

54'-2"

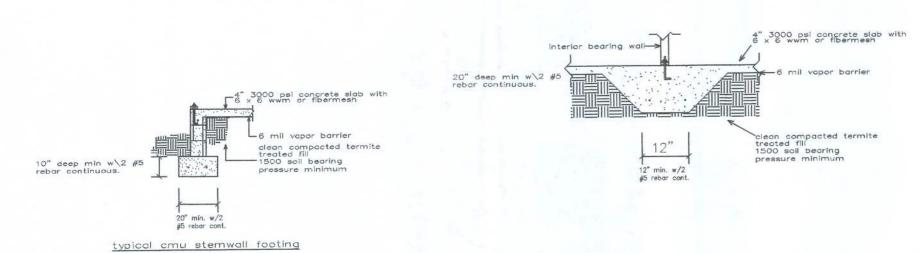
Stoucture! eboo

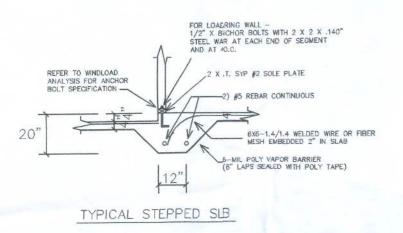
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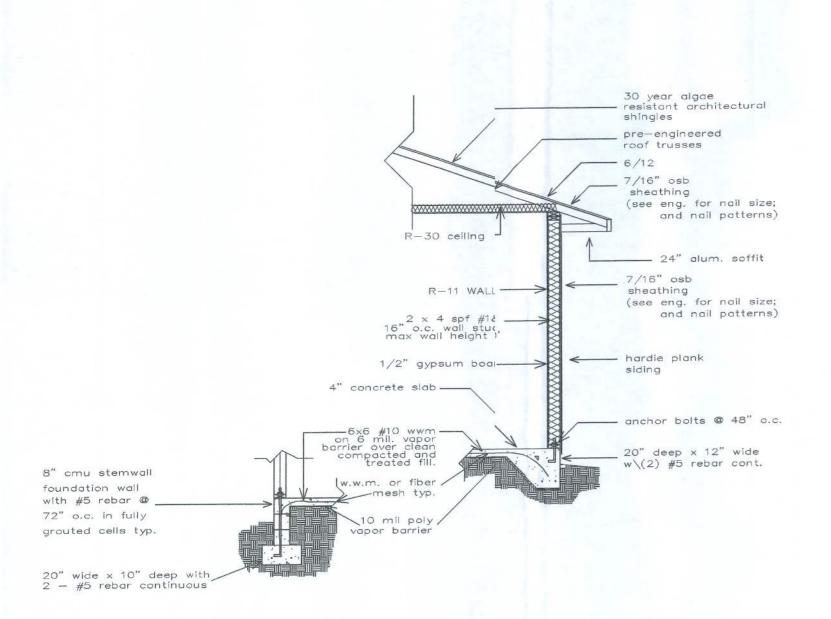
FOUNDATION NOTES 4" THICK SLAB WITH FIBER MESH OR 6 x6 W.W.M. OVER 6 MIL VAPOR BARRIER ON CLEAN TERMITE TREATED SO. FIBER MESH MAY BE USED. ALL STEEL MUST BE GRADE 40 MIN. 150(PSF SOIL BEARING PRESSURE MIN. 8" C.M.U. STEMWALL WITH (1) #5 REBAR ERTICAL FILLED CELL W/ CONCRETE AT ALL CORNERS AND 6'.C. MAX. SPACING. 10" DEEP X 20" WIDE WITH (2) 5 REBARCONT. STEMWALL FOOTING. THICKEN EDGE OF MONOLITHIC SLAB TO 1" WIDE X 20" DEEP WITH (2) #5 REBAR CONTINUOUS. Contractors to verify all dimensions, code and designs to comply with authorities havingjurisdiction. CODE STATEMENT: CODE REQUIREMENTS IN EFFECT AT THE ME OF DESIGN: 2020 FLORIDA RESIDENTIAL BUILDING COD

