Hampton Residence, Columbia County FL

Addendum for Wind Load Analysis Requirements

(In Compliance with the 2004 Florida Building Code and Amendments)

Prepared By: Marty J. Humphries, P.E. # 51976
7932 240th St., O'Brien, FL 32071
(386)935-2406

Additional Foundation Reanalysis\Requirements:

The stem-wall geometry and longitudinal steel requirements have been reanalyzed for this project. After re-evaluating the structural capacity of the footer for this project the requirement of 3-#5 rebar continuous may be reduced to 2-#5 rebar continuous in the footer. All other requirements shall remain in effect.

26569

Marty J. Humphries

3-19-08

Location: Project Name:

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and the product approval number(s) on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit on or after April 1, 2004. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s
A. EXTERIOR DOORS			FL 4242-R
1. Swinging			
2. Sliding			
3. Sectional			
4. Roll up			
5. Automatic			
6. Other			
B. WINDOWS			
1. Single hung			FL 5108
2. Horizontal Slider			FL 5451
3. Casement			
4. Double Hung			
5. Fixed			FL 5418
6. Awning			
7. Pass -through			
8. Projected			
9. Mullion			
10. Wind Breaker			
11 Dual Action			
12. Other			
C. PANEL WALL			
1. Siding			FL 889-R2
2. Soffits			FL 4899
3. EIFS		Vinyl Siding D5	FL 4905
4. Storefronts			
5. Curtain walls			
6. Wall louver			
7. Glass block			FL 3820-R1
8. Membrane			
9. Greenhouse			
10. Other			
D. ROOFING PRODUCTS			
1. Asphalt Shingles			FL 586-R2
2. Underlayments			FL 1814-R1
3. Roofing Fasteners			
4. Non-structural Metal Rf			
5. Built-Up Roofing			
6. Modified Bitumen			
7. Single Ply Roofing Sys.			
8. Roofing Tiles			
9. Roofing Insulation			
10. Waterproofing			
11. Wood shingles /shakes			
12. Roofing Slate			

Category/Subcategory (cont.)	Manufacturer	Product Description	Approval Number(s)
13. Liquid Applied Roof Sys			
14. Cements-Adhesives – Coatings			FL. 1960-R1
15. Roof Tile Adhesive			
16. Spray Applied Polyurethane Roof			
17. Other			
E. SHUTTERS			
1. Accordion			
2. Bahama			
3. Storm Panels			
4. Colonial			
5. Roll-up			
6. Equipment			
7. Others			
F. SKYLIGHTS			FL 457-R1
1. Skylight			
2. Other			
G. STRUCTURAL COMPONENTS			
1. Wood connector/anchor			FL. 474-R1
2. Truss plates			
3. Engineered lumber			FL 1008-R1
4. Railing			
5. Coolers-freezers			
6. Concrete Admixtures			
7. Material			
8. Insulation Forms			
9. Plastics			
10. Deck-Roof			
11. Wall			
12. Sheds			
13. Other			
H. NEW EXTERIOR ENVELOPE PRODUCTS			
1.			
2.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) the performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements.

I understand these products may have to be removed if approval cannot be demonstrated during inspection

Contractor or Contractor's Authorized Agent Signature

Print Name

Date

Location

Permit # (FOR STAFF USE ONLY)

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
 Document ID:1TDH8228Z0126091007

Truss Fabricator:	Anderson Truss Company
Job Identification:	7-380--GARY JOHNSON Hampton -- , **
Truss Count:	9
Model Code:	Florida Building Code 2004 and 2006 Supplement
Truss Criteria:	ANSI/TPI-2002(STD)/FBC
Engineering Software:	Alpine Software,Versions 7.36, 7.37.
Structural Engineer of Record:	The identity of the structural EOR did not exist as of
Address:	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads:	Roof - 40.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 110 MPH ASCE 7-02 -Closed

Seal Date: 12/26/2007

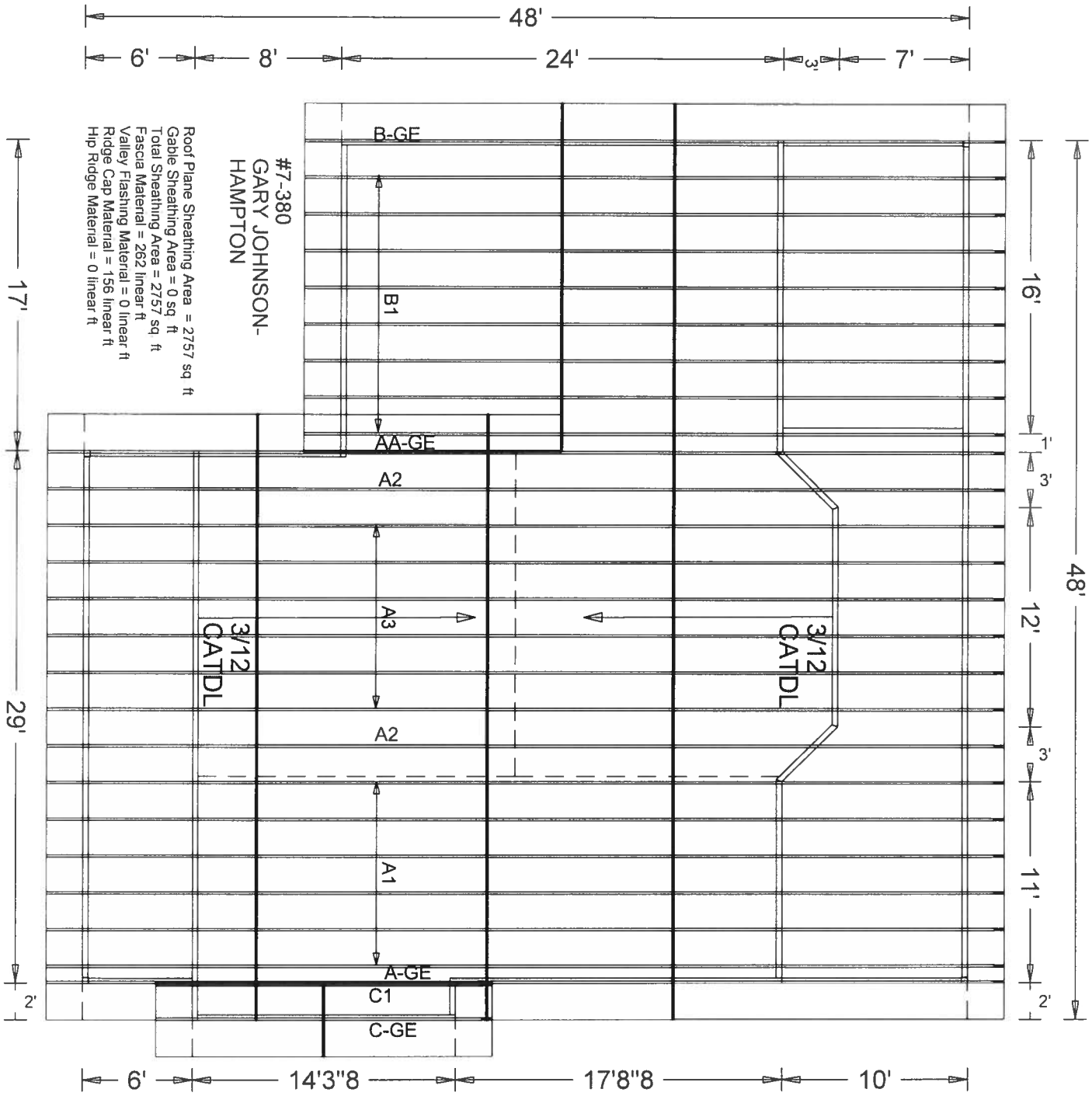
-Truss Design Engineer-
Doug Fleming
Florida License Number: 66648
1950 Marley Drive
Haines City, FL 33844

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

Details: 140GS-BRCLBSUB-140GC-A11015EE-GBLLETIN-

#	Ref	Description	Drawing#	Date
1	18825	A-GE	07360002	12/26/07
2	18826	A3	07360003	12/26/07
3	18827	A2	07360004	12/26/07
4	18828	AA-GE	07360005	12/26/07
5	18829	A1	07360006	12/26/07
6	18830	B1	07360009	12/26/07
7	18831	B-GE	07360007	12/26/07
8	18832	C1	07360008	12/26/07
9	18833	C-GE	07360001	12/26/07



JOB DESCRIPTION:: GARY JOHNSON /: Hampton	JOB NO: 7-380	PAGE NO: 1 OF 1

(7 380 GARY JOHNSON Hampton , ** A GE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP #2 Dense:
:Stack Chord SC2 2x4 SP #2 Dense:

Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3",min.)nails @ 6" OC.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

SEE DRW HCUSR001 02086012 FOR GABLE DETAILS.

