

DESIGN SPECIFICATIONS

DESIGN CODE:
2020 FLORIDA BUILDING CODE – RESIDENTIAL
DESIGN IS VOID ONE YEAR AFTER THE DATE OF THE ORIGINAL PLANS.
UNLESS PLANS HAVE BEEN REVIEWED FOR CODE COMPLIANCE.

DESIGN LOADS: ACTUAL AND UNIFORM
ROOF:
(cd=1.25)
TOP CHORD LIVE LOAD 20 psf
TOP CHORD DEAD LOAD 7 psf (ARCH SHINGLES)
BOTTOM CHORD LIVE LOAD 20 psf (TILE SHINGLES)
BOTTOM CHORD DEAD LOAD 5 psf
FLOOR:
(cd=1.00)
40 psf
10 psf
0 psf
5 psf

DEFLECTION CRITERIA:
ROOF FRAMING: LIVE LOAD L/240 TOTAL LOAD L/180
FLOOR FRAMING: LIVE LOAD L/360 & TOTAL LOAD L/240
0.75" MAX ANY CASE

WIND LOADING:
ASCE 7/16 FOR WIND UPLIFT, TRUSSES SHALL BE DESIGNED WITH A MIN. DEAD LOAD CONDITION OF 5 PSF TOP CHORD AND 5 PSF BOTTOM CHORD. REACTIONS CALCULATED FOR THE BEARING POINTS OF ROOF TRUSSES SHALL BE REDUCED. SPECIFICALLY, ATTIC FLOOR LIVE LOADS COMBINED WITH ROOF LIVE LOADS SHALL BE MULTIPLIED BY 0.75 WHEN COMBINED W/ DEAD LOAD.

BASIC WIND SPEED (ASCE 7-16) ----- **130 MPH**
IMPORTANCE FACTOR ----- 1.00
MEAN ROOF HEIGHT ----- 20.0 FT
ROOF PITCH ----- 7/12
BUILDING CATEGORY ----- II
EXPOSURE CATEGORY ----- C
ENCLOSURE CLASSIFICATION ----- ENCLOSURED
INTERNAL PRESSURE COEFFICIENT ----- ± .18

MATERIAL SPECIFICATIONS

HARDWARE AND ANCHORS:
ANCHOR BOLTS & THREADED ROD: SHALL BE IN ACCORDANCE WITH ASTM A 307 OR ASTM F 1554 GRADE 36.
WASHERS: SHALL BE IN ACCORDANCE WITH ASTM A500 (GRADE B).
NUTS: SHALL BE IN ACCORDANCE WITH ASTM A 563 GRADE A HEX.
METAL CONNECTORS: ALL METAL CONNECTORS WHICH ARE EXPOSED TO EXTERIOR SHALL BE GALVANIZED.
REINFORCING REBAR/ROD INSTALLATION: EMBEDMENT OF RODS OR REBAR COWELS SHALL BE 12 BAR DIAMETER MINIMUM. HOLES SHALL BE 1/4" LARGER THAN REBAR SIX AND 1/2" LARGER THAN THREADED ROD SIZE. (U.O.N.)
ANCHORING ADHESIVE: SHALL BE ONE OF THE FOLLOWING PRODUCTS (DUAL CARTRIDGE INSTALLATION ONLY):
EPOXY: ITW RED HEAD A7
REINFORCING STEEL: SHALL BE ASTM A615, GRADE 60.
STRUCTURAL STEEL: SHALL BE ASTM A992, GRADE 50.
WELDED WIRE FABRIC (WWF): SHALL BE ASTM A185.
LAMINATED VENEER LUMBER (LVL): ALL LAMINATED VENEER LUMBER SHALL MEET OR EXCEED THE FOLLOWING DESIGN PROPERTIES – ELASTIC MODULUS (E): 900ksi, BENDING STRESS (Fb) 2600psi

TRIBUTARY AREA (sf)	COMPONENTS & CLADDING ALLOWABLE DESIGN PRESSURES		GARAGE DOOR PRESSURES (PSF)	
	INTERIOR ZONE (PSF)	EDGE STRIP (PSF)	1 CAR GARAGE DOOR (6'x7')	2 CAR GARAGE DOOR (16'x7')
10	+25.6 –27.7	+25.6 –34.2	+22.9 –28.8	+21.8 –23.9
50	+22.9 –25.0	+22.9 –28.8		
100	+21.8 –23.9	+21.8 –26.6		

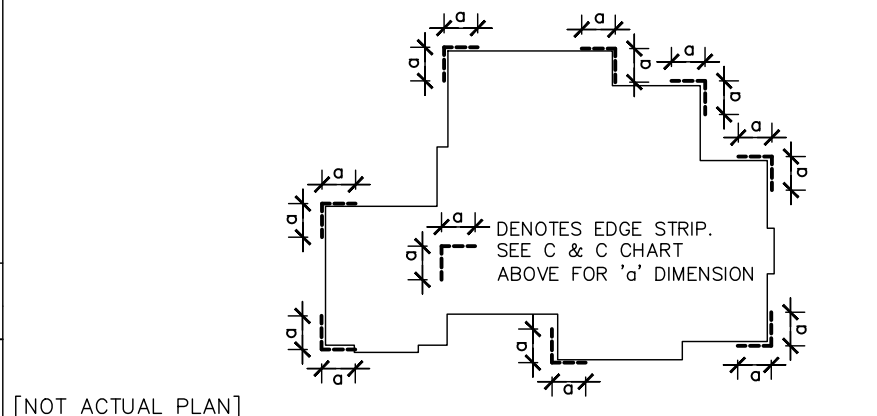
THE VALUES ABOVE ARE ALLOWABLE WIND PRESSURE VALUES (ASD). THE ABOVE WIND PRESSURES HAVE BEEN REDUCE BY 0.60 AS PERMITTED BY THE ALLOWABLE STRESS DESIGN METHODOLOGY. NO FURTHER REDUCTION SHALL BE PERMITTED.

COMPONENT & CLADDING WALL ELEMENTS SHALL BE DESIGNED FOR BOTH POSITIVE AND NEGATIVE PRESSURES SHOWN IN TABLE ABOVE.

LINEAR INTERPOLATION IS PERMISSIBLE.

PLUS = PRESSURE AND MINUS = SUCTION.

DESIGN OF WINDOWS/DOORS FASTENING TO THE WALL FRAMING IS THE RESPONSIBILITY OF THE WINDOW/DOOR MANUF./SUPPLIER & SHALL MEET THE ABOVE NOTED POSITIVE AND NEGATIVE PRESSURES.



SCOPE OF SERVICE

MEANS AND METHODS:
THE STRUCTURAL ENGINEER SHALL NOT HAVE CONTROL OR BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, PROCEDURES, OR SEQUENCES; FOR THE ACTS OR OMISSIONS OF THE CONTRACTOR OR ANY OTHER PERSONS PERFORMING THE WORK OR FOR THE FAILURE FOR ANY OF THEM TO CONSTRUCT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

LIMITS OF STRUCTURAL ENGINEERING DESIGN RESPONSIBILITIES:
THE ITEMS SPECIFICALLY DESIGNED BY THE STRUCTURAL ENGINEER ARE LIMITED TO THE FOLLOWING: CONTINUOUS LOAD PATH FOR WIND UPLIFT, WOOD PANEL SHEARWALLS, WALL FRAMING AND REQUIRED SHEATHING AND HEADERS DIRECTLY SUPPORTING ROOF FRAMING. ITEMS NOT DESIGNED PRE-ENGINEERED WOOD FLOOR AND ROOF TRUSSES, FLOOR FRAMING NOT SPECIFICALLY ADDRESSED, TRUSS-TO-TRUSS CONNECTION, AND ANY ARCHITECTURAL, MECHANICAL OR ELECTRICAL SYSTEM.

GENERAL NOTES & CONSTRUCTION SPECIFICATIONS

FLOOR SHEATHING SPECIFICATIONS
23/32" T&G OSB OR PLYWOOD SHEATHING, GLUE AND NAIL WITH 10d COMMON @ 6" O.C. EDGE & FIELD
ROOF SHEATHING SPECIFICATIONS:
SHINGLE – MIN. 15/32", 32/16, APA RATED OSB OR PLYWOOD SHEATHING, NAILED W/ 0.113x2 1/8" RING SHANK NAILS @ 6" O.C. EDGE & 6" O.C. FIELD (AT GABLE ENDS DECREASE EDGE NAIL SPACING TO 4" O.C. WITHIN 4'-0" OF ROOF EDGE).
TILE – MIN. 15/32" 32/16, APA RATED PLYWOOD SHEATHING, NAILED W/ 0.113x2 1/8" RING SHANK @ 6" O.C. EDGE & 6" O.C. FIELD (AT GABLE ENDS DECREASE EDGE NAIL SPACING TO 4" O.C. WITHIN 4'-0" OF ROOF EDGE).
METAL – MIN. 1/2", 24/16, APA RATED PLYWOOD SHEATHING, NAILED W/ 0.113x2 1/8" RING SHANK NAILS @ 6" O.C. EDGE & 6" O.C. FIELD (AT GABLE ENDS DECREASE EDGE NAIL SPACING TO 4" O.C. WITHIN 4'-0" OF ROOF EDGE).
WALL SHEATHING SPECIFICATIONS:
FLEXIBLE FINISH–MIN. 7/16", 24/16, APA RATED OSB OR PLYWOOD SHEATHING, FASTENED W/ 8d @ 6" O.C. EDGE AND 6" O.C. FIELD. SHEATHING SHALL EXTEND FULL HEIGHT FROM BOTTOM PLATE TO UPPER TOP PLATE. FLEXIBLE FINISH WALLS INCLUDE: WOOD, CEMENT, OR VINYL SIDING, HARD PANEL & BRICK. ALL OTHER WALL SHALL BE CONSIDERED BRITTLE FINISH.
STUCCO FINISH–MIN. 7/16", 24/16, APA RATED OSB OR PLYWOOD SHEATHING, FASTENED W/ 8d @ 6" O.C. EDGE AND 6" O.C. FIELD. SHEATHING SHALL ORIENTED WITH THE LONG DIMENSION PERPENDICULAR TO THE STUDS. CONTRACTOR MAY USE 3/8" STRUCTURAL 1 GRADE SHEATHING OR 1 1/2" OSB SHEATHING AND ORIENT THE PANELS VERTICALLY.

MASONRY SPECIFICATIONS:
MASONRY HAS BEEN DESIGNED IN ACCORDANCE WITH ACI 530-05, AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH ACI305.1-05. GROUT SHALL BE IN ACCORDANCE WITH ASTM C476 WITH A MINIMUM OF 28 DAY COMPRESSIVE STRENGTH OF 3000 psi PER ASTM C1019. GROUT SHALL HAVE A MAXIMUM COURSE SIZE 3" PLACED AT AN 8" TO 11" SLUMP. MORTAR SHALL CONFORM TO ASTM C270 AND TYPE M OR S. TYPE N MORTAR MAY BE USED IN BRICK VENEER. CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF ALL FLASHING.

CONCRETE MASONRY UNITS (CMU):
CMU SHALL BE IN ACCORDANCE WITH ASTM C90-75, HOLLOW LOAD-BEARING (CMU), TYPE 1, GRADE N-1, NORMAL WEIGHT, WITH A MINIMUM COMPRESSIVE STRENGTH OF 1900 psi (f'm=1500 psi). GROUT ALL CELLS CONTAINING VERTICAL REINFORCEMENT IN 5'-0" MAXIMUM LIFTS PROVIDE CLEANOUTS PER ACI 530.1-02 IN THE BOTTOM OF COURSE OF MASONRY WHEN THE WALL HEIGHT EXCEEDS 5'-0".

MASONRY STEM WALLS: ALL CONCRETE MASONRY UNITS SHALL BE COMPOSED OF ASTM C90E, E GRADE N-1 HOLLOW CONCRETE MASONRY UNITS WITH TYPE "S" MORTAR. WALL COURSING SHALL BE RUNNING BONDS, STACK BOND SHALL NOT BE USED. GROUT ALL CELLS CONTAINING VERTICAL REINFORCEMENT WITH 3000 PSI PEA ROCK CONCRETE GROUT. SPLICES IN REINFORCING, WHERE PERMITTED, SHALL BE 48 BAR DIAMETERS. ALL EXTERIOR WALLS SHALL BE REINFORCED FULL HEIGHT WITH #4 @ 4'-0" O.C. MAX. AND AT EACH CORNER, WALL AND WALL INTERSECTIONS. PROVIDE CONTINUITY OF REINFORCING AT INTERSECTIONS OF PERPENDICULAR MASONRY ELEMENTS BY INSTALLING CORNER BARS, MINIMUM OF 40 BAR DIAMETERS INTO EACH ELEMENT. AT STEMWALL CONSTRUCTED OF 5 OR MORE COURSES, PROVIDE HORIZONTAL JOINT REINFORCEMENT AT 16" O.C. VERTICALLY, (EVERY OTHER COURSE), AND VERTICAL REINF. SHALL BE INCREASED AS NOTED ON 1/51.0. UNLESS NOTED OTHERWISE, LAP JOINT REINFORCING SHALL BE A MINIMUM OF 6".

CONCRETE SPECIFICATIONS:
ALL CONCRETE HAS BEEN DESIGNED IN ACCORDANCE WITH ACI 318-08, AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH ACI 301. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS
CONCRETE AT GARAGE AND PORCH SLABS SHALL HAVE A COMPRESSIVE STRENGTH OF 3000 PSI.

GENERAL NOTES:
FOOTING AND FOUNDATIONS:
FOOTINGS AND FOUNDATIONS SHALL BE IN ACCORDANCE WITH LOCAL BUILDING CODES. FOOTING HAVE BEEN DESIGNED WITH A SOIL BEARING (DESIGN MAXIMUM) OF 2000 PSF. A SOILS INVESTIGATION REPORT IS RECOMMENDED TO VERIFY SUITABLE SUBSURFACE CONDITIONS. IF THE FOOTING ELEVATIONS SHOWN OCCUR IN A DISTURBED OR UNSTABLE SOIL, THE ENGINEER SHALL BE NOTIFIED. SOIL SHALL BE FREE OF ORGANIC MATERIAL AND COHESIVE (CLAY) SOILS. SOIL COMPACTION AND FILL SHALL BE COMPACTED TO A MIN. OF 95% MODIFIED PROCTOR IN ACCORDANCE WITH ASTM D 1557.