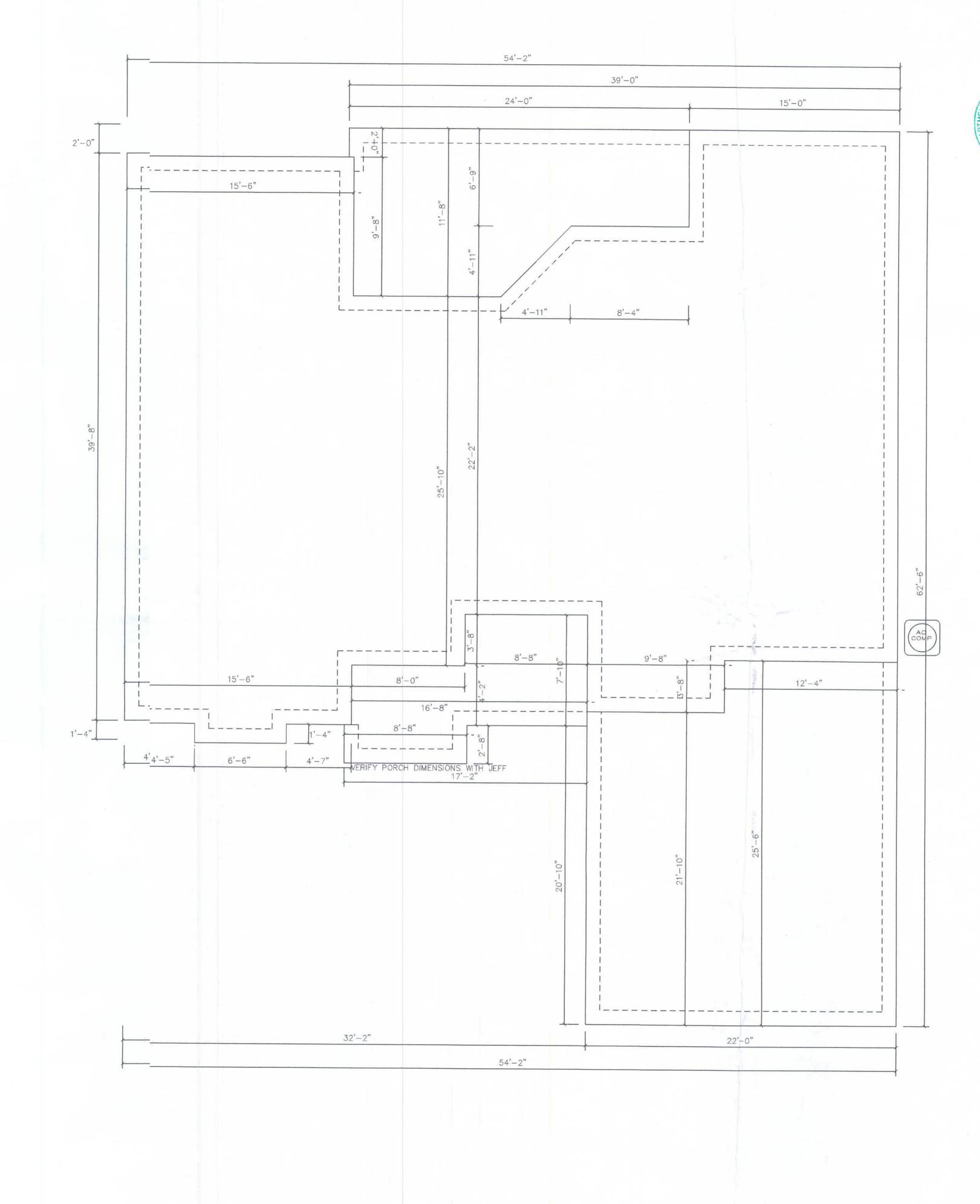
#". 3000 psj. concrete slab wi all steel must be grade 40 minimum

typical monolithic footing









4" 3000 psi concrete slab with 6 x 6 wwm or fibermesh

typical cmu stemwall footing FLOADBEARING WALL — 1 X 8" ANCHOR BOLTS WITH 2 X 2 X .140" S. WASHER AT EACH END OF SEGMENT AAT 48" O.C. (2) #5 REBAR CONTINUOUS

TYPICAL STEPPEISLAB

R-30 ceilir —

2 x 4 spf1&2 -16" o.c. wall uds max wall heig 10'

1/2" gypsum lard —

4" concrete slab

w.w.m. or fiber / mesh typ.