(**) 7 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

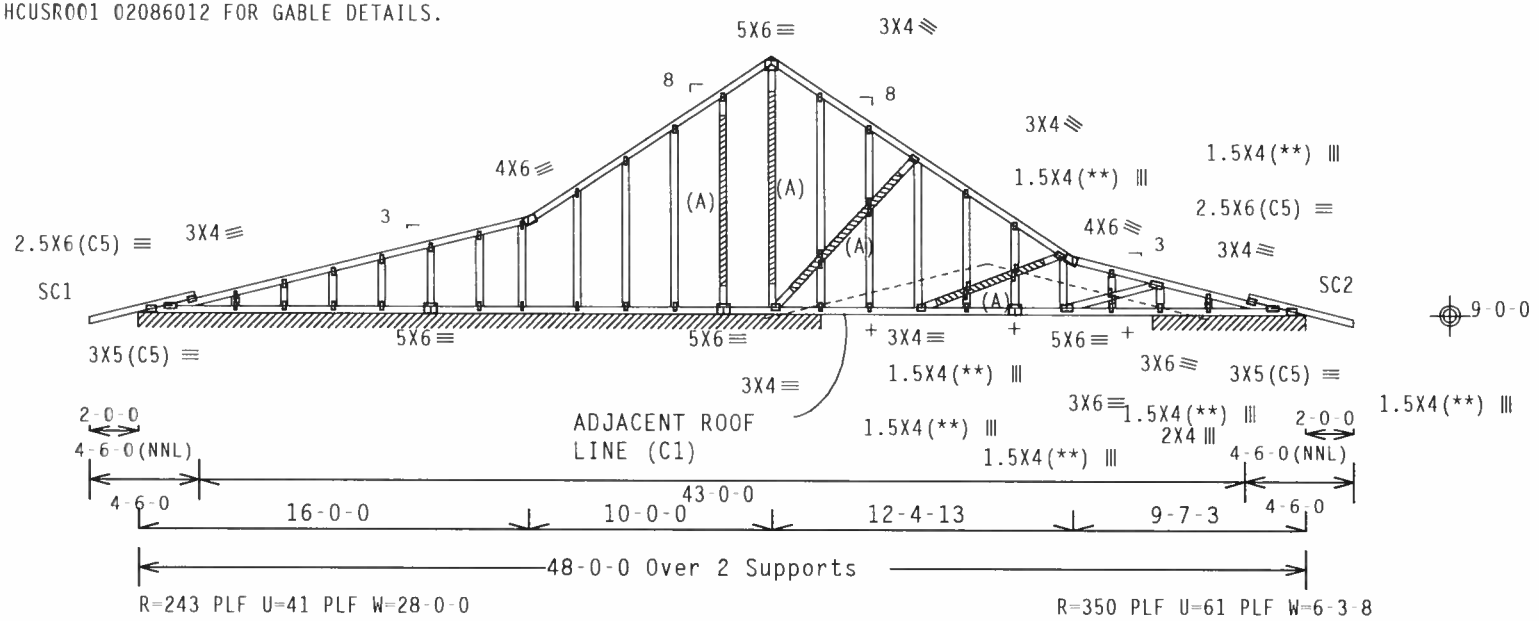
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

+ MEMBER TO BE LATERALLY BRACED FOR WIND LOADS PERPENDICULAR TO TRUSS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

The building designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be designed by the building designer.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.042 QTY:1 FL/-/4/-/-/R/- Scale =.125"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AFAPA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
26 '07

TC LL	20.0 PSF	REF	R8228- 18825
TC DL	10.0 PSF	DATE	12/26/07
BC DL	10.0 PSF	DRW	HCUSR8228 07360002
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	67215
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TDH8228Z01

(7 380 GARY JOHNSON Hampton , ** A3)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

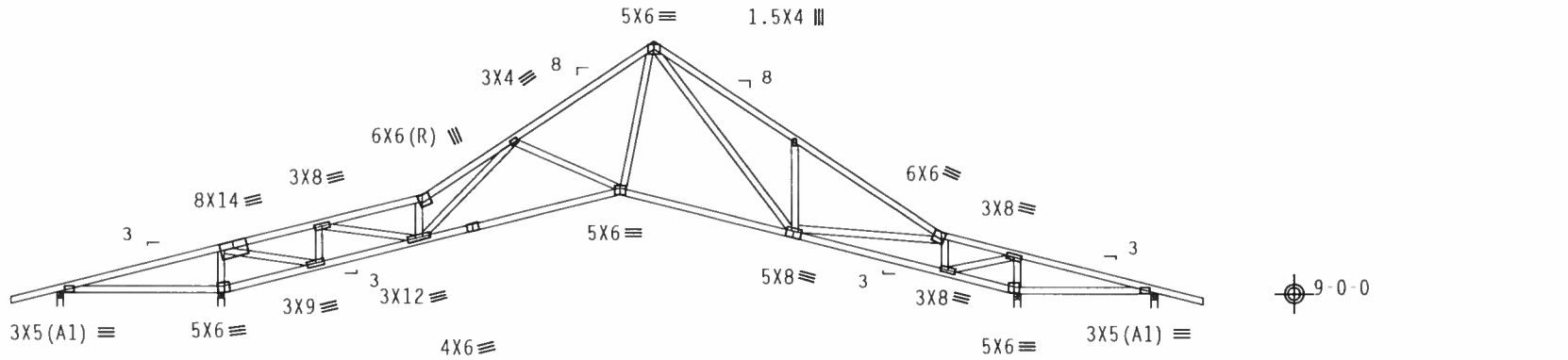
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

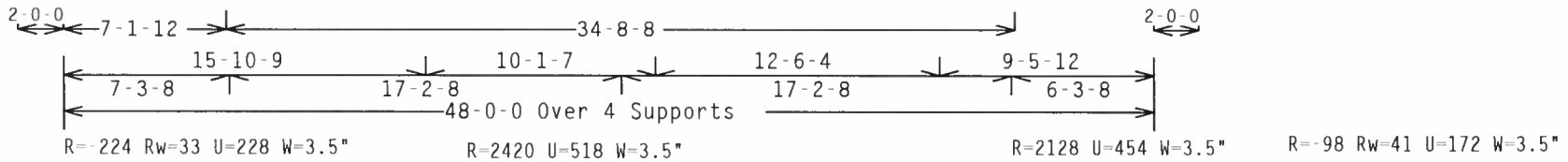
WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Negative reaction(s) of -223# MAX. (See below) from a non-wind load case requires uplift connection.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.





Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.043 QTY:1 FL/-/4/-/-/R/- Scale =.125"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
26 '07

TC LL	20.0 PSF	REF	R8228- 18826
TC DL	10.0 PSF	DATE	12/26/07
BC DL	10.0 PSF	DRW	HCUSR8228 07360003
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	67285
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TDH8228Z01

(7-380--GARY JOHNSON Hampton -- , ** - A2)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

