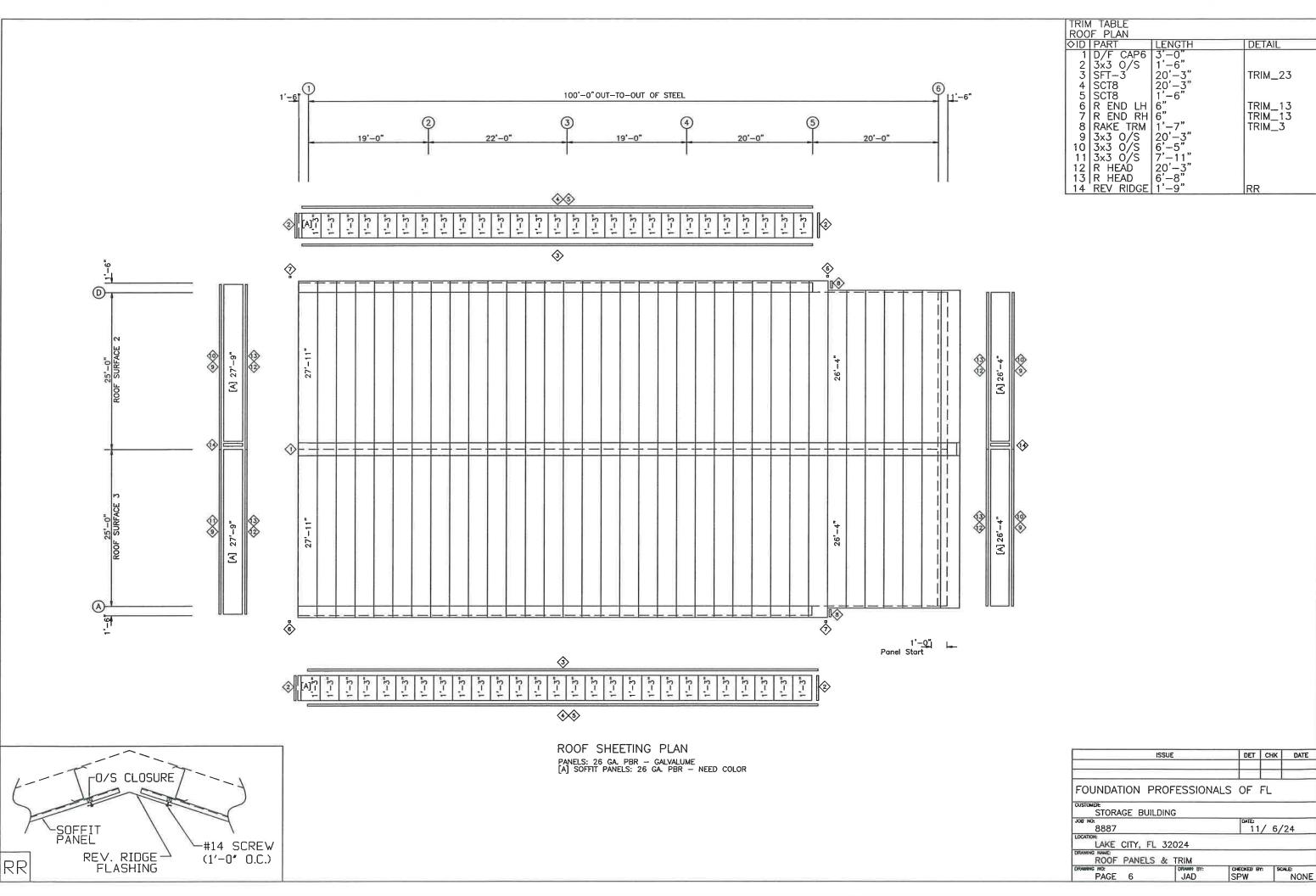
TRIM NOTES:

- [1] SEAL TRIM SPLICES WITH TUBE CAULK.
- [2] SECURE GUTTER SPLICES AND END PLUGS WITH RIVETS.
- [3] SECURE ALL OTHER ROOF TRIM SPLICES WITH TRIM SCREWS UNLESS NOTED OTHERWISE.
- [4] TRIM SCREWS ARE LOCATED 24" ON CENTER UNLESS NOTED OTHERWISE.
- [5] STD. TRIM SPLICES ARE 3" TOTAL UNLESS NOTED OTHERWISE.

