

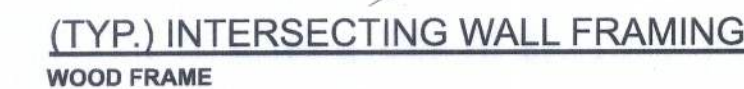
EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

		Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

		Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOADINGS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER TO OBTAIN THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBOR REQUIRED FOR THE TRUSS. THE TRUSS MANUFACTURER SHALL REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL MOVEMENT OF THE TRUSS MEMBER. THE TRUSS MANUFACTURER IS RESPONSIBLE BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT DESIGNER FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.



SILL PLATE SPANS FOR 10'-0" WALL HEIGHT					
DESIGN WIND SPEED	MAX. SPANS FOR SPF #2				BASED ON WFCM TABLE A-3.238
	(1) 2x4	(2) 2x4	(1) 2x6	(2) 2x6	
130 MPH EXP. C	5'-2"	7'-9"	7'-7"	11'-3"	FOR OTHER WALL HEIGHTS (H) SILL SPAN SHALL BE DIVIDED BY (H/10)



DESIGN CRITERIA & LOADS:	
BUILDING CODE	7TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2020)
CODE FOR DESIGN LOADS	ASCE 7-16
WIND LOADS	
BASIC WIND SPEED (ASCE 7-16, 3S GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7.44 DEGREES
MEAN ROOF HEIGHT	30 FT
C&S DESIGN PRESSURES	SEE TABLE
FLOOR LOADING	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
ROOF LOADING	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO 12:12	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
SOIL BEARING CAPACITY	1500 PSF
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE

COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C)			
EFFECTIVE WIND AREA (FT ²)	ZONE 4 INTERIOR	ZONE 5 END 4 FROM ALL OUTSIDE CORNER	
0 - 20	+25.6(V _{asd}) -27.8(V _{asd})	+25.6(V _{asd})	-34.2(V _{asd})
0 - 20	+42.6(V _{ult}) -46.2(V _{ult})	+42.6(V _{ult})	-57(V _{ult})

GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C)		
9x7 GARAGE DOOR	+22.6(V _{asd})	-25.5(V _{asd})
16x7 GARAGE DOOR	+21.7(V _{asd})	-24.1(V _{asd})

PROJECT ADDRESS:
CR 240
Columbia County, FL

DIMENSIONS:
Stated dimensions supercede scalar
dimensions. Refer all questions to
Mark Disosway, P.E. for resolution.
Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

A circular red ink stamp is overlaid on the bottom right of the page. The text around the perimeter reads "MARK DISOWAY III" at the top and "STATE OF FLORIDA" at the bottom. In the center, it says "LICENSE" and "No. 53915". There is a handwritten signature in blue ink across the seal.

Thursday, December 9, 2021

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JOB NJMBER:
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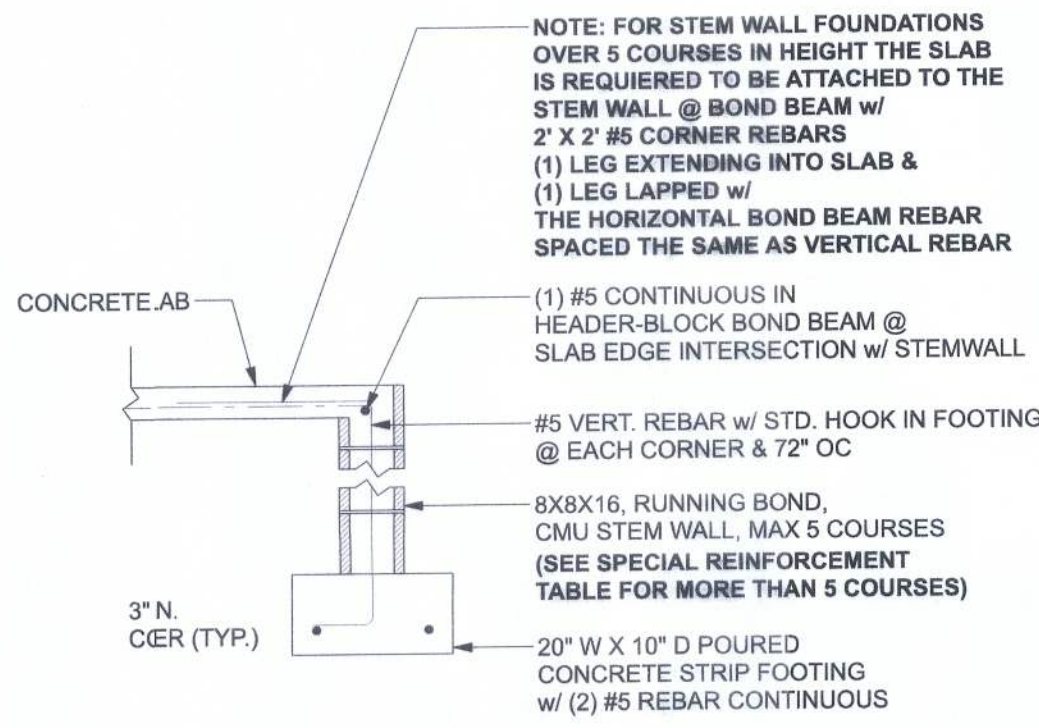
S-1
OF 33HEE