8" cmu stemwall

foundation wall

with #5 rebar @ _ 72" o.c. in fully grouted cells typ.

20" wide x 10" deep with 2 - #5 rebar continuous

R-11 V_LS ---

30 year algae — resistant architectural shingles

(see eng. for nail size; and nail patterns)

(see eng. for nail size; and nail patterns)

—— anchor bolts @ 48" o.c.

20" deep x 12" wide w\(2) #5 rebar cont.

__ pre-engineered roof trusses

- 6/12 7/16" osb sheathing

hardie plank

FOUNDATION NOTES

all steel must be grade

8" C.M.U. STEMWALL WITH (1) #5 REBR VERTICAL FILLED CELL W/ CONCRETE AT ALL CORNERS AND; O.C. MAX. SPACING.

10" DEEP X 20" WIDE WITH (2) 5 REAR CONT. STEMWALL FOOTING.

Contractors to verify all dimensions, odes and designs to comply with authorities hang jurisdiction.

typical monolithic footing

CODE STATEMENT: CODE REQUIREMENTS IN EFFECT AT TE TIME OF DESIGN: 2020 FLORIDA RESIDENTIAL BUILDING ODE

Verify all footings with contractor and truss company's truss layout.

6 mil vapor barrier

4" 3000 psi concrete slab with

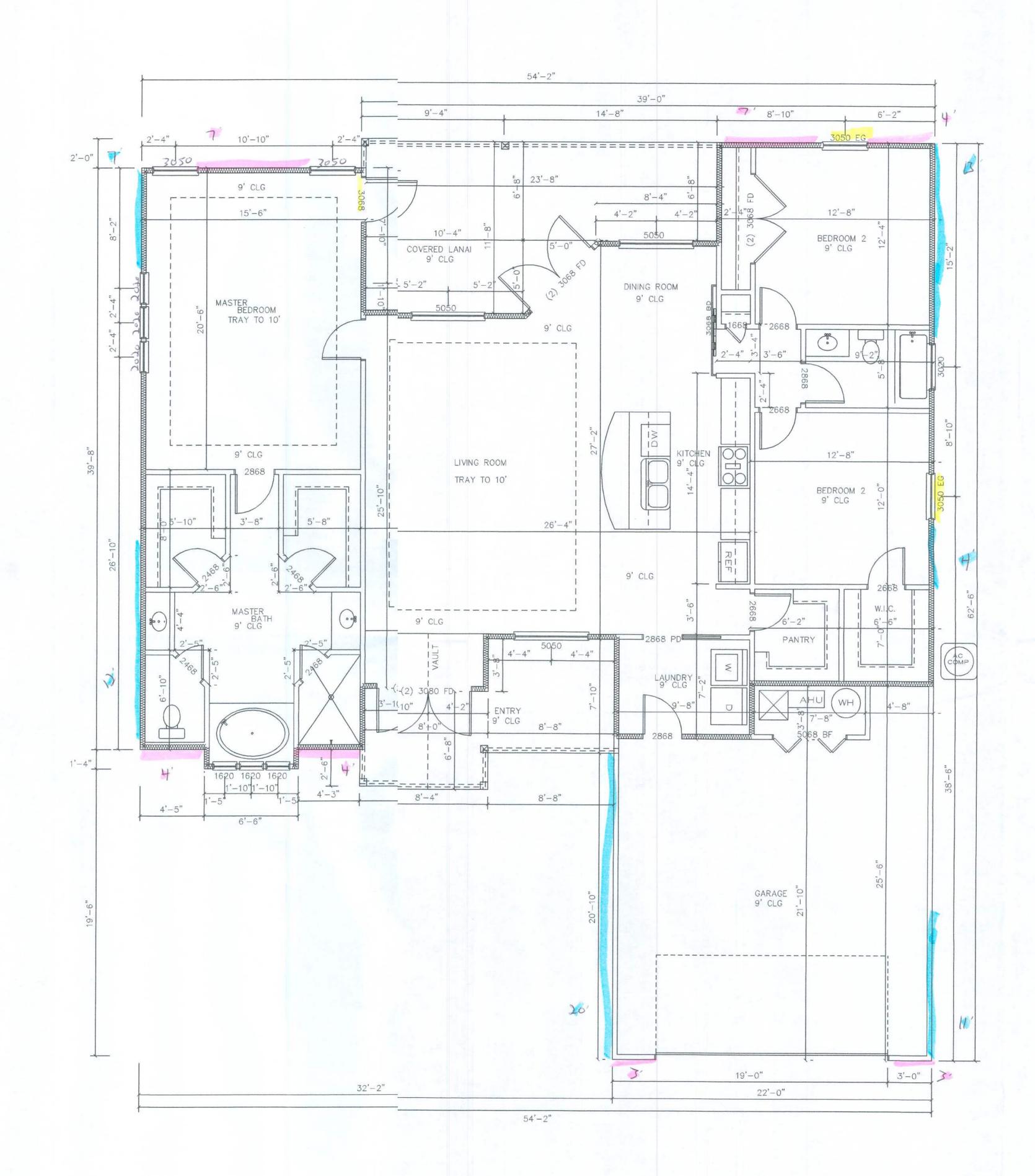
-6 mil vapor barrier

4"- 3000 psi concrete b with 6 x 6 wwm or fiberme

4" THICK SLAB WITH FIBER MESH OR × 6 W.W.M. OVER 6 MIL VAPOR BARRIER ON CLEAN TERMITE TREATEDSOIL. FIBER MESH MAY BE USED. ALL STEEL MUST BE GRADE 40 MIN. 300 PSF SOIL BEARING PRESSURE MIN. THICKEN EDGE OF MONOLITHIC SLAB 1 12" WIDE X 20" DEEP WITH (2) #5 REBAR CONTINUOUS.

M N N 20

