DATE 12/22/2011

Columbia County Building Permit

This Permit Must Be Prominently Posted on Premises During Construction

PERMIT
000029843

APPLICANT MARY ANN CRAWFORD PHONE 752-5152

ADDRESS 1482 SW COMMERCIAL GLEN LAKE CITY FL 32025

OWNER CASON CONSTRUCTION & DEVELOPMET, INC PHONE 386-623-2806

ADDRESS 187 SW ASHEVILLE WAY LAKE CITY FL 32025

CONTRACTOR STANLEY CRAWFORD PHONE 752-5152

LOCATION OF PROPERTY 47 S, R 242, L ASHEVILLE WAY, LOT IS AT END ON LEFT

TYPE DEVELOPMENT SFD, UTILITY ESTIMATED COST OF CONSTRUCTION 131350.00

HEATED FLOOR AREA 1855.00 TOTAL AREA 2627.00 HEIGHT 21.00 STORIES 1

FOUNDATION CONCRETE WALLS FRAMED ROOF PITCH 8/12 FLOOR SLAB

LAND USE & ZONING PRD MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 10.00 SIDE 15.00

NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 25-4S-16-03124-105 SUBDIVISION HICKORY COVE

LOT 5 BLOCK PHASE UNIT TOTAL ACRES 0.34

Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor
		RG0042896	

Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident
WAIVER	11-0530	BK	TC	N

COMMENTS: NOC ON FILE

MINIMUM FLOOR ELEVATION @ 88.5 FT PER PLAT, NEED ELEVATION

CONFIRMATION LETTER AT SLAB Check # or Cash 3804

FOR BUILDING & ZONING DEPARTMENT ONLY

Temporary Power date/app. by Foundation date/app. by Monolithic (footer/Slab) date/app. by

Under slab rough-in plumbing date/app. by Slab date/app. by Sheathing/Nailing date/app. by

Framing date/app. by Insulation date/app. by

Rough-in plumbing above slab and below wood floor date/app. by Electrical rough-in date/app. by

Heat & Air Duct date/app. by Peri. beam (Lintel) date/app. by Pool date/app. by

Permanent power date/app. by C.O. Final date/app. by Culvert date/app. by

Pump pole date/app. by Utility Pole date/app. by M/H tie downs, blocking, electricity and plumbing date/app. by

Reconnection date/app. by RV date/app. by Re-roof date/app. by

BUILDING PERMIT FEE $ 660.00 CERTIFICATION FEE $ 13.13 SURCHARGE FEE $ 13.13

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ **TOTAL FEE** 761.26

INSPECTORS OFFICE L. H CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED NOT SUSPENDED, ABANDONED OR INVALID WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS OT THE PREVIOUS INSPECTION.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

☑ PATSY BOWEN LIABILITY
☑ LYMAN BEAR LIABILITY

Columbia County Building Permit Application

☑ TRUSS PAGE

For Office Use Only Application # 1112-36 Date Received 12/12 By JW Permit # 29843/1912

Zoning Official B2K Date 21 DEC 2011 Flood Zone X Land Use RES. LOW DEN Zoning PRD

FEMA Map # N/A Elevation N/A MFE 88.5 ft per plans River N/A Plans Examiner T.C. Date 12-21-11

Comments Elevation Confirmation Letter Required at Slab

☑ NOC ☐ EH ☑ Deed or PA ☐ Site Plan N/A ☑ State Road Info ☑ Well letter ☐ 911 Sheet ☐ Parent Parcel #

☐ Dev Permit # ☐ In Floodway ☐ Letter of Auth. from Contractor ☐ F W Comp. letter

IMPACT FEES: EMS Fire Corr ☐ Sub VF Form

Road/Code School = TOTAL (Suspended) ☑ App Fee Paid

✓ Septic Permit No. 11-0530 Fax 386-755-2165

Name Authorized Person Signing Permit Mary Ann Crawford Phone 386-752-5152

Address 1482 SW Commercial Glen Lake City, FL 32025

Property Owners Name Cason Construction + Development, Inc Phone 386-623-2806

✓ 911 Address 187 SW Asheville Way Lake City, FL 32025

Contractors Name Stanley Crawford Construction Phone 386-752-5152

Address 1482 SW Commercial Glen Lake City, FL 32025

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Nicholas Geisler, 1758 NW Brown Rd Lake C

Mortgage Lenders Name & Address None FL 32

Circle the correct power company – FL Power & Light – Clay Elec. – Suwannee Valley Elec. – Progress Energy

Property ID Number 35-4S-16-03124-105 Estimated Cost of Construction $ 75,000.00

Subdivision Name Hickory Cove Lot 5 Block ____ Unit ____ Phase ____

Driving Directions From 47S, turn right on CR242, turn left on Asheville Way. Lot is at end on left.

Number of Existing Dwellings on Property 0

Construction of single family Dwelling Total Acreage .34 Lot Size ____

Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive Total Building Height 21' 3"

Actual Distance of Structure from Property Lines - Front 39' Side 60' Side 12' Rear 44'

Number of Stories 1½ Heated Floor Area 1855 Total Floor Area 2627 Roof Pitch 8/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction. **CODE:** Florida Building Code 2007 with 2009 Supplements and the 2008 National Electrical Code.

Page 1 of 2 (Both Pages must be submitted together.) **Revised 1-11**

JW spoke w/ Lobrano 12.21.11 : RE: signed NOC plans.
(CK 3804) Permit (ck 3805 waiver)

Columbia County Building Permit Application

TIME LIMITATIONS OF APPLICATION : An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

TIME LIMITATIONS OF PERMITS: Every permit issued shall become invalid unless the work authorized by such permit is commenced within 180 days after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of 180 days after the time work is commenced. A valid permit receives an approved inspection every 180 days. Work shall be considered not suspended, abandoned or invalid when the permit has received an approved inspection within 180 days of the previous approved inspection.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment: According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full.
This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE: ***YOU ARE HEREBY NOTIFIED*** as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: I CERTIFY THAT ALL THE FOREGOING INFORMATION IS ACCURATE AND THAT ALL WORK WILL BE DONE IN COMPLIANCE WITH ALL APPLICABLE LAWS REGULATING CONSTRUCTION AND ZONING.

NOTICE TO OWNER: There are some properties that may have deed restrictions recorded upon them. These restrictions may limit or prohibit the work applied for in your building permit. You must verify if your property is encumbered by any restrictions or face possible litigation and or fines.

Stanley Crawford Matt DeCe **(Owners Must Sign All Applications Before Permit Issuance.)**

Owners Signature ****OWNER BUILDERS MUST PERSONALLY APPEAR AND SIGN THE BUILDING PERMIT.**

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit including all application and permit time limitations.

Stanley Crawford

Contractor's Signature (Permitee)

Contractor's License Number RG0042896
Columbia County
Competency Card Number 64

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 16 **day of** Dec **20** 11.

Personally known N **or Produced Identification**

Erika Cushman

State of Florida Notary Signature (For the Contractor)

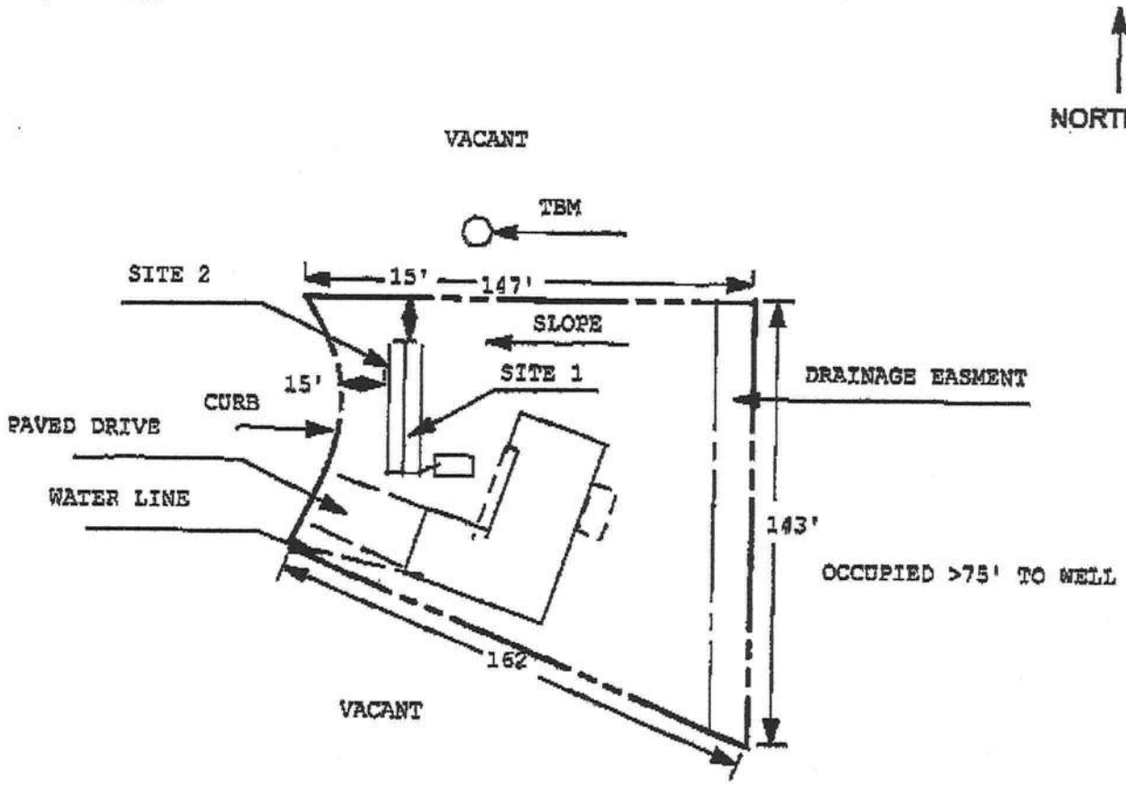
SEAL: ERIKA CUSHMAN
MY COMMISSION # DD964084
EXPIRES March 26, 2014
(407) 398-0153 FloridaNotaryService.com

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 11-3538

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

CR# 10-5336



1 inch = 50 feet

Site Plan Submitted By Karl Lloyd Date 12/13/11
Plan Approved X Not Approved ___ Date 12-21-11
By Sallie Ford- Env. Health Director CPHU
Columbia CHD

Notes:

STATE OF FLORIDA
DEPARTMENT OF HEALTH
ONSITE SEWAGE TREATMENT AND DISPOSAL SYSTEM
CONSTRUCTION PERMIT

11-5336
CR # 10-5336
PERMIT NO. 1454871
DATE PAID: 12/16/11
FEE PAID: 350.00
RECEIPT #: 1496519

CONSTRUCTION PERMIT FOR:
[X] New System [ ] Existing System [ ] Holding Tank [ ] Innovative
[ ] Repair [ ] Abandonment [ ] Temporary [ ] _____

APPLICANT: Cason Construction and Development Inc.

PROPERTY ADDRESS: 187 SW ASHEVILLE WAY

LOT: 5 BLOCK: N/A SUBDIVISION: HICKORY COVE

PROPERTY ID #: 25-4S-16-03124-105 [SECTION, TOWNSHIP, RANGE, PARCEL NUMBER] [OR TAX ID NUMBER]

SYSTEM MUST BE CONSTRUCTED IN ACCORDANCE WITH SPECIFICATIONS AND STANDARDS OF SECTION 381.0065, F.S., AND CHAPTER 64E-6, F.A.C. DEPARTMENT APPROVAL OF SYSTEM DOES NOT GUARANTEE SATISFACTORY PERFORMANCE FOR ANY SPECIFIC PERIOD OF TIME. ANY CHANGE IN MATERIAL FACTS, WHICH SERVED AS A BASIS FOR ISSUANCE OF THIS PERMIT, REQUIRE THE APPLICANT TO MODIFY THE PERMIT APPLICATION. SUCH MODIFICATIONS MAY RESULT IN THIS PERMIT BEING MADE NULL AND VOID. ISSUANCE OF THIS PERMIT DOES NOT EXEMPT THE APPLICANT FROM COMPLIANCE WITH OTHER FEDERAL, STATE, OR LOCAL PERMITTING REQUIRED FOR DEVELOPMENT OF THIS PROPERTY.

SYSTEM DESIGN AND SPECIFICATIONS

TANK: [900] GALLONS / GPD SEPTIC TANK/AEROBIC UNIT CAPACITY MULTI-CHAMBERED/IN-SERIES [ ]
[ ] GALLONS / GPD _____ CAPACITY MULTI-CHAMBERED/IN-SERIES [ ]
[ ] GALLONS GREASE INTERCEPTOR CAPACITY [MAXIMUM CAPACITY SINGLE TANK: 1250 GALLONS]
[ ] GALLONS DOSING TANK CAPACITY [ ] GALLONS @ [ ] DOSES PER 24 HRS # PUMPS [ ]

DRAINFIELD: [375] SQUARE FEET PRIMARY DRAINFIELD SYSTEM
[ ] SQUARE FEET _____ SYSTEM
TYPE SYSTEM: [X] STANDARD [ ] FILLED [ ] MOUND [ ]
CONFIGURATION: [X] TRENCH [ ] BED [ ] _____

INFIELD:
LOCATION OF BENCHMARK: NAIL IN 30" PINE TREE NORTH OF SYSTEM SITE
ELEVATION OF PROPOSED SYSTEM SITE [36] [INCHES] [BELOW] BENCHMARK/REFERENCE POINT
BOTTOM OF DRAINFIELD TO BE [66] [INCHES] [BELOW] BENCHMARK/REFERENCE POINT
FILL REQUIRED: [0.0] INCHES EXCAVATION REQUIRED: [0] INCHES

OTHER:

SPECIFICATIONS BY: PAUL LLOYD TITLE: SOIL SCIENTIST

APPROVED BY: Sallie Ford TITLE: Env Health Director COLUMBIA CHD

DATE ISSUED: 12-21-11 EXPIRATION DATE: 6-21-13

DH 4016, 08/09 (Obsoletes all previous editions which may not be used)
Incorporated: 64E-6.003, FAC

SUBCONTRACTOR VERIFICATION FORM

APPLICATION NUMBER 1112-36 CONTRACTOR Stanley Crawford PHONE 752-5152

THIS FORM MUST BE SUBMITTED PRIOR TO THE ISSUANCE OF A PERMIT

In Columbia County one permit will cover all trades doing work at the permitted site. It is **REQUIRED** that we have records of the subcontractors who actually did the trade specific work under the permit. Per Florida Statute 440 and Ordinance 89-6, a contractor shall require all subcontractors to provide evidence of workers' compensation or exemption, general liability insurance and a valid Certificate of Competency license in Columbia County.

Any changes, the permitted contractor is responsible for the corrected form being submitted to this office prior to the start of that subcontractor beginning any work. Violations will result in stop work orders and/or fines.

ELECTRICAL 380	Print Name Donald R Davis License #: EC 0006 2306	Signature Donald R Davis Phone #: 386 623 0499
MECHANICAL/ A/C 802	Print Name Clint Wilson License #: CAC057886	Signature Clint Wilson Phone #: 386-754-9408
PLUMBING/ GAS 955	Print Name Robert E. Hamilton License #: CFC057315	Signature [illegible] Phone #: 352-2452706
ROOFING	Print Name Stanley Crawford License #: 000064	Signature Stanley Crawford Phone #: 386-752-5152
SHEET METAL	Print Name License #:	Signature Phone #:
FIRE SYSTEM/ SPRINKLER	Print Name License#:	Signature Phone #:
SOLAR	Print Name License #:	Signature Phone #:

Specialty Licensing	License Number	Sub-Contractors Printed Name	Sub-Contractors Signature
MASON	000712	Colin Gay Masonry	Colin Gay
CONCRETE FINISHER	218	Jordan Concrete	Jerry Jordan
FRAMING	CCC057896	Stanley Crawford	Stanley Crawford
INSULATION	000741	SunCoast Insulators	Cathy Bowen
STUCCO	N/A	N/A	N/A
DRYWALL			
PLASTER	CG0042896	Stanley Crawford	Stanley Crawford
CABINET INSTALLER	000064	Stanley Crawford Const	Stanley Crawford
PAINTING	000064	Stanley Crawford Const	Stanley Crawford
ACOUSTICAL CEILING		N/A	
GLASS			
CERAMIC TILE	CG0042896	Stanley Crawford	Stanley Crawford
FLOOR COVERING	CG0042896	Stanley Crawford	Stanley Crawford
ALUM/VINYL SIDING	000312	Columbia Exteriors	Paul R.
GARAGE DOOR	COC125116	LAMAR BEAR	[illegible]
METAL BLDG ERECTOR			

F. S. 440.103 Building permits; identification of minimum premium policy.--Every employer shall, as a condition to applying for and receiving a building permit, show proof and certify to the permit issuer that it has secured compensation for its employees under this chapter as provided in ss. 440.10 and 440.38, and shall be presented each time the employer applies for a building permit.

CLYATT WELL DRILLING, INC.
(Established in 1971)
Post Office Box 180
Worthington Springs, FL 32697
*Phone (386)496-2488 *** FAX (386)496-4640*

WELL DESCRIPTION

DESCRIPTION DATE
8/3/2011

CUSTOMER NAME AND ADDRESS
Stanley Crawford Construction, Inc. 1482 SW Commercial Glen Lake City, Florida 32025

DESCRIPTION OF WORK
4" Well and Pump

DESCRIPTION
Feet 4" Well 1 HP Submersible Pump (18 GPM) Feet 1-1/4" Drop Pipe Feet 14/4 Submersible Pump Wire 4 X 1-1/4 Well Seal 81 Gallon Captive Air Tank (20 Gallon Drawdown) Pressure Relief Valve Controls and Fittings Sales Tax @ 7%

COLUMBIA COUNTY BUILDING DEPARTMENT
Received for FILE COPY
Code Compliance
PLANS EXAMINER

The above description is provided to give a brief description of the water well to be constructed by Clyatt Well Drilling, Inc.

FLORIDA DEPARTMENT OF Community Affairs

Product Approval
USER: Public User

FL #	FL13196
Application Type	New
Code Version	2007
Application Status	Approved
Comments	
Archived	

COLUMBIA COUNTY BUILDING DEPARTMENT
Received for FILE COPY
Code Compliance
PLANS EXAMINER

Product Manufacturer Address/Phone/Email	MARITECH WINDOWS 1813 KELLY BLVD CARROLLTON, TX 75006 (469) 568-5636 Ext 307 scrawford@maritechwindow
Authorized Signature	Luis Lomas rllomas@lrlomaspe.com
Technical Representative Address/Phone/Email	
Quality Assurance Representative Address/Phone/Email	
Category	Windows
Subcategory	Single Hung
Compliance Method	Certification Mark or Listing
Certification Agency	American Architectural Man
Validated By	American Architectural Man

Referenced Standard and Year (of Standard)	**Standard** AAMA/WDMA/CSA 101/I.S.
Equivalence of Product Standards Certified By	
Product Approval Method	Method 1 Option A
Date Submitted	10/30/2009
Date Validated	11/05/2009
Date Pending FBC Approval	11/06/2009
Date Approved	12/09/2009

Summary of Products

FL #	Model, Number or Name	Description
13196.1	310 SH PVC SINGLE HUNG WINDOW	310 SH PVC SING
Limits of Use **Approved for use in HVHZ:** No **Approved for use outside HVHZ:** Yes **Impact Resistant:** No **Design Pressure:** +50/-50 **Other:**		**Certification Age** FL13196_R0_C_C **Quality Assuran** 08/08/2012 **Installation Inst** FL13196_R0_II_C Verified By: Luis I Created by Indep **Evaluation Repo** Created by Indep
13196.2	450 SH PVC SINGLE HUNG WINDOW	450 SH PVC SING
Limits of Use **Approved for use in HVHZ:** No **Approved for use outside HVHZ:** Yes **Impact Resistant:** No **Design Pressure:** +50/-50 **Other:**		**Certification Age** FL13196_R0_C_C **Quality Assuran** 02/29/2012 **Installation Inst** FL13196_R0_II_C Verified By: Luis I Created by Indep **Evaluation Repo** Created by Indep

FLORIDA DEPARTMENT OF Community Affairs

Product Approval
USER: Public User

Search Criteria

Code Version	2007	FL#	
Application Type	ALL	Product Manufacturer	
Category	Windows	Subcategory	
Application Status	ALL	Compliance Method	
Quality Assurance Entity	ALL	Quality Assurance Entity Contract E	
Product Model, Number or Name	2100	Product Description	
Approved for use in HVHZ	ALL	Approved for use outside HVHZ	
Impact Resistant	ALL	Design Pressure	
Other	ALL		

Search Results - Applications

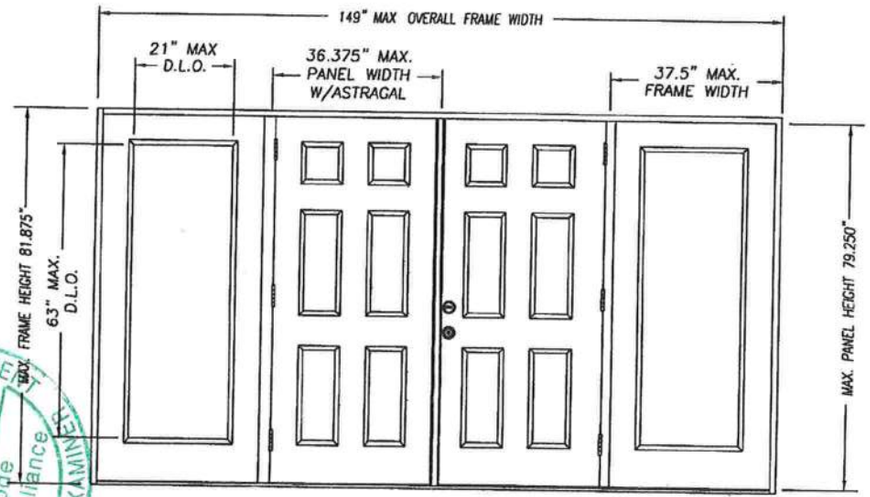
FL#	Type	Manufacturer	Validated By
FL10287-R3 History	Revision	PGT Industries **Category:** Windows **Subcategory:** Single Hung	Steven M. Urich, PE (717) 932-8500
FL10289-R2 History	Revision	PGT Industries **Category:** Windows **Subcategory:** Fixed	Keystone Certificatior (717) 932-8500
FL13867	New	PGT Industries **Category:** Windows **Subcategory:** Mullions	Steven M. Urich, PE (717) 932-8500

Department of Community Affairs
Florida Building Code Online
Codes and Standards
2555 Shumard Oak Boulevard
Tallahassee, Florida 32399-2100
(850) 487-1824, Fax (850) 414-8436

Product Approval Accepts:

Masonite

SIDE-HINGED WOOD-EDGE STEEL DOOR UNIT
6'-8" DOUBLE DOOR WITH / WITHOUT SIDELITES



DOUBLE INSWING UNIT W/SIDELITES

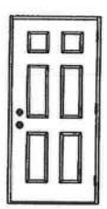
GENERAL NOTES

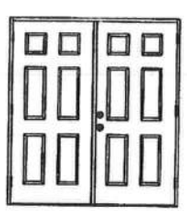
1. EVALUATED FOR USE IN LOCATIONS ADHERING TO THE FLORIDA BUILDING CODE AND WHERE PRESSURE REQUIREMENTS AS DETERMINED BY ASCE 7, MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES, DOES NOT EXCEED THE DESIGN PRESSURES LISTED
2. HURRICANE PROTECTIVE SYSTEM (SHUTTERS) IS NOT REQUIRED ON OPAQUE PANELS, BUT IS REQUIRED ON GLAZED SIDELITES
3. POLYURETHANE CORE FLAME SPREAD INDEX OF 50 AND SMOKE DEVELOPED INDEX OF 60 PER ASTM E84
4. PLASTICS TESTING OF LITE FRAME MATERIAL:

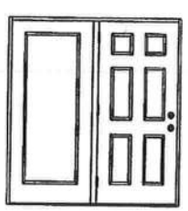
TEST DESCRIPTION	DESIGNATION	RESULT
SELF IGNITION TEMP	ASTM D1929	680 °F > 650 °F
RATE OF BURNING	ASTM D635	1.10 IN/MIN
SMOKE DENSITY	ASTM D2843	69.6%
TENSILE STRENGTH*	ASTM D638	-7.48% DIFF

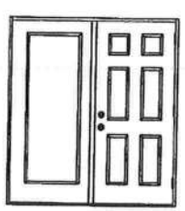
* COMPARATIVE TENSILE STRENGTH AFTER WEATHERING 4500 HOURS XENON ARC METHOD 1

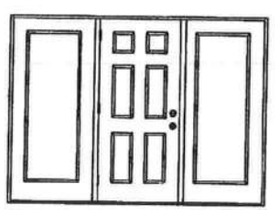
Addendum to NAMI
Certification No.: NI006110
Reviewed By:
Date Reviewed: 8/10/05


SINGLE DOOR UNIT


DOUBLE DOOR UNIT


SINGLE DOOR UNIT WITH SIDELITE


SINGLE DOOR UNIT WITH SIDELITE


SINGLE DOOR UNIT W/SIDELITES

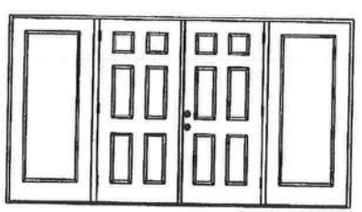

DOUBLE DOOR UNIT W/SIDELITES

TABLE OF CONTENTS

CONFIG	MAX WIDTH	DESIGN PRESSURE RATING		WHERE WATER INFILTRATION PERFORMANCE IS REQUIRED TO BE 15% OF DESIGN PRESSURE	
		INSWING	OUTSWING	INSWING	OUTSWING
X	37.5"	+76.0 / -76.0	+76.0 / -76.0	+19.0 / -19.0	+55.0 / -55.0
XX	74"	+55.0 / -55.0	+55.0 / -55.0	+19.0 / -19.0	+55.0 / -55.0
OX or XO	75"	+55.0 / -55.0	+55.0 / -55.0	+19.0 / -19.0	+55.0 / -55.0
OXO	112.5"	+55.0 / -55.0	+55.0 / -55.0	+19.0 / -19.0	+55.0 / -55.0
OXXO	149"	+55.0 / -55.0	+55.0 / -55.0	+19.0 / -19.0	+55.0 / -55.0

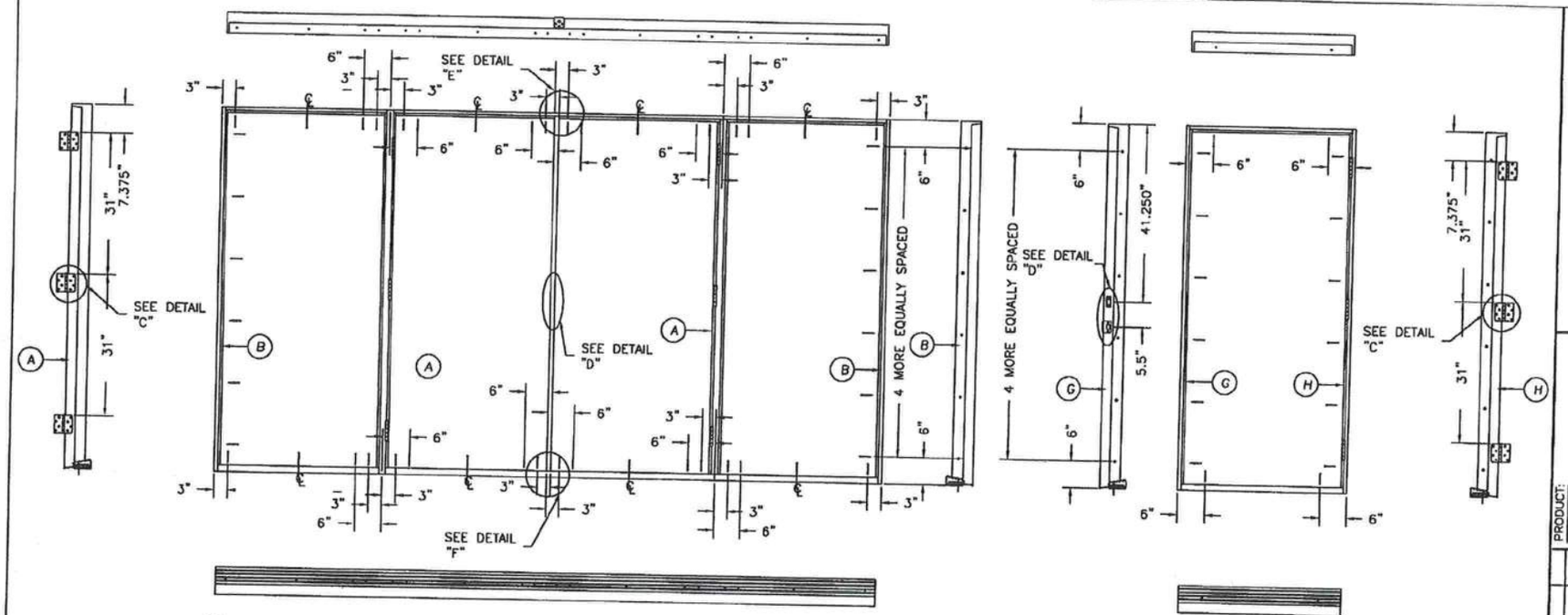
MASONITE INTERNATIONAL CORP.
7300 REAMES RD.
CHARLOTTE, NC 28216

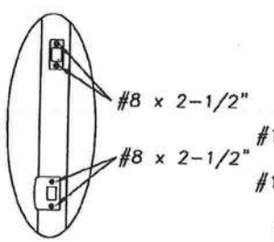
PRODUCT: "EXTERIOR DOOR PRODUCT" DOUBLE 6'8" OPAQUE WOOD-EDGE STEEL DOOR

PART OR ASSEMBLY: TYPICAL ELEVATIONS & GENERAL NOTES

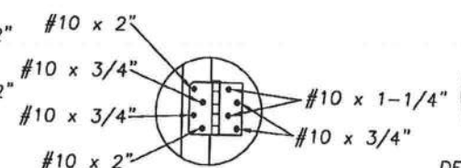
NO.	DATE	REVISIONS	BY

DATE: 7/11/05
SCALE: N.T.S.
DWG. BY: SWS
CHK. BY:
DRAWING NO.: DWG-MA-FL0128-05
SHEET 1 OF 3

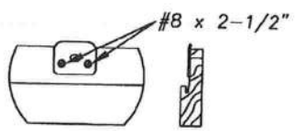




DETAIL "D"

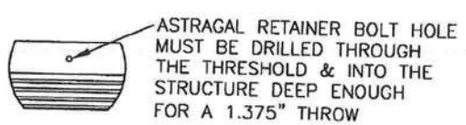


DETAIL "C"

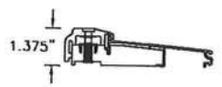


DETAIL "E" ASTRAGAL

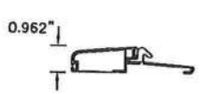
ATTACH ASTRAGAL RETAINER BOLT STRIKE PLATE TO FRAME AS SHOWN.



DETAIL "F" ASTRAGAL

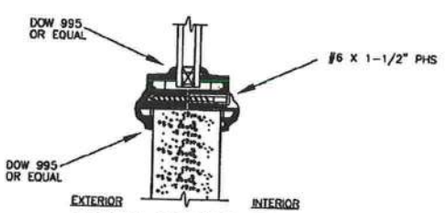


INSWING THRESHOLD



OUTSWING THRESHOLD

Addendum to NAMI
Certification No.: NT006110
Reviewed By:
Date Reviewed: 8/10/05



TYPICAL GLAZING DETAIL

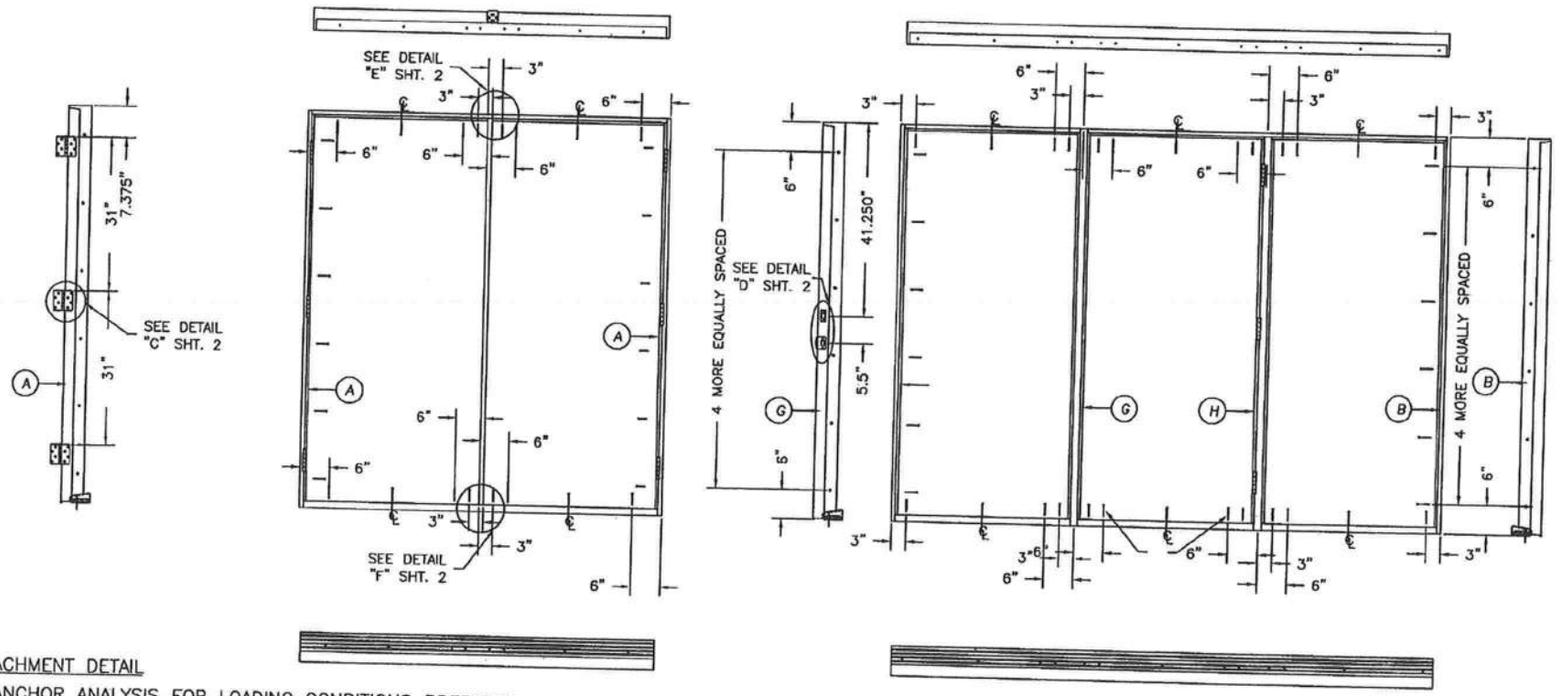
MASONITE INTERNATIONAL CORP.
7300 REAMES RD.
CHARLOTTE, NC 28216

PRODUCT: "EXTERIOR DOOR PRODUCT" DOUBLE 6'-8" OPAQUE WOOD-EDGE STEEL DOOR

PART OR ASSEMBLY: ANCHORING LOCATIONS & DETAILS

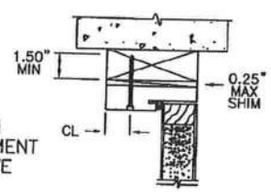
NO.	DATE	REVISIONS	BY

DATE: 7/11/05
SCALE: N.T.S.
DWG. BY: SWS
CHK. BY:
DRAWING NO.: DWG-MA-FL0128-05
SHEET 2 OF 3



ATTACHMENT DETAIL

1. ANCHOR ANALYSIS FOR LOADING CONDITIONS PREPARED, SIGNED AND SEALED BY HAROLD E. RUPP, PE (FLORIDA #15935) WITH THE LOWEST (LEAST) FASTENER RATING FROM THE DIFFERENT FASTENERS BEING CONSIDERED FOR USE. JAMB, HEAD, AND THRESHOLD FASTENERS ANALYZED FOR THIS UNIT INCLUDE #10 WOOD SCREWS OR 3/16" TAPCONS. A PHYSICAL SHIM MUST BE PLACED IN SHIM SPACE AT EACH ANCHOR LOCATION.
2. THE WOOD SCREW SINGLE SHEAR DESIGN VALUES COME FROM ANSI/AF&PA NDA FOR SOUTHERN PINE LUMBER AND ACHEIVEMENT OF 1-1/2" MINIMUM EMBEDMENT. THE TAPCON MUST ACHIEVE MINIMUM EMBEDMENT OF 1-1/4".
3. WOOD BUCKS BY OTHERS MUST BE ANCHORED PROPERLY TO TRANSFER LOADS TO STRUCTURE.
4. MINIMUM DESIGN VALUE STRENGTH OF ANCHORS 171 LBS.



TYPICAL ANCHOR INSTALLATION

HARDWARE SCHEDULE

1.	KWIKSET OR SCHLEGE ANSI/BHMA GRADE 3 OR BETTER CYLINDRICAL AND DEADLOCK HARDWARE TO BE INSTALLED AT 5-1/2" CENTERLINE.
2.	4" X 4" FULL MORTISE BUTT HINGES

Addendum to NAMI
Certification No.: NT006110
Reviewed By:
Date Reviewed: 8/10/05

MASONITE INTERNATIONAL CORP.
7300 REAMES RD.
CHARLOTTE, NC 28216

PRODUCT: "EXTERIOR DOOR PRODUCT" 6'-8" WOOD-EDGE STEEL OPAQUE DOUBLE DOOR UNIT
PART OR ASSEMBLY: ANCHORING LOCATIONS & DETAILS

NO.	DATE	REVISIONS	BY

DATE: 7/11/05
SCALE: N.T.S.
DWG. BY: SWS
CHK. BY:
DRAWING NO.: DWG-MA-FL0128-D5
SHEET 3 OF 3

PRODUCT APPROVAL

Product Type Detail

User: Public User - Not Associated with Organization -

Need Help ?

