Hampton Residence, Columbia County FL Addendum for Wind Load Analysis Requirements

(In Compliance with the 2004 Florida Building Code and Amendments)

Prepared By: Marty J. Humphries, P.E. # 51976 7932 240th St., O'Brien, FL 32071 (386)935-2406

Additional Foundation Reanalysis\Requirements:

The stem-wall geometry and longitudinal steel requirements have been reanalyzed for this project. After re-evaluating the structural capacity of the footer for this project the requirement of 3-#5 rebar continuous may be reduced to 2-#5 rebar continuous in the footer. All other requirements shall remain in effect.

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Location:

Project name:

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and the product approval number(s) on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit on or after April 1, 2004. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at wood increasuration are

Category/Subcategory	Manufacturer	Product Description	Approval Number(
A. EXTERIOR DOORS	25.2.24		FL 4242-R
1. Swinging			
2. Sliding			
3. Sectional			
4. Roll up			
5. Automatic			
6. Other			
B. WINDOWS			
1. Single hung			FC 5100
2. Horizontal Slider			FL. 5451
3. Casement			*
4. Double Hung			
5. Fixed			FL 5418
6. Awning			
7. Pass -through	2000		
8. Projected			
9. Mullion			
10. Wind Breaker			
11 Dual Action			
12. Other			
C. PANEL WALL			
1. Siding			FL. 889-22
2. Soffits			FL 4899
3. EIFS		Vind Siden D5	FL 4905
4. Storefronts			
5. Curtain walls			
6. Wall louver			
7. Glass block			FL 3820-R1
8. Membrane			
9 Greenhouse		·	
10. Other			
. ROOFING PRODUCTS	***********		
Asphalt Shingles			FL 586-RZ
2. Underlayments			FL 1814-21
3. Roofing Fasteners			
4. Non-structural Metal R	f		
5. Built-Up Roofing			
6. Modified Bitumen	i i		
7. Single Ply Roofing Sys.			
8. Roofing Tiles		*************************************	7
9. Roofing Insulation			
10. Waterproofing			
11. Wood shingles /shake:			
12. Roofing Slate			

Category/Subcategory (cont. 13. Liquid Applied Roof Sys 14. Cements-Adhesives - Coatings 15. Roof Tile Adhesive 16. Spray Applied			FL. 1960-R1
14. Cements-Adhesives - Coatings 15. Roof Tile Adhesive 16. Spray Applied			FL. 1960-121
Coatings 15. Roof Tile Adhesive 16. Spray Applied			
15. Roof Tile Adhesive			
16 Spray Applied			
Dallingthan Doof			
Polyurethane Roof			
17. Other			
E. SHUTTERS	<u> </u>		
1. Accordion			
2. Bahama			
3. Storm Panels			
4. Colonial	000-000-000		
5. Roll-up			
6. Equipment			
7. Others			
SKYLIGHTS	+		F 4451 - R
Skylight Other			
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3. STRUCTURAL			
COMPONENTS			72.474-R1
 Wood connector/ancho 	<u></u>		
2. Truss plates			FL1008121
3. Engineered lumber			10,000
4. Railing			
5. Coolers-freezers			
6. Concrete Admixtures			
7. Material			
8. Insulation Forms			
9. Plastics			
10. Deck-Roof			
11. Wall			
12. Sheds			
13. Other			
H. NEW EXTERIOR			
ENVELOPE PRODUCTS			_
1.	- -		
ime of inspection of these obsite; 1) copy of the produ and certified to comply with,	products, the fol uct approval, 2) t , 3) copy of the a	ate product approval at plan revie llowing information must be availa the performance characteristics w applicable manufacturers installation to removed if approval cannot be control of the control of	thich the product was tested ion requirements.
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Samples of Controlled Authorize	ed Agent Signature	Print Name	Date
Contractor or Contractor's Authorize	ed Agent Signature	Print Name	Date

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID:1TDH8228Z0126091007

Truss Fabricator: Anderson Truss Company

Job Identification: 7-380--GARY JOHNSON Hampton -- , **

Truss Count: 9

Model Code: Florida Building Code 2004 and 2006 Supplement

Truss Criteria: ANSI/TPI-2002(STD)/FBC

Engineering Software: Alpine Software, Versions 7.36, 7.37.

Structural Engineer of Record: The identity of the structural EOR did not exist as of

Address: the seal date per section 61G15-31.003(5a) of the FAC

Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration

Floor - N/A

Wind - 110 MPH ASCE 7-02 -Closed

Notes:

 Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1

2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.

3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

Details: 140GS-BRCLBSUB-140GC-A11015EE-GBLLETIN-

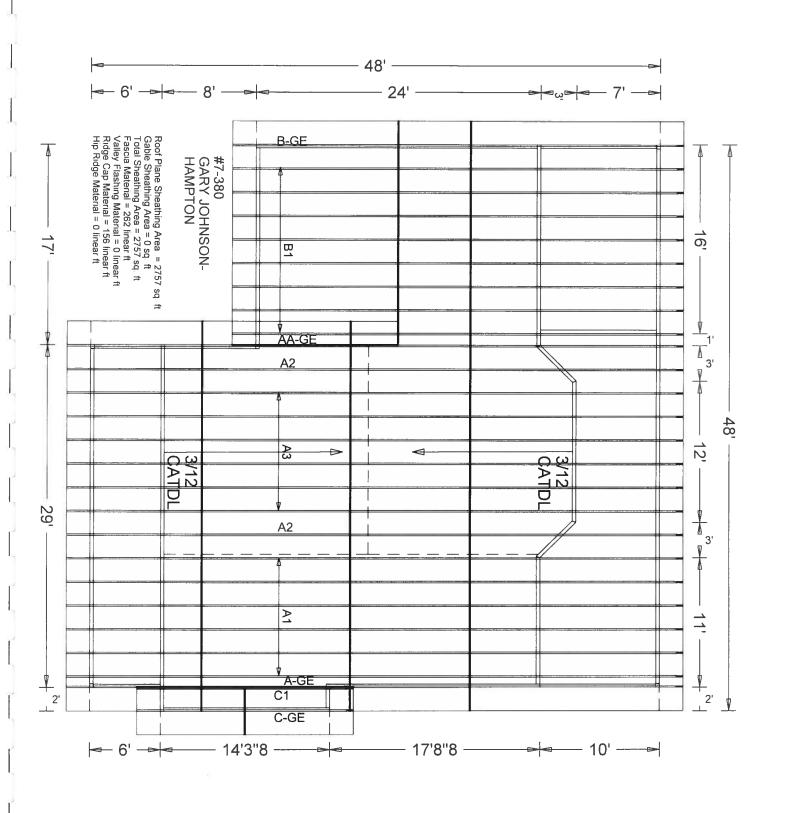
#	Ref Description	Drawing#	Date
1	18825 A - GE	07360002	12/26/07
2	18826A3	07360003	12/26/07
3	18827 A2	07360004	12/26/07
4	18828 AA - GE	07360005	12/26/07
5	18829 A1	07360006	12/26/07
6	18830 B1	07360009	12/26/07
7	18831B-GE	07360007	12/26/07
8	18832C1	07360008	12/26/07
9	18833C-GE	07360001	12/26/07

97

Seal Date: 12/26/2007

-Truss Design Engineer-Doug Fleming Florida License Number: 66648 1950 Marley Drive Haines City, FL 33844





PAGE NO:	JOB NO: 7-380	JOB DESCRIPTION:: GARY JOHNSON /: Hampton	
111			

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP #2 D
:Stack Chord SC2 2x4 SP #2 D Dense: Dense:

Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

member. (A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3",min.)nails @ 6"

00

contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below. WARNING: Furnish a copy of this DWG to the installation

(**) 7 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

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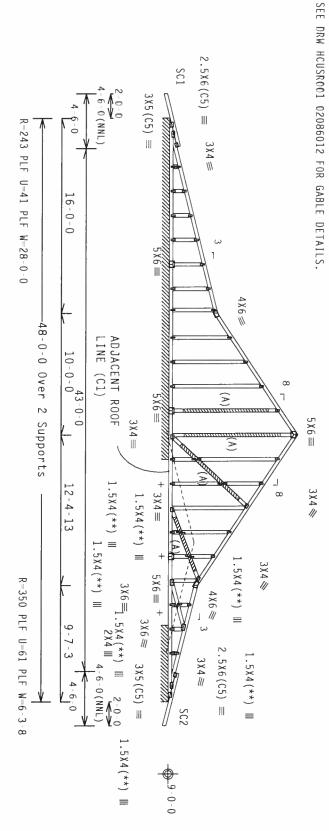
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MEMBER TO BE LATERALLY BRACED FOR WIND PERPENDICULAR TO TRUSS. BRACING SYSTEM DESIGNED AND FURNISHED BY OTHERS LOADS

designed by the building The building designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be designed by the building designer.



