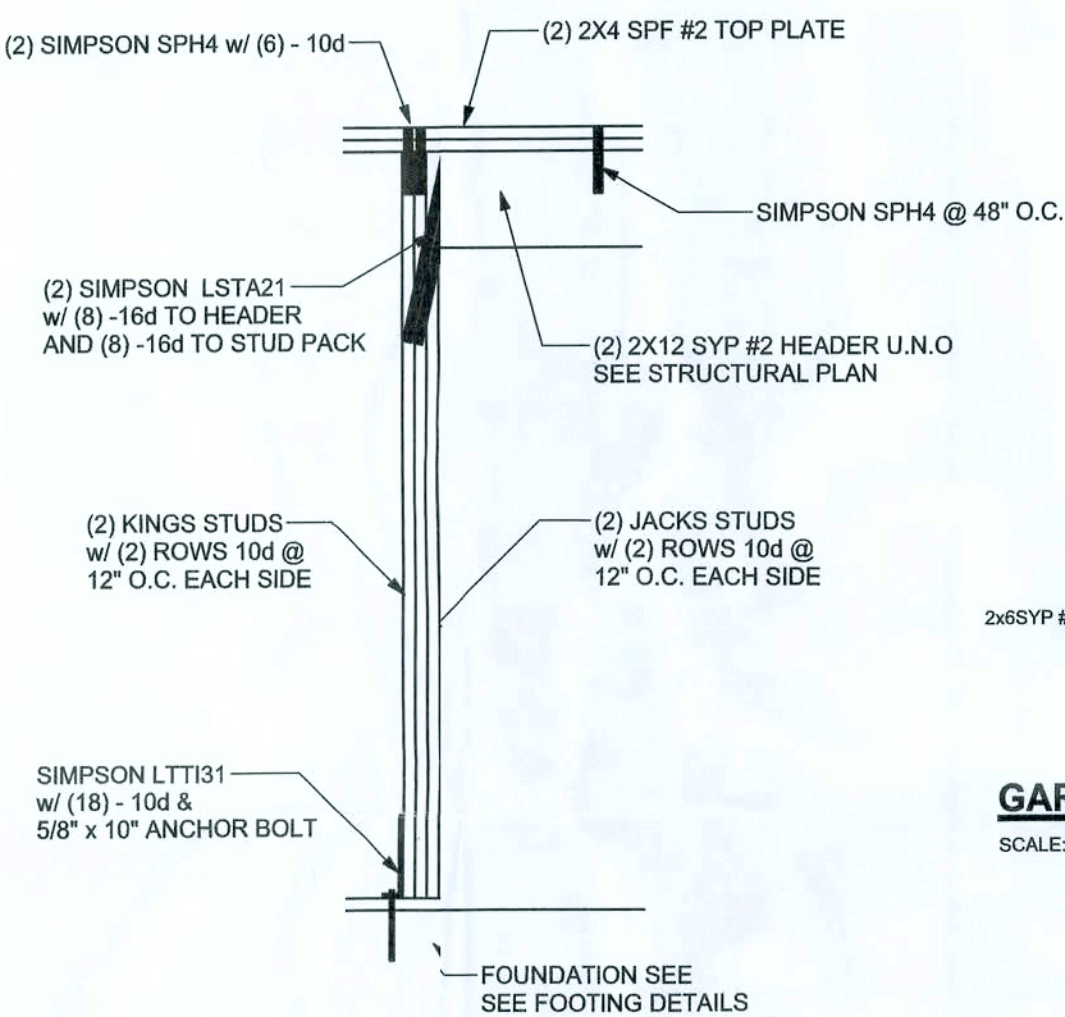
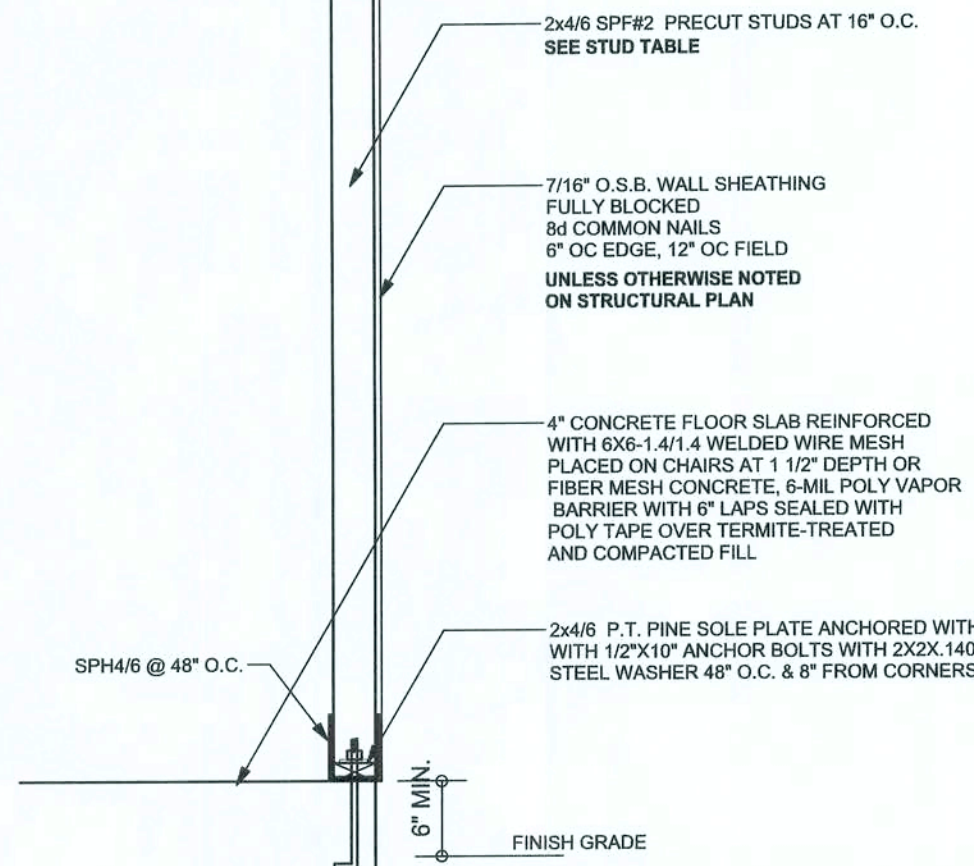