Application #:	FL4904
Date Submitted:	07/25/2005
Code Version:	2004
Product Manufacturer:	Masonite International
Address/Phone/email:	One North Dale Mabry Suite 950 Tampa, FL 33609 (615) 441-4258
Category:	Exterior Doors
Subcategory:	Swinging
Evaluation Method:	Certification Mark or Listing

Referenced Standards from the Florida Building Code:

Section	Standard	Year
	TAS 201	1994
	TAS 202	1994
	TAS 203	1994
	ASTM E1300	1998
	ASTM E1300	2002

Section
2612 HVHZ
PI

Certification Agency:	National Accreditation & Management Institute,
Quality Assurance Entity:	
Validation Entity:	
Authorized Signature:	Steve Schreiber sschreiber@masonite com

Evaluation/Test Reports Uploaded:

Installation Documents Uploaded: PTID_4904_I_Install 68 WE Glazed.pdf
PTID_4904_I_Install 68 WE Opaque.pdf
PTID_4904_I_Install 80 WE Glazed.pdf
PTID_4904_I_Install 80 WE Opaque.pdf

Product Approval Method: Method I Option A

Application Status: Approved
Date Validated: 09/27/2005
Date Approved: 10/06/2005
Date Certified to the 2004 Code:

Page: 1
Go

Page 1 / 1

App/Seq #	Product Model # or Name	Model Description	Limits of Use
4904 1	Wood-edge Steel Side-Hinged Door Units	6'-8" Opaque I/S and O/S Single Door	Evaluated for use in locations adhering to the Florida Building Code including the High Velocity Hurricane Zone, and where pressure requirements as determined by ASCE 7, Minimum Design Loads for Buildings and Other Structures, does not exceed the design pressures listed. 3'-0" x 6'-8" max nominal size Max DP = +/- 76.0. When large missile impact resistance is required, hurricane protective system is NOT required. See installation drawing DWG-MA-FL0128-05 for additional information.
4904.2	Wood-edge Steel Side-Hinged Door Units	8'-0" Opaque I/S and O/S Single Door	Evaluated for use in locations adhering to the Florida Building Code including the High Velocity Hurricane Zone, and where pressure requirements as determined by ASCE 7, Minimum Design Loads for Buildings and Other Structures, does not exceed the design pressures listed. 3'-0" x 8'-

			0" max nominal size Max DP = +/- 70.0. When large missile impact resistance is required, hurricane protective system is NOT required. See installation drawing DWG-MA-FL0129-05 for additional information.
4904.3	Wood-edge Steel Side-Hinged Door Units	6'-8" Opaque I/S and O/S Door w/ or w/o Sidelites	Evaluated for use in locations adhering to the Florida Building Code including the High Velocity Hurricane Zone, and where pressure requirements as determined by ASCE 7, Minimum Design Loads for Buildings and Other Structures, does not exceed the design pressures listed. 12'-0" x 6'-8" max nominal size. Max DP = +/- 55.0. When large missile impact resistance is required, hurricane protective system is NOT required on opaque panels, but is required on glazed panels. See installation drawing DWG-MA-FL0128-05 for additional information.
4904.4	Wood-edge Steel Side-Hinged Door Units	8'-0" Opaque I/S Door w/ or w/o Sidelites	Evaluated for use in locations adhering to the Florida Building Code including the High Velocity Hurricane Zone, and where pressure requirements as determined by ASCE 7, Minimum Design Loads for Buildings and Other Structures, does not exceed the design pressures listed 12'-0" x 8'-0" max nominal size. Max DP = + 45.0 / -50.0. When large missile impact resistance is required, hurricane protective system is NOT required on opaque panels, but is required on glazed panels. See installation drawing DWG-MA-FL0129-05 for additional information.
			Evaluated for use in

4904.5	Wood-edge Steel Side-Hinged Door Units	8'-0" Opaque O/S w/ or w/o Sidelites	locations adhering to the Florida Building Code including the High Velocity Hurricane Zone, and where pressure requirements as determined by ASCE 7, Minimum Design Loads for Buildings and Other Structures, does not exceed the design pressures listed. 12'-0" x 8'-0" max nominal size. Max DP = + 50.0 / -45.0. When large missile impact resistance is required, hurricane protective system is NOT required on opaque panels, but is required on glazed panels. See installation drawing DWG-MA-FL0129-05 for additional information.
4904.6	Wood-edge Steel Side-Hinged Door Units	6'-8" Glazed I/S and O/S Door w/ or w/o Sidelites	Evaluated for use in locations adhering to the Florida Building Code including the High Velocity Hurricane Zone, and where pressure requirements as determined by ASCE 7, Minimum Design Loads for Buildings and Other Structures, does not exceed the design pressures listed. 12'-0" x 6'-8" max nominal size. Max DP = +/- 50 5. When large missile impact resistance is required, hurricane protective system is required. See installation drawing DWG-MA-FL0130-05 for additional information.
4904.7	Wood-edge Steel Side-Hinged Door Units	8'-0" Glazed I/S Door w/ or w/o Sidelites	Evaluated for use in locations adhering to the Florida Building Code including the High Velocity Hurricane Zone, and where pressure requirements as determined by ASCE 7, Minimum Design Loads for Buildings and Other Structures, does not exceed the design pressures listed 12'-0" x 8'-0" max nominal size

			Max DP = +40 0 / -45.0. When large missile impact resistance is required, hurricane protective system is required. See installation drawing DWG-MA-FL0131-05 for additional information.
4904 8	Wood-edge Steel Side-Hinged Door Units	8'-0" Glazed O/S Door w/ or w/o Sidelites	Evaluated for use in locations adhering to the Florida Building Code including the High Velocity Hurricane Zone, and where pressure requirements as determined by ASCE 7, Minimum Design Loads for Buildings and Other Structures, does not exceed the design pressures listed. 12'-0" x 8'-0" max nominal size. Max DP = + 45.0 / -40.0. When large missile impact resistance is required, hurricane protective system is required. See installation drawing DWG-MA-FL0131-05 for additional information.

Next

Product Approval
USER: Public User

FL #	FL7154-R2
Application Type	Revision
Code Version	2007
Application Status	Approved
Comments	
Archived	☐
Product Manufacturer	TAMKO Building Products, Inc.
Address/Phone/Email	PO Box 1404 Joplin, MO 64802 (417) 624-6644 Ext 2305 kerri_eden@tamko.com
Authorized Signature	Kerri Eden kerri_eden@tamko.com
Technical Representative	Kerri Eden
Address/Phone/Email	PO Box 1404 Joplin, MO 64802 (417) 624-6644 Ext 2305 kerri_eden@tamko.com
Quality Assurance Representative	
Address/Phone/Email	
Category	Roofing
Subcategory	Asphalt Shingles
Compliance Method	Certification Mark or Listing
Certification Agency	Underwriters Laboratories Inc.
Validated By	Robert J. M. Nieminen, PE
	☑ Validation Checklist - Hardcopy Received

COLUMBIA COUNTY BUILDING DEPARTMENT
Received for FILE COPY
Code Compliance
PLANS EXAMINER

Referenced Standard and Year (of Standard)

Standard	Year
ASTM D 3161	2003
ASTM D3462	2004
ASTM D7158	2005

Equivalence of Product Standards Certified By

Product Approval Method	Method 1 Option A
Date Submitted	04/13/2010
Date Validated	05/03/2010
Date Pending FBC Approval	05/05/2010
Date Approved	06/08/2010

Summary of Products

FL #	Model, Number or Name	Description

7154.1	Heritage Vintage	A heavy weight dimensional asphalt shingle.

Limits of Use **Approved for use in HVHZ:** No **Approved for use outside HVHZ:** Yes **Impact Resistant:** N/A **Design Pressure:** N/A **Other:**	**Certification Agency Certificate** FL7154_R2_C_CAC_Tamko Service Conf FL 7154_2010.pdf FL7154_R2_C_CAC_Tamko shingles.pdf **Quality Assurance Contract Expiration Date** 03/19/2013 **Installation Instructions** FL7154_R2_II heritage vintage app inst april 10.pdf Verified By: Robert Nieminen PE 59166 Created by Independent Third Party: No **Evaluation Reports** Created by Independent Third Party:

Back Next

Department of Community Affairs
Florida Building Code Online
Codes and Standards
2555 Shumard Oak Boulevard
Tallahassee, Florida 32399-2100
(850) 487-1824, Fax (850) 414-8436

Product Approval Accepts:

Product Approval
USER: Public User

FL #	FL1956-R4
Application Type	Revision
Code Version	2007
Application Status	Approved *Approved by DCA. Approvals by DCA shall be reviewed and ratified by the POC and/or the Commission if necessary.
Comments	
Archived	☐
Product Manufacturer	TAMKO Building Products, Inc.
Address/Phone/Email	PO Box 1404 Joplin, MO 64802 (417) 624-6644 Ext 2305 kerri_eden@tamko.com
Authorized Signature	Kerri Eden kerri_eden@tamko.com
Technical Representative	Kerri Eden
Address/Phone/Email	PO Box 1404 Joplin, MO 64802 (417) 624-6644 Ext 2305 kerri_eden@tamko.com
Quality Assurance Representative	
Address/Phone/Email	
Category	Roofing
Subcategory	Asphalt Shingles
Compliance Method	Certification Mark or Listing
Certification Agency	Underwriters Laboratories Inc.
Validated By	Robert J. M. Nieminen, PE ☑ Validation Checklist - Hardcopy Received

Referenced Standard and Year (of Standard)

Standard	Year
ASTM D3161	2003
ASTM D3462	2004
ASTM D7158	2005

Equivalence of Product Standards Certified By	
Product Approval Method	Method 1 Option A
Date Submitted	02/18/2011
Date Validated	02/23/2011
Date Pending FBC Approval	
Date Approved	02/27/2011

Summary of Products

FL #	Model, Number or Name	Description
1956.1	Elite Glass-Seal	A three tab asphalt shingle.
Limits of Use **Approved for use in HVHZ:** No **Approved for use outside HVHZ:** Yes **Impact Resistant:** N/A **Design Pressure:** N/A **Other:** Asphalt shingles shall be used only on roof slopes of 2:12 or greater.		**Certification Agency Certificate** FL1956_R4_C_CAC_Tamko Service Conf FL 1956_2010.pdf FL1956_R4_C_CAC_Tamko shingles.pdf **Quality Assurance Contract Expiration Date** 03/19/2013 **Installation Instructions** FL1956_R4_II_glass_seal_elite_app_inst_april_10.pdf FL1956_R4_II_glass_seal_elite_app_inst_f_april_10.pdf Verified By: Robert Nieminen 59166 Created by Independent Third Party: No **Evaluation Reports** Created by Independent Third Party:
1956.2	Glass-Seal	A three tab asphalt shingle.
Limits of Use **Approved for use in HVHZ:** No **Approved for use outside HVHZ:** Yes **Impact Resistant:** N/A **Design Pressure:** N/A **Other:** Asphalt shingles shall be used only on roof slopes of 2:12 or greater.		**Certification Agency Certificate** FL1956_R4_C_CAC_Tamko Service Conf FL 1956_2010.pdf FL1956_R4_C_CAC_Tamko shingles.pdf **Quality Assurance Contract Expiration Date** 03/19/2013 **Installation Instructions** FL1956_R4_II_glass_seal_elite_app_inst_april_10.pdf Verified By: Robert Nieminen PE 59166 Created by Independent Third Party: No **Evaluation Reports** Created by Independent Third Party:
1956.3	Heritage	A dimensional asphalt shingle.
Limits of Use **Approved for use in HVHZ:** No **Approved for use outside HVHZ:** Yes **Impact Resistant:** N/A **Design Pressure:** N/A **Other:**		**Certification Agency Certificate** FL1956_R4_C_CAC_fbc 1956 - ul third party inspections.pdf FL1956_R4_C_CAC_ul ltr - shingle name change.pdf **Quality Assurance Contract Expiration Date** 03/19/2013 **Installation Instructions** FL1956_R4_II_heritage_app_inst.pdf FL1956_R4_II_heritage_app_inst_frederick.pdf Verified By: Underwriters Laboratories Inc. Created by Independent Third Party: **Evaluation Reports** Created by Independent Third Party:
1956.4	Heritage 30	A dimensional asphalt shingle.
Limits of Use **Approved for use in HVHZ:** No **Approved for use outside HVHZ:** Yes **Impact Resistant:** N/A **Design Pressure:** N/A **Other:** Asphalt shingles shall be used only on roof slopes of 2:12 or greater.		**Certification Agency Certificate** FL1956_R4_C_CAC_Tamko Service Conf FL 1956_2010.pdf FL1956_R4_C_CAC_Tamko shingles.pdf **Quality Assurance Contract Expiration Date** 03/19/2013 **Installation Instructions** FL1956_R4_II_heritage_30_app_inst_april_10.pdf FL1956_R4_II_heritage_30_app_inst_f_april_10.pdf Verified By: Robert Nieminen PE 59166 Created by Independent Third Party: No **Evaluation Reports** Created by Independent Third Party:
1956.5	Heritage 50	A dimensional asphalt shingle.
Limits of Use **Approved for use in HVHZ:** No **Approved for use outside HVHZ:** Yes **Impact Resistant:** N/A **Design Pressure:** N/A **Other:** Asphalt shingles shall be used only on roof slopes of 2:12 or greater.		**Certification Agency Certificate** FL1956_R4_C_CAC_Tamko Service Conf FL 1956_2010.pdf FL1956_R4_C_CAC_Tamko shingles.pdf **Quality Assurance Contract Expiration Date** 03/19/2013 **Installation Instructions** FL1956_R4_II_heritage_50_app_inst_april_10.pdf FL1956_R4_II_heritage_50_app_inst_f_april_10.pdf Verified By: Robert Nieminen PE 59166 Created by Independent Third Party: No **Evaluation Reports** Created by Independent Third Party:
1956.6	Heritage Premium	A dimensional asphalt shingle.
Limits of Use **Approved for use in HVHZ:** No **Approved for use outside HVHZ:** Yes **Impact Resistant:** N/A **Design Pressure:** N/A **Other:**		**Certification Agency Certificate** FL1956_R4_C_CAC_fbc 1956 - ul third party inspections.pdf FL1956_R4_C_CAC_ul ltr - shingle name change.pdf **Quality Assurance Contract Expiration Date** 03/19/2013 **Installation Instructions** FL1956_R4_II_heritage_prem_app_inst.pdf FL1956_R4_II_heritage_prem_app_inst_frederick.pdf Verified By: Underwriters Laboratories Inc.

		Created by Independent Third Party: **Evaluation Reports** Created by Independent Third Party:
1956.7	Heritage XL	A dimensional asphalt shingle.
Limits of Use **Approved for use in HVHZ:** No **Approved for use outside HVHZ:** Yes **Impact Resistant:** N/A **Design Pressure:** N/A **Other:** Asphalt shingles shall be used only on roof slopes of 2:12 or greater.		**Certification Agency Certificate** FL1956_R4_C_CAC_Tamko_Service_Conf_FL_1956_2010.pdf FL1956_R4_C_CAC_Tamko_shingles.pdf **Quality Assurance Contract Expiration Date** 03/19/2013 **Installation Instructions** FL1956_R4_II_heritage_xl_app_inst_d_april_10.pdf FL1956_R4_II_heritage_xl_app_inst_f_april_10.pdf Verified By: Robert Nieminen PE 59166 Created by Independent Third Party: No **Evaluation Reports** Created by Independent Third Party:

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Department of Community Affairs
Florida Building Code Online
Codes and Standards
2555 Shumard Oak Boulevard
Tallahassee, Florida 32399-2100
(850) 487-1824, Fax (850) 414-8436

Product Approval Accepts:

WILSON
HEAT & AIR, INC.
Sales • Service • Installation
License No. CACO 57886

Load Short Form
Entire House
WILSON HEAT & AIR, INC

Job: SPEC HOUSE
Date: NOVEMBER 29, 2011
By: KIM SHATTO

P O BOX 531, LAKE BUTLER, FL 32054 Phone: 386-496-9000 Fax: 386-754-1998 License: CAC057886

Project Information

For: SCCI
1482 SW COMMERCIAL GLEN, LAKE CITY, FL 32025

Design Information

	Htg	Clg
Outside db (°F)	33	92
Inside db (°F)	70	75
Design TD (°F)	37	17
Daily range	-	M
Inside humidity (%)	30	50
Moisture difference (gr/lb)	11	52

Infiltration

Method	Simplified
Construction quality	Average
Fireplaces	0

HEATING EQUIPMENT

Make Ruud
Trade RUUD SERIES
Model 13PJL42A01
AHRI ref no3847249

Efficiency	8.5 HSPF	
Heating input		
Heating output	41500	Btuh @ 47°F
Temperature rise	24	°F
Actual air flow	1600	cfm
Air flow factor	0.046	cfm/Btuh
Static pressure	0	in H2O
Space thermostat		

COOLING EQUIPMENT

Make Ruud
Trade RUUD SERIES
Cond 13PJL42A01
Coil RHSL-HM4217JA
AHRI ref no3847249

Efficiency	10.6 EER, 13 SEER	
Sensible cooling	28700	Btuh
Latent cooling	12300	Btuh
Total cooling	41000	Btuh
Actual air flow	1600	cfm
Air flow factor	0.041	cfm/Btuh
Static pressure	0	in H2O
Load sensible heat ratio	0.93	

ROOM NAME	Area (ft²)	Htg load (Btuh)	Clg load (Btuh)	Htg AVF (cfm)	Clg AVF (cfm)
MASTER WIC	80	1791	1795	82	73
MASTER BEDROOM	210	2649	2817	121	115
MASTER BATH	216	3622	4574	166	186
UTILITY/STAIRS	60	287	151	13	6
KITCHEN	154	2061	2002	94	81
DINING	132	766	530	35	22
GREAT ROOM	270	4215	5695	193	232
FOYER	70	1715	1651	78	67
BEDROOM # 3	150	3359	3496	154	142
CLOSET #3	30	500	581	23	24
BATH	72	1145	1364	52	56
LINEN	6	73	101	3	4
BEDROOM # 2	150	3268	3559	149	145
HALL	27	326	456	15	19
BONUS ROOM	441	9202	10537	421	429

Calculations approved by ACCA to meet all requirements of Manual J 8th Ed.

Entire House	d	2068	34980	39310	1600	1600
Other equip loads			0	0		
Equip. @ 0.97 RSM				38130		
Latent cooling				2826		
TOTALS		2068	34980	40957	1600	1600

Calculations approved by ACCA to meet all requirements of Manual J 8th Ed.

AHRI CERTIFIED™
www.ahridirectory.org

Certificate of Product Ratings

AHRI Certified Reference Number: 3847249 Date: 11/29/2011

Product: Split System: Heat Pump with Remote Outdoor Unit-Air-Source
Outdoor Unit Model Number: 13PJL42JA01
Indoor Unit Model Number: RHSL-HM4221+RCSL-H*4821
Manufacturer: RHEEM MANUFACTURING COMPANY
Trade/Brand name: SURE

Manufacturer responsible for the rating of this system combination is RHEEM MANUFACTURING COMPANY

Rated as follows in accordance with AHRI Standard 210/240-2008 for Unitary Air-Conditioning and Air-Source Heat Pump Equipment and subject to verification of rating accuracy by AHRI-sponsored, independent, third party testing:

Cooling Capacity (Btuh):	41000
EER Rating (Cooling):	11.05
SEER Rating (Cooling):	13.00
Heating Capacity(Btuh) @ 47 F:	41500
Region IV HSPF Rating (Heating):	8.50
Heating Capacity(Btuh) @ 17 F:	27600

* Ratings followed by an asterisk (*) indicate a voluntary rerate of previously published data, unless accompanied with a WAS, which indicates an involuntary rerate.

CERTIFICATE VERIFICATION
The information for the model cited on this certificate can be verified at www.ahridirectory.org, click on "Verify Certificate" link and enter the AHRI Certified Reference Number and the date on which the certificate was issued, which is listed above, and the Certificate No., which is listed below.

AHRI Air-Conditioning, Heating, and Refrigeration Institute

CERTIFICATE NO.: 129670541623944619

C.H.I. OVERHEAD DOORS
WINDBREAKER SERIES

Zone 3 Supplemental Instructions

Pan Doors: Raised Panel

18'-0" wide

Design pressure: 18.7 pos / 20.8 neg
Test pressure: 28.1 pos / 31.2 neg

⚠ CAUTION	**Higher wind pressures and larger doors require additional reinforcement.** **Premature failure of door system may result from improper application.** **Use these instructions only for the wind pressures and door sizes as listed above.**

⚠ WARNING	**These supplemental instructions do not contain basic door installation steps and related safety information.** **Failure to follow basic installation steps and related safety information may result in injury or death.** **Door installers must follow a primary instruction manual for basic door installation steps and related safety information.**

Garage door reinforcement details include:

- Top fixture type and attachment.
- Strut attachment.
- Flag bracket attachment to the wall and track system.
- Track bracket quantity and placement.
- End hinge type and attachment.
- Strut type and placement.

A locking system must be installed if the door is not electrically operated.

Stop molding is required. A minimum 2-1/2" long nail or screw must be used on an 8" spacing.

The correct selection of door and framing materials is the responsibility of the building owner/designer following local building code directives. Use of a reinforced garage door does not constitute automatic compliance with any building code. Local building code officials determine compliance criteria.

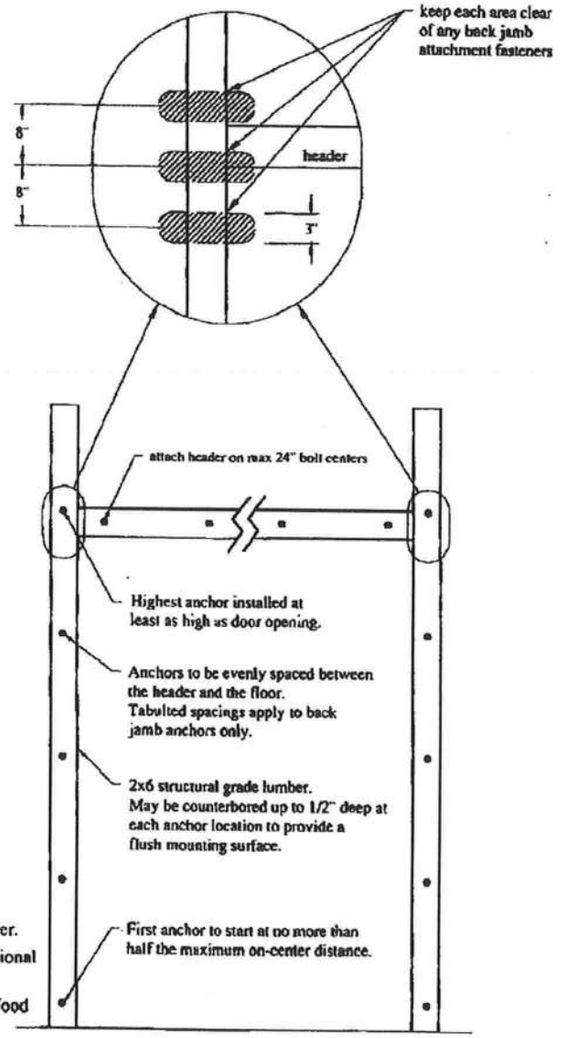
6/18/08
John E. Sopec, P.E.
1411 LeMay Street #205
Carrollton, Texas 75007
Florida P.E. # 51737
This document includes 4 pages.
Professional Engineer's seal provided only for verification of windload construction details

FL 10474

2000 psi minimum poured concrete wall; allowable load per anchor = 280 lbs.
Grout-filled (min 2000 psi concrete) C90 block; allowable load per anchor = 154 lbs.
Southern Yellow Pine specific gravity = 0.55
Spruce Pine Fir specific gravity = 0.42
Anchor: 1/4" dia x 3" long ITW Ramset / Redhead Tapcon.
Washer: 1" O.D. / 5/32" I.D. min; must conform to ANSI B18.22.1 type B.
2-1/2" minimum lag screw edge distance required.
1-1/2" minimum embedment required.
Maximum spacing shown in chart. Lesser spacing may be used to avoid interference with door component system.
Spring pad connection not included.
Maximum load per jamb = 0.5 x door width x max positive pressure x door height

Door Width	Zone / Max Positive Pressure	Maximum Fastener On-Center Spacing	
		C-90 Block *	2000 PSI Concrete *
9	Zone 1 / 12.8	24	24
	Zone 2 / 15.9	24	24
	Zone 3 / 19.5	22	24
	Zone 4 / 23.2	18	24
	Zone 5 / 27.3	16	24
	Zone 6 / 31.4	14	24
	Zone 7 / 36.0	12	21
	Zone 8 / 38.7	11	20
	Zone 9 / 44.3	10	17
	Zone 10 / 50.7	9	15
10	Zone 1 / 12.8	24	24
	Zone 2 / 15.9	24	24
	Zone 3 / 19.5	19	24
	Zone 4 / 23.2	16	24
	Zone 5 / 27.3	14	24
	Zone 6 / 31.4	12	22
	Zone 7 / 36.0	11	19
	Zone 8 / 38.7	10	18
	Zone 9 / 44.3	9	16
	Zone 10 / 50.7	8	14

Door Width	Zone / Max Positive Pressure	Maximum Fastener On-Center Spacing	
		C-90 Block *	2000 PSI Concrete *
16	Zone 1 / 12.4	19	24
	Zone 2 / 15.3	16	24
	Zone 3 / 18.7	13	23
	Zone 4 / 22.2	11	19
	Zone 5 / 26.2	9	17
	Zone 6 / 30.1	8	14
	Zone 7 / 34.5	7	13
	Zone 8 / 39.5	6	11
	Zone 9 / 42.1	6	10
	Zone 10 / 48.2	5	9
18	Zone 1 / 12.4	17	24
	Zone 2 / 15.3	14	24
	Zone 3 / 18.7	11	20
	Zone 4 / 22.2	10	17
	Zone 5 / 26.2	8	15
	Zone 6 / 30.1	7	13
	Zone 7 / 34.5	6	11
	Zone 8 / 39.5	6	10
	Zone 9 / 42.1	5	9
	Zone 10 / 48.2	5	8



* 2000 psi minimum poured concrete wall. Wood back jamb material is the limiting factor in fastener specification.

Alternative design may be approved by a registered professional engineer.

Supporting structural elements shall be designed by a registered professional engineer for wind loads in addition to other loads.

Calculation reference: DASMA TDS #161A, 2005 AF&PA NDS for Wood Construction and ITW Ramset / Redhead data.

John E. Scott, P.E. 6/18/08
1411 LeMay Street #208
Carrollton, Texas 75007
Florida P.E. # 51737

Professional Engineer's seal provided only for verification of windload construction details

C.H.I. OVERHEAD DOORS

SCALE: none

DATE: 02-12-2008

Back Jamb Attachment Detail
Self-Tapping Concrete Anchor

C.H.I. Drawing: BJA-002-03

C.H.I. OVERHEAD DOORS
WINDBREAKER SERIES

Zone 3 Supplemental Instructions

Pan Doors: Raised Panel

18'-0" wide

Design pressure: 18.7 pos / 20.8 neg
Test pressure: 28.1 pos / 31.2 neg

⚠ CAUTION	**Higher wind pressures and larger doors require additional reinforcement.** **Premature failure of door system may result from improper application.** **Use these instructions only for the wind pressures and door sizes as listed above.**

⚠ WARNING	**These supplemental instructions do not contain basic door installation steps and related safety information.** **Failure to follow basic installation steps and related safety information may result in injury or death.** **Door installers must follow a primary instruction manual for basic door installation steps and related safety information.**

Garage door reinforcement details include:

- Top fixture type and attachment.
- Strut attachment.
- Flag bracket attachment to the wall and track system.
- Track bracket quantity and placement.
- End hinge type and attachment.
- Strut type and placement.

A locking system must be installed if the door is not electrically operated.

Stop molding is required. A minimum 2-1/2" long nail or screw must be used on an 8" spacing.

The correct selection of door and framing materials is the responsibility of the building owner/designer following local building code directives. Use of a reinforced garage door does not constitute automatic compliance with any building code. Local building code officials determine compliance criteria.

6/19/08

John E. Scott, P.E.
1411 LeMay Street #205
Carrollton, Texas 75007
Florida P.E. # 61737

This document includes 4 pages.
Professional Engineer's seal provided only for verification of windload construction details

FL 10474

2000 psi minimum poured concrete wall.

Southern Yellow Pine specific gravity = 0.55; allowable load per anchor = 364 lbs.

Spruce Pine Fir specific gravity = 0.42; allowable load per anchor = 274 lbs.

Anchor: 3/8" dia x 3-1/2" long Simpson Strong-Tie Wedge-All.

Washer: 7/8" O.D. / 7/16" I.D. min; must conform to ANSI B18.22.1 type A.

3" minimum anchor edge distance required.

1-3/4" minimum embedment required.

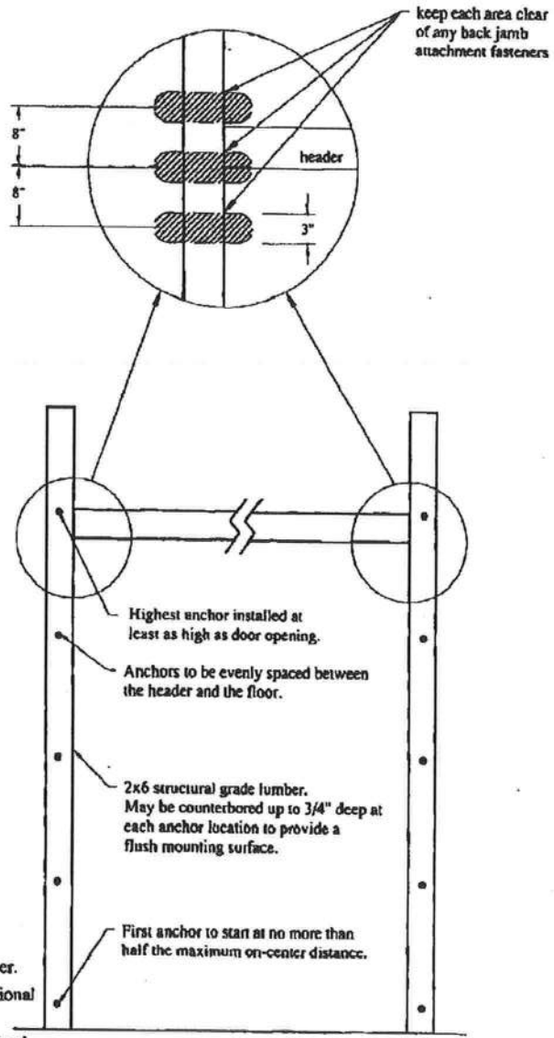
Maximum spacing shown in chart. Lesser spacing may be used to avoid interference with door component system.

Spring pad connection not included.

Maximum load per jamb = 0.5 x door width x max positive pressure x door height

Door Width	Zone / Max Positive Pressure	Maximum Fastener On-Center Spacing	
		Southern Yellow Pine*	Spruce Pine Fir*
9	Zone 1 / 12.8	24	24
	Zone 2 / 15.9	24	24
	Zone 3 / 19.5	24	24
	Zone 4 / 23.2	24	24
	Zone 5 / 27.3	24	24
	Zone 6 / 31.4	24	24
	Zone 7 / 36.0	24	21
	Zone 8 / 38.7	24	19
	Zone 9 / 44.3	22	17
	Zone 10 / 50.7	20	15
10	Zone 1 / 12.8	24	24
	Zone 2 / 15.9	24	24
	Zone 3 / 19.5	24	24
	Zone 4 / 23.2	24	24
	Zone 5 / 27.3	24	24
	Zone 6 / 31.4	24	21
	Zone 7 / 36.0	24	19
	Zone 8 / 38.7	23	17
	Zone 9 / 44.3	20	15
	Zone 10 / 50.7	18	13

Door Width	Zone / Max Positive Pressure	Maximum Fastener On-Center Spacing	
		Southern Yellow Pine*	Spruce Pine Fir*
16	Zone 1 / 12.4	24	24
	Zone 2 / 15.3	24	24
	Zone 3 / 18.7	24	22
	Zone 4 / 22.2	24	19
	Zone 5 / 26.2	21	16
	Zone 6 / 30.1	19	14
	Zone 7 / 34.5	16	12
	Zone 8 / 39.5	14	11
	Zone 9 / 42.1	13	10
	Zone 10 / 48.2	12	9
18	Zone 1 / 12.4	24	24
	Zone 2 / 15.3	24	24
	Zone 3 / 18.7	24	20
	Zone 4 / 22.2	22	17
	Zone 5 / 26.2	19	14
	Zone 6 / 30.1	17	13
	Zone 7 / 34.5	15	11
	Zone 8 / 39.5	13	10
	Zone 9 / 42.1	12	9
	Zone 10 / 48.2	11	8



* 2000 psi minimum poured concrete wall. Wood back jamb material is the limiting factor in faster specification.

Alternative design may be approved by a registered professional engineer.

Supporting structural elements shall be designed by a registered professional engineer for wind loads in addition to other loads.

Calculation reference: DASMA TDS #161C, 2005 AF&PA NDS for Wood Construction and Simpson Strong-Tie data.

John E. Soates, P.E. 6/18/08
1411 LeMay Street #205
Carrollton, Texas 75007
Florida P.E. # 51737

Professional Engineer's seal provided only for verification of windload construction details

C.H.I. OVERHEAD DOORS

SCALE: none

DATE: 02-12-2008

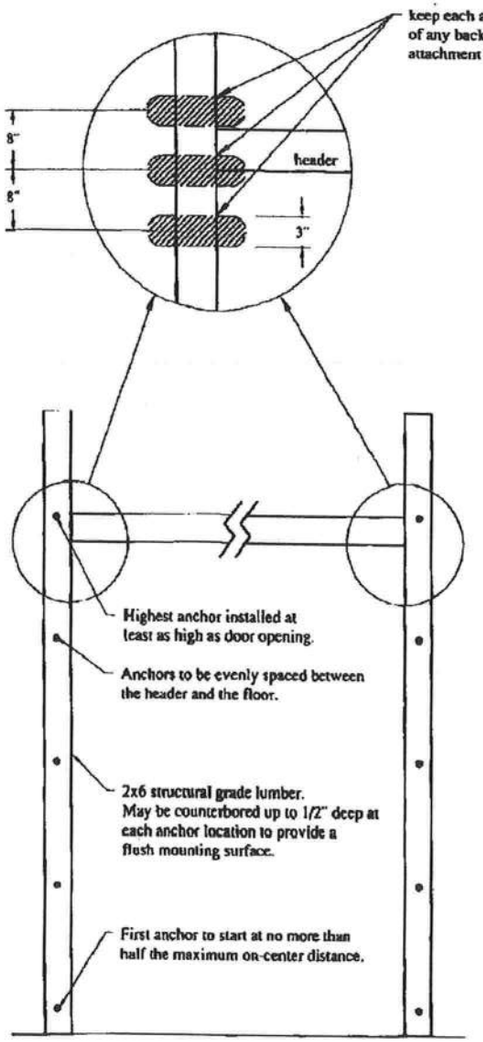
Back Jamb Attachment Detail
Concrete Expansion Anchor

C.H.I. Drawing: BJA-003-03

Southern Yellow Pine specific gravity = 0.55; allowable load per anchor = 577 lbs.
Spruce Pine Fir specific gravity = 0.42; allowable load per anchor = 385 lbs.
Lag screw: 3/8" dia x 3" long ; must conform to ANSI/ASME B18.2.1
Washer: 1-1/8" O.D. / 3/8" I.D. min; must conform to ANSI B18.22.1 type A.
Pre-drill 1/4" dia pilot holes for lag screw insertion.
1-1/2" minimum lag screw edge distance required.
1-1/2" minimum embedment required.
Maximum spacing shown in chart. Lesser spacing may be used to avoid interference with door component system.
Spring pad connection not included.
Maximum load per jamb = 0.5 x door width x max positive pressure x door height

Door Width	Zone / Max Positive Pressure	Maximum Fastener On-Center Spacing	
		Southern Yellow Pine	Spruce Pine Fir
9	Zone 1 / 12.8	24	24
	Zone 2 / 15.9	24	24
	Zone 3 / 19.5	24	24
	Zone 4 / 23.2	24	24
	Zone 5 / 27.3	24	24
	Zone 6 / 31.4	24	24
	Zone 7 / 36.0	24	24
	Zone 8 / 38.7	24	24
	Zone 9 / 44.3	24	24
	Zone 10 / 50.7	24	21
10	Zone 1 / 12.8	24	24
	Zone 2 / 15.9	24	24
	Zone 3 / 19.5	24	24
	Zone 4 / 23.2	24	24
	Zone 5 / 27.3	24	24
	Zone 6 / 31.4	24	24
	Zone 7 / 36.0	24	24
	Zone 8 / 38.7	24	24
	Zone 9 / 44.3	24	21
	Zone 10 / 50.7	24	19

Door Width	Zone / Max Positive Pressure	Maximum Fastener On-Center Spacing	
		Southern Yellow Pine	Spruce Pine Fir
16	Zone 1 / 12.4	24	24
	Zone 2 / 15.3	24	24
	Zone 3 / 18.7	24	24
	Zone 4 / 22.2	24	24
	Zone 5 / 26.2	24	23
	Zone 6 / 30.1	24	20
	Zone 7 / 34.5	24	17
	Zone 8 / 39.5	22	15
	Zone 9 / 42.1	21	14
	Zone 10 / 48.2	18	12
18	Zone 1 / 12.4	24	24
	Zone 2 / 15.3	24	24
	Zone 3 / 18.7	24	24
	Zone 4 / 22.2	24	24
	Zone 5 / 26.2	24	20
	Zone 6 / 30.1	24	18
	Zone 7 / 34.5	23	15
	Zone 8 / 39.5	20	13
	Zone 9 / 42.1	19	13
	Zone 10 / 48.2	16	11



Alternative design may be approved by a registered professional engineer.
Supporting structural elements shall be designed by a registered professional engineer for wind loads in addition to other loads.
Calculation reference: DASMA TDS #161E and 2005 AF&PA NDS for Wood Construction.

John E. Scates, P.E. 6/18/08
1411 LeMay Street #205
Carrollton, Texas 75007
Florida P.E. # 51737

Professional Engineer's seal provided only for verification of windload construction details

C.H.I. OVERHEAD DOORS

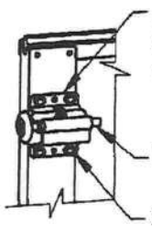
SCALE: none
DATE: 02-12-2008

Back Jamb Attachment Detail Lag Screw

C.H.I. Drawing: BJA-004-03

Top Fixture Detail

(Strut, if applicable, not shown for clarity.)

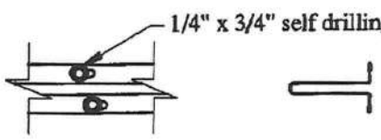


16 gauge galvanized steel top fixture. Each fixture attached with four 1/4" x 3/4" screws.

Push nut

1/4" x 3/4" self drilling screw

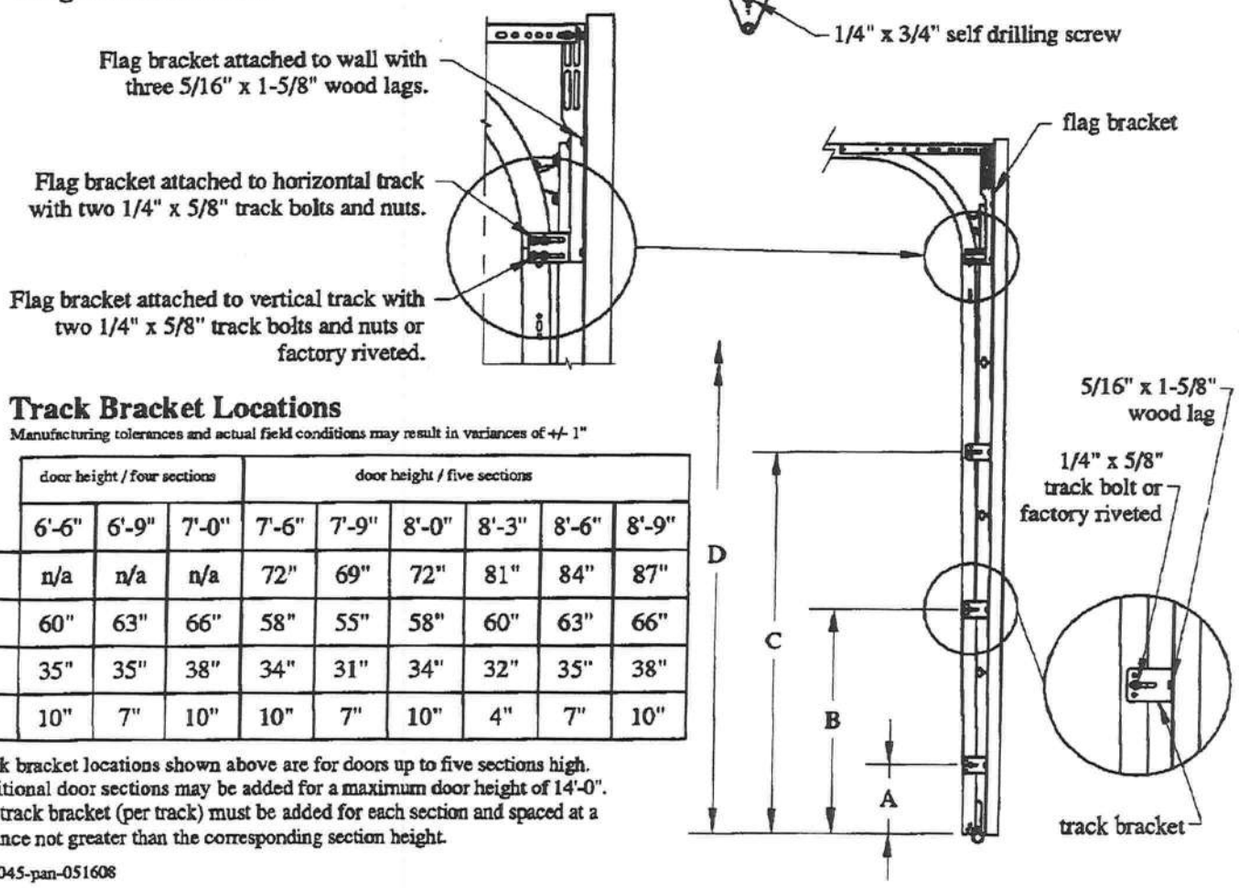
Strut Attachment Detail



1/4" x 3/4" self drilling screw

17 gauge 50ksi galvanized steel 3" strut attached with two 1/4" x 3/4" screws to every stile.