Note: All Plates Are 1.5X4 Except As Shown. Design Crit:

TYP.

Wave

A PROPERLY ATTACHED RIGID CEILING TPI-2002 (STD) /FBC Cq/RT=1.00(1.25) /0(0)

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY OUVLATION FROM THIS DESIGN: ANY FAILURE TO BUILD THE REUSE IN COMPORMANCE WITH FPI; OR FABRICATION, HANDLING, SHIPPIDE, HISTALLING A BRACHER OF FRUSSES. DESIGN ACTION, DESIGN CONTROLLING, SHEPPIDE, HISTALLING A BRACHER OF FRUSSES. ATRAYA AND FPI.

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Haines City, FL 33844
FL Certificate of Anthorization # 0 778

ITW Building Components Group, Inc.

ALPINE

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PSF PSF

DATE REF

12/26/07

Scale

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R8228-

18825

10.0 PSF

DRW HCUSR8228 07360002

0.0 PSF

HC-ENG

JB/DF

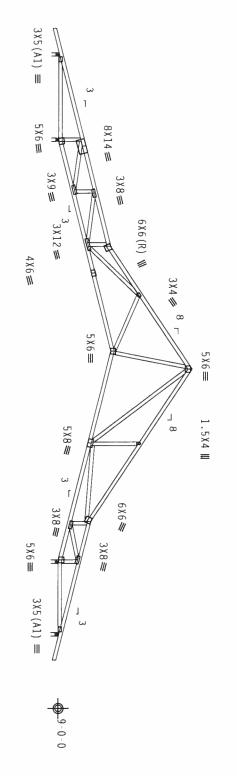
PSF

SEQN-

67215

24.0" 1.25 JREF-FROM 1TDH8228Z01

Top chord 2x4 SP # Bot chord 2x4 SP # Webs 2x4 SP # Deflection factor for WARNING: Furnish a copy of this DNG to the installation contractor. Special care must be taken during handling, and installation of trusses. See "WARNING" note below. 380 GARY JOHNSON Hampton meets L/240 live and L/180 total load. Creep increase dead load is 1.50. #2 Dense #2 Dense #3 A3) shipping 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, I located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55 Wind reactions based on MWFRS pressures. Negative reaction(s) of -223# MAX. (S load case requires uplift connection. 223# MAX. (See below) from a non-wind





R = -

-98 Rw=41 U=172 W=3.5"

WARNING IRUSSES REQUIRE EXTREME CARE IN FARRICATION. MANDEING. SHIPPING, INSTALLING AND BRACING. RECER TO BEST (BUILDING COMPONENT SAFETY INFORMATION). PURLISING DRY FIT (1805S PLATE INSTITUTE, 218 MORTH LEE STREET, SUITE 3175, ALEXANDRIA, VA. 22314) AND MICA, 1400D TRUSS. COUNCIL OF AMERICA. 6300 ENTIFERPRISE LANE, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERMISE INFORMATION FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS A PROPERTY ATTACHED TO COROBO SHALL HAVE PROPERTY ATTACHED STRUCTURAL PARILES AND BOTTOM CHORD SHALL HAVE Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0) CENS

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IMPORTANT*URNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE RUSS IN CONTORNANCE WITH IP) OR FARELAND LING. SHAPLING, SHAP

ITW Building Components Group, Inc. Haines City, FL 33844 FL Cartificate of Authorization # 0 778

DESIGN SHOWN. THE SUITABILITY AND USE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

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12/26/07

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HC-ENG

JB/DF 67285

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Top chord 2x4 SP / Bot chord 2x4 SP / Webs 2x4 SP / ITW Building Components Group, Inc. Haines City, FL 33844 FL Cartificate of Authorization # 0 270 PLT Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. (7-380--GARY JOHNSON Hampton TYP. ALPINE Wave 10 $3X5(A1) \equiv$ #2 Dense #2 Dense #3 138 8-4-15 φ **IMPORTANT**GURMISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BCR RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE FURSES IN COMPORMANCE HITH TPI; OR FARRICATING, HANDILGE, SHIPPIDL, HISTALLING, BRACHIC OF TRUSSES.

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THE SHIPPING SHIPPING SHOWN SHIPPING THE SHOWN SHIPPING IS THE RESPONSIBILITY OF THE DESIGN SHIPPING IN THE SHIPPING SHIP 1.5X4 DESIGN SHOWN. THE SUTTABILITY AND USE OF THIS BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2. A PROPERLY ATTACHED RIGID CEILING Rw=20 U=190 W=3.5' 2 5X8(R) Ⅲ 15-10-9 4 X 6 ≡ 4 X 5 ≡ A2) 3×9≡ Design Crit: 6X6(R) ₩ 3 X 1 2 ≡ ω 16 - 1R=2383 U=504 W=4.938" 4×6≡ 18-0-0 Over 4 Supports 3×4 // TPI-2002 (STD) /FBC Cq/RT=1.00(1.25) /0(0) 10 - 1 - 7œ 33-7-12 $5 \times 6 =$ 5 X 6 ≡ 1.5X4 | 5X8# 2 6 WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below. 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55 Wind reactions based on MWFRS pressures. 2-8 4 R=2031 U=434 W=3.5" w 36. DOUGHAS FLE .04 6X6₩ 3X8# No. 66648 3 X 8 # 5 X 6 **≡** 9 ŝ 6-3-8 -12 $3X5(A1) \equiv$ 07 2-0-0 V R_{\parallel} 50 Rw=40 U=158 W=3.5" SPACING BC LL BC DL TC DL DUR.FAC. TC TOT.LD. FL/-/4/-/-/R/-10.0 40.0 20.0 0-0-24.0" 10.0 PSF 1.25 0.0 PSF PSF PSF PSF JREF-SEQN DATE REF FROM DRW HCUSR8228 07360004 HC-ENG Scale R8228- 18827 1TDH8228Z01 =.125"/Ft. JB/DF 67299 12/26/07

(7-380--GARY JOHNSON Hampton AA-GE)

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Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 :W6 2x6 SP :W9, W23 2x4 SP #2 Dense: #2:

Wind reactions based on MWFRS pressures.

Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, and installation of trusses. See "WARNING" note below. shipping

MEMBER TO BE LATERALLY BRACED FOR WIND LOADS PERPENDICULAR TO TRUSS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

SEE DRW HCUSROO1 02086012 FOR GABLE DETAILS

(**) 14 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

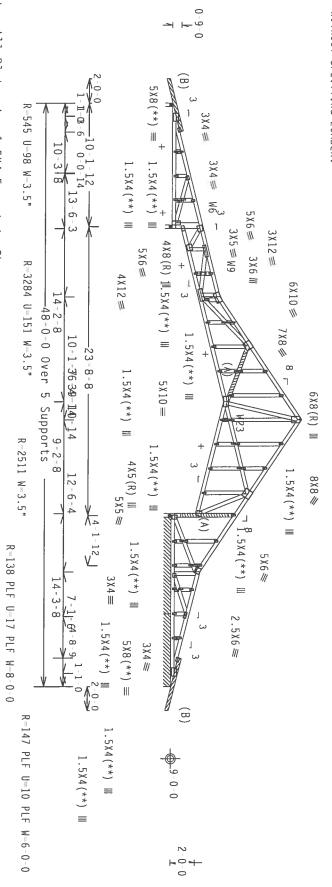
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3",min.)nails @ 6,

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

The building designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous designed by the building designer ateral restraint to the gable end. All connections to

(B) (1) 2X4X FULL LENGTH SP#2 DENSE SCAB; ATTACH TO ONE FACE OF TRUSS WITH 10d BOX(0.128"X3.0") NAILS @ 4" OC WITHOUT SPLITTING LUMBER.