INTERIOR BEARING WALL  
SCALE: 1/2" = 1'-0"

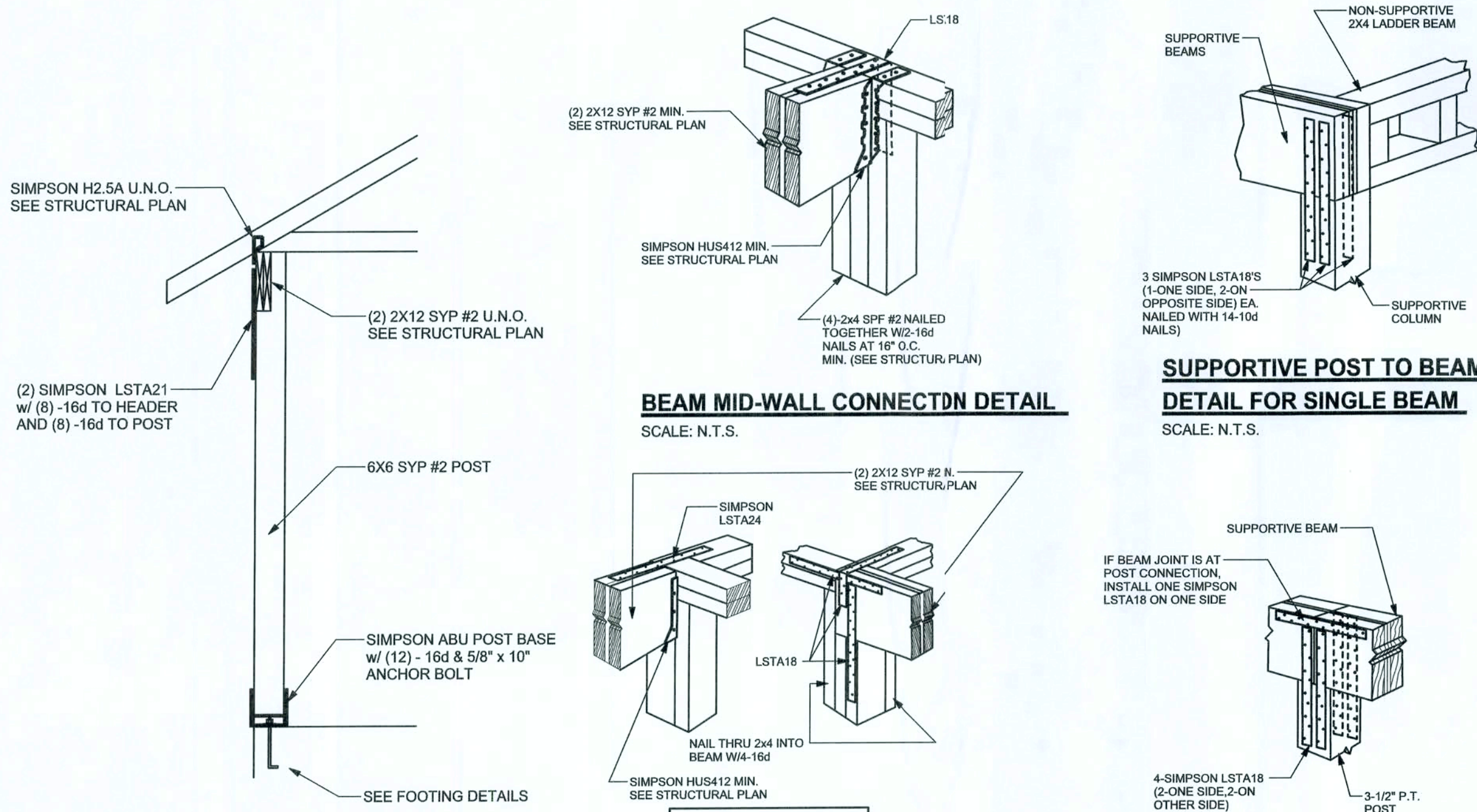


EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

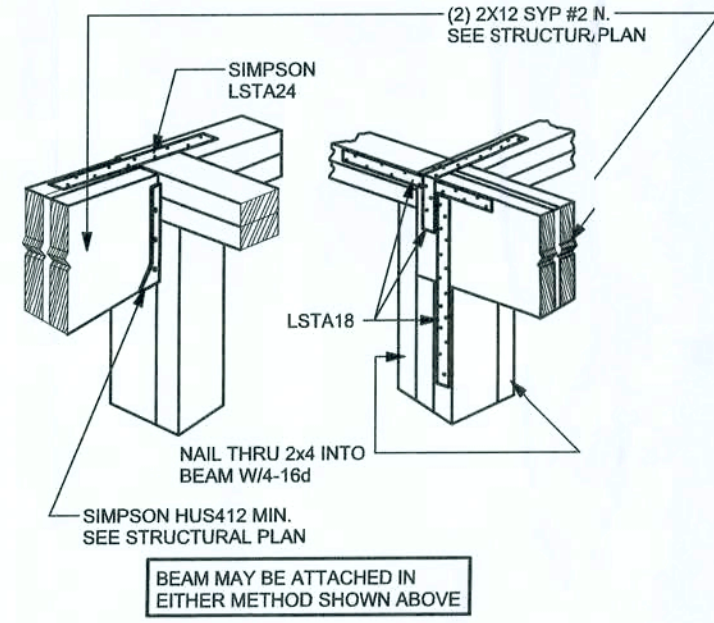
(1) 2x4 @ 16" O.C.	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" O.C.	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" O.C.	TO 18'-0" STUD HEIGHT
(1) 2x6 @ 12" O.C.	TO 20'-0" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B. EXTERIOR LOAD BEARING & NON LOAD BEARING STUDS RESISTING INTERIOR ZONE WIND LOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.5" O.C.

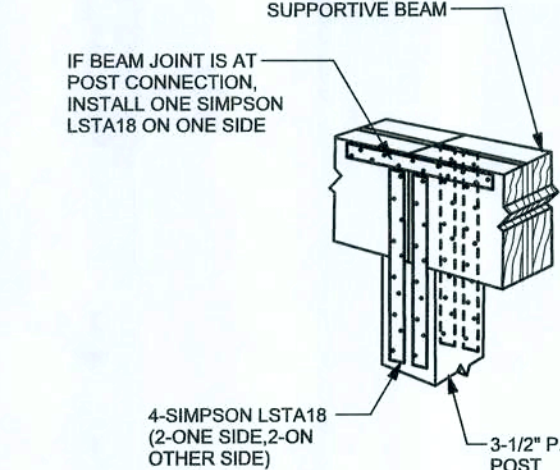
TYPICAL GARAGE DOOR HEADER STRAPING DETAIL  