FOUNDATION PLAN ONLY CONVEYS STRUCTURAL INFORMATION, FOR GENERAL FEATURES, CONDUITS, ELECTRICAL EMBEDS, STEP HEIGHTS, ETC., SEE ARCHITECTURAL PLANS. DO NOT SCALE FOOTING DIMENSIONS AND LOCATION FROM THE FOUNDATION PLAN SHOWN ON 1.0. DO NOT DETERMINE FOOTING LOCATION BASED ON EITHER THE ARCHITECTURAL PLAN OR FRAMING PLAN, BUT BY DIMENSIONS PROVIDED ON FOUNDATION PLAN. IF FOOTING SIZE OR LOCATION IS NOT DETERMINED ON PLAN, THEN CONTACT ENGINEER OF RECORD (EOR)

UNLESS OTHERWISE NOTED ON DRAWINGS, MINIMUM CONCRETE COVER FOR REINFORCING SHALL BE 3" IN FOOTINGS AND MESH SHALL BE CENTERED IN SLAB ON GRADE. IN ALL CONTINUOUS FOOTINGS PROVIDE #3 @ 48" O.C. OR ROD CHAIRS. PROVIDE CONTINUITY OF REINFORCING AT INTERSECTIONS OF PERPENDICULAR CONCRETE ELEMENTS BY INSTALLING CORNER BARS, MINIMUM OF 40 BAR DIAMETERS INTO EACH ELEMENT. SPLICES IN REINFORCING, WHERE PERMITTED, SHALL BE 48 BAR DIAMETERS

CONCRETE SLABS ON GRADE:
SHALL BE INSTALLED OVER MINIMUM 6 MIL POLYETHYLENE VAPOR RETARDER WITH JOINTS LAPPED 6" AND SEALED OVER CLEAN, COMPACTED EARTH OR FILL WITH APPROVED CHEMICAL SOIL TREATMENT FOR PREVENTION OF SUBTERRANEAN TERMITES. SAWCUTS: FOR CONTROLLED CRACKING CUT A 1" SAWCUT INTO SLAB IN A 12"x12" GRID WITHIN 12 HOURS OF CONCRETE PLACEMENT, PROVIDE SAWCUTS THROUGH OUT SLAB CALL EOR FOR ALTERNATIVE METHODS.

WOOD FRAMING SPECIFICATIONS:
ALL WOOD FRAMING HAS BEEN DESIGNED IN ACCORDANCE WITH NATIONAL DESIGN SPECIFICATIONS (NDS) FOR WOOD CONSTRUCTION, LATEST EDITION. ALL WOOD MEMBERS EXPOSED TO WEATHER OR IN CONTACT WITH MASONRY, CONCRETE OR SOIL SHALL BE PRESSURE-TREATED; IF, ACQ OR NON-DOT BORATE PRESERVATIVE TREATMENT IS USED, ALL ATTACHED FASTENERS SHALL BE HOT DIPPED GALVANIZED; IF ACZA PRESERVATIVE IS USED, ALL ATTACHED FASTENERS SHALL BE STAINLESS STEEL.

PRE-ENGINEERED WOOD TRUSSES:
SHALL BEAR THE SEAL OF AN ENGINEER IN THE STATE WHERE PROJECT IS BEING BUILT AND SHALL COMPLY WITH NFPA, TPI, AND AITC 100. CONTRACTOR SHALL VERIFY THAT ADEQUATE TRUSS BEARING IS INSTALLED AT ALL TRUSSES AS INDICATED IN THE TRUSS SHOP DRAWINGS. ALL TRUSS-TO-TRUSS CONNECTIONS AND TRUSS PROFILES ARE THE RESPONSIBILITY OF THE DELEGATED TRUSS ENGINEER. ALL TRUSSES SHALL HAVE TEMPORARY BRACING PER "COMMENTARY" AND RECOMMENDATION FOR HANDLING, INSTALLING & BRACING METAL PLATE CONNECTED WOOD TRUSSES, HIB-91." AT MULTIPLE STRAP CONNECTIONS, SPREAD STRAPS TO AVOID NAILING CONFLICTS THROUGH TRUSS. WHEN USING (2) STRAPS ON SINGLE PLY TRUSSES, PLACE STRAPS DIAGONALLY ACROSS DBL. TOP PLATE FROM E.A. OTHER.

ROOF COVERING SPECIFICATIONS:
THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF THE ROOF COVERING SYSTEM. ASPHALT SHINGS SHALL COMPLY WITH ASTM D3161 AND BE INSTALLED ACCORDING TO THE MANUFACTURER'S REQUIREMENTS. CLAY AND TILE ROOFS SHALL BE INSTALLED PER THE "CONCRETE AND CLAY ROOF TILE INSTALLATION MANUAL" AND THE MANUFACTURER'S REQUIREMENTS. STANDING SEAM METAL ROOFS SHALL COMPLY WITH ASTM E1514 AND BE INSTALLED ACCORDING TO THE MANUFACTURER'S REQUIREMENTS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF ALL METAL FLASHING AND VALLEY MATERIALS.

WATERPROOFING:
THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN/INSTALLATION OF ALL WATER PROOFING.

WOOD FASTENING SCHEDULE

MEMBERS	CONNECTION TYPE	FASTENER
TOP PLATE TO TOP PLATE	FACE NAIL	2-GUN NAILS @ 12" STAG.
TOP PLATE, LAPS/INTERSECTION	FACE NAIL	(2-16d) 3-GUN NAILS
DBL. TOP PLATE TO STUD	FACE NAIL	(2-16d) 3-GUN NAILS
RIM JOIST TO TOP PLATE	TOE NAIL	(8d @ 6") GUN NAIL @ 6"
CEILING JOIST TO TOP PLATE	TOE NAIL	(3-8d) 5-GUN NAILS
CEILING JOIST, OVER PARTITIONS	FACE NAIL	(3-16d) 4-GUN NAILS
CEILING JOIST TO ROOF RAFTER	FACE NAIL	(2-16d) 8-GUN NAILS
JOIST/TRUSS TO PLATE	TOE NAIL	(6-16d) 3-GUN NAILS
RAFTER TO PLATE	TOE NAIL	(3-8d) 3-GUN NAILS
JACK RAFTER TO HIP	TOE NAIL	(3-10d) 4-GUN NAILS
ROOF RAFTER TO 2x... RIDGE BM.	TOE NAIL	(2-16d) 3-GUN NAILS
CONF. HEADER, TWO PIECES	FACE NAIL	16d @ 16" O.C. @ EDGE
CONF. HEADER TO STUD	TOE NAIL	(3-16d) 4-GUN NAILS
STUD TO SOLE PLATE	TOE NAIL	(3-16d) 4-GUN NAILS
SOLE PLATE TO JOIST/BLOCKING	FACE NAIL	(16d @ 16") GUN NAIL @ 8"

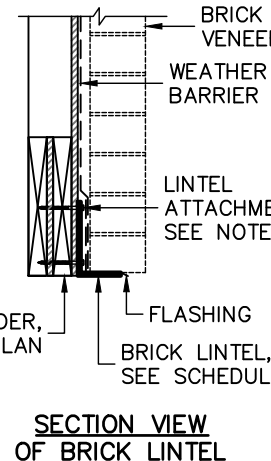
LINTEL DIMENSION	MIN. BRG.	MAX. SPAN
L3 1/2"x3 1/2"x1/4"	4"	6'-0"
L4x3 1/2"x1/4"	6"	8'-0"
L5x3 1/2"x1/4"	6"	10'-0"
L6x3 1/2"x1/4"	6"	12'-0"
L7x3 1/2"x1/4"	6"	16'-0"

1. STEEL LINTEL MUST BE MINIMAL 36" LINTEL MUST HAVE CORROSION RESISTANT COATING OF EPOXY BASED PAINT.
2. LINTEL MORE THAN 8'-0", SHOULD BE LATERALLY SUPPORTED NOT TO EXCEED 6 FT. O.C. W/ 2-1/2"x3" WD. SCREWS INTO HEADER PROVIDE A 1/2" VERTICAL SLOTTED HOLE FOR SCREW.
3. BRICK VENEER ATTACHMENT: HORIZONTAL TIES @ 24" O.C., VERT. TIES @ 12" O.C. (FOR 10mph WIND-ZONE VERT. TIES @ 16" O.C.). AT ALL OPENINGS SPACE TIES WITHIN 12" OF OPENINGS. PROVIDE 3/4" WEEP HOLES @ 33" O.C. IMMEDIATELY ABOVE FLASHING.

PLAN LEGEND AND ABBREVIATIONS

INTERIOR LOAD BEARING WALL
GABLE X-BRACE, SEE DETAIL 10/SO.1
DESIGNATES SHEARWALL, THE HIDDEN LINE DESIGNATES SIDE OF WALL THE SHEARWALL SHEATHING TO BE APPLIED. 8d @ 3/8" DESIGNATES 8d COMMON @ 3/8" O.C. EDGE & 6" O.C. "IN THE FIELD"
LG – Long
MANUF – Manufacture
MONO – Monolithic
OC – On Center
OSB – Oriented Strand Board
PERP – Perpendicular
DIA – DIAMETER
EA – EACH
E.E. – EACH END
EOR – ENGINEER OF RECORD
EQ – EQUAL
EXT – EXTERIOR
FBC – FLORIDA BUILDING CODE
FDN – FOUNDATION
FT – FOOT
FTG – FOOTING
HORZ – HORIZONTAL
LBS – POUNDS
BM – BEAM
BOT – BOTTOM
BRG – BEARING
CMU – CONCRETE MASONRY UNIT
DBL – DOUBLE
DIA – DIAMETER
EA – EACH
E.E. – EACH END
EOR – ENGINEER OF RECORD
EQ – EQUAL
EXT – EXTERIOR
FBC – FLORIDA BUILDING CODE
FDN – FOUNDATION
FT – FOOT
FTG – FOOTING
HORZ – HORIZONTAL
LBS – POUNDS

(2)2x8-1/2 HEADER SIZE, JACK AND KING STUD QUANTITY.



SECTION VIEW OF BRICK LINTEL

HEADER SIZE, JACK AND KING STUD QUANTITY.

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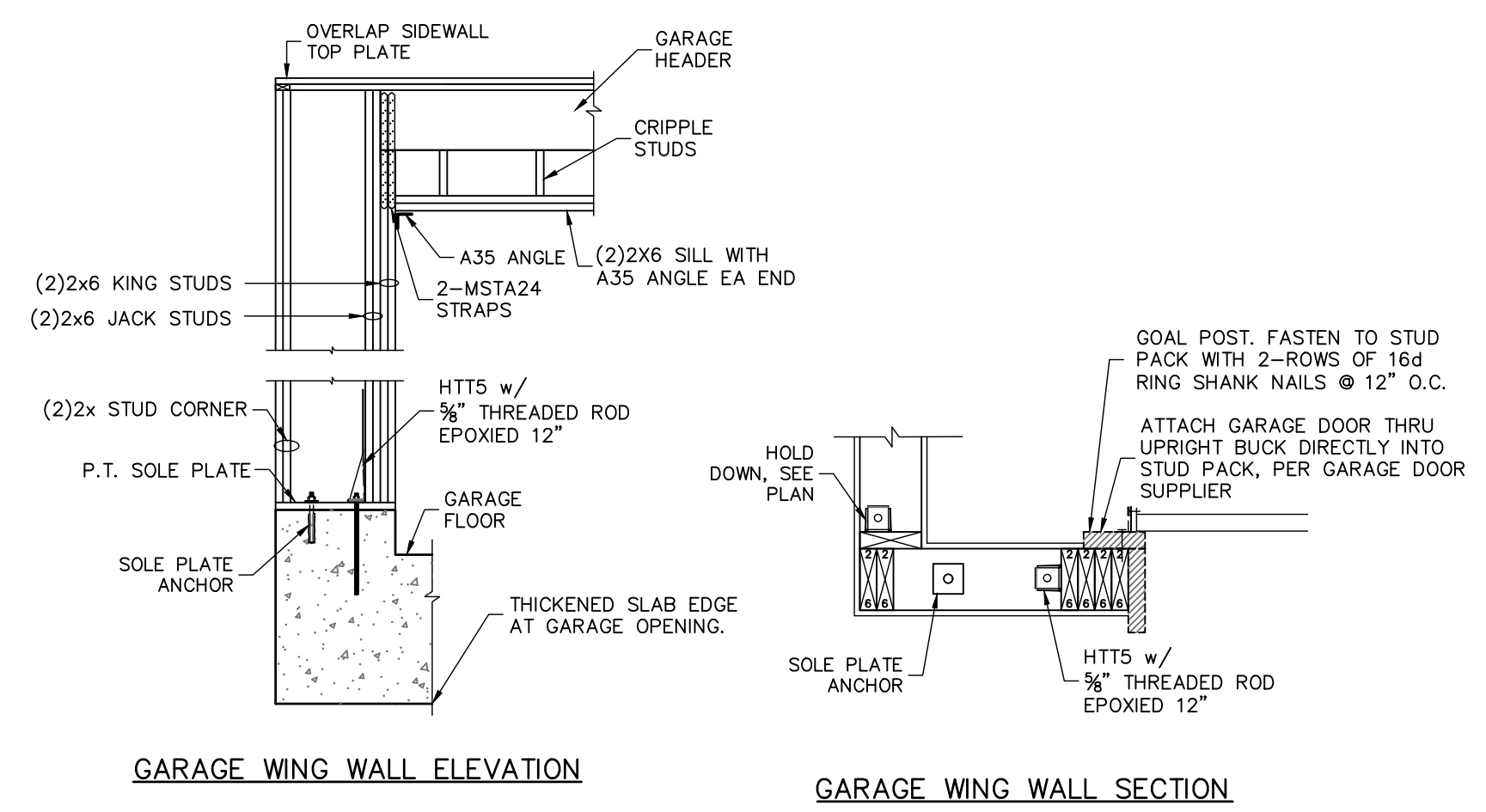
SECTION VIEW OF BRICK LINTEL

HEADER SIZE, JACK AND KING STUD QUANTITY.

