

AREA TABLE:

BARN AREA = 1800 FT2
LEAN-TO AREA = 600 FT2

TOTAL AREA = 2400 FT2

	ELECTRICAL LEGEND
$\dot{\diamondsuit}$	LIGHT FIXTURE
\oplus	DUPLEX OUTLET
\bigoplus	220v OUTLET
GF GF	GFI DUPLEX OUTLET
\$	WALL SWITCH
\$3	3 WAY WALL SWITCH
Ф	WATER PROOF GFI OUTLET
	CARBON MONOXIDE ALARM

E - 2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED. E - 3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE. ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY
BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL
BE INTERLOCKED TOGETHER. INSTALL INSIDE AND
NEAR ALL BEDROOMS. TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE
DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S
DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE
SECTIONS OF NEC-LATEST EDITION. E - 6 ELECTRICAL CONT'R SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS. E - 7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY. ALL 120-VOLT, SINGLE-PHASE, 15- AND 20-AMPERE BRANCH CIRCUITS SUPPLYING OUTLETS INSTALLED IN DWELLING UNIT FAMILY ROOMS, DINING ROOMS, LIVING ROOMS, PARLORS, LIBRARIES, DENS, BEDROOMS,

E - 8 SUN ROOMS, RECREATION ROOMS, CLOSETS, HALLWAYS, OR SIMILAR
ROOMS OR AREAS SHALL BE PROTECTED BY A LISTED ARC-FAULT CIRCUIT
INTERRUPTER, COMBINATION-TYPE INSTALLED TO PROVIDE PROTECTION OF THE BRANCH CIRCUIT. E - 9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC E - 10 CONDUCTORS ENTER THE BUILDING.
SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL APPROVAL OF THE BUILDING OFFICIAL - 11

CARBON MONOXIDE ALARMS SHALL BE REQUIRED WITHIN 10'

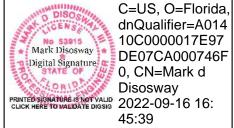
OF ALL ROOMS FOR SLEEPING PURPOSES IN BUILDINGS HAVING A FOSSIL-FUEL-BURNING HEATER OR APPLIANCE, A FIREPLACE, OR ATTACHED GARAGE. E - 12 ALL OUTLETS LOCATED IN RESIDENTIAL TO BE TAMPER-RESISTANT PER NEC. .13 A MINIMUM OF 75% OF PERMANENTLY INSTALLED LAMPS OR LIGHTING FIXTURES SHALL BE HIGH EFFICACY FBC EC SEC. R404.1