SCALE: 1/2" = 1'-0"



BEAM MID-WALL CONNECTION DETAIL  
SCALE: N.T.S.



BEAM CORNER CONNECTION DETAIL  
SCALE: N.T.S.

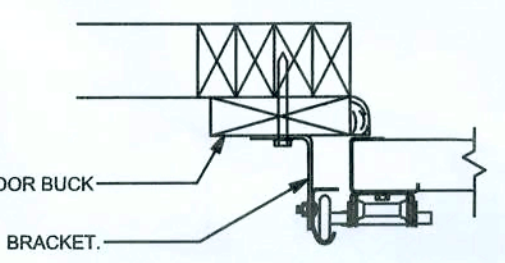


SUPPORTIVE CENTER POST TO BEAM DETAIL  
SCALE: N.T.S.

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT  
ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4" ON PER TABLE BELOW:

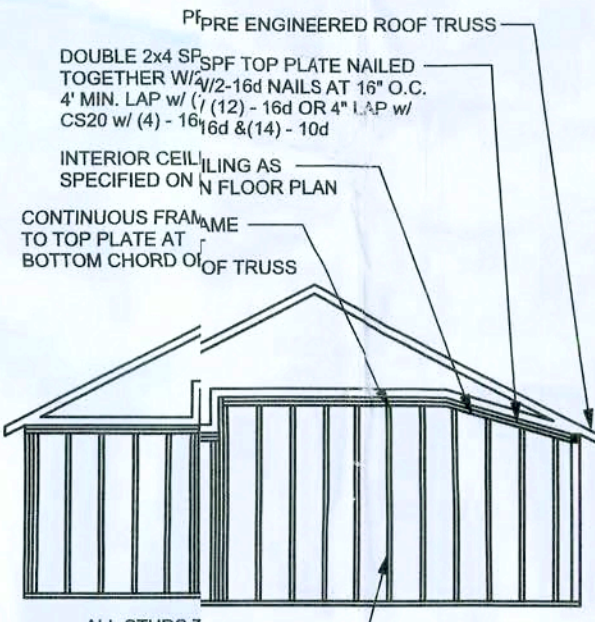
DOOR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF .131 x 3 1/4" G.N.
8' - 10'	24" O.C.	5" O.C.	5" O.C.
11' - 15'	18" O.C.	4" O.C.	4" O.C.
16' - 18'	16" O.C.	3" O.C.	3" O.C.

GARAGE DOOR BUCK INSTALLATION DETAIL  
SCALE: N.T.S.

