#4 Bottom of footing to be below local frost depth and per local code.

#5 Top of center columns are to be same height as responsibility of others. II, All American Homes, LLC generated informaticis for reference only and should be verified acceptable on a local jurisdictional level by oths.

#2 All horizontal, verticaand elevation dimensions shall have a tolerance one more than 1/4"

#5 Top of center columns are to be same height as for other top of the 2x8 plate, (no more than 1/2" reduction of sill plate thickness). Anchor bolt/nut/washer shall be recessed flush with the top of the 2x8 plate, (no more than 1/2" reduction of sill plate thickness). Anchor bolts must be placed within 1'-0" of plate ends and not over lenstall 2x8x18" plate on center columns.

#6 PLATES

Is all horizontal, verticaand elevation dimensions shall have a tolerance one more than 1/4"

#7 FOUNDATION WALL; Exterior foundation wall footing shall be 24"x8" in regions where wind speed is 110 mph V3s or greater.

#8 ANCHOR ROLTS: 1/2" diameter anchor bolts shall

#9 *Window locations are subject to change #10 STEEL BEAM CALCULATION LO. OADS

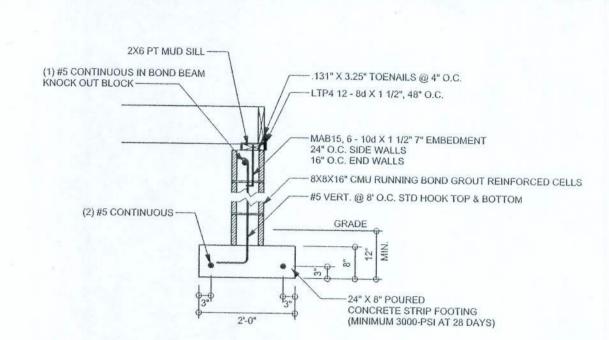
-Center floor beam = 95/54 ptf.

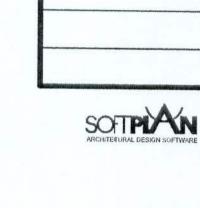
-Front or rear walls = 105/094.5 ptf. #3 All diagonal dimension shall have a tolerance of no more than 1/2" +.

#8 ANCHOR BOLTS: 1/2" diameter anchor bolts shall extend 15" into masonry or 7" into concrete. *Window locations are subject to change in relation to garage and door locations. * FIELD WORK *CRAVL DOOR AND FOUNDATION VENT LOCATION ARE SUBJECT TO CHANGE IN RELATION TO GARAGE AND DOOR LOCATIONS. - 16"x8" VENTS 3'-7 1/8' - 3'-7 1/8' | 3'-8 1/4' i-- i i-- i -- - i -- - i -- - i L __ _ J ─763_{92#} 7632# -3816# ---7632# -7632# ---7632# ---7632# 7632# 7632# 7632# -7632#_ 7632#-APPROVED NOTE: 19' FOOTING WIDTH WITH 1500 PSF 5/10/2006 BEARING SOIL Eric Schreiner 16"x8" CONCRETE FOOTING PFS Corporation (SEE NOTE) ----8" CONCRETE WALL ---Raleigh, NC WBR WBR ALL AMERICAN HOMES, LLC. | FOUNDATION | C 2005 ALL AMERICAN HOMES, LLC. | FOUNDATION PRELIMINARY 11/02/05 1,440 FINISHED SQ. FT. BUILDER SET 04/20/06 WC 2456 3 2 SB EU (B) STD E2 8/10/05 AQG 8 1/4"

application# - 06/043

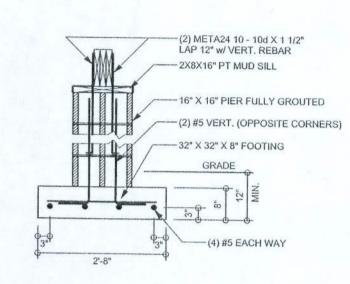
ALL AMERICAN HOMES SN# 56784 PG. 10





REVISIONS







MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approva
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metaties not completely embedded in mortar o grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

GENERAL NOTES:

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" × 6" W1.4 × W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3".

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302, JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, FY = 40 KSI. ALL LAP SPLICES 48 * DB (30" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE

SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO

LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

WINDLOAD ENGIEER: Mark Disosway, PE No.53915, POE868, Lake City, FL 32056, 386-754-549 DIMENSIONS: Stated dimension: supercede scaled

dimensions. Referall questions to
Mark Disosway, PE. for resolution.
Do not proceed whout clarification.

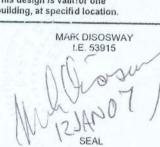
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hese instruments of service. This document

not to be reprodued, altered or copied in any form or manner whout first the express writt permission and cosent of Mark Disosway.

CERTIFICATION: hereby certify that I have examined this foundation plan, and that the applicable portion of the foundation plan, relating to wind enjineering comply with sectic R301.2.1, florida building code residential 204, to the best of my

LIMITATION: This eal is for foundation design only I am not engineer of record for the building This design is valit for one building, at specifid location.



SH Enterprises

Spæ House Elose Street

ADDRESS: Elsise Street ColumbiaCounty, Florida

Tax ID:333S-17-06827-000

Mark Dsosway P.E. P.O Box 868 Lake City Florida 32056 Phone: (386) 754 - 5419

Fax: (385) 269 - 4871

PRIFTED DATE:

Januay 11, 2007

DRAWN BY: STRUCTURAL

DRAWN BY: STRUCTURAL BY David Disosway

FINALS DATE: 11 / Jan / 07

> 7)1111 DRAWNG NUMBER

JOB NUMBER:

S-1
OF: SHEETS