TALL STEM WALL TABLE:							
The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16" OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.							
STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

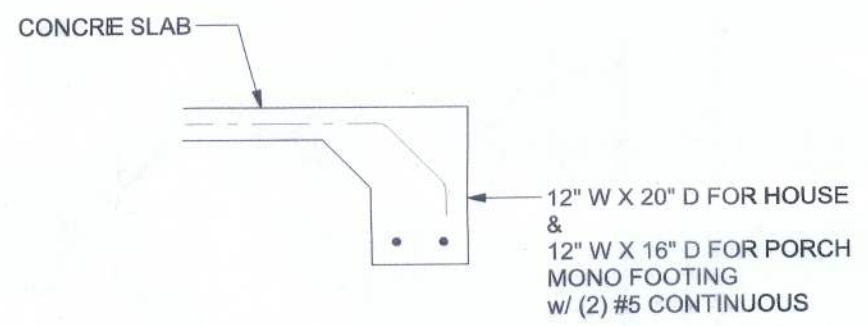
MASONRY NOTE:
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 8/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI 530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls Fm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 40, Fy = 40 ksi, Lap splices min 40 bar dia, (25" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A225, Class C80, 0.60 oz/ft2 or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

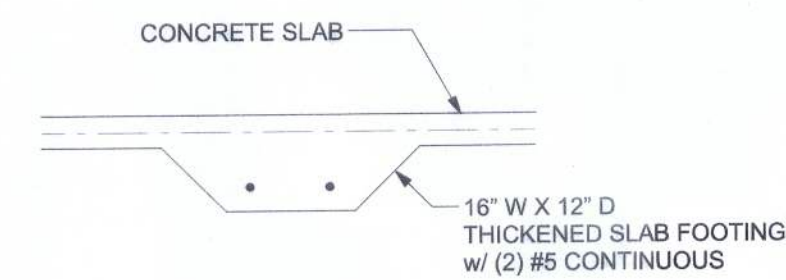
BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL



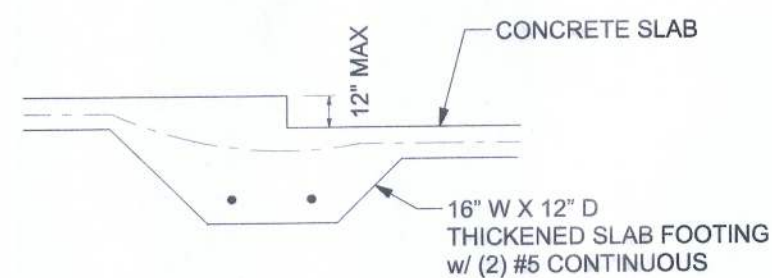
STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



OPTIONAL MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"



INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"