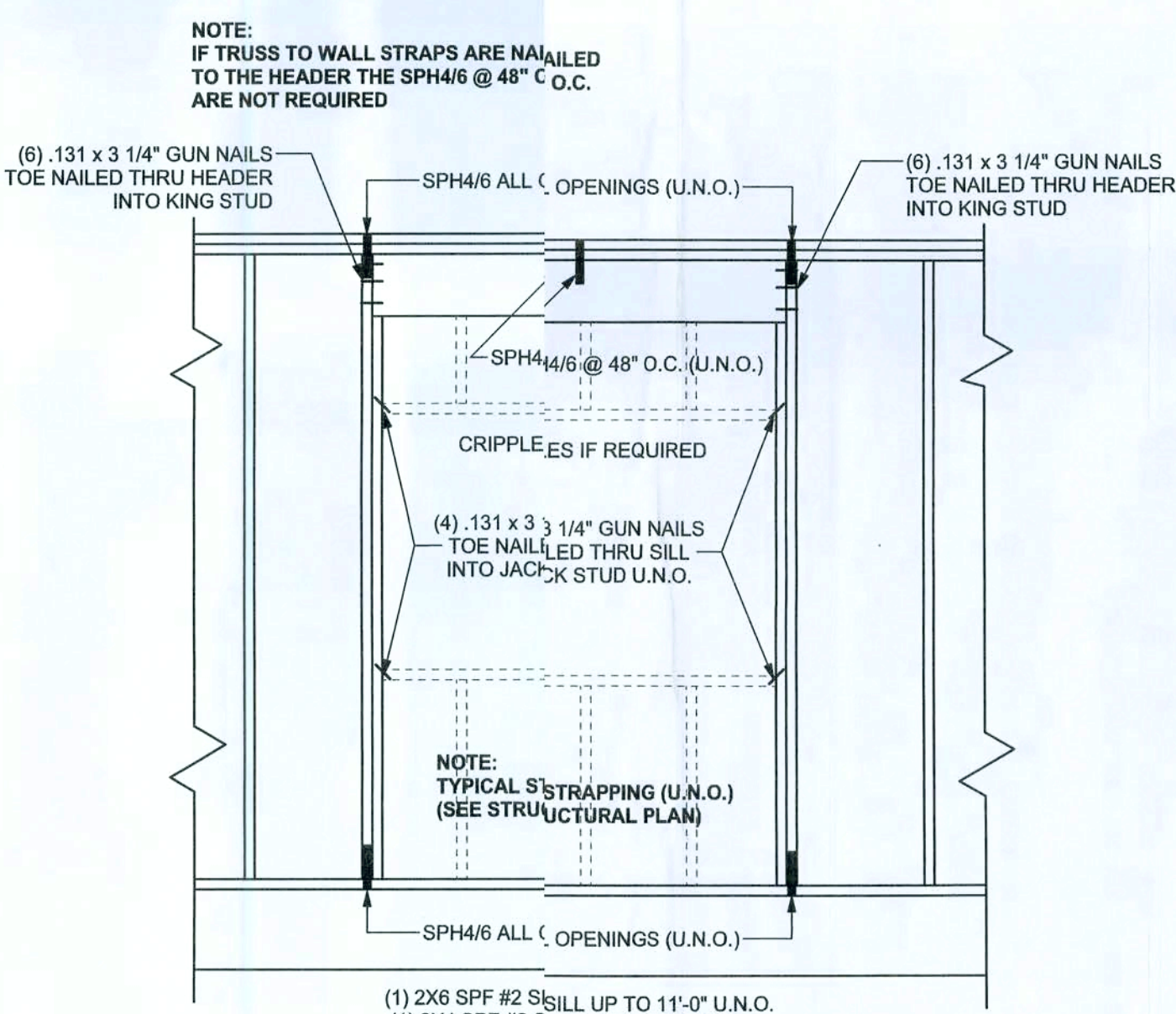


GRADE & SPECIES TABLE

		Fb (psi)	E (10 <sup>6</sup> psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0



CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL  
SCALE: N.T.S.



TYPICAL HEADER STRAPPING DETAIL  
SCALE: 1/2" = 1'-0"

## GENERAL NOTES:

**TRUSSES:** TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

**SITE PREPARATION:** SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

**FOUNDATION:** CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

**CONCRETE:** MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F<sub>c</sub> = 3000 PSI.

**WELDED WIRE REINFORCED SLAB:** 8" x 6" W4 x W4 A FB = 8KSL. WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF SLAB. SLAB PLACED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3".

**FIBER CONCRETE SLAB:** CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

**CONTROL JOINTS:** WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WMM OR REINFORCING STEEL (RECOMMENDED) LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

**REBAR:** ASTM A 615, GRADE 60, DEFORMED BARS, F<sub>y</sub> = 60 KSI. ALL LAP SPICES 40" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

**GLULAM BEAMS:** GLULAM BEAM, GLB, 24F-V3SP, F<sub>b</sub> = 2,400psi, E = 1,800ksi. UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCULATIONS. **ROOF SHEATHING:** ALL ROOFS ARE HORIZONTAL DIAPHRAGMS. 7/16" OSB SHEATHING, UNLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS. GABLE ENDS AND DIAPHRAGM BOUNDARY, 4"OC, UNO.

**STRUCTURAL CONNECTORS:** MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

**ANCHOR BOLTS:** A 307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED CONCRETE BEAM OR 12" IN GROUTED CMU.

**WASHERS:** WASHERS USED WITH 1/2" BOLTS TO BE 7" x 2" x 9/64". WITH 5/8" BOLTS TO BE 3" x 3" x 9/64". WITH 3/4" BOLTS TO BE 3" x 3" x 9/64". UNO.

**NAILS:** ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

## BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

## ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE WIND DESIGNER. THE BUILDER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED RESPONSIBILITY SHEETS.

## MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI530.1-02 Section	Specific Requirements
1.4A Compressive strength	8" block bearing walls F <sub>m</sub> = 1500 psi
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4 Reinforcing bars, #3 - #11	ASTM A 615, Grade 60, F <sub>y</sub> = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/lb or 304SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

## ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H8	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LG12	14 - 16d	14 - 16d	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MG1		22 - 10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 - 10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 - 10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 - 10d	2-5/8" THREADED ROD 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3-10d		4-10d
< 455	< 420	SSP SINGLE SILL PLATE	1-10d		4-10d
< 825	< 825	DSP DOUBLE TOP PLATE	6-10d		8-10d
< 825	< 600	DSP SINGLE SILL PLATE	2-10d		8-10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1085	SP4H			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1085	SP6H			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*		TO STUDS	TO FOUNDATION
< 1350	< 1305	LT119	8-16d		1/2" AB
< 2310	< 2310	LT131	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HT116	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

## DESIGN DATA

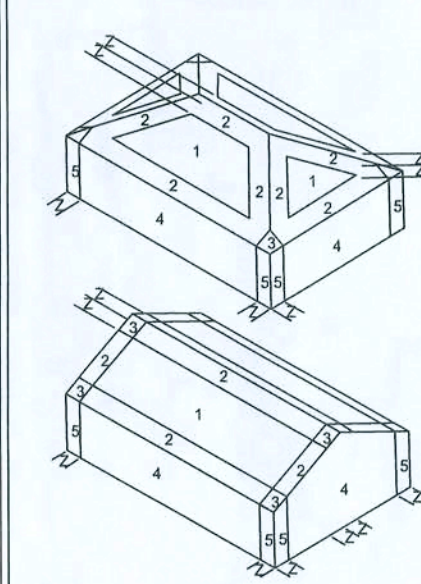
WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1

ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

- BASIC WIND SPEED = 110 MPH
- WIND EXPOSURE = B
- WIND IMPORTANCE FACTOR = 1.0
- BUILDING CATEGORY = II
- ROOF ANGLE = 10-45 DEGREES
- MEAN ROOF HEIGHT = <30 FT
- INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
- COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



Zone	Effective Wind Area (ft <sup>2</sup> )	10	100
1	19.9 - 21.8	18.1	-18.1
2	19.9 - 25.5	18.1	-21.8
2 Other		-40.6	-40.6
3	19.9 - 25.5	18.1	-21.8
3 Other		-48.3	-42.4
4	21.8 - 23.6	18.5	-20.4
5	21.8 - 29.1	18.5	-22.6
Doors & Windows		21.8	-29.1
Worst Case (Zone 5, 10 ft2)			
8x7 Garage Door		19.5	-22.9
12x7 Garage Door		18.5	-21.0

## DESIGN LOADS

- FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)
- 30 PSF (SLEEPING ROOMS)
- 30 PSF (ATTICS WITH STORAGE)
- 10 PSF (ATTICS WITHOUT STORAGE, <3:12)
- ROOF 20 PSF (FLAT OR <4:12)
- 16 PSF (4:12 TO <12:12)
- 12 PSF (12:12 AND GREATER)
- STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)
- SOIL BEARING CAPACITY 1000PSF
- NOT IN FLOOD ZONE (BUILDER TO VERIFY)

## REVISIONS


SOFTPLAN  
ARCHITECTURAL DESK SOFTWARE

WINDLOAD ENGINEER: Mark Disoway, PE No.53915, POB 866, Lake City, FL 32056, 386-754-5419

**DIMENSIONS:** Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.

MARK DISOWAY  
P.E. 53915

04 Jan 08  
SEA

Michael Sreicher  
Residence

ADDRESS:  
Country Lake Glen  
Woodborough S/D  
Columbia County, Florida

Mark Disoway P.E.  
P.O. Box 868  
Lake City, Florida 32056  
Phone: (386)754 - 5419  
Fax: (386) 289 - 4871

PRINTED DATE:  
January 04 2008

STRUCTURAL BY:

FINALS DATE:  
04 / Jan / 08

JOB NUMBER:  
712-11  
DRAWING NUMBER

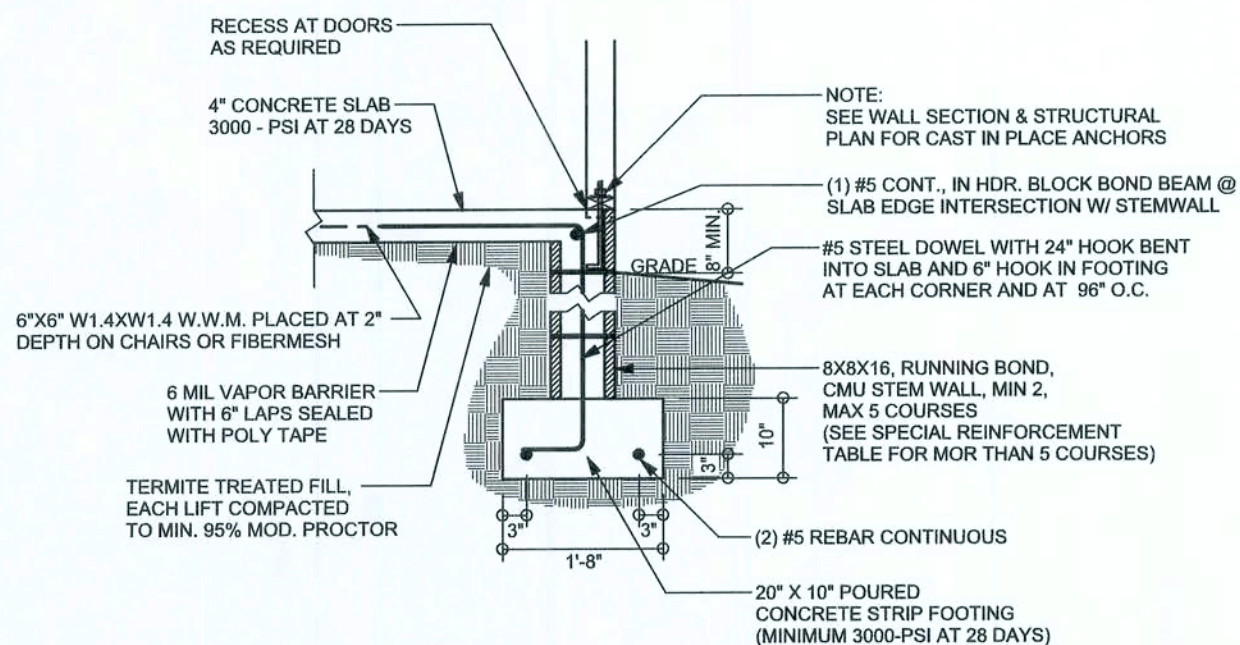
S-I

OF 3 SHEETS

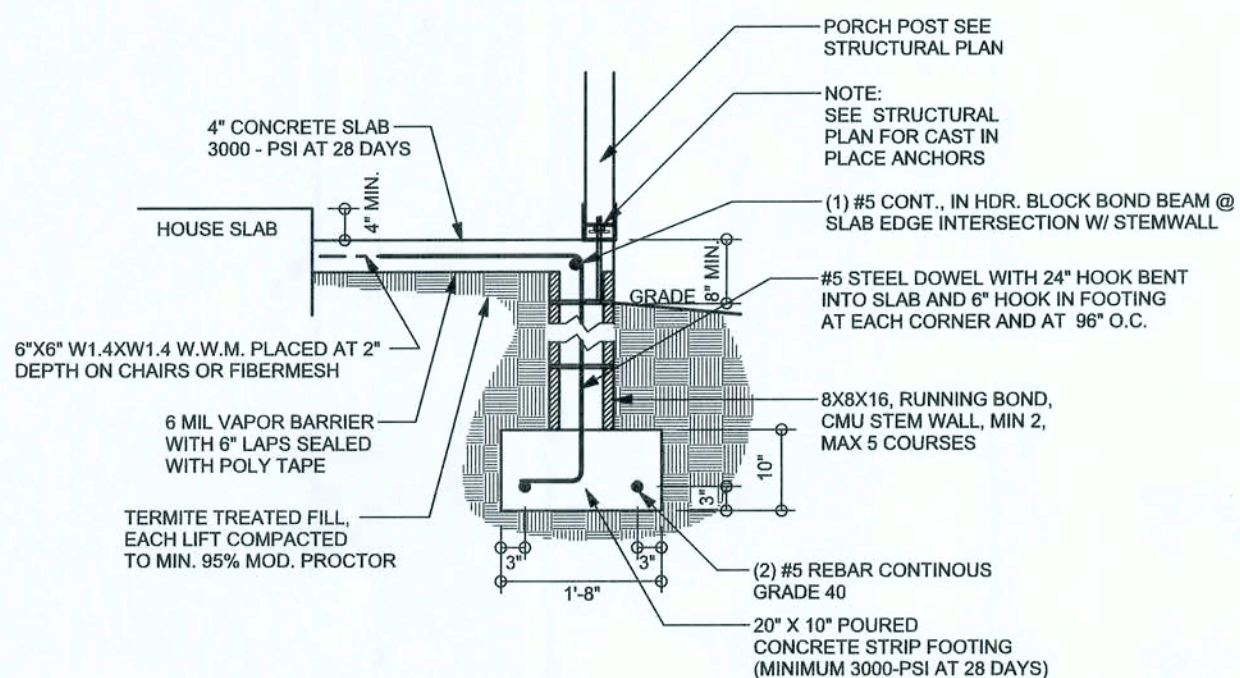


REVISIONS

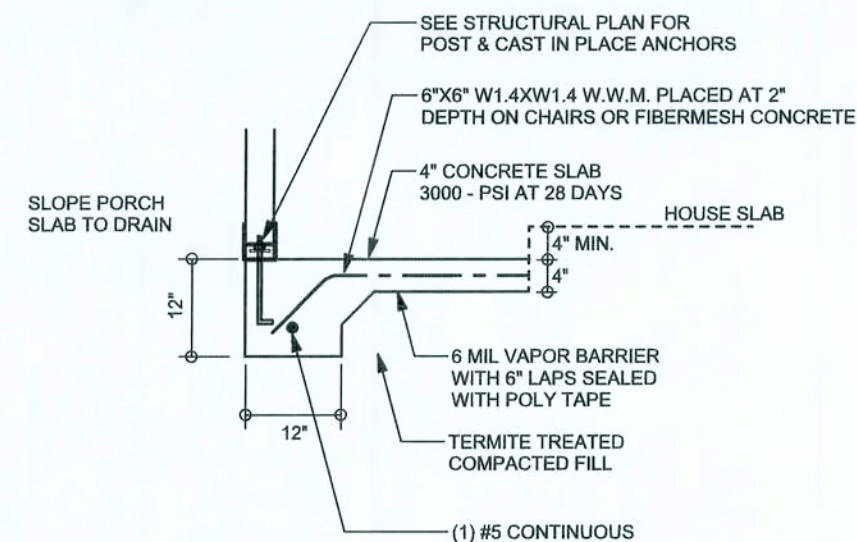
SOFTPLAN  
ARCHITECTURAL DESIGN SOFTWARE



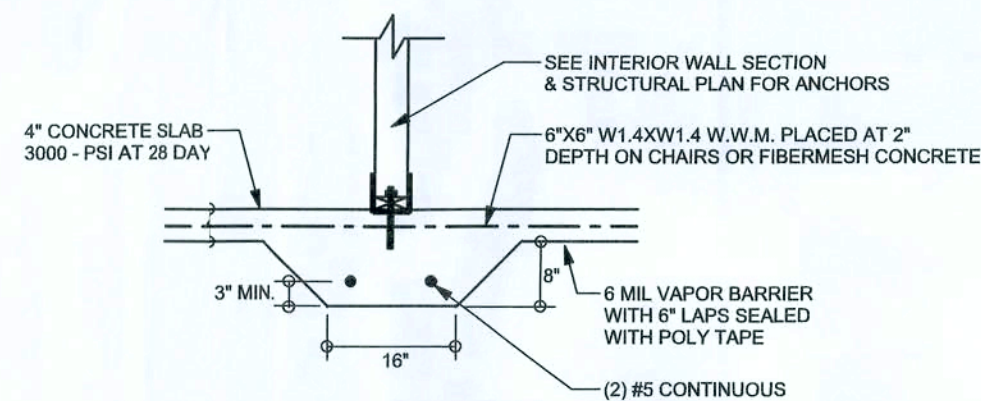
**F9 S-2** STEM WALL FOOTING  
SCALE: 1/2" = 1'-0"