Code



FRS-67' Long-32'

LIVING AREA FRONT ENTRY COVERED LANAI GARAGE

WHITE STATION E, FLORIDA

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STRUCTURAL NOTES

FOUNDATION

SOIL TO BE COMPACTED TO AT LEAST 95% OF MAX. DRY DENSITY AS DETERMINED BY ASTM-1557

CAST IN PLACE CONCRETE

1. ALL CONCRETE SHALL HAVE A MIN. COMPRESSIVE STRENGTH AT 28 DAYS OF 3000 P.S.I. SLUMP OF 4" AND HAVE 2 TO 4% AIR ENTRAINMENT WITH A CEMENT / WATER RATIO OF 0.58 PERCENT.

2. ALL REINFORCING STEEL SHALL BE NEW DOMESTIC DEFORMED BILLET STEEL CONFORMING TO ASTM-615 GRADE 40. 3. WELDED WIRE MESH SHALL CONFORM TO ASTM A-185, WWM SHALL BE LAPPED AT LEAST 8". AND CONTAIN AT LEAST ONE CROSS WIRE WITHIN THE 8". FIBER MESH MAY BE USED IN SLAB.

4. HOOKS SHALL BE PROVIDED AT DISCONTINUED ENDS OF ALL TOP BARS OF BEAMS.

5. HORIZONTAL FOOTING BARS SHALL HAVE A 1'-0" HOOK LENGTH OF CORNER BARS WITH A MIN. 25" LAP PROVIDED. 6. 25" MIN. LAP SPLICES ON ALL REBAR. ALL REBAR TO BE GRADE 40. 7. 3" MIN. CONCRETE COVERAGE WHEN EXPOSED TO EARTH OR 1-1/2" TO FORM.

MASONRY WALL CONSTRUCTION

1. HOLLOW LOAD BEARING UNITS SHALL BE NORMAL WEIGHT, GRADE N, TYPE 2, CONFORMING TO ASTM C90, WITH A MIN. NET COMPRESSIVE STRENGTH OF 1900 PSI

2. MORTAR SHALL BE TYPE "M" OR "S" CONFORMING TO ASM C270 3, COARSE GROUT SHALL CONFORM TO ASTM C476 WITH A MAX. AGGREGATE SIZE OF 3/8" AND MIN. COMPRESSIVE STRENGTH OF 3000 PSI SLUMP 8" TO 11".