FL PE 53915

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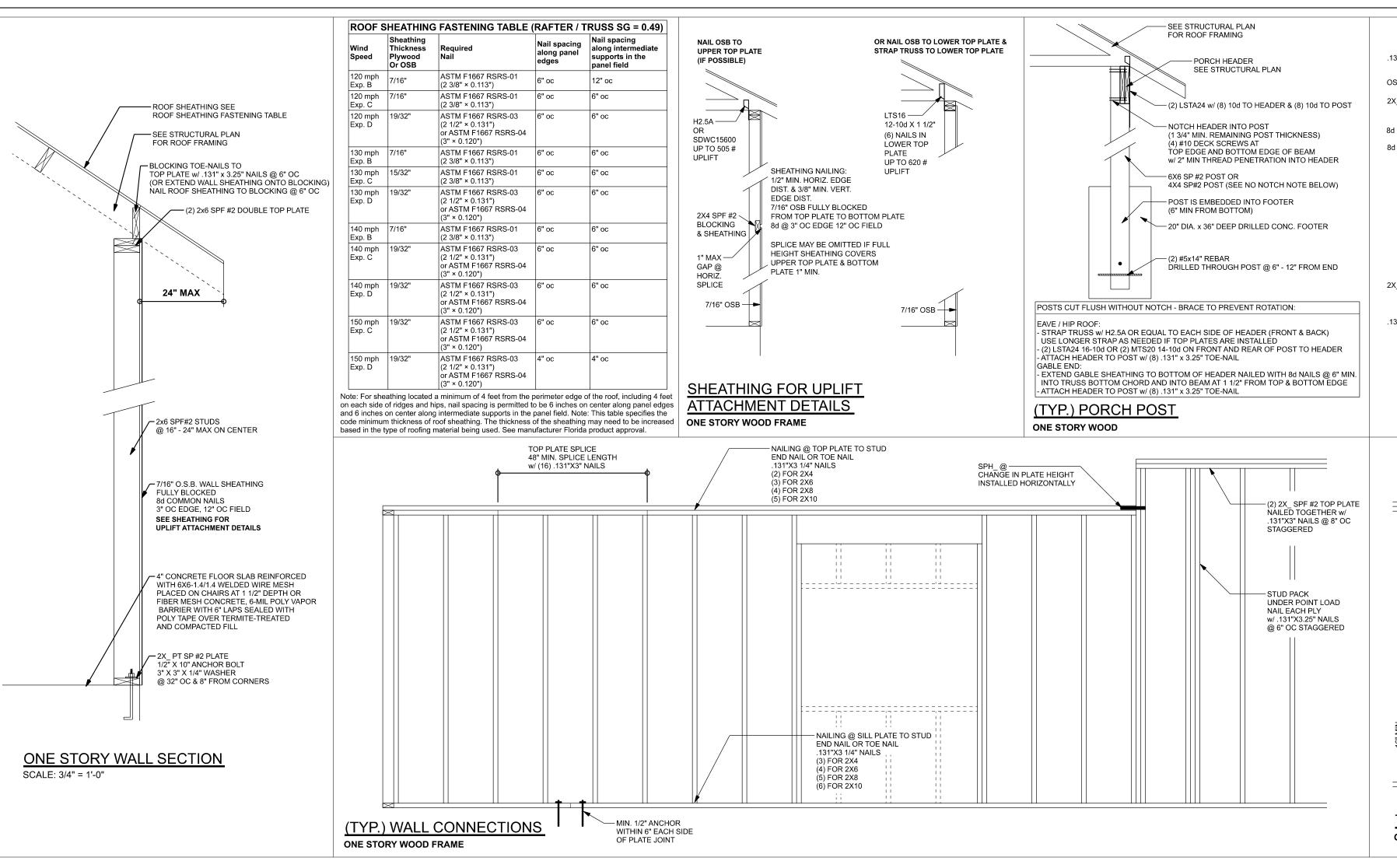
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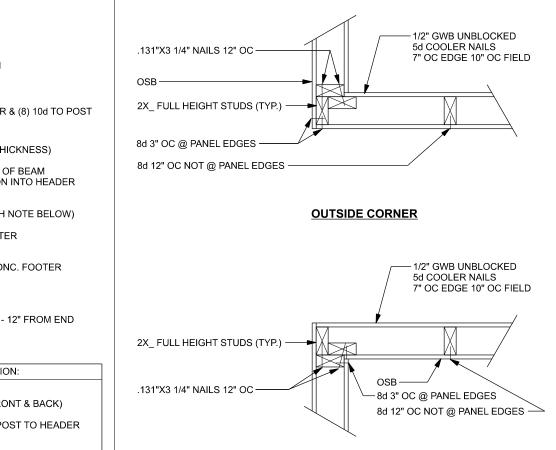
CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

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JOB NUMBER: 221101 #1
OF 3 SHEETS





(TYP.) CORNER FRAMING

— SEE "STRUCTURAL PLAN NOTES" ——

FOR (U.N.O) STRAPING OR SCREWS

ALL OTHER STRAPING OR SCREWS LESS OR MORE

IS NOTED ON STRUCTURAL PLANS

—(6) .131"X3 1/4" TOE NAILED —

OR BACK NAILED THRU

KING STUD INTO HEADER

SHEATHING MUST BE NAILED TO TOP PLATES

w/ 8d 3" OC (NAILING MAY BE STAGGERED)