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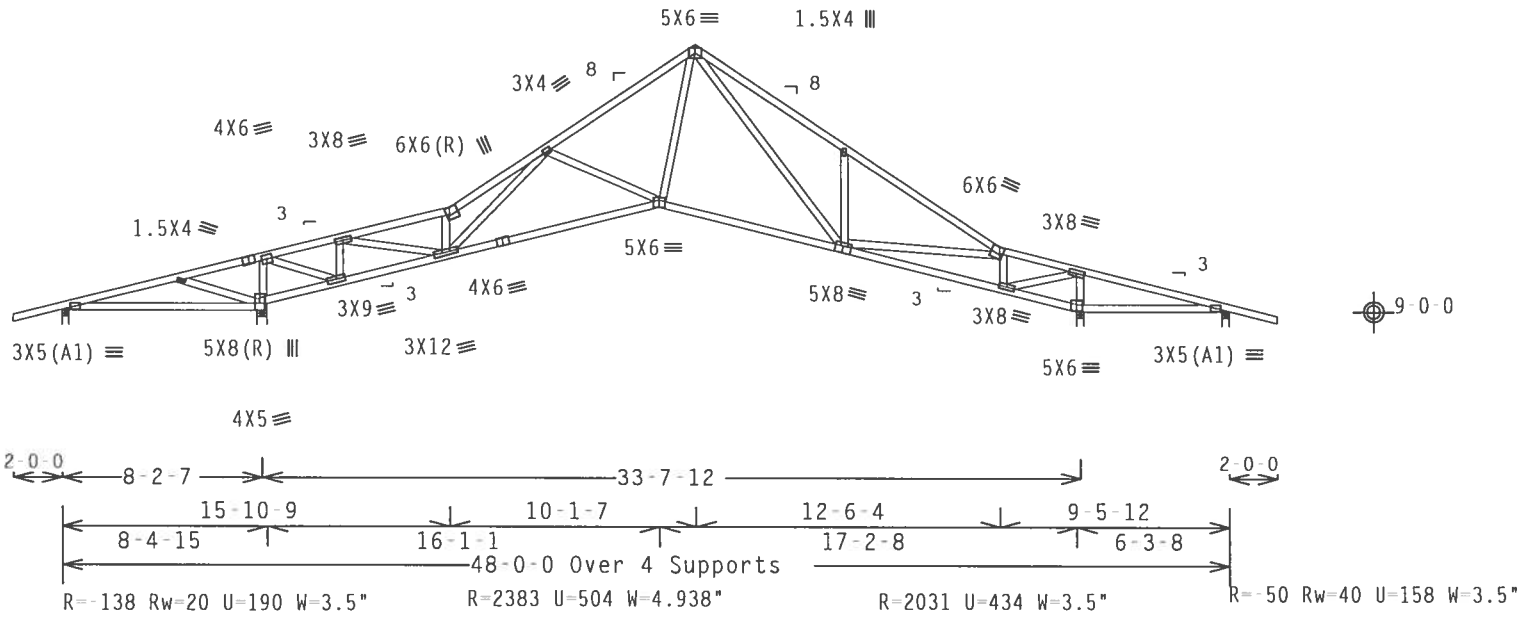

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.042 QTY:1 FL/-/4/-/-/R/- Scale =.125"/Ft.

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ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 279

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
DEC 26 '07

TC LL	20.0 PSF	REF	R8228- 18827
TC DL	10.0 PSF	DATE	12/26/07
BC DL	10.0 PSF	DRW	HCUSR8228 07360004
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	67299
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TDH8228Z01

(7-380--GARY JOHNSON Hampton -- , ** - AA-GE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3 :W6 2x6 SP #2:
:W9, W23 2x4 SP #2 Dense:

Wind reactions based on MWFRS pressures.

Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

+ MEMBER TO BE LATERALLY BRACED FOR WIND LOADS PERPENDICULAR TO TRUSS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

SEE DRW HCUSR001 02086012 FOR GABLE DETAILS.

(B) (1) 2X4X FULL LENGTH SP#2 DENSE SCAB; ATTACH TO ONE FACE OF TRUSS WITH 10d BOX(0.128"X3.0") NAILS @ 4" OC WITHOUT SPLITTING LUMBER.

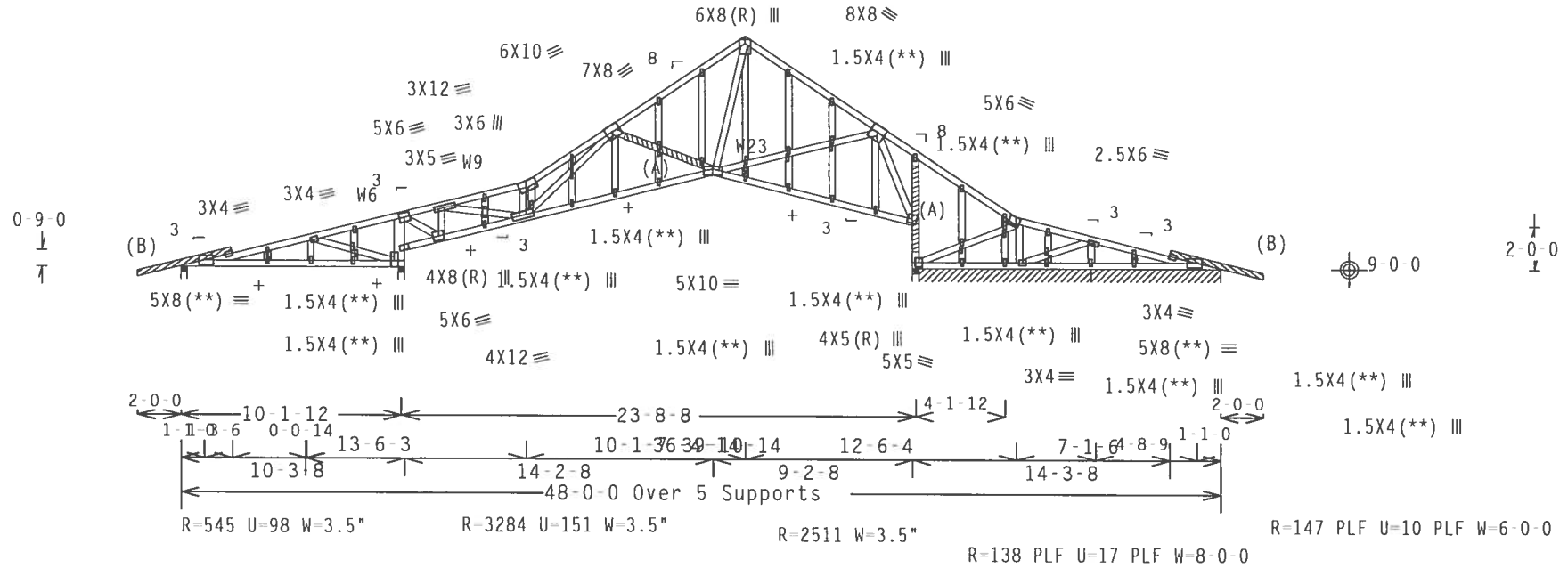
(**) 14 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

The building designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be designed by the building designer.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.37.052[illegible] QTY:1 FL/-/4/-/-/R/- Scale =.125"/Ft.

ALPINE

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FL Certificate of Authorization # 0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER [illegible] 26 '07

TC LL	20.0 PSF	REF	R8228- 18828
TC DL	10.0 PSF	DATE	12/26/07
BC DL	10.0 PSF	DRW	HCUSR8228 07360005
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	8871 REV
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TDH8228Z01

(7 380 GARY JOHNSON Hampton , ** A1)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

(A) Continuous lateral bracing equally spaced on member.

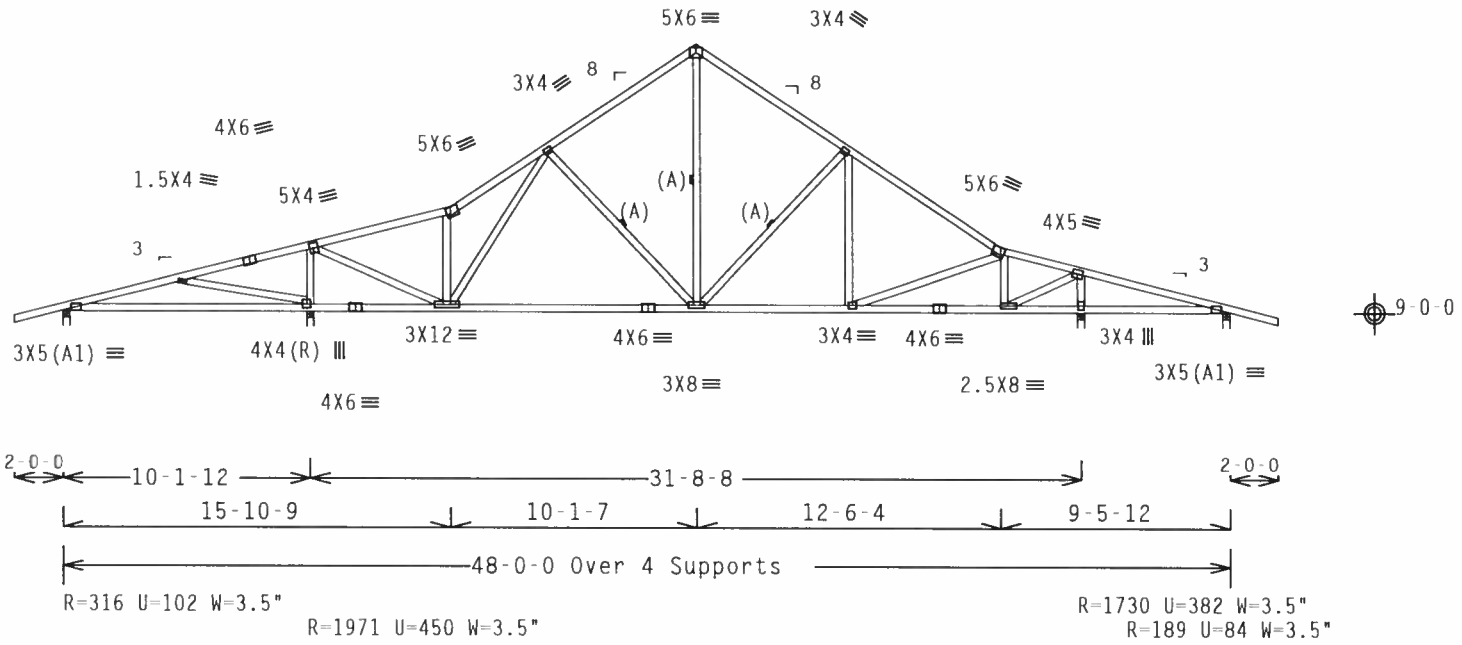
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