4. VERTICAL REINFORCEMENT SPACING IS NOTED ON THIS SHEET AND TO BE FULLY GROUTED CELLS.

5. VERTICAL REINFORCEMENT SHALL BE HELD IN POSITION AT THE TOP AND BOTTOM AND AT MAX. SPACING OF 192 BAR DIAMETERS, REINFORCEMENT SHALL BE PLACED IN CENTER OF THE MASONRY CELL TYPICAL UNLESS OTHERWISE NOTED.

FLORIDA BUILDING CODES 2020 EDITION
REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318) IN TEST EDITION
SPECIFICATIONS FOR STRUCTURAL CONCRETE BUILDING (ACI 301) IN TEST EDITION
NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION IN TEST EDITION
APA PLYWOOD DESIGN SPECIFICATION.

RESIDENTIAL FLOOR, UNLESS OTHERWISE STATED

THESE DRAWINGS PREPARED USING FBC 2020 AND ASCE 7-16
CONCRETE STRENGTH ALL CONCRETE UNLESS OTHERWISE INDICATED 3000PSI @ 28 AYS.
REINFORCING WELDED WIRE FABRIC SHALL CONFORM TO ASTM A 185
ALL REINFORCING BARS, TIES AND STIRRUPS ASTM A 615
STRUCTURAL STEEL ALL BOLTS CAST IN CONCRETE ASTM 36 OR ASTM 307

SHEATING
ROOF DECKING; EXTERIOR CDX PLYWOOD OR OSB WALL SHEATING; EXTERIOR CDX PLYWOOD OR OSB SOIL BEARING VALUE ALLOWABLE SOIL BEARING PRESSURE AFTER COMPACTION 1500PSF
SEE DRAWINGS FOR SPECIAL CONCENTRATED LOADS AS
SPECIFIED. IF SOIL CONDITIONS IN THIS PROJECT DOES NOT
MEET OR EXCEED THE CAPACITY, THE CONTRACTOR
WILL CONTACT SCHAFER ENGINEERING PRIOR TO FOUNDATION
POUR FOR VERIFICATION OF FOUNDATION DESIGN.
SOIL TO BE COMPACTED TO AT LEAST 95% OF MAX DRY DENSITY
AS DETERMINED BY ASTM—1557 (MODIFIED PROCTOR) AS DETERMINED BY ASTM-1557 (MODIFIED PROCTOR)

WOOD CONSTRUCTION

1.ALL WOOD CONST. SHALL CONFORM TO THE NDS

2. ALL EXTERIOR WOOD STUD WALLS, BEARING WALLS,
SHEARWALLS AND MISC. STRUCTURAL WOOD FRAMING
MEMBERS(I.E. BLOCKING OR GABLE END BRACING)
SHALL BE EITHER SOUTHERN PINE OR S.P.F. NUMBER 2 DEN. GRADE OR BETTER SHALL BE USED REGARDLESS OF SPECIES.

PREFABRICATED WOOD TRUSSES

1. ALL PREFABRICATED TRUSSES SHALL BE SECURELY FASTENED TO THEIR SUPPIORTING WALLS OR BEAMS AS PER TRUSS ENG REQ.

2. PREFABRICATED WOOD TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THE LATEST EDITION OF THE NDS AS RECOMMENDED BY THE NFPA.

3. TRUSS MEMBERS AND CONNECTIONS SHALL BE PROPORTIONED (WITH A MAX. ALLOWABLE STRESS INCREASE FOR ALL LOAD DURATIONS OF TPI RECOMMENDATIONS). DURATIONS OF TPI RECOMMENDATIONS).