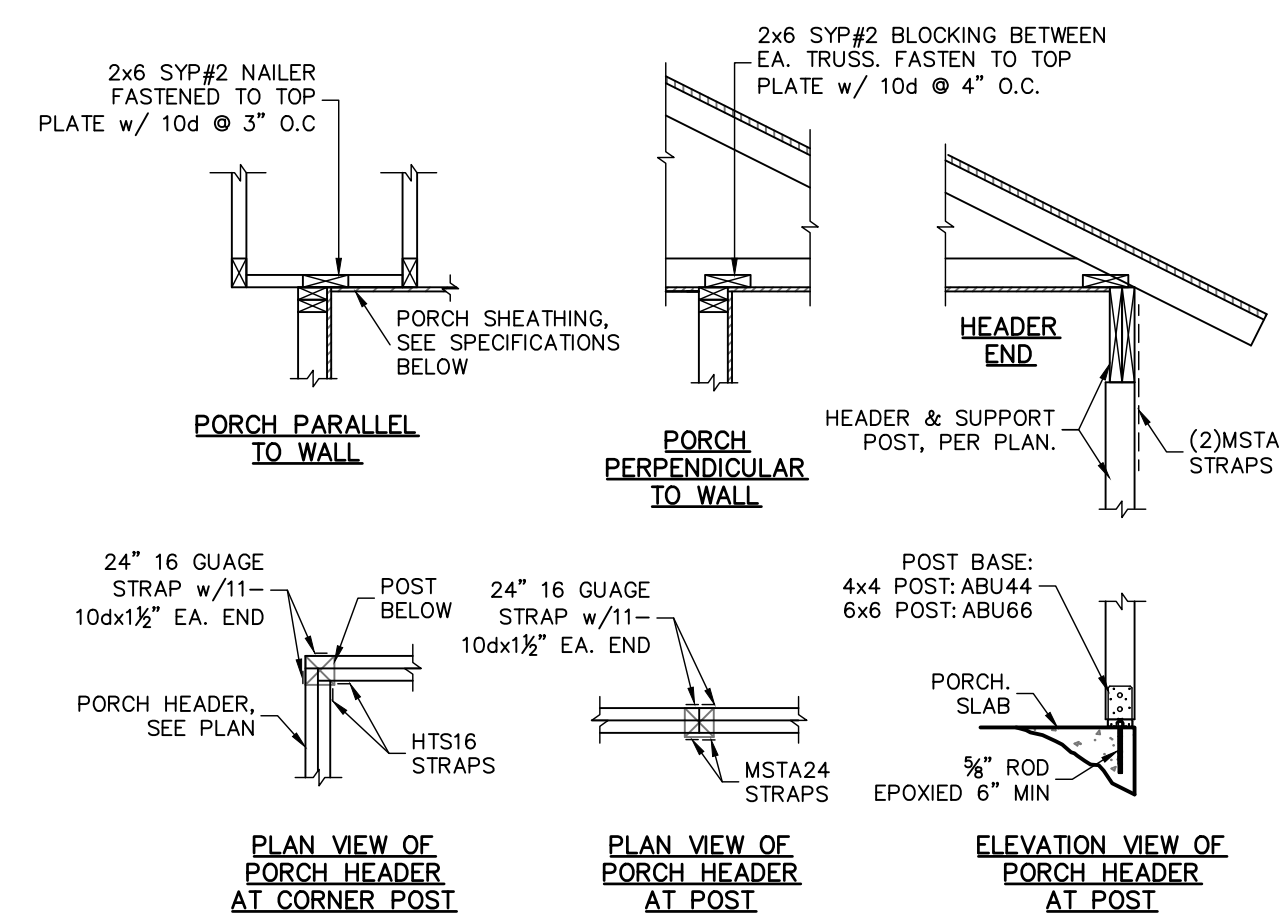
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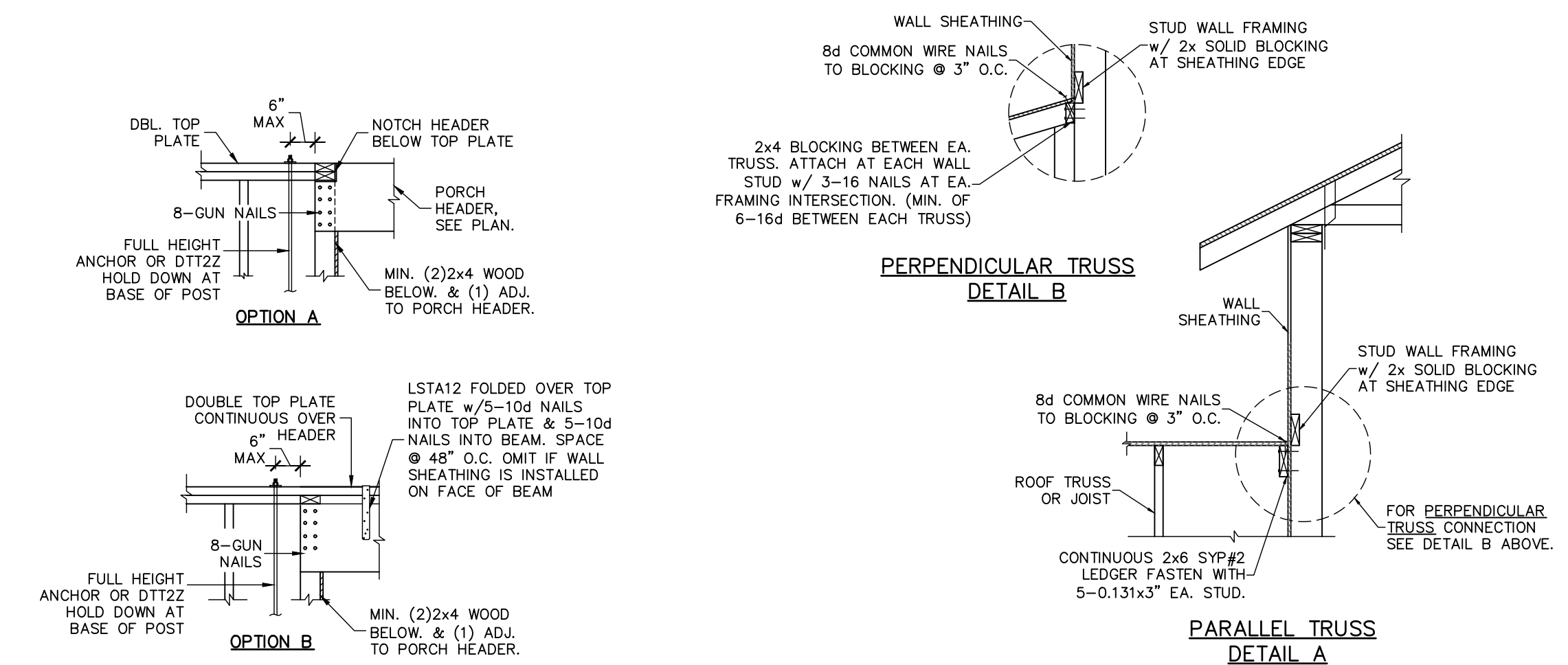
SECTION VIEW OF BRICK LINTEL



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SO.1
GARAGE HEADER FRAMING
SCALE: N.T.S.

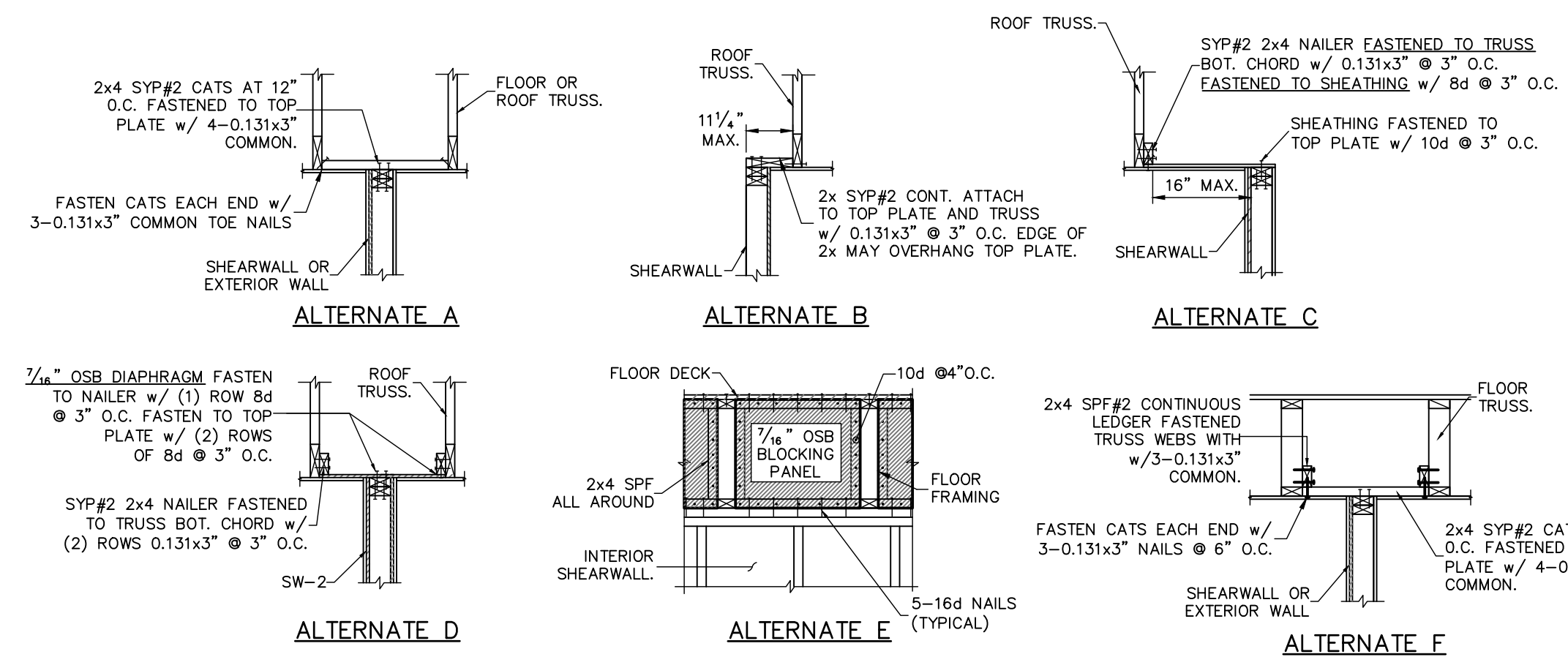


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SO.1
TYPICAL PORCH FRAMING DETAILS
SCALE: N.T.S.

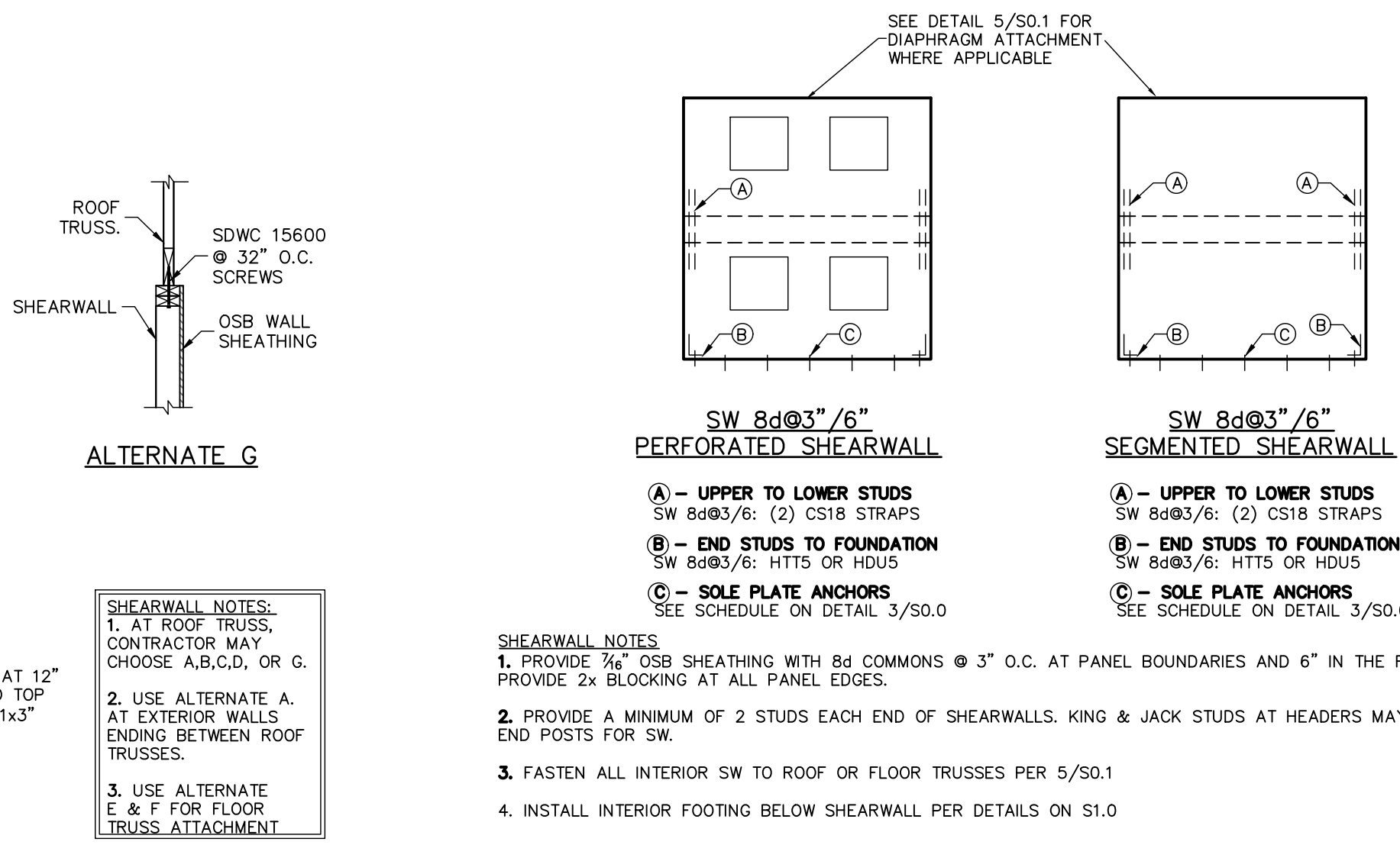


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SO.1
TYPICAL PORCH BEAM CONNECTION
SCALE: N.T.S.