Flag Bracket Detail



Flag bracket attached to wall with three 5/16" x 1-5/8" wood lags.

Flag bracket attached to horizontal track with two 1/4" x 5/8" track bolts and nuts.

Flag bracket attached to vertical track with two 1/4" x 5/8" track bolts and nuts or factory riveted.

flag bracket

5/16" x 1-5/8" wood lag

1/4" x 5/8" track bolt or factory riveted

D

C

B

A

track bracket

Track Bracket Locations

Manufacturing tolerances and actual field conditions may result in variances of +/- 1"

	door height / four sections			door height / five sections					
	6'-6"	6'-9"	7'-0"	7'-6"	7'-9"	8'-0"	8'-3"	8'-6"	8'-9"
D	n/a	n/a	n/a	72"	69"	72"	81"	84"	87"
C	60"	63"	66"	58"	55"	58"	60"	63"	66"
B	35"	35"	38"	34"	31"	34"	32"	35"	38"
A	10"	7"	10"	10"	7"	10"	4"	7"	10"

Track bracket locations shown above are for doors up to five sections high. Additional door sections may be added for a maximum door height of 14'-0". One track bracket (per track) must be added for each section and spaced at a distance not greater than the corresponding section height.

Z3-18045-pan-051608

End Hinge Detail

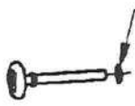
14 gauge galvanized steel end hinge fastened to section with four 1/4" x 3/4" screws.

Push nut

1/4" x 3/4" self drilling screw

Push Nut Detail (use on all rollers)

use 3/8" I. D. on bottom fixture roller stem
use 7/16" I. D. on end hinge and top fixture roller stems



Push nut: Slide roller into fixture and tap push nut onto roller stem using a 1/2" socket and hammer. Leave an 1/8" to 1/4" space between push nut and fixture.

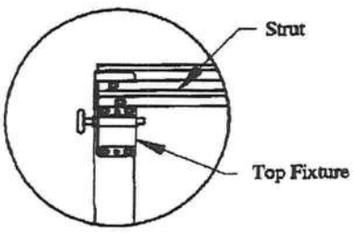
Intermediate Hinge Detail

16 gauge galvanized steel hinge fastened to section with four 1/4" x 3/4" screws.

1/4" x 3/4" self drilling screw

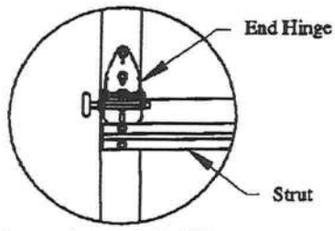
Four Section High Doors
18'-0" wide

Strut Placement



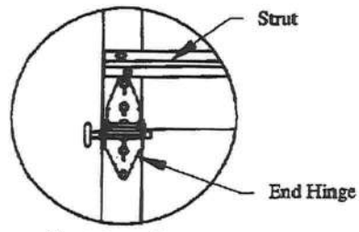
Strut above top fixture

Detail 1



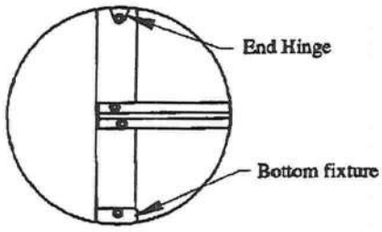
Strut on bottom half of hinge

Detail 2



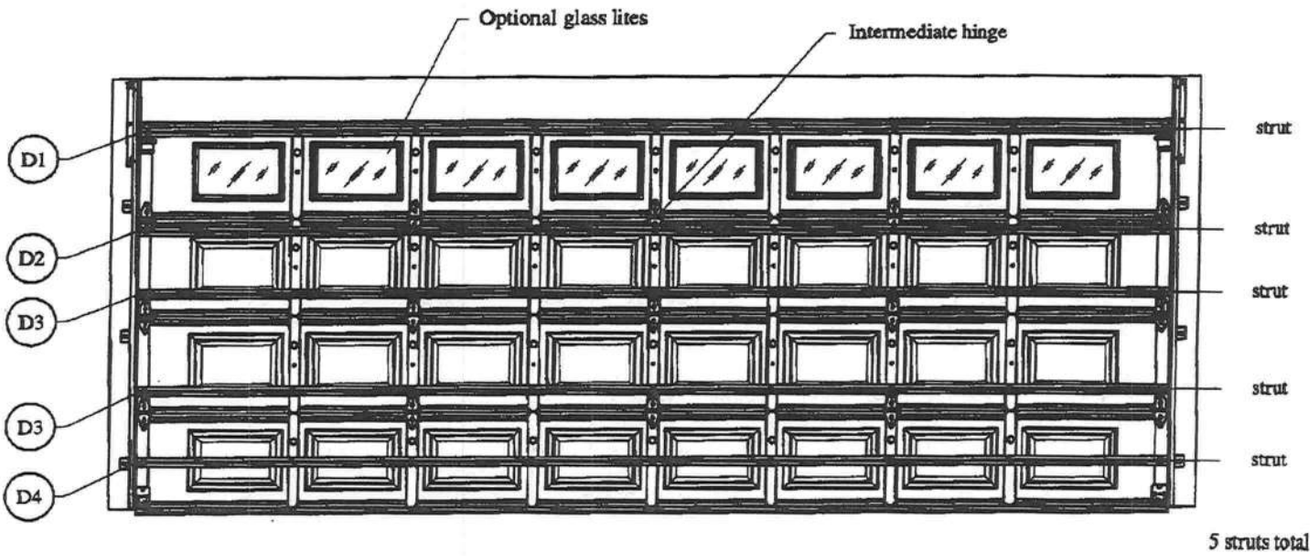
Strut above hinge

Detail 3



Strut in center of bottom section

Detail 4

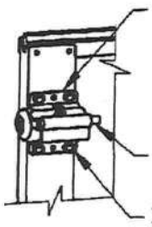


5 struts total

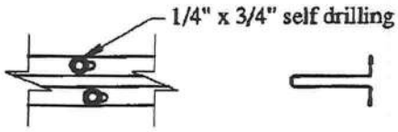
Z3-18045-pan-051608

Top Fixture Detail

(Strut, if applicable, not shown for clarity.)

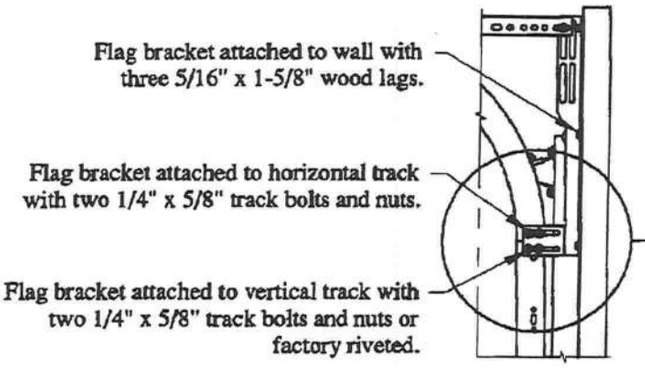


Strut Attachment Detail



17 gauge 50ksi galvanized steel 3" strut attached with two 1/4" x 3/4" screws to every stile.

Flag Bracket Detail



Track Bracket Locations

Manufacturing tolerances and actual field conditions may result in variances of +/- 1"

	door height / four sections			door height / five sections					
	6'-6"	6'-9"	7'-0"	7'-6"	7'-9"	8'-0"	8'-3"	8'-6"	8'-9"
D	n/a	n/a	n/a	72"	69"	72"	81"	84"	87"
C	60"	63"	66"	58"	55"	58"	60"	63"	66"
B	35"	35"	38"	34"	31"	34"	32"	35"	38"
A	10"	7"	10"	10"	7"	10"	4"	7"	10"

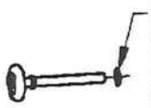
Track bracket locations shown above are for doors up to five sections high. Additional door sections may be added for a maximum door height of 14'-0". One track bracket (per track) must be added for each section and spaced at a distance not greater than the corresponding section height.

Z3-18045-pan-051608

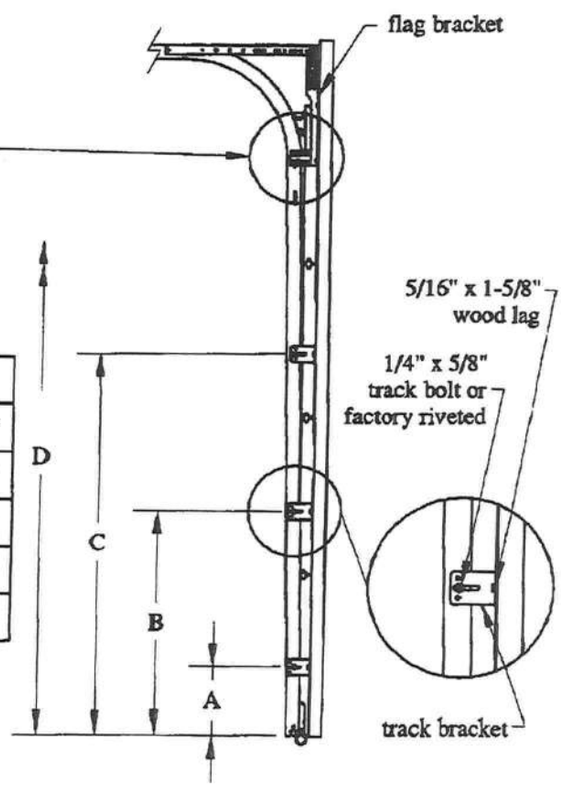
End Hinge Detail

Push Nut Detail (use on all rollers)

use 3/8" I. D. on bottom fixture roller stem
use 7/16" I. D. on end hinge and top fixture roller stems

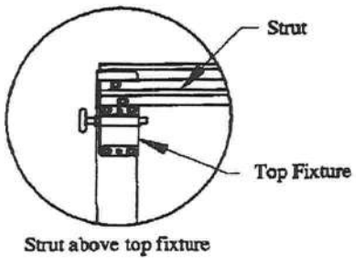


Intermediate Hinge Detail



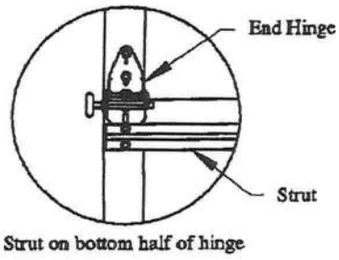
Four Section High Doors
18'-0" wide

Strut Placement



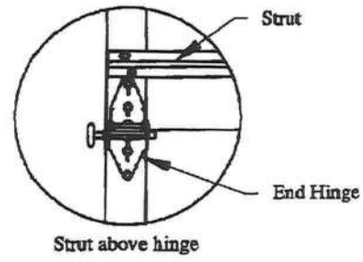
Strut above top fixture

Detail 1



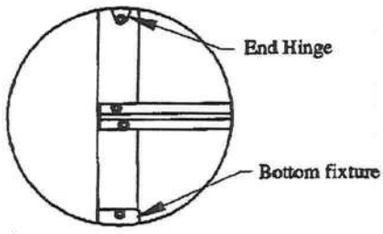
Strut on bottom half of hinge

Detail 2



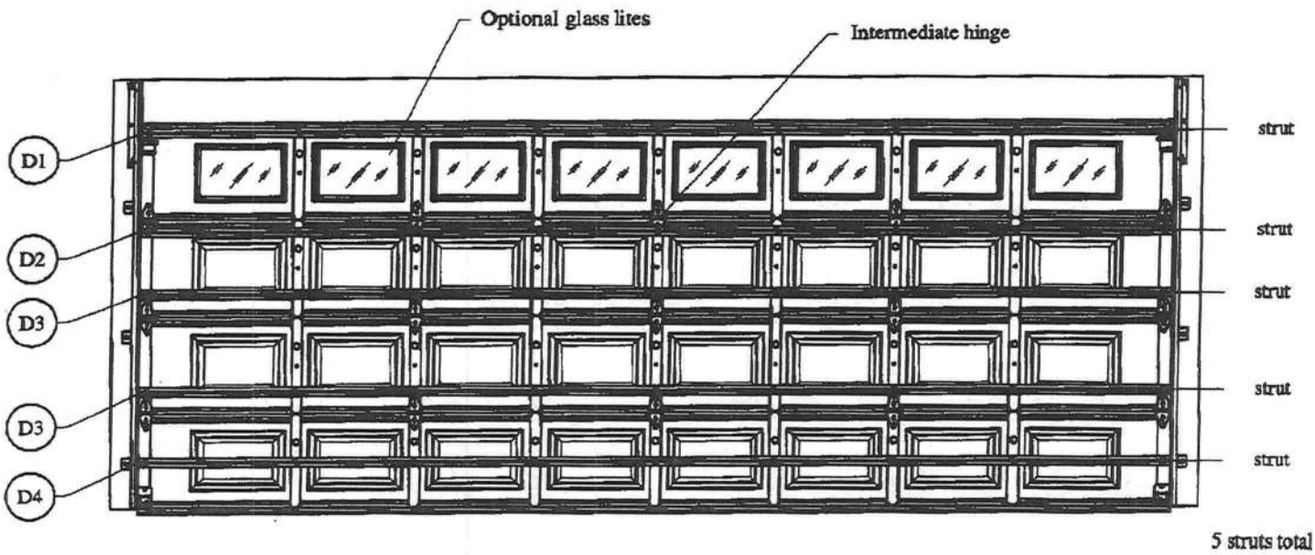
Strut above hinge

Detail 3



Strut in center of bottom section

Detail 4



5 struts total

Z3-18045-pan-051608

FORM 1100A-08

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs Residential Performance Method A

Project Name:	SCCI-1855 model with bonus	Builder Name:	SCCI
Street:	187 SW Asheville Wau	Permit Office:	
City, State, Zip:	Lake City , Fl ,	Permit Number:	
Owner:	SCCI	Jurisdiction:	224000
Design Location:	FL, Jacksonville		

1. New construction or existing	New (From Plans)
2. Single family or multiple family	Single-family
3. Number of units, if multiple family	1
4. Number of Bedrooms	3
5. Is this a worst case?	No
6. Conditioned floor area (ft²)	1855

7. Windows(239.0 sqft.)

	Description	Area
a. U-Factor: SHGC:	Dbl, U=0.30 SHGC=0.30	239.00 ft²
b. U-Factor: SHGC:	N/A	ft²
c. U-Factor: SHGC:	N/A	ft²
d. U-Factor: SHGC:	N/A	ft²
e. U-Factor: SHGC:	N/A	ft²

8. Floor Types (1855.0 sqft.)

	Insulation	Area
a. Slab-On-Grade Edge Insulation	R=0.0	1628.00 ft²
b. Floor over Garage	R=19.0	227.00 ft²
c. N/A	R=	ft²

9. Wall Types (1686.7 sqft.)

	Insulation	Area
a. Frame - Wood, Exterior	R=13.0	1483.30 ft²
b. Frame - Wood, Adjacent	R=13.0	203.33 ft²
c. N/A	R=	ft²
d. N/A	R=	ft²

10. Ceiling Types (2167.0 sqft.)

	Insulation	Area
a. Under Attic (Vented)	R=30.0	1855.00 ft²
b. Knee Wall (Vented)	R=19.0	312.00 ft²
c. N/A	R=	ft²

11. Ducts

a. Sup: Attic Ret: Attic AH: Attic Sup. R= 6, 371 ft²

12. Cooling systems

a. Central Unit — Cap: 41.0 kBtu/hr, SEER: 13

13. Heating systems

a. Electric Heat Pump — Cap: 41.5 kBtu/hr, HSPF: 8.5

14. Hot water systems

a. Propane — Cap: 1 gallons, EF: 0.81

b. Conservation features: None

15. Credits: CF, Pstat

Glass/Floor Area: 0.129

Total As-Built Modified Loads: 33.98

Total Baseline Loads: 40.19

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Suncoast Insulator

DATE: 12/13/11

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: [signature]

DATE: 12/14/11

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

GREAT SEAL OF THE STATE OF FLORIDA — IN GOD WE TRUST

BUILDING OFFICIAL: _____

DATE: _____

COLUMBIA COUNTY BUILDING DEPARTMENT — Received for FILE COPY — Code Compliance — PLANS EXAMINER

PROJECT

Title:	SCCI- Daisy model with bonu	Bedrooms:	3	Adress Type:	Street Address
Building Type:	FLAsBuilt	Conditioned Area:	1855	Lot #	
Owner:	SCCI	Total Stories:	2	Block/SubDivision:	
# of Units:	1	Worst Case:	No	PlatBook:	
Builder Name:	SCCI	Rotate Angle:	0	Street:	187 SW Asheville Wau
Permit Office:		Cross Ventilation:		County:	Columbia
Jurisdiction:		Whole House Fan:		City, State, Zip:	Lake City , Fl ,
Family Type:	Single-family				
New/Existing:	New (From Plans)				
Comment:					

CLIMATE

√	Design Location	TMY Site	IECC Zone	Design Temp 97.5 %	Design Temp 2.5 %	Int Design Temp Winter	Int Design Temp Summer	Heating Degree Days	Design Moisture	Daily Temp Range
____	FL, Jacksonville	FL_JACKSONVILLE_INT	2	32	93	75	70	1281	49	Medium

FLOORS

√	#	Floor Type	Perimeter	Perimeter R-Value	Area	Joist R-Value	Tile	Wood	Carpet
____	1	Slab-On-Grade Edge Insulatio	195 ft	0	1628 ft²		0	0	1
____	2	Floor over Garage			227 ft²	19	0	0	1

ROOF

√	#	Type	Materials	Roof Area	Gable Area	Roof Color	Solar Absor.	Tested	Deck Insul.	Pitch
____	1	Hip	Composition shingles	2230 ft²	0 ft²	Medium	0.96	No	0	33.7 deg

ATTIC

√	#	Type	Ventilation	Vent Ratio (1 in)	Area	RBS	IRCC
____	1	Full attic	Vented	300	1855 ft²	N	N

CEILING

√	#	Ceiling Type	R-Value	Area	Framing Frac	Truss Type
____	1	Under Attic (Vented)	30	1855 ft²	0.11	Wood
____	2	Knee Wall (Vented)	19	312 ft²	0.11	Wood

WALLS

√	#	Ornt	Adjacent To	Wall Type	Cavity R-Value	Area	Sheathing R-Value	Framing Fraction	Solar Absor.
____	1	W	Garage	Frame - Wood	13	203.3333		0.23	0.01
____	2	W	Exterior	Frame - Wood	13	88 ft²		0.23	0.75
____	3	W	Exterior	Frame - Wood	13	393.3333		0.23	0.75
____	4	N	Exterior	Frame - Wood	13	250.6666		0.23	0.75
____	5	E	Exterior	Frame - Wood	13	579.3333		0.23	0.75
____	6	S	Exterior	Frame - Wood	13	172 ft²		0.23	0.75

DOORS

✓	#	Ornt	Door Type	Storms	U-Value	Area
____	1	W	Insulated	None	0.460000	17.77777
____	2	W	Insulated	None	0.460000	24 ft²

WINDOWS

Orientation shown is the entered, asBuilt orientation.

✓	#	Ornt	Frame	Panes	NFRC	U-Factor	SHGC	Storms	Area	Overhang Depth	Overhang Separation	Int Shade	Screening
____	1	W	Vinyl	Low-E Double	Yes	0.3	0.3	N	6 ft²	2 ft 0 in	6 ft 0 in	HERS 2006	None
____	2	W	Vinyl	Low-E Double	Yes	0.3	0.3	N	60 ft²	2 ft 0 in	6 ft 0 in	HERS 2006	None
____	3	W	Vinyl	Low-E Double	Yes	0.3	0.3	N	16 ft²	2 ft 0 in	6 ft 0 in	HERS 2006	None
____	4	N	Vinyl	Low-E Double	Yes	0.3	0.3	N	4 ft²	2 ft 0 in	5 ft 0 in	HERS 2006	None
____	5	E	Vinyl	Low-E Double	Yes	0.3	0.3	N	60 ft²	2 ft 0 in	5 ft 0 in	HERS 2006	None
____	6	E	Vinyl	Low-E Double	Yes	0.3	0.3	N	54 ft²	2 ft 0 in	5 ft 0 in	HERS 2006	None
____	7	E	Vinyl	Low-E Double	Yes	0.3	0.3	N	24 ft²	2 ft 0 in	8 ft 0 in	HERS 2006	None
____	8	E	Vinyl	Low-E Double	Yes	0.3	0.3	N	15 ft²	2 ft 0 in	5 ft 0 in	HERS 2006	None

INFILTRATION & VENTING

✓	Method	SLA	CFM 50	ACH 50	ELA	EqLA	---- Forced Ventilation ---- Supply CFM	Exhaust CFM	Run Time Fraction	Fan Watts
____	Default	0.00036	1752	7.08	96.2	180.8	0 cfm	0 cfm	0	0

GARAGE

✓	#	Floor Area	Ceiling Area	Exposed Wall Perimeter	Avg. Wall Height	Exposed Wall Insulation
____	1	473.8 ft²	247 ft²	62.5 ft	8 ft	1

COOLING SYSTEM

✓	#	System Type	Subtype	Efficiency	Capacity	Air Flow	SHR	Ducts
____	1	Central Unit	None	SEER: 13	41 kBtu/hr	1230 cfm	0.75	sys#0

HEATING SYSTEM

✓	#	System Type	Subtype	Efficiency	Capacity	Ducts
____	1	Electric Heat Pump	None	HSPF: 8.5	41.5 kBtu/hr	sys#0

HOT WATER SYSTEM

✓	#	System Type	EF	Cap	Use	SetPnt	Conservation
____	1	Propane	0.81	1 gal	60 gal	120 deg	None

SOLAR HOT WATER SYSTEM

✓	FSEC Cert #	Company Name	System Model #	Collector Model #	Collector Area	Storage Volume	FEF
_____	None	None			ft²		

DUCTS

✓	#	---- Supply ---- Location	R-Value	Area	---- Return ---- Location	Area	Leakage Type	Air Handler	CFM 25	Percent Leakage	QN	RLF
_____	1	Attic	6	371 ft²	Attic	92.75 ft	Default Leakage	Attic	(Default)	(Default) %		

TEMPERATURES

Programable Thermostat: Y

Ceiling Fans:

	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec
Cooling	[X]	[X]	[X]	[X]	[X]	[X]	[X]	[X]	[X]	[X]	[X]	[X]
Heating	[X]	[X]	[X]	[X]	[X]	[X]	[X]	[X]	[X]	[X]	[X]	[X]
Venting	[X]	[X]	[X]	[X]	[X]	[X]	[X]	[X]	[X]	[X]	[X]	[X]

Thermostat Schedule: HERS 2006 Reference

Schedule Type		Hours 1	2	3	4	5	6	7	8	9	10	11	12
Cooling (WD)	AM	78	78	78	78	78	78	78	78	80	80	80	80
	PM	80	80	78	78	78	78	78	78	78	78	78	78
Cooling (WEH)	AM	78	78	78	78	78	78	78	78	78	78	78	78
	PM	78	78	78	78	78	78	78	78	78	78	78	78
Heating (WD)	AM	66	66	66	66	66	68	68	68	68	68	68	68
	PM	68	68	68	68	68	68	68	68	68	68	66	66
Heating (WEH)	AM	66	66	66	66	66	68	68	68	68	68	68	68
	PM	68	68	68	68	68	68	68	68	68	68	66	66

FORM 1100A-08

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: 187 SW Asheville Wau
Lake City, Fl,

PERMIT #:

INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	N1106.AB.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	N1106.AB.1.2	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	N1106.AB.1.2	Penetrations/openings > 1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	N1106.AB.1.2	Between walls & ceilings; penetrations of ceiling plane to top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	N1106.AB.1.2	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	N1106.AB.1.2	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	N1106.AB.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	N1112.AB.3	Comply with efficiency requirements in Table N1112.ABC.3 Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	N1112.AB.2.3	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%. Heat pump pool heaters shall have a minimum COP of 4.0.	
Shower heads	N1112.AB.2.4	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	N1110.AB	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated and installed in accordance with the criteria of Section N1110.AB. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	N1107.AB.2	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	N1104.AB.1 N1102.B.1.1	Ceilings-Min. R-19. Common walls-frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
 Document ID:1UHT487-Z0112103427

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/12/2011

Truss Fabricator:	Anderson Truss Company
Job Identification:	11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , **
Truss Count:	53
Model Code:	Florida Building Code 2007 and 2009 Supplement
Truss Criteria:	FBC2007Res/TPI-2002(STD)
Engineering Software:	Alpine Software,Version 10.03.
Structural Engineer of Record:	The identity of the structural EOR did not exist as of
Address:	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads:	Roof - 40.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 110 MPH ASCE 7-05 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

Douglas Fleming
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

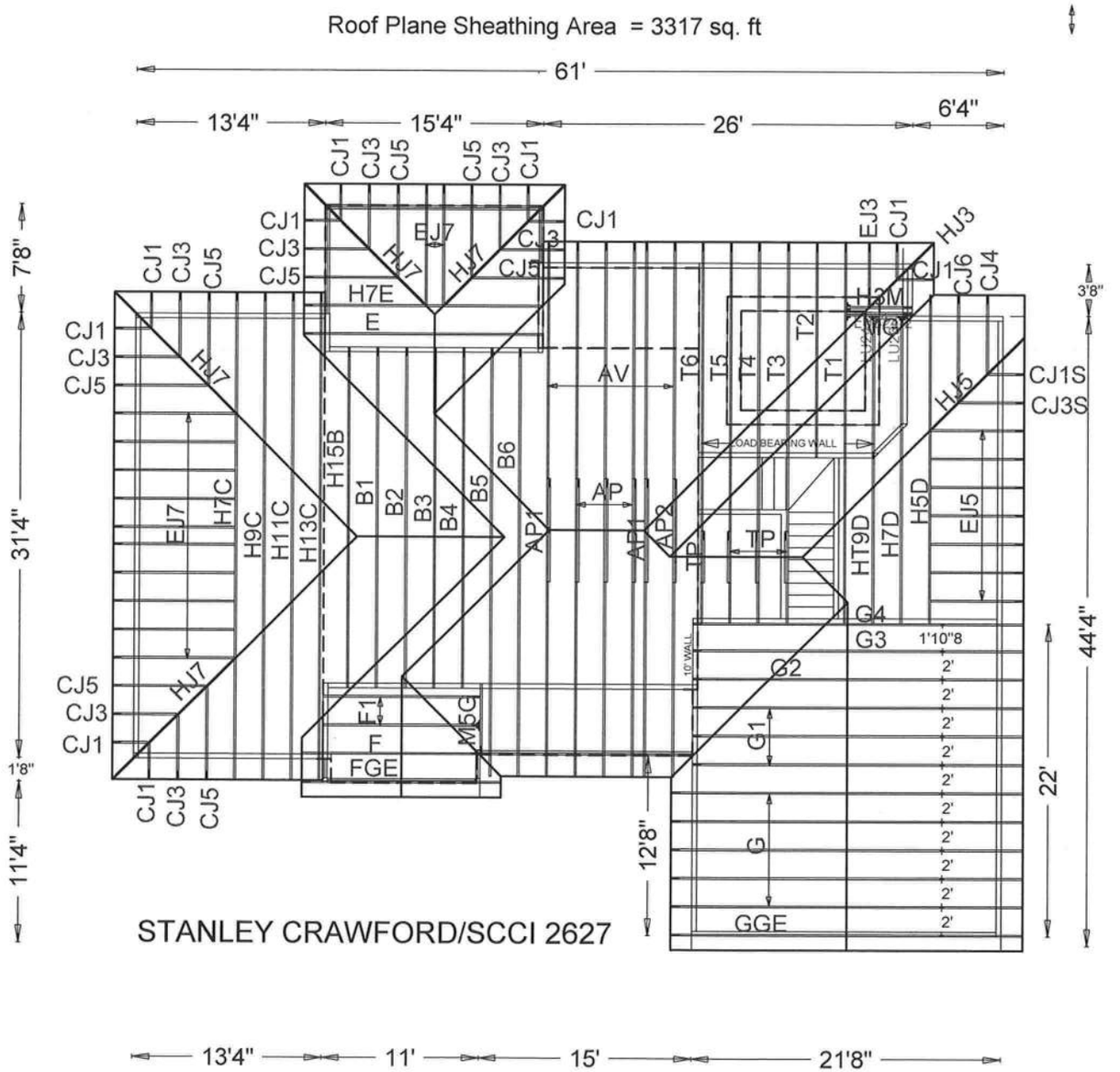
Details: A1101505-GBLLETIN-BRCLBSUB-PB120-

#	Ref Description	Drawing#	Date
1	16760--AV	11346035	12/12/11
2	16761--H15B	11346010	12/12/11
3	16762--B1	11346026	12/12/11
4	16763--B2	11346027	12/12/11
5	16764--B3	11346028	12/12/11
6	16765--B4	11346030	12/12/11
7	16766--B5	11346029	12/12/11
8	16767--B6	11346032	12/12/11
9	16768--H7C	11346004	12/12/11
10	16769--H9C	11346047	12/12/11
11	16770--H11C	11346048	12/12/11
12	16771--H13C	11346049	12/12/11
13	16772--H5D	11346011	12/12/11
14	16773--H7D	11346041	12/12/11
15	16774--HT9D	11346014	12/12/11
16	16775--H7E	11346007	12/12/11
17	16776--E	11346009	12/12/11
18	16777--FGE	11346001	12/12/11
19	16778--F	11346002	12/12/11
20	16779--F1	11346003	12/12/11
21	16780--G	11346001	12/12/11
22	16781--G1	11346012	12/12/11
23	16782--GGE	11346044	12/12/11
24	16783--G4	11346033	12/12/11
25	16784--G3	11346021	12/12/11
26	16785--G2	11346022	12/12/11
27	16786--CJ1	11346002	12/12/11
28	16787--CJ3	11346003	12/12/11
29	16788--CJ5	11346038	12/12/11
30	16789--EJ7	11346005	12/12/11
31	16790--HJ7	11346006	12/12/11
32	16791--EJ5	11346019	12/12/11
33	16792--CJ1S	11346018	12/12/11
34	16793--CJ3S	11346016	12/12/11
35	16794--CJ4	11346017	12/12/11
36	16795--CJ6	11346015	12/12/11

#	Ref Description	Drawing#	Date
37	16796--HJ5	11346042	12/12/11
38	16797--EJ3	11346037	12/12/11
39	16798--HJ3	11346025	12/12/11
40	16799--H3M	11346013	12/12/11
41	16800--MG	11346031	12/12/11
42	16801--M5G	11346004	12/12/11
43	16802--KGE	11346008	12/12/11
44	16803--AP	11346046	12/12/11
45	16804--AP1	11346045	12/12/11
46	16805--AP2	11346024	12/12/11
47	16806--TP	11346043	12/12/11
48	16807--T2	11346040	12/12/11
49	16808--T4	11346036	12/12/11
50	16809--T5	11346034	12/12/11
51	16810--T6	11346039	12/12/11
52	16811--T1	11346020	12/12/11
53	16812--T3	11346023	12/12/11

COLUMBIA COUNTY BUILDING DEPARTMENT Received for FILE COPY Code Compliance PLANS EXAMINER

Roof Plane Sheathing Area = 3317 sq. ft



STANLEY CRAWFORD/SCCI 2627

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - AV)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

(b) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

Negative reaction(s) of -374# MAX. (See below) from a non-wind load case requires uplift connection.

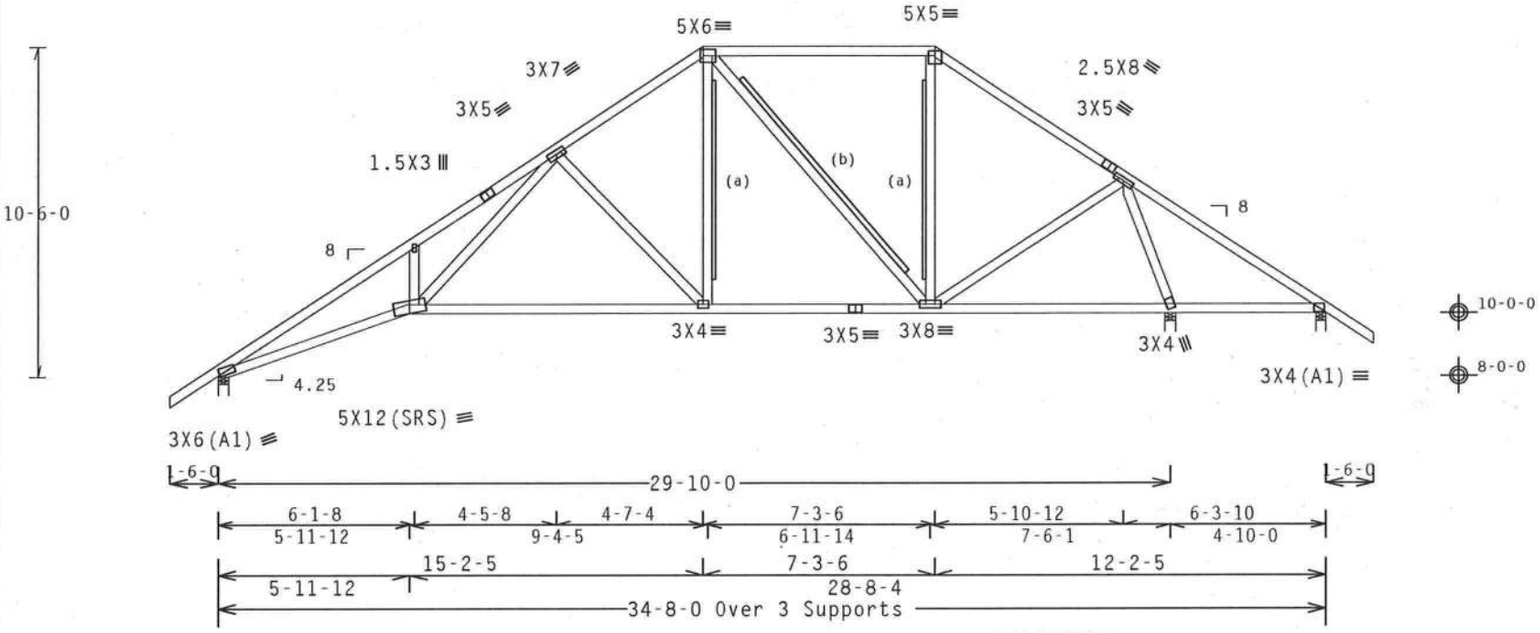
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Shim all supports to solid bearing.



R=1370 U=0 W=4"
RL=273/-258

R=2348 U=0 W=4"
R=-374 Rw=141 U=275 W=3.5"

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.03.04.0601.15 QTY 6

FL/-/4/-/-/R/-

Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing or bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: this job's general notes page; ITW-BCG: www.itwbcg.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

TC LL	20.0 PSF	REF	R487-- 16760
TC DL	10.0 PSF	DATE	12/12/11
BC DL	10.0 PSF	DRW	HCUSR487 11346035
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	252738
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - B1)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

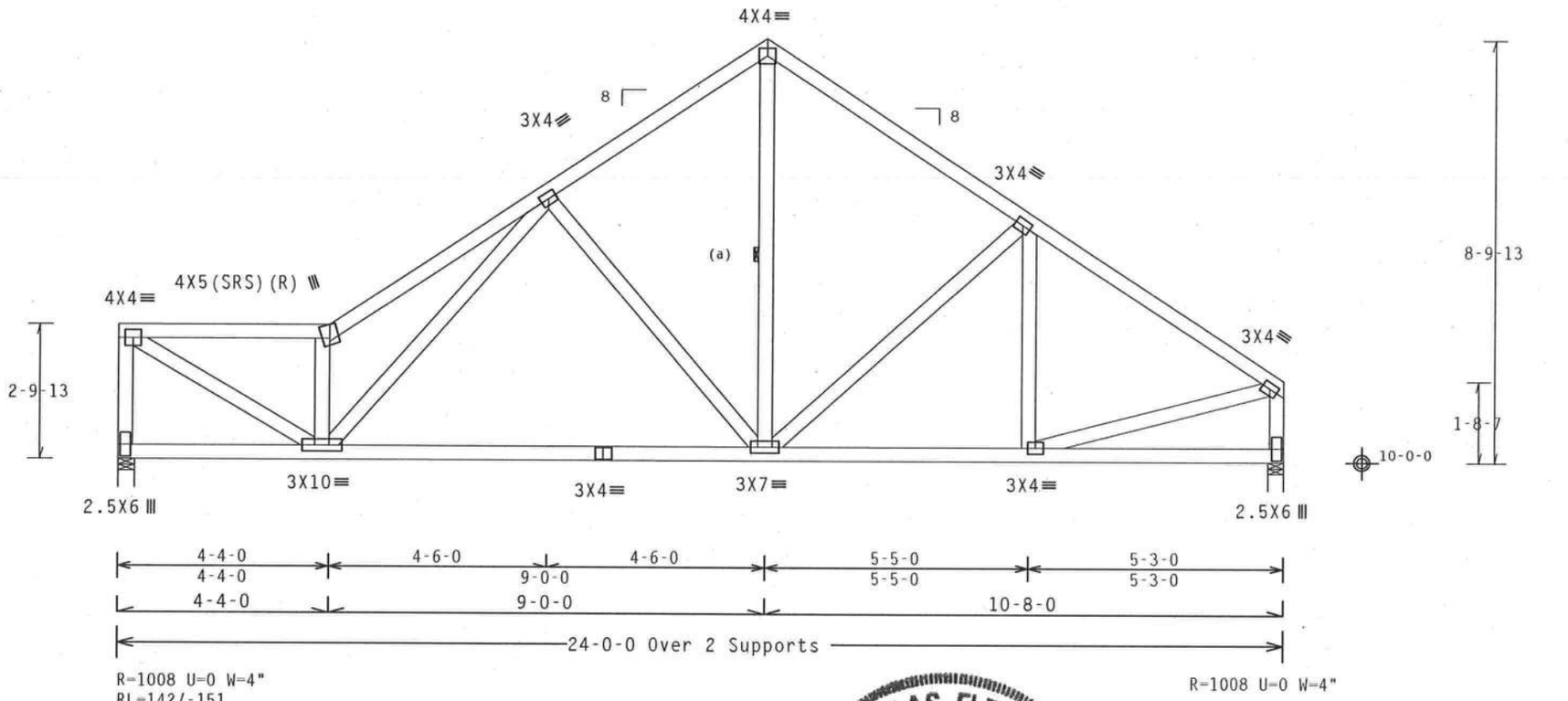
MWFRS loads based on trusses located at least 15.26 ft. from roof edge.