TRUSS MAY NOT BE INSTALLED END FOR END.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.04 QTY:1 FL/-/4/-/-/R/- Scale =.125"/Ft.

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ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 279

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

26 '07

TC LL	20.0 PSF	REF	R8228- 18829
TC DL	10.0 PSF	DATE	12/26/07
BC DL	10.0 PSF	DRW	HCUSR8228 07360006
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	67367
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JRFF-	1TDH8228Z01

(7 380 GARY JOHNSON Hampton , ** B1)

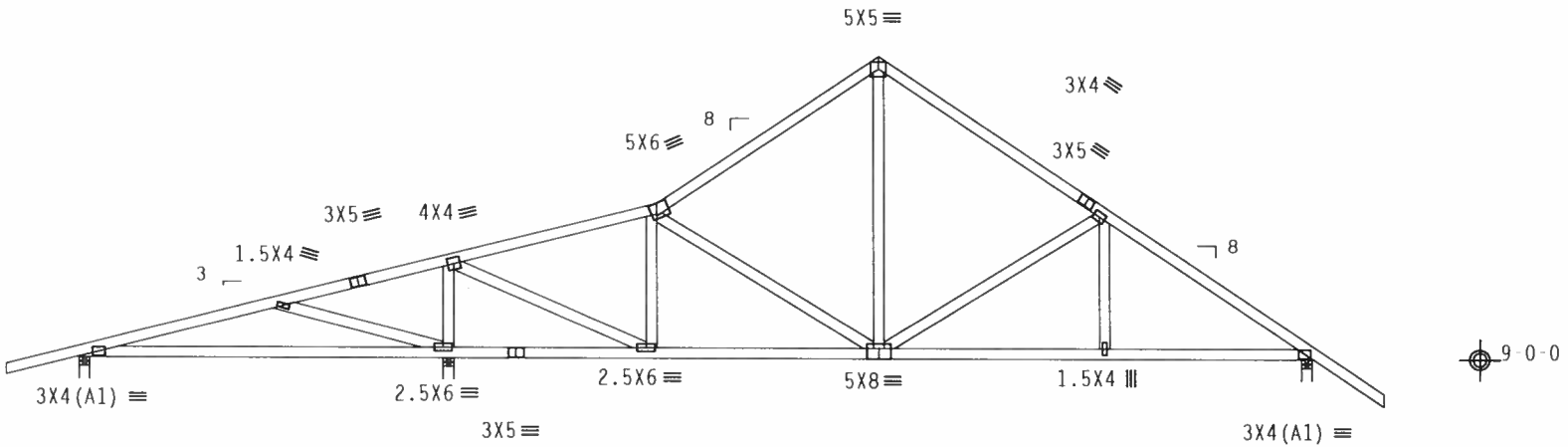
THIS END PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

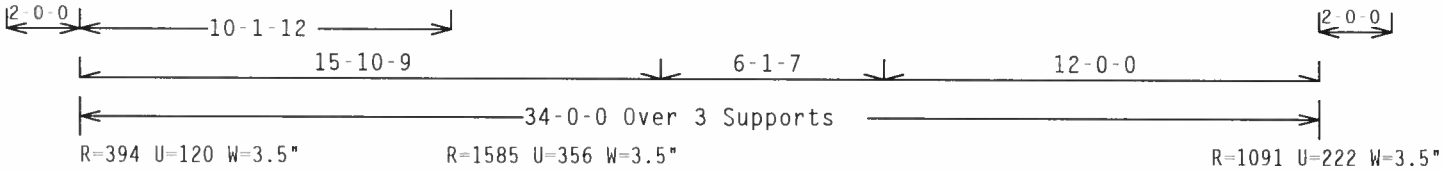
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.





Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/0(0) 7.36.04 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

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ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 270

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

DEC 26 '07

TC LL	20.0 PSF	REF	R8228- 18830
TC DL	10.0 PSF	DATE	12/26/07
BC DL	10.0 PSF	DRW	HCUSR8228 07360009
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	67148
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JRFF-	1TDH8228Z01

(7 380 GARY JOHNSON Hampton , ** B GE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP #2 Dense:

Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

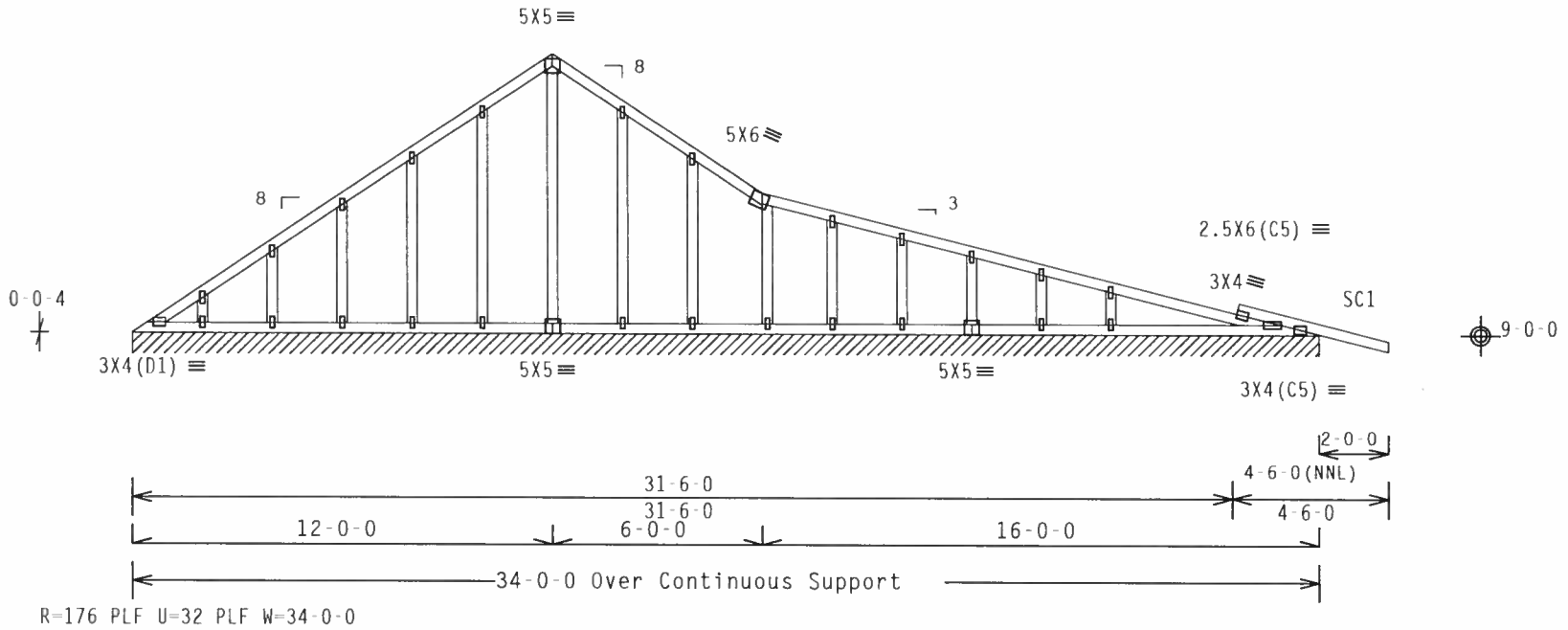
SEE DRW HCUSR001 02086015 FOR GABLE DETAILS.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The building designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be designed by the building designer.



