

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

ONE STORY WALL SECTION

SCALE: 3/4" = 1'-0"

SIMPSON H2.5A U.N.O. -

SEE STRUCTURAL PLAN

(2) SIMPSON LSTA21-

w/ (8) -16d TO HEADER

AND (8) -16d TO POST

-		
	(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
	(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
	(1) 2x6 @ 16" OC	TO 18'-10' STUD HEIGHT
	(1) 2x6 @ 12" OC	TO 20.0' STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

-(2) 2X12 SYP #2 U.N.O.

-6X6 SYP #2 POST

SEE STRUCTURAL PLAN

-SIMPSON ABU POST BASE

w/ (12) - 16d & 5/8" x 10"

SEE FOOTING DETAILS

ANCHOR BOLT

TYPICAL PORCH POST DETAIL

SCALE: 1/2" = 1'-0"

48" OC. BLOCKING REQUIRED BETWEEN OUTRIGGERS — - 2X4 BARGE RAFTER CONT. (3) .131 X 3 1/4 " GUN NAILS - SHINGLE STRIP 2X4 BLOCKING @ SHEATHING JOINT 4' FROM GABLE END -- FASCIA TOP CHORD OF GABLE END TRUSS 2X4 SCAB CONT. TOP TO DROP 3 1/2" BOTTOM CHORD@ 8' FROM GABLE -- CONT. 2X4 SCAB FROM TOP TO BOTTOM CHORD @ X-BRACING 4 - 10d NAILS OR 4 - .131"x 3.25" (PROVIDE ADDITIONAL 2X4'S @ TYPICAL AT ALL CONNECTIONS ackslush VERTICAL IF HIGHER THAN 48", TO FORM AN "L" SHAPE.) 2X4 SCAB IF VERT. WEB IS NOT PRESENT -- TOE NAIL TRUSS TO DOUBLE PLATE w/ 16d COM @8" OC. - BOTTOM CHORD OF GABLE CONT. 2X4X8' #2 SYP LATERAL END TRUSS BRACE @ 48" OC. -- 2 - 2X4 TOP PLATE - SIMPSON LSTA 24 @ 48" OC 2X4 BLOCKING @ 48" OC. TO TOP P PLATE AT BOTTOM // CHORD OF TRUSS BETWEEN GABLE AND FIRST - 2X4 STUDS @16" OC. TRUSS. 2X4 X-BRACE @ 6'-0" OC.

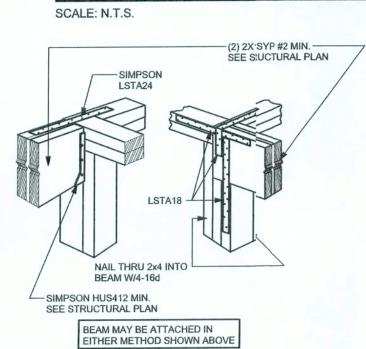
TYPICAL GABLE END (X-BRACING) ALL MEMBERS SHALL BE SYP

7/16" STRUCTURAL ROOF SHEATHING -

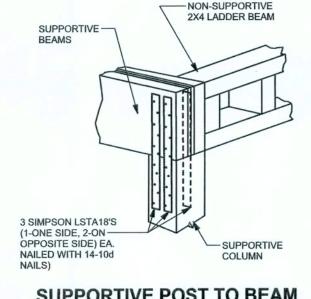
2X4 OUTRIGGER @ 48" OC. —

(2) 2X12 SYP #2 MIN. -SEE STRUCTURAL PLAN SEE STRUCTURAL PLAN -(4)-2x4 SPF NAILED NAILS AT 1O.C. MIN. (SEE S:UCTURAL PLAN)

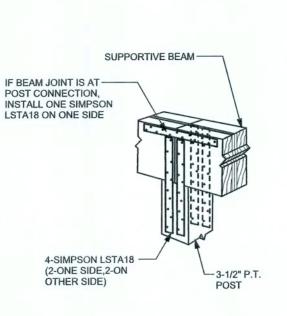
BEAM MID-WALL CONECTION DETAIL



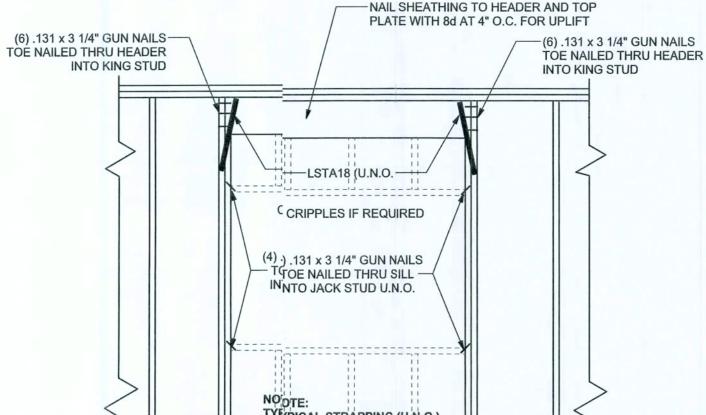
BEAM CORNER CONNECTIO. DETAIL SCALE: N.T.S.



SUPPORTIVE POST TO BEAM **DETAIL FOR SINGLE BEAM**



SUPPORTIVE CENTER POST TO BEAM DETAIL SCALE: N.T.S.



TYPICAL STRAPPING (U.N.O.) (SEEE STRUCTURAL PLAN) -SPP4 OR (2) H2.5A OR (2) SSP---/ ALLL OPENINGS (U.N.O.) (1) 2X6 S_{SPF} #2 SILL UP TO 11'-0" U.N.O. (1) 2X4 5 SPF #2 SILL UP TO 7'-3" U.N.O. (FOR: 110 h MPH, 10'-0" WALL HIGHT U.N.O.)

GRADE & SPECIES TABLE

1200

1050

975

2400

1600

2900

1.6

1.6

1.8

1.9

SYP #2

SYP#2

SYP #2

24F-V3 SP

LSL TIMBERSTRAND 1700

MICROLAM

PARALAM

PRE ENGINEERED ROOF TRUSS-

DOUEJBLE 2x4 SPF TOP PLATE NAILED -

TOGE ETHER W/2-16d NAILS AT 16" O.C. 4' MININ. LAP W/ (12) - 16d OR 4" LAP W/

INTEFERIOR CEILING AS -SPEC:CIFIED ON FLOOR PLAN

ALILL STUDS TO BE 2x4 — SP;PF NAILED TO TOP

ANNO BOTTOM PLATES

CODNTINUOUS FRAME TO

CEILING DIAPHRAGM DETAIL

WINTH 2-16d NAILS

SCALLE: N.T.S.