110 mph wind, 15.26 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.



Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.03.04.0601.16 QTY 1

FL/-/4/-/-/R/-

Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/12/2011

TC LL	20.0 PSF	REF	R487-- 16762
TC DL	10.0 PSF	DATE	12/12/11
BC DL	10.0 PSF	DRW	HCUSR487 11346026
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	253135
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - B2)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

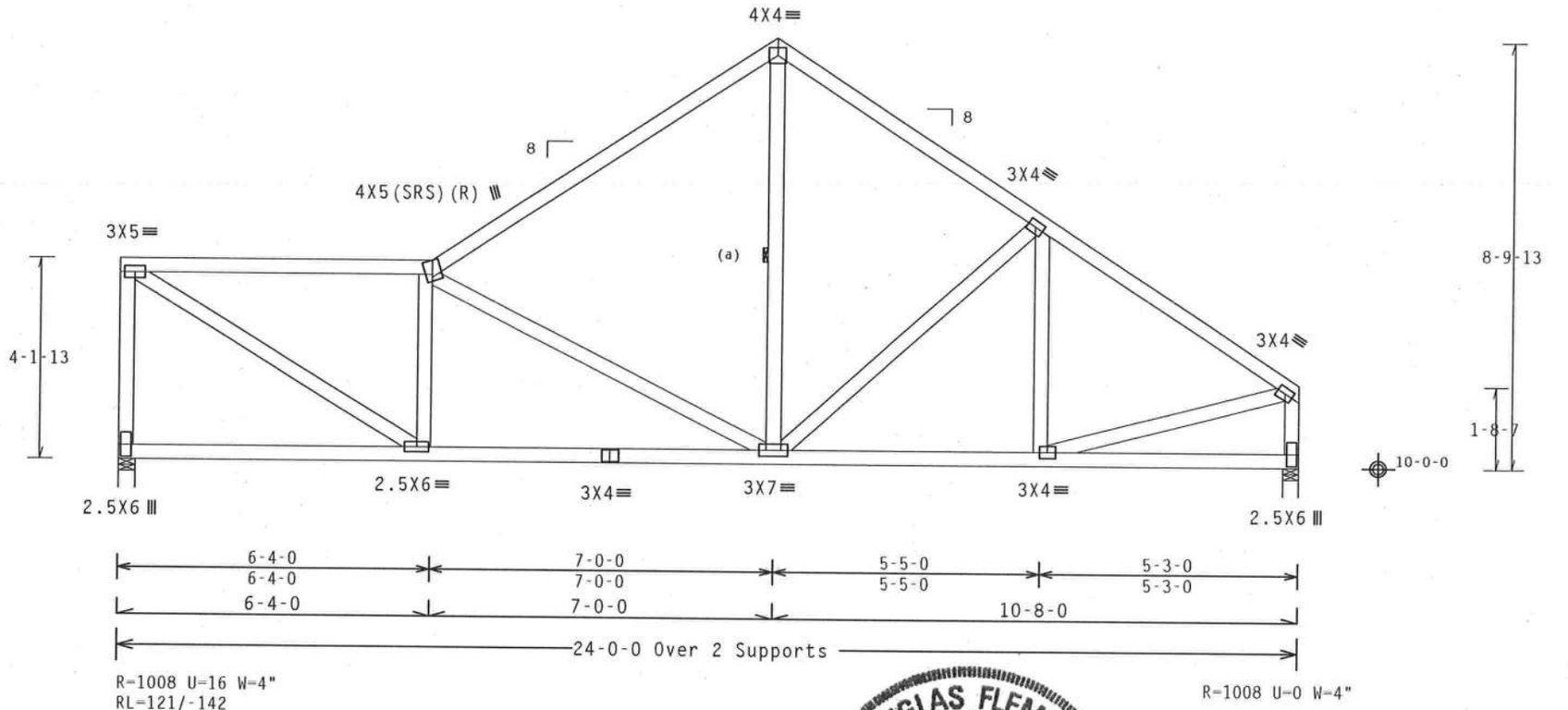
MWFRS loads based on trusses located at least 15.26 ft. from roof edge.

110 mph wind, 15.26 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.



R=1008 U=16 W=4"
RL=121/-142

R=1008 U=0 W=4"

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.03.04.0603.16

QTY:

FL/-/4/-/-/R/-

Scale =.3125"/Ft.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 16763
TC DL	10.0 PSF	DATE	12/12/11
BC DL	10.0 PSF	DRW	HCUSR487 11346027
BC LL	0.0 PSF		HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN-	253144
DUR.FAC.	1.25		
SPACING	24.0"		JREF- 1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - B3)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

End verticals not exposed to wind pressure.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

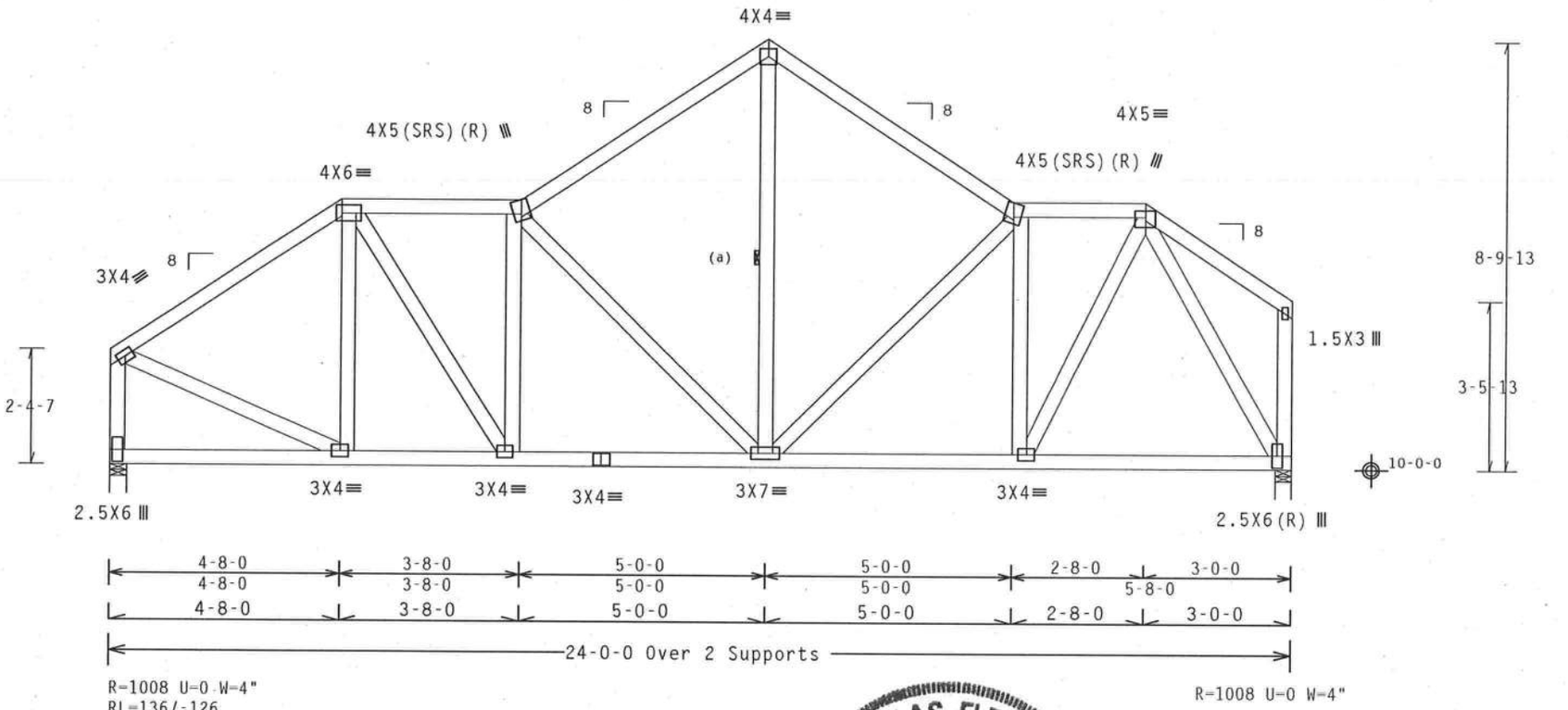
MWFRS loads based on trusses located at least 15.59 ft. from roof edge.

110 mph wind, 15.59 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.



Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.03.04.0601.15 QTY 1

FL/-/4/-/-/R/-

Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

TC LL	20.0 PSF	REF	R487-- 16764
TC DL	10.0 PSF	DATE	12/12/11
BC DL	10.0 PSF	DRW	HCUSR487 11346028
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	253151
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - B4)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

End verticals not exposed to wind pressure.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

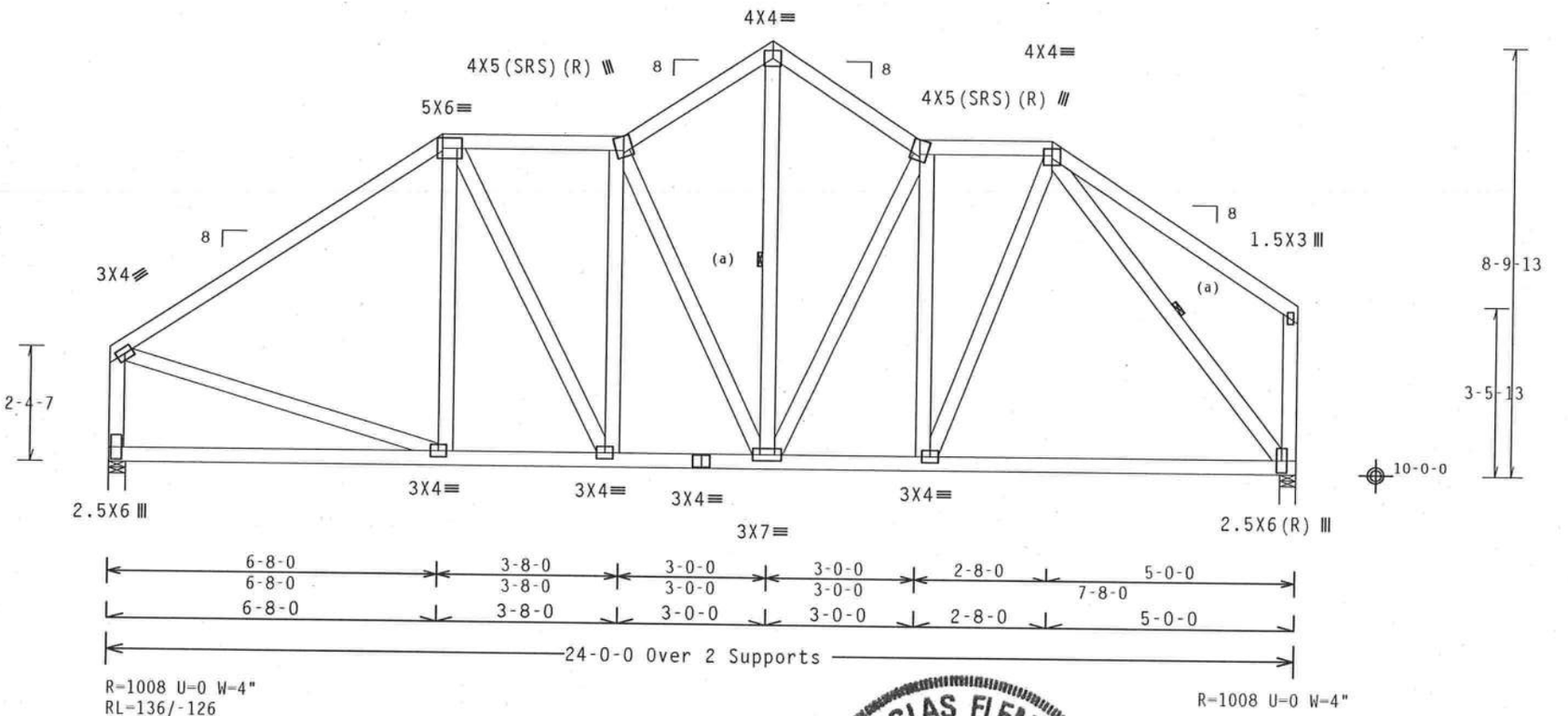
MWFRS loads based on trusses located at least 15.59 ft. from roof edge.

110 mph wind, 15.59 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.04.0601.16 QTY:

FL/-/4/-/-/R/- Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

TC LL	20.0 PSF	REF	R487-- 16765
TC DL	10.0 PSF	DATE	12/12/11
BC DL	10.0 PSF	DRW	HCUSR487 11346030
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	253156
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - B5)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

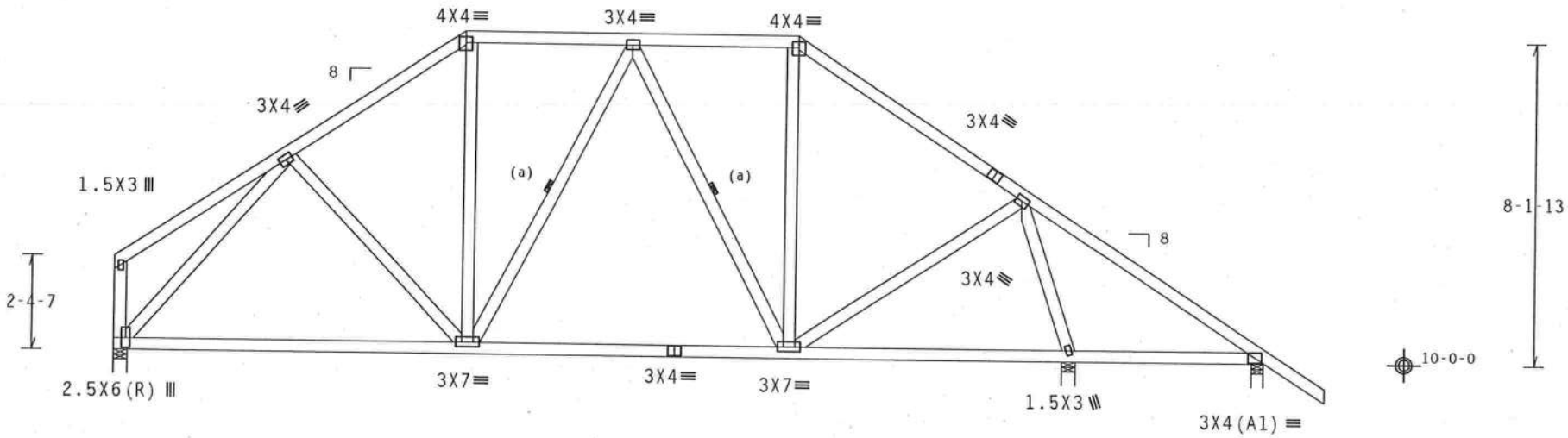
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

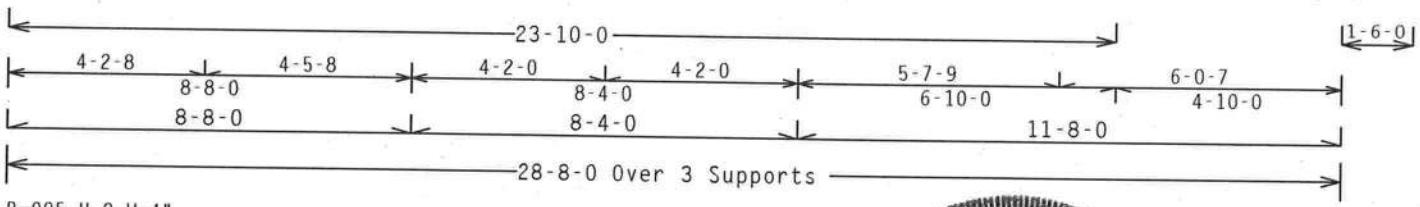
Wind loads and reactions based on MWFRS with additional C&C member design.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.





R=995 U=0 W=4"
RL=171/-175

R=1218 U=0 W=4"
R=299 U=24 W=3.5"

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.03.04.0601.16

QTY: 1

FL/-/4/-/-/R/-

Scale =.25"/Ft.

TC LL	20.0 PSF	REF R487-- 16766
TC DL	10.0 PSF	DATE 12/12/11
BC DL	10.0 PSF	DRW HCUSR487 11346029
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 253170
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UHT487_Z01

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - B6)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

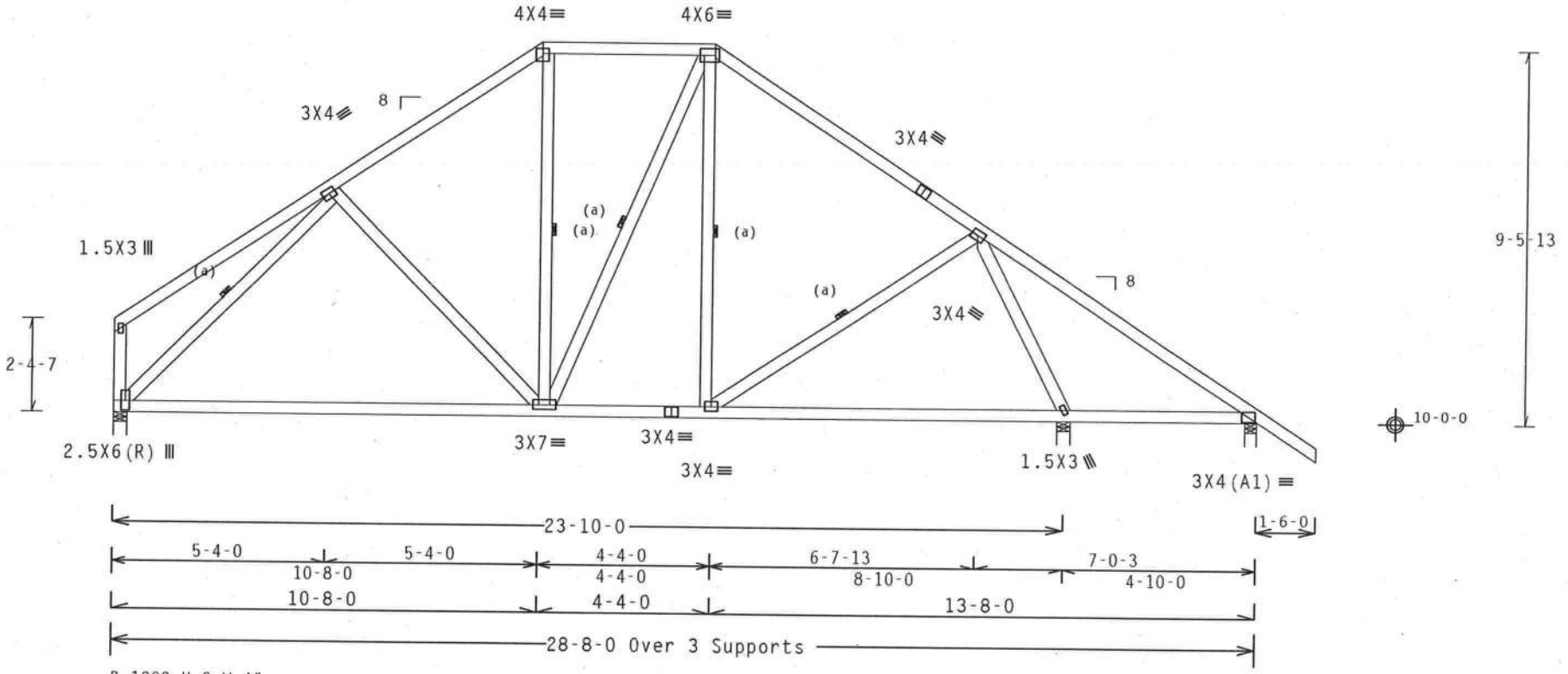
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



R=1003 U=0 W=4"
RL=201/-205

R=1171 U=0 W=4"
R=338 U=25 W=3.5"

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.03.04.0601.16 QTY: 1 FL/-/4/-/-/R/- Scale =.25"/Ft.

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF R487-- 16767
TC DL	10.0 PSF	DATE 12/12/11
BC DL	10.0 PSF	DRW HCUSR487 11346032
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 253179
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - H7C)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1 :T2, T3 2x6 SP #2:
Bot chord 2x6 SP #2
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

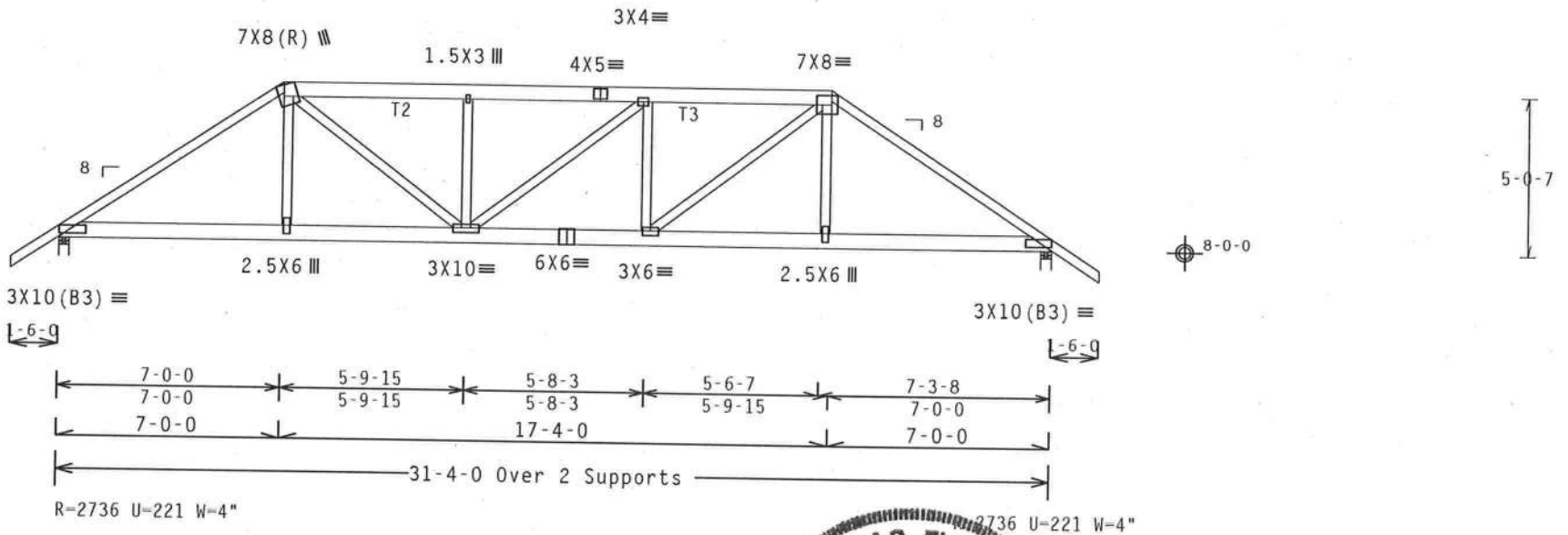
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCp1(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

#1 hip supports 7-0-0 jacks with no webs.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

DOUGLAS FLEMING LICENSE 10.03.04.0601.16 No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 12/12/2011

QTY 1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF R487-- 16768
TC DL	10.0 PSF	DATE 12/12/11
BC DL	10.0 PSF	DRW HCUSR487 11346004
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 253084
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - H9C)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load.

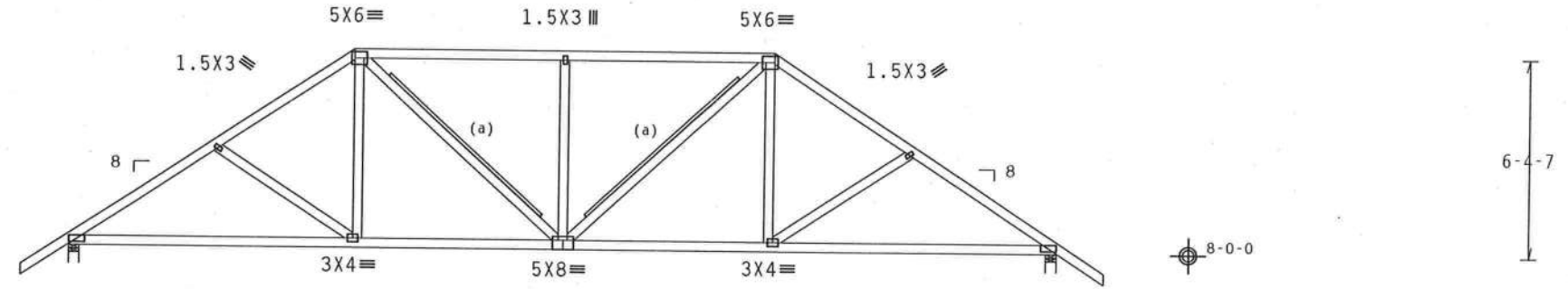
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.03.04.0601.16

QTY:

FL/-/4/-/-/R/-

Scale =.1875"/Ft.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 12/12/2011

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R487-- 16769
DATE	12/12/11
DRW	HCUSR487 11346047
HC-ENG	DF/DF *
SEQN-	253091
JREF-	1UHT487_Z01

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - H11C)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load.

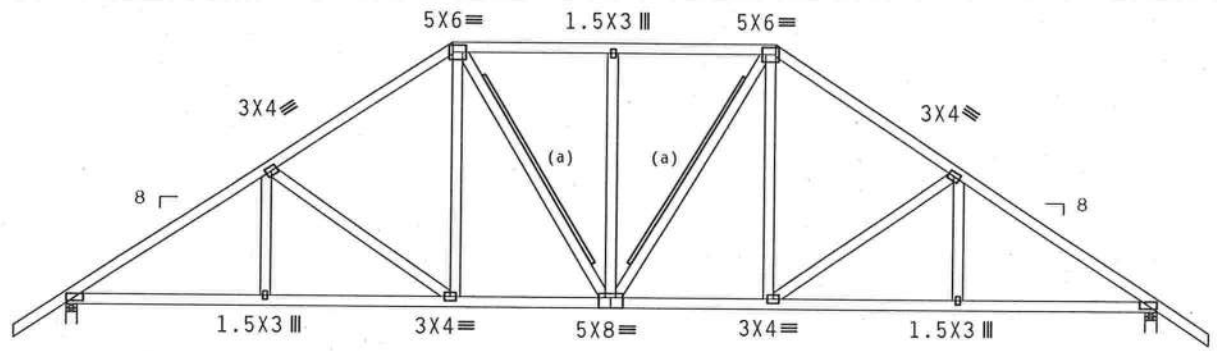
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCp1(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.



R=1420 U=127 W=4"
RL=209/-209

R=1420 U=127 W=4"

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.03.04.0601.16 QTY 1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/12/2011

TC LL	20.0 PSF	REF R487-- 16770
TC DL	10.0 PSF	DATE 12/12/11
BC DL	10.0 PSF	DRW HCUSR487 11346048
BC LL	0.0 PSF	HC-ENG DF/DF *
TOT.LD.	40.0 PSF	SEQN- 253098
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - H13C)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

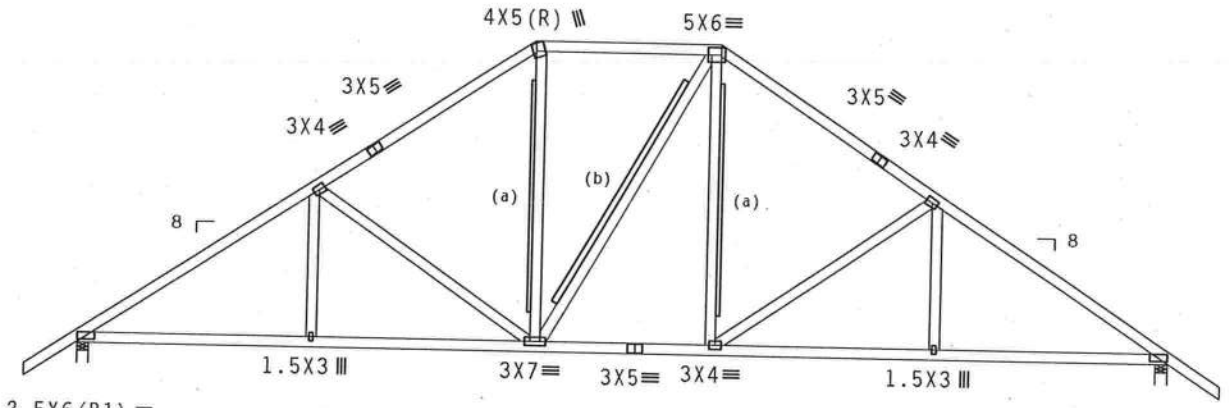
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

(b) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.



R=1420 U=123 W=4"
RL=240/-240

R=1420 U=123 W=4"

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

10.03.04.0601.16 QTY 1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

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12/12/2011

TC LL	20.0 PSF	REF R487-- 16771
TC DL	10.0 PSF	DATE 12/12/11
BC DL	10.0 PSF	DRW HCUSR487 11346049
BC LL	0.0 PSF	HC-ENG DF/DF *
TOT.LD.	40.0 PSF	SEQN- 253112
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - H5D)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

(a) 1x4 #3SRB SPF-S or better "T" brace, 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

#1 hip supports 8-0-0 jacks W/2 panel TC and no end vert.

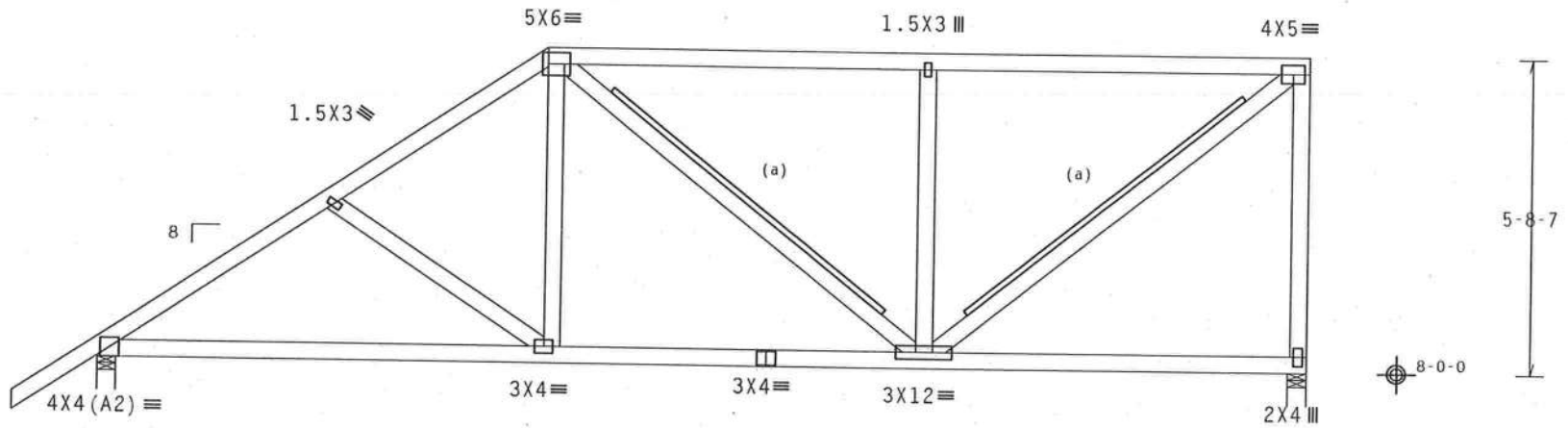
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

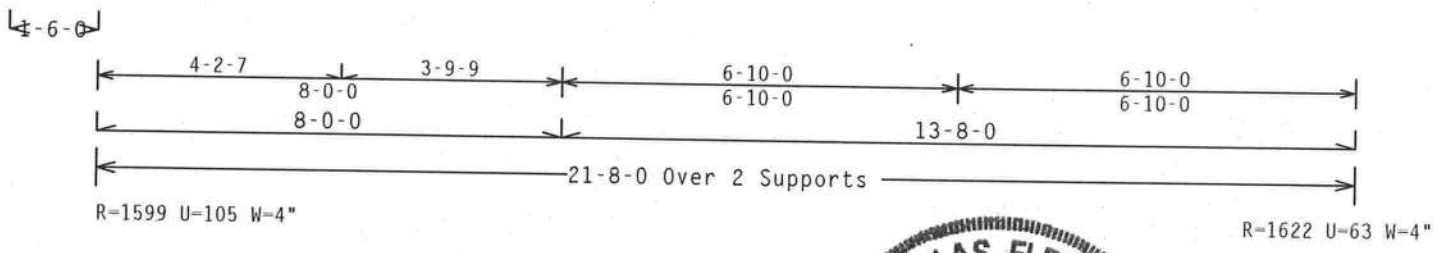
Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.





PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.03.04.0601.16 QTY: 1 FL/-/4/-/-/R/- Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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12/12/2011

TC LL	20.0 PSF	REF R487-- 16772
TC DL	10.0 PSF	DATE 12/12/11
BC DL	10.0 PSF	DRW HCUSR487 11346011
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 252839
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - H7D)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

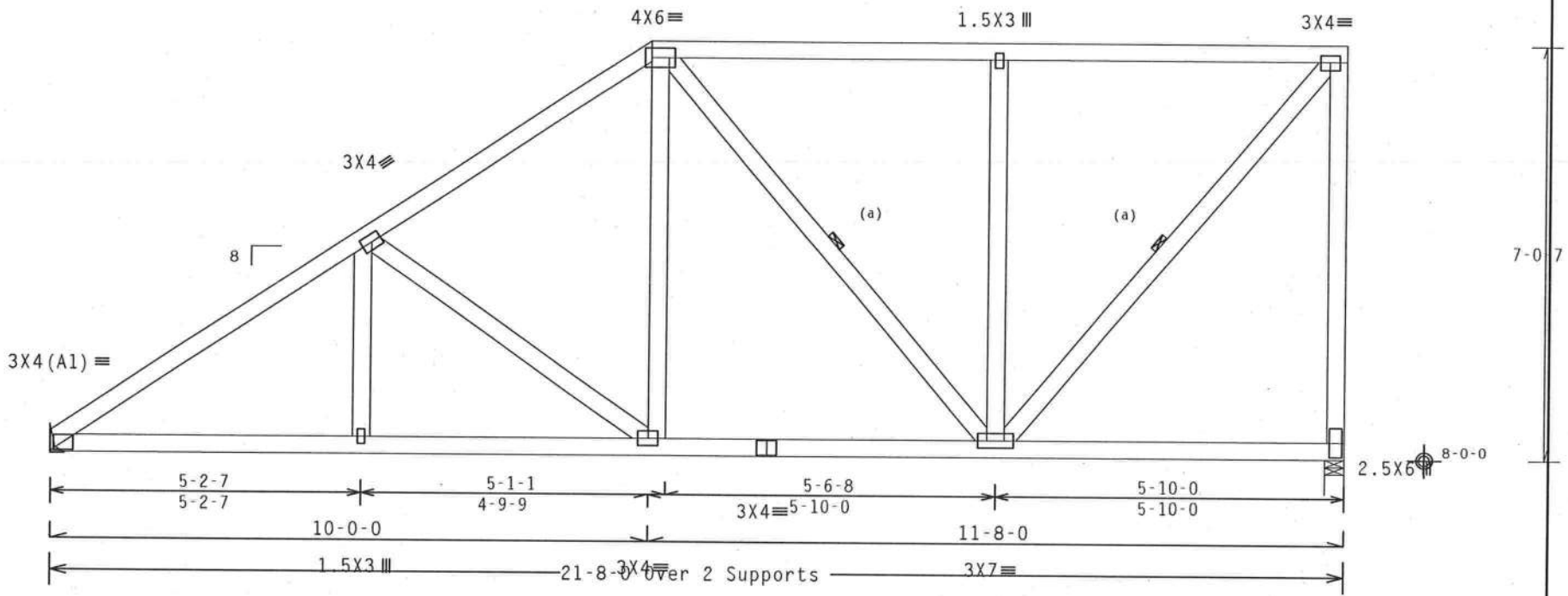
MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load.



R=916 U=52
RL=153/-47

R=905 U=112 W=4"

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.03.04.0601.16 QTY 1 FL/-/4/-/-/R/- Scale =.375"/Ft.

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12/12/2011

ALPINE
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Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 16773
TC DL	10.0 PSF	DATE	12/12/11
BC DL	10.0 PSF	DRW	HCUSR487 11346041
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	252881
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - HT9D)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

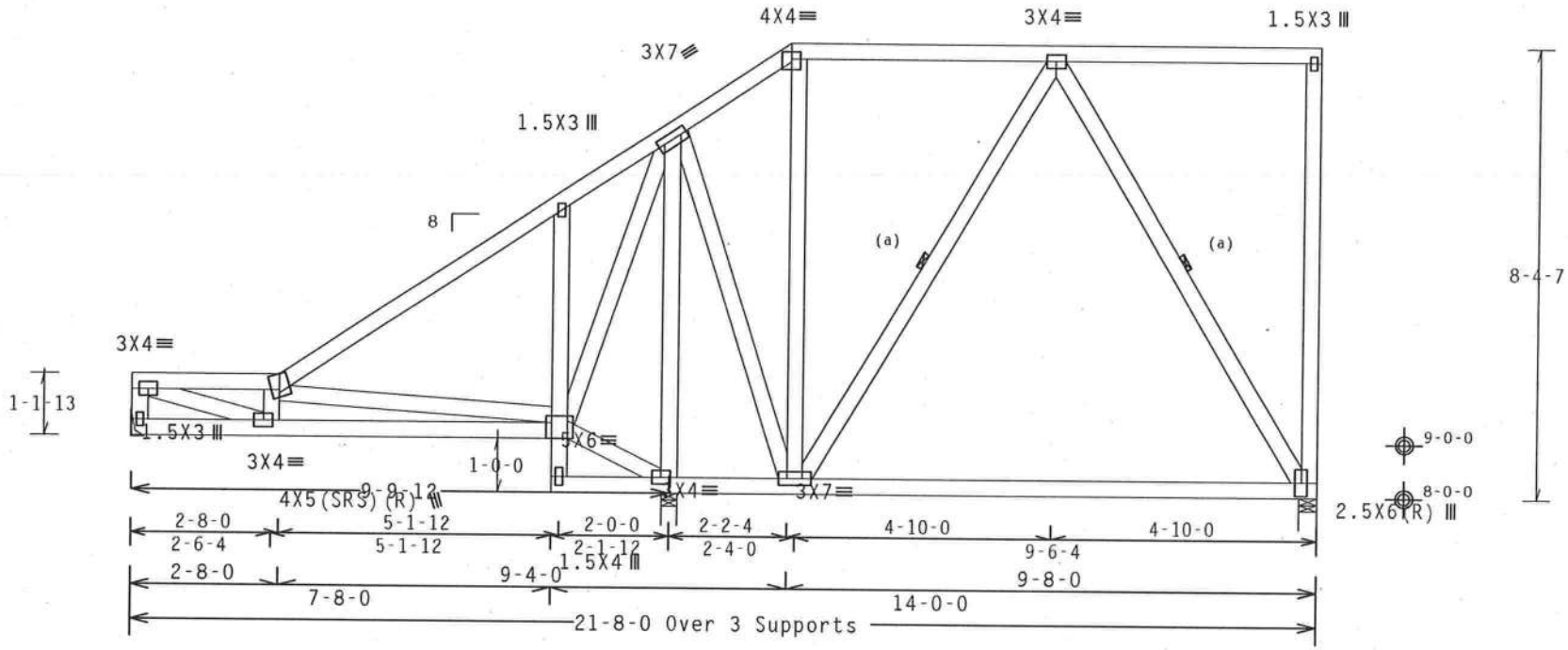
MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load.