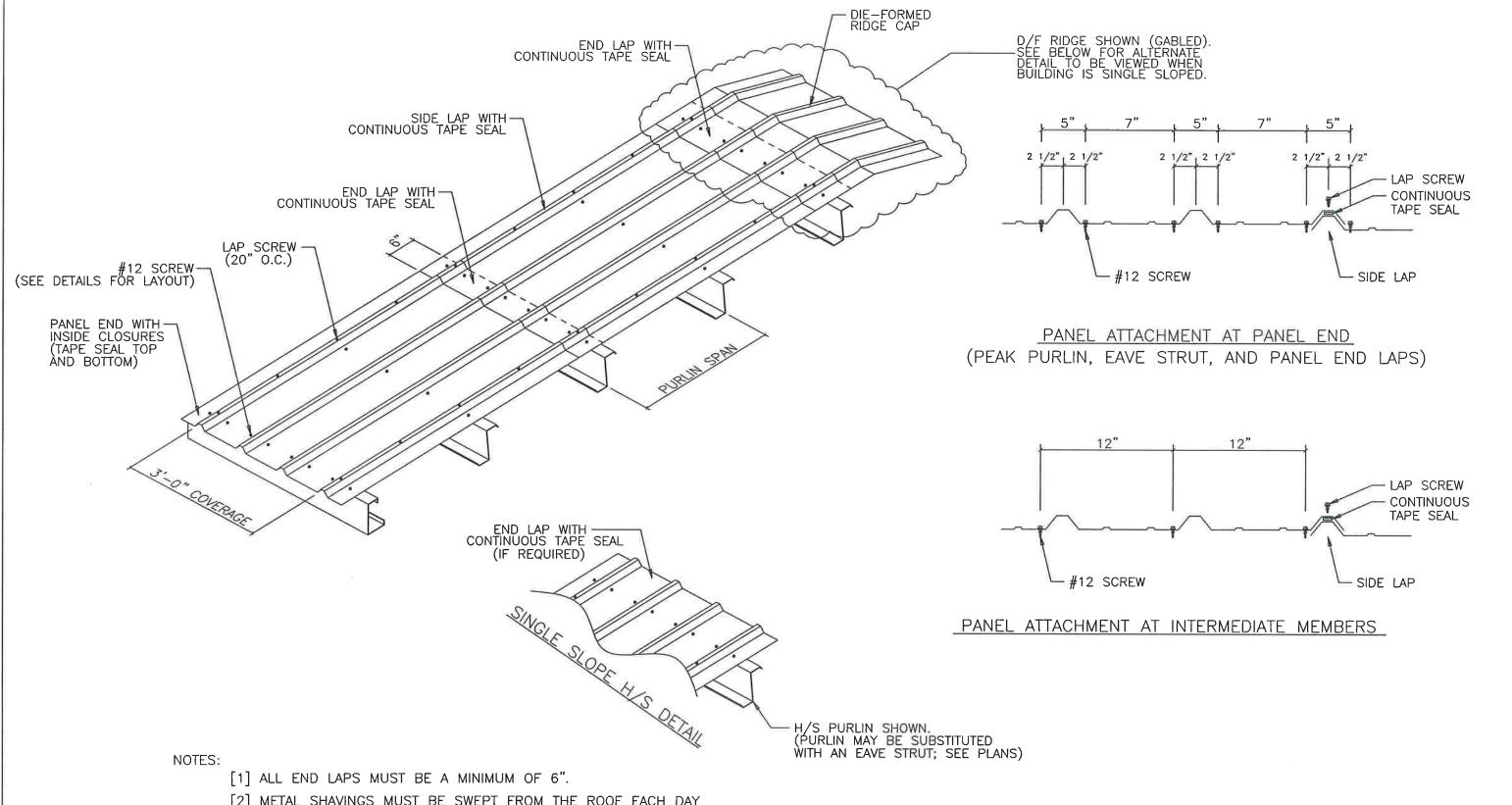
BUILT-UP MEMBER LEGEND

BEAM	BEAM DEPTH	FLANGE WIDTH	FLANGE THK.	WEB THK.
	08	5	4	
B= BUILT-UP	08= 8" 10= 10" 12= 12" 14= 14" ETC.	5,6,8,10 OR 12 (INCHES)	MEASURED IN 16ths. (4= 1/4", 5= 5/16" EIC.)	1= 10ga 3= 3/16" ETC.

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STORAGE BUILDING	}			
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LAKE CITY, FL 320	024			
FRAMING DETAILS				
DACE 5.4	DRAWN BY:	CHECKED B	r: Sx	NONE

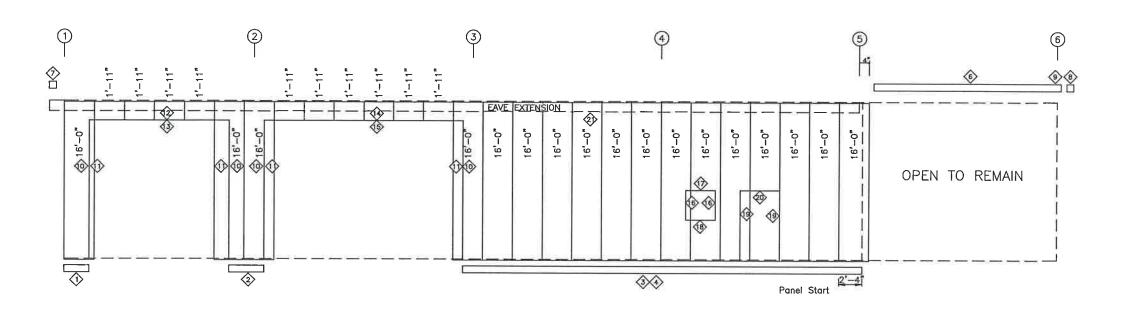


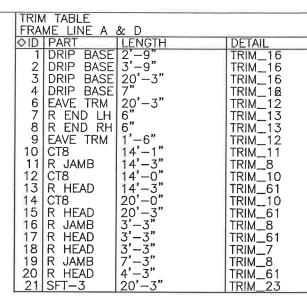
NONE



- [2] METAL SHAVINGS MUST BE SWEPT FROM THE ROOF EACH DAY DURING ERECTION TO PREVENT SURFACE RUSTING.
- [3] TAPE SEAL MUST BE APPLIED WITH NO GAPS OR BREAKS.
- [4] #12 SCREWS ARE USED TO ATTACH THE PANEL TO THE PURLINS. #14 LAP SCREWS ARE USED AT THE PANEL—TO—PANEL ATTACHMENTS. ALL FASTENERS ARE SELF—DRILLING.