ALL HEADER JACK & KING STUDS SHALL

BE FASTENED TO EACH OTHER w/ (2) ROWS

– WINDOW SILL PLATE —

(PER TABLE BELOW) TOE NAIL ENDS OF EACH PLY W/

2x4 = (4) .131" x 3.25" NAILS

 $2x6 = (6) .131'' \times 3.25'' NAILS$

CRIPPLES (F REQUIRED

& SHEATHING NAILED TO HEADER

w/ (2) ROWS OF 8d @ 6" OC

10d @ 8" OC STAGGERED

INSIDE CORNER

		CONNECTO	OR TABLE			GLINEIXAL INOTE	GLINEITAL ING I LO.	GENERAL NOTES:
Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter				TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LIC
805	505	SDWC15600	-	-				ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHA
415	290	H3	4-8dx1 1/2"	4-8dx1 1/2"				DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANEN TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACT
615	540	H2.5A	5-8dx1 1/2"	5-8dx1 1/2"	ALL BEARING	ALL BEARING LOCATIONS. TRU	ALL BEARING LOCATIONS. TRUSS ENGINEERIN	ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RE
1340	1015	H10A	9-10d1 1/2"	9-10d1 1/2"				TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY N
720	620	LTS12-20	6-10d1 1/2"	6-10d1 1/2"				FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO
1000	860	MTS12-30	7-10d1 1/2"	7-10d1 1/2"	BASED ON T	BASED ON TRUSS ENGINEERI	BASED ON TRUSS ENGINEERING UPLIFT AND F	BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FO
1450	1245	HTS20-30	12-10d1 1/2"	12-10d1 1/2"				WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCT
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member				WITH MIN. UPLIFT CONNECTION 415LB EACH END; 2X8 RAF
1235	1235	LSTA21	8-10d	8-10d	OUTE DDEDAG	OUTE DDEDADATION OUTE AND	OUTE DEEDA DATION, OUTE ANALYSIS AND DEED	CITE PREDADATION, CITE ANALYSIS AND PREDADATION IS
1640	1455	MSTA24	9-10d	9-10d	SITE PREPAR	SITE PREPARATION: SITE ANA	SITE PREPARATION: SITE ANALYSIS AND PREPA	SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS N
1030	1030	CS20	7-10d	7-10d				FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN &
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate				GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARIN
585	535	SP1	6-10d	4-10d	VISUAL OBSE	VISUAL OBSERVATION OR SOL	VISUAL OBSERVATION OR SOILS TEST PROVES	VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWIS
1065	605	SP2	6-10d	6-10d	CONCRETE:	CONCRETE: MINIMUM COMPR	CONCRETE: MINIMUM COMPRESSIVE STRENG	CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONC
771	771	LSTA24	10-10d	wrap under or over plate	WELDED WIE	WEI DED WIDE BEINEOBOED (WELDED WIDE DEINEODOED SLAB: 6" v 6" W1 /	WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4. FB =
1235	1235	LSTA24	14-10d	wrap under or over plate				REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM
Uplift SP	Uplift SPF	Holdowns @ Stemwall	To Stud / Post	Anchor				OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS O
1825	1800	DTT2Z	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD	NOT TO EXC	NOT TO EXCEED 3'.	NOT TO EXCEED 3'.	NOT TO EXCEED 3'.
4235	3640	HTT4	18-16dx2 1/2"	1/2"x12" Titen HD	FIBER CONC	FIBER CONCRETE SLAB: CONC	FIBER CONCRETE SLAB: CONCRETE SLABS OF	FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND C
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor				REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. D
1825	1800	DTT2Z	8-SDS 1/4"x1 1/2"	1/2"x6" Titen HD				TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURE FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PRO
4235	3640	HTT4	18-16dx2 1/2"	1/2"x12" Titen HD				CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BU
Uplift SP	Uplift SPF	Post Bases @ Stemwall	To Post	Anchor	CONTROL IC	CONTROL JOINTS: WHERE SR	CONTROL IOINTS: WHERE SPECIFIED SAWN (CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JO
1900		ABU44Z	12-16d	5/8"x12" Drill & Epoxy				BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE C
2475		ABU66Z	12-16d	5/8"x12" Drill & Epoxy	PLACEMENT.	PLACEMENT. THE LENGTH / W	PLACEMENT. THE LENGTH / WIDTH RATIOS OF	PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS
2410	Unlift SPF	Post Bases @ Mono	To Post	Anchor				TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWN (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJE
					ILKECOMINEN			
	<u> </u>	ABU44Z	12-16d	5/8"x7" Drill & Epoxy				CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NO

HEADER SCREWS TABLE							
Option	Uplift	Top Connection	Bottom Connection				
#1	< 510	Attach king stud to top plate w/ (1) Simpson SDWC15600	Attach king stud to bottom plate w/ (2) Simpson SDWC15450 1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer must be located within 6" of king stud @ all door locations				
#2	< 895	Attach king stud to top plate w/ (2) Simpson SDWC15600	Attach king stud to bottom plate w/ (3) Simpson SDWC15450 1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer must be located within 6" of king stud @ all door locations				