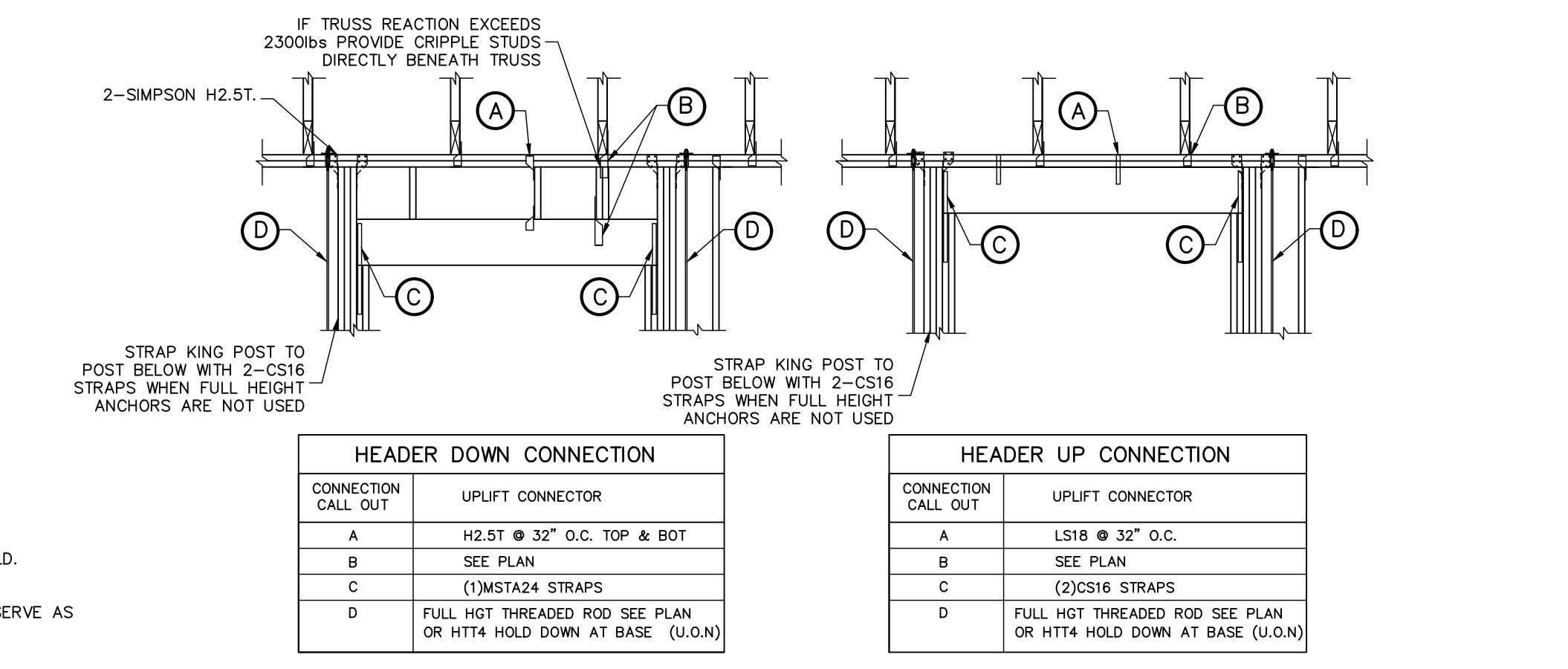
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SO.1
WALL ADJ. TO ROOF CONNECTION
SCALE: N.T.S.



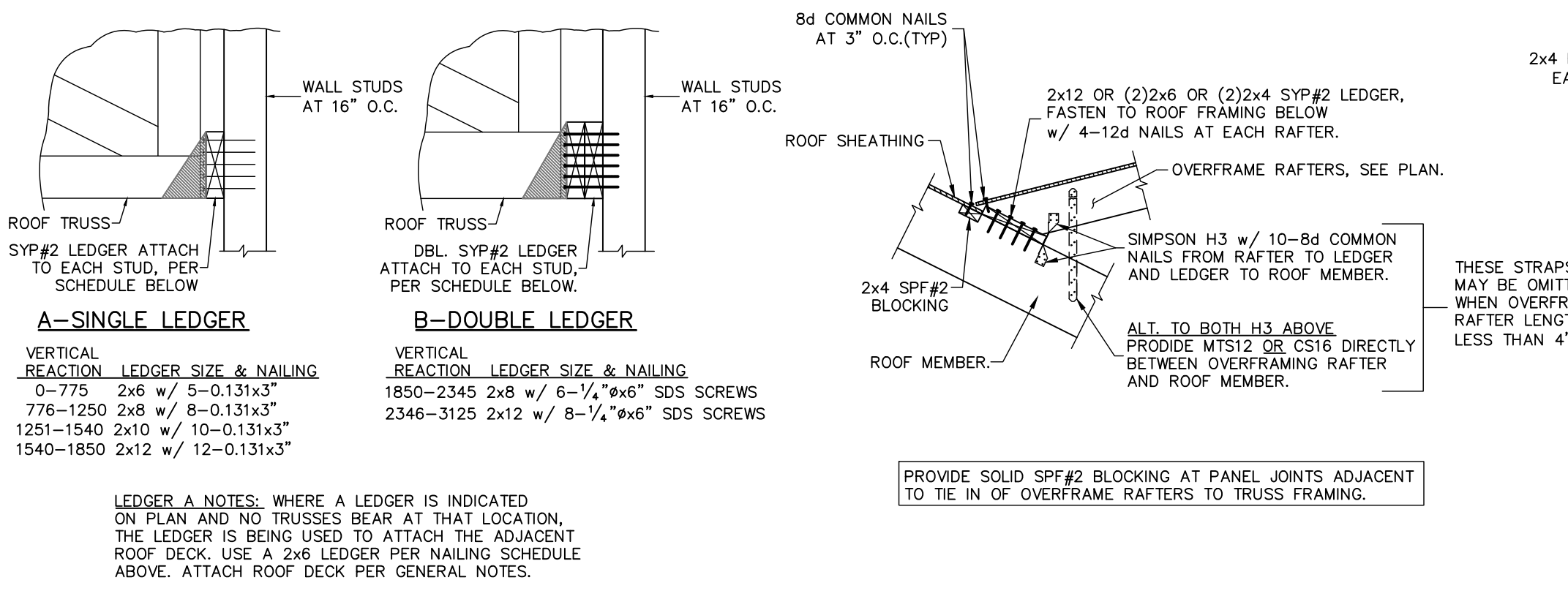
5
SO.1
SHEARWALL ATTACHMENT AT ROOF & FLOOR
SCALE: N.T.S.



6
SO.1
TYPICAL SHEARWALL ELEVATION
SCALE: N.T.S.

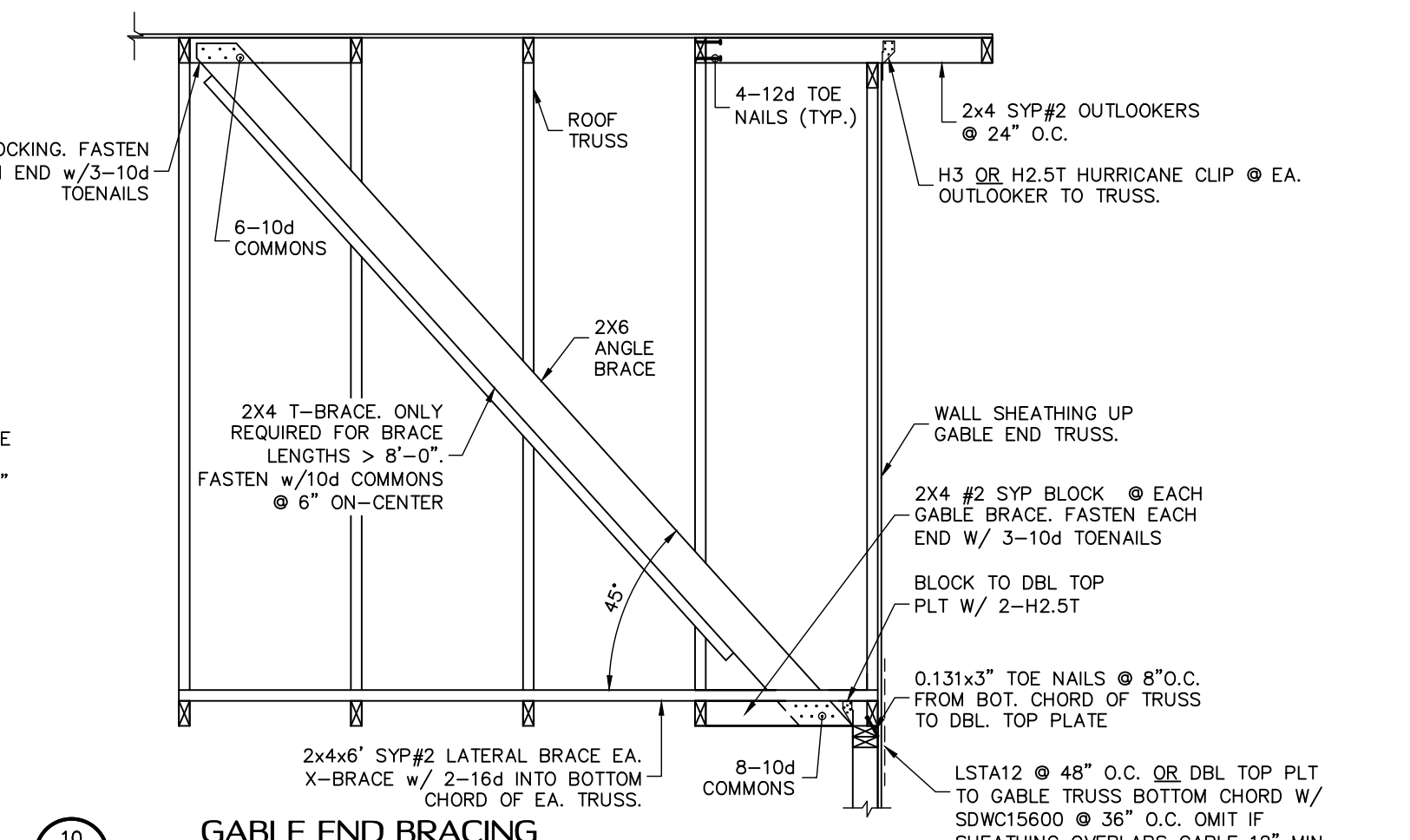


7
SO.1
HEADER TIE DOWN
SCALE: N.T.S.

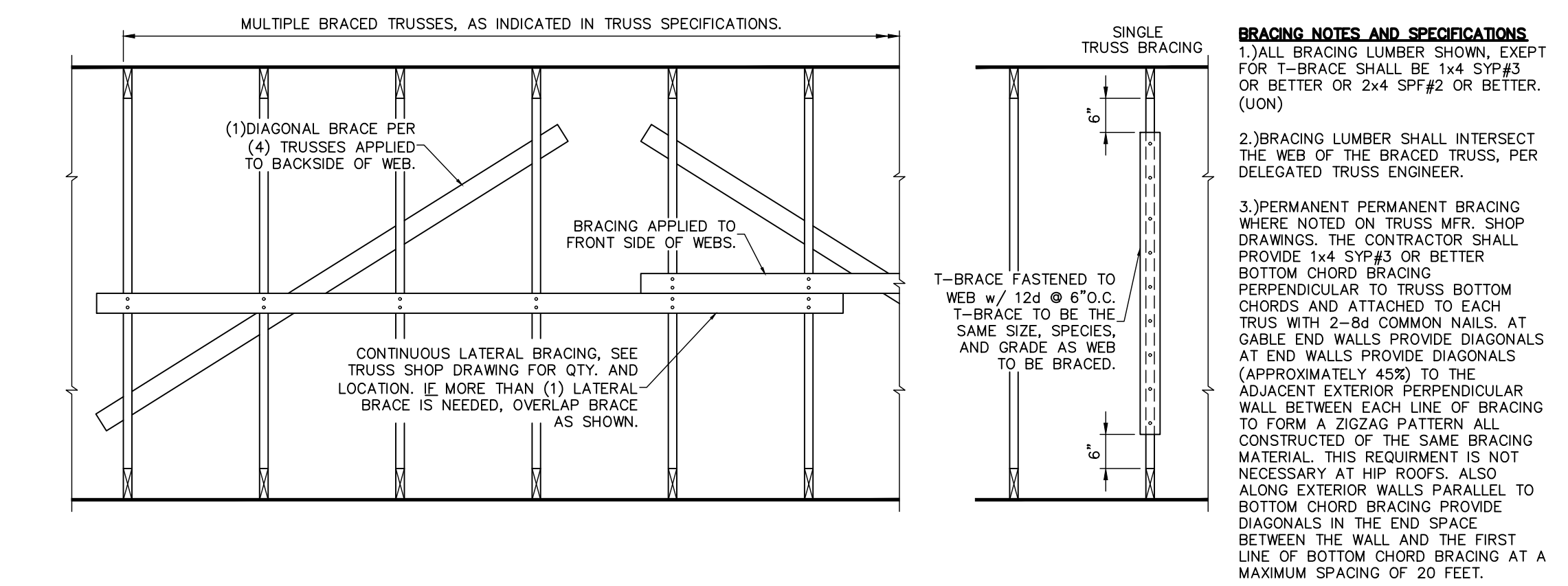


8
SO.1
LEDGER CONNECTION
SCALE: N.T.S.

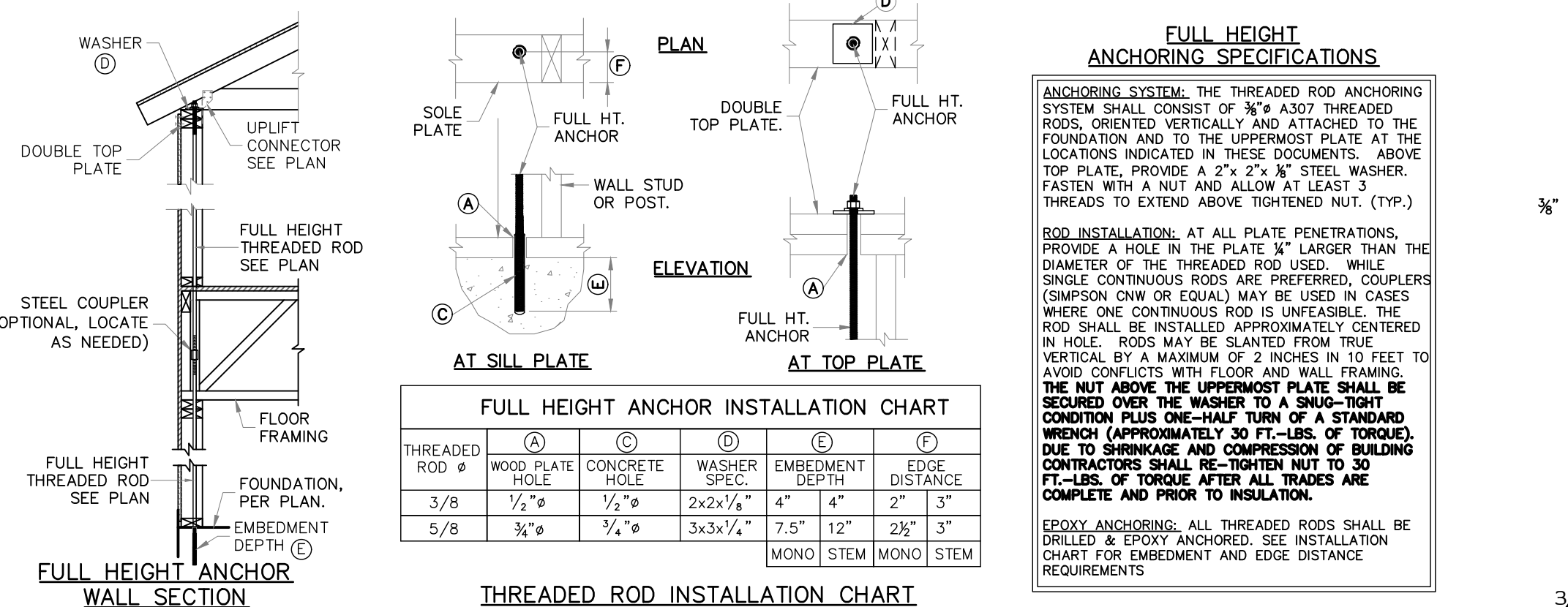
9
SO.1
DECK LEDGER AT OVERFRAME RAFTERS
SCALE: N.T.S.