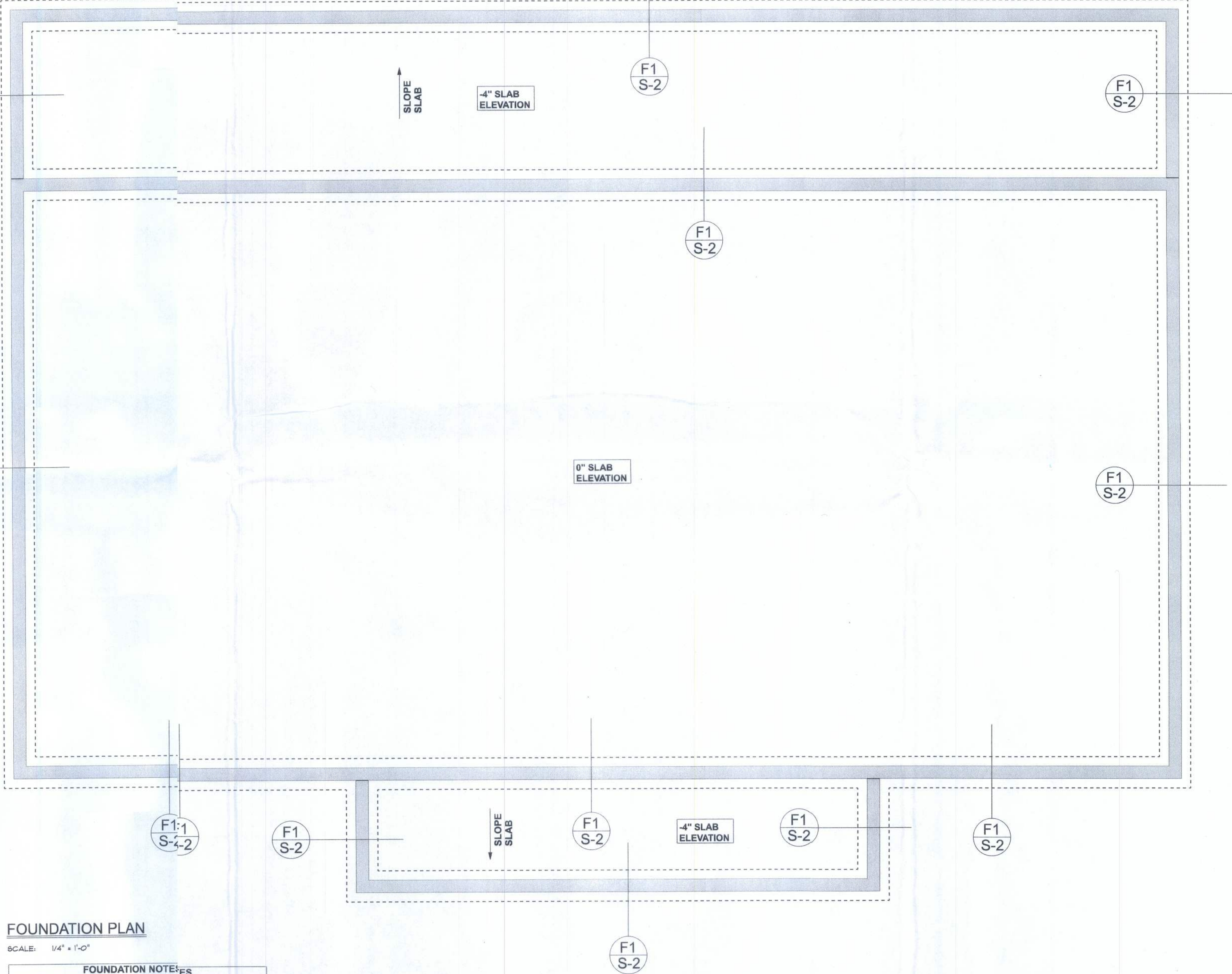


INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"

FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

FOUNDATION NOTES	
FN-1	DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES, STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
FN-2	CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.
FN-3	THE SLAB SHALL BE 4" CONCRETE SLAB REINFORCED w/ (5#5) 4" x 4" WELDED WIRE MESH PLACED ON CHAIRS @ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER w/ 6" LAPS SEALED w/ 6-MIL POLY TAPE OVER TERMITES-TREATED & ALEAD w/ & COMPACTED FILL