2x8

2x10

2x12

GLB

- HURRICANE CLIP H-2.5 OR EQUAL

TYPICAL HEEADER STRAPING DETAIL

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS

VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" x 4" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

	OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH AR T PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
	TIONS, FOUNDATION BEARING CAPACITY, GRADE AND ID SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
	AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 THE STATED WIND VELOCITY AND DESIGN PRESSURES.
	US LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU IITS A CONTINUOUS LOAD PATH CONNECTION, CALL IEER IMMEDIATELY.
DESIGN, PLACEMENT	NUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, NNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

MASONRY NOTES: MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approva
mediun		ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F	2.4F Coating for corrosion protection Anchors, sheet metal ties comp embedded in mortar or grout, A A525, Class G60, 0.60 oz/ft2 or	
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet meta ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

ANCHOR TABLE

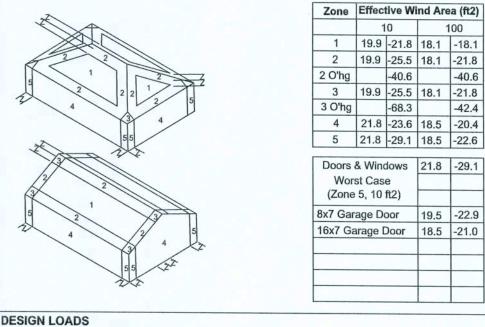
OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	Н3	4-8d	4-8d	
< 415 < 365 H2.5 < 600 < 535 H2.5A		5-8d	5-8d		
		H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24	12 130 1 1/2	12 100 1 1/2	
< 2050	< 1785	LGT2	14 -16d	14 -16d	
			14-100	14-100	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED RO 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		0.0 712
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		
	2020	ADOOU	10 - 100		2-5/8" AB

UPLIFT LBS. SYP UPLIFT LBS. SPF TRUSS CONNECTOR* TO PLATES TO RAFTER/TRUSS TO STUDS

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1 (ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS: MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.) BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION 1.) BASIC WIND SPEED = 110 MPH 2.) WIND EXPOSURE = B 3.) WIND IMPORTANCE FACTOR = 1.0 4.) BUILDING CATEGORY = II 5.) ROOF ANGLE = 10-45 DEGREES 6.) MEAN ROOF HEIGHT = <30 FT .) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING) 8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2)) 1 19.9 -21.8 18.1 -18.1 19.9 -25.5 18.1 -21.8



ESIGN	LOADS
LOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
OOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
TAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
OIL BE	ARING CAPACITY 1000PSF
OT IN	FLOOD ZONE (BUILDER TO VERIFY)

REVISIONS

VINDLOAD ENGINEER: Mark Disosway PE No.53915, FOB 868, Lake City, FL Stated dimensions supercede scaled mensions. Reer all questions to

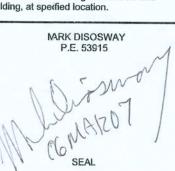
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Mark Disosway P.E. for resolution

amined this pan, and that the applicable rtions of the ran, relating to wind enginee comply with secion R301.2.1, florida building code residential2004, to the best of my

CERTIFICATIOI: I hereby certify that I have

LIMITATION: Tils design is valid for one ilding, at speified location.



Harold Barry Residence

ADDRESS: Morrell Road Section 11, T-2-S, R-16-E. Columia County, Florida

MarkDisosway P.E. P.D. Box 868 Lake Ciy, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