R=270 U=0
RL=143/-43

R=1170 U=118 W=3.45"

R=381 U=65 W=4"

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.03.04.0601.16 QTY 1 FL/-/4/-/-/R/- Scale =.3125"/Ft.

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/12/2011

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 16774
TC DL	10.0 PSF	DATE	12/12/11
BC DL	10.0 PSF	DRW	HCUSR487 11346014
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	252859
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - H7E)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

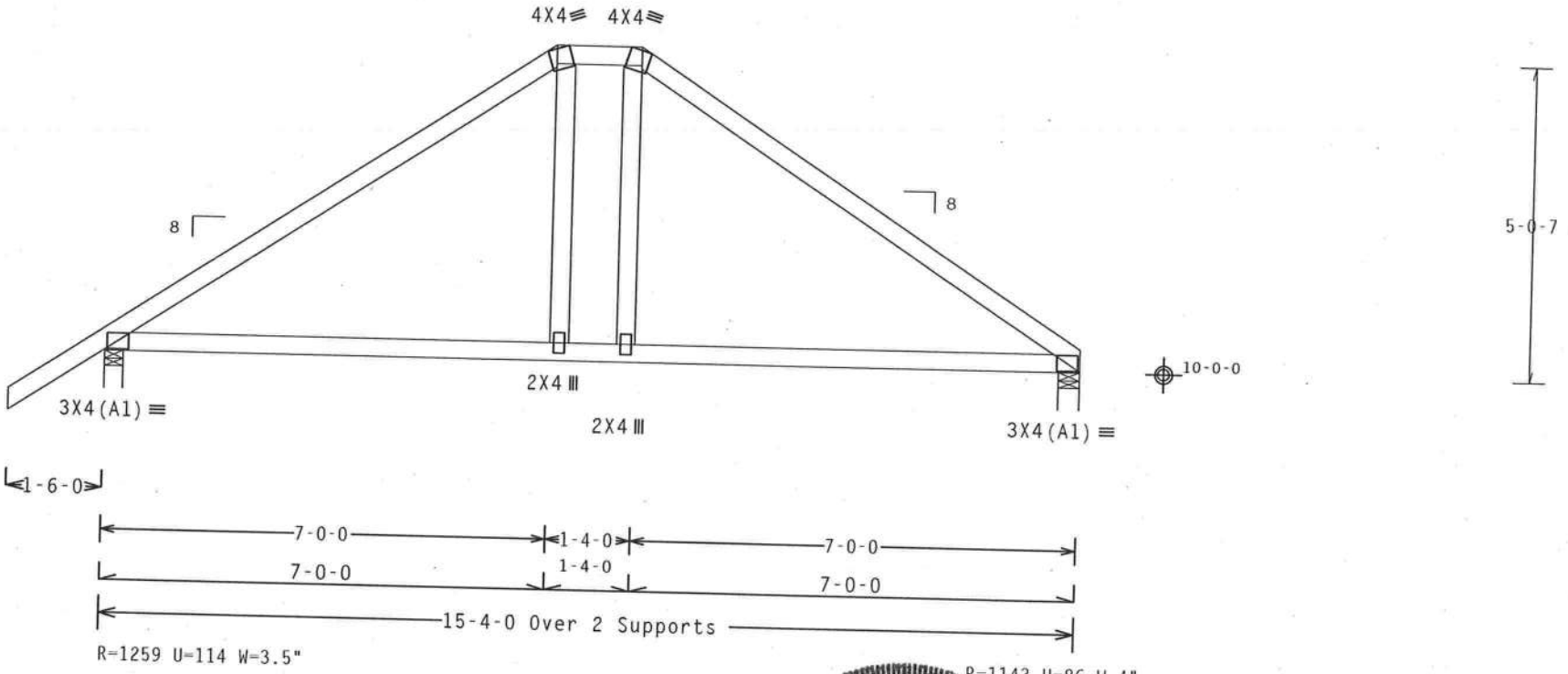
Left side jacks have 7-0-0 setback with 0-0-0 cant and 1-6-0 overhang. End jacks have 7-0-0 setback with 0-0-0 cant and 1-6-0 overhang. Right side jacks have 7-0-0 setback with 0-0-0 cant and 1-6-0 overhang.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

#1 hip supports 7-0-0 jacks with no webs.

Deflection meets L/240 live and L/180 total load.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.03.04.0601.16 QTY: 1 FL/-/4/-/-/R/- Scale =.375"/Ft.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 12/12/2011

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 16775
TC DL	10.0 PSF	DATE	12/12/11
BC DL	10.0 PSF	DRW	HCUSR487 11346007
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	253067
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - E)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

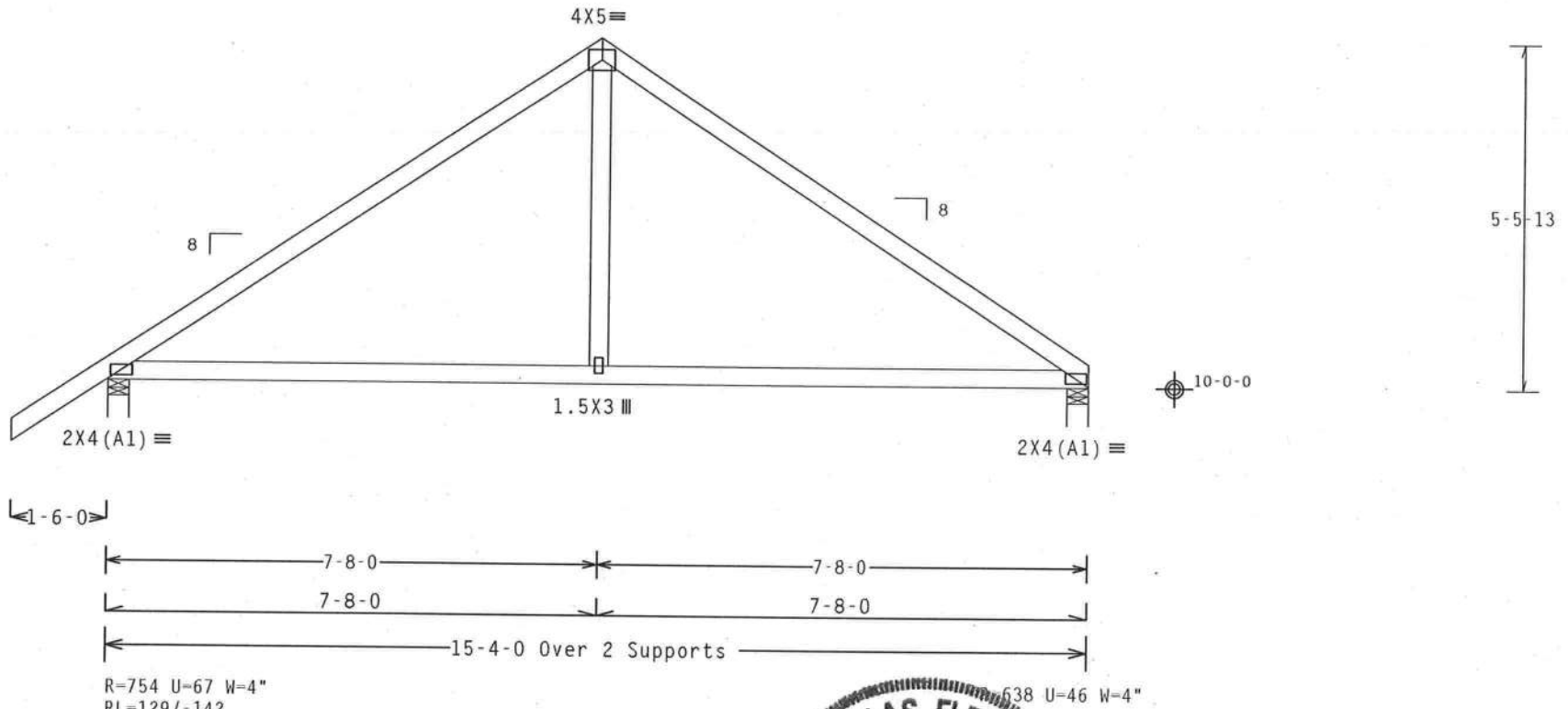
Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.03.04.0601.16 QTY 1

FL/-/4/-/-/R/-

Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

TC LL	20.0 PSF	REF R487-- 16776
TC DL	10.0 PSF	DATE 12/12/11
BC DL	10.0 PSF	DRW HCUSR487 11346009
BC LL	0.0 PSF	HC-ENG DF/DF *
TOT.LD.	40.0 PSF	SEQN- 253070
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - FGE)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP #1::Stack Chord SC2 2x4 SP #1:

Roof overhang supports 2.00 psf soffit load.

See DWGS A11015050109 & GBLLETIN0109 for more requirements.

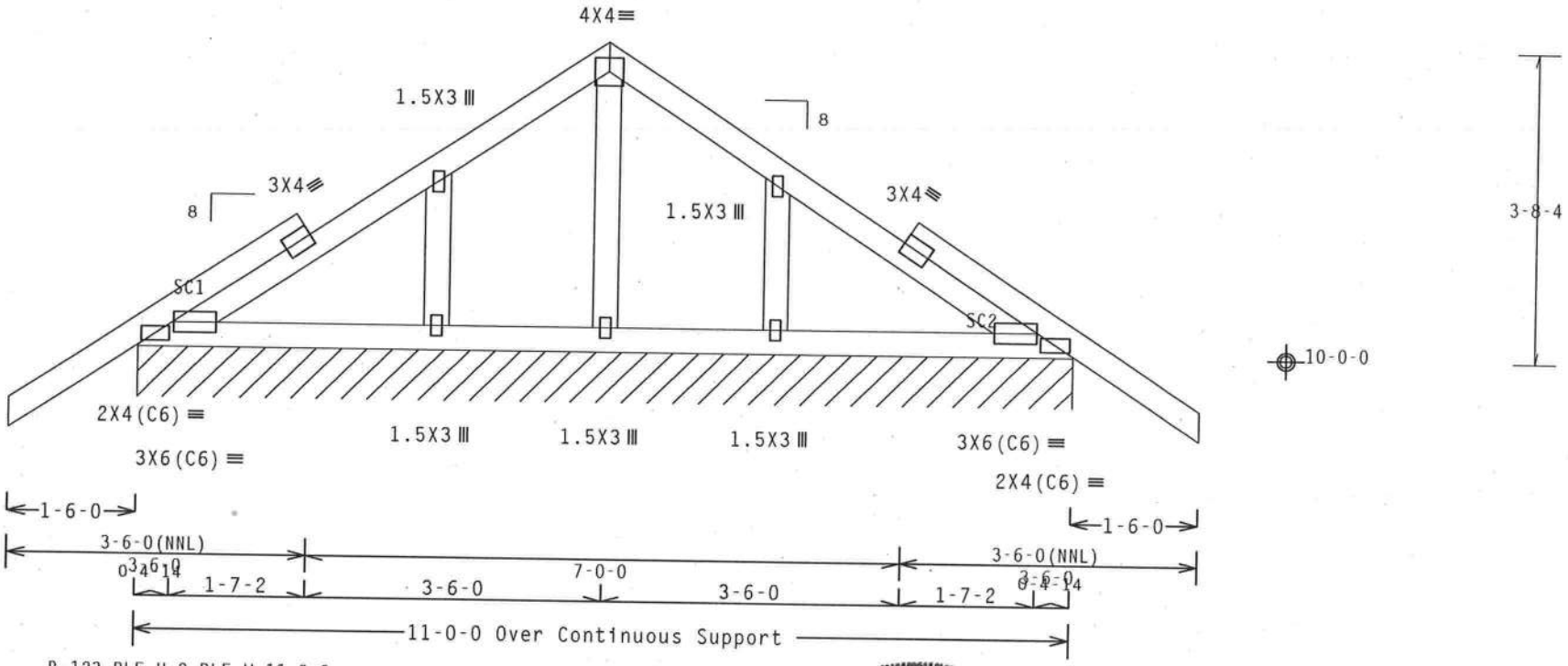
Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



R=123 PLF U=9 PLF W=11-0-0
RL=11/-11 PLF

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.03.04.0601.16

QTY: 8

FL/-/4/-/-/R/-

Scale =.5"/Ft.

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R487-- 16777
DATE	12/12/11
DRW	HCUSR487 11346001
HC-ENG	DF/DF *
SEQN-	255075
JREF-	1UHT487_Z01

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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12/12/2011

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - F)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

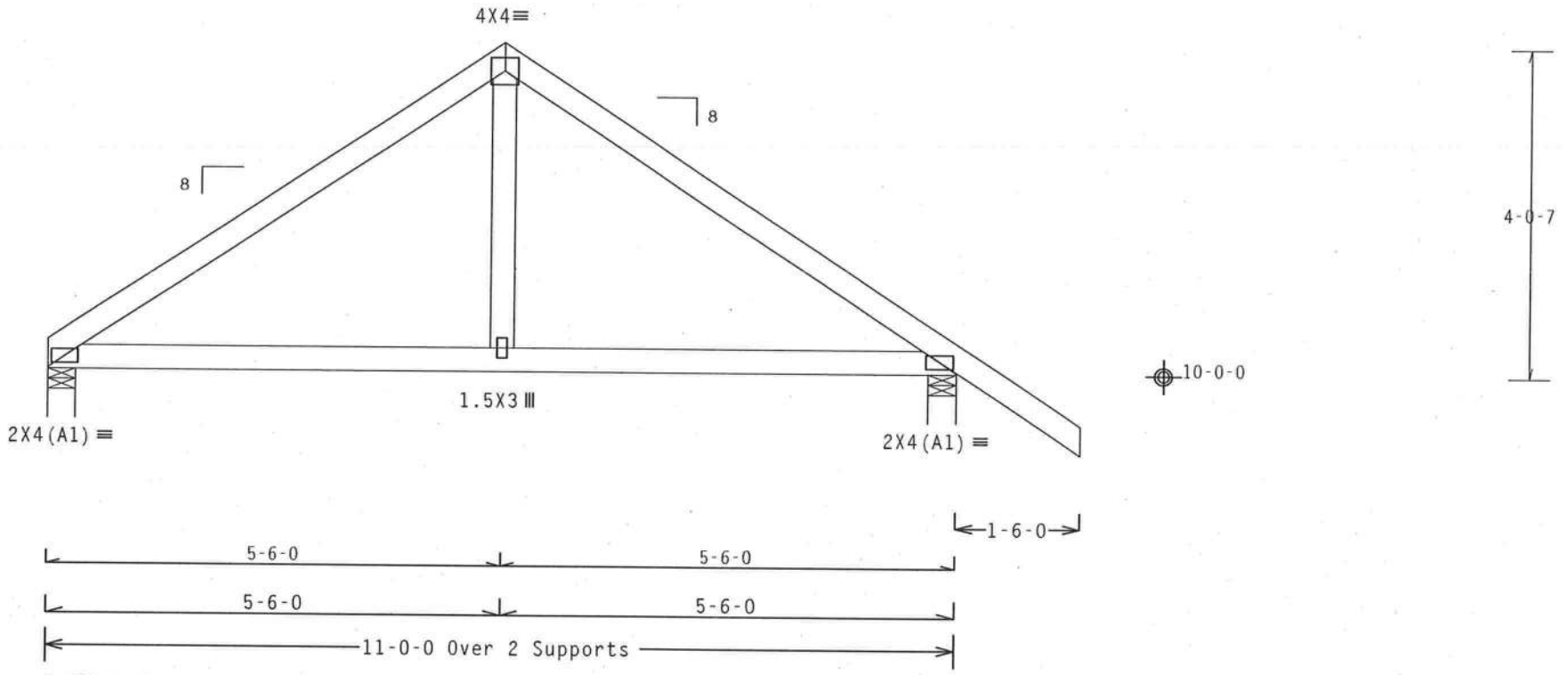
Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.03.04.060/1.16 QTY 1

FL/-/4/-/-/R/-

Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

TC LL	20.0 PSF	REF R487-- 16778
TC DL	10.0 PSF	DATE 12/12/11
BC DL	10.0 PSF	DRW HCUSR487 11346002
BC LL	0.0 PSF	HC-ENG DF/DF *
TOT.LD.	40.0 PSF	SEQN- 255072
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - F1)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

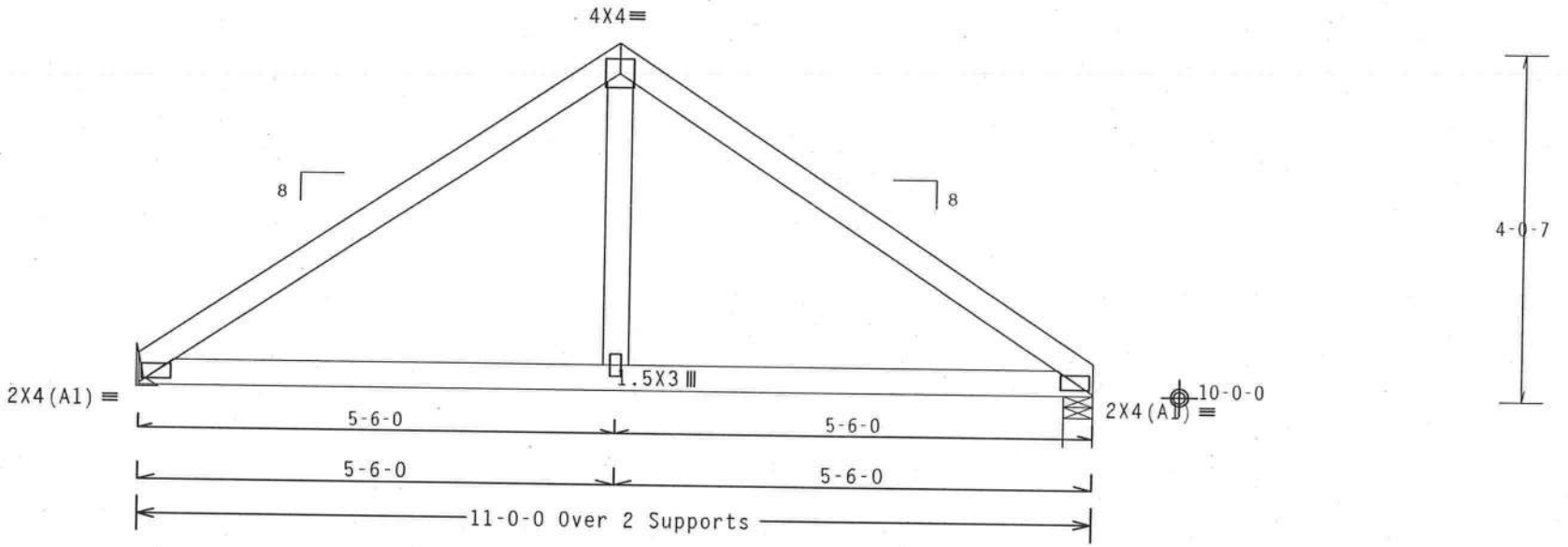
Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.



R=461 U=34
RL=81/-81

463 U=34 W=4"

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.03.04 0601 18

QTY:2

FL/-/4/-/-/R/-

Scale =.5"/Ft.

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R487-- 16779
DATE	12/12/11
DRW	HCUSR487 11346003
HC-ENG	DF/DF
SEQN-	255069
JREF-	1UHT487_Z01

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - G)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1 :T2, T4 2x6 SP #1 Dense:
Bot chord 2x4 SP #1 :B2 2x8 SP #1 Dense:
Webs 2x4 SP #3

End verticals not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.11" due to live load and 0.14" due to dead load.

Bottom chord checked for 10.00 psf non-concurrent live load.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 5-4-0 to 16-4-0.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

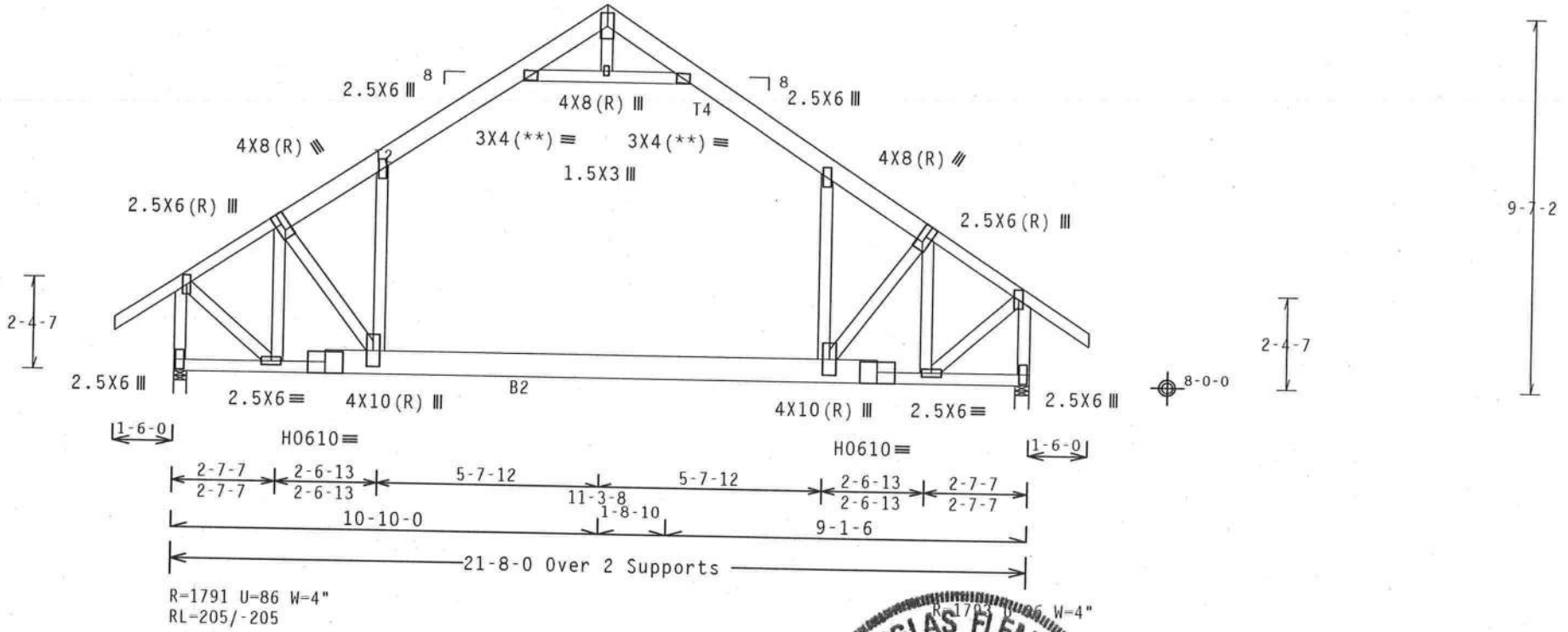
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets L/240 live and L/180 total load.



PLT TYP. 20 Gauge HS,Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE
10.03.04.0601.16
No. 66648
STATE OF FLORIDA
PROFESSIONAL ENGINEER
12/12/2011

QTY:

FL/-/4/-/-/R/-		Scale =.25"/Ft.
TC LL	20.0 PSF	REF R487-- 16780
TC DL	10.0 PSF	DATE 12/12/11
BC DL	10.0 PSF	DRW HCUSR487 11346001
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 253343
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - G1)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1 :T2, T4 2x6 SP #1 Dense:
Bot chord 2x4 SP #1 :B2 2x8 SP #1 Dense:
Webs 2x4 SP #3

End verticals not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.11" due to live load and 0.14" due to dead load.

Bottom chord checked for 10.00 psf non-concurrent live load.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 5-4-0 to 16-4-0.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

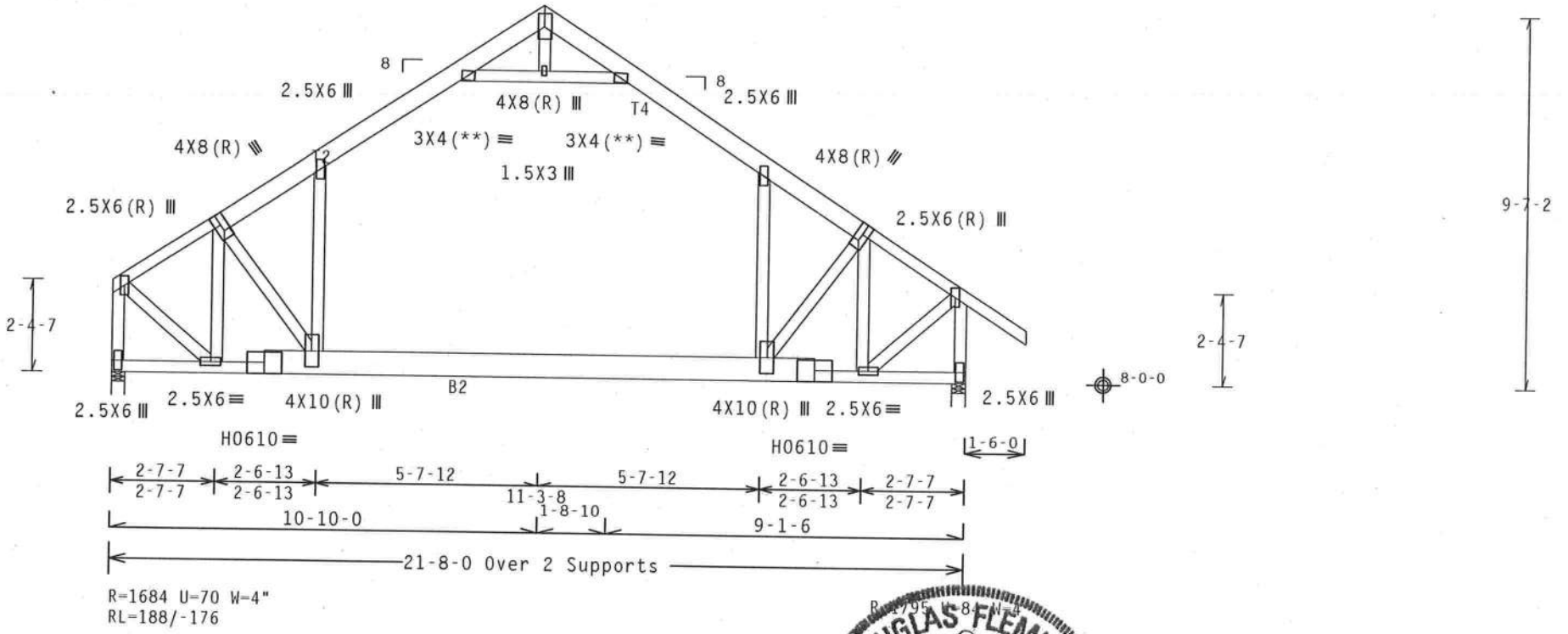
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets L/240 live and L/180 total load.



R=1684 U=70 W=4"
RL=188/-176

R=1795 U=84 W=4"

PLT TYP. 20 Gauge HS,Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.03.04 0601.16 QTY:2

FL/-/4/-/-/R/-

Scale =.25"/Ft.

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R487-- 16781
DATE	12/12/11
DRW	HCUSR487 11346012
HC-ENG	DF/DF
SEQN-	253191
JREF-	1UHT487_Z01

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING
LICENSE
No. 66648
STATE OF
FLORIDA
PROFESSIONAL ENGINEER
12/12/2011

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - GGE)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1 :T3, T5 2x6 SP #1 Dense:
Bot chord 2x4 SP #1 :B2 2x8 SP #1 Dense:
Webs 2x4 SP #3

End verticals not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

Gable end supports 8" max rake overhang.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets L/240 live and L/180 total load.

(**) 3 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

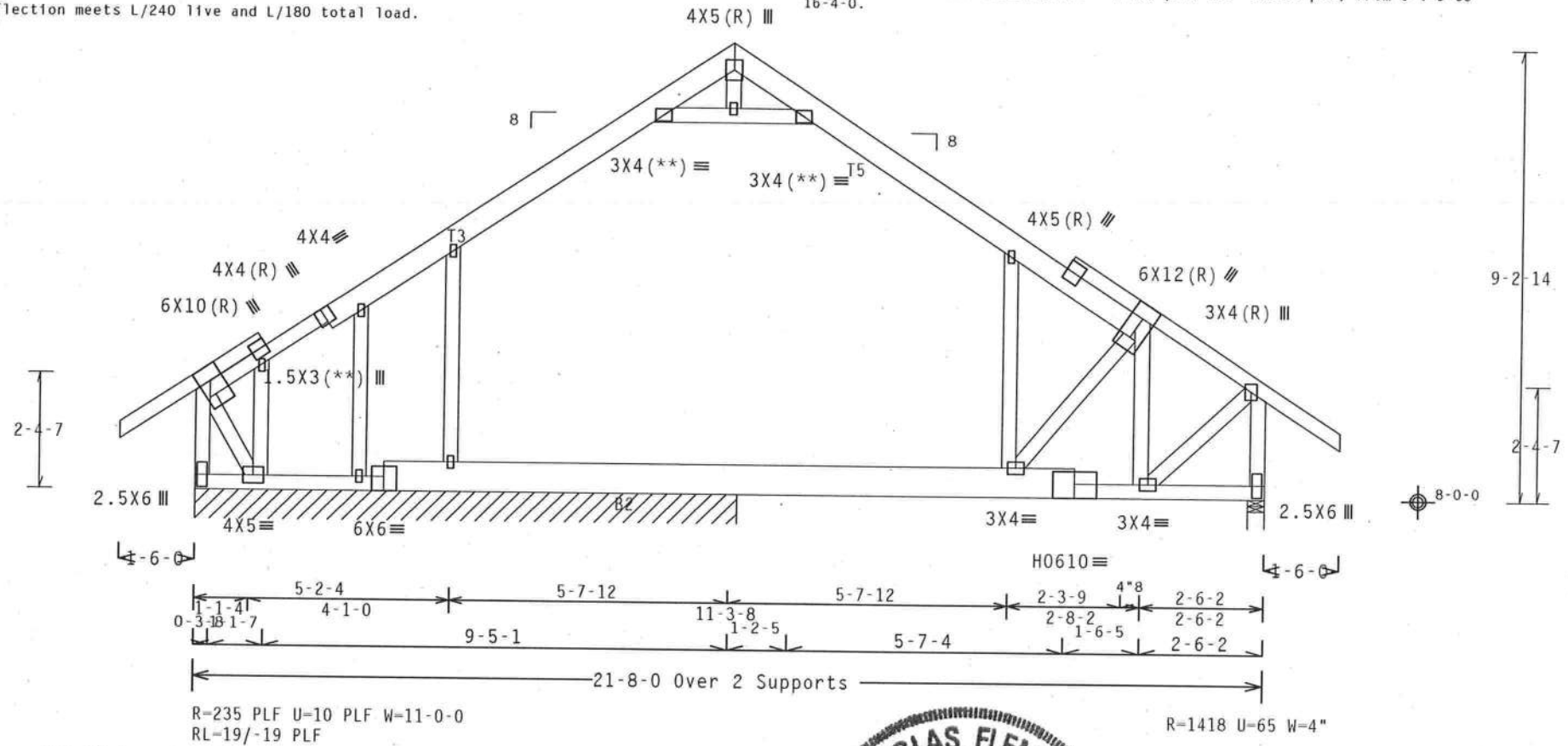
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

See DWGS A11015050109 & GBLLETIN0109 for more requirements.

Bottom chord checked for 10.00 psf non-concurrent live load.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 5-4-0 to 16-4-0.



R=235 PLF U=10 PLF W=11-0-0
RL=19/-19 PLF

R=1418 U=65 W=4"

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. 20 Gauge HS,Wave

FT/RT=10%(0%)/0(0)

10.03.04.0601.16 QTY 1

FL/-/4/-/-/R/-

Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R487-- 16782
DATE	12/12/11
DRW	HCUSR487 11346044
HC-ENG	DF/DF
SEQN-	253322
JREF-	1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - G4)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1 :T2, T4 2x6 SP #1 Dense:
Bot chord 2x6 SP #2 :B2 2x8 SP #1 Dense:
:B3 2x4 SP #1:
Webs 2x4 SP #3 :W1 2x4 SP #2:
:Lt Bearing Leg 2x6 SP #2:

Left cantilever is exposed to wind

Roof overhang supports 2.00 psf soffit load.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 5-4-0 to 16-2-4.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

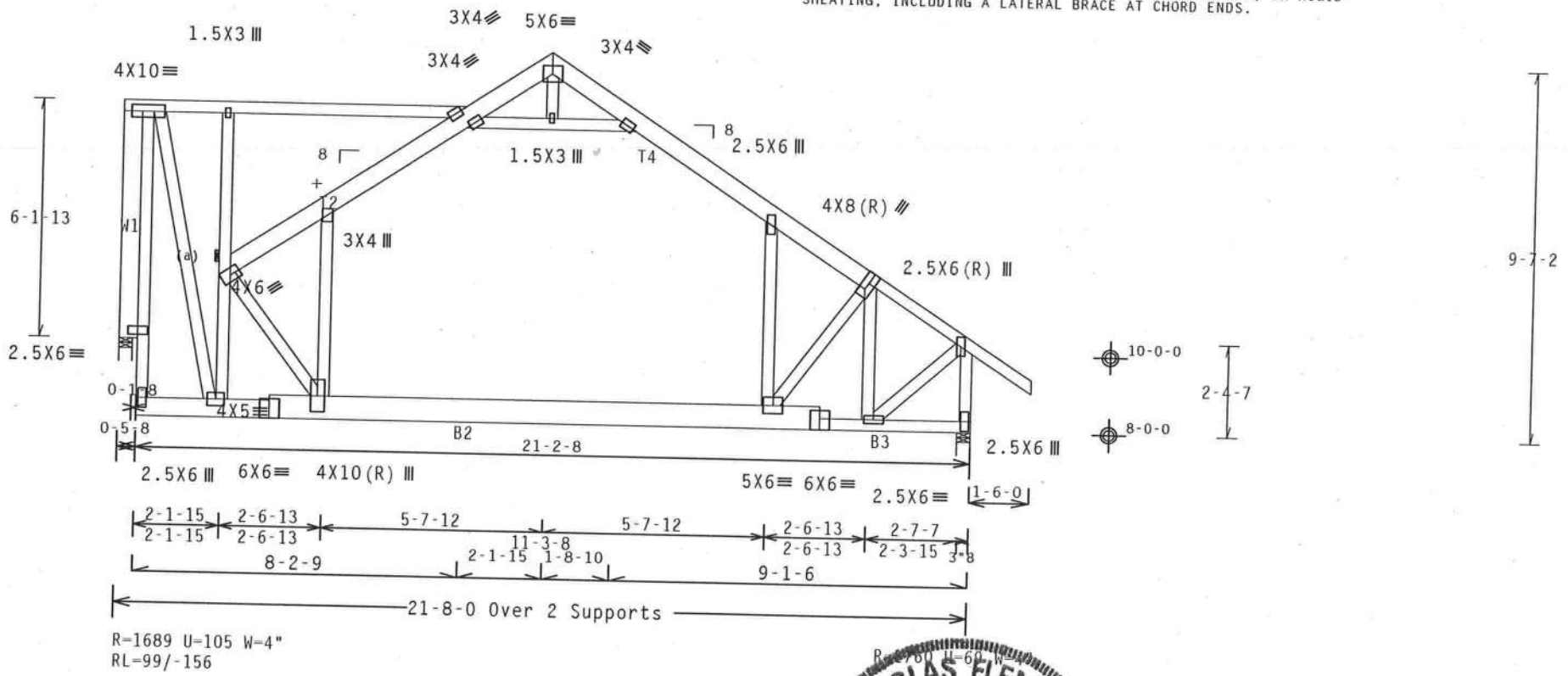
Max JT VERT DEFL: LL: 0.12" DL: 0.14" recommended camber 1/4"

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Collar-tie braced with continuous lateral bracing at 24" OC, or rigid ceiling.

Deflection meets L/240 live and L/180 total load.

+LATERALLY BRACE TOP CHORD BELOW FILLER AT 24" O.C, OR RIGID SHEATING, INCLUDING A LATERAL BRACE AT CHORD ENDS.



R=1689 U=105 W=4"
RL=99/-156

R=760 U=60 W=4"

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.0 8.0 4.0 0 6 0 1.1 6

QTY:

FL/-/4/-/-/R/-

Scale =.25"/Ft.

TC LL	20.0 PSF	REF	R487-- 16783
TC DL	10.0 PSF	DATE	12/12/11
BC DL	10.0 PSF	DRW	HCUSR487 11346033
BC LL	0.0 PSF		HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN-	253340
DUR.FAC.	1.25		
SPACING	24.0"		JREF- 1UHT487_Z01

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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12/12/2011

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - G3)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1 :T2, T4 2x6 SP #1 Dense:
Bot chord 2x4 SP #1 :B2 2x8 SP #1 Dense:
Webs 2x4 SP #3
:Lt Bearing Leg 2x6 SP #2:

Left cantilever is exposed to wind

Roof overhang supports 2.00 psf soffit load.

Max JT VERT DEFL: LL: 0.12" DL: 0.15" recommended camber 1/4"

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 5-4-0 to 16-4-0.

+LATERALLY BRACE TOP CHORD BELOW FILLER AT 24" O.C, OR RIGID SHEATING, INCLUDING A LATERAL BRACE AT CHORD ENDS.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

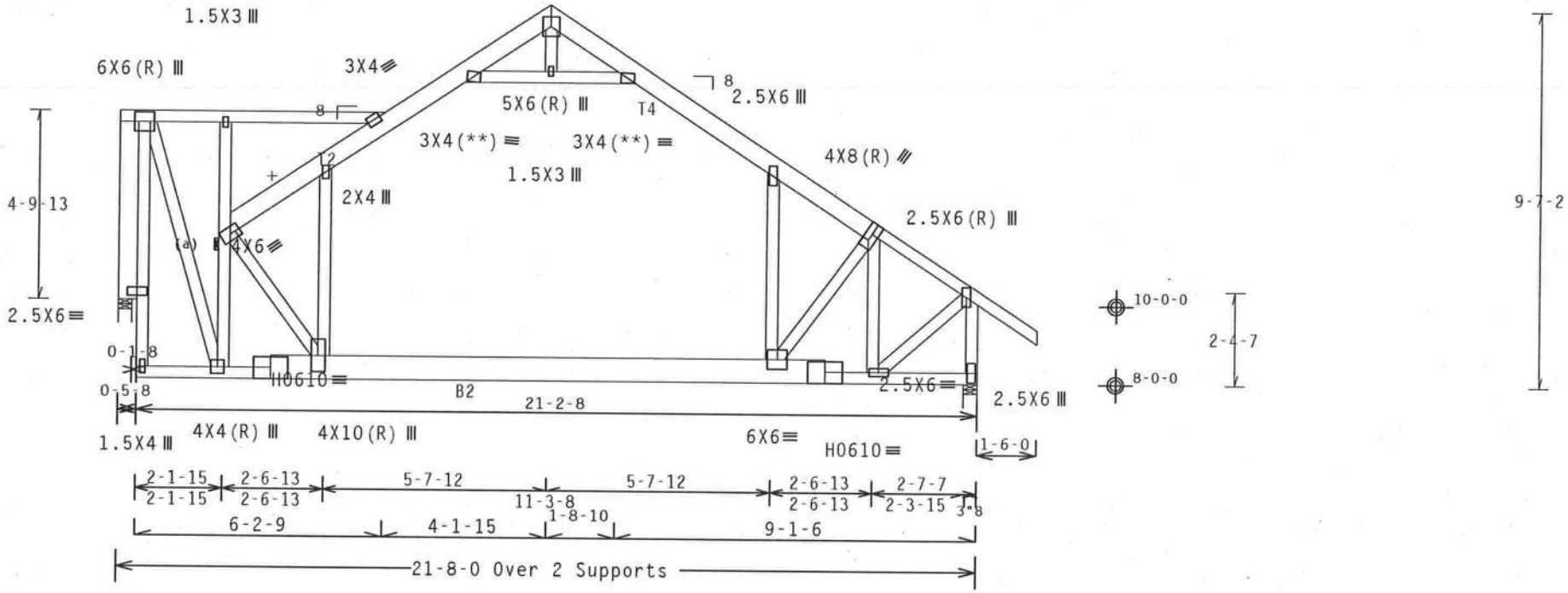
Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets L/240 live and L/180 total load.