4. BRIDGING FOR PRE—ENGINEERED TRUSSES SHALL BE SPECIFIED BY THE TRUSS MANF.

5. TRUSS ELEVATIONS AND SECTIONS ARE FOR GENERAL CONFIGURATION OF TRUSSES ONLY.

6. DESIGN SPECIFICATION FOR LIGHTWEIGHT METAL PLATE CONNECTED WOOD TRUSSES PER TPI.

7. PRE—ENGINEERED WOOD TRUSSES SHALL BE DESIGNED BY THE MANF. IN ACCORDANCE WITH SPECIFIED LOADS AND GOVERNING CODES.

8. THE TRUSS MANF. SHALL DETERMINE ALL SPANS BEARING POINT 8. THE TRUSS MANF. SHALL DETERMINE ALL SPANS, BEARING POINTS AND SIMILAR CONDITIONS. TRUSS SHOP DRAWINGS SHALL SHOW ALL TRUSSES, ALL BRACING MEMBERS, AND ALL TRUSS TO TRUSS CONDITIONS. UPLIFT CONNECTORS

1. UPLIFT CONNECTORS SUCH AS HURRICANE CLIPS, TRUSS ANCHORS AND ANCHOR BOLTS ARE REQUIRED ON MEMBERS IN WALLS THAT ARE EXPOSED TO UPLIFT FORCES. INTERIOR LOAD BEARING WALLS ARE NOT ALWAYS EXPOSED TO UPLIFT FORCES; THE MEMBERS OF THESE WALLS MAY NOT NEED TO HAVE CONNECTORS APPLIED, CONSULT THE TRUSS MANF. FOR THE LOCATION OF THESE WALLS.

2. THE CAPACITIES OF THE TRUSS CONNECTORS SPECIFIED BY TRUSS MANF. SHALL BE VERIFIED BY THE CONTRACTOR TO EXCEED THE LOADS IN THE SIGNED AND SEALED TRUSS ENGINEERING.

1. MISSED (J) BOLTS FOR WOOD BEARING WALLS MAY BE SUBSTITUTED WITH 1/2" X 10" WITH 7" EMBEDMENT USING AN APPROVED EPOXY FOLLOWING ALL MANF. RECCOMMENDATIONS. 2. HURRICANE STRAPS MAY BE SUBSTITUTED WITH A STRAP OF EQUAL OR GREATER VALUES.

1. CONTRACTOR TO VERIFIY ALL MEASUREMENTS AND DEMENSIONS BEFORE CONSTRUCTION OF THESE DRAWINGS BEGIN. 2. THIS STRUCTURE TO BE BUILT IN ACCORDENCE WITH F.B.C. 2020.

3. ANY DEFECTS OR ERRORS FOUND IN THESE PLANS AFTER THE START OF THE CONSTUCTION BECOME THE SOLE RESPONSIBILITY OF THE CONTRACTOR. 4. TRUSS MANF, TO ENGINEER TRUSSES TO WITHSTAND 135 MPH WIND LOAD AS PER 2020 F.B.C.

5. GRADE REQUIREMENTS MAY VARY ACCORDING TO SOIL CONDITIONS. 6. WINDOWS TO BE INSTALLED TO MANF, SPECS. TO MEET WINDLOADS AS PER 2020 F.B.C.

4" THICK SLAB WITH 6" X 6" 10/10 GA W.W.M. OVER 6 MIL VAPOR BARRIER ON CLEAN TERMITE TREATED SOIL. FIBER MESH MAY BE USED. 8" C.M.U. STEMWALL WITH (1) #5 REBAR VERTICAL FILLED CELL W/ CONCRETE AT ALL CORNERS AND 6' O.C. MAX. SPACING.

10" DEEP X 20" WIDE WITH (2) 5 REBAR CONT. STEMWALL FOOTING. THICKEN EDGE OF MONOLITHIC SLAB TO 12" WIDE X 20" DEEP WITH

NOTICE TO CONTRACTOR
IT IS THE INTENT OF THE DESIGNER THAT THESE PLANS
ARE ACCURATE AND ARE CLEAR ENOUGH FOR THE STATE LICENSED
CONTRACTOR TO CONSTRUCT THIS PROJECT, IN THE EVENT THAT
SOMETHING IS UNCLEAR OR NEEDS CLARIFICATION STOP AND
CALL THE DESIGNER. IT IS THE RESPONSIBILITY OF THE STATE
LICENSED CONTRACTOR THAT IS CONSTRUCTING THIS PROJECT TO
REVIEW THESE PLANS BEFORE CONSTRUCTION AND IF NEEDED
COORDINATE WITH THE DESIGNER OF ANY CORRECTIONS TO BE
MADE BEFORE CONSTRUCTION BEGINS. GENERAL NOTES