RINTED DATE: Mach 06, 2007 CHECKED BY: David Disosvay

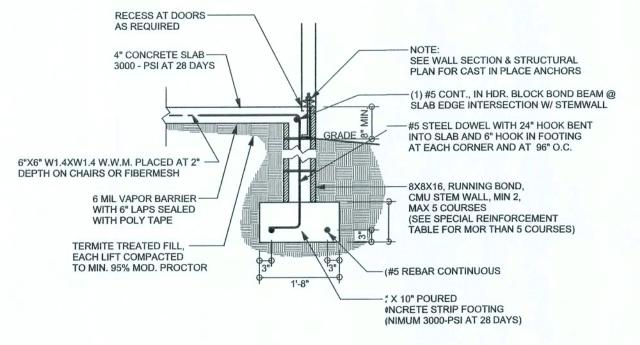
FINALS DATE: 06 / Mar / 17

> JOE NUMBER: 703056 DRAVING NUMBER

> > **S-1**

CF 3 SHEETS

REVISIONS

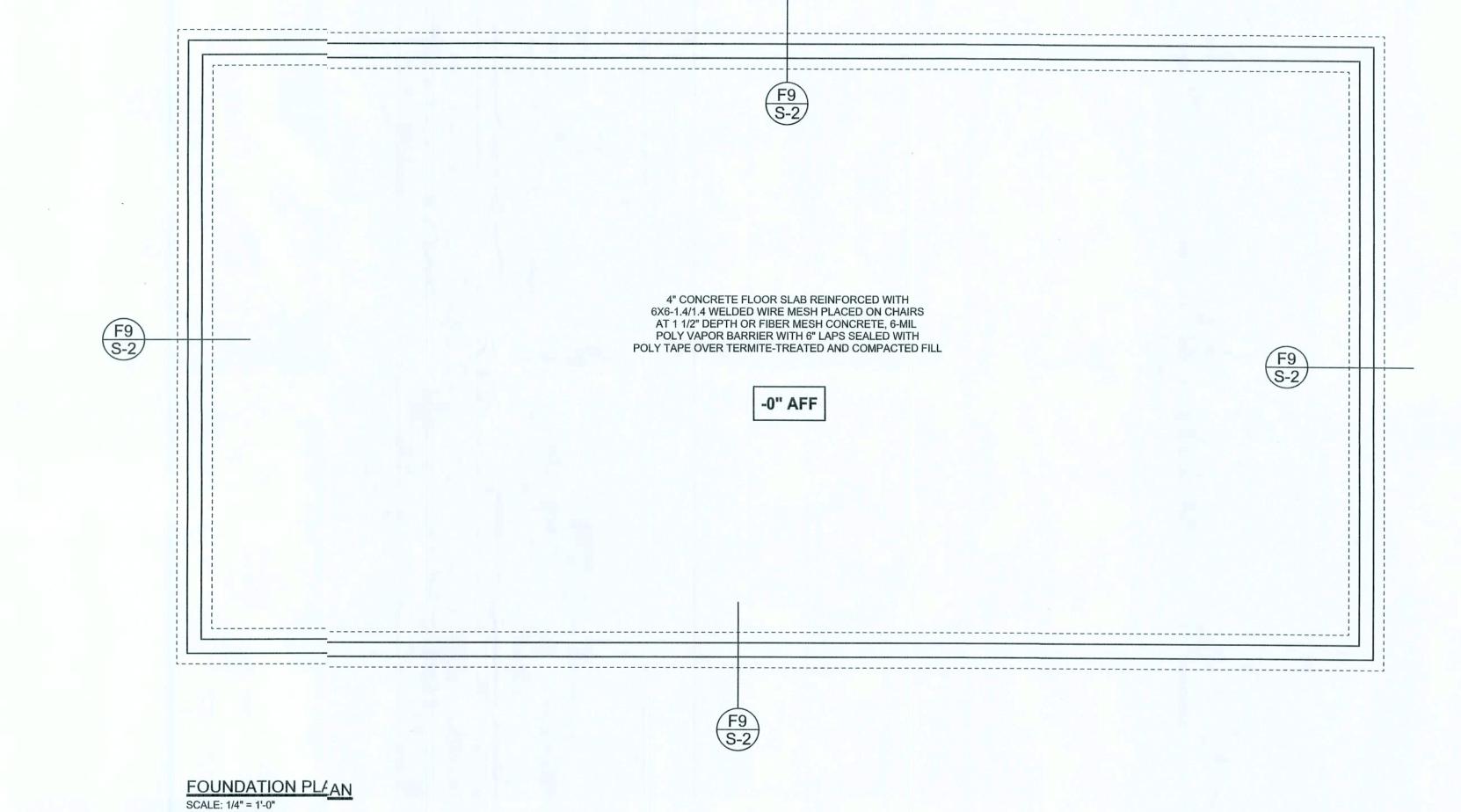


STEM WALL FOOTING SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the fring and bent 24" into the reinforced slab at the top. The vertical steel is to be placed tow the tension side of the CMU wall (away from the soil pressure, within 2" of the exteriorde of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC verally or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the all 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMEN FOR 8" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	8(56	96	96
7.3	7.0	24	40	5€	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



DIMENSIONS ON STRUCTURIRAL SHEETS
ARE NOT EXACT. REFER TOO ARCHITECTURAL
FLOOR PLAN FOR ACTUAL I DIMENSIONS

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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its common law copyrights and property right in
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form or manner without first the express written
permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915

Harold Barry Residence

ADDRESS: Morrell Road Section 11, T-2-S, R-16-E. Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

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DRAWN BY: CHECKED BY: David Disosway

FINALS DATE: 06 / Mar / 07

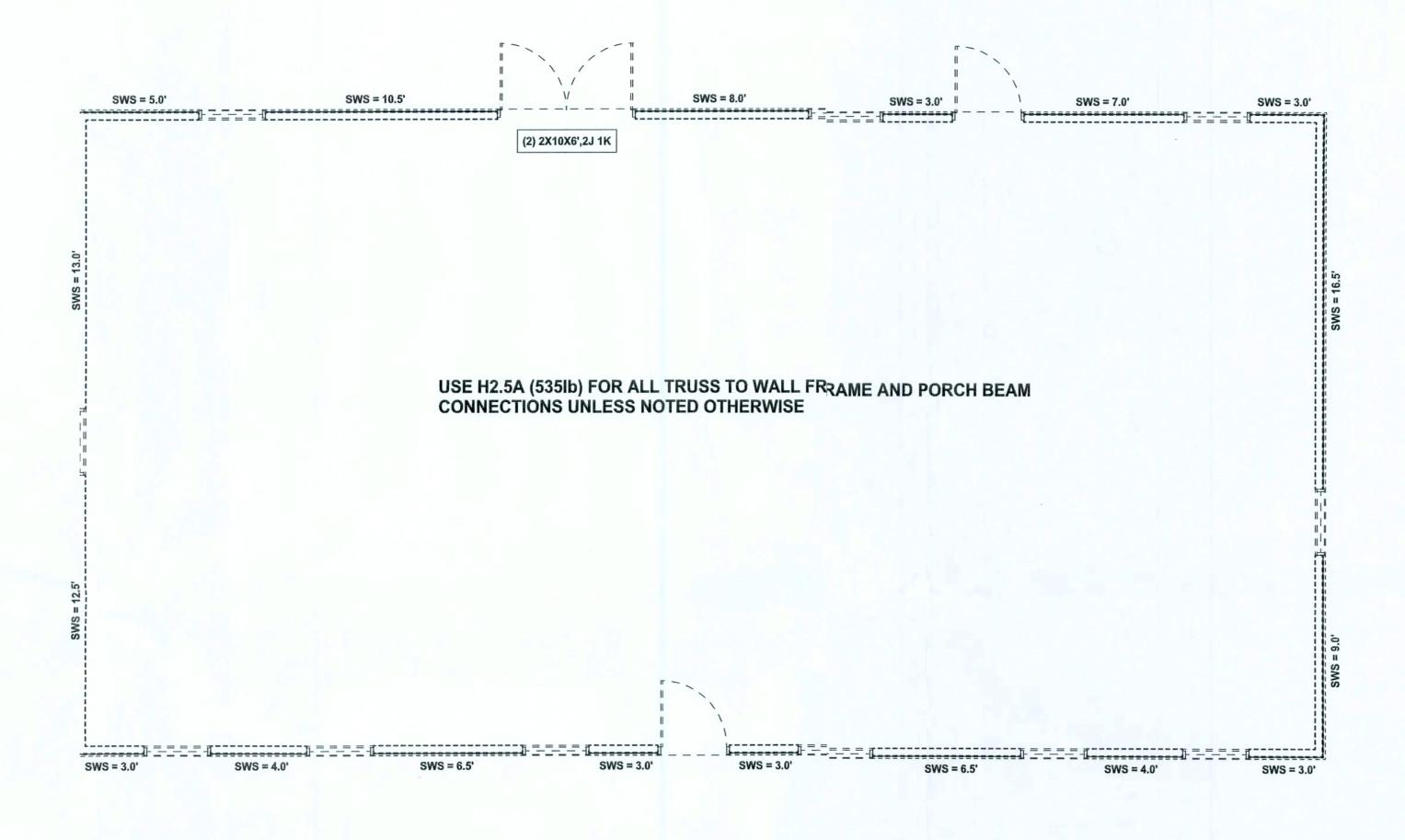
JOB NUMBER: 703056

DRAWING NUMBER **S-2**

OF 3 SHEETS

REVISIONS

SOFTPIAN APPRINTED AND APPRINTED APP



STRUCTURAL PLAN SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP #2 (U.N.O.)

3N-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

5N-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

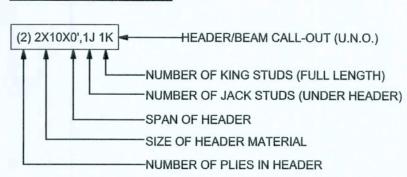
PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS.

LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3, BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

sws = 0.0'	1ST FLOODR EXTERIOR WALL WITH 7/16" O.S.Ę,B. WALL SHEATHING FULLY BLOCKED 8d COMMCION NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
SWS = 0.0'	2ND FLOO _{OR} EXTERIOR WALL WITH 7/16" O.S.Ę,B. WALL SHEATHING FULLY BLOCKED 8d COMMC _{ION} NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
IBW \$0000001 = = = = 10000000	1ST FLOO'DR INTERIOR BEARING WALLS SEE DETA'AILS ON SHEET S-1
IBW	2ND FLOO _{OR} INTERIOR BEARING WALLS SEE DETA-AILS ON SHEET S-1
	OLE DETAAILS ON SHEET 5-1

HEADER LEGEND



TOTAL SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	32.5'	51.0'
LONGITUDINAL	28.5'	69.5'

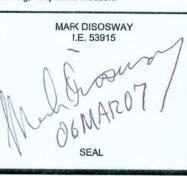
WINDLOAD ENGILEER: Mark Disosway, PE No.53915, POI 868, Lake City, FL 32056, 386-754-549

Stated dimensionssupercede scaled dimensions. Referall questions to Mark Disosway, PE. for resolution. Do not proceed whout clarification.

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CERTIFICATION: hereby certify that I have examined this plar and that the applicable portions of the plat, relating to wind engineering comply with sectio R301.2.1, florida building code residential 204, to the best of my

LIMITATION: This lesign is valid for one building, at specifid location.



Harold Birry Residence

ADDRESS:
Morrell Road
Section 11, T-2-S, R-16-E.
Columbia County, Florida

Mark Dsosway P.E. P.Q Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRNTED DATE:
Marcł 06, 2007

DRAWN BY: CHECKED BY:
David Disosway

FINALS DATE:

JOB NUMBER: 703056

S-3
OF3 SHEETS

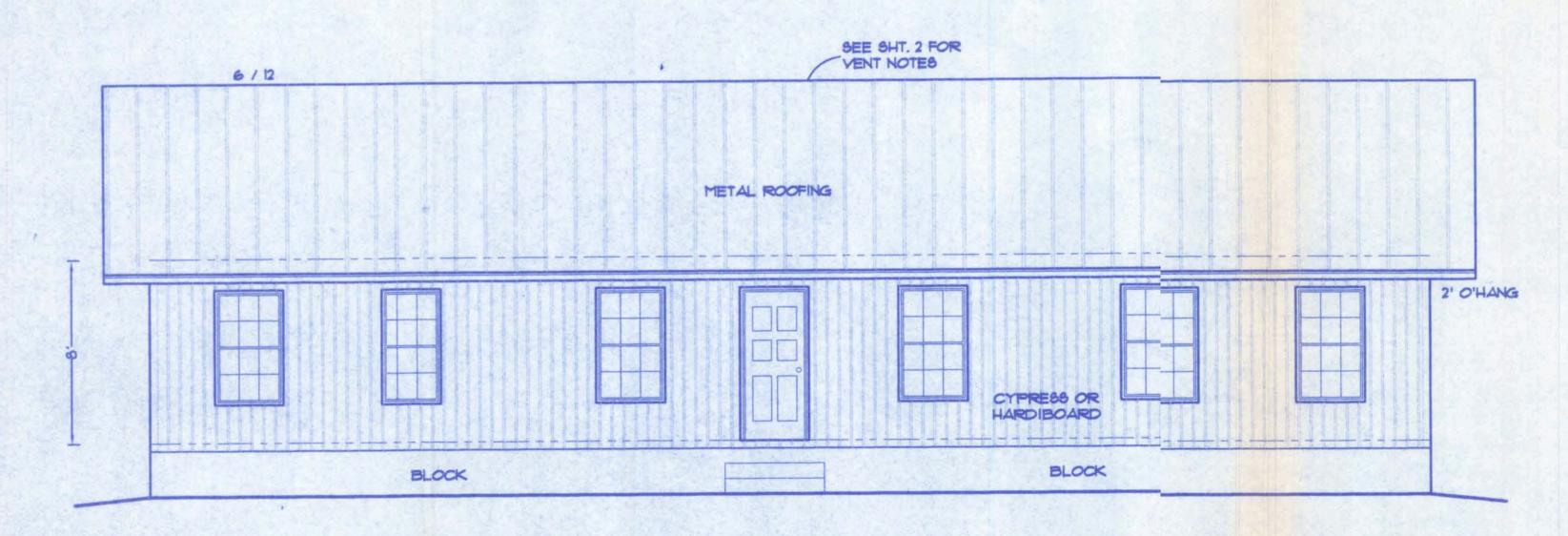
CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. ANDERSON TRUSS JOB #7-060

2' O'HANG 7'-2 1/8'-3030 4'-3 7/8'-----6'-10 7/8'---/ -5'-6' FRENCH EGRESS KITCHEN BEDRM. 2 DINING 11-4 × 11-9 13-3 × 11-8 10-8 x 11-9 65-4 x 6-6 BATH 2 -6'-8'-4'-4'-4'-12'-6' LIVING RM. 22-0 x 16-5 BEDRM. 3 16-9 x 13-1 13-3 x 11-1