PLT TYP. 20 Gauge HS,Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.03.04.0601.16 QTY 1

FL/-/4/-/-/R/-

Scale =.25"/Ft.

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

TC LL	20.0 PSF	REF R487-- 16784
TC DL	10.0 PSF	DATE 12/12/11
BC DL	10.0 PSF	DRW HCUSR487 11346021
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 253337
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UHT487_Z01

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - G2)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1 :T2, T4 2x6 SP #1 Dense:
Bot chord 2x4 SP #1 :B2 2x8 SP #1 Dense:
Webs 2x4 SP #3
:Lt Bearing Leg 2x6 SP #2:

Left cantilever is exposed to wind

Roof overhang supports 2.00 psf soffit load.

Max JT VERT DEFL: LL: 0.12" DL: 0.15" recommended camber 1/4"

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets L/240 live and L/180 total load.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

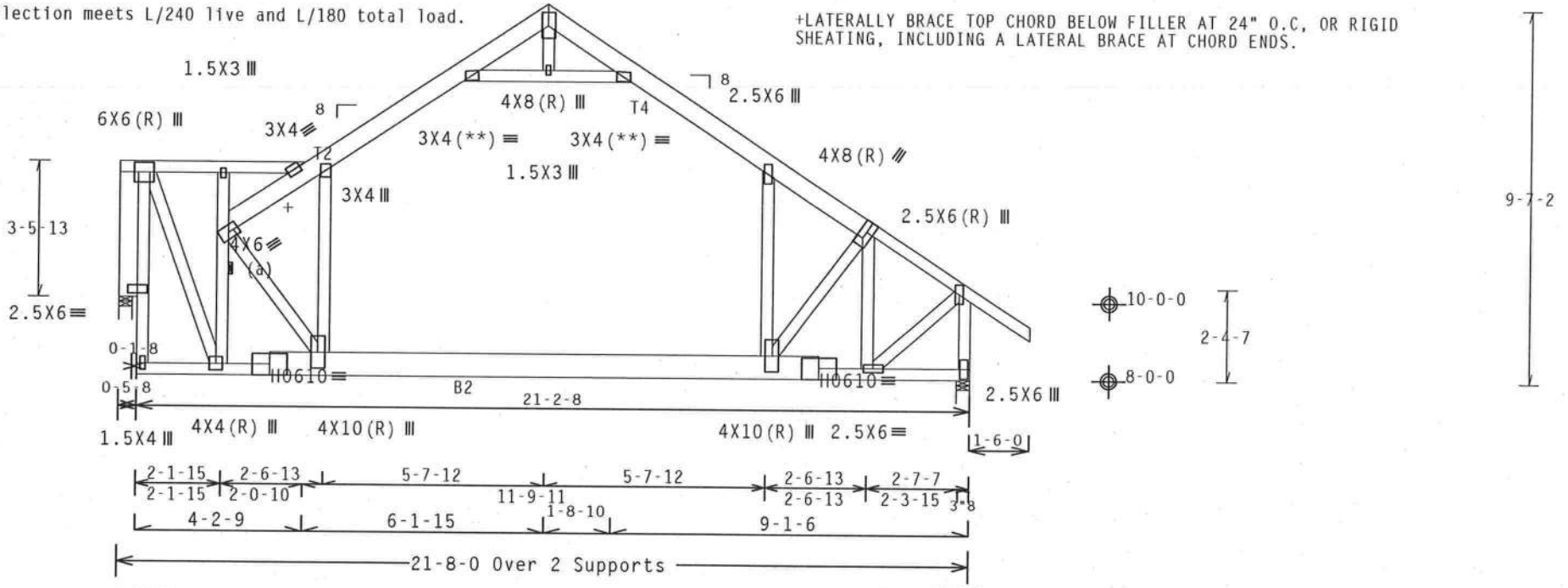
Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 5-4-0 to 16-4-0.

+LATERALLY BRACE TOP CHORD BELOW FILLER AT 24" O.C, OR RIGID SHEATING, INCLUDING A LATERAL BRACE AT CHORD ENDS.



R=1696 U=86 W=4"
RL=140/-154

R=1696 U=86 W=4"

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. 20 Gauge HS,Wave

FT/RT=10%(0%)/0(0)

10.03.04.0601.16

QTY 1

FL/-/4/-/-/R/-

Scale =.25"/Ft.

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

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TC LL	20.0 PSF	REF R487-- 16785
TC DL	10.0 PSF	DATE 12/12/11
BC DL	10.0 PSF	DRW HCUSR487 11346022
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 253391
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - CJ1)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1

Roof overhang supports 2.00 psf soffit load.

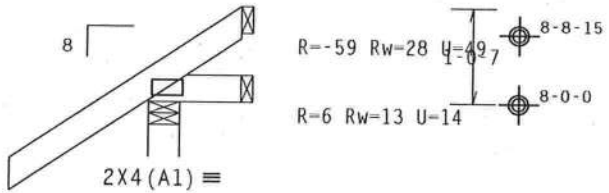
Bottom chord checked for 10.00 psf non-concurrent live load.

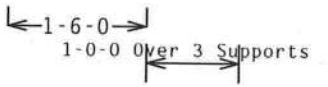
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCp1(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load.





R=261 U=44 W=4"
RL=38/-32

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.03.04.0601.16 QTY 10 FL/-/4/-/-/R/- Scale =.5"/Ft.

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 16786
TC DL	10.0 PSF	DATE	12/12/11
BC DL	10.0 PSF	DRW	HCUSR487 11346002
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	252741
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - CJ3)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1

Roof overhang supports 2.00 psf soffit load.

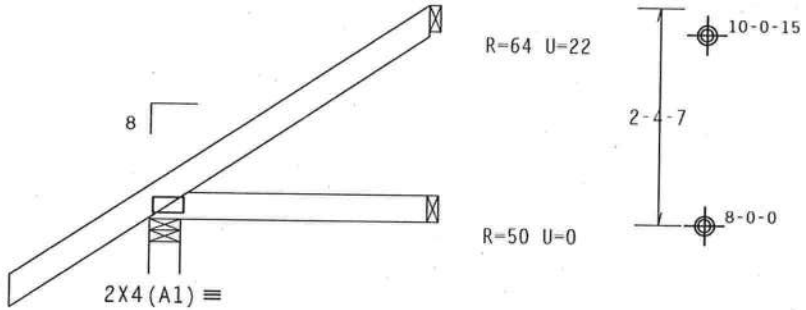
Bottom chord checked for 10.00 psf non-concurrent live load.

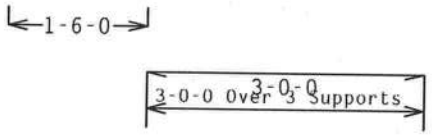
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00
GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load.





R=268 U=17 W=4"
RL=69/-42

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.05.04.0601.16 QTY: 8

FL/-/4/-/-/R/- Scale =.5"/Ft.

TC LL	20.0 PSF	REF	R487-- 16787
TC DL	10.0 PSF	DATE	12/12/11
BC DL	10.0 PSF	DRW	HCUSR487 11346003
BC LL	0.0 PSF	HC-ENG	DF/DF *
TOT.LD.	40.0 PSF	SEQN-	252744
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UHT487_Z01

DOUGLAS FLEMING LICENSE No. 66848 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/12/2011

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - CJ5)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1

Roof overhang supports 2.00 psf soffit load.

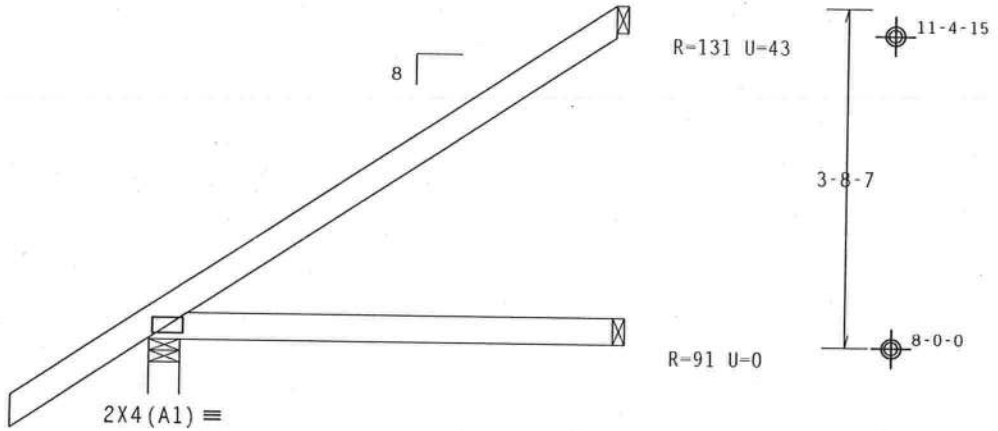
Bottom chord checked for 10.00 psf non-concurrent live load.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load.



R=339 U=10 W=4"
RL=99/-51

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.03.04.0601.15 QTY 8 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

TC LL	20.0 PSF	REF R487-- 16788
TC DL	10.0 PSF	DATE 12/12/11
BC DL	10.0 PSF	DRW HCUSR487 11346038
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 252747
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - EJ7)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

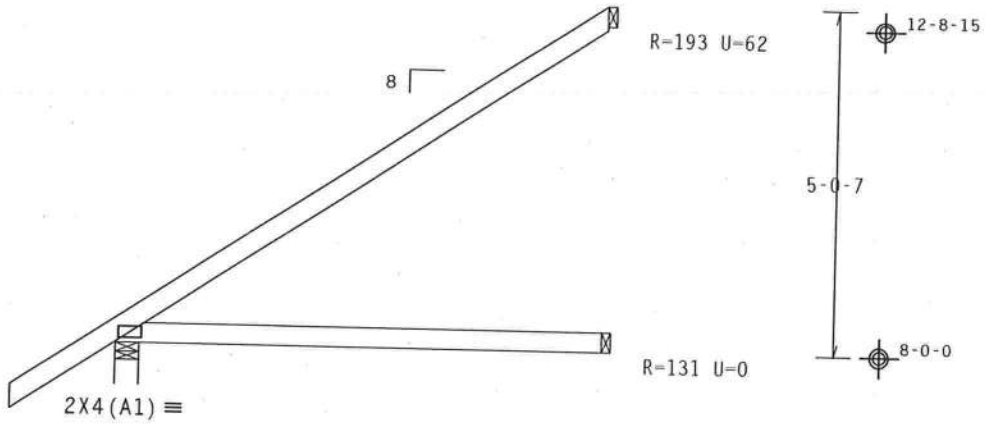
MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

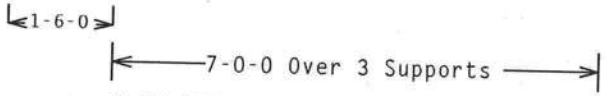
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.





R=417 U=5 W=4"
RL=130/-61

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.00.04.0603.16 QTY: 2 FL/-/4/-/-/R/-

Scale =.375"/Ft.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R487-- 16789
DATE	12/12/11
DRW	HCUSR487 11346005
HC-ENG	DF/DF
SEQN-	252750
JREF-	1UHT487_Z01

(11-225--F111 in later STANLEY CRAWFORD/SCCI 262 -- , ** - HJ7)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

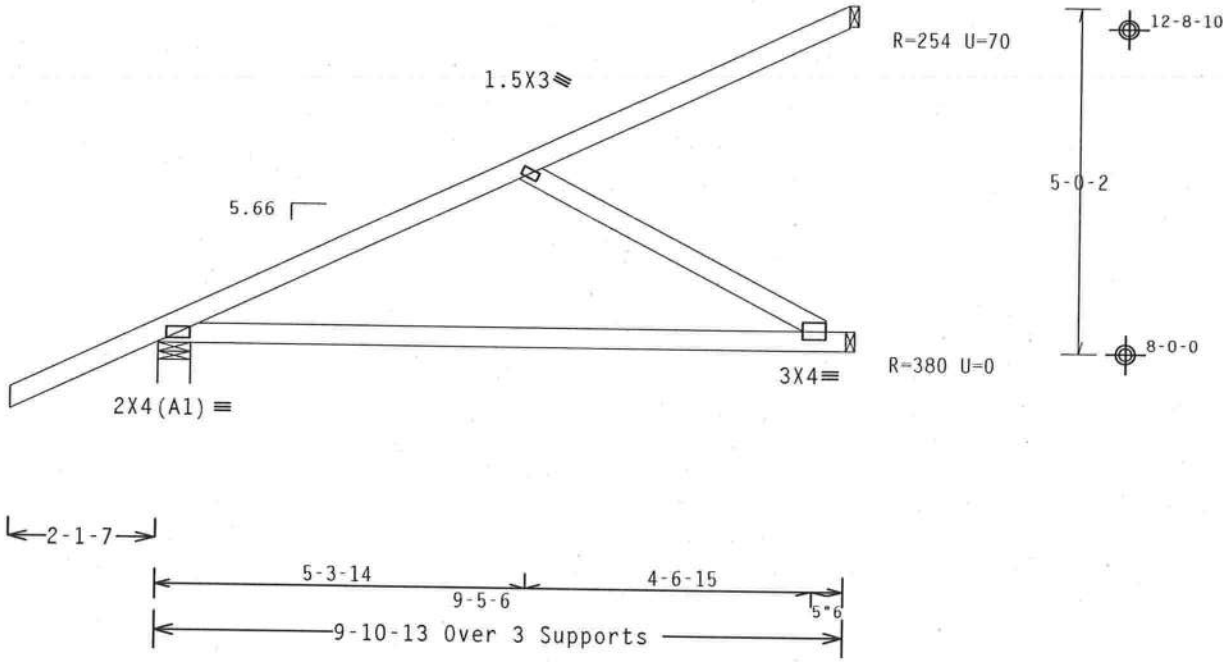
Hipjack supports 7-0-0 setback jacks with no webs.

Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00
GCp1(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (3) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.03.04.0601.16 QTY: 4 FL/-/4/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

TC LL	20.0 PSF	REF	R487-- 16790
TC DL	10.0 PSF	DATE	12/12/11
BC DL	10.0 PSF	DRW	HCUSR487 11346006
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	252756
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - EJ5)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Left end vertical exposed to wind pressure. Deflection meets L/240 criteria for brittle and flexible wall coverings.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load.

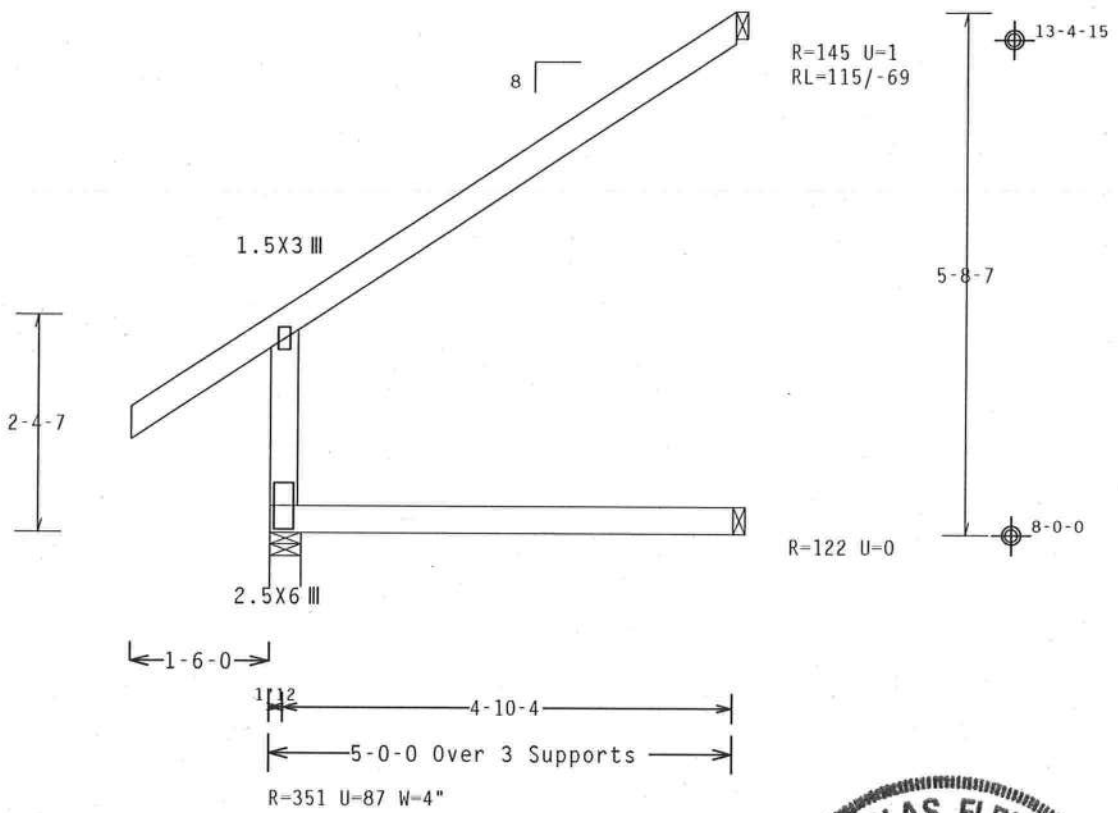
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.03.04.0601.16 QTY:

FL/-/4/-/-/R/-

Scale =.5"/Ft.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF R487-- 16791
TC DL	10.0 PSF	DATE 12/12/11
BC DL	10.0 PSF	DRW HCUSR487 11346019
BC LL	0.0 PSF	HC-ENG DF/DF *
TOT.LD.	40.0 PSF	SEQN- 252759
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - CJ1S)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Left end vertical exposed to wind pressure. Deflection meets L/240 criteria for brittle and flexible wall coverings.

Bottom chord checked for 10.00 psf non-concurrent live load.

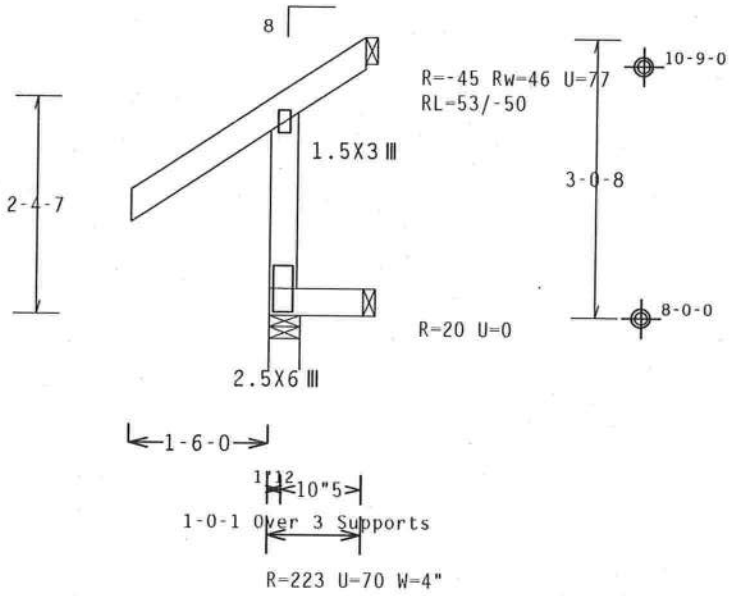
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.03.04.0601.16 QTY.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

FL/-/4/-/-/R/-		Scale =.5"/Ft.
TC LL	20.0 PSF	REF R487-- 16792
TC DL	10.0 PSF	DATE 12/12/11
BC DL	10.0 PSF	DRW HCUSR487 11346018
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 252762
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UHT487_Z01

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - CJ3S)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Left end vertical exposed to wind pressure. Deflection meets L/240 criteria for brittle and flexible wall coverings.

Bottom chord checked for 10.00 psf non-concurrent live load.

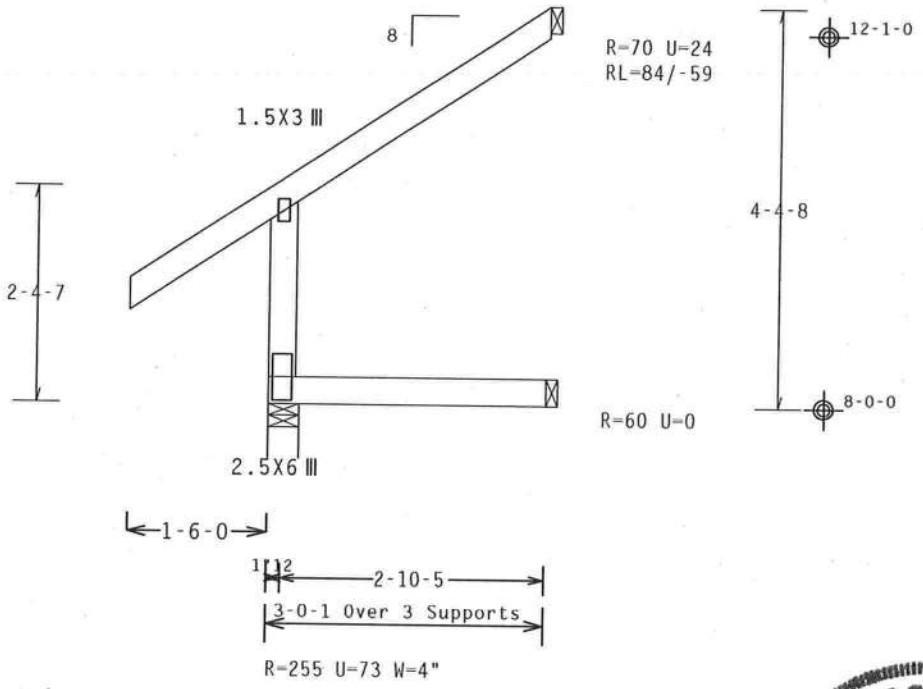
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.03.04.0601.16 QTY 1 FL/-/4/-/-/R/- Scale =.5"/Ft.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 16793
TC DL	10.0 PSF	DATE	12/12/11
BC DL	10.0 PSF	DRW	HCUSR487 11346016
BC LL	0.0 PSF	HC-ENG	DF/DF *
TOT.LD.	40.0 PSF	SEQN-	252765
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - CJ4)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1

Roof overhang supports 2.00 psf soffit load.

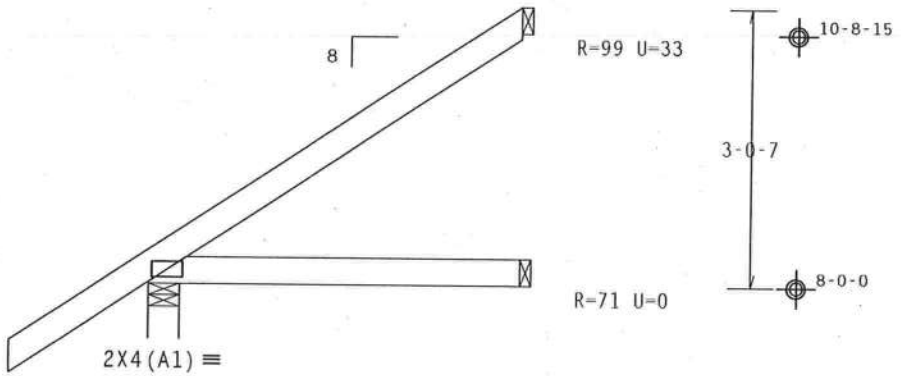
Bottom chord checked for 10.00 psf non-concurrent live load.

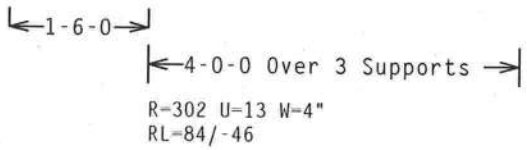
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCp1(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load.





R=302 U=13 W=4"
RL=84/-46

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.03.04.0601.16 QTY: 1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

TC LL	20.0 PSF	REF R487-- 16794
TC DL	10.0 PSF	DATE 12/12/11
BC DL	10.0 PSF	DRW HCUSR487 11346017
BC LL	0.0 PSF	HC-ENG DF/DF *
TOT.LD.	40.0 PSF	SEQN- 252768
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - CJ6)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1

Roof overhang supports 2.00 psf soffit load.

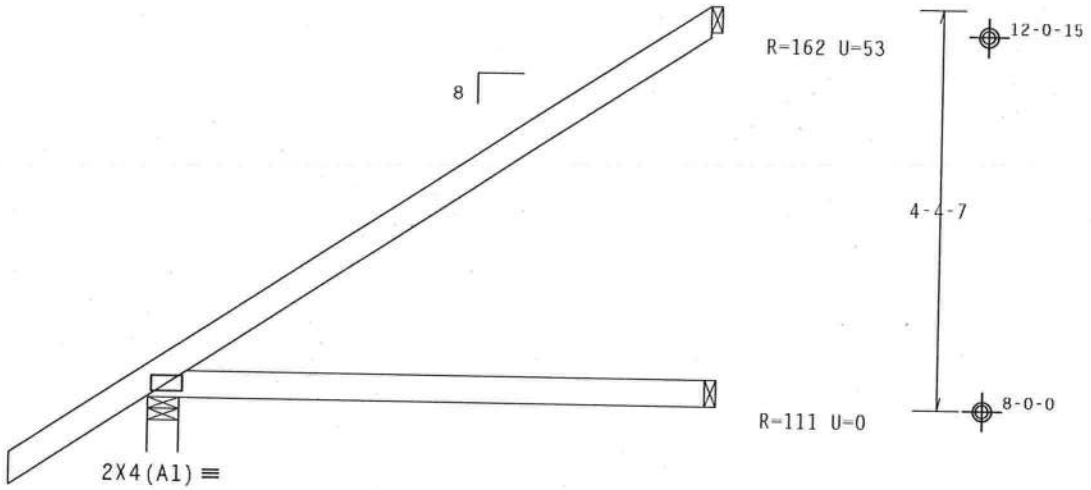
Bottom chord checked for 10.00 psf non-concurrent live load.

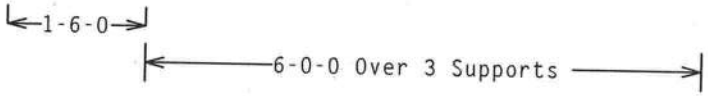
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load.





R=377 U=7 W=4"
RL=115/-56

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.03.04.060/1.16 QTY 1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/12/2011

TC LL	20.0 PSF	REF R487-- 16795
TC DL	10.0 PSF	DATE 12/12/11
BC DL	10.0 PSF	DRW HCUSR487 11346015
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 252771
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UHT487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - HJ5)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

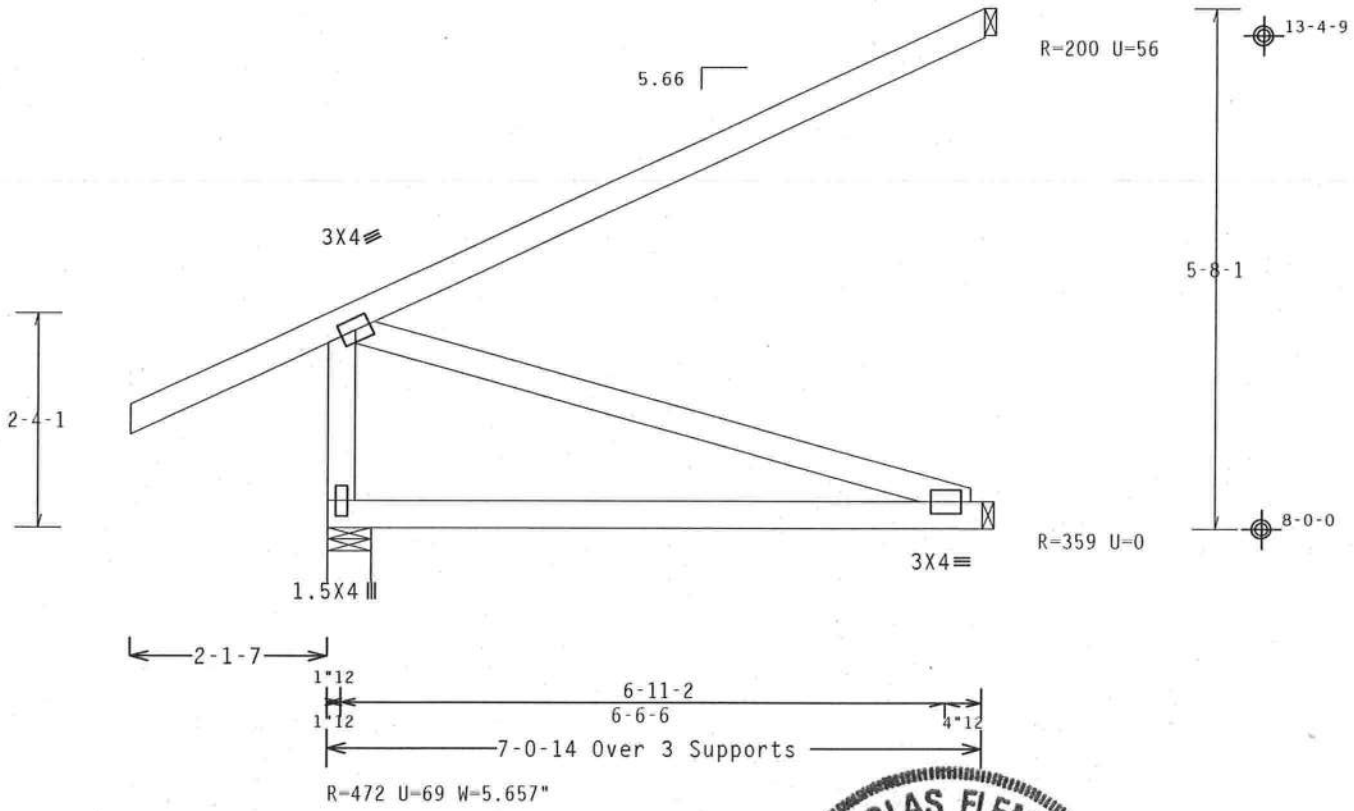
Hipjack supports 8-0-0 setback one face and 5-0-0 setback opposite face.
Jacks up to 7' have no webs. Longer jacks supported to BC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (3) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.03.04.0601.16 QTY: 1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/12/2011

TC LL	20.0 PSF	REF	R487-- 16796
TC DL	10.0 PSF	DATE	12/12/11
BC DL	10.0 PSF	DRW	HCUSR487 11346042
BC LL	0.0 PSF		HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN-	252779
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - EJ3)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

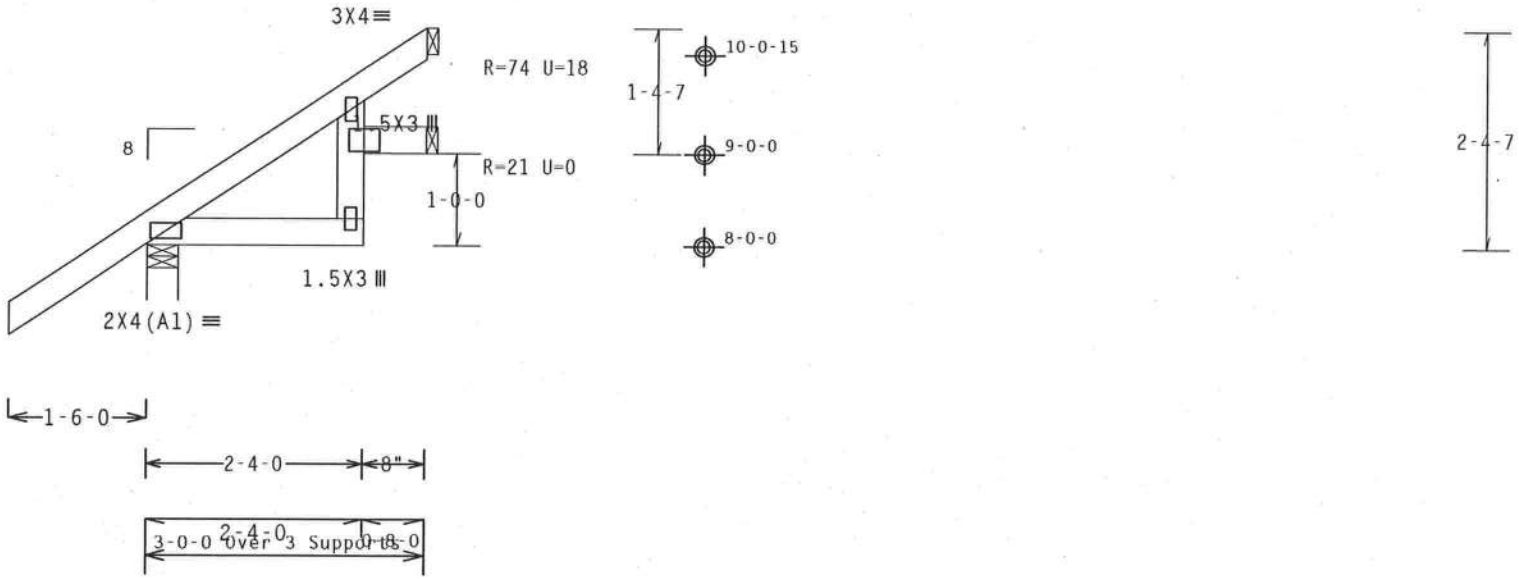
Bottom chord checked for 10.00 psf non-concurrent live load.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.03.04.0601.16 QTY 1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

TC LL	20.0 PSF	REF R487-- 16797
TC DL	10.0 PSF	DATE 12/12/11
BC DL	10.0 PSF	DRW HCUSR487 11346037
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 252783
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - HJ3)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

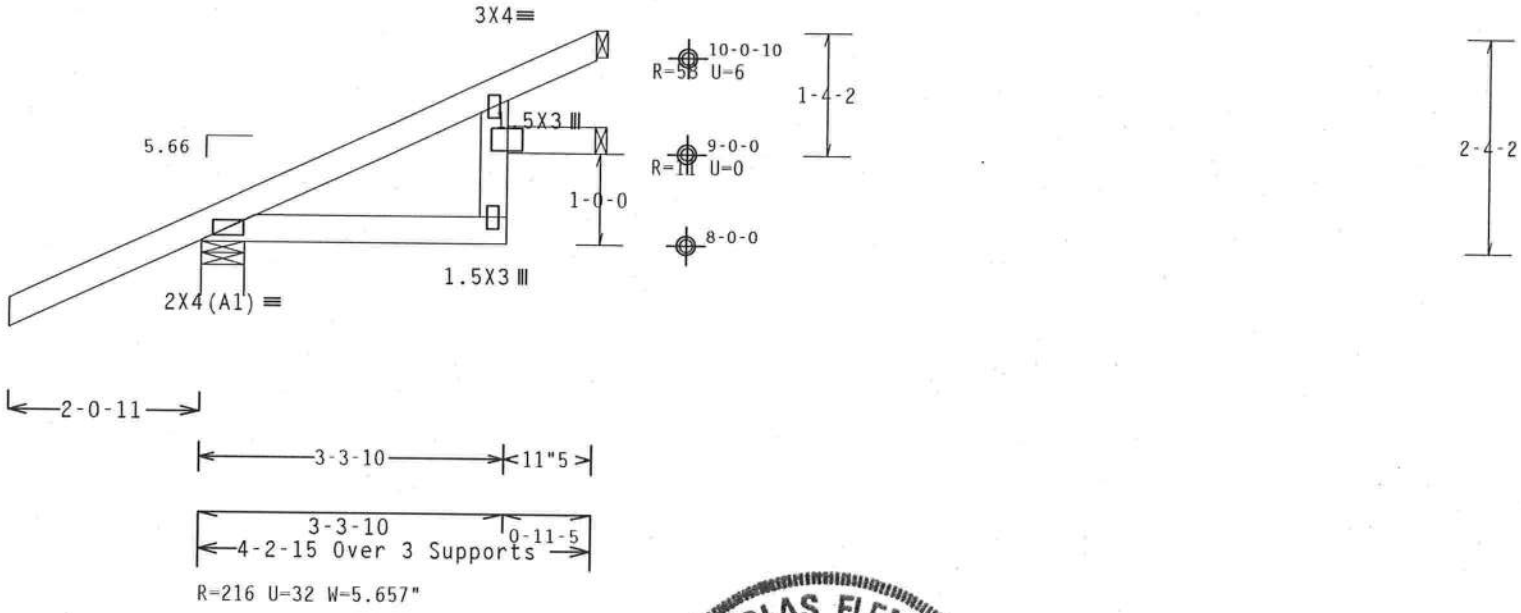
Hipjack supports 3-0-0 setback jacks with no webs.

Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00
GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



R=216 U=32 W=5.657"

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.05.04.0601.16 QTY:

FL/-/4/-/-/R/-

Scale =.5"/Ft.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/12/2011

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R487-- 16798
DATE	12/12/11
DRW	HCUSR487 11346025
HC-ENG	DF/DF
SEQN-	252787
JREF-	1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - H3M)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

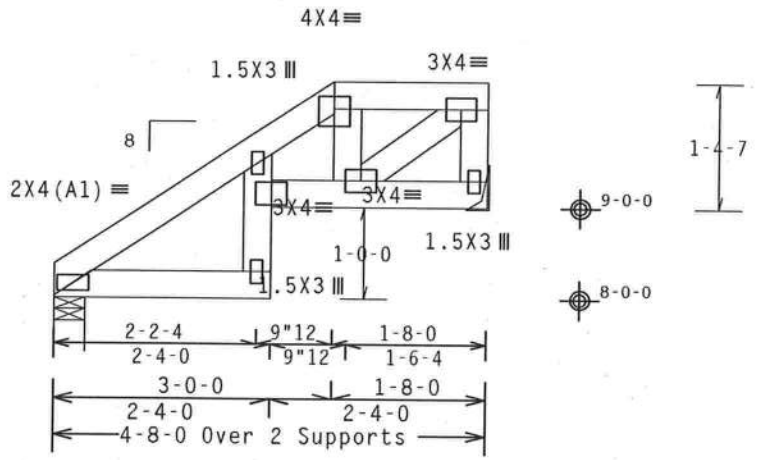
Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 64 plf at 0.00 to 64 plf at 3.00
TC- From 64 plf at 3.00 to 64 plf at 4.67
BC- From 20 plf at 0.00 to 20 plf at 2.33
BC- From 20 plf at 2.33 to 20 plf at 4.67
TC- 131.55 lb Conc. Load at 3.02
BC- 31.99 lb Conc. Load at 3.02

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.



R=262 U=27 W=4" R=294 U=32

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.03.04.0601.16 QTY: 6

FL/-/4/-/-/R/-

Scale =.5"/Ft.