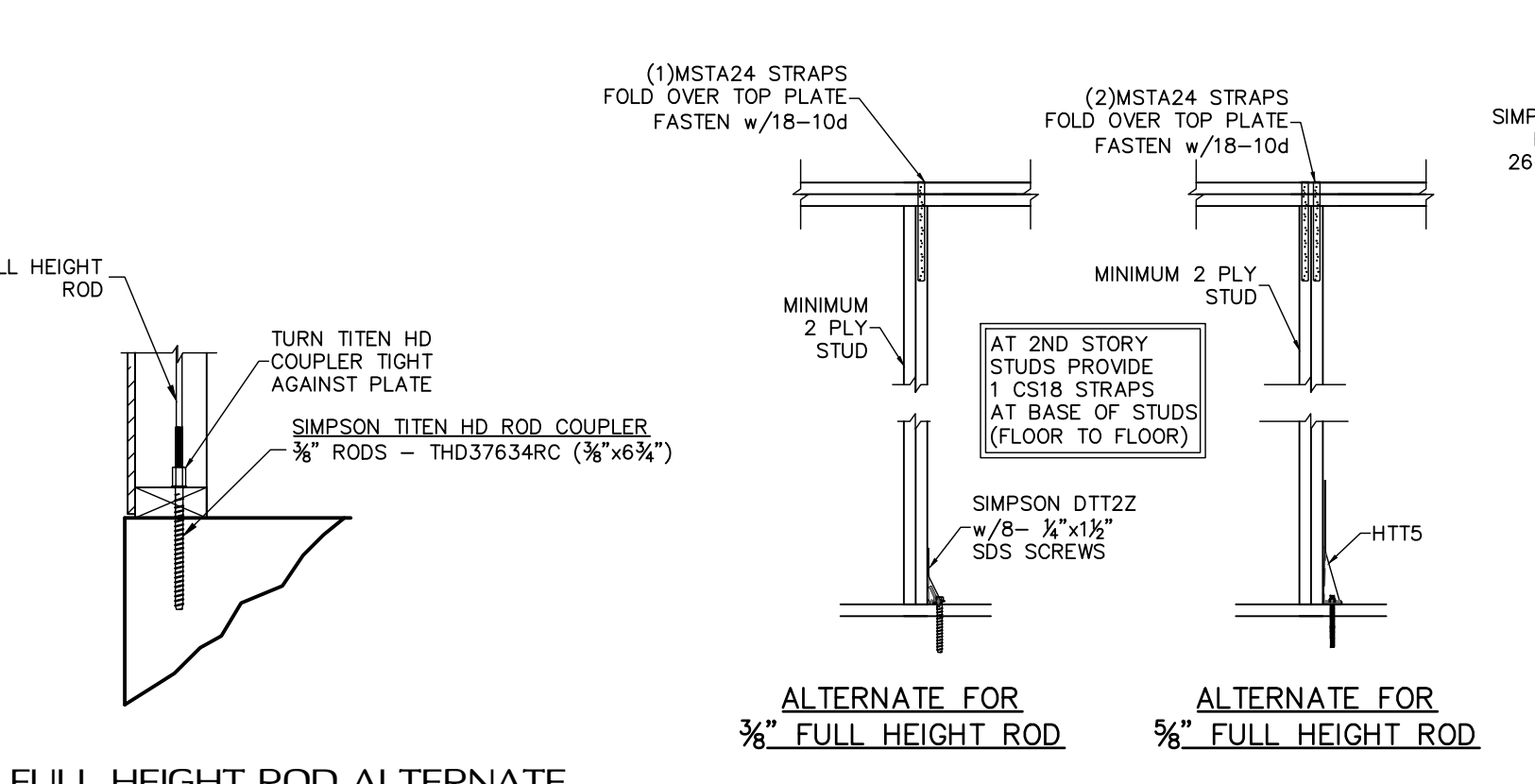
10
SO.1
GABLE END BRACING
SCALE: N.T.S.



11
SO.1
PERMANENT TRUSS BRACING
SCALE: 3/4" = 1'-0"

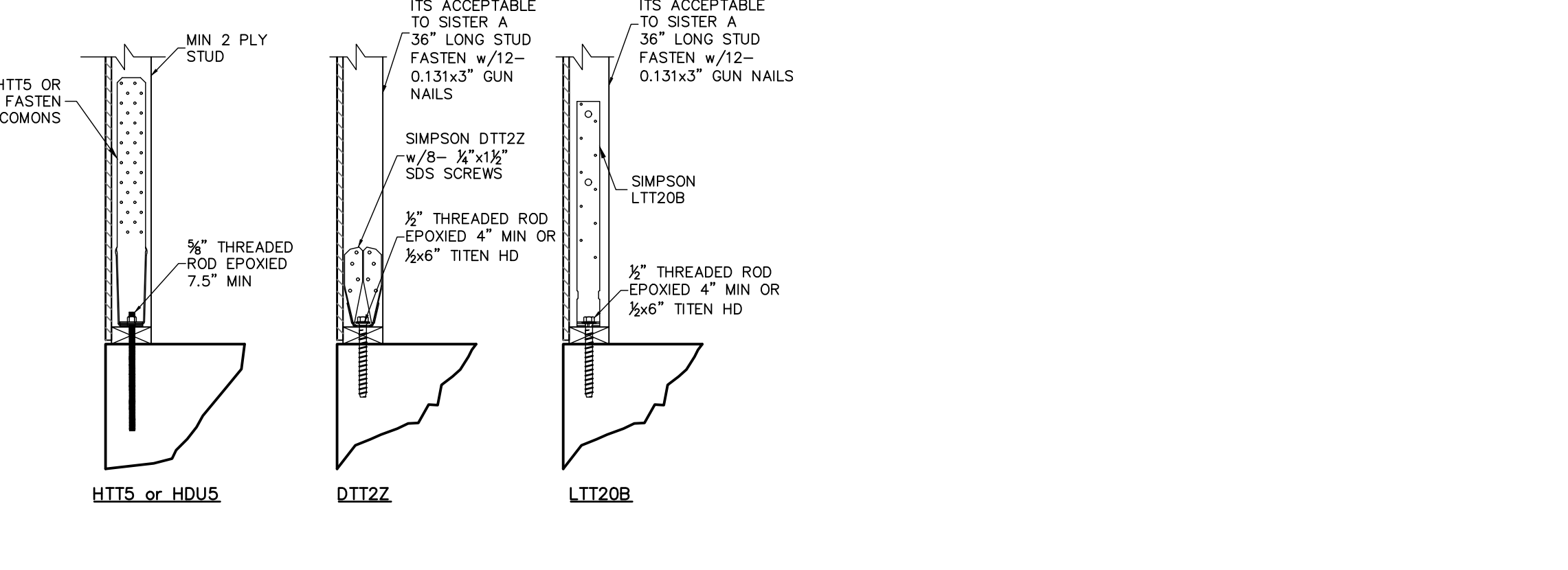


12
SO.1
FULL HEIGHT WOOD FRAME WALL ANCHORING SYSTEM
SCALE: N.T.S.

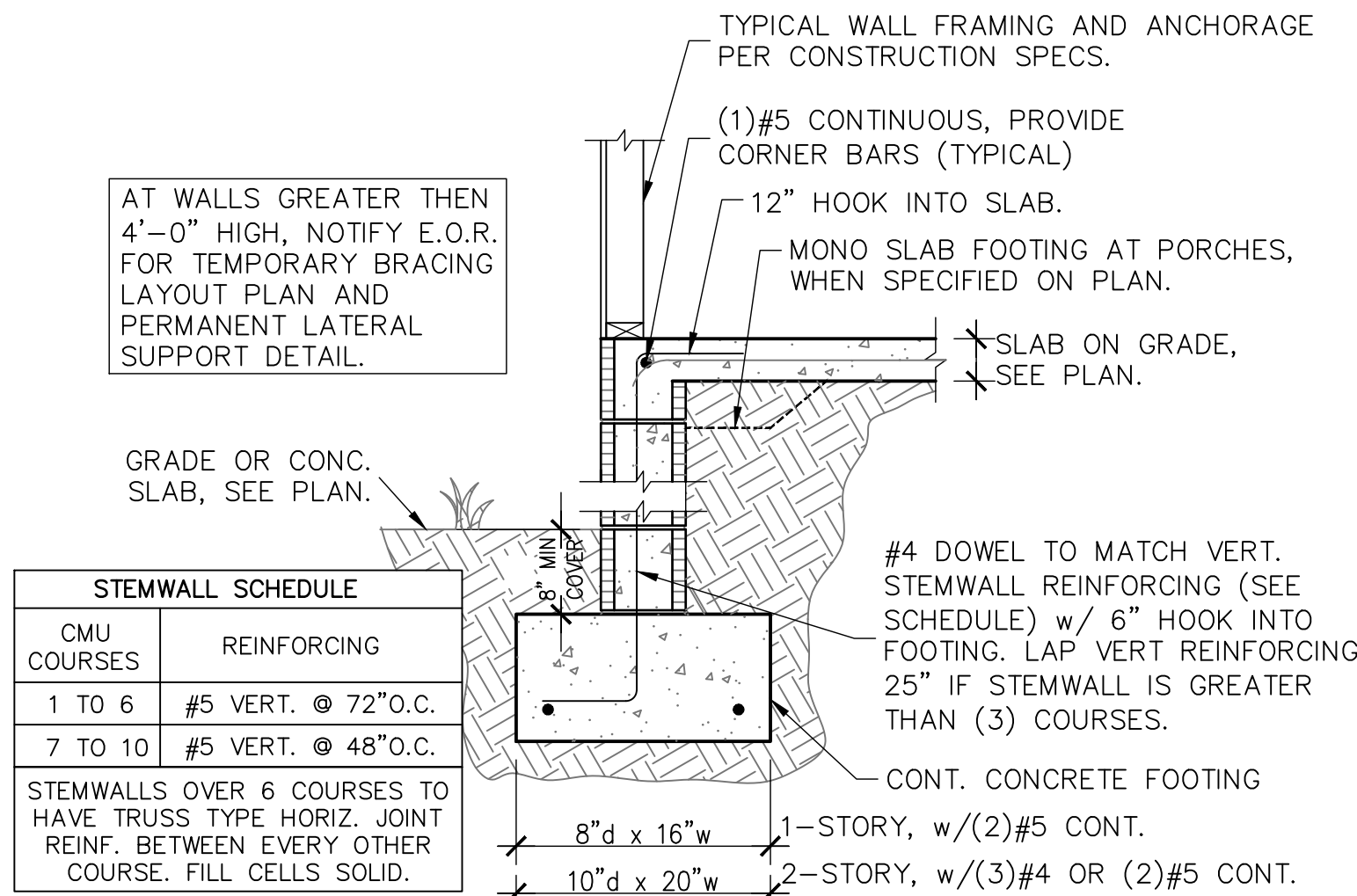


13
SO.1
3/8" FULL HEIGHT ROD ALTERNATE ATTACHMENT
SCALE: N.T.S.

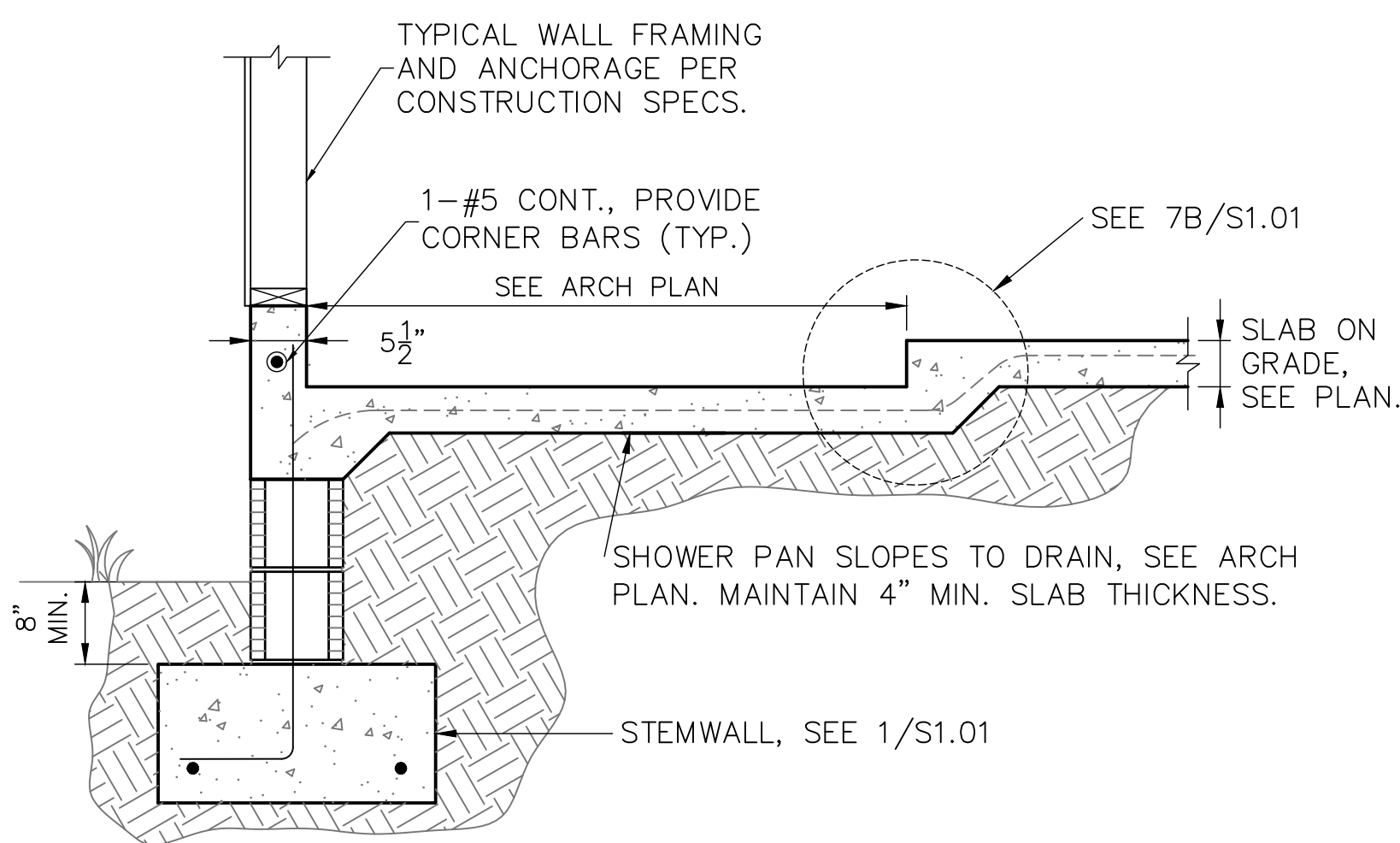
14
SO.1
FULL HEIGHT THREADED ROD ALTERNATE
SCALE: N.T.S.