FLOOR PLAN

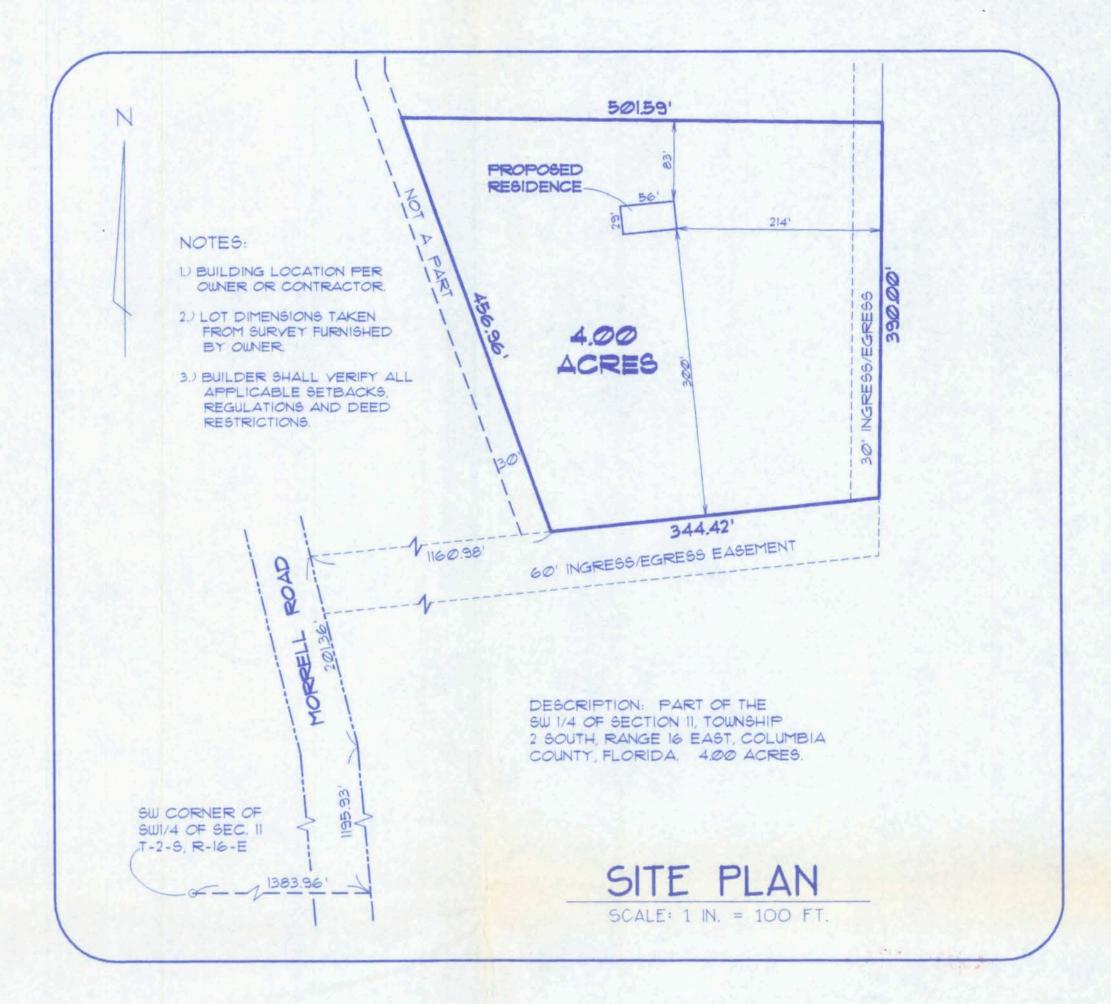
SCALE: 1/4 IN. = 1 FT.

1624 SQ. FT. LIVING AREA



FRONT ELEVATION SCALE: 1/4 IN. = 1 FT.

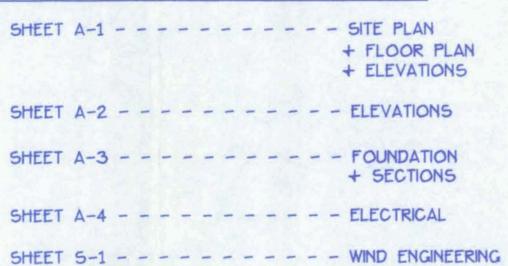
Barry Residence



SWS = Indicates a shearwall segment location referring to the labeled section of wall lying between the adjacent window / door openings in either direction. The shearwall areas have a height/width aspect ratio of 3-1/2 : 1 or wider.

1624 SQ. FT. LIVING AREA

Index to Sheets



WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

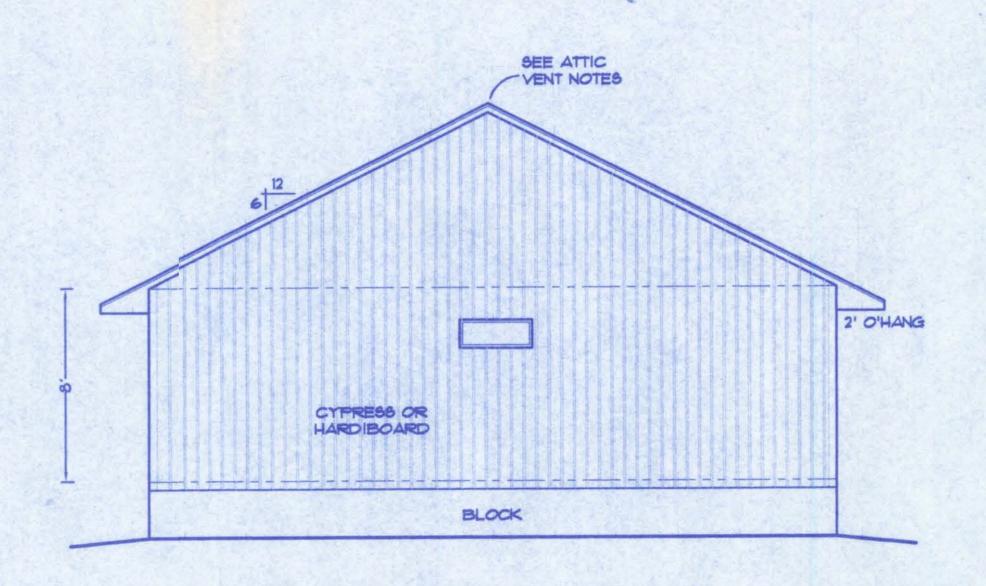
CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

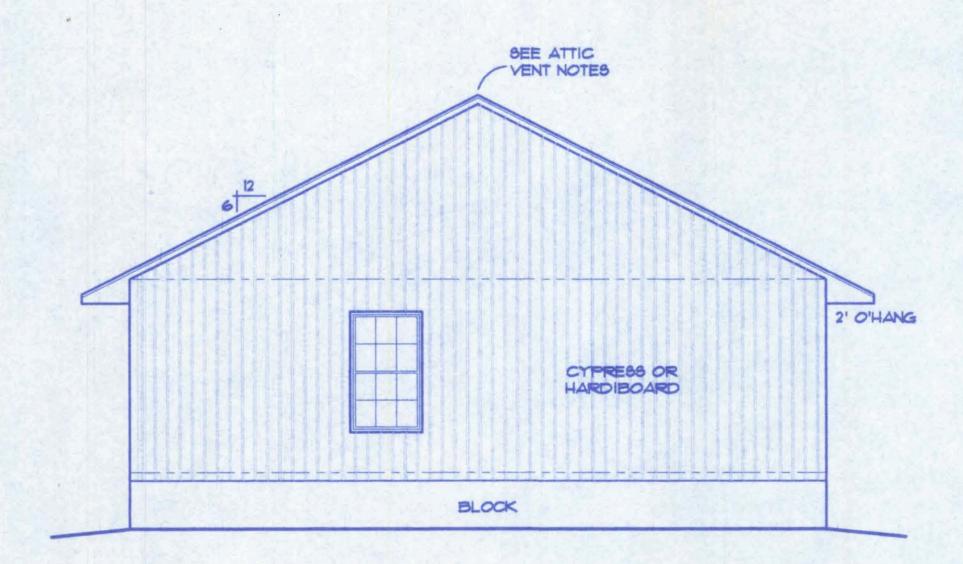
MORRELL ROAD - SECTION Location: 11, T-2-5, R-16-E.