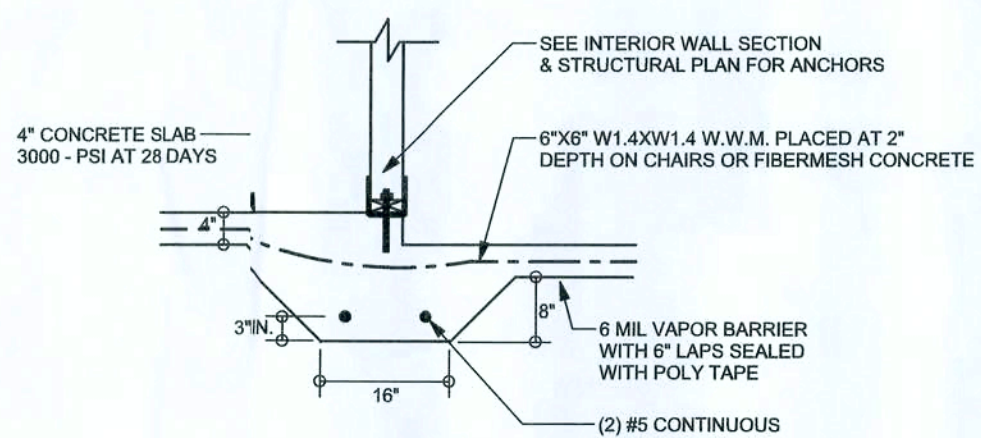
**F12 S-2** ALT. STEM WALL PORCH FOOTING  
SCALE: 1/2" = 1'-0"



**F5 S-2** PORCH FOOTING  
SCALE: 1/2" = 1'-0"



**F2 S-2** INTERIOR BEARING FOOTING  
SCALE: 1/2" = 1'-0"



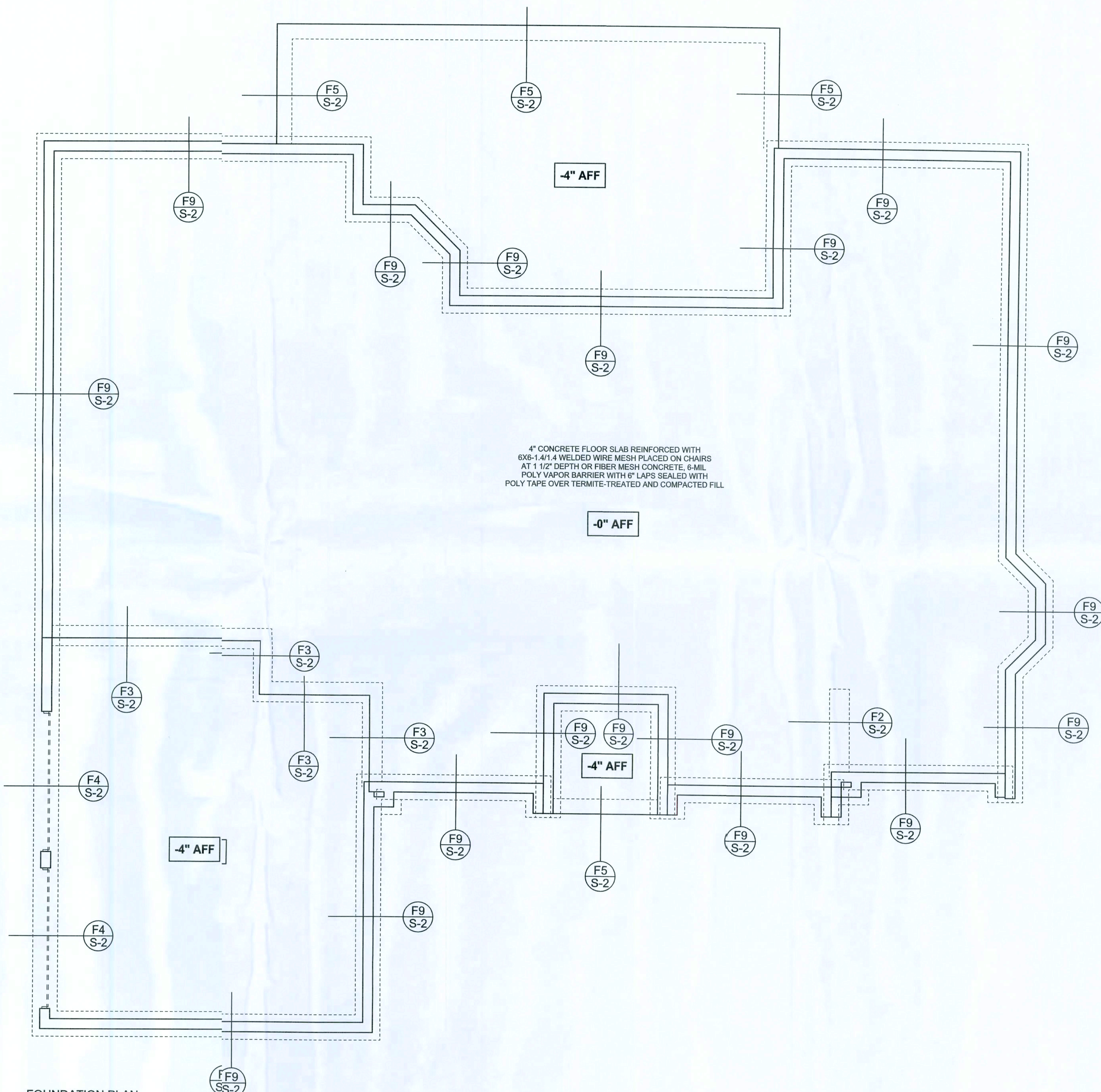
**F3 S-2** INTERIOR BEARING STEP FOOTING  
SCALE: 1/2" = 1'-0"