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FOUNDATION PR	OFESSION	ALS OF F	^L	
CUSTOMERE STORAGE BUILD	ING			
JOB: NO: 8887		DATE:	/ 6/	24
LAKE CITY, FL	32024			
DRAWING NAME: ROOF PANEL D	ETAILS			
DRAWING NO: PAGE 6.1	JAD	SPW	SCA	NONE





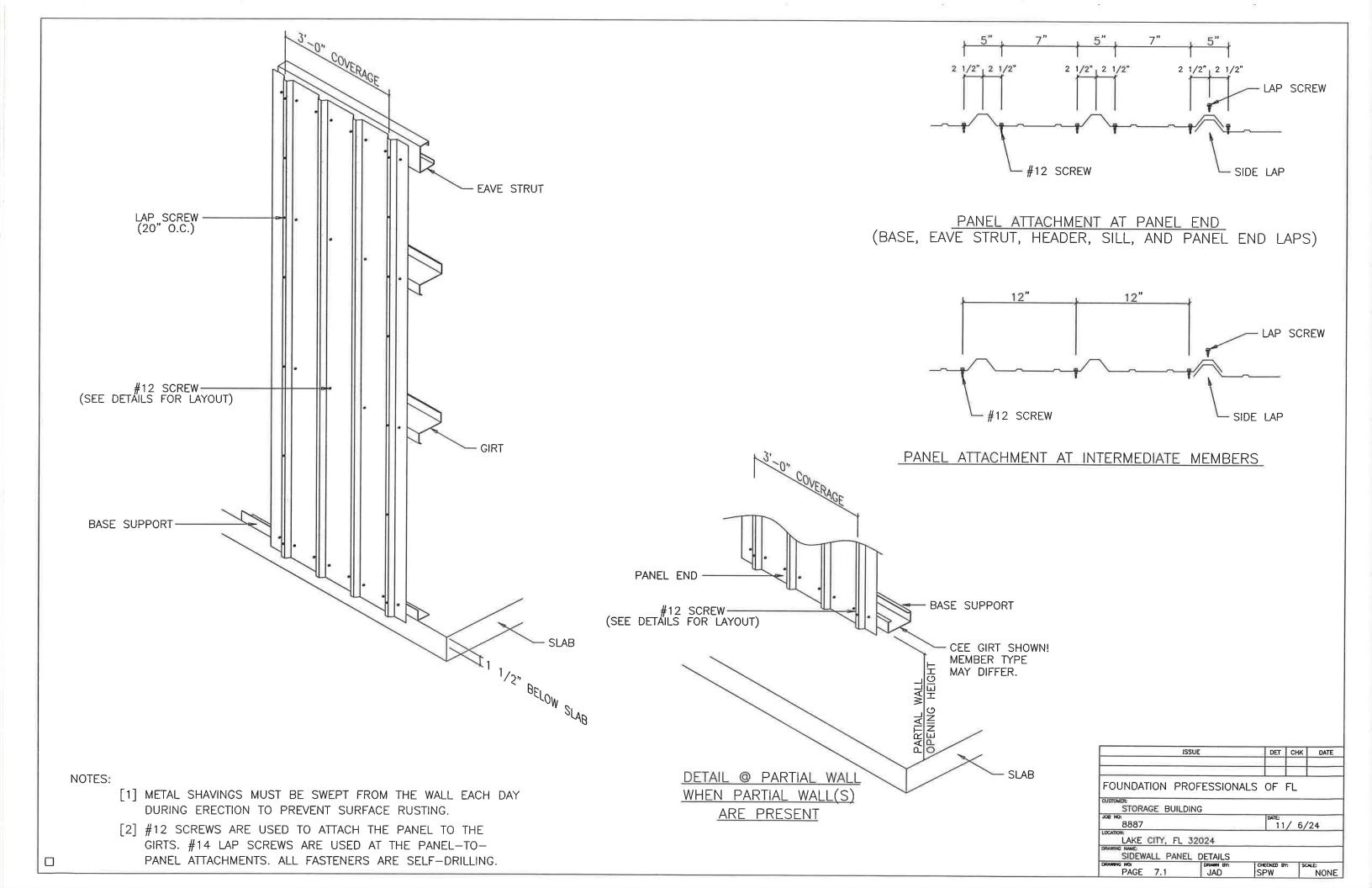
SIDEWALL SHEETING & TRIM: FRAME LINE A
PANELS: 26 GA. PBR - COLOR