B&B Homes
Clint & Saret Bulard Doe.
PROJECT ADDRESS:
Gainesville, FL
Columbia County, FL

DIMENSIONS:
Stated dimensions are scaled dimensions. Refer all questions to Mark Disosway, P.E. to resolution. Do not proceed without verification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915



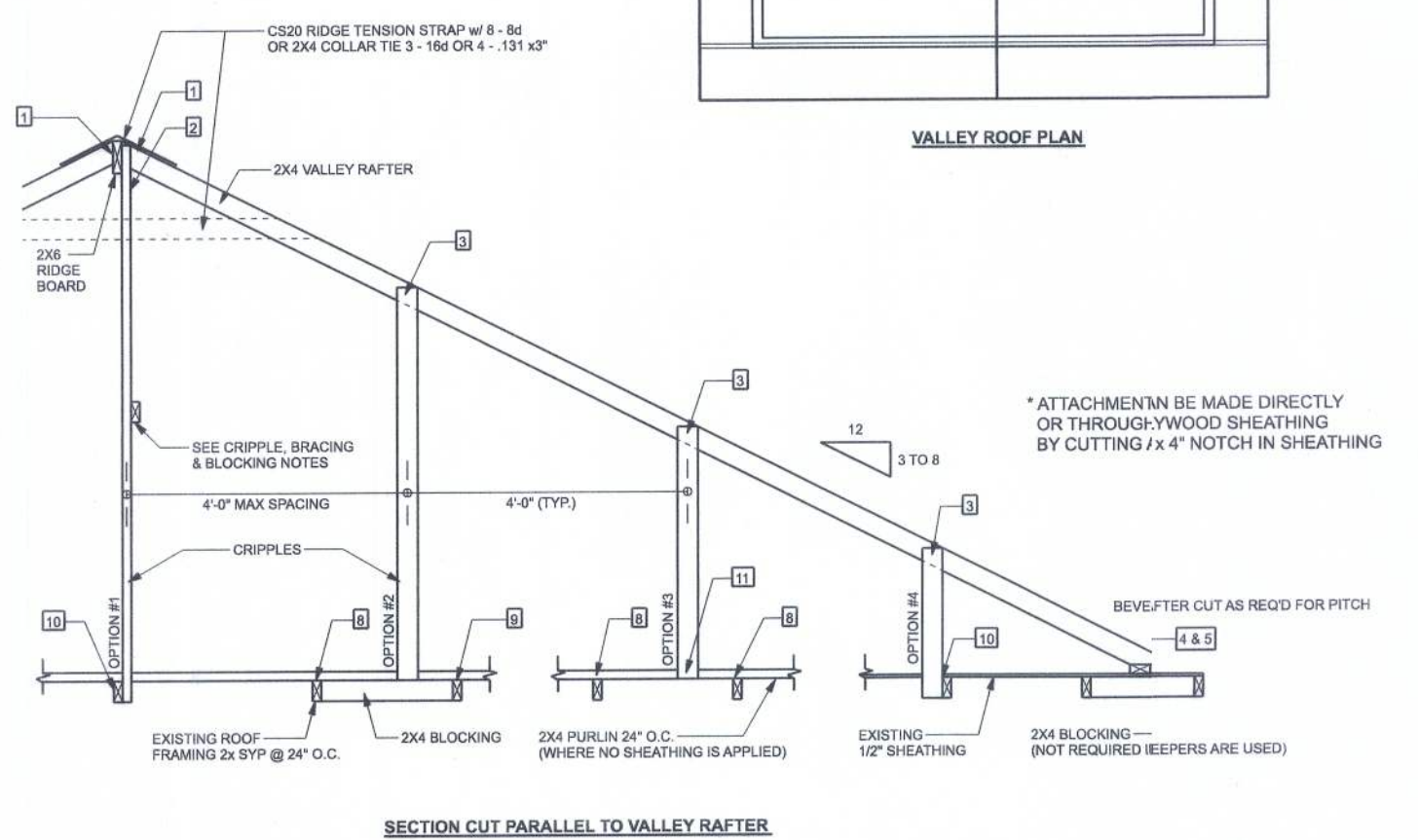
Thursday, December 9, 2021

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JOB NUMBER:
211620

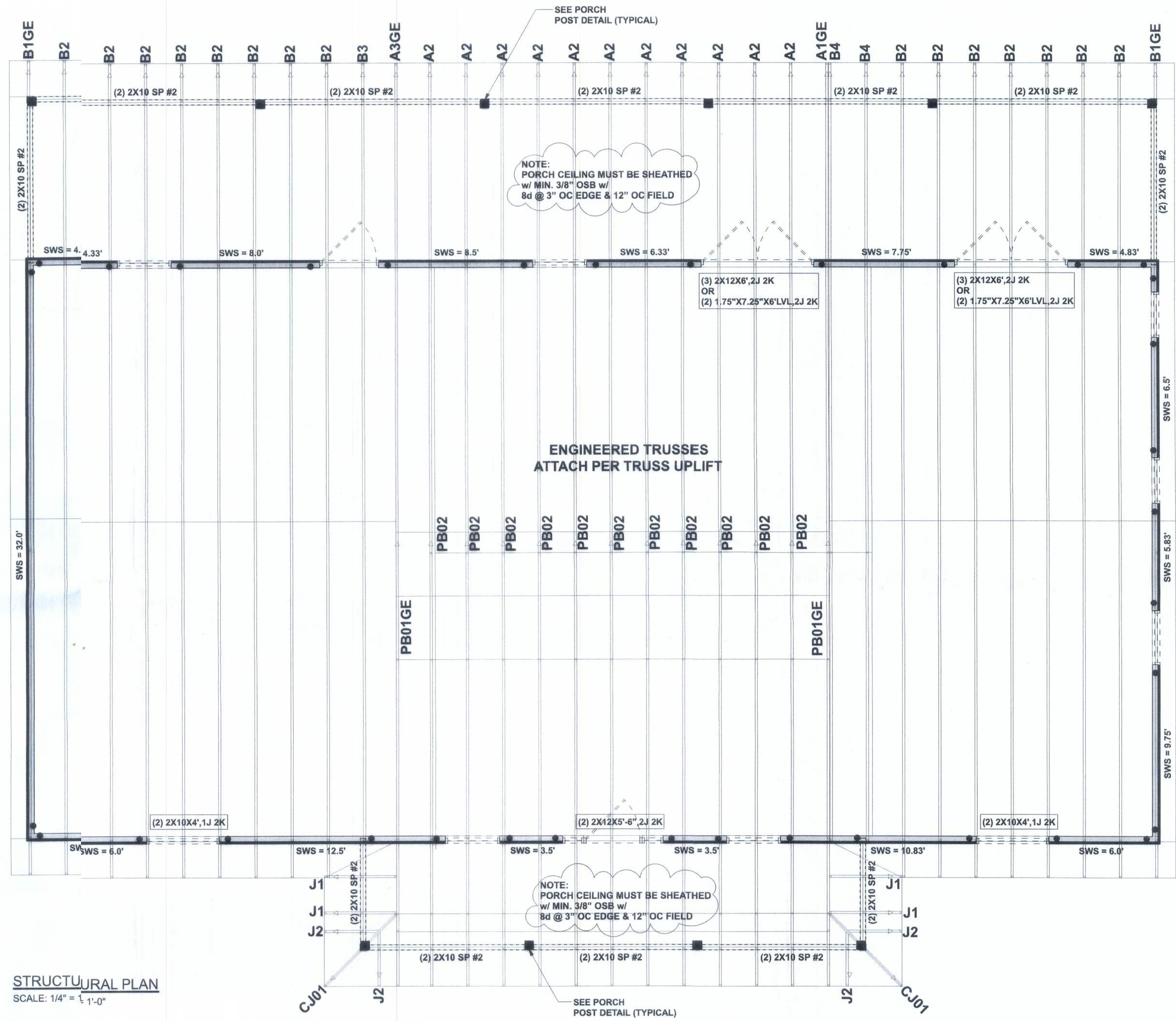
S-2
OF 3 SHEETS

LUMBER SIZE & GRADE MINIMUM REQUIREMENTS	
TRUSS BOARD	2X8 SYP #2
RAFTER SPANS 20' 0" OR LESS	2X4 SYP #2
PURLIN LATERAL BRACING	2X4 SYP #2
SLEEPERS	2X (WIDTH OF RAFTER SEAT CUT) SYP #2 OR 2X (SPAN) SYP #2
CRIPPLES & BLOCKING	2X4 SYP #2 OR BETTER
TRUSS BELOW	SEE TRUSS DESIGN - SOUTHERN PINE MATERIAL