Note: All Plates Are 1.5X4 Except As Shown. Design Crit:

PLT

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Wave

MARNING HRUSE'S REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING.
BETER TO EST.

(BRILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, ZIB

HORRIL LEE SIRELT, SUITE JIZ, ALEXANDRAIA, NA, AZZIA) AND HICAG (MODO TRUSS COUNCIL OF AMERICA. 6300

ENTERPRISE LANE, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THEST FUNCTIONS. BULLES

OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAHELS AND BOTTOM CHORD SHALL HAVE

A PROPERLY ATTACHED REGID CELLING. TPI-2002(STD) Cq/RT=1.00(1.25)/0(0)

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BEG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BRILLD THE TRUSS IN COMPORMANCE HITH TPI; OR FARRICATING, HANDLIGG, SHAPPING, HISTALLIG A BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF DDS (MAIDINAL DESIGN SECT, BY AFRAY) AND PPI.

PIATES TO EACH FACE OF TRUSS AND. UNICES OTHERSISE LOCATED ON THIS DESIGN, POSITION PER DRAWHIGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE FER ANHEX 3 OF TPI1-2002 SEC.3. A STAL ON THIS DESIGN SECT. SHAPPLY ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE FER ANHEX AS OF TPI1-2002 TREAS.

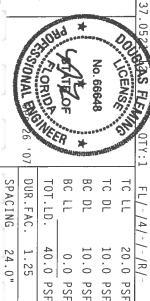
DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL FROM HITS DESIGN OF TPI1-2002 TREAS.

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL FROM HITS DESIGN SOLLY FOR THE TRUSS COMPONENT DESIGN SHAPPLY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

Haines City, FL 33844
FL Carifforde of Aminaminion # 2 279

ntion # 0 770

ALPINE



PSF

HC - ENG

JB/DF 8871

REV

DRW HCUSR8228 07360005

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PSF

DATE REF

12/26/07 18828

PSF

R8228-

Scale

=.125"/Ft.

ITW Building Components Group, Inc. Haines City, FL 33844 FL Continue of Authorization # 0 770 Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 TRUSS MAY NOT BE INSTALLED END FOR END Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. \geq (7 380 GARY JOHNSON Hampton Continuous lateral bracing equally spaced on member TYP. ALPINE Wave 3X5 (A1) R = 316Ш **IMPORTANT**FURMISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR THE BEG. INC. SMALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BRILLED HE FROSS IN COMPORMANCE WITH FIT: OR FLAREACHING, HANDING, SHAPPIG, HISTALLIGA BRACHER OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF MDS (MATIONAL DESIGN SPEC, BY AFRAY) AND FIT:

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF MDS (MATIONAL DESIGN SPEC, BY AFRAY) AND FIT:

DESIGN FOR FLARES ARE HADE OF 20/18/160A (M.H.755/M.) ASTH A653 GRADE 40/60 (M.K./M.S.SM.)

PLATES TO EACH FACE OF TRUSS AND. UNICSS OTHERSISE LOCALED ON HIS DESIGN, POSITION FOR DRAWHRGS 160A Z.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SMALL BE FER ANNEX A3 OF FIT! 200ELY FOR THE FRUSS COMPONENT

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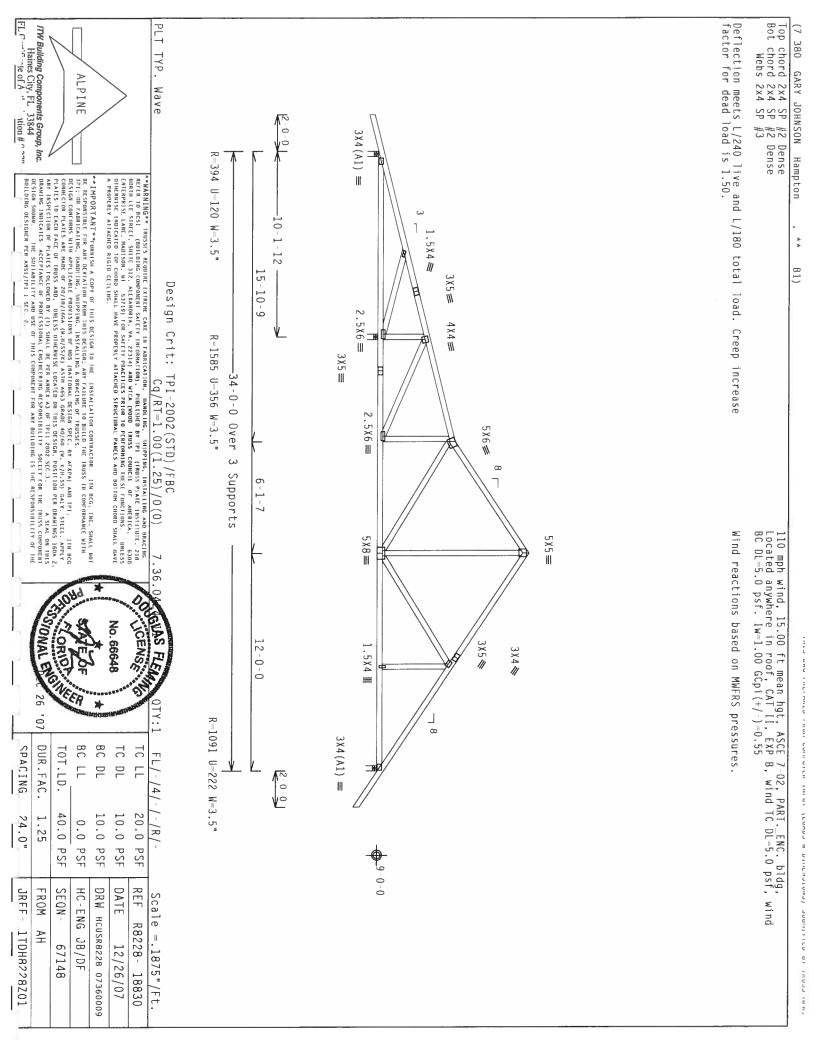
DRAWHIG THOLOGIES ACCEPTANCE OF THIS COMPONENT FOR MAY BUILDING IS THE RESPONSIBILITY OF THE U=102 W=3.5" 1.5X4 == **WARNING** IRUSSES REQUIRE EXYMENE CARE IN FABRICATION. HANDLING, SHIPPING, INSTALLING AND BRACING. RETER TO SECT. (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY FET (TRUSS PLATE INSTITUTE, 218 MORTH LEE SHEET, SHITE 315, ALEXANDRAN, VA. 223-14) AND HICA (MORD DRIVES COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MAISSON, HI 55319) FOR SAFETY PRACTICES PRIOR TO PREFORMHIE HIES, FUNCTIONS, UNLESS OTHERHISE (MORTALIED FOR MORD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SMALL HAVE 10-1-12 DESIGN SHOWN: THE SUITABILITY AND USE OF THIS BUILDING DESIGNER PER ANSI/IPI 1 SEC. 2. 15-10-9 4×6= 4 X 4 (R) Ⅲ * 5 X 4 ≡ R=1971 U=450 W=3.5" A1) 4 X 6 ≡ Design Crit: $3 \times 12 =$ 5×6# 48-0-0 Over 3×4/ TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/0(0) 10-1-7 œ 4 X 6 == 4 31-8-8 Ξ Supports 3 X 8 ≡ 5 X 6 ≡ 12 3 X 4 /// 3 X 4 ≡ 6 WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below. Wind reactions based on MWFRS pressures. 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi(+/-)=0.55 $4 \times 6 =$ COURS FLE 5 X 6 🚚 $2.5 \times 8 \equiv$ CENSE No. 66648 4 X 5 # 9-5-12 R=1730 U=382 W=3.5" R=189 U=84 W=3.5" 3 X 4 III 3X5(A1) =0.7 **√**2-0-0 BC DL TC DL DUR.FAC. TC LL TOT.LD. FL/-/4/-/-/R/-40.0 10.0 20.0 1.25 0.0 10.0 PSF PSF PSF PSF PSF manual contraction of themse FROM SEQN-DATE REF HC-ENG DRW HCUSR8228 07360006 Scale =.125"/Ft. R8228- 18829 JB/DF 67367 12/26/07