**TALL STEM WALL TABLE**

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8" high, Diagonal ladder reinforcement at 18" O.C. vertically or a horizontal bond beam with 1" continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL BALANCED HEIGHT (FEET)	BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

**F4 S-2** GARAGE DOOR FOOTING  
SCALE: 1/2" = 1'-0"



**FOUNDATION PLAN**

SCALE: 1/4" = 1'-0"  
DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

WINDLOAD ENGINEER: Mark Disosway, P.E. No. 53915, PCB 868, Lake City, FL 32056, 386-754-5419

**DIMENSIONS:**  
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relate to wind engineering comply with section F501.1.1, Florida building code residential 2004, to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.

MARK DISOSWAY  
P.E. 53915

*Mark Disosway*  
04/Jan/08  
SEAL

Michael Sreicher  
Residence

ADDRESS:  
Country Lake Glen  
Woodborough S/D  
Columbia County, Florida

Mark Disosway P.E.  
P.O. Box 868  
Lake City, Florida 32056  
Phone: (386) 754 - 5419  
Fax: (386) 249 - 4871

PRINTED DATE:  
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STRUCTURAL BY:

FINALS DATE:  
04 / Jan / 08

JOB NUMBER:  
712111

DRAWING NUMBER

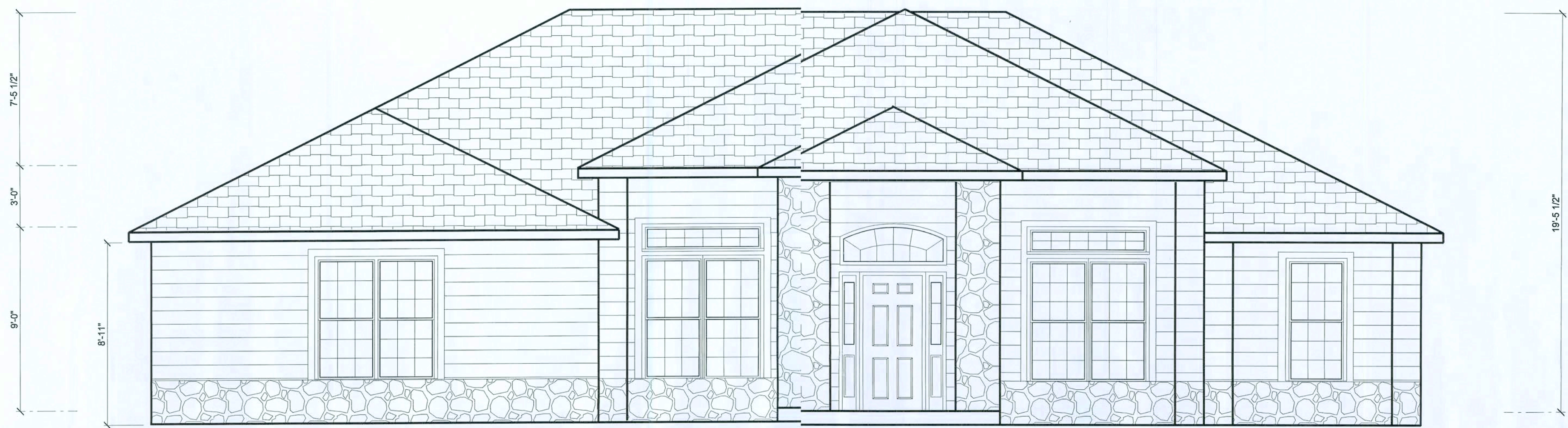
**S-2**

OF 3 SHEETS