	HEADER STRAP TABLE						
Option	Uplift	Top Connection	Bottom Connection				
#1	< 1235	LSTA24, 14-10d wrap over plate	LSTA24, 14-10d wrap under plate 1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer must be located within 6" of king stud @ all door locations				
#2	< 1455	MSTA24, 18-10d header to jacks	DTT2Z				
#3	< 1800	(2) MSTA24, 18-10d header to jacks	DTT2Z				
#4	< 2910	(2) MSTA24, 18-10d header to jacks	HTT4				

SILI	SILL PLATE SPANS FOR 10'-0" WALL HEIGHT									
DESIGN WIND SPEED	MAX	. SPANS FO	BASED ON WFCM							
	(1) 2x4	(2) 2x4	(1) 2x6	(2) 2x6	TABLE A-3.23B					
					FOR OTHER WALL					
30 MPH EXP. C	5'-2"	7'-9"	7'-7"	11'-3"	HEIGHTS (H) SILL SPAN SHALL BE					
					DIVIDED BY (H/10)					

EACH SIDE OF DOOR OPENING WITH 3/8"X4" LAG

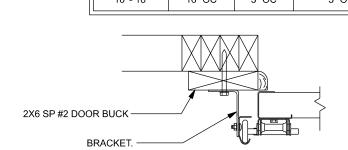
SCREWS w/ 1" WASHER LAG SCREWS MAY BE

COUNTERSUNK, HORIZONTAL JAMBS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR

TYPICAL HEADER STRAPING OR SCREWS DETAIL ONE STORY WOOD FRAME w/ STRAPS & ANCHORS

2X6 SP #2 GARAGE DOOR BUCK ATTACHMENT ATTACH GARAGE DOOR BUCK TO STUD PACK AT

DOOR WIDTH	3/8"X4" LAG	16d STAGGER	(2) ROWS OF .131"X3 1/4" NAILS				
8' - 10'	24" OC	5" OC	5" OC				
11' - 15'	18" OC	4" OC	4" OC				
16' - 18' 16" OC 3" OC 3" OC							



(TYP.) GARAGE DOOR BUCK INSTALLATION

EXTERIOR	WALL S	TUD T	ABLE	FOR	SPF	#2	STU	IDS

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, FY = 40 KSI. ALL LAP SPLICES 40 * DB

UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING

ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS,

AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED

LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED

FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE

SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS,

TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND

BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL

RUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS

THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR. IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN

COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS. MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN

PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED

I OADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO

REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT

RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY TH

TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES

RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR

(25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; SHEATHING,

ACCORDANCE WITH ACI 315-96, U.N.O.

TO ACHIEVE RATED LOADS.

15" IN GROUTED CMU.

DESIGN PRESSURES.

MEMBERS, WITH PANEL EDGES STAGGERED.

BUILDER'S RESPONSIBILITY:

THE WIND LOAD ENGINEER IMMEDIATELY

ROOF SYSTEM DESIGN:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5. EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

(1) 2x4 @ 16 OC	TO 10-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

GRA	DE & SPECIES TA	BLE	
		Fb	Е
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

16x7 GARAGE DOOR

DESIGN CRITERIA & LOAD	S:
BUILDING CODE	7TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2020)
CODE FOR DESIGN LOADS	ASCE 7-16
WINDLOADS	
BASIC WIND SPEED (ASCE 7-16, 3S GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	С
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	I
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
C&C DESIGN PRESSURES	SEE TABLE
FLOOR LOADING	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
ROOF LOADING	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
SOIL BEARING CAPACITY	1500 PSF
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE

COMPONENT & CLADING DESIGN PRESSURES 130 MPH (EXP C)									
EFFECTIVE WIND AREA (FT2)	ZONE 4 INTERIOR			ZONE 5 END 4' FROM AL OUTSIDE CORN					
0 - 20	+25.6(Vas	sd) -27.8(Va	asd)	+25.6(Vasd)	-34.2(Vasd)				
0 - 20	+42.6(Vu	ult) -46.2(Vu	ult)	+42.6(Vult)	-57(Vult)				
GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C)									
9x7 GARAGE DOOR		+22.6(Vasd)	-25.5	(Vasd)					

+21.7(Vasd) -24.1(Vasd)

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