SPACING

24.0"

JRFF-

1TDH8228Z01



GE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP #2 Dense:

Truss spaced at 24.0 $^{\circ}$ OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

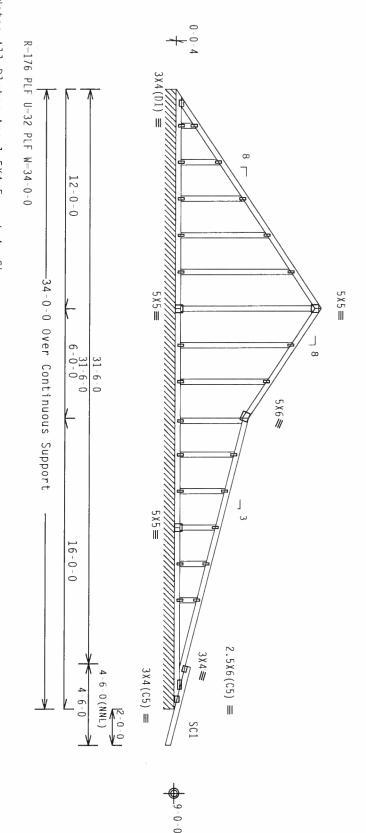
SEE DRW HCUSROO1 02086015 FOR GABLE DETAILS.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The building designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be designed by the building designer.



Note: All Plates Are 1.5X4 Except As Shown.
PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/0(0)

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, INHABILING, SHIPPING, INSTALLING AND BRACHIG, RELIER TO BEST (BUILDING COMPONENT SAFETY INTORALIZON), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 HORTH LEE SREET), SHIPE 312, ALEXANDRIA, N. A. 22214) AND BITA, 4000 TRUSS COUNCI, OF AMERICA. 6300 ENTERPRISE LANE, HADISON, NI \$3279) FOR SAFETY PRACTICES OR 108 FORMERS COUNCI, OF AMERICA. 6300 OTHERSPISE LANE, HADISON, NI \$3279) FOR SAFETY PRACTICES OR 108 FORMERS COUNCI, OF AMERICA. 6300 OTHERSPISE LANE, HADISON, NI \$3279) FOR SAFETY PRACTICES OR 108 FORMERS COUNCI, OF AMERICA. 6300 OTHERSPISE LANE, HADISON, NI \$3279) FOR SAFETY PRACTICES OR 108 FORMERS COUNCI, OF AMERICA, 6300 OTHERSPISE LANE, HADISON, NI \$3279) FOR SAFETY PRACTICES OR 108 FORMERS COUNCI, OF AMERICA, 6300 OTHERSPISE LANE, HADISON, NI \$3279) FOR SAFETY PRACTICES OR 108 FORMERS COUNCI, OF AMERICA, 6300 OTHERSPISE LANE, HADISON, NI \$3279) FOR SAFETY PRACTICES OR 108 FORMERS COUNCI, OF AMERICA, 6300 OTHERSPISE COUNCIL OF AMERICA, 6300

MADORTANTTURNISH A GOPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BGG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FALLURE TO BUILD THE TRANS IN COMPORNANCE WITH PIC OR FARRICATING, NAMOUNG, SHEPPIG, HISTALLING A BRACHING OF TRUSSES, DESIGN AND TOTAL THE DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF MIDS (MATIONAL DESIGN SPEC, BY ATRA) AND TOTAL CONFORMS WITH APPLICABLE PROVISIONS OF MIDS (MATIONAL DESIGN SPEC, BY ATRA) AND TOTAL CONFORMS WITH APPLICABLE PROVISIONS OF MIDS (MATIONAL DESIGN SPEC, BY ATRA) AND TOTAL CONFORMS WITH APPLICABLE PROVISIONS OF MIDS (MATIONAL DESIGN SPEC, BY ATRA) AND TOTAL CONFORMS WITH APPLICABLE PROVISION OF PROVISION O

ALPINE

PLATES TO LACH TACE OF TRUSS AND, UNLESS OTHERWISE TO CATE OUT THIS DESIGN, POSITION FER BRANDINGS

TWO Building Components Group, Inc.

Haines City, FL 33844

EL Confirming of Annian indicates acceptance of professional engineering responsibility society for the rings component for any building is the responsibility for the second any building is the responsibility of the component for any building is the responsibility of the component for any building is the responsibility of the component for any building is the responsibility of the component for any building is the responsibility of the component for any building is the responsibility of the component for any building is the responsibility of the component for any building is the responsibility of the component for any building is the responsibility of the component for any building is the responsibility of the component for any building is the responsibility of the component for any building is the responsibility of the component for any building is the responsibility of the component for any building is the responsibility of the component for any building is the responsibility of the component for any building is the responsibility of the responsibility o



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228201

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PSF

SEQN-

20.0

PSF

REF DATE

12/26/07

PSF

-/R/

Scale = .1875"/Ft. REF R8228- 18831

10.0 PSF

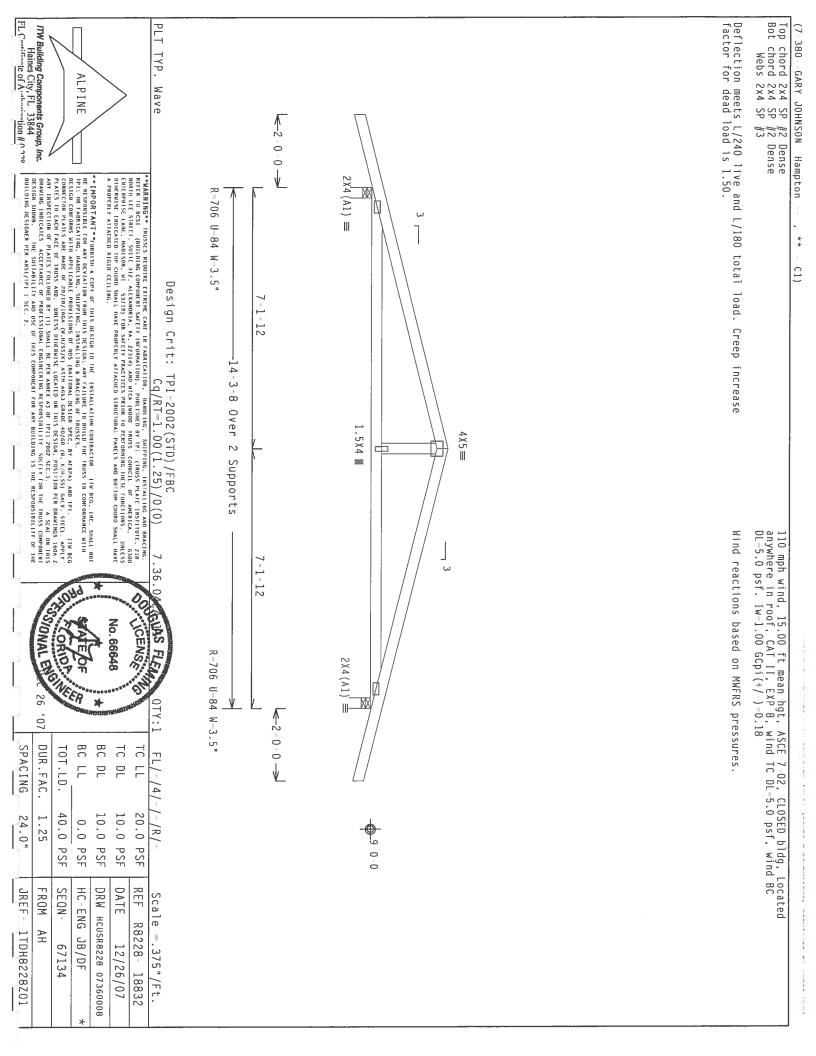
DRW HCUSR8228 07360007

0.0

PSF

HC-ENG

JB/DF 67390



Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP #2 D
:Stack Chord SC2 2x4 SP #2 D ITW Building Components Group, Inc. Haines City, FL 33844 FL Cariffonte of Authorization # 0 279 Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chamust not be cut or notched. supporting shear walls. Shear walls must provide continual lateral restraint to the gable end. All connections to be designed by the building designer. The building designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous Deflection meets L/240 live and L/180 factor for dead load is $1.50.\,$ (7 380 GARY JOHNSON Hampton TYP. ALPINE Wave 2-0-0-2X4(C5) =R=177 PLF U=1 PLF W=14-3 4-0-0 (NNL) $2.5 \times 6 (C5) =$ SCI 4-0-0 **IMPORTANT**TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BEG. THC. SHALL HOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BHILD THE TRUSS IN COMPORMANCE WITH FIT. OR LAREALING, HANDLING, SHIPPING, HISALLING & BRALLING OF TRUSSES.