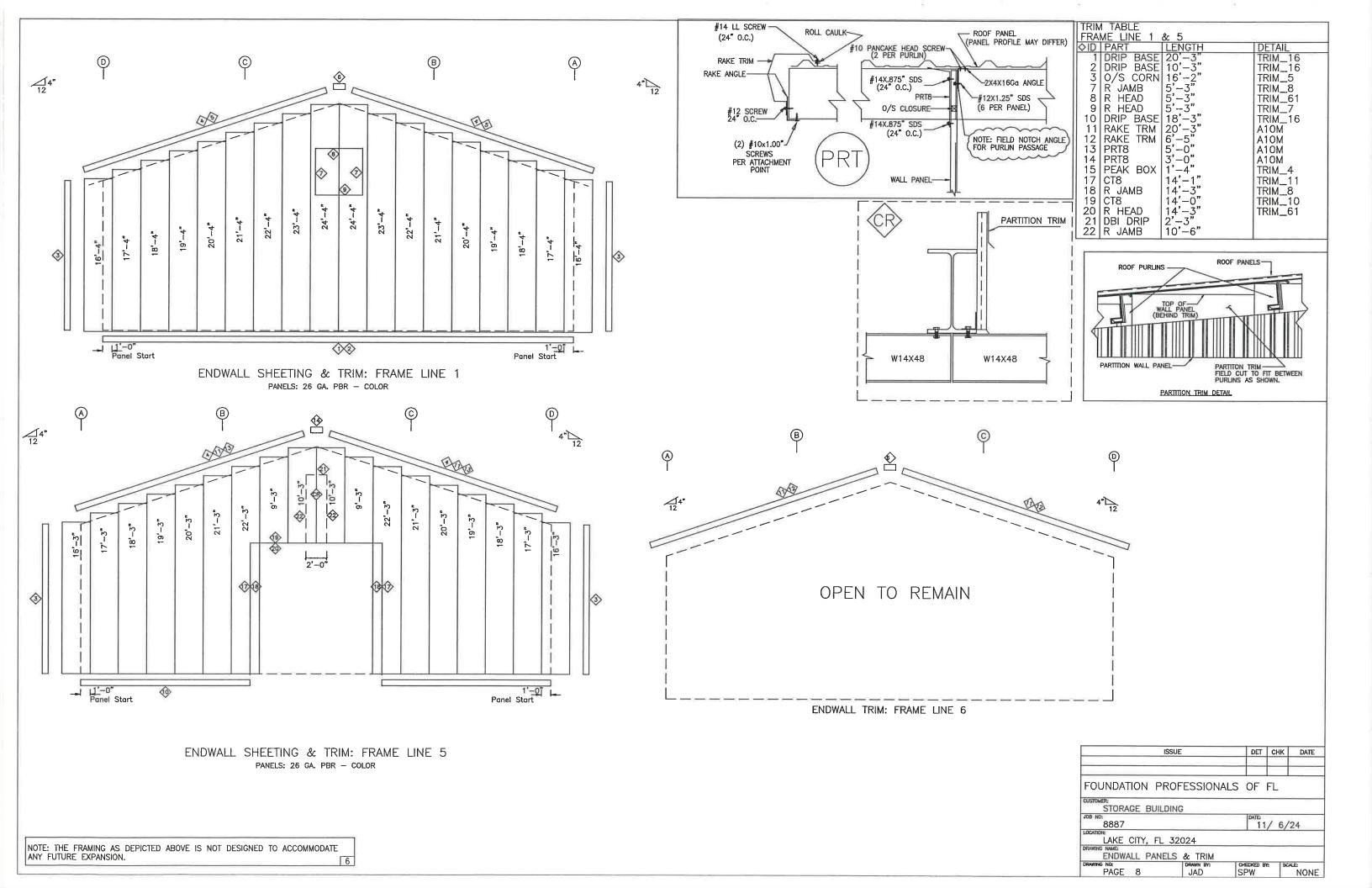
(6) (2) (9) (6)	(5) 4* 						(4)						3							2	ı					(D
OPEN TO REMAIN		16'-0"	16'-0"	16'-0"	16'-0"	16'-0"	16'-0"	16'-0"	16'-0"	16'-0"	16'-0"	16'-0"	16'-0"	16'-0"	16'-0"	16'-0"	16'-0"	16'-0"	16'-0"	16'-0"	16'-0"	16'-0"	16'-0"	16'-0"	16'-0"	16'-0"	16'-0"	
	<u> 2'-</u>	<u>t</u> j Panel	Start								34	>																Ĵ