THE FOLLOWING SHALL COMPLY WITH THE F.B.C. PORCHES AND BALCONIES SECTION R312 EGRESS WINDOWS SECTION R310 R310.1.1 GARAGE SEPERATION R309 R309.2 1. ALL OPENINGS SHALL COMPLY WITH F.B.C. AS STATED BELOW ATTACHMENT OF WINDOWS, DOORS, SLIDING GLASS DOORS, AND! OVER HEAD GARAGE DOORS ARE TO BE DELIGATED TO THE MANF. OF THESE ITEMS. THE MANF. OF THESE ITEMS WILL SUMIT ATTACHMENTS TO CONTRACTOR OF RECORD.

> ROOF VENTING CALCULATIONS SQ FT TOTAL 2692 SF /600 SF

SF OF VENT AREA REQ. 4.5 SF NUMBER OF VENTS REQ. 6

L-----------______ __________ _____ !----TIE-DIDOWN TABLES HEAD DER STRAPPING Uplift Lt Lbs Top Connector Rating Lbs Bottom Connector Rating Lbs LSTA19 635 LSTA12 795 2-H3 640 to 1265 65 LSTA18 1110 LTT19 1305 to 1750 50 2-LSTA12 1810 LTT20 1750 o 2530,30 2-LSTA18 2530 HD2A-2.5 2165 to 2865,65 3-LSTA18 3255 HD2A-3.5 o 3700,00 3-LSTA24 3880 HD5A-3 3130 Total the uplift for each truss sitting on the header and divide by 2 to determine uplift ion the header. Use proper bolt anchors sufficient to support required loads. ______ TRUSSSES \ GIRDERS Uplift Lb Lbs Top Connector Bottom Connector Rating Lbs to 535 5 to 1015 15 to 1215 15 H2.5A H10A TS22 LTT19 1305 2-TS22 LTT20 1750 2-TS22 2775 3-TS22 4010 2-MST37 5250 1to 9660 60 2-MST60 HD10A 9540 Two 12d bearing 2d common toenials are required per truss for each it is the ground point into top plate. I load pat the contractors responsibility to provide a continuous 2dth from truss to foundation. RATING BOTTOM RATING LBS BEAM SE SEATS LSTA18 1110 LTT19 1305

ABU44 2300

2220

2-LSTA18

1. Simps:
For nc
For nc
multip nailing into spruce members,
tiply table values by .86 2. See tr truss engineering for anchor uplift values.

3. This s replact schedule is not meant to be a replact accement to the specified values of manufactures values.

| This s replact schedule is not meant to be a replact schedule in the specified values of manufactures values.

CONTRA SANTA SPRINGS

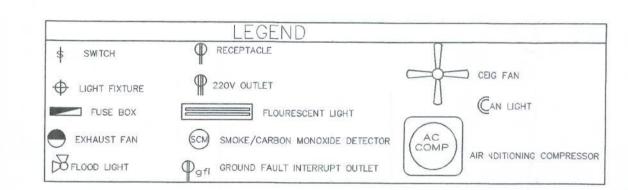
DWC 30 NE HIGH SF (386)