DESIGN CONTROPS WITH APPLICABLE PROVISIONS OF HOS (MATIONAL DESIGN SPEC, BY ATRA) AND IP!. IN BCG CONNECTION PLATES ARE HADE OF ZO/JRJ BGAGA (H.HUSS/X) ASTENDANDE AND TO STORM FOR DRAWINGS AND. HIS DESIGN SPEC, BY ATRA) AND IP!. PLATES TO EACH FACE OF TRUSS AND. HISES OTHERWISE LOCATED ON HIS DESIGN, POSITION FER DRAWINGS 160A Z. ANY HISPECTION OF PLATES TOLLOWED BY (1) SHALL BE FER ANKEX AS OF TPI ZOOZS SEC.3. AS AS AND ON THIS DESIGN SHOWN. HIS DESIGN SHOWN. **WARNING** IRUSSES REQUIRE EXIREHE CARL IN FABRICATION, IMBDLING, SHIPPING, INSTALLING AND BRACING, REFLEX TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE (TRUSS PLATE INSTITUE, ZIB UNDRIN LET STREET, SUITE JIZ, ALEXANDRIA, VA, 22314) AND BITCA (AUGOD TRUSS COUNCIL OF AMERICA, 6300 CHITERPRISC LINE, MADISON, NI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNITES OTHERWISC INDICATED INFORMED SMALL HAVE PROPERLY ATTACHED RIGID CELLING. Dense: 3 X 4 ≡ total load. Creep increase GE) Design Crit: w 7-1-12 4 3-8 Top chord .5X4 TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/0(0) 毌 \Box Over Continuous Support 10-3 4 X 4 == ۵ .5X4 III Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, iw=1.00 GCpi(+/-)=0.18 Wind reactions based on MWFRS pressures. chord in notchable area using 3x6. See DWGS A11015EE0207 & GBLLETIN0207 for more requirements 7-1-12 ODD TO FISH W No. 66648 3 X 4 **#** $2.5 \times 6 (C5) \equiv$ 4-0-0 (NNL 4-0-0 2X4(C5) =SC2 0 BC DL TC DL DUR.FAC. TC LL TOT.LD. FL/-/4/-/-/R/ 40.0 10.0 20.0 10.0 PSF 1.25 0.0 9-0-0 PSF PSF PSF PSF SEQN-DATE REF FROM HC-ENG DRW HCUSR8228 07360001 Scale R8228-=.375"/Ft. JB/DF 67167 12/26/07 18833

tion # 0 770

SPACING

24.0"

JREF-

1TDH8228Z01

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES

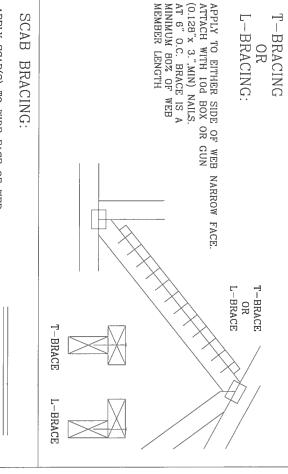
THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

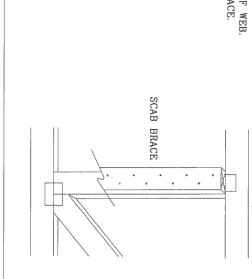
2-2X6(*)	2X6	2 ROWS	2X8
1-2X8	2X6	1 ROW	2X8
2-2X4(*)	2X6	2 ROWS	2X6
1-2X6	2X4	1 ROW	2X6
2-2X4	2X6	2 ROWS	0R
1-2X4	2X4	1 ROW	2X3 OR 2X4
SCAB BRACE	T OR L-BRACE	BRACING	SIZE
ALTERNATIVE BRACING	ALTERNATIV	SPECIFIED CLB	WEB MEMBER

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

* CENTER SCAB ON WIDE FACE OF WEB. FACE OF WEB APPLY (1) SCAB TO EACH



APPLY SCAB(S) TO WIDE FACE OF WEB. NO MORE THAN (1) SCAB PER FACE. ATTACH WITH 10d BOX OR GUN 80% OF WEB MEMBER LENGTH (0.128"x 3.",MIN) NAILS. AT 6" O.C. BRACE IS A MINIMUM



THIS DRAWING REPLACES DRAWING 579,640

A STATE OF THE STA	ILDING DESIGNER, PER	ANCE OF PROFESSIONAL STATE OF	GRADE 40/50 (4X/HASS) GRADE 10/50 (4X/HASS) R LIBCATED DIN THIS TO BY ATS SHAW HE PEP	BUILD THE TRUSS IN RACING OF TRUSSES.	CHANGE OF THE PARTY OF THE PART	IOR TO PERFORMING THESE ATTACHED STRUCTURAL	IPPING, INSTALLING AND LED BY TPI CIRUSE PLATE
The second secon	SPACING	DUR. FAC.	TOT. LD.	BC LL	BC DL	TC DL	TC LL
			PSF	PSF	PSF	PSF	PSF REF
				-ENG	PSF DRWG	DATE	REF
				PSF -ENG MLH/KAR	BRCLBSUB0207	PSF DATE 2/23/07	CLB SUBST.



AVARNING* FRUSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIP BRACKING, REFER TO BESTS (BUILDING COMPONENT SAFETY INTORANTION), PUBLISHE BRACKING, REFER TO BESTS (BUILDING COMPONENT SAFETY INTORANTION), PUBLISHE INSTITUTE, 218 NORTH LEE STR. SUITE 312, ALEXANDRIA, VA. 22314) AND VICA AMERICA, 6300 ENTERPRISE LN. HADDISIN, VI 53719) FOR SAFETY PRACTICES PRID FUNCTIONS. (INJEKES OTHERWISE INDICATED, TIP CHAND SHALL HAVE PRIDERLY APPARENT AND BOTTOM CHORD SHALL HAVE PRIDERLY ATTACHED RIGID CEILING.

MANDER MATERAL FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR BY NOT BE RESPONSED FOR ANY FALLURE TO BE DEPERDMENT. WITH THIS LONG ANY FALLURE TO BE DEPERDMENT. WITH THIS LONG ANY FALLURE TO BE DEPERDMENT. WITH THIS LONG AND FALLURE SHOULD FER DESIGN CONNECTION OF PLATES. ARE MADE OF PANIS IN MASS AND THE MA