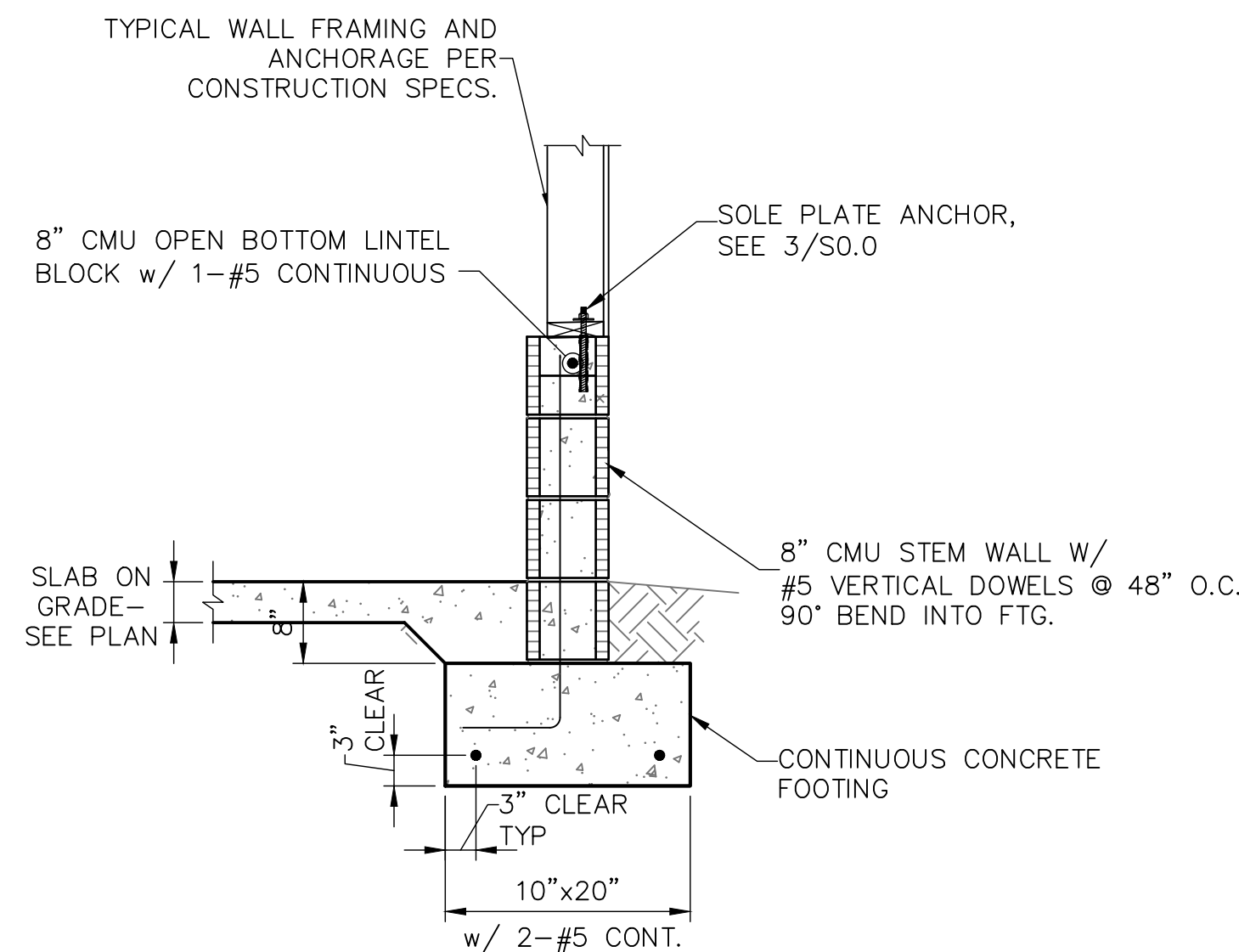
15
SO.1
HOLD DOWN ATTACHMENT DETAIL
SCALE: N.T.S.



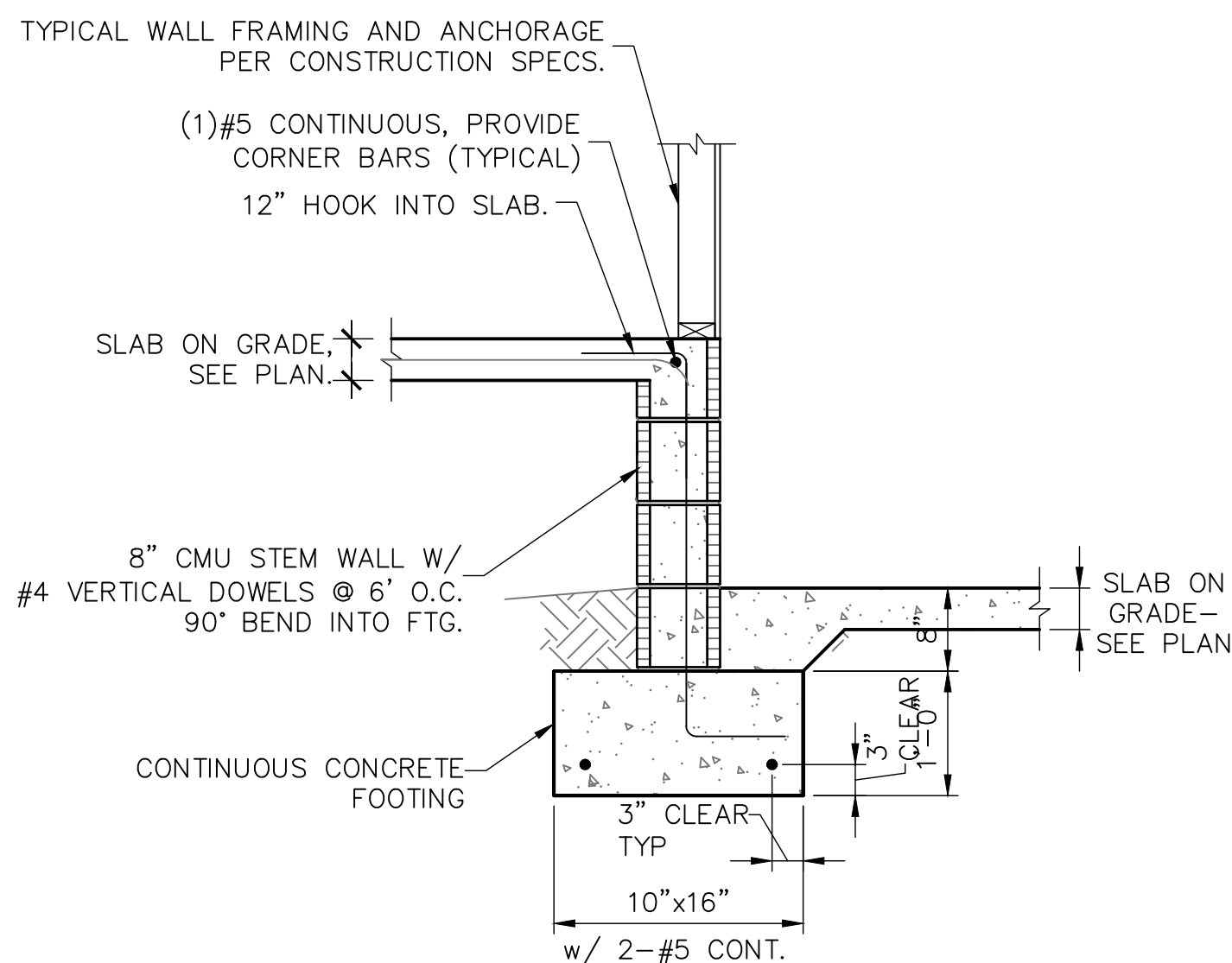
1 STEMWALL FOOTING
S1.01 SCALE: 3/4" = 1'-0"



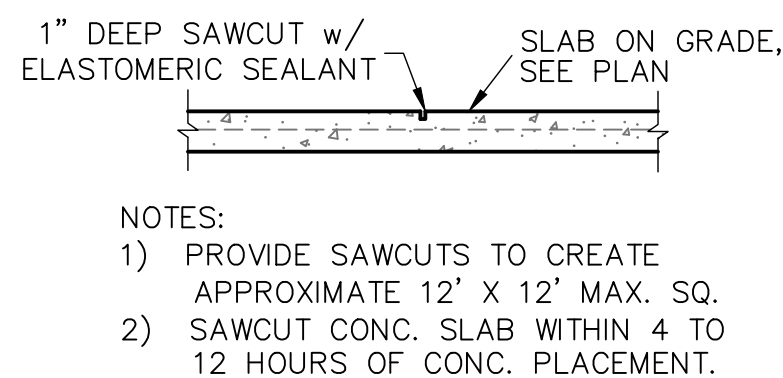
2 FOOTING W/ SHOWER RECESS
S1.01 SCALE: 3/4" = 1'-0"



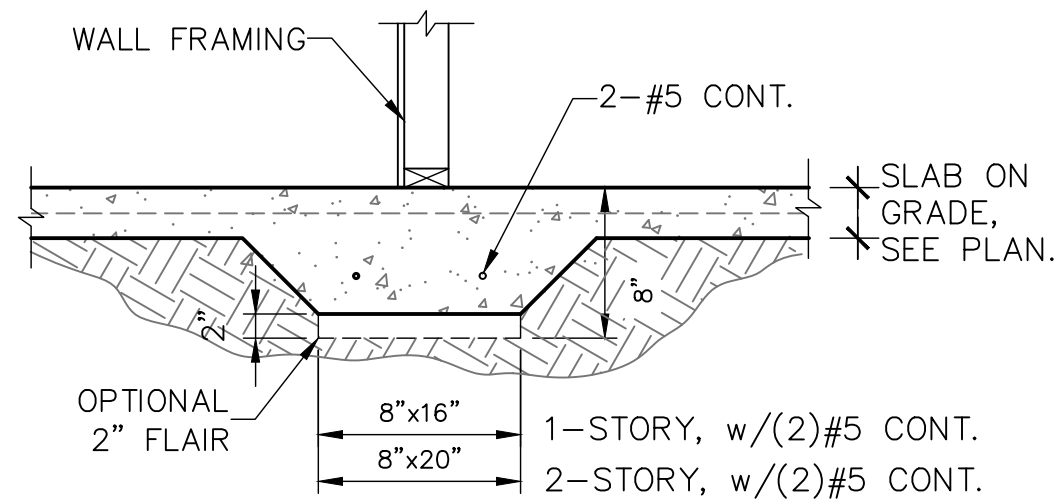
3 GARAGE STEM WALL
S1.01 SCALE: 3/4" = 1'-0"



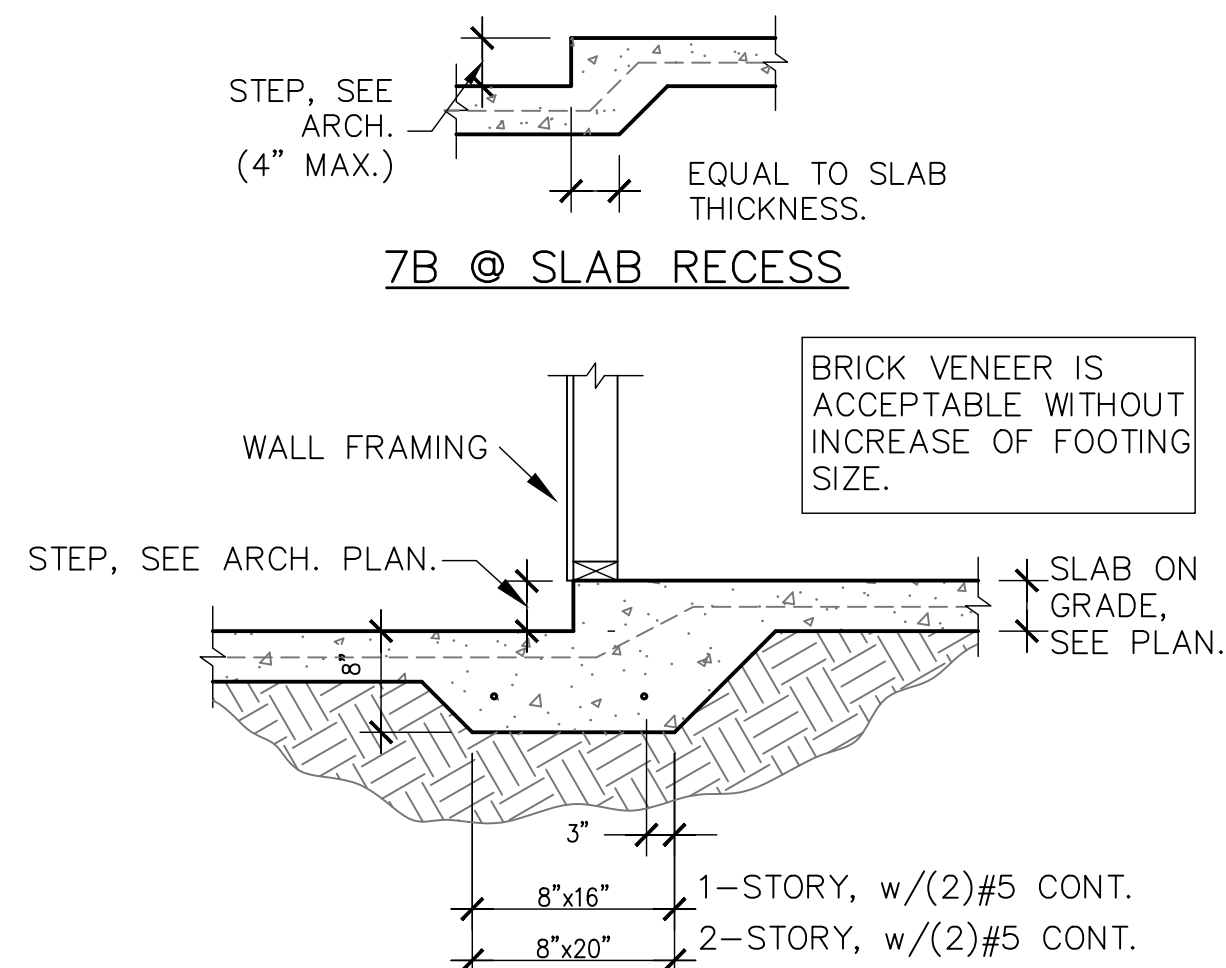
4 STEMWALL AT GARAGE
S1.01 SCALE: 3/4" = 1'-0"