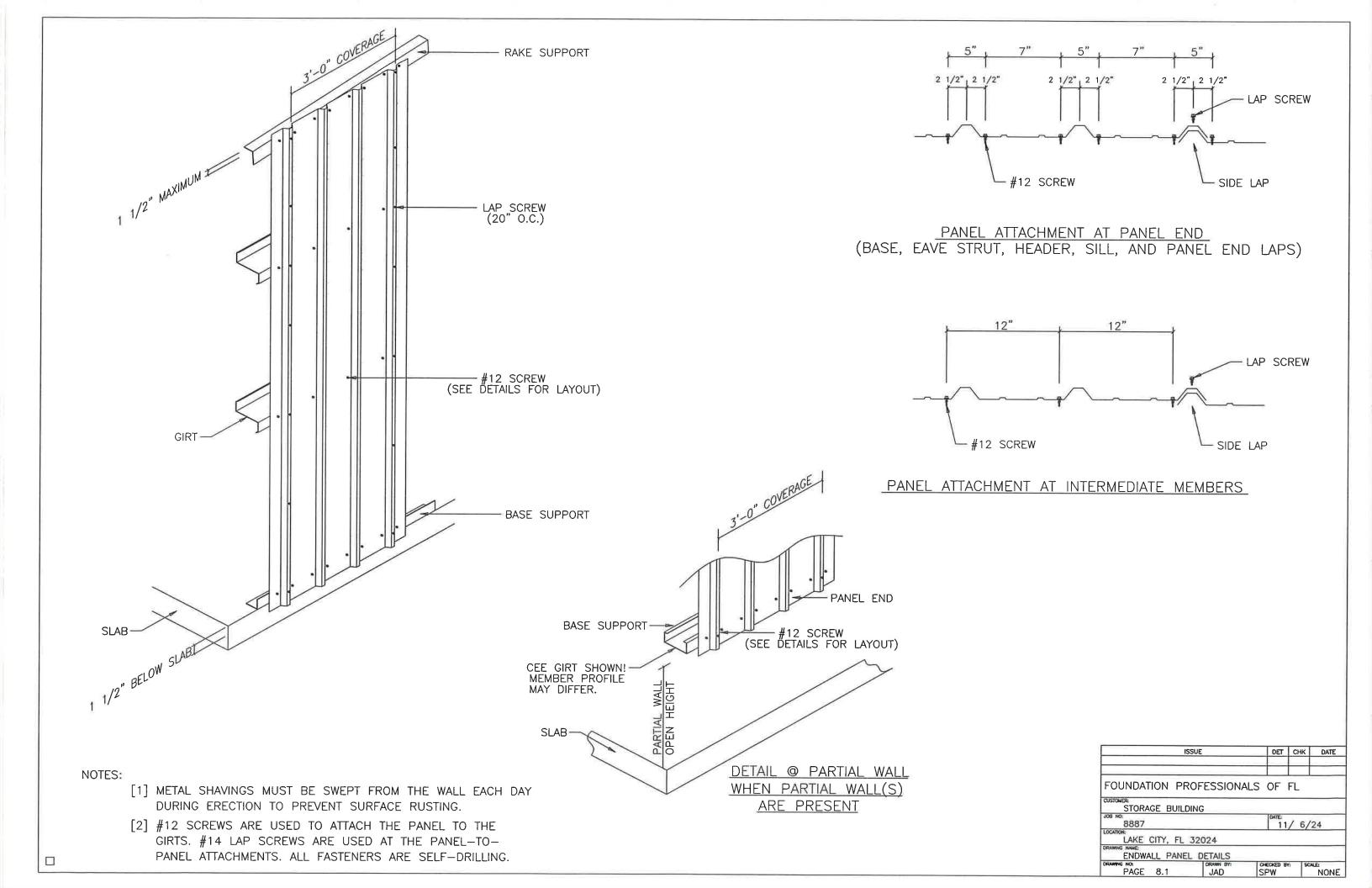
SIDEWALL SHEETING & TRIM: FRAME LINE D
PANELS: 26 GA. PBR - COLOR

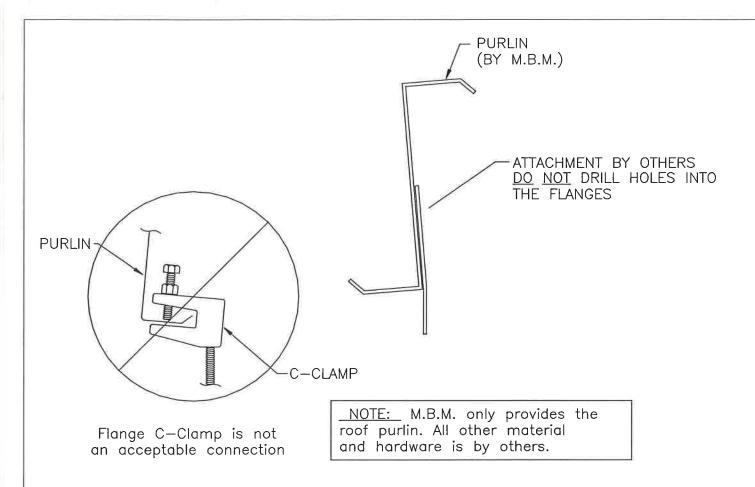
ALL VEHICULAR FRAMED OPENINGS SUPPLIED ON THIS PROJECT HAVE BEEN DESIGNED TO SUPPORT WIND LOADS NORMAL TO A DOOR SYSTEM, BASED ON THE STANDARD BUILDING CODE CRITERIA. THE VEHICULAR FRAMED OPENING HAS NOT BEEN DESIGNED FOR ANY ADDITIONAL MOMENT OR CATENARY FORCE FROM THE DOOR SYSTEM. ANY CHANGES TO THE INFORMATION SHOWN HERE WOULD REQUIRE AN ENGINEERING INVESTIGATION AND POSSIBLE BUILDING REINFORCEMENT.