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R487-- 16799
DATE	12/12/11
DRW	HCUSR487 11346013
HC-ENG	DF/DF
SEQN-	252877
JREF-	1UHT487_Z01

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/12/2011

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
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(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - MG)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1 :B1 2x6 SP #2:
Webs 2x4 SP #3

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 64 plf at 0.00 to 64 plf at 3.42
TC- From 64 plf at 3.42 to 64 plf at 4.37
BC- From 20 plf at 0.00 to 20 plf at 2.33
BC- From 20 plf at 2.33 to 20 plf at 3.33
BC- From 20 plf at 3.33 to 20 plf at 4.67
PLB- 915.68 lb Conc. Load at (0.73, 8.04)
PLB- 270.36 lb Conc. Load at (2.73, 9.04)

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

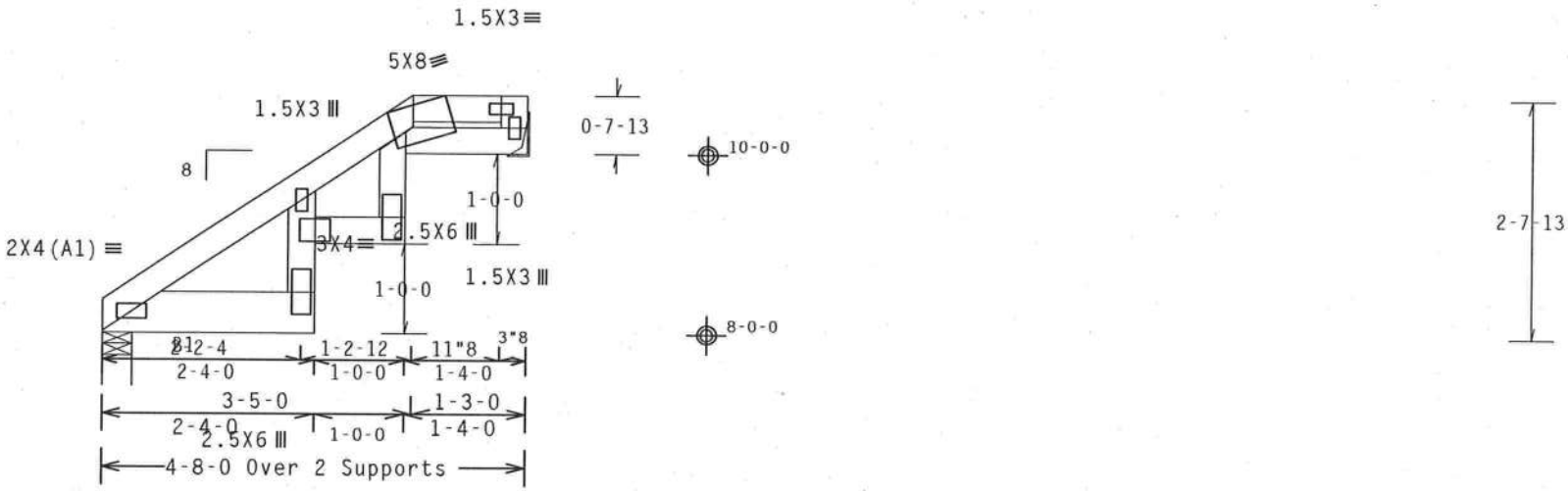
2 COMPLETE TRUSSES REQUIRED

Nail Schedule:0.131"x3" nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @ 6.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load.



R=1115 U=75 W=4" R=444 U=30

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.03.04.0601.16 QTY 1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

TC LL	20.0 PSF	REF R487-- 16800
TC DL	10.0 PSF	DATE 12/12/11
BC DL	10.0 PSF	DRW HCUSR487 11346031
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 252887
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - M5G)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

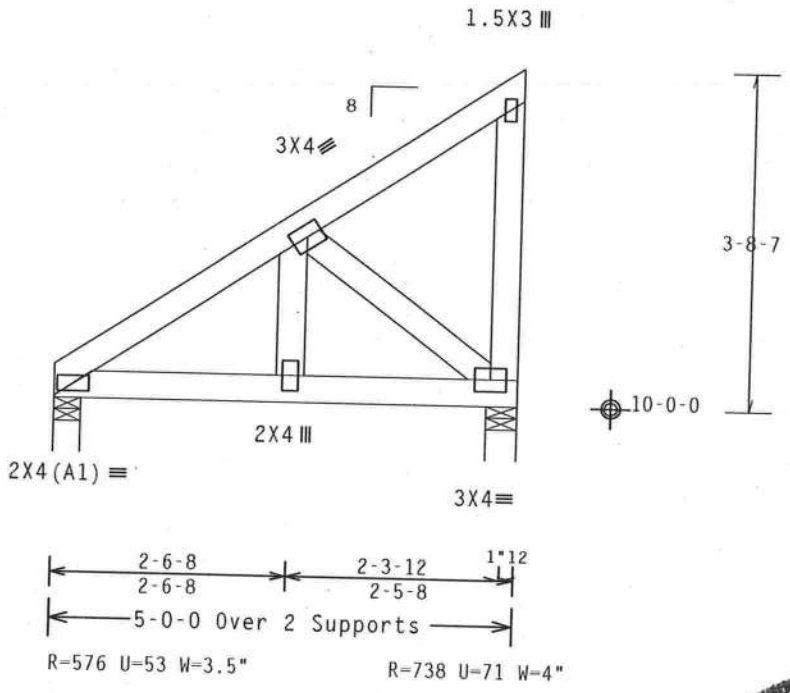
Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 64 plf at 0.00 to 64 plf at 5.00
BC- From 20 plf at 0.00 to 20 plf at 2.06
BC- From 10 plf at 2.06 to 10 plf at 5.00
BC- 461.31 lb Conc. Load at 2.06, 4.06

Right end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.03.04.0601.16 QTY: 1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

TC LL	20.0 PSF	REF R487-- 16801
TC DL	10.0 PSF	DATE 12/12/11
BC DL	10.0 PSF	DRW HCUSR487 11346004
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 255078
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - KGE)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP #1::Stack Chord SC2 2x4 SP #1:

End verticals exposed to wind pressure. Deflection meets L/240 criteria for brittle and flexible wall coverings.

Roof overhang supports 2.00 psf soffit load.

See DWGS A11015050109 & GBLLETIN0109 for more requirements.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

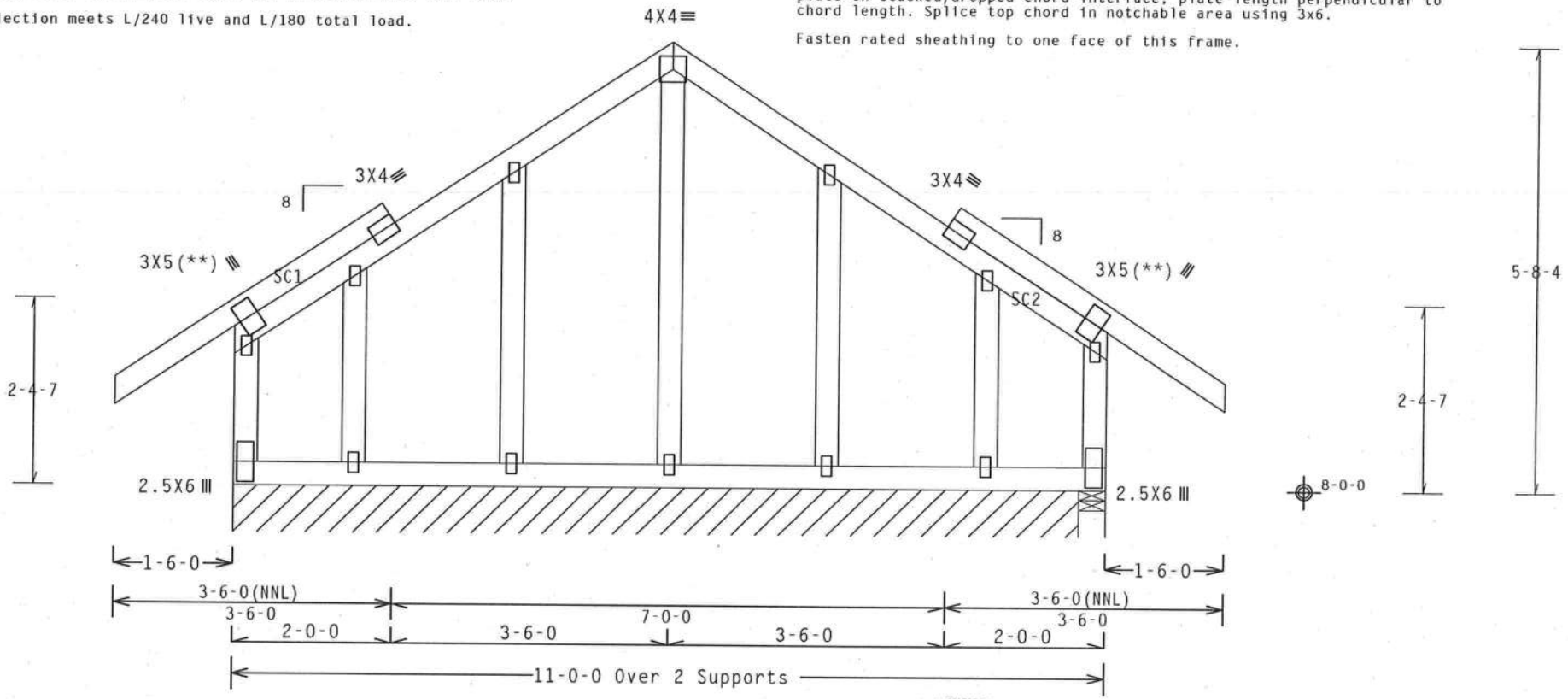
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Gable end supports 8" max rake overhang.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

Fasten rated sheathing to one face of this frame.



R=124 PLF U=5 PLF W=10-8-0
RL=16/-16 PLF

R=650 U=74 W=4"

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.04.0601.16

QTY: 1

FL/-/4/-/-/R/-

Scale =.5"/Ft.

TC LL	20.0 PSF	REF	R487-- 16802
TC DL	10.0 PSF	DATE	12/12/11
BC DL	10.0 PSF	DRW	HCUSR487 11346008
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	252834
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UHT487_Z01

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - AP)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

110 mph wind, 19.71 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18

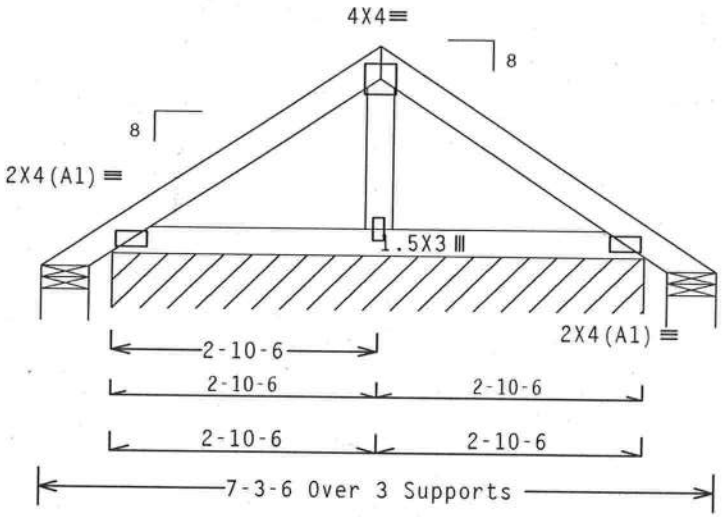
Wind loads and reactions based on MWFRS with additional C&C member design.

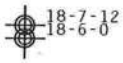
Refer to DWG PB1200310 for piggyback details.

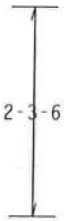
Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 64 plf at 0.00 to 64 plf at 3.64
TC- From 64 plf at 3.64 to 64 plf at 7.28
BC- From 4 plf at 0.00 to 4 plf at 7.28

Deflection meets L/240 live and L/180 total load.

MWFRS loads based on trusses located at least 19.71 ft. from roof edge.







R=-11 Rw=41 U=40 W=6.31"
RL=53/R584 PLF U=7 PLF W=5-8-12

R=-11 Rw=11 U=10 W=5.31"

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.03.04 0603 16

QTY: 3

FL/-/4/-/-/R/-

Scale =.5"/Ft.

ALPINE
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Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

TC LL	20.0 PSF	REF R487-- 16803
TC DL	10.0 PSF	DATE 12/12/11
BC DL	10.0 PSF	DRW HCUSR487 11346046
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 253346
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - AP1)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

110 mph wind, 19.66 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00
GCpi(+/-)=0.18

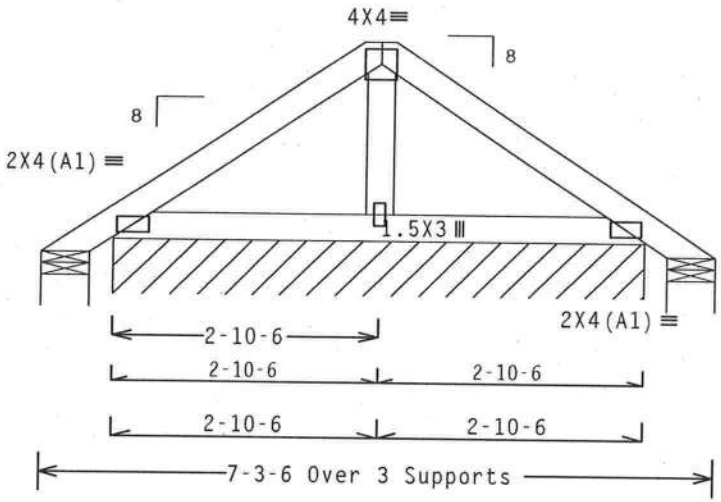
Wind loads and reactions based on MWFRS with additional C&C member design.

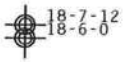
Refer to DWG PB1200310 for piggyback details.

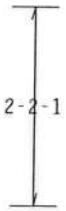
Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 64 plf at 0.00 to 64 plf at 3.64
TC- From 64 plf at 3.64 to 64 plf at 7.28
BC- From 4 plf at 0.00 to 4 plf at 7.28

Deflection meets L/240 live and L/180 total load.

MWFRS loads based on trusses located at least 19.66 ft. from roof edge.







R=-11 Rw=40 U=39 W=6.31"
RL=52/R584 PLF U=7 PLF W=5-8-12

R=-11 Rw=11 U=10 W=4.91

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

10.03.04.0601.15 QTY 2 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

TC LL	20.0 PSF	REF	R487-- 16804
TC DL	10.0 PSF	DATE	12/12/11
BC DL	10.0 PSF	DRW	HCUSR487 11346045
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	252728
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - AP2)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 18.99 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Refer to DWG PB1200310 for piggyback details.

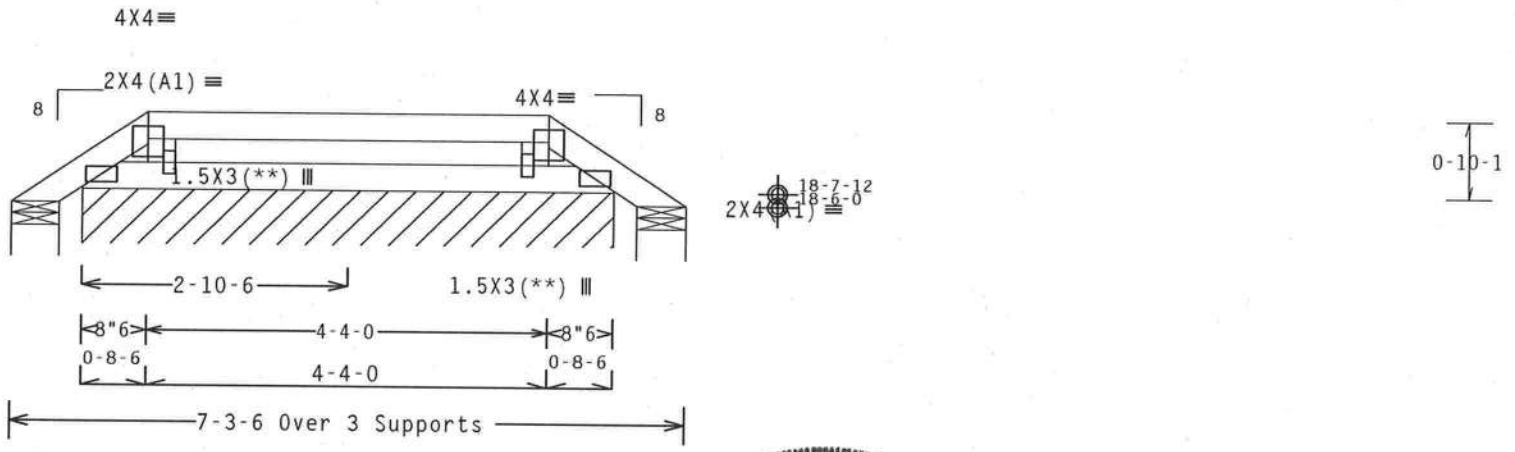
Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC- From	64 plf at	0.00 to	64 plf at	1.47
TC- From	64 plf at	1.47 to	64 plf at	5.81
TC- From	64 plf at	5.81 to	64 plf at	7.28
BC- From	4 plf at	0.00 to	4 plf at	7.28

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

MWFRS loads based on trusses located at least 18.99 ft. from roof edge.



Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.00.04.0601.16 QTY:

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

FL/-/4/-/-/R/-

Scale =.5"/Ft.

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF R487-- 16805
DATE 12/12/11
DRW HCUSR487 11346024
HC-ENG DF/DF
SEQN- 252733
JREF- 1UHT487_Z01

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - TP)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

110 mph wind, 19.10 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf, Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

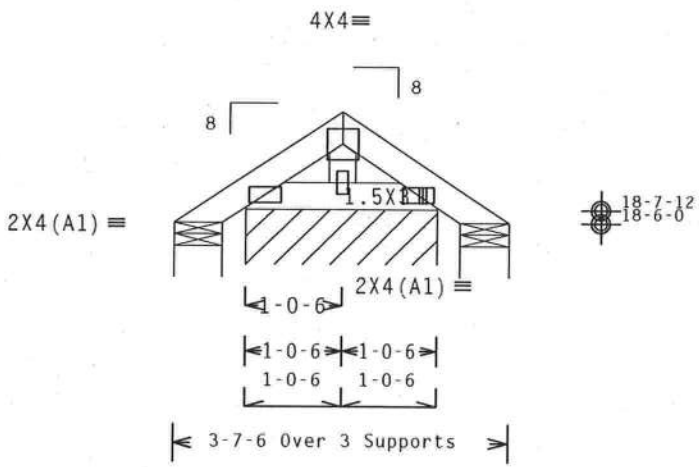
MWFRS loads based on trusses located at least 9.55 ft. from roof edge.

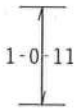
Refer to DWG PB1200310 for piggyback details.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 64 plf at 0.00 to 64 plf at 1.81
TC- From 64 plf at 1.81 to 64 plf at 3.61
BC- From 4 plf at 0.00 to 4 plf at 3.61

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Deflection meets L/240 live and L/180 total load.





R=19 U=10 W=6.31" R=19 U=7 W=6.31"
RL=24/R=283 PLF U=15 PLF W=2-0-12

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.03.04.0601.16 QTY:

FL/-/4/-/-/R/-

Scale =.5"/Ft.

TC LL	20.0 PSF	REF	R487-- 16806
TC DL	10.0 PSF	DATE	12/12/11
BC DL	10.0 PSF	DRW	HCUSR487 11346043
BC LL	0.0 PSF		HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN-	252961
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UHT487_Z01

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - T2)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.32" due to live load and 0.34" due to dead load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

Laterally brace chord member above filler @ 24" O.C. or as specified, including a lateral brace at chord ends.

(++) - This plate works for both joints covered.

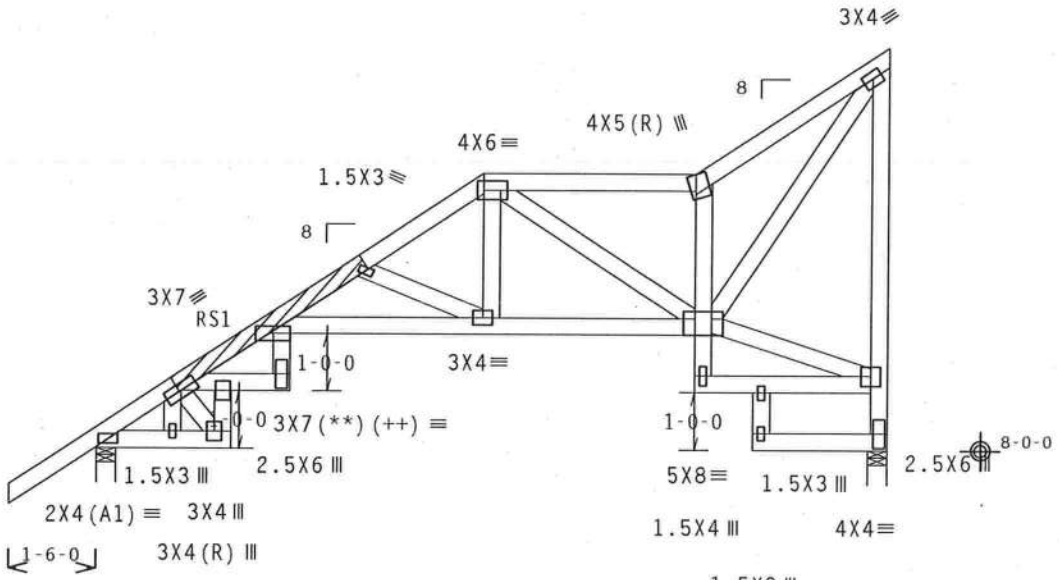
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

RS1 (1) 2x4X3-10-10 SP #1 Top chord scab centered 3-0-11 from left end. Attach to one face of chord with (2) rows of 0.131"x3" nails @ 6" O.C., staggered 3".



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.04.0601.16 QTY 1 FL/-/4/-/-/R/-

Scale =.3125"/Ft.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 16807
TC DL	10.0 PSF	DATE	12/12/11
BC DL	10.0 PSF	DRW	HCUSR487 11346040
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	252927
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - T4)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3 :W5 2x4 SP #2:

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.12" due to live load and 0.13" due to dead load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

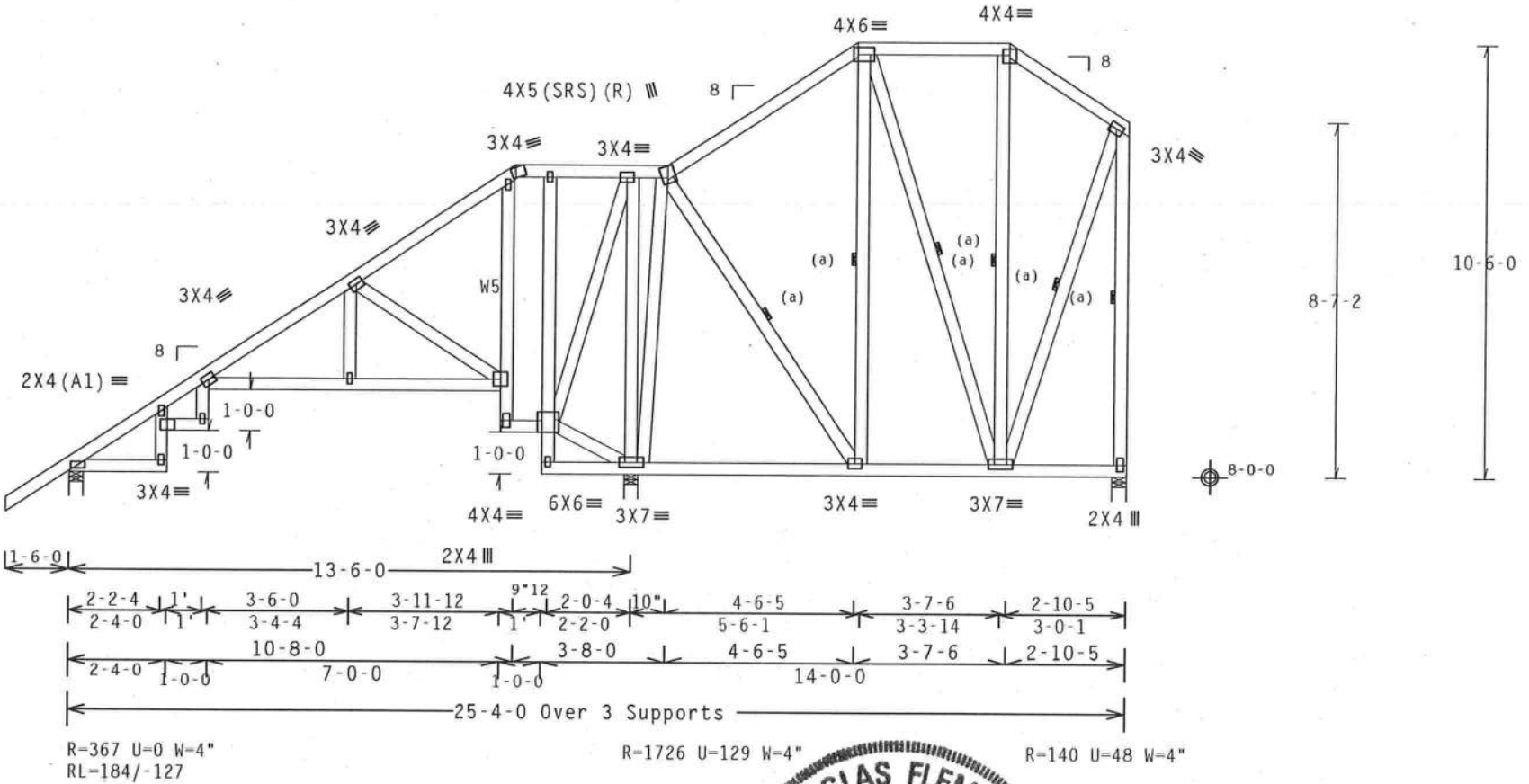
Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



R=367 U=0 W=4"
RL=184/-127

R=1726 U=129 W=4"

R=140 U=48 W=4"

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.04.0601.16

QTY 1

FL/-/4/-/-/R/-

Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

TC LL	20.0 PSF	REF	R487-- 16808
TC DL	10.0 PSF	DATE	12/12/11
BC DL	10.0 PSF	DRW	HCUSR487 11346036
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	252935
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - T5)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3 :W1 2x4 SP #2:

Roof overhang supports 2.00 psf soffit load.

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

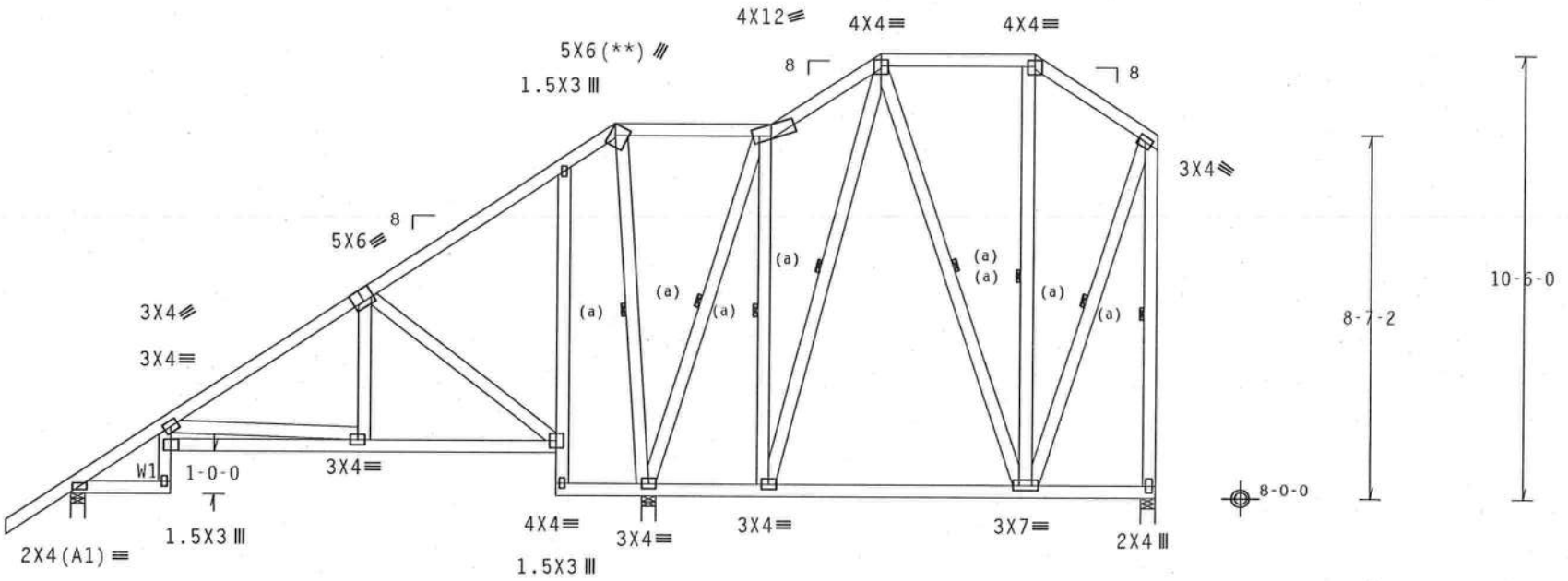
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.04.0601.16

QTY 1

FL/-/4/-/-/R/-

Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

TC LL	20.0 PSF	REF R487-- 16809
TC DL	10.0 PSF	DATE 12/12/11
BC DL	10.0 PSF	DRW HCUSR487 11346034
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 252951
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - T6)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

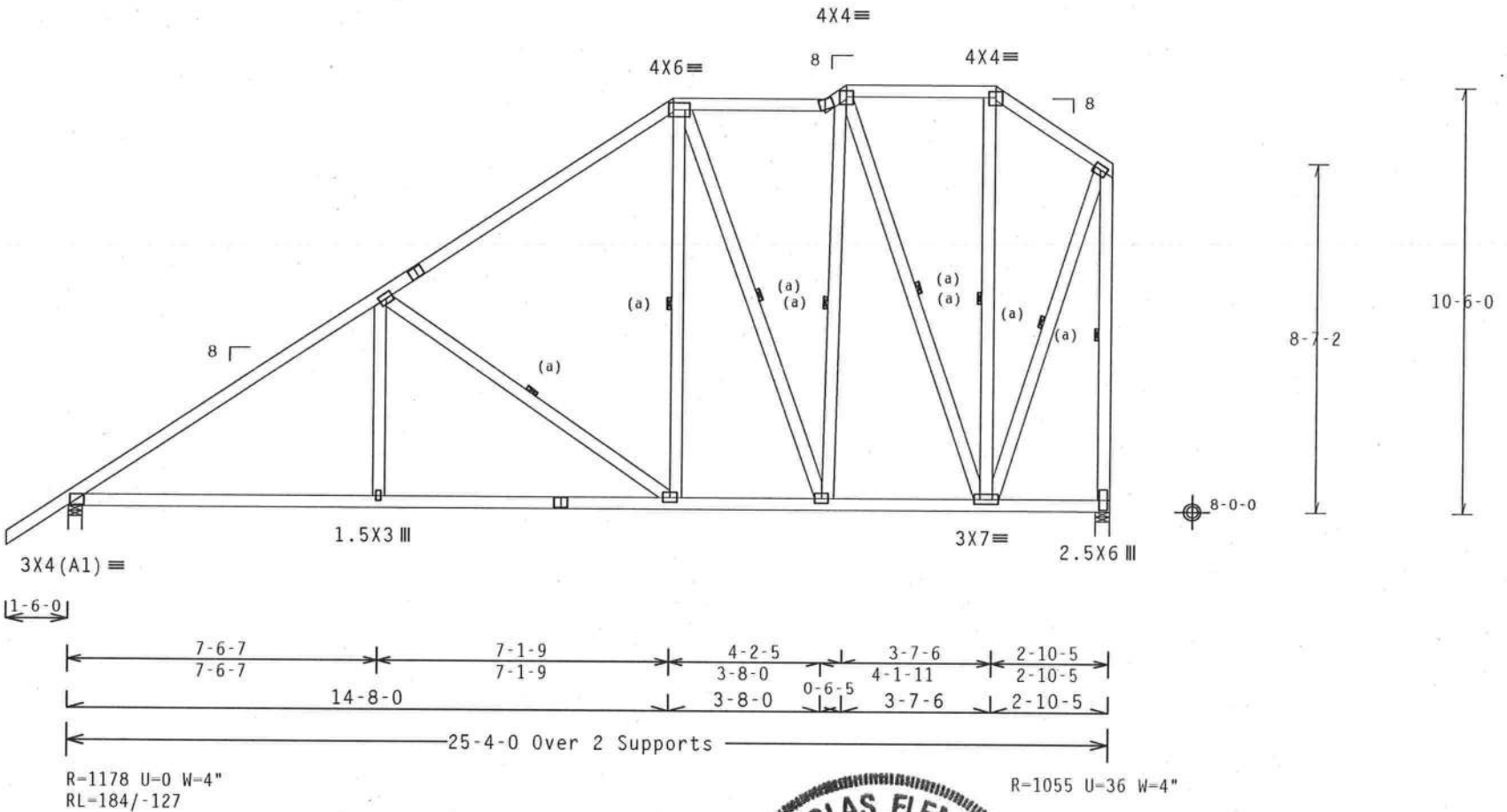
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.03.04.0601.16 QTY 1 FL/-/4/-/-/R/- Scale =.25"/Ft.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF R487-- 16810
TC DL	10.0 PSF	DATE 12/12/11
BC DL	10.0 PSF	DRW HCUSR487 11346039
BC LL	0.0 PSF	HC-ENG DF/DF *
TOT.LD.	40.0 PSF	SEQN- 252958
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - T1)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 64 plf at -1.50 to 64 plf at 4.67
TC- From 64 plf at 4.67 to 64 plf at 8.33
TC- From 64 plf at 8.33 to 64 plf at 13.67
TC- From 96 plf at 13.67 to 96 plf at 17.66
TC- From 96 plf at 17.66 to 96 plf at 23.67
TC- From 96 plf at 23.67 to 96 plf at 25.33
BC- From 5 plf at -1.50 to 5 plf at 0.00
BC- From 20 plf at 0.00 to 20 plf at 2.33
BC- From 20 plf at 2.33 to 20 plf at 3.33
BC- From 20 plf at 3.33 to 20 plf at 11.33
BC- From 20 plf at 11.33 to 20 plf at 13.67
BC- From 30 plf at 13.67 to 30 plf at 25.33
BC- 294.12 lb Conc. Load at 3.06
BC- 444.49 lb Conc. Load at 3.54

Wind loads and reactions based on MWFRS with additional C&C member design.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCp1(+/-)=0.18

Right end vertical not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

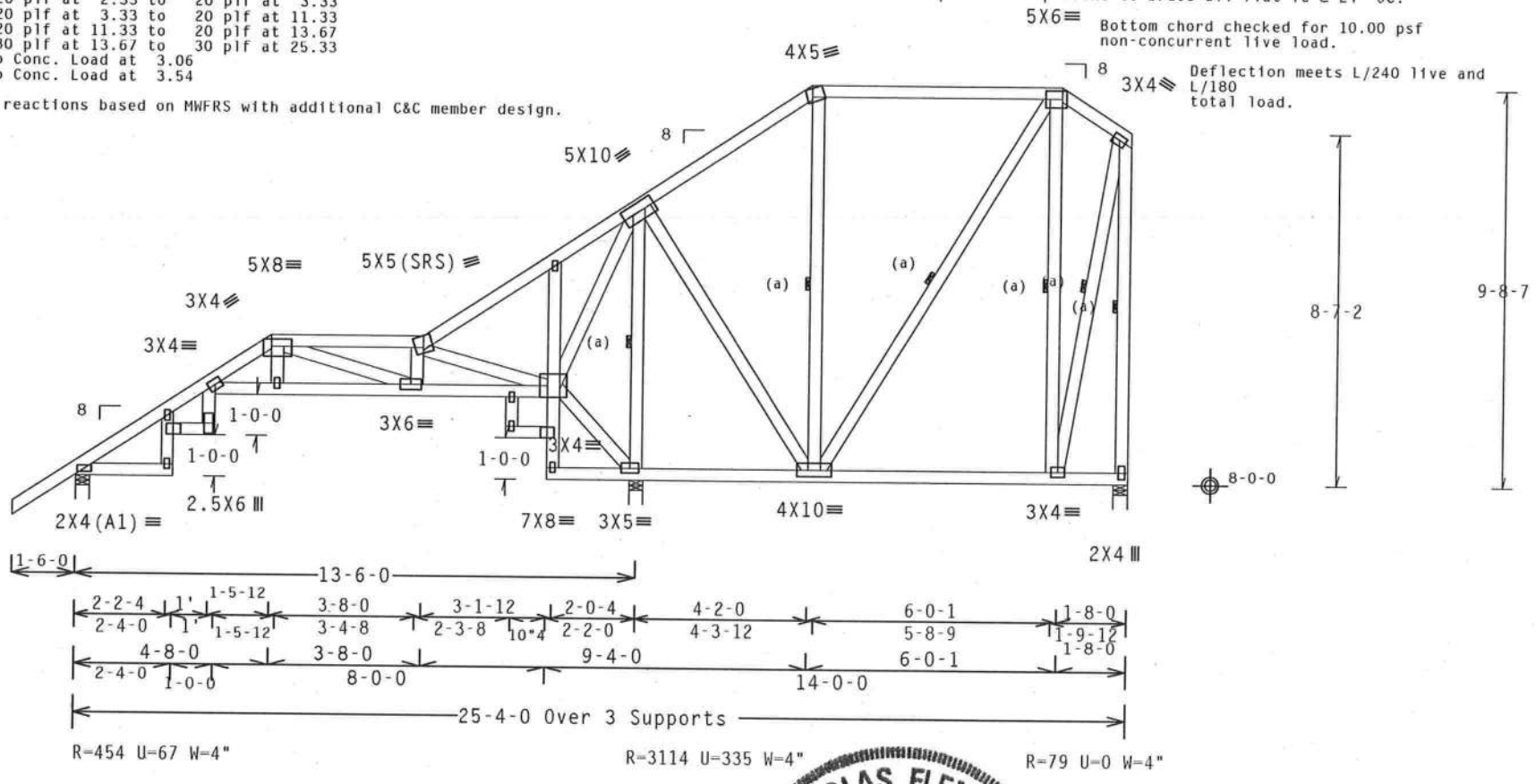
Calculated horizontal deflection is 0.14" due to live load and 0.15" due to dead load.

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.



Note: All Plates Are 1.5X3 Except As Shown.