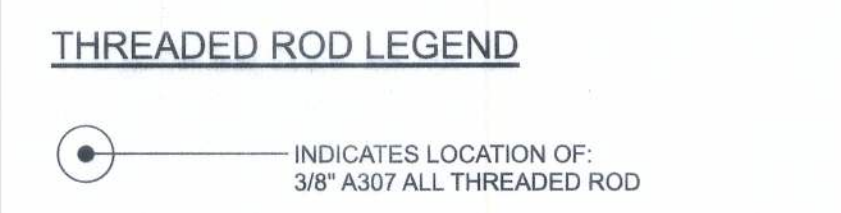
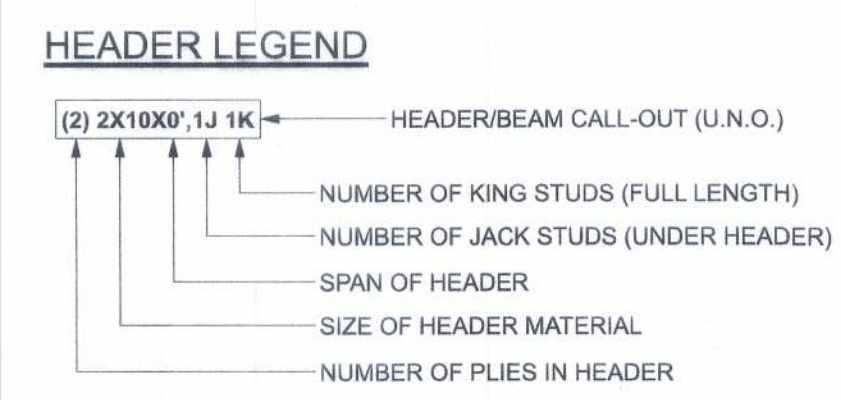
ROOF OVER FRAMING & BRACING DETAIL
SCALE: N.T.S.