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CUSTOMER: STORAGE BUIL	DING					
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SIDEWALL PAN	ELS & TRIM					
DRAWING NO: PAGE 7	JAD	SPW SPW	SCA	ve: NONE		









Recommended Connection Detail

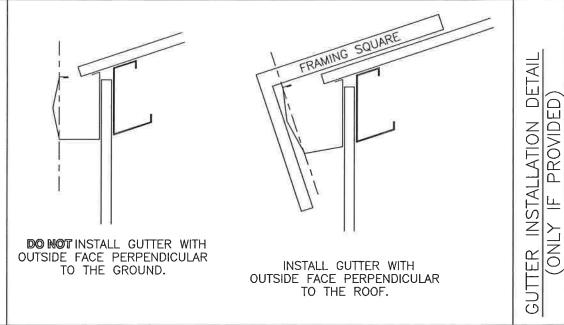
NOTE

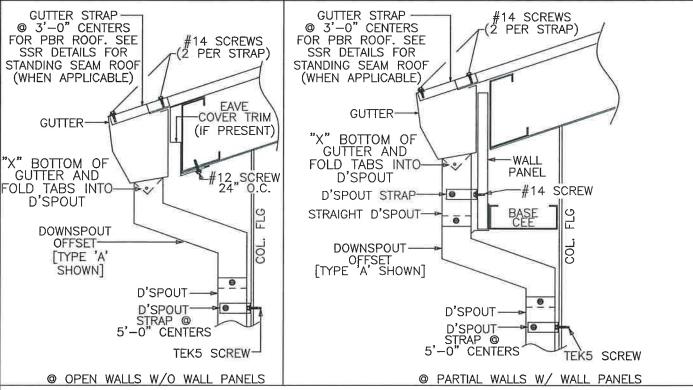
MANY FACTORS BEYOND THE CONTROL OF THE METAL BUILDING SUPPLIER AFFECT THE ABILITY OF A PURLIN TO SAFELY SUPPORT HANGING LOADS COMBINED WITH OTHER REQUIRED ROOF LOADS. DUE TO THE VARIABLES INVOLVED IN HANGING LOADS AND THEIR ATTACHMENTS TO THE PURLINS, THE METAL BUILDING SUPPLIER CANNOT ASSURE THAT THE PURLINS FOR A PARTICULAR BUILDING PROJECT CAN SAFELY SUPPORT THE MAXIMUM ALLOWABLE HANGING LOADS IN COMBINATION WITH OTHER ROOF LOADS.

IT IS THE RESPONSIBILITY OF THE HANGER SYSTEM INSTALLER TO COORDINATE WITH THE ENGINEER OF RECORD FOR THE OVERALL PROJECT TO ENSURE A SAFE HANGING LOAD INSTALLATION. THE METAL BUILDING ENGINEER IS NOT THE ENGINEER OF RECORD FOR THE OVERALL PROJECT. WITHOUT SPECIFIC CERTIFICATION FOR INDIVIDUAL HANGING LOADS, THE NET EFFECTS OF APPLIED HANGER LOADS INSTALLED ON A PARTICULAR PURLIN SHALL NOT EXCEED THE NET EFFECTS OF THE CERTIFIED UNIFORMLY APPLIED DESIGN COLLATERAL LOAD.

HANGING LOADS SHOULD NOT BE APPLIED TO THE PURLIN LIP. WHERE PERMISSIBLE, THE BEST PRACTICE FOR HANGING LOADS IS TO ATTACH TO THE PURLIN WEB USING A BOLT AND NUT, OR SELF—DRILLING SCREWS.