Milsosway 08 MAR 67

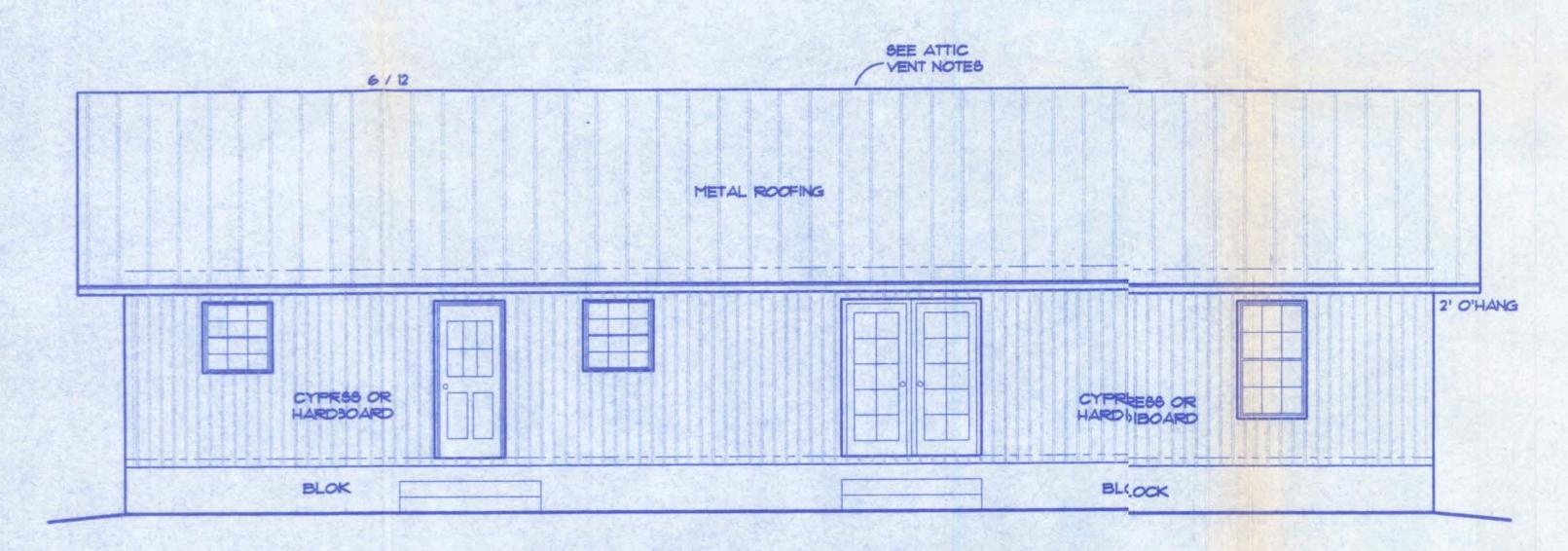
FILE: 07-002	BARRY	SHEET: 1 OF 4
2-20-07	RESIDENCE	CAD FILE 07002
RAWN: T A D	PREPARED BY: TIM DELBENE Drafting + Technical Services	REV:
CHECK:	192 SW Sagewood Gin., Lake City, FL 32024 Phone (386) 755-5891	REV:



LEFT ELEVATION SCALE: 1/4 IN. = 1 FT.



RIGHT ELEVATION SCALE: 1/4 IN. = 1 FT.



REAR ELEVATION SCALE: 1/4 IN. = 1 FT.

ATTIC VINTILATION

Enclosed attics and enclosed rafter spaces formed where ceilings are applied directly the underside of roof rafters shall have cross ventilation for ech separate space by ventilating openings protected against the entrace of rain. Ventilating openings shall be provided with corrosion-resistat wire mesh, wit h 1 / 8 inch (3.2 mm) minimum to ¼ inch (6.4 mm) jaximum openings.

The total net fre ventilating area shall not be less than 1 to 150 of the area of the spae ventilated except that the total area is permitted to be reduced to 1 to 300, provided at least 50 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper poion of the space to be ventilated at least 3 feet (914 mm) above eave or cornice vents with the balance of the required ventilation provided by eave or cornice vents.

GENERAL NOTES

- 1.) See 'Wind Load Detail Sheet S-1' and Wind Engineer's Notes for data pertaining to Wind Design and compliance w/ Florida Building Code.
- 2.) All concrete used to be 2500 PSI strength or greater.
- 3.) HVAC duct and unit size/design is by engineered shop drawings from the AC contractor.
- 4.) Windows to be alum. framed and double glazed. Sizes shown are nominal and may vary with manufacturer.
- 5.) Roof Truss design is the responsibility of the supplier.
- 6.) The Truss Manufactuer shall prepare Shop Drawings indicating Truss placement. Girder locations, Truss-to-Truss Connections and any point loads. The Contractor shall notify the Designer of any point loads in excess of 2.0k for Fnd. Modification.
- 7.) Site analysis or preparation information is not a part of this plan and is the responsibility of the owner.
- 8.) Cabinet and millwork detail is not a part of this plan. The plan is a general design and details shall be the responsibility of the owner and/or contractor.

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

MORRELL ROAD - SECTION Location: 11, T-2-5, R-16-E. FILE:

07-002

BARRY

1 0F 4

1 0F 4

CAD FILE:

07002

DRAWN:

T A D

PREPARED BY:

TIM DELBENE

Drafting + Technical Services

CHECK:

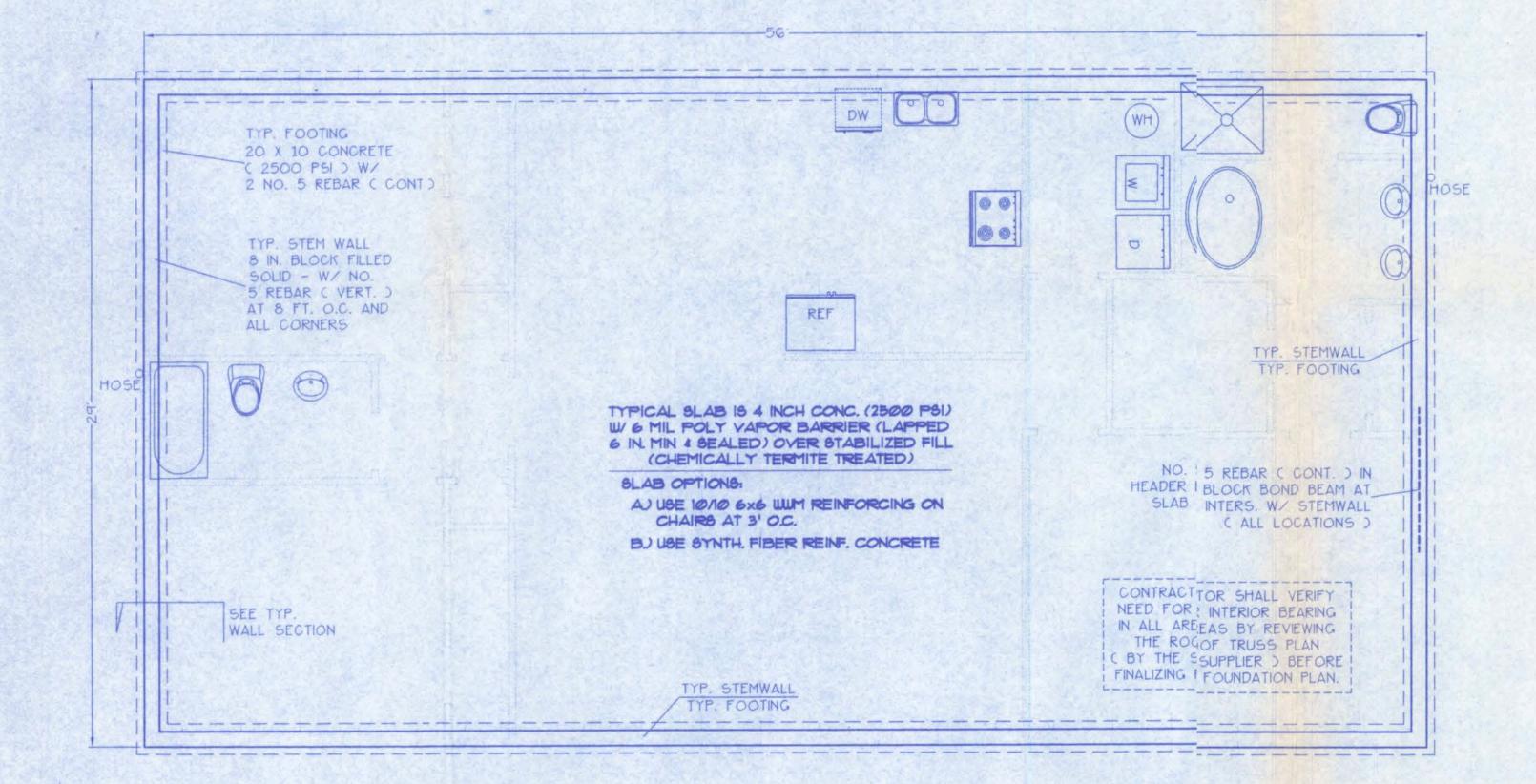
192 5W Sagewood Gln. Lake City. FL 32024

Phone (38G) 755-5891

08MAR07

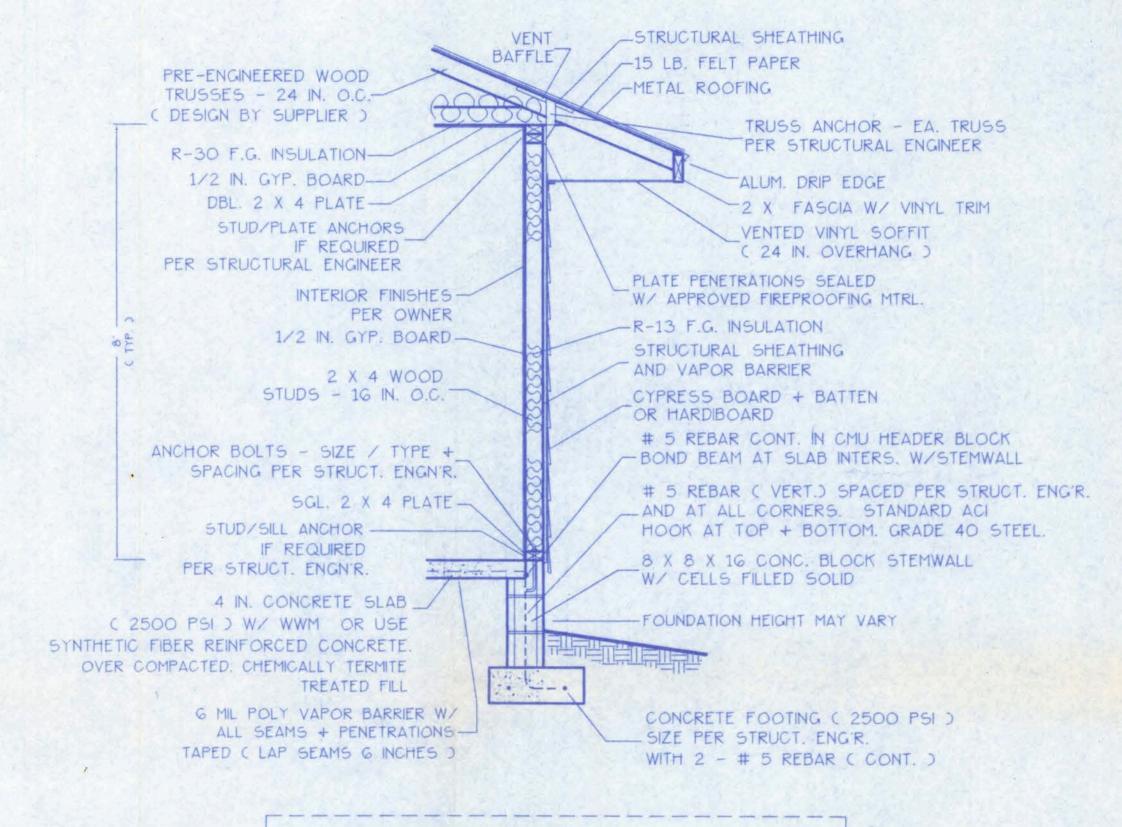
FOUNDATION NOTES:

- CONTRACTOR SHALL EXAMINE ROOF TRUSS PLAN
 (BY SUPPLIER) TO DETERMINE ANY ADDITIONAL
 BEARING REQUIREMENTS BEFORE FINALIZING THE
 FOUNDATION PLAN.
- ALL CONCRETE IS 2500 PSI STRENGTH (MIN.)
- VERIFY DIMENSIONS WITH FLOOR PLAN
- SITE ANALYSIS AND PREPARATION DATA IS NOT A PART OF THIS PLAN AND IS THE RESPONSIBLITY OF THE CONTRACTOR / OWNER.



FOUNDATION PLAN

SCALE: 1/4 IN. = 1 FT.