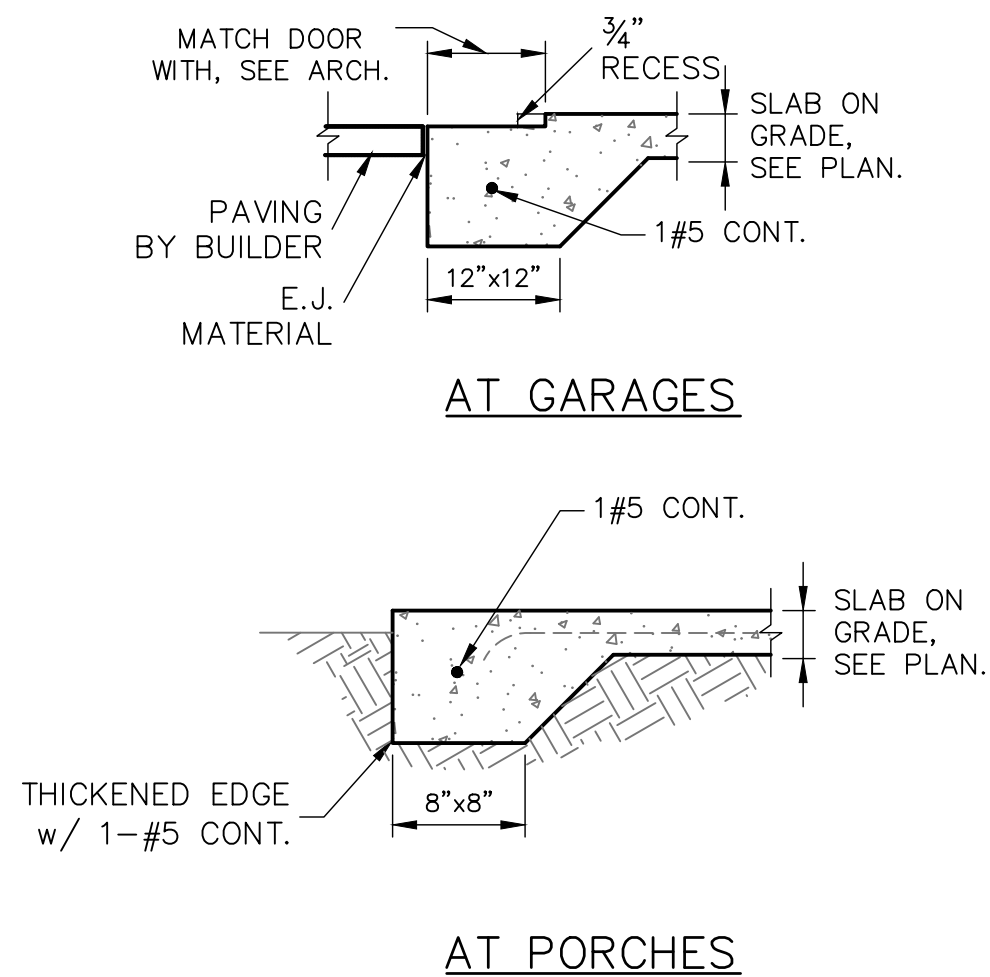
5 SAW CUT DETAIL
S1.01 SCALE: 3/4" = 1'-0"



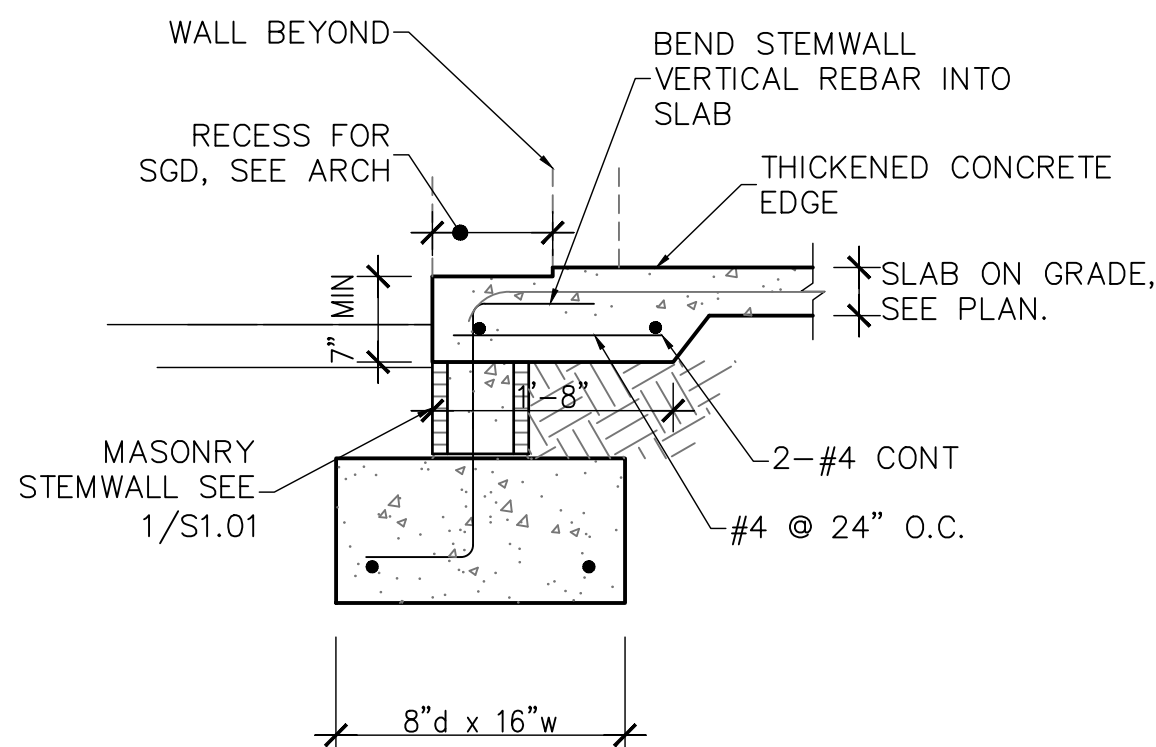
6 BEARING AT INTERIOR
S1.01 SCALE: 3/4" = 1'-0"



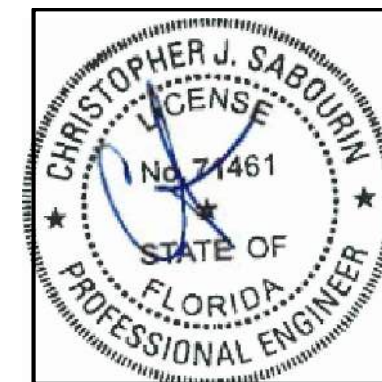
7 MONO. FOOTING AT STEP-DOWN
S1.01 SCALE: 3/4" = 1'-0"



8 THICKENED SLAB
S1.01 SCALE: 3/4" = 1'-0"



9 STEMWALL FOOTING AT SLIDER
S1.01 SCALE: 3/4" = 1'-0"



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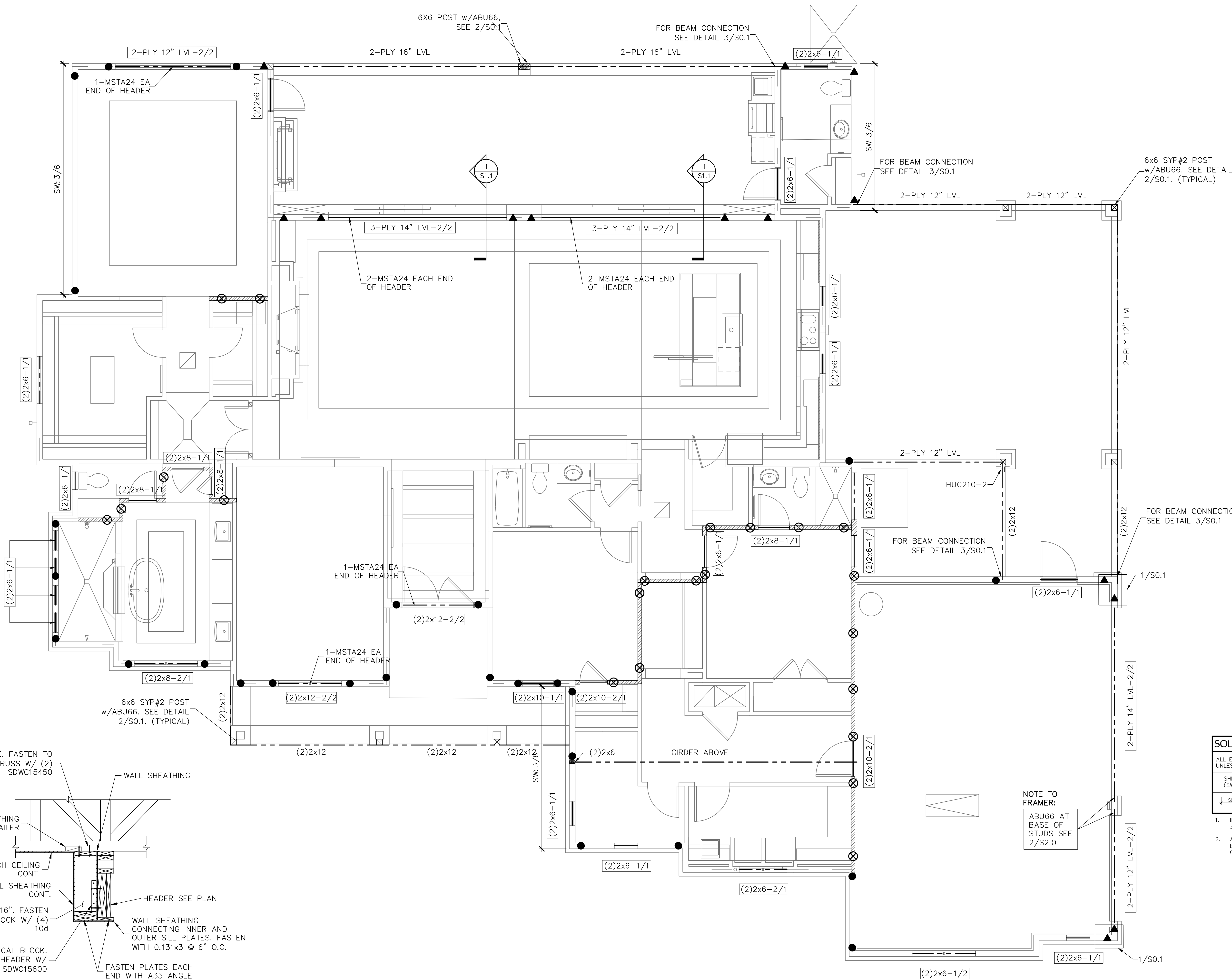
STRUCTURAL ENGINEERING FOR
THE DICKS RESIDENCE

FIELD ALTERATION
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SCALING
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FOUNDATION
DETAILS

SHEET
S1.01
SHEET 4 OF 7



TRUSSES PERPENDICULAR

FIRST FLOOR FRAMING PLAN

SCALE: 1/4" = 1'-0"

1 WOOD FRAMED HEAD ABOVE SLIDING GLASS
S1.1

SYMBOLS LEGEND

	DESIGNATES OSB SHEARWALL. THE HIDDEN LINE DESIGNATES SIDE OF WALL. THE SHEARWALL SHEATHING TO BE APPLIED. 8d @ 3\"/>
	DESIGNATES THE HEADER SIZE, NUMBER OF PLYS & JACKING STUDS NEEDED FOR SUPPORT HEADER.
	BEAM OR TRUSS, SEE PLAN

ANCHOR LEGEND

	3/8\" A307 DIAMETER FULL HEIGHT THREADED ROD, SEE DETAIL 12/SO.1
	3/8\" A307 DIAMETER FULL HEIGHT THREADED ROD, SEE DETAIL 12/SO.1
	3/8\" A307 DIAMETER THREADED ROD TERMINATES AT FIRST FLOOR TOP PLATE, SEE DETAIL 12/SO.1
	3/8\" A307 DIAMETER THREADED ROD TERMINATES AT FIRST FLOOR TOP PLATE, SEE DETAIL 12/SO.1
	SIMPSON HTTS SEE DETAIL 15/SO.1
	SIMPSON DT22 SEE DETAIL 15/SO.1
	SIMPSON LT20B SEE DETAIL 15/SO.1

WALL STUD SCHEDULE

LOCATION	PLATE HEIGHT	STUD SIZE & SPACING
EXTERIOR	9'-1\"/>	2x4 SPF#2 @ 16\"/>
EXTERIOR	10'-1\"/>	2x6 SPF#2 @ 16\"/>
EXTERIOR	10'-1\"/>	2x4 SPF#2 @ 12\"/>
EXTERIOR	14'-0\"/>	2x6 SPF#2 @ 16\"/>
INTERIOR	10'-0\"/>	2x4 SPF#2 @ 16\"/>
INTERIOR	12'-0\"/>	2x6 SPF#2 @ 16\"/>
INTERIOR	12'-0\"/>	2x4 SPF#2 @ 12\"/>

- STUD NOTES:**
- 1.) WALL STUDS SPECIFIED ON PLAN SUPERSEDE THIS TABLE
 - 2.) MINIMUM STUD SIZE AND SPACING ARE SHOWN. CONTRACTOR MAY INCREASE STUD SIZE TO MEET ARCHITECTURAL REQUIREMENTS.
 - 3.) SPF DENOTES SPRUCE PINE FIR.. SYP DENOTES SOUTHERN YELLOW PINE.
 - 4.) USE SYP#2 FOR ALL TOP PLATES AND SOLE PLATES.
 - 5.) FASTEN BOTTOM PLATE OF INTERIOR LOAD BEARING WALLS TO CONCRETE SLAB W/16d MASONRY CUT NAILS @ 16\"/>

COMBINED USE PANEL NOTES

1. EXTERIOR WALL SHEATHING SHALL BE CONTINUOUS FROM BOTTOM PLATE TO UPPER MOST TOP PLATE. SEE DETAIL 1/SO.1 FOR SHEATHING SPLICE LOCATIONS FOR MULTI STORY CONDITIONS
2. SEE SHEET SO.0 FOR WALL SHEATHING SPECIFICATIONS.
3. UPPER MOST TOP PLATE SUPPORTING ROOF MEMBERS SHALL BE STRAPPED AS SHOWN IN DETAIL 1/SO.0
4. INSTALL SOLE PLATE ANCHORS PER DETAIL 3/SO.0

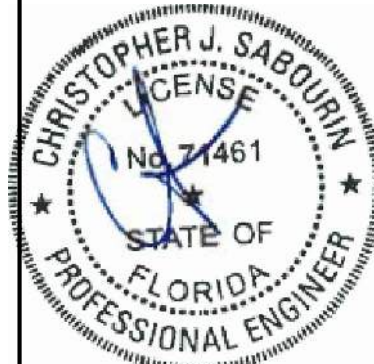
GENERAL NOTES

1. SEE DETAIL 2/SO.0 FOR WALL FRAMING DETAIL. SEE WALL STUD SCHEDULE THIS SHEET FOR STUD SIZES AND SPACING. AT GIRDERS AND BEAMS, PROVIDE STUDS BELOW TO MATCH BEAM/GIRDER PLYS.
2. SEE SHEET SO.0 FOR ROOF AND FLOOR SHEATHING SPECIFICATIONS.
3. WHERE FRAMING MEMBERS CONSIST OF MULTIPLE PLYS (BEAMS, HEADER, AND STUDS) FASTEN PLYS TOGETHER PER DETAIL 6/SO.0
4. INSTALL SOLE PLATE ANCHORS PER DETAIL 3/SO.0
5. AT SHEARWALLS, PROVIDE DIAPHRAGM ATTACHMENT PER DETAIL 5/SO.1
6. FOR ATTACHMENT OF EXTERIOR WALLS THAT TERMINATE BETWEEN TRUSSES, SEE 5A/SO.1
7. AT PORCHES, SEE DETAIL 2/SO.1 FOR FRAMING AND HOLD DOWNS

SOLE PLATE ANCHOR SPACING SCHD

ALL EXTERIOR WALL UNLESS OTHER NOTED	42\"/>
SHEARWALLS (SW 8d@3\"/>	24\"/>
	WHEN NOTED ON PLAN SEE NOTE 2

1. INSTALL SOLE PLATE ANCHORS PER DETAIL 3/SO.0
2. ANCHOR SPACING SHALL BE AS NOTED. FOR EXAMPLE - SOLE PLT @ 36\"/>



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FIRST FLOOR
FRAMING
PLAN

SHEET
S1.1
SHEET 5 OF 7

SYMBOLS LEGEND

HTS16

DESIGNATES UPLIFT CONNECTION.

FRAMING PLAN NOTES:

1. FOR TYPICAL ROOF SHEATHING AND FRAMING, SEE SHEET S0.0.
2. FOR SPECIFIC UPLIFT CONNECTORS, SEE PLAN, MIN. (1)SDWC CONNECTOR.
3. FOR GENERAL DESIGN SPECIFICATIONS SEE SHEET S0.0.
4. WHEN USING (2)2x12 CLIPS ON 1 1/2" WIDE LUMBER, PLACE CLIPS DIAGONALLY ACROSS DOUBLE TOP PLATE FROM EACH OTHER.

02.15.21

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ROOF TRUSS

PLACEMENT

PLAN

SHEET

S1.2

SHEET 6 OF 7

TRUSS FASTENING DETAILS

STUD DIRECTLY BELOW TRUSS

SDWC15600

TOP PLATE TO STUD SDWC15600

TRUSS TIE DOWN WITH SIMPSON SDWC

Rafter to Top Plate shown
Truss to Top Plate similar

Optimal 22 1/2°

30°

10°

0°

1/2" Max

STUD DIRECTLY BELOW TRUSS

SDWC15600

TOP PLATE TO STUD SDWC15600

Note:

1. Sloped-roof rafters may be sloped up to and including a 12:12 pitch and must be "birds-mouth" cut.
2. Reference detail 4 for installation instructions.

SIMPSON SDWC INSTALLATION RANGE

STUD NOT DIRECTLY BELOW TRUSS

SDWC15600

Note:

Reference detail 2a for installation angle limit

SDWC INSTALLATION

Rafter to Top Plate shown
(Truss to Top Plate similar)

1 1/2" max

Do not install SDWC in hatched area

SDWC15600

Overhang

1 1/2" MIN

2" MAX

STUD NOT DIRECTLY BELOW TRUSS

SDWC INSTALLATION RANGE

8" minimum edge distance for full values (with or without a plate splice)

Rafter or Truss

Splice may be in upper or lower plate

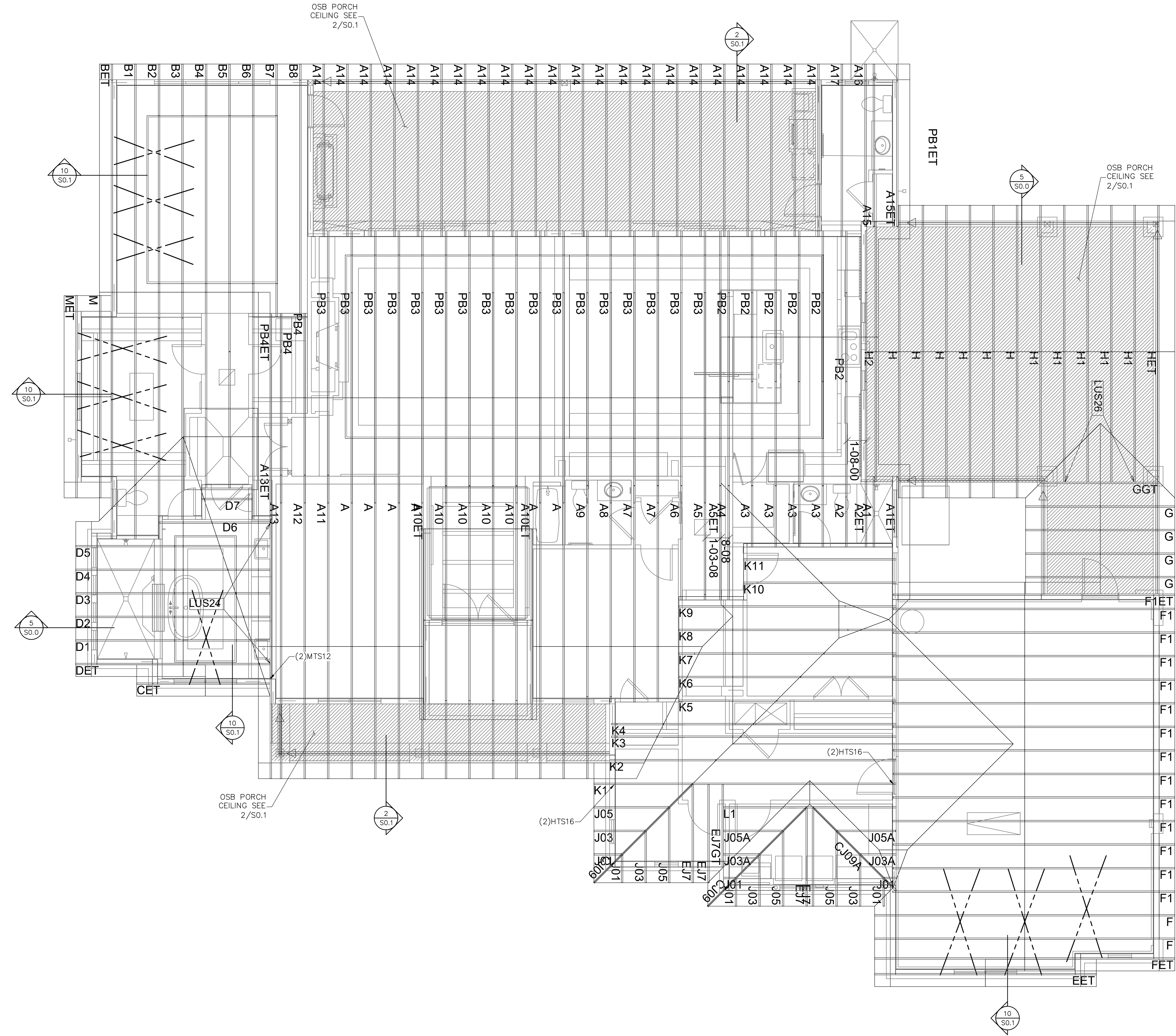
8" from top plate

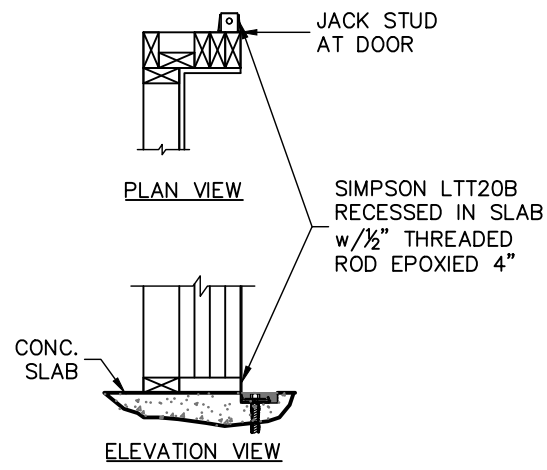
splice Offset for full values

STUD NOT DIRECTLY BELOW TRUSS

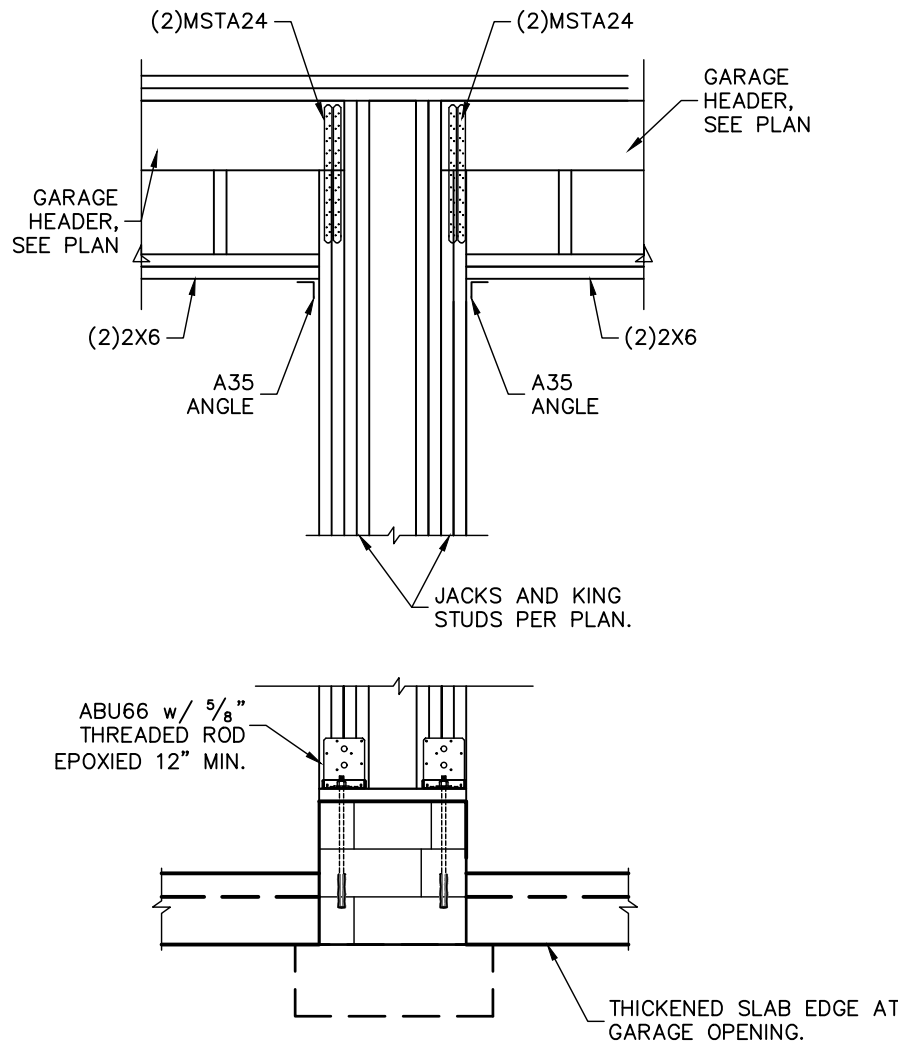
SDWC AT TOP PLATE SPLICE

STUD NOT DIRECTLY BELOW TRUSS

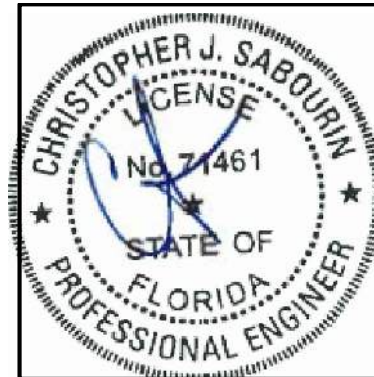




1 DOOR JAMB FASTENING
THIS DETAIL ONLY APPLIES WHEN NOTED ON PLAN



2 GARAGE CENTER WALL FRAMING
SCALE: 3/4" = 1'-0"



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MISC
DETAILS

SHEET
S2.0
SHEET 7 OF 7