ASCE 7-02: 110 MPH WIND SPEED, 15 MEAN HEIGHT, ENCLOSED, ||1.00, **EXPOSURE** \bigcirc

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	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	GRADE	BRACE	
	4' 11"	5.	5. 0	5. Ω	5' 4"	4' 9"	4, 9,	4 9	4' 11"	4. 5.	4 6"	4' 6"	4' 9"	4' 10"	4' 4"	4' 4"	4' 4"	4′ 5"	3' 10"	4' 0"	4' 0"	4' 2"	4'3"	3' 9"	3, 9,	3' 9"	3' 10"	BRACES	Z O	
	7' 5"	8, 5,	8' 5"	8 5"	8, 5,	7' 3"	8' 5"	8 5	8, 5,	G 5	7' 6"	7' 7"	7' 8"	7' 8"	6' 4"	7' 4"	7' 4"	7' 8"	5. 3.	6' 1"	6, 2,"	6' 8"	6' 8"	5' 2"	6'0"	6'0"	6, 8,	GROUP A	(1) 1X4 "L"	
	7' 5"	8' 7"	8' 5"	9' 1"	9' 1"	7' 3"	B' 5"	ත <u>්</u>	8' 8"	6, 5,	7' 6"	7' 7"	8' 3"	α. α.	6' 4"	7' 4"	7' 4"	7' 10"	5' 3"	6' 1"	6, 5,	7' 2"	7' 2"	5, 2,	6' 0"	6' 0"	6' 10"	GROUP B	L' BRACE *	
	9' 10"	10' 0"	10' 0"	10' 0"	10' 0"	9' 7"	10' 0"	10' 0"	10' 0"	8' 6"	9' 1"	9' 1"	9' 1"	9' 1"	8' 4"	9' 1"	9' 1"	9' 1"	6' 11"	7' 11"	7' 11"	7' 11"	7' 11"	6, 9,,	7' 11"	7' 11"	7' 11"	GROUP A	(1) 2X4 "	
MAS		10' 6"	10′ 6″	10' 9"	10' 9"	9' 7"	10' 0"	10' 0"	10' 3"	8' 6"	9' 6"	9' 6"	9' 9"	9' 9"	8' 4"	9' 1"	9' 1"	9' 4"	6' 11"	8' 0"	8, 1,,	8, 6,	B' 6"	6' 9"	7' 11"	7' 11"	8' 1"	GROUP B	"L" BRACE *	
JI WMAS	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"	10′ 10"	10' 10"		10' 10"	9' 4"	9' 5"	9' 5"	9,	9' 5"	9' 1"	9' 5"	9' 5"	9' 5"	GROUP A	(2) 2X4 "L"	
	12' 3"	12' 6"	12' 6"	12' 10"	12' 10"	11' 11"	11' 11"	11' 11"	12' 3"	11' 1"	11' 4"	11' 4"	11' 8"	11' 8"	10' 10"	10' 10"	10' 10"	11, 1,	9' 4"	9' 11"	9' 11"	10' 2"	10' 2"	9' 1"	ي ه	9, 5,	9' 8"	GROUP B	" BRACE **	
	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	12' 11"	14' 0"	14' 0"	14' 0"	10' 10"		12' 5"	1	12' 5"	٦,	12' 3"	- 1	12, 5,	GROUP A	(1) 2X6 "L"	
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	14' 0"	- 1	- 1	14' 0"	-[14' 0"		14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	- 1	14' 0"	14' 0"		14' 0"		1	- 1		14' 0"	14' 0"	GROUP A	(2) 2X6 "L"	
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DOUGLAS FIR-LARCH #3 STUD

SOUTHERN PINE #3 STUD

STANDARD

STANDARD

GROUP

В

HEM-FIR

SPRUCE-PINE-FIR
#1 / #2 STANDARD
#3 STUD

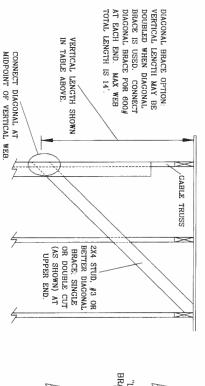
BRACING GROUP SPECIES AND GRADES:

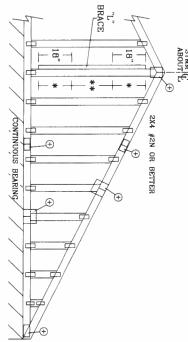
GROUP

A: #3

HEM-FIR

STANDARD





ATTACH EACH "L" BRACE WITH 10d NAILS.

* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C.

* FOR (2) "L" BRACES: AND 4" O.C. BETWEEN ZONES.

** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C.

IN 18" END ZONES AND 6" O.C. BETWEEN ZONES. GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER LIVE LOAD DEFLECTION CRITERIA IS L/240.

GABLE TRUSS DETAIL NOTES:

SOUTHERN PINE

DOUGLAS FIR-LARCH

12

72 1

CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

MEMBER LENGTH. "L" BRACING MUST BE A MINIMUM OF 80% OF WEB

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.	GREATER THAN 11' 6"	GREATER THAN 4' 0", BUT LESS THAN 11' 6"	LESS THAN 4' 0"	VERTICAL LENGTH	GABLE VERTICAL PLATE SIZES
DESIGN FOR PLATES.	2.5 X4	2 X4	1X4 OR 2X3	NO SPLICE	TE SIZES

AVANHUG TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BESS (BUILDING COMPINENT SAFETY INFORMATION), DUBLISHED BY TET CIRUSS PLATE INSTITUTE, 218 NORTH LEE STR, SUITE 312, ALEXANDRIA, VA. 22314) AND VICA CVOIDD TUSS COUNCIL BARRICA, 6300 ENTERPRISE LN, HANDSIN, VI 53719) FOR SAFETY PRACTICES PRIBE TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED. TOP CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL

REFER

TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

WHIPDER/ANIMA FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITY BCG, INC., SMALL

ODM/COMMANCE WITH 1PT; DR FABRICATION FROM THIS DESIGN, ANY FAILURE ID BUILD IM FIGUSS IN

DESIGN COMMENCED WITH APPLICABLE PROFUSIONS OF MUSIC AND MAIS FOR THE SECOND ANY FAILURE AND THE SECOND AND THIS MEDICABLE PROFUSIONS OF MUSIC AND MAIS AND ASSEMBLE OF THE SECOND AND THIS MEDICABLE PROFUSIONS OF MUSIC AND ANY AND SECOND AND AND THIS MEDICABLE PROFUSIONS OF MUSIC AND ANY AND SHARE AND AND THIS MEDICABLE PROFUSIONS OF MUSIC AND AND AND THE MUSIC AND AND THE SECOND AND THE BUILDING DESIGNER, FER ANSIATION AND THE BUILDING DESIGNER, FER ANSIATION AND THE BUILDING DESIGNER, FER ANSIATION AND THE SECOND AND THE BUILDING DESIGNER, FER ANSIATION AND THE SECOND AND THE BUILDING DESIGNER, FER ANSIATION AND THE SECOND AND THE BUILDING DESIGNER, FER ANSIATION AND THE SECOND AND THE BUILDING DESIGNER, FER ANSIATION AND THE SECOND AND THE SECOND AND THE BUILDING DESIGNER, FER ANSIATION AND THE SECOND AND THE SECOND AND THE BUILDING DESIGNER, FER ANSIATION AND THE SECOND AN

ITW BUILDING COMPONENTS GROUP, INC. POMPANO BEACH, FLORIDA

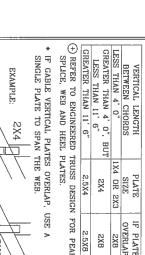
ALPINE

W. SOKIE	E.E.	STATE OF X	ec 66 No.66648	SOUIGENS, TEMPS
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			6	

30 PSF DRWG DATE A11015EE0207 2/23/07 ASCE7-02-GAB11015

24.0"

GABLE FOR LET-IN VERTICALS DETAIL









ATTACH EACH "T" REINFORCING MEMBER WITH PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN

HAND DRIVEN NAILS:

GUN DRIVEN NAILS: 10d COMMON (0.148"X 3.",MIN) TOENAILS AT 4" O.C. PLUS (4) 16d COMMON (0.162" X 3.5",MIN) TOENAILS IN TOP AND BOTTOM CHORD

8d COMMON (0.131"X 2.5", MIN) TOENAILS AT 4" O.C. PLUS (4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD,

ASCE 7-93 GABLE DETAIL DRAWINGS

REINFORCING MEMBER

4 TOENAILS

RIGID SHEATHING

GABLE TRUSS

TOENAILS SPACED AT 4" O.C.