WALL SECTION NOTES:

- This Typical Wall Section is for Estimating purposes only.
- All data shown in this Wall Section shall be subject to review and final input by the Structural Engineer.

DESIGN WALL SECTION

NON-STRUCTURAL DATA

SCALE: 1/2 IN. = 1 FT.

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

MORRELL ROAD - SECTION Location: 11, T-2-9, R-16-E.

T-2-9, R-16-E. Job No.:___

A-3	
BARRY	SHEET: 1 OF 4
RESIDENCE	CAD FILE: 07002
PREPARED BY: TIM DELBENE Drafting + Technical Services	REV:

192 SW Sagewood Gh. Lake City. FL 32024 REV:

Phone (386) 755-5891

08MAR07/

My your

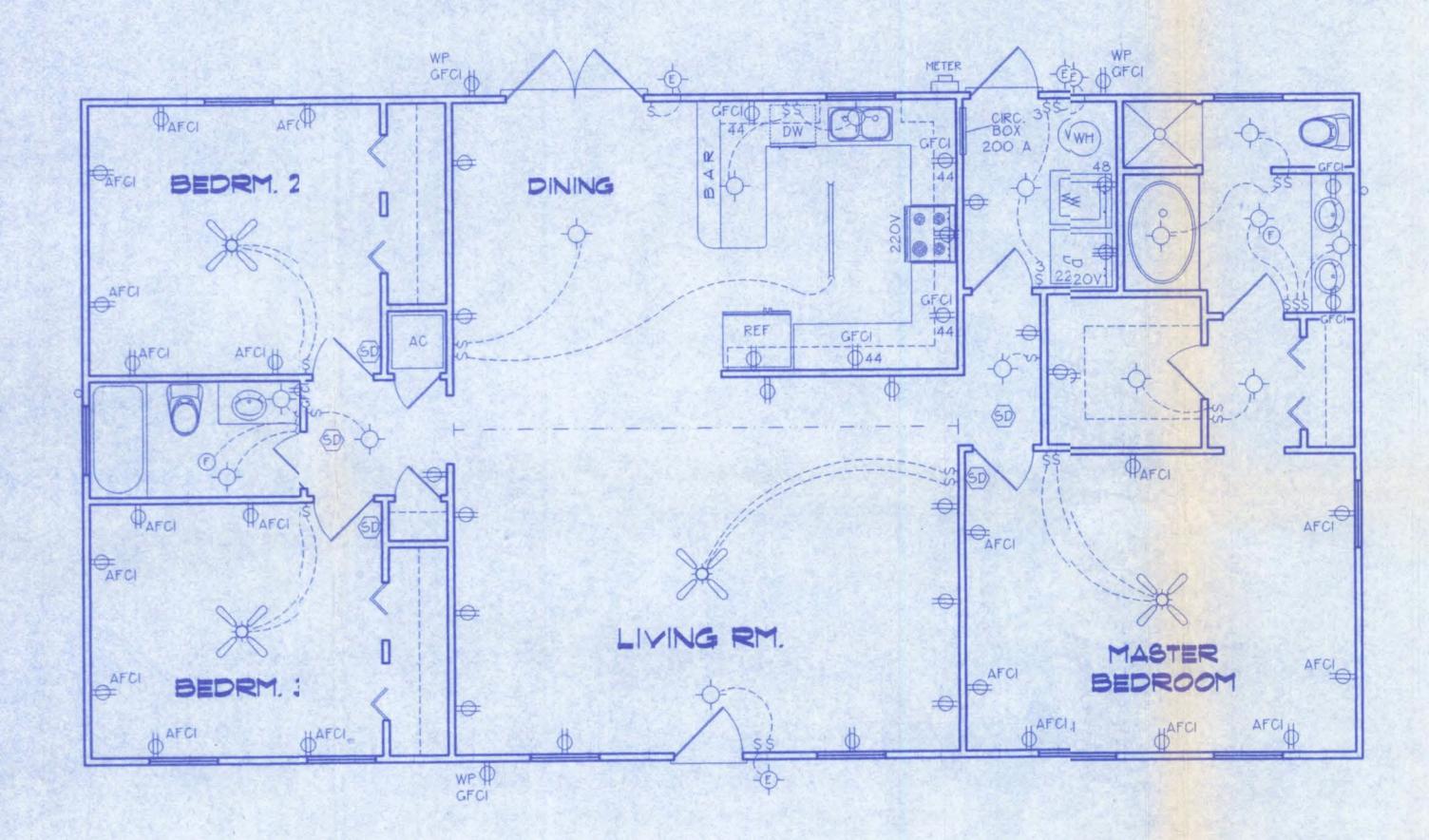
07-002

2-20-07

TAD

RAWN:

CHECK:



ELECTRICAL PLAN
NOT TO SCALE

ELECTRICAL SYMBOL LEGEND				
-	= FLOURESCENT LIGHTING FIXTURE.			
\(\rightarrow \)	= CEILING LIGHT FIXTURE			
-\$	= EXTERIOR LIGHTING FIXTURE			
\$	= LIGHT SWITCH.			
\$3	= THREE-WAY SWITCH.			
ф	= 110 V. DUPLEX OUTLET.			
\$42	= SPECIAL HEIGHT 110 V. DUPLEX OUTLET			
Ø ^{GFCI}	= GROUND FAULT CIRC. OUTLET			
∯ ^{AFCI}	= ARC FAULT CIRC. OUTLET			
ф	= 110 V. SINGLE RECEPTACLE OUTLET.			
€ 220V	= 220 VOLT OUTLET (4 WIRE)			
×	= FAN LOCATION (CEILING)			
Ð	= FAN LOCATION C EXHAUST)			
SD	= SMOKE DETECTOR			

ELECTRICAL PLAN NOTES

- -WIRE ALL APPLIANCES. HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- -CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- -ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- -ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE. AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- -TELEPHONE. TELEVISION AND OTHER LOW VOLTAGE
 DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S
 DIRECTIONS. + IN ACCORDANCE W/ APPLICABLE
 SECTIONS OF NEC-LATEST EDITION.
- -ELECTRICAL CONTR SHALL BE RESPONSIBLE FOR THE DESIGN + SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- -ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD)
 TO BE DETERMINED BY POWER COMPANY.



MORRELL ROAD - SECTION 11, T-2-9, R-16-E.

FILE: 07-002 DATE: 2-20-07	BARRY RESIDENCE	SHEET: 1 CF 4 CAD FILE: 07002			
DRAWN: T A D	PREPARED BY: TIM DELBENE Drafting + Technical Services	REV:			
CHECK:	192 SW Sagewood Gh. Lake City. FL 32024 Phone (386) 755-5891	REV:			