R=176 PLF U=32 PLF W=34-0-0

Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.042 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 279

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
DEC 26 '07

TC LL	20.0 PSF	REF R8228- 18831
TC DL	10.0 PSF	DATE 12/26/07
BC DL	10.0 PSF	DRW HCUSR8228 07360007
BC LL	0.0 PSF	HC-ENG JB/DF
TOT.LD.	40.0 PSF	SEQN- 67390
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TDH8228Z01

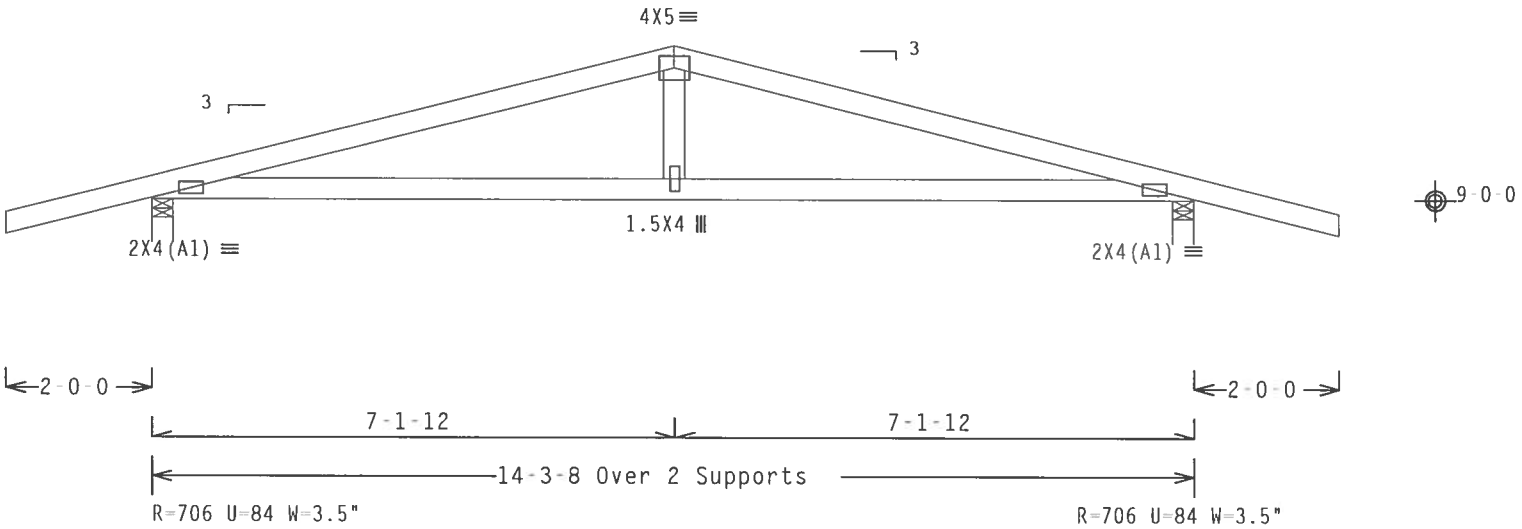
(7-380--GARY JOHNSON Hampton -- , ** - C1)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/0(0) 7.36.04 [illegible] QTY:1 FL/-/4/-/-/R/- Scale =.375"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 279

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
[illegible] 26 '07

TC LL	20.0 PSF	REF	R8228-18832
TC DL	10.0 PSF	DATE	12/26/07
BC DL	10.0 PSF	DRW	HCUSR8228 07360008
BC LL	0.0 PSF	HC-ENG	JB/DF *
TOT.LD.	40.0 PSF	SEQN-	67134
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TDH8228Z01

(7 380 GARY JOHNSON Hampton , ** C GE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP #2 Dense:
:Stack Chord SC2 2x4 SP #2 Dense:

Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

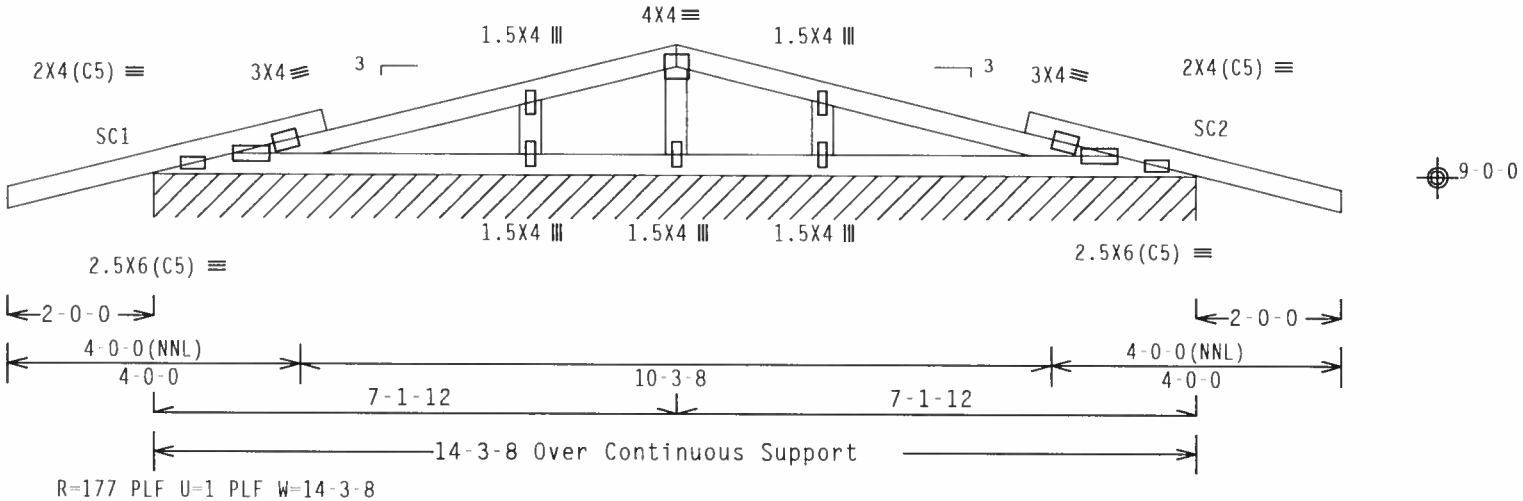
The building designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be designed by the building designer.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

See DWGS A11015EE0207 & GBLLETIN0207 for more requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.04 QTY:1 FL/-/4/-/-/R/- Scale =.375"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

DEC 26 '07

TC LL	20.0 PSF	REF	R8228- 18833
TC DL	10.0 PSF	DATE	12/26/07
BC DL	10.0 PSF	DRW	HCUSR8228 07360001
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	67167
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TDH8228Z01

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

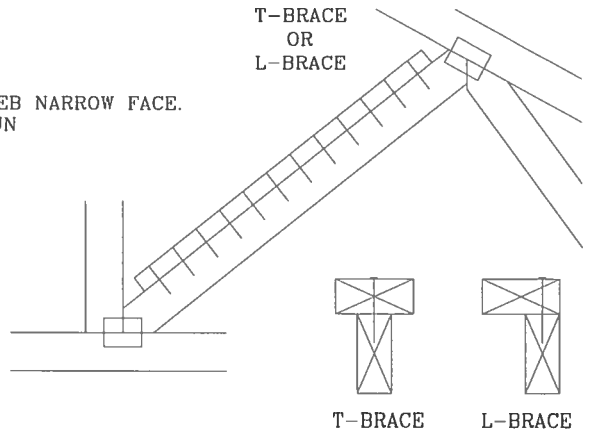
WEB MEMBER SIZE	SPECIFIED CLB BRACING	ALTERNATIVE BRACING T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

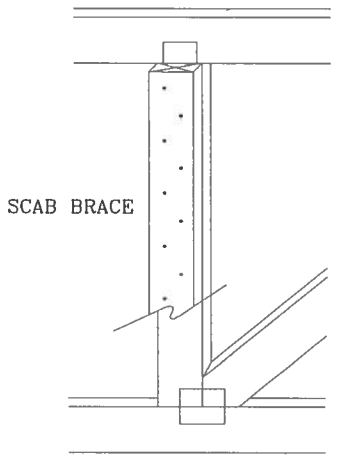
T-BRACING OR L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE. ATTACH WITH 10d BOX OR GUN (0.128"x 3.",MIN) NAILS. AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB. NO MORE THAN (1) SCAB PER FACE. ATTACH WITH 10d BOX OR GUN (0.128"x 3.",MIN) NAILS. AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



THIS DRAWING REPLACES DRAWING 579,640

ALPINE
ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

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DOUGLAS FLEMING LICENSE No. 66648 DEC 26 '07 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	PSF	REF CLB SUBST.
TC DL	PSF	DATE 2/23/07
BC DL	PSF	DRWG BRCLBSUB0207
BC LL	PSF	-ENG MLH/KAR
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

ASCE 7-02: 110 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 3"	12' 3"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	5' 2"	6' 9"	6' 9"	9' 1"	9' 1"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 0"	9' 5"	9' 11"	12' 5"	12' 6"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
16" O.C.	SPF HF	#1 / #2	4' 5"	7' 8"	7' 10"	9' 1"	9' 4"	10' 10"	11' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 4"	6' 4"	6' 4"	8' 4"	8' 4"	10' 10"	10' 10"	12' 11"	12' 11"	14' 0"	14' 0"
	SP DFL	#1	4' 10"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 9"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 6"	7' 7"	7' 7"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 6"	7' 6"	7' 6"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 5"	6' 5"	6' 5"	8' 6"	8' 6"	10' 10"	11' 1"	13' 3"	13' 3"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 11"	8' 5"	8' 8"	10' 0"	10' 3"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 9"	7' 3"	7' 3"	9' 7"	9' 7"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 4"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 3"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 0"	8' 5"	8' 5"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	5' 0"	8' 5"	8' 7"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 11"	7' 5"	7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

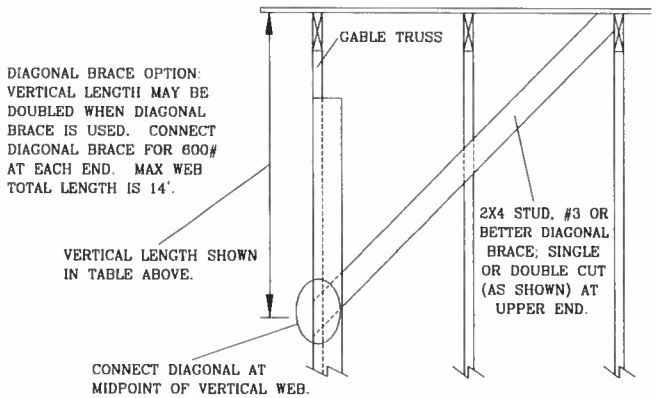
SPRUCE-PINE-FIR		HEM-FIR	
#1 / #2	STANDARD	#2	STUD
#3	STUD	#3	STANDARD

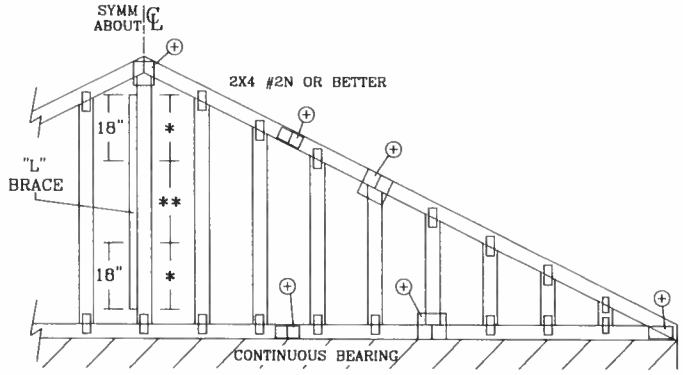
DOUGLAS FIR-LARCH	SOUTHERN PINE
#3	#3
STUD	STUD
STANDARD	STANDARD

GROUP B:

HEM-FIR	
#1 & BTR	
#1	

SOUTHERN PINE	DOUGLAS FIR-LARCH
#1	#1
#2	#2





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

ALPINE

ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

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IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITW BCG, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI 1. ITW, BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W,K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

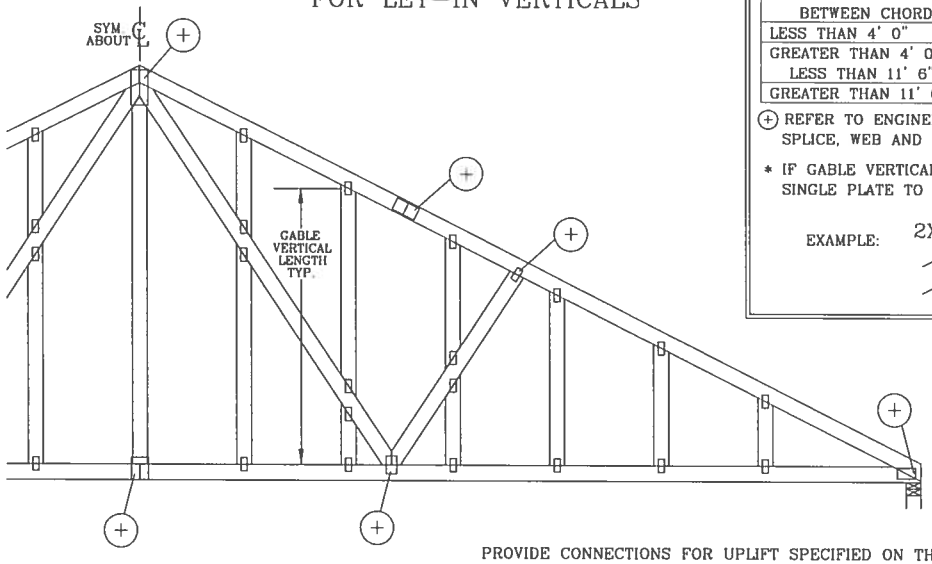
Dec 26 '07

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-02-GAB11015
DATE	2/23/07
DRWG	A11015EE0207
-ENG	

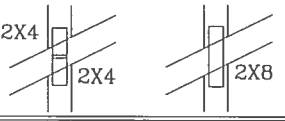
GABLE DETAIL
FOR LET-IN VERTICALS

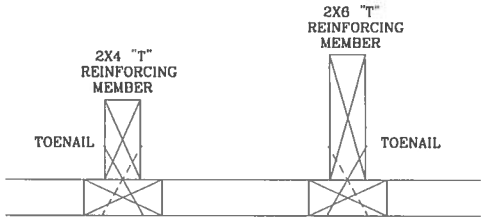


GABLE VERTICAL PLATE SIZES		
VERTICAL LENGTH BETWEEN CHORDS	PLATE SIZE	IF PLATES OVERLAP*
LESS THAN 4' 0"	1X4 OR 2X3	2X8
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4	2X8
GREATER THAN 11' 6"	2.5X4	2.5X8

⊕ REFER TO ENGINEERED TRUSS DESIGN FOR PEAK, SPLICE, WEB AND HEEL PLATES.

* IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE TO SPAN THE WEB.

EXAMPLE: 



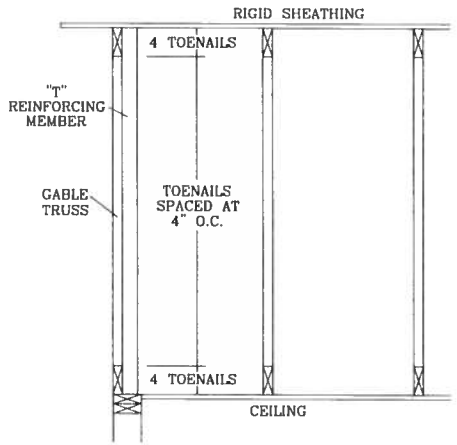
TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" FACTOR BY LENGTH (BASED ON GABLE VERTICAL SPECIES, GRADE AND SPACING) FOR (1) 2X4 "L" BRACE, GROUP A, OBTAINED FROM THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WEB LENGTH INCREASE W/ "T" BRACE

WIND SPEED AND MRH	"T" REINF. MBR. SIZE	SBCCI	ASCE
110 MPH	2x4	10 %	10 %
15 FT	2x6	40 %	50 %
110 MPH	2x4	10 %	10 %
30 FT	2x6	50 %	50 %
100 MPH	2x4	10 %	10 %
15 FT	2x6	30 %	50 %
100 MPH	2x4	10 %	10 %
30 FT	2x6	40 %	40 %
90 MPH	2x4	20 %	10 %
15 FT	2x6	20 %	40 %
90 MPH	2x4	10 %	10 %
30 FT	2x6	30 %	50 %
80 MPH	2x4	10 %	20 %
15 FT	2x6	10 %	30 %
80 MPH	2x4	20 %	10 %
30 FT	2x6	20 %	40 %
70 MPH	2x4	0 %	20 %
15 FT	2x6	0 %	20 %
70 MPH	2x4	10 %	20 %
30 FT	2x6	10 %	30 %

EXAMPLE:
ASCE WIND SPEED = 100 MPH
MEAN ROOF HEIGHT = 30 FT
GABLE VERTICAL = 24" O.C. SP #3
"T" REINFORCING MEMBER SIZE = 2X4
"T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10
(1) 2X4 "L" BRACE LENGTH = 6' 7"
MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH
1.10 x 6' 7" = 7' 3"



PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN.

ATTACH EACH "T" REINFORCING MEMBER WITH
HAND DRIVEN NAILS:
10d COMMON (0.148"X 3.",MIN) TOENAILS AT 4" O.C. PLUS
(4) 16d COMMON (0.162" X 3.5",MIN) TOENAILS IN TOP AND BOTTOM CHORD.
GUN DRIVEN NAILS:
8d COMMON (0.131"X 2.5",MIN) TOENAILS AT 4" O.C. PLUS
(4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.
ASCE 7-93 GABLE DETAIL DRAWINGS
A11015EN0207, A10015EN0207, A09015EN0207, A08015EN0207, A07015EN0207, A11030EN0207, A10030EN0207, A09030EN0207, A08030EN0207, A07030EN0207
ASCE 7-98 GABLE DETAIL DRAWINGS
A13015EC0207, A12015EC0207, A11015EC0207, A10015EC0207, A08515EC0207, A13030EC0207, A12030EC0207, A11030EC0207, A10030EC0207, A08530EC0207
ASCE 7-02 GABLE DETAIL DRAWINGS
A13015EE0207, A12015EE0207, A11015EE0207, A10015EE0207, A08515EE0207, A13030EE0207, A12030EE0207, A11030EE0207, A10030EE0207, A08530EE0207
ASCE 7-05 GABLE DETAIL DRAWINGS
A13015E50207, A12015E50207, A11015E50207, A10015E50207, A08515E50207, A13030E50207, A12030E50207, A11030E50207, A10030E50207, A08530E50207

SEE APPROPRIATE ALPINE GABLE DETAIL (ASCE OR SBCCI WIND LOAD) FOR MAXIMUM UNREINFORCED GABLE VERTICAL LENGTH.

THIS DRAWING REPLACES DRAWINGS GAB98117 876,719 & HC26294035

ALPINE
ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

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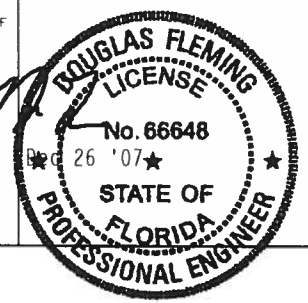


MAX TOT. LD. 60 PSF
DUR. FAC. ANY
MAX SPACING 24.0"

REF	LET-IN VERT
DATE	2/23/07
DRWG	GBLLETIN0207
-ENG	DLJ/KAR

(ASCE 7 98 GABLE END DETAILS ,140 MPH,EXP C, PART. ENC)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(**) 2X4 SO. PINE #3 GABLE STUDS. ATTACH TO TOP CHORD, DIAGONAL WEBS, AND BOTTOM CHORD WITH W2X4 ALPINE PLATES. ALL (**) GABLE STUDS REQUIRED REINFORCING MEMBER. REINFORCING MEMBER MUST BE TOENAILED TO GABLE STUD WITH 0.131"X3" GUN NAILS AT 4" O.C. PLUS A CLUSTER OF 0.131"X3" TOENAILS AT THE TOP AND BOTTOM CHORD.SEE DETAIL FOR NAILING. SEE CHART FOR STUD BRACING AND SPACING OF VERTICALS.

NOTE:TRUSS ERECTOR IS RESPONSIBLE FOR PERMANENT WEB BRACING. WHEN BRACING IS REQUIRED, FURNISH A COPY OF THIS DRAWING TO TRUSS ERECTOR.

+PLATE AS REQUIRED ON APPROPRIATE DRAWING.

IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO DESIGN THE ROOF AND CEILING DIAPHRAGMS AND SPECIFY CONNECTIONS TO TRANSFER ALL OUT-OF-PLANE LOADS INTO THE ROOF AND CEILING DIAPHRAGMS.

NOTE: NAIL STEPS OF LADDER TRUSS ONTO THE OUTSIDE PIECES WITH 2-16D NAILS AT EACH END.

NOTE: ATTACH LADDER TRUSS TO TOP CHORD OF GABLE TRUSS WITH TWO ROWS OF 16D NAILS @ 8" O.C. STAGGERED 4"

+++ 7/16 MINIMUM APA RATED SHEATHING PROPERLY ATTACHED WITH LONG DIMENSION PERPENDICULAR TO SUPPORTS.

140 MPH WIND, 30.00 FT MEAN HGT, ASCE 7-98, PART. ENCLOSED BLDG, CAT II, EXP. C.

140 MPH WIND, 30.00 FT MEAN HGT, ASCE 7-02, PART. ENCLOSED BLDG, CAT II, EXP. C.

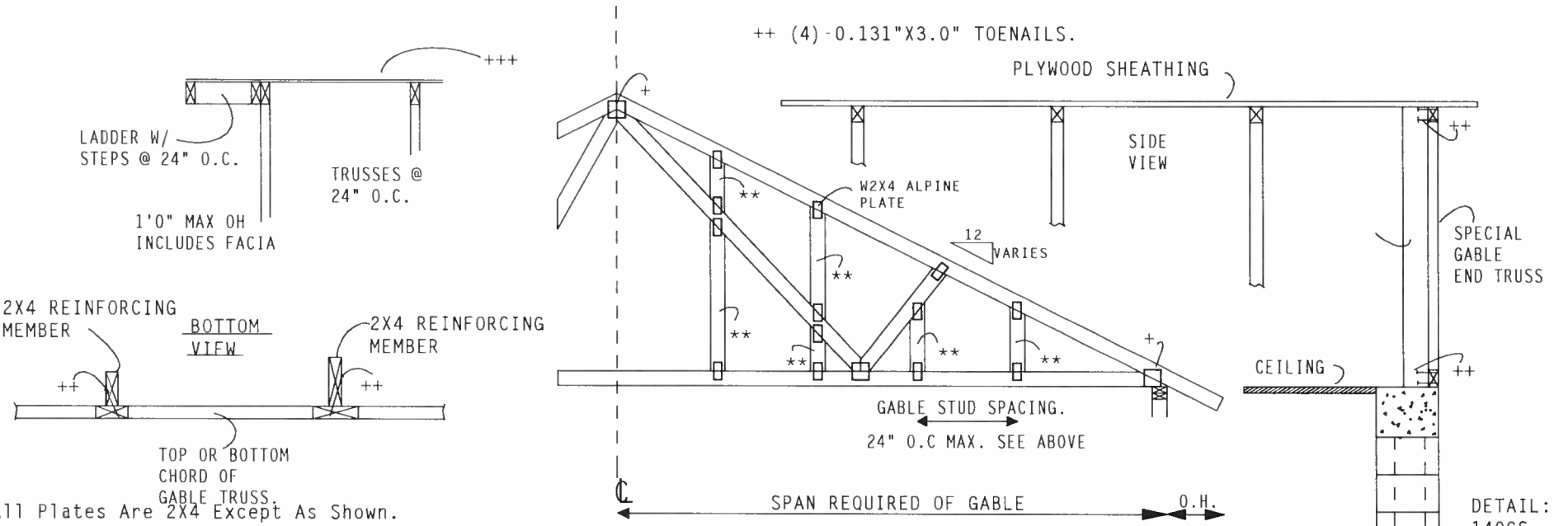
SEE APPROPRIATE ALPINE DRAWING FOR LUMBER,PLATES AND OTHER DATA NOT SHOWN HERE.

** STUD MUST BE ATTACHED TO CHORDS AND DIAGONAL

REINFORCING MEMBER REQUIRED	SPACING	MAX.LENGTH
2X4 SO. PINE #3	24"O.C.	2'- 1"
2X4 SO. PINE #3	16"O.C.	2'-10"
2X4 SO. PINE #3	12"O.C.	3'- 5"
2X6 SO. PINE #2 N	24"O.C.	5'- 0"
2X6 SO. PINE #2 N	16"O.C.	6'- 2"
2X6 SO. PINE #2 N	12"O.C.	7'- 1"
2X8 SO. PINE #2 N	24"O.C.	7'- 6"
2X8 SO. PINE #2 N	16"O.C.	9'- 1"
2X8 SO. PINE #2 N	12"O.C.	10'- 4"

R2: REVISED FOR ASCE 7-02
DLJ 09/30/2005

R3: REVISED DIAPHRAGM NOTE.
DLJ 02/27/2006



Note: All Plates Are 2X4 Except As Shown.

DETAIL: 140GS

PLT TYP. Wave TPI-95

Design Criteria: TPI(STD)

REVISED 2002 JWC

QTY:1 HI/-/1/-/-/R/-

Scale =.3125"/Ft.

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
Certificate of Authorization # 567

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER Dec 26, 06

TC LL	30.0 PSF	REF R001-- 0
TC DL	15.0 PSF	DATE 03/27/02
BC DL	10.0 PSF	DRW HCUSR001 02086012
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	55.0 PSF	SEQN - 24104
DUR.FAC.	1.33	FROM HC
		JREF- 1SV3001 R03

(Copy ASCE 7 98 DETAILS GABLE)

140 MPH WIND, 30.0 FT MEAN HGT, ASCE 7-98, PART. ENC. BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TCDL=5.0 PSF, WIND BCDL=5.0 PSF.

140 MPH WIND, 30.0 FT MEAN HGT, ASCE 7-02, PART. ENC. BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TCDL=5.0 PSF, WIND BCDL=5.0 PSF.

+ FOR VERTICAL WEBS LESS THAN 4'0": W1X4
FOR VERTICAL WEBS GREATER THAN 4'0" BUT NO MORE THAN 11'6": W2X4.

* SPLICE, PEAK, AND HEEL PLATES TO MATCH COMMON TRUSS.

** 2X4 OR GREATER CHORDS.

DROP GABLE WILL SUPPORT 4'0" OUTLOOKERS WITH 2'0" OVERHANG (DROP HEEL GABLE) SPACED 24" O.C., OR THE LOAD FROM 12" PLYWOOD OVERHANG (NOMINAL HEEL GABLE).

IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO DESIGN THE ROOF AND CEILING DIAPHRAGMS AND SPECIFY CONNECTIONS TO TRANSFER ALL OUT-OF-PLANE LOADS INTO THE ROOF AND CEILING DIAPHRAGMS.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE GABLE SHEAR WALL DESIGN, CEILING AND ROOF SHEATHING DIAPHRAGM CONNECTIONS, AND ALL TRUSS TO WALL CONNECTIONS.

++ 7/16 MINIMUM APA RATED SHEATHING PROPERLY ATTACHED WITH LONG DIMENSION PERPENDICULAR TO SUPPORTS.

R1 NOTE: NAIL STEPS OF LADDER TRUSS ONTO THE OUTSIDE PIECES WITH 2-16D NAILS AT EACH END.

R1 NOTE: ATTACH LADDER TRUSS TO TOP CHORD OF GABLE TRUSS WITH TWO ROWS OF 16D NAILS @ 8" O.C. STAGGERED 4"

ALT. GABLE SHAPES:

BRACING DEFINITIONS:
NOTE: "END ZONE" EXISTS 18" AT BOTH ENDS OF VERTICAL WEB.

(A) (1) 2X4 SP #3 "L" BRACE. ATTACH WITH 0.128"X3" NAILS @ 2" OC. IN END ZONES; 4" OC. BETWEEN ZONES.

(B) (2) 2X4 SP #3 "L" BRACES. ATTACH EACH WITH 0.128"X3" NAILS @ 3" OC. IN END ZONES; 6" OC. BETWEEN ZONES.

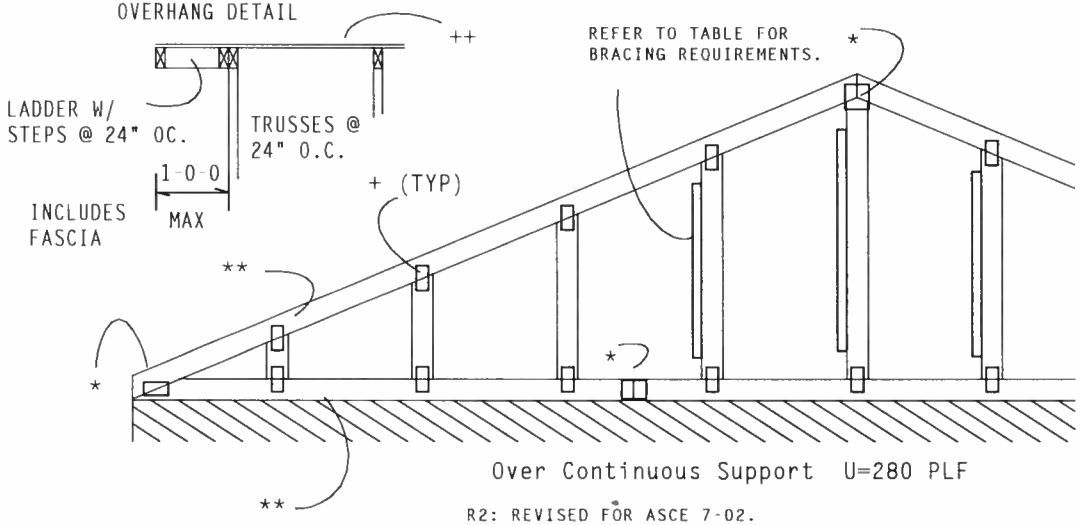
(C) (1) 2X6 SP #2 N "L" BRACE. ATTACH WITH 0.128"X3" NAILS @ 2" OC. IN END ZONES; 4" OC. BETWEEN ZONES.

(D) (2) 2X6 SP #2 N "L" BRACES. ATTACH EACH WITH 0.128"X3" NAILS @ 3" OC. IN END ZONES; 6" OC. BETWEEN ZONES.

STUD SPACING / BRACING TABLE:

2X4 SP #3 STUD SPACING	DEFLECTION CRITERIA	NO BRACE	(1) 2X4 "L" BRACE TYPE (A)	(2) 2X4 "L" BRACE TYPE (B)	(1) 2X6 "L" BRACE TYPE (C)	(2) 2X6 "L" BRACE TYPE (D)
24"	L/360	-----	3' 1"	4' 2"	6' 3"	8' 0"
24"	L/180	-----	3' 4"	5' 7"	6' 3"	11' 0"
16"	L/360	-----	3'11"	5' 3"	7'10"	9'11"
16"	L/180	-----	4' 9"	7' 4"	9' 6"	11' 0"
12"	L/360	-----	4' 7"	6' 1"	8'11"	11' 0"
12"	L/180	-----	5'11"	8' 5"	11' 0"	11' 0"

OVERHANG DETAIL



Note: All Plates Are 2X4 Except As Shown.

PLT TYP. Wave TPI-95 Design Crit: TPI-1995(STD)

R3: REVISED DIAPHRAGM NOTE.
DLJ 02/27/200G

R2: REVISED FOR ASCE 7-02.
DLJ 09/30/2005
R1 REV 2-5-02 JWC
HI/-/1/-/-/R/-

DETAIL: 140GC
Scale =.375"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER Dec. 26, 20

TC LL	30.0 PSF	REF	R001-- 0
TC DL	7.0 PSF	DATE	03/27/02
BC DL	10.0 PSF	DRW	HCUSR001 02086015
BC LL	0.0 PSF	HC-ENG	DLJ/DLJ
TOT.LD.	47.0 PSF	SEQN-	24860
DUR.FAC.	1.33		
SPACING	24.0"	JREF-	1SV3001 R03

CERTIFICATE OF

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 17-5S-16-03641-000 **Building permit No.** 000026569

Use Classification SFD/UTILITY **Fire:** 36.63

Permit Holder JACK S.HAMPTON **Waste:** 50.25

Owner of Building JACK HAMPTON **Total:** 86.88

Location: 1189 SW CARPENTER RD.,LAKE CITY, FL

Date: 07/30/2008

Wayne H. Rust

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)