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

10.03.04.0601.16 QTY 1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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12/12/2011

TC LL	20.0 PSF	REF	R487-- 16811
TC DL	10.0 PSF	DATE	12/12/11
BC DL	10.0 PSF	DRW	HCUSR487 11346020
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	253026
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UHT487_Z01

(11-225--Fill in later STANLEY CRAWFORD/SCCI 262 -- , ** - T3)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3 :W6, W9 2x4 SP #2:

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC- From	64 plf at	-1.50 to	64 plf at	8.67
TC- From	64 plf at	8.67 to	64 plf at	12.33
TC- From	64 plf at	12.33 to	64 plf at	13.67
TC- From	96 plf at	13.67 to	96 plf at	18.86
TC- From	96 plf at	18.86 to	96 plf at	22.47
TC- From	96 plf at	22.47 to	96 plf at	25.33
BC- From	5 plf at	-1.50 to	5 plf at	0.00
BC- From	20 plf at	0.00 to	20 plf at	2.33
BC- From	20 plf at	2.33 to	20 plf at	3.33
BC- From	20 plf at	3.33 to	20 plf at	13.37
BC- From	20 plf at	13.37 to	20 plf at	13.67
BC- From	30 plf at	13.67 to	30 plf at	17.33
BC- From	30 plf at	17.33 to	30 plf at	25.17
BC- From	20 plf at	25.17 to	20 plf at	25.33

Wind loads and reactions based on MWFRS with additional C&C member design.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

Laterally brace chord member above filler @ 24" O.C. or as specified, including a lateral brace at chord ends.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Right end vertical not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

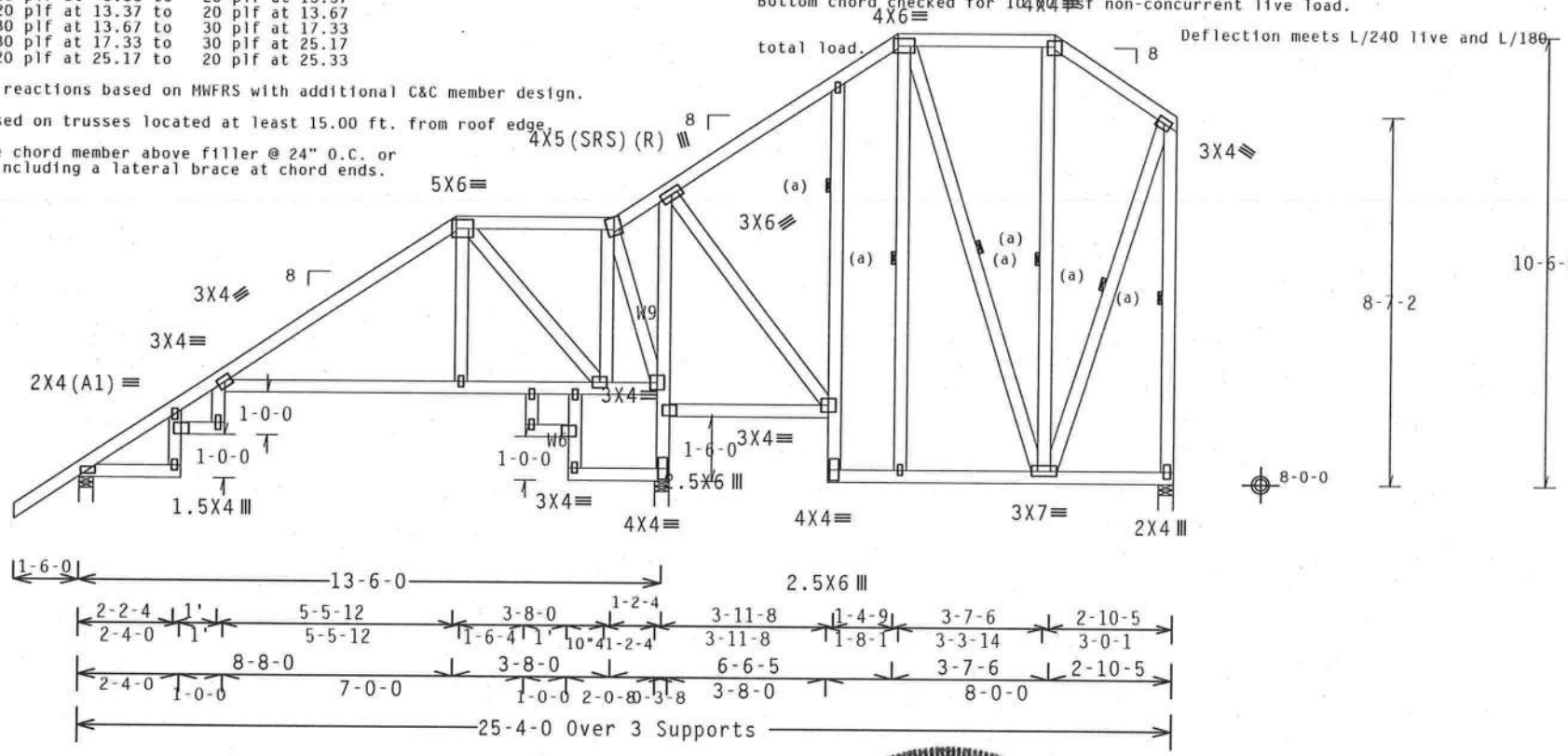
Calculated horizontal deflection is 0.19" due to live load and 0.20" due to dead load.

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.



R=371 U=40 W=4"
RL=184/-127

R=1961 U=211 W=4"

R=388 U=42 W=4"

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.04.0603.16

QTY:

FL/-/4/-/-/R/-

Scale =.25"/Ft.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

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TC LL	20.0 PSF	REF R487-- 16812
TC DL	10.0 PSF	DATE 12/12/11
BC DL	10.0 PSF	DRW HCUSR487 11346023
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 253059
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UHT487_Z01

GABLE STUD REINFORCEMENT DETAIL

ASCE 7-05: 110 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C, Kzt = 1.00

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 'L' BRACE * GROUP A	(1) 1X4 'L' BRACE * GROUP B	(1) 2X4 'L' BRACE * GROUP A	(1) 2X4 'L' BRACE * GROUP B	(2) 2X4 'L' BRACE ** GROUP A	(2) 2X4 'L' BRACE ** GROUP B	(1) 2X6 'L' BRACE * GROUP A	(1) 2X6 'L' BRACE * GROUP B	(2) 2X6 'L' BRACE ** GROUP A	(2) 2X6 'L' BRACE ** GROUP B
24" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 3"	12' 3"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	5' 2"	6' 9"	6' 9"	9' 1"	9' 1"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 0"	9' 5"	9' 11"	12' 5"	12' 6"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
16" O.C.	SPF HF	#1 / #2	4' 5"	7' 8"	7' 10"	9' 1"	9' 4"	10' 10"	11' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 4"	6' 4"	6' 4"	8' 4"	8' 4"	10' 10"	10' 10"	12' 11"	12' 11"	14' 0"	14' 0"
	SP DFL	#1	4' 10"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 9"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 6"	7' 7"	7' 7"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 6"	7' 6"	7' 6"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 5"	6' 5"	6' 5"	8' 6"	8' 6"	10' 10"	11' 1"	13' 3"	13' 3"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 11"	8' 5"	8' 8"	10' 0"	10' 3"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 9"	7' 3"	7' 3"	9' 7"	9' 7"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 4"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 3"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 0"	8' 5"	8' 5"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	5' 0"	8' 5"	8' 7"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 11"	7' 5"	7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

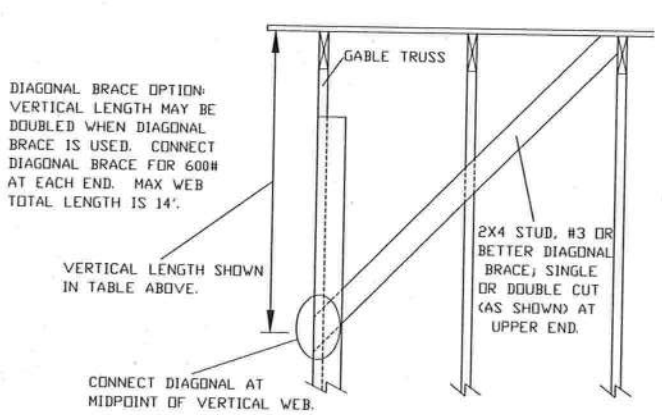
SOUTHERN PINE
#3
STUD
STANDARD

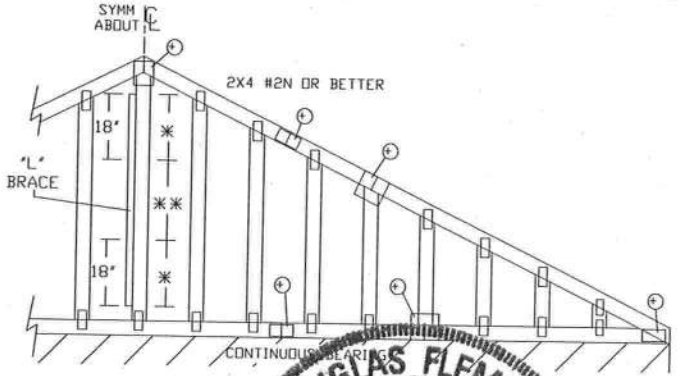
GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#1
#2

DOUGLAS FIR-LARCH
#1
#2





REFER TO CHART ABOVE FOR MAX. GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH 'L' BRACE WITH 10d NAILS. (0.128"x3" min

* FOR (1) 'L' BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) 'L' BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

'L' BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2.5X4
GREATER THAN 11' 6"	3X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

TW Building Components Group Inc.

Earth City, MO 63045

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ITW-BCG: www.itwbcg.com; TPI: www.tpinst.com; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

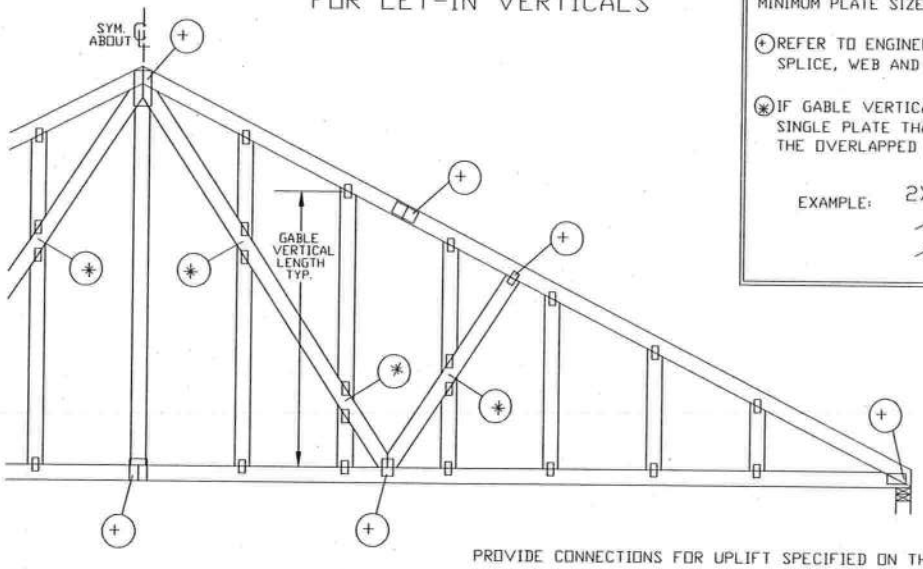
DOC 12/12/2011

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-05-GAB11015
DATE	1/1/09
DRWG	A11015050109

GABLE DETAIL
FOR LET-IN VERTICALS



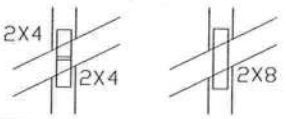
GABLE TRUSS PLATE SIZES

REFER TO APPROPRIATE ITW GABLE DETAIL FOR MINIMUM PLATE SIZES FOR VERTICAL STUDS.

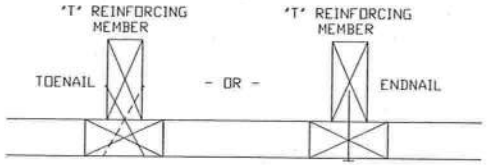
(+) REFER TO ENGINEERED TRUSS DESIGN FOR PEAK, SPLICE, WEB AND HEEL PLATES.

(*) IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE THAT COVERS THE TOTAL AREA OF THE OVERLAPPED PLATES TO SPAN THE WEB.

EXAMPLE:



'T' REINFORCEMENT ATTACHMENT DETAIL



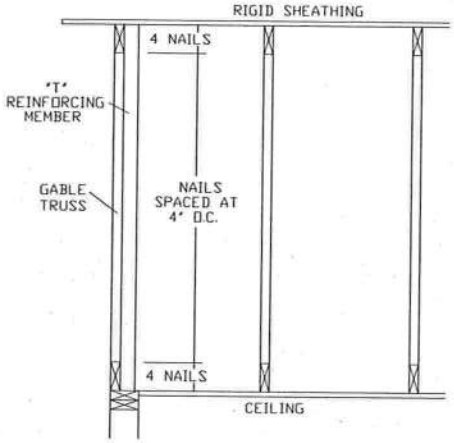
TO CONVERT FROM 'L' TO 'T' REINFORCING MEMBERS, MULTIPLY 'T' INCREASE BY LENGTH (BASED ON APPROPRIATE ITW GABLE DETAIL).

MAXIMUM ALLOWABLE 'T' REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WEB LENGTH INCREASE W/ "T" BRACE

WIND SPEED AND MRH	'T' REINF. MBR. SIZE	'T' INCREASE
140 MPH	2x4	10 %
15 FT	2x6	50 %
140 MPH	2x4	10 %
30 FT	2x6	50 %
130 MPH	2x4	10 %
15 FT	2x6	50 %
130 MPH	2x4	10 %
30 FT	2x6	50 %
120 MPH	2x4	10 %
15 FT	2x6	50 %
120 MPH	2x4	10 %
30 FT	2x6	40 %
110 MPH	2x4	10 %
15 FT	2x6	40 %
110 MPH	2x4	10 %
30 FT	2x6	50 %
100 MPH	2x4	20 %
15 FT	2x6	30 %
100 MPH	2x4	10 %
30 FT	2x6	40 %
90 MPH	2x4	20 %
15 FT	2x6	20 %
90 MPH	2x4	20 %
30 FT	2x6	30 %

EXAMPLE:
ASCE WIND SPEED = 100 MPH
MEAN ROOF HEIGHT = 30 FT, Kzt = 1.00
GABLE VERTICAL = 24" O.C. SP #3
'T' REINFORCING MEMBER SIZE = 2X4
'T' BRACE INCREASE (FROM ABOVE) = 10% = 1.10
(1) 2X4 'L' BRACE LENGTH = 6' 7"
MAXIMUM 'T' REINFORCED GABLE VERTICAL LENGTH
1.10 x 6' 7" = 7' 3"



PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN.

ATTACH EACH 'T' REINFORCING MEMBER WITH
END DRIVEN NAILS:
10d COMMON (0.148"X 3",MIN) NAILS AT 4" O.C. PLUS
(4) NAILS IN TOP AND BOTTOM CHORD.

TOENAILED NAILS:
10d COMMON (0.148"x3",MIN) TOENAILS AT 4" O.C. PLUS
(4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ITW GABLE DETAIL FOR ASCE WIND LOAD.
ASCE 7-98 GABLE DETAIL DRAWINGS
A13015980109, A12015980109, A11015980109, A10015980109,
A13030980109, A12030980109, A11030980109, A10030980109
ASCE 7-02 GABLE DETAIL DRAWINGS
A13015020109, A12015020109, A11015020109, A10015020109, A14015020109,
A13030020109, A12030020109, A11030020109, A10030020109, A14030020109
ASCE 7-05 GABLE DETAIL DRAWINGS
A13015050109, A12015050109, A11015050109, A10015050109, A14015050109,
A13030050109, A12030050109, A11030050109, A10030050109, A14030050109

SEE APPROPRIATE ITW GABLE DETAIL FOR MAXIMUM UNREINFORCED GABLE VERTICAL LENGTH.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

Date: 12/12/2011

ITW Building Components Group Inc.

Earth City, MO 63045

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ITW-BCG: www.itwbcg.com; TPI: www.tpinst.com; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

MAX TOT. LD. 60 PSF
DUR. FAC. ANY
MAX SPACING 24.0"

REF	LET-IN VERT
DATE	1/1/09
DRWG	GBLLETIN0109

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON A TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

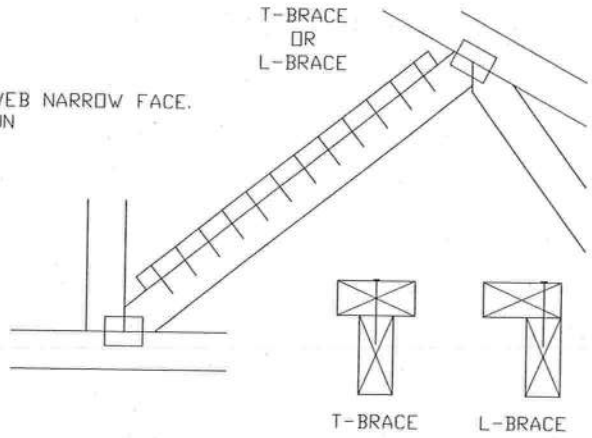
WEB MEMBER SIZE	SPECIFIED CLB BRACING	ALTERNATIVE BRACING T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

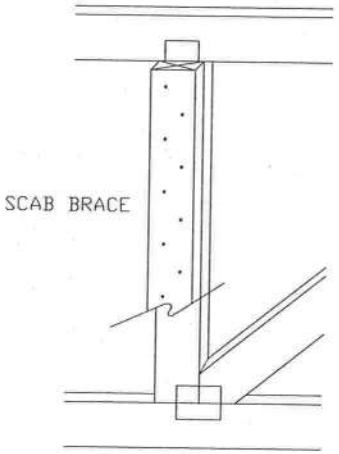
T-BRACING OR L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE.
ATTACH WITH 10d BOX OR GUN (0.128"x 3.",MIN) NAILS.
AT 6" O.C.
BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB.
NO MORE THAN (1) SCAB PER FACE.
ATTACH WITH 10d BOX OR GUN (0.128"x 3.",MIN) NAILS.
AT 6" O.C.
BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



ITW Building Components Group Inc.

Earth City, MO 63045

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

DATE: 12/12/2011

TC LL	PSF	REF CLB SUBST.
TC DL	PSF	DATE 1/1/09
BC DL	PSF	DRWG BRCLBSUB0109
BC LL	PSF	
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

120 PIGGYBACK DETAIL

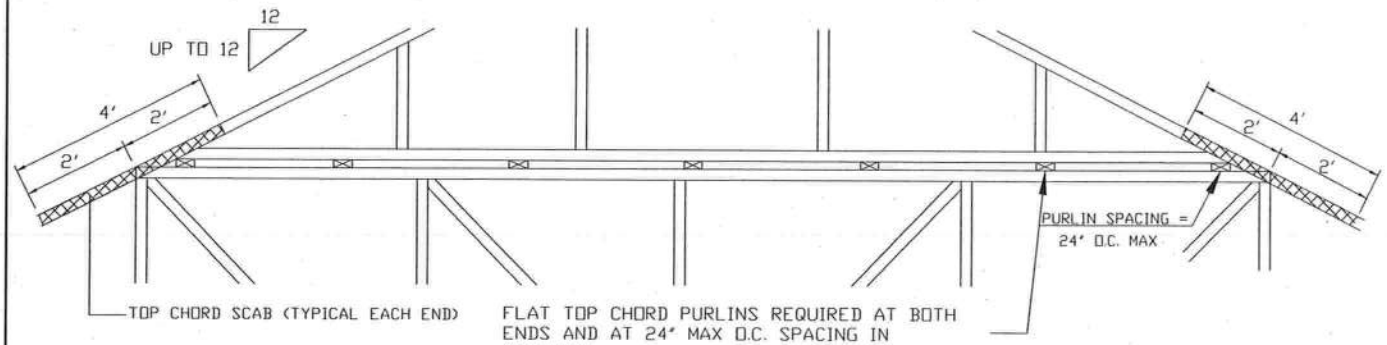
UP TO 120 MPH WIND, 30.00 FT MEAN HGT, ASCE 7-02 OR ASCE 7-05, ENCLOSED BLDG. LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND DL= 5.0 PSF (MIN), Kzt=1.0.

MAXIMUM TRUSS SPACING IS 24" O.C.
DETAIL IS NOT APPLICABLE IF CAP SUPPORTS ADDITIONAL LOADS SUCH AS CUPOLA, STEEPLE, CHIMNEY OR DRAG STRUT LOADS.

NOTE: TOP CHORDS OF TRUSSES SUPPORTING PIGGYBACK CAP TRUSSES MUST BE ADEQUATELY BRACED BY SHEATHING OR PURLINS. THE BUILDING ENGINEER OF RECORD SHALL PROVIDE DIAGONAL BRACING OR ANY OTHER SUITABLE ANCHORAGE TO PERMANENTLY RESTRAIN PURLINS, AND LATERAL BRACING FOR OUT OF PLANE LOADS OVER GABLE ENDS.

** REFER TO ENGINEER'S SEALED TRUSS DESIGN DRAWING FOR PIGGYBACK AND BASE TRUSS SPECIFICATIONS.

DETAIL A : PURLIN SPACING = 24" O.C. OR LESS



PIGGYBACK CAP TRUSS SLANT NAILED TO ALL TOP CHORD PURLIN BRACING WITH (2) 16d BOX NAILS (0.135"x3.5") AND SECURE TOP CHORD WITH 2x4 #3 GRADE SCAB (1 SIDE ONLY AT EACH END) ATTACHED WITH 2 ROWS OF 10d BOX NAILS (0.128"x3") AT 4' O.C.

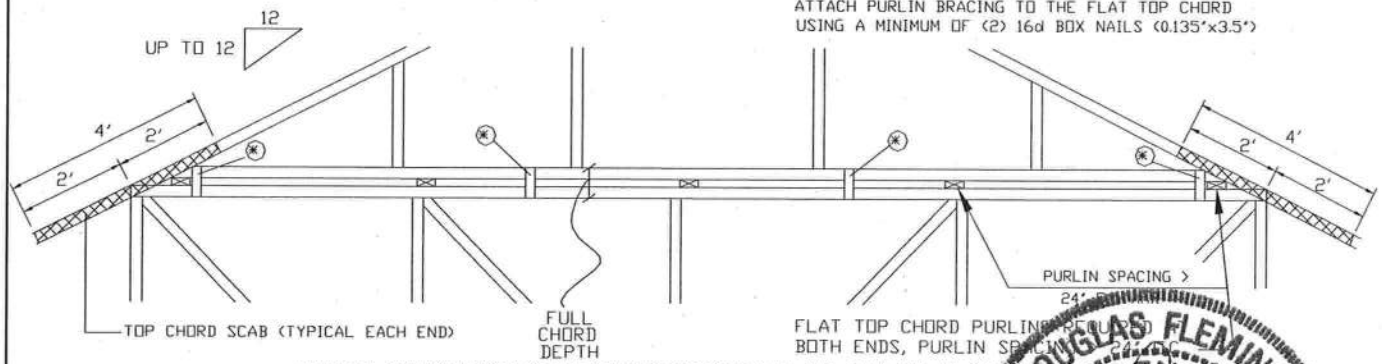
ATTACH PURLIN BRACING TO THE FLAT TOP CHORD USING (2) 16d BOX NAILS (0.135"x3.5")

THE TOP CHORD #3 GRADE 2x4 SCAB MAY BE REPLACED WITH EITHER OF THE FOLLOWING: (1) 3x8 TRULOX PLATE ATTACHED WITH (8) 0.120"x1.375" NAILS, (4) INTO CAP TC & (4) INTO BASE TRUSS TC OR (1) 28PB WAVE PIGGYBACK PLATE PLATED TO THE PIGGYBACK TRUSS TC AND ATTACHED TO THE BASE TRUSS TC WITH (4) 0.120"x1.375" NAILS. NOTE: NAILING THRU HOLES OF WAVE PLATE IS ACCEPTABLE.

DETAIL B : PURLIN SPACING > 24" O.C.

PIGGYBACK CAP TRUSS SLANT NAILED TO ALL TOP CHORD PURLIN BRACING WITH (2) 16d BOX NAILS (0.135"x3.5") AND SECURE TOP CHORD WITH 2x4 #3 GRADE SCAB (1 SIDE ONLY AT EACH END) ATTACHED WITH 2 ROWS OF 10d BOX NAILS (0.128"x3") AT 4' O.C.

ATTACH PURLIN BRACING TO THE FLAT TOP CHORD USING A MINIMUM OF (2) 16d BOX NAILS (0.135"x3.5")



NOTE: IF PURLINS OR SHEATHING ARE NOT SPECIFIED ON THE FLAT TOP OF THE TRUSS, PURLINS MUST BE INSTALLED AT 24" O.C. MAX. AND USE DETAIL A.

* IN ADDITION, PROVIDE CONNECTION WITH ONE OF THE FOLLOWING METHODS:

TRULOX
USE 3x8 TRULOX PLATES FOR 2x4 CHORD MEMBER, AND 3x10 TRULOX PLATES FOR 2x6 AND LARGER CHORD MEMBERS. ATTACH TO EACH FACE @ 8' O.C. WITH (4) 0.120"x1.375" NAILS INTO CAP BOTTOM CHORD AND (4) IN BASE TRUSS TOP CHORD. TRULOX PLATES MAY BE STAGGERED 4' O.C. FRONT TO BACK FACES.

APA RATED GUSSET
8"x8"x7/16" (MIN) APA RATED SHEATHING GUSSETS (EACH FACE). ATTACH @ 8' O.C. WITH (8) 6d COMMON (0.113"x2") NAILS PER GUSSET, (4) IN CAP BOTTOM CHORD AND (4) IN BASE TRUSS TOP CHORD. GUSSETS MAY BE STAGGERED 4' O.C. FRONT TO BACK FACES.

2x4 VERTICAL SCABS
2x4 SPF#2, FULL CHORD DEPTH SCABS (EACH FACE). ATTACH @ 8' O.C. WITH (6) 10d BOX NAILS (0.128"x3") PER SCAB, (3) IN CAP BOTTOM CHORD AND (3) IN BASE TRUSS TOP CHORD. SCABS MAY BE STAGGERED 4' O.C. FRONT TO BACK FACE

28PB WAVE PIGGYBACK PLATE
ONE 28PB WAVE PIGGYBACK PLATE TO EACH FACE @ 8' O.C. ATTACH TEETH TO PIGGYBACK AT TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120"x1.375" NAILS PER FACE PER PLY. PIGGYBACK PLATES MAY BE STAGGERED 4' O.C. FRONT TO BACK FACES.

ITW
Building Components Group Inc.
Earth City, MO 63045

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow BCSI (Building Component Safety Information, by TPI and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural panels and botton chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3 & B7. See this job's general notes page for more information.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with TPI, or fabricating, handling, shipping, installing & bracing of trusses. ITWBCG connector plates are made of 20/18/16GA (W,H/S/K) ASTM A653 grade 37/40/60 (K/W/H,S) galv. steel. Apply plates to each face of truss, positioned as shown on this and on Joint Details.
A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2.
ITW-BCG: www.itwbcg.com; TPI: www.tpinst.com; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/12/2011

SPACING 24.0"

REF	PIGGYBACK
DATE	03/15/10
DRWG	PB1200310

Rec. 18.50
Doc. 2,625.00

THIS INSTRUMENT WAS PREPARED BY:

TERRY McDAVID
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328

RETURN TO:

TERRY McDAVID
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328

File No. 08-243

Property Appraiser's
Parcel Identification Nos.
03124-101; 03124-102;
03124-104; 03124-105;
03124-106; 03124-107;
03124-108; 03124-110;
03124-111; 03124-112;

Inst:200812015665 Date:8/25/2008 Time:8:57 AM
Doc Stamp-Deed:2625.00
DC,P.DeWitt Cason,Columbia County Page 1 of 2 B:1157 P:149

WARRANTY DEED

THIS INDENTURE, made this 22nd day of August 2008, BETWEEN FRONTIER CAPITAL, L.L.C., a Florida Limited Liability Company, whose post office address is Post Office Box 3566, Lake City, Florida 32056, of the County of Columbia, State of Florida, grantor*, and CASON CONSTRUCTION & DEVELOPMENT, INC., a Florida Corporation, whose post office address is 2910 SW CR 242, Lake City, Florida 32024, of the County of Columbia, State of Florida, grantee*.

WITNESSETH: that said grantor, for and in consideration of the sum of Ten Dollars ($10.00), and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in Columbia County, Florida, to-wit:

> Lots 1, 2, 4, 5, 6, 7, 8, 10, 11 and 12, HICKORY COVE, a subdivision according to the plat thereof as recorded in Plat Book 9, Pages 12-14 of the public records of Columbia County, Florida.
>
> SUBJECT TO: Restrictions, easements and outstanding mineral rights of record, if any, and taxes for the current year.

and said grantor does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons whomsoever.

*"Grantor" and "grantee" are used for singular or plural, as context requires.

IN WITNESS WHEREOF, grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered in the presence of:

First Witness

Terry McDavid
(Printed Name)

Second Witness

Myrtle Ann McElroy
(Printed Name)

FRONTIER CAPITAL, L.L.C., a Florida Limited Liability Company

By: _____
Charles S. Sparks
Managing Member

By: _____
Isaac Bratkovich
Managing Member

STATE OF FLORIDA
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 22nd day of August 2008, by CHARLES S. SPARKS and ISAAC BRATKOVICH, Managing Members of FRONTIER CAPITAL, L.L.C., a Florida Limited Liability Company, on behalf of the company. They are personally known to me and did not take an oath.

Notary Public
My commission expires: _____

TERRY MCDAVID
MY COMMISSION # [illegible]
EXPIRES: January [illegible]
Bonded Thru Notary Public [illegible]

Columbia County Property Appraiser

DB Last Updated: 11/15/2011

2011 Tax Year

Tax Collector | Tax Estimator | Property Card | Parcel List Generator | Interactive GIS Map | Print

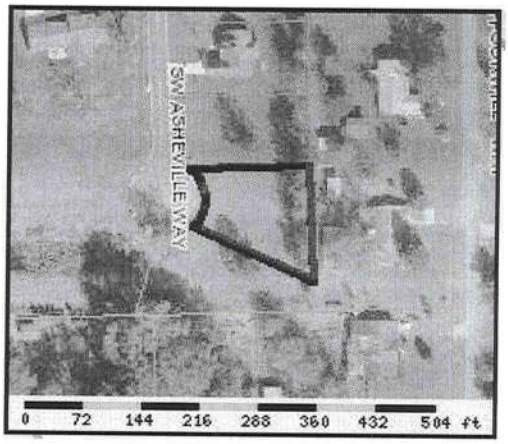
Parcel: 25-4S-16-03124-105

<< Next Lower Parcel | Next Higher Parcel >>

Search Result: 1 of 1

Owner & Property Info

Owner's Name	CASON CONSTRUCTION &		
Mailing Address	DEVELOPMENT INC 2910 SW CR 242 LAKE CITY, FL 32024		
Site Address	187 SW ASHEVILLE WAY		
Use Desc. (code)	VACANT (000000)		
Tax District	2 (County)	**Neighborhood**	25416
Land Area	0.340 ACRES	**Market Area**	06
Description	NOTE: This description is not to be used as the Legal Description for this parcel in any legal transaction.		
LOT 5 HICKORY COVE. WD 1157-149			



Property & Assessment Values

2011 Certified Values		
Mkt Land Value	cnt: (0)	$13,770.00
Ag Land Value	cnt: (1)	$0.00
Building Value	cnt: (0)	$0.00
XFOB Value	cnt: (0)	$0.00
Total Appraised Value		$13,770.00
Just Value		$13,770.00
Class Value		$0.00
Assessed Value		$13,770.00
Exempt Value		$0.00
Total Taxable Value		Cnty: $13,770 Other: $13,770 | Schl: $13,770

2012 Working Values

NOTE:
2012 Working Values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.

Show Working Values

Sales History

Show Similar Sales within 1/2 mile

Sale Date	OR Book/Page	OR Code	Vacant / Improved	Qualified Sale	Sale RCode	Sale Price
8/22/2008	1157/149	WD	V	Q		$375,000.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000000	VAC RES (MKT)	1 LT - (0000000.340AC)	1.00/1.00/1.00/1.00	$11,016.00	$11,016.00

Columbia County Property Appraiser

DB Last Updated: 11/15/2011

STATE OF FLORIDA
COUNTY OF Columbia

TAX NO: 35-4S-16-03194-105

This instrument was Prepared By:
Stanley Crawford Construction, Inc.
1482 S.W. Commercial Glen
Lake City, Florida 32025

NOTICE OF COMMENCEMENT

The undersigned hereby gives notice that improvement will be made to certain real Property, and in accordance with Chapter 713, Florida Statutes, the following information Is provided in this Notice of Commencement.

1. Description of property: Lot 5 Hickory Cove
   187 SW Asheville Way Lake City, FL 32024
2. General description of improvement: Single Family
3. Owner Name & Address: Cason Construction & Development, Inc.
   178 SW Asheville Way Lake City, FL 32024
4. Interest in property: Fee Simple
5. Name and address of fee simple title holder (if other than owner): NONE
6. Contractor: Stanley Crawford Construction, Inc
   1482 SW Commercial Glen
   Lake City, Florida 32025
7. Surety N/A
   a. Name and address:
   b. Amount of bond:

Inst.201112019489 Date:12/20/2011 Time:8:50 AM
DC P.DeWitt Cason,Columbia County Page 1 of 1 B:1226 P:1968

8. Lender: N/A
9. Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13 (1) (a) 7. Florida Statutes: NONE
10. In addition to himself, Owner designates _____ _____ to receive a copy of the Lienor's Notice as provided in section 713.13 (1) (b), Florida Statutes.
11. Expiration date of notice of commencement (the expiration date is 1 year from The date of recording unless a different date is specified).

Matt D Cason

The foregoing instrument was acknowledged before me this 16 day of December, 2011, by MATT D. CASON, who are personally known to me and who did not take an oath.

Erika Cushman
Notary Public
My Commission Expires:

ERIKA CUSHMAN
MY COMMISSION # DD984084
EXPIRES March 26, 2014
(407) 398-0153 FloridaNotaryService.com

CLYATT WELL DRILLING, INC.
(Established in 1971)
Post Office Box 180
Worthington Springs, FL 32697
*Phone (386)496-2488 *** FAX (386)496-4640*

WELL DESCRIPTION

DESCRIPTION DATE
8/3/2011

CUSTOMER NAME AND ADDRESS
Stanley Crawford Construction, Inc. 1482 SW Commercial Glen Lake City, Florida 32025

DESCRIPTION OF WORK
4" Well and Pump

DESCRIPTION
Feet 4" Well
1 HP Submersible Pump (18 GPM)
Feet 1-1/4" Drop Pipe
Feet 14/4 Submersible Pump Wire
4 X 1-1/4 Well Seal
81 Gallon Captive Air Tank (20 Gallon Drawdown)
Pressure Relief Valve
Controls and Fittings
Sales Tax @ 7%

The above description is provided to give a brief description of the water well to be constructed by Clyatt Well Drilling, Inc.

142.34'
146.65'
161.71'
S 65°25'06" E
77'
63'8"
60'
31'4"
44'
61'
septic
112'
39'
Drive
Lot 3
C13
Hickory Cove
Community Water
LOT 6
50' R
CP
1/4
1/4
12/13/2011 11:29 3867199985

Janice Williams

From: Ron Croft
Sent: Wednesday, December 21, 2011 12:58 PM
To: Janice Williams
Subject: RE: FYI ADDRESS VERIFICATION

PARCEL_N	ADDRESS	CITY	STATE	ZIP
25-4S-16-03124-105	187 SW ASHEVILLE WAY	LAKE CITY	FL	32024

Ronal N. Croft
Columbia County 911 Addressing / GIS Department
P.O. Box 1787
Lake City, FL 32056-1787
Phone: 386-758-1125
Fax: 386-758-1365
E-Mail: ron_croft@columbiacountyfla.com

From: Janice Williams
Sent: Wednesday, December 21, 2011 12:44 PM
To: Ron Croft
Subject: FYI ADDRESS VERIFICATION

HEY RON!

PLZ VERIFY 187 SW ASHEVILLE WAY.....R-03124-105

ASAP!

Hi Connie

Columbia County Building Department
Culvert Waiver

Culvert Waiver No. 000001925

DATE: 12/22/2011 BUILDING PERMIT NO. 29843

APPLICANT MARY ANN CRAWFORD PHONE 386-752-5152

ADDRESS 1482 SW COMMERCIAL GLEN LAKE CITY FL 32025

OWNER CASON CONSTRUCTION & DEVELOPMENT INC PHONE 386-623-2806

ADDRESS 187 SW ASHEVILLE WAY LAKE CITY FL 32025

CONTRACTOR STANLEY CRAWFORD PHONE 752-5152

LOCATION OF PROPERTY 47 S, R 242, L ASHEVILLE WAY, LOT IS AT THE END ON THE LEFT

SUBDIVISION/LOT/BLOCK/PHASE/UNIT HICKORY COVE 5

PARCEL ID # 25-4S-16-03124-105

I HEREBY CERTIFY THAT I UNDERSTAND AND WILL FULLY COMPLY WITH THE DECISION OF THE COLUMBIA COUNTY PUBLIC WORKS DEPARTMENT IN CONNECTION WITH THE HEREIN PROPOSED APPLICATION.

SIGNATURE: Mary Ann Crawford

A SEPARATE CHECK IS REQUIRED
MAKE CHECKS PAYABLE TO BCC

Amount Paid 50.00

PUBLIC WORKS DEPARTMENT USE ONLY

I HEREBY CERTIFY THAT I HAVE EXAMINED THIS APPLICATION AND DETERMINED THAT THE CULVERT WAIVER IS:

✓ APPROVED _____ NOT APPROVED - NEEDS A CULVERT PERMIT

COMMENTS: _____

SIGNED: James Dunn DATE: 12-27-11

ANY QUESTIONS PLEASE CONTACT THE PUBLIC WORKS DEPARTMENT AT 386-752-5955.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

COLUMBIA COUNTY GATEWAY TO FLORIDA

Britt Surveying and Mapping, LLC
2086 SW Main Blvd Ste 112
Lake City, FL 32025

03/15/12

Re: Stanley Crawford

To Whom It May Concern:

The finished floor elevation on Lot 5 in Hickory Cove, is found to be at an elevation of 88.52 feet. The lowest adjacent grade is 87.2 feet and the highest adjacent grade is 87.5 feet.

Sincerely,

L. Scott Britt
PLS 5757

#29843

Britt Surveying and Mapping, LLC
2086 SW Main Blvd Ste 112
Lake City, FL 32025

OK
3 April 2012
BLK

03/15/12

Re: Stanley Crawford

To Whom It May Concern:

The finished floor elevation on Lot 5 in Hickory Cove, is found to be at an elevation of 88.52 feet. The lowest adjacent grade is 87.2 feet and the highest adjacent grade is 87.5 feet.

Sincerely,

L. Scott Britt
PLS 5757

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 25-4S-16-03124-105 **Building permit No.** 000029843

Use Classification SFD, UTILITY **Fire:** 46.34

Permit Holder STANLEY CRAWFORD **Waste:** 33.50

Owner of Building CASON CONSTRUCTION & DEVELOPMENT, INC **Total:** 46.34

Location: 187 SW ASHEVILLE WAY, LAKE CITY, FL 32025

Date: 08/17/2012

Troy Crews By: David Lee

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

Notice of Treatment

Applicator: Florida Pest Control & Chemical Co. (www.flapest .com)
Address: 536 SE Boya Dr.
City Lake City Phone 752-1703

Site Location: Subdivision _____
Lot # _____ Block# _____ Permit # 29843
Address 187 SW Asheville Way LC.

Product used	Active Ingredient	% Concentration
☑ Premise	Imidacloprid	0.1%
☐ Termidor	Fipronil	0.12%
☐ _____		

Type treatment: ☑ Soil

Area Treated	Square feet	Linear feet	Gallons Applied
Main body	2320	224	200

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____.

1-13-12	11:50	F082 B.H.
Date	Time	Print Technician's Name

Remarks: _____

Applicator - White Permit File - Canary Permit Holder - Pink

7/11 ©