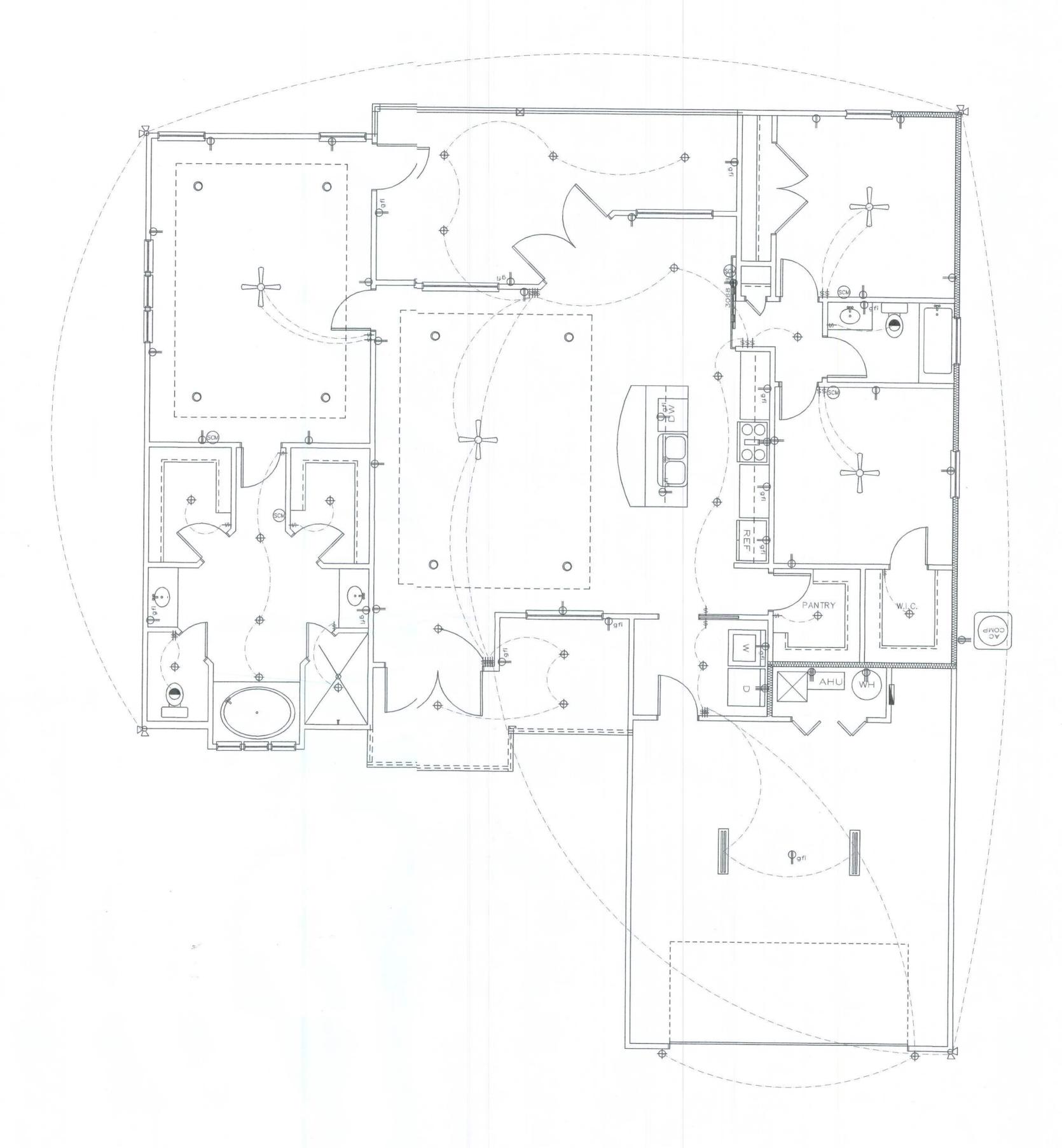
THIS ELECTRICAL PLAN IS A SCHEMATIC WITH SUGESTED SWITCH, RECEPTACLE AND LIGHT FIXTURE LOCATINS, DUE TO VARYING LOCAL AND STATE CODES, REGULADNS, AND STATUTES. IT IS THE RESPONSIBILITY OF THE OWER AND/OR CONTRACTOR TO COMPLY WITH ALL LOCAL AND TATE CODES, REGULATIONS AND STATUTES.

ELECTRICAL NOTES:

INSTALLATION SHALL BE PER 2017 NAT'L. ELECTIC CODE.

NOTE: CONTRACTORS TO VERIFY ALL DIMENSIONS, CDES AND STRUCTURAL DESIGNS TO COMPLY WITH .LL AUTHORITIES HAVING JURISDICTION.





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IT IS THE RESPONSIBILITY OF THE STATE LICENSED CONTRACTORS TO VERIFY ALL DIMENSIONS, CODES AND STRUCTRUAL DESIGNS TO COMPLY WITH ALL AUTHORITIES HAVING JURISDICTIONS.