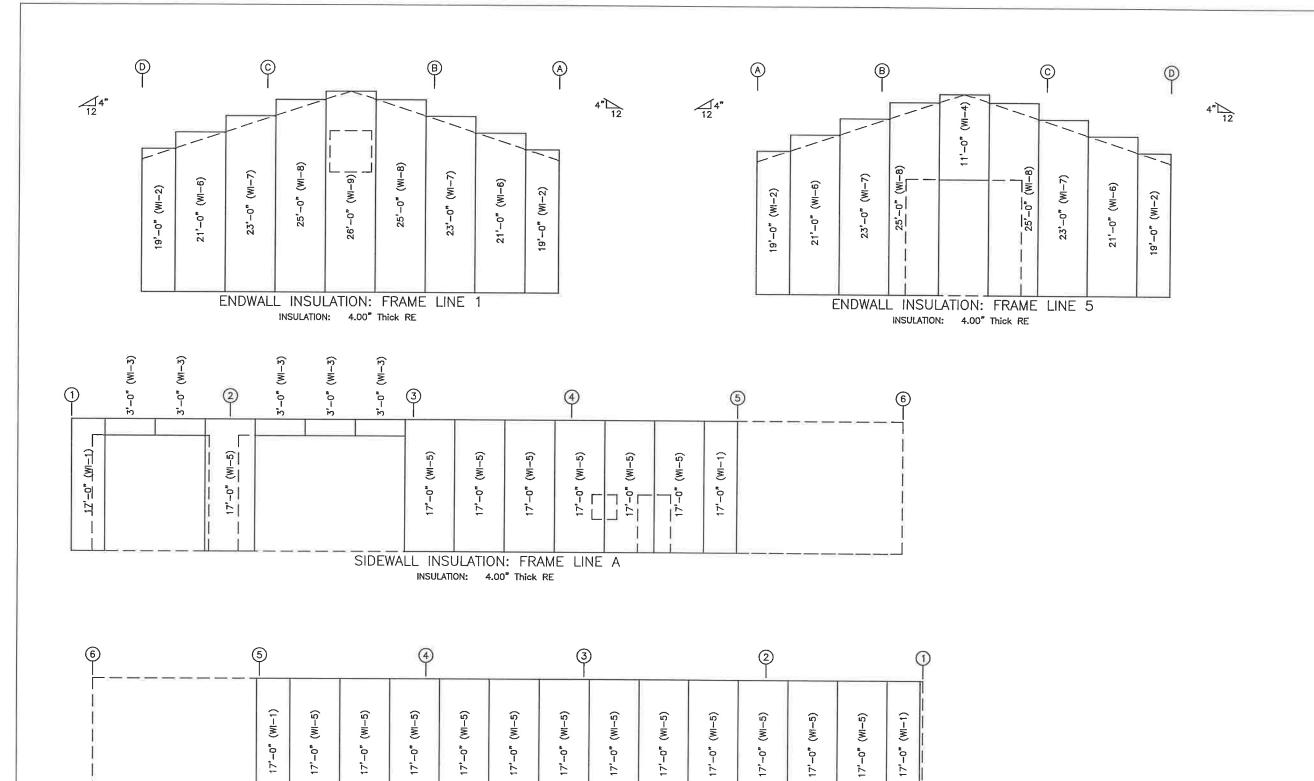
HANGING UNIFORM LOADS SUCH AS SPRINKLER MAINS OR HVAC EQUIPMENT SHOULD BE DISTRIBUTED OVER SEVERAL PURLINS, AND SHOULD NEVER EXCEED THE COLLATERAL LOAD ALLOWANCE FOR THE ROOF SYSTEM. FOR UNIFORM LOADS THAT RUN PARALLEL TO THE PURLINS, IT MAY BE NECESSARY TO USE TRANSVERSE SUPPORT CHANNELS(A.KA. TRAPEZE BEAMS) ATTACHED TO THE WEBS OR FLANGES OF ADJACENT PURLINS TO SPREAD THE LOAD BETWEEN TWO OR MORE PURLINS. IN SUCH CASES, CONTACT THE BUILDING MANUFACTURER OR A LOCAL PROFESSIONAL ENGINEER PRIOR TO ATTEMPTING TO HANG LOADS FROM THE PURLINS





NOTE: REGARDLESS OF DOWNSPOUT OFFSET SCENARIO, TEK5 SCREWS MUST BE USED TO ATTACH DOWNSPOUT STRAPS TO PEMB FRAMING. WHEN WALL PANELS SPAN FROM GROUND TO EAVE (FULL SPAN), #14 SCREWS WILL BE USED TO ATTACH DOWNSPOUT STRAPS TO WALL PANELS.

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FOUNDATION PROFESSIONALS OF FL										
CUSTOMER: STORAGE BUILDIN	NG									
лов ио: 8887			DATE:	/ 6	/24					
LAKE CITY, FL 3	2024									
DRAWING MAKE: SPECIAL DETAILS										
DRAWING NO: PAGE 9	JAD	SF	OKED B	SCALE: NONE						