ASCE 7-98 GABLE DETAIL DRAWINGS A11015EN0207, A10015EN0207, A09015EN0207, A08015EN0207, A07015EN0207, A11030EN0207, A10030EN0207, A09030EN0207, A09050EN0207, A09050EN0207, A09050EN0207, A09050EN0207, A09050EN0207, A09050EN0207, A09050EN0207, A09050EN0207, A0

ASCE 7-02 GABLE DETAIL DRAWINGS A13015EC0207, A12015EC0207, A11015EC0207, A10015EC0207, A08515EC0207, A13030EC0207, A12030EC0207, A11030EC0207, A10030EC0207, A08530EC0207, A10030EC0207, A10050EC0207, A10050EC0207, A10050EC0207, A10050EC0207, A1 A08530EC0207

EXAMPLE:

ASCE WIND SPEED = 100 MPH
MEAN ROOF HEIGHT = 30 FT
GABLE VERTICAL = 24" O.C. SP #3
"T" REINFORCING MEMBER SIZE = 2X4

ASCE 7-05 GABLE DETAIL DRAWINGS A13015EE0207, A12015EE0207, A11015EE0207, A10015EE0207, A08515EE0207, A13030EE0207, A12030EE0207, A11030EE0207, A10030EE0207, A08530EE0207 A08530EE0207

A13015E50207, A12015E50207, A11015E50207, A10015E50207, A08515E50207, A13030E50207, A12030E50207, A11030E50207, A10030E50207, A08530E50207

SEE APPROPRIATE ALPINE GABLE DETAIL (ASCE OR SBCCI WIND LOAD) FOR MAXIMUM UNREINFORCED GABLE

VERTICAL LENGTH.

4 TOENAILS

CEILING

EXAMPLE: 2X4	IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE TO SPAN THE WEB.) REFER TO ENGINEERED TRUSS SPLICE, WEB AND HEEL PLATES.	GREATER THAN 11' 6"	GREATER THAN 4' 0", BUT LESS THAN 11' 6"	LESS THAN 4' 0"	BETWEEN CHORDS	VERTICAL LENGTH	GABLE VERTICAL PLATE SIZES
2X4	ES OVERLAP, THE WEB.	TRUSS DESIGN FOR PEAK, PLATES.	2.5X4	2X4	1X4 OR 2X3	SIZE	PLATE	L PLATE SI
2X8	USE A	FOR PEAK,	2.5XB	2X8	2XB	OVERLAP*	IF PLATES	ZES

TOENAIL

TOENAIL

2X4 "T" REINFORCING MEMBER

2X6 "T" REINFORCING MEMBER

APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD. TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" FACTOR BY LENGTH (BASED ON GABLE MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD. 2X4 "L" BRACE, GROUP A, OBTAINED FROM THE VERTICAL SPECIES, GRADE AND SPACING) FOR (1)

WEB LENGTH INCREASE W/ BRACE

30 FT	70 MPH	15 FT	70 MPH	30 FT	80 MPH	15 FT	80 MPH	30 FT	90 MPH	15 FT	90 MPH	30	100 MPH	15 FT	100 MPH	30	110 MPH	15 FT	110 MPH	AND MRH
13	HA	FT	lΗ	FT	PH	FT	łРН	7	PH	F	{PH	TH	MPH	FT	MPH	FT	MPH	FT	MPH	MRH
2x6	2x4	2x6	2x4	2x6	2x4	2x6	2 x4	2x6	2 x 4	2x6	2x4	2x6	2 x4	2x6	2x4	2x6	2x4	2x6	2x4	MBR. SIZE
2 01	2 01	0 %	0 %	20 %	20 %	10 %	10 %	30 %	10 %	20 %	20 %	40 %	2 01	30 %	2, 01	50 %	2 01	40 %	10 %	SBCCI
30 %	20 %	20 %	20 %	40 %	10 %	30 %	20 %	50 %	10 %	40 %	10 %	40 %	10 %	50 %	10 %	50 %	10 %	50 %	10 %	ASCE
	_																			

MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH $1.10~\times~6^\circ~7^\circ=^\dagger~7^\circ~3^\circ$ "T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10 (1) 2X4 "L" BRACE LENGTH = 6' 7"

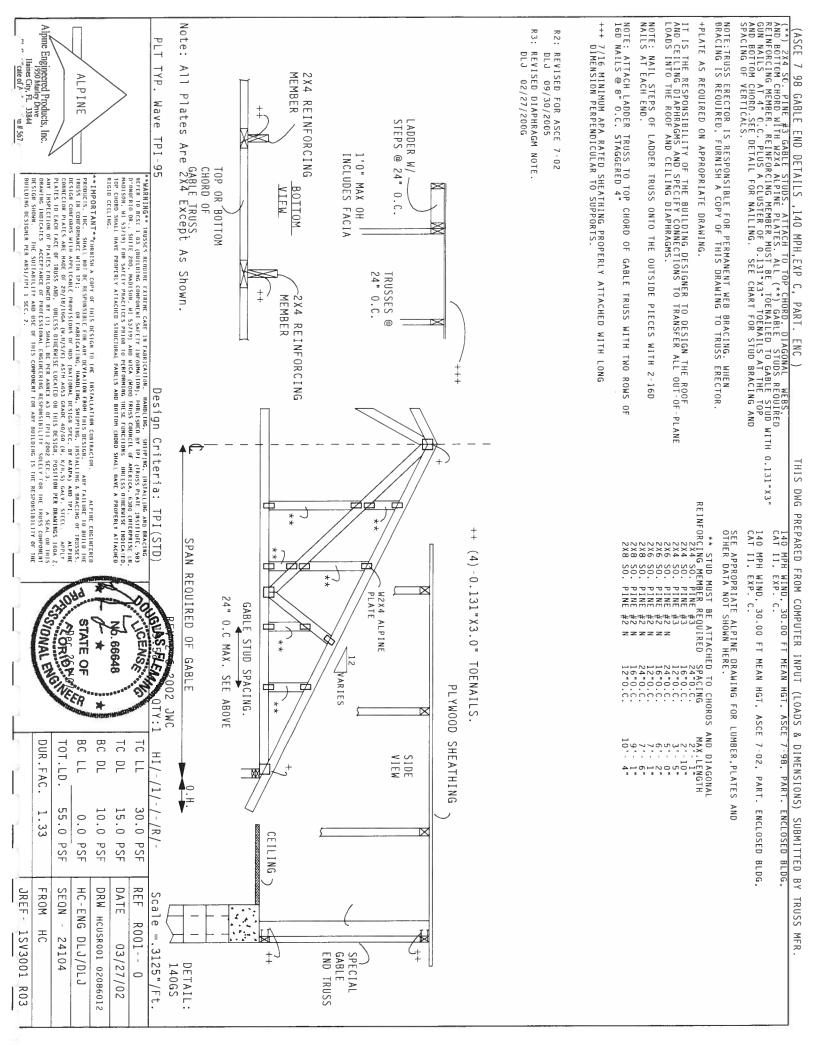
THIS DRAWING REPLACES DRAWINGS GAB98117 HC26294035

C. C. Connector Co.	BUILDING DESIGNER, PER	EPTANCE OF PROFESSIONAL STATE OF STATE OF	Bed 26	TO BUILD THE TRUSS IN THE TRUSSES. SPECIFIC BY AFRADA AND THE TRUSSES.	1	PRIOR TO PERFORMING THESE RLY ATTACHED STRUCTURAL	SHIPPING, INSTALLING AND LISHED BY TRI CRUSS PLATE	THIS DEAVING REPLACES DEAVINGS GABBELL 876,719 & HC26294035
	MAX	DUR. FAC.	MAX					PLACES
	MAX SPACING 24.0"	FAC.	MAX TOT. LD. 60 PSF					DKAW
	ING	ANY	LD. 6					INGS
	24.0	YV	30 P					GABS
5,55,000	19		뜅	П	ם	D	z	7.119
				ENG	RWG	ATE	REF	876,7
				-ENG DLJ/KAR	DRWG GBLLETIN0207	DATE 2/23/07	LET-IN VERT	19 % HC



MAKARHINGA TRUSSES REQUIRE EXTREME CARE IN FABRICATHIR, HANDLING, S
BRACING. REFER TO BESI (BUILDING COMPONENT SAFETY INFORMATION), POBLI
INSTITUTE, 218 NORTH LEE STR, SUITE 312, ALEXANDRIA, VA 22314) AND VTI
MARRICA, 6300 ENTERPRISE LN, HADISON, VI 53719) FOR SAFETY PRACTICES P
FUNCTIONS. UNESS DITHERYSEE INDICATED, TOP CHARD SHALL HAVE PROPERLY
PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILIN

WHERDER/ANIMA FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTON
NOT BE RESPONSUBLE FOR ANY DEVIAINING MISSISSIAN ANY FAILURE IT
COMPONENCE VATIL FOR PARRICATING HANDLING, SHEPPING, INSTALLING &
BESSEN AND COMPONENCE OF THE PROPERTY OF THE PROPERTY OF THE STORY AND THE AND THE STORY AND THE PROPERTY OF THE THE STORY OF THE STORY AND THE STORY OF THE STORY AND THE STORY AND



140 MPH WIND, 30.0 FT MEAN HGT, ASCE 7-98, PARI. ENC. BLUB, LIN ROOF, CAT II, EXP C, WIND TCDL=5.0 PSF, WIND BCDL=5.0 PSF. 30.0 FT MEAN HGT, ASCE 7-98, PART. ENC. BLDG, LOCATED ANYWHERE

140 MPH WIND, 30.0 FT MEAN HGT, ASCE 7-02, PART. ENC. BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TCDL=5.0 PSF, WIND BCDL=5.0 PSF.

- VERTICAL VERTICAL WEBS LESS THAN 4'0": W1X4
 GREATER THAN 4'0" BUT
- NO MORE THAN 11'6": W2X4.
- PEAK, AND HEEL PLATES TO MATCH COMMON TRUSS.
- OR GREATER CHORDS

DROP GABLE WILL SUPPORT 4'0" SPACED 24" O.C., OR THE LOAD OUTLOOKERS WITH 2'0" OVERHANG (DROP HEEL GABLE) FROM 12" PLYWOOD OVERHANG (NOMINAL HEEL GABLE).

IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO DESIGN THE ROOF AND CEILING DIAPHRAGMS AND SPECIFY CONNECTIONS TO TRANSFER ALL OUT-OF-PLANE LOADS INTO THE ROOF AND CEILING DIAPHRAGMS.

AND ROOF SHEATHING DIAPHRAGM CONNECTIONS, THE BUILDING DESIGNER IS RESPONSIBLE FOR THE GABLE SHEAR WALL DESIGN. CEILING AND ALL TRUSS TO WALL CONNECTIONS

7/16 MINIMUM APA RATED SHEATHING PROPERLY ATTACHED WITH LONG DIMENSION PERPENDICULAR TO SUPPORTS.

R1 NOTE: STEPS OF LADDER TRUSS ONTO 2-16D NAILS AT EACH END. THE OUTSIDE PIECES

ATTACH LADDER TRUSS TO TOP CHORD OF GABLE TRUSS WITH TWO ROWS OF 16D NAILS @ 8" O.C.

> BRACING DEFINITIONS: NOTE: "END ZONE" EXISTS 18" Ą BOTH ENDS OF VERTICAL WE B

- END 2X4 SP ZONES: 4 # 3 00. BETWEEN BRACE. ZONES. ATTACH HTIW 0.128"X3" NAILS @ 2 00. Ï
- (2) 2X4 SP 6" OC. BRACES. ATTACH EACH WITH 0.128"X3" NAILS ø ω 0C.

(B)

- END BETWEEN ZONES Z
- <u>C</u> $\widehat{\mathbb{H}}$ 2X6 SP #2 ZONES: 4" OC. BETWEEN ZONES. N "L" BRACE. ATTACH HIIM 0.128"X3" NAILS 00.
- 0 (2) 2X6 SP ZONES; OC. N "L" BRACES. BETWEEN ZONES. ATTACH EACH WITH 0.128"X3" NAILS e ω * OC. Z

STOD SPACE	SIUU SPACING / BRACING TABLE:	IG TABLE:				
2X4 SP #3 STUD SPACING	DEFLEC- TION CRITERIA	NO BRACE	(1) 2X4 "L" BRACE TYPE (A)	(2) 2X4 "L" BRACE TYPE (B)	(1) 2x6 "L" BRACE TYPE (C)	(2) 2X6 "L" BRACE TYPE (D)
2 / "	1 /360		 - - -	A - 0 =	3 *	ρ.
24"	L/180		3. 4.	5' 7"	6, 3,	11'0"
16"	L/360		3'11"	5' 3"	7'10"	9'11"
16"	L/180	-	4' 9"	7' 4"	9' 6"	11' 0"
12"	L/360	:	4' 7"	6' 1"	8'11"	11' 0"
12"	L/180		5'11"	8, 5,	11' 0"	11' 0"

STEPS @ 24" INCLUDES OVERHANG DETAIL 0C. MAX TRUSSES @ Over Continuous Support REFER TO TABLE FOR BRACING REQUIREMENTS. U=280 PLF

Note:

All Plates Are 2X4 Except As Shown.

GABLE SHAPES

TYP. Wave TPI-95

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. RETER TO BESS IT TO ROBILIDING COMPONENT SACTLY INFORMATION), PUBLISHED BY FI (TRUSS PLATE INSTITUTE, 593 D'OHOFRIO DE. SUITE ZOO, HADISON, HI SAZIO) AND HICK, MODIO FRUSS COUNCIL OF AFRICA, SADO ENTERRAISE LU, THADISON, HI SAZIO FOR SACTLY PRACTICES PRIOR TO PREFEMBRE THE THADISON, HI SAZIO FOR SACTLY PRACTICES PRIOR TO PREFEMBRE THE THADISON, HI SAZIO FOR SMALL HAVE PROPERLY ATTACHED RIGHD CEILING. Design Crit: TPI-1995 (STD) R3: REVISED DIAPHRAGM NOTE.
DLJ 02/27/200G

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.

ALPINE ENGLISE, SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION ROOM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH PI;

OESIGN CONFORMANCE WITH PI;

OESIGN CONFORMS WITH APPLICANCE PROVISIONS OF DDS (MATIONAL DESIGN SPEC. BY ATRAN) AND TP!

CONNECTOR PLATES ARE HADE OF ZO/IE/16GA (M.H./SM.) ASIM AGS3 GRADE 40/56G (M. K/M.S) GALV. STEEL. APPLY
PLATES TO LACH FACE OF TRUSS AND. UNLESS OTHERWISE (DCATED ON THIS DOSIGN, POSITION PER DRAWINGS 160A. Z.

ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX AS OT TPIT 200Z SEC. 3.

BRANING INDICATES ACCEPTANCE OF PROFESSIONAL ENGLUER AND RESPONSIBILITY SOLETY FOR THIS THESE COMPONENT

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGLUER AND RESPONSIBILITY SOLETY FOR THIS THE RESPONSIBILITY OF THE DESIGN SHOWN. THE SUITABILITY AND USE OF BUILDING DESIGNER PER ANSI/TPT I SEC. 2.

Alpine Engineered Products, Inc.

ALPINE

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33844 . n#567

SOONAL ENGINE STATE OF Ng. 66648 A Brack

BC DL TC DL TC DUR.FAC. REV 2-5-02 TOT.LD. 47.0 JWC /-/R 30.0 1.33 0.0 10.0 PSF 7.0 PSF PSF PSF PSF

> DATE REF

03/27/02

Scale =.375"/Ft.

R001--

0

DETAIL: 140GC

R2:

REVISED FOR ASCE 7-02.

SPACING 24.0" JREF-1SV3001 R03

SEQN-

24860

HC-ENG

DLJ/DLJ

DRW HCUSR001 02086015



OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection
This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in

Parcel Number 17-5S-16-03641-000 accordance with the Columbia County Building Code.

Use Classification SFD/UTILITY

Fire: 36.63

Building permit No. 000026569

Waste: 50.25

Owner of Building JACK HAMPTON

Permit Holder JACK S.HAMPTON

Total: 86.88

Location: 1189 SW CARPENTER RD., LAKE CITY, FL

Date: 07/30/2008

we Dr. Know

Building Inspector

POST IN A CONSPICUOUS PLACE (Business Places Only)