VALLEY ROOF PLAN MEMBER LEGEND	
---	TRUSS
---	TRUSS UNDER VALLEY FRAMING
---	VALLEY RAFTER OR RIDGE
---	CRIPPLE
CRIPPLES 4" O.C. FOR 20 psf (TL) AND 10 psf (TD) (TYP. SHINGLE ROOF) MAX	
CONNECTION REQUIREMENT NOTES	
1. 2X4 RAFTERS TO RIDGE	3-16d OR 6-131 x 3" TOE NAILS
2. CRIPPLE TO RIDGE	3-16d OR 6-131 x 3" FACE NAILS
3. CRIPPLE TO RAFTERS	3-16d OR 6-131 x 3" FACE NAILS
4. RAFTER TO SLEEPER OR BLOCKING	4-16d OR 12-131 x 3" TOE NAILS
5. SLEEPER TO TRUSS	4-16d OR 8-131 x 3" FACE NAILS EACH TRUSS
6. RIDGE BOARD TO RIDGE BLOCK	3-16d OR 6-131 x 3" TOE NAILS
7. RIDGE BOARD TO TRUSS	3-16d OR 6-131 x 3" TOE NAILS
8. PURLIN TO TRUSS (TYP.)	3-16d OR 6-131 x 3" NAILS
9. PURLIN TO TRUSS (IF CRIPPLE IS ATTACHED TO PURLIN)	4-16d OR 6-131 x 3" NAILS
10. TRUSS TO BLOCKING	3-16d OR 6-131 x 3" END NAILS
11. CRIPPLE TO TRUSS	3-16d OR 6-131 x 3" FACE NAILS
12. CRIPPLE TO PURLIN	3-16d OR 6-131 x 3" FACE NAILS
GENERAL NOTES	
MAXIMUM RAFTER SPAN: 8' 0" FOR 2X4 SYP #2 OR SYP #2; 10' 0" FOR 2X6 SYP #2 OR SYP #2.	
MAXIMUM ROOF AREA PER SUPPORT: 1000 SQ. FT. FOR 2X4 SYP #2; 1400 SQ. FT. FOR 2X6 SYP #2. (EXAMPLE: 4' 0" O.C. X 4' 0" SPAN = 1600 SQ. FT. X 4' 0" SPAN = 1600 SQ. FT.)	
PURLIN REQUIRED 2" O.C. IF EXISTING SHEATHING IS REMOVED. PURLIN SHOULD OVERLAP SHEATHING ONE TRUSS SPACING MINIMUM. IN CASES THAT THIS IS IMPRACTICAL, OVERLAP SHEATHING A MINIMUM OF 8" AND NAIL UPWARDS THROUGH SHEATHING INTO PURLIN WITH A MINIMUM OF 8-16 COMMON WIRE NAILS.	
THIS DRAWING APPLIES TO VALLEYS WITH THE FOLLOWING CONDITIONS:	
- SPAN (DISTANCE BETWEEN HEBL) 12' 0" OR LESS	
- MAXIMUM WIND SPEED: 130 MPH	
- MAXIMUM MEAN ROOF HEIGHT: 30 FEET	
- MAXIMUM TOTAL LOADING: 60 psf	
- MEETS PRE-SPANNED 7-10 WIND REQUIREMENTS	
- EXPOSURE CATEGORY "C" (1-10, R01-10)	
- ENCLOSED BUILDING	
CRIPPLE, BRACING, & BLOCKING NOTES	
2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 6' 0" TO 10' 0" LONG NAILED W/ 2-16d NAILS OR 2X4 "T" OR 2X6 BRACE NAILED TO VAL EDEGE OF CRIPPLE WITH 8-16d NAILS @ 4" O.C. "T" OR 2X6 MUST BE 8' 0" OR LONGER. CRIPPLES OVER 10' 0" LONG REQUIRE TWO CLB'S ON BOTH FACES W/ "T" OR 2X6. USE STRESS GRADUALLY CURVED LUMBER ON COMMON NAILS.	
NARROW EDGE OF CRIPPLE CAN FACE RIDGE OR RAFTER, AS LONG AS THE PROPER NUMBER OF NAILS ARE INSTALLED INTO RIDGE BOARD.	
INSTALL BLOCKING UNDER RAFTER IF SLEEPERS ARE NOT USED.	
INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FALL BETWEEN LOWER TRUSS TOP CHORDS AND LATERAL BRACING IS NOT USED.	
APPLY ALL NAILING IN ACCORDANCE TO NDS-1997 SECTION 12. NAILS ARE COMMON WIRE NAILS UNLESS NOTED OTHERWISE.	



STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

- STRUCTURAL PLAN NOTES**
- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SP #2 (U.N.O.)
 - SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
 - SN-3 USE ONE JACK STUD GIRDER SUPPORT PER 2500 LB LOAD
 - SN-4 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
 - SN-5 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE



ACTUAL vs REQUIRED SHEARWALL		
	TRANSVERSE	LONGITUDINAL
ACTUAL	18928 LBF	19696 LBF
REQUIRED	16492 LBF	10635 LBF

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER, MAYO TRUSS CO. JOB #BULLARD

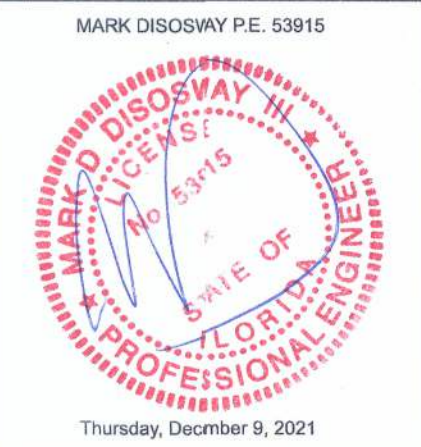
B&B Homes
Clint & Starlett Bullard Res.
PROJECT ADDRESS:
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JOB NUMBER:
211420
S-3
OF 3 SHEETS