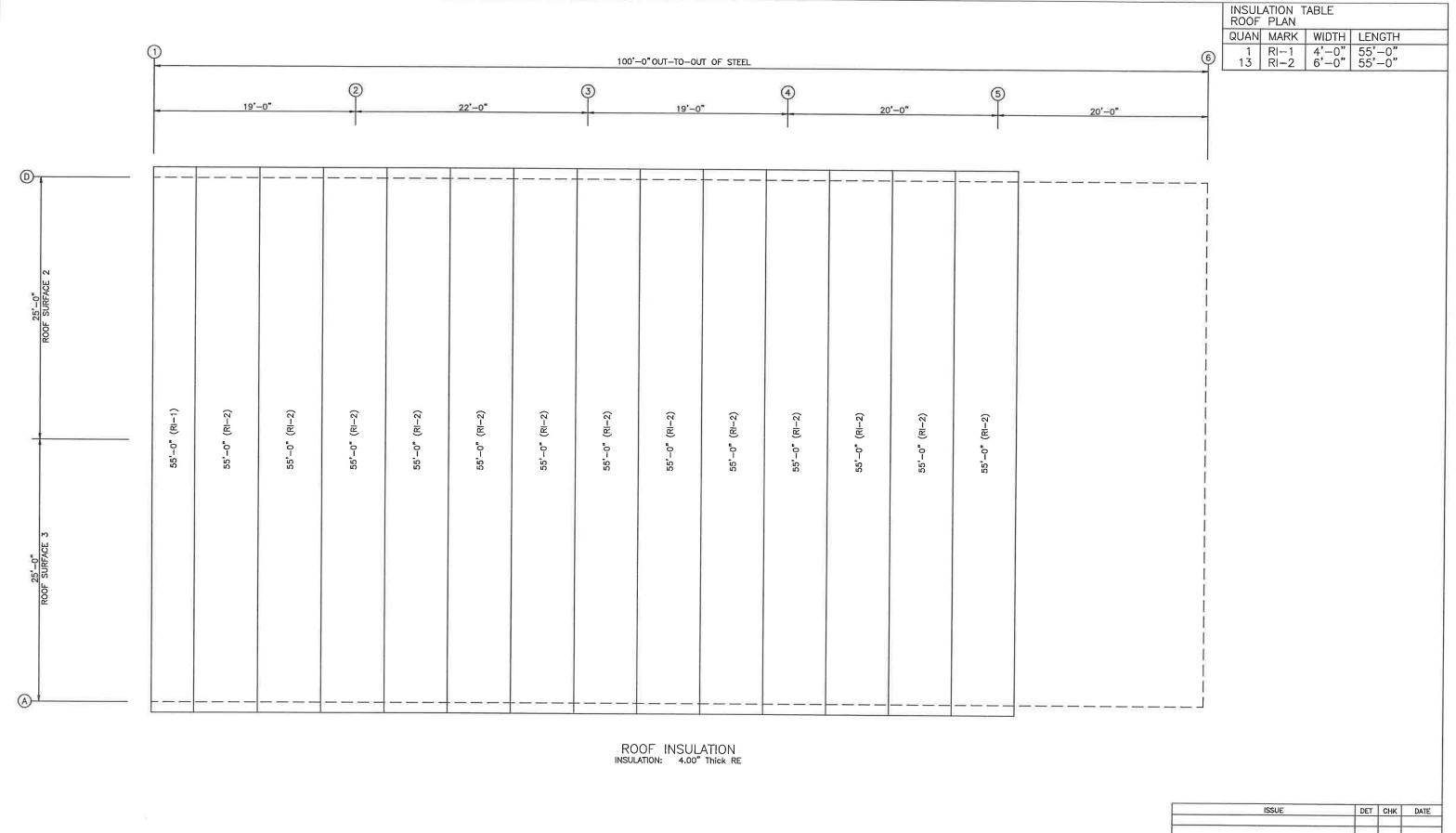


17'-0"

SIDEWALL INSULATION: FRAME LINE D INSULATION: 4.00" Thick RE

	ATION T E LINE	ABLE 1 A 6 I	D	
QUAN	MARK	WIDTH	LENGTH	
4 5 1 19 4 4	WI-1 WI-2 WI-3 WI-4 WI-5 WI-6 WI-7 WI-8	4'-0" 4'-0" 6'-0" 6'-0" 6'-0" 6'-0"	17'-0" 21'-0" 23'-0" 25'-0"	
1	WI-9	6'-0"	26'-0"	

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STORAGE BUILDIN	G				
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LAKE CITY, FL 32	2024				
PRAWING NAME: INSULATION LAYOU	JT				
PAGE 10	JAD	SPW	Y: SC	NONE	



FOUNDATION PROFESSIONALS OF FL

CUSTOMER:
STORAGE BUILDING

JOB. NO:
8887

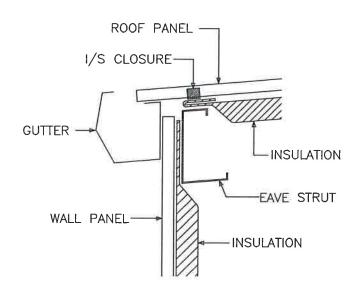
LOCATION:
LAKE CITY, FL 32024

DRAWING NAME:
INSULATION LAYOUT

DRAWING NO:
PAGE 11

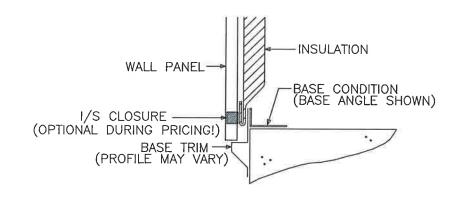
DRAWIN BY:
CHECKED BY:
SPW

NONE



EAVE DETAIL

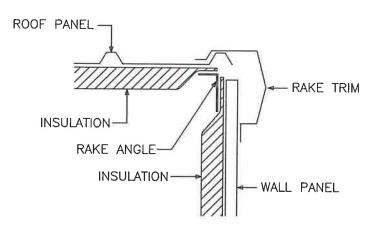
NOTE: FOLD ROOF INSULATION BACK 3" TO 6".



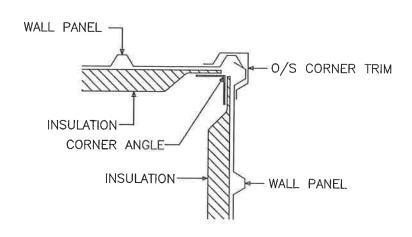
BASE DETAIL

NOTE: FOLD INSULATION BACK 3" TO 6".

CAUTION: FAILURE TO FOLD FACING OF INSULATION BACK FROM THE PANEL EDGE AT THE BASE AND EAVE COULD RESULT IN PANEL DAMAGE AND WILL VOID THE PANEL WARRANTY.



RAKE DETAIL



CORNER DETAIL

ISSUE		DET	СНК	DATE	
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STORAGE BUILI	DING				
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8887			11/ 6/24		
CATION:	70004				
LAKE CITY, FL	32024				
DAMING NAME:	TAU 0				
INSULATION DE					
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PAGE 12	JAD	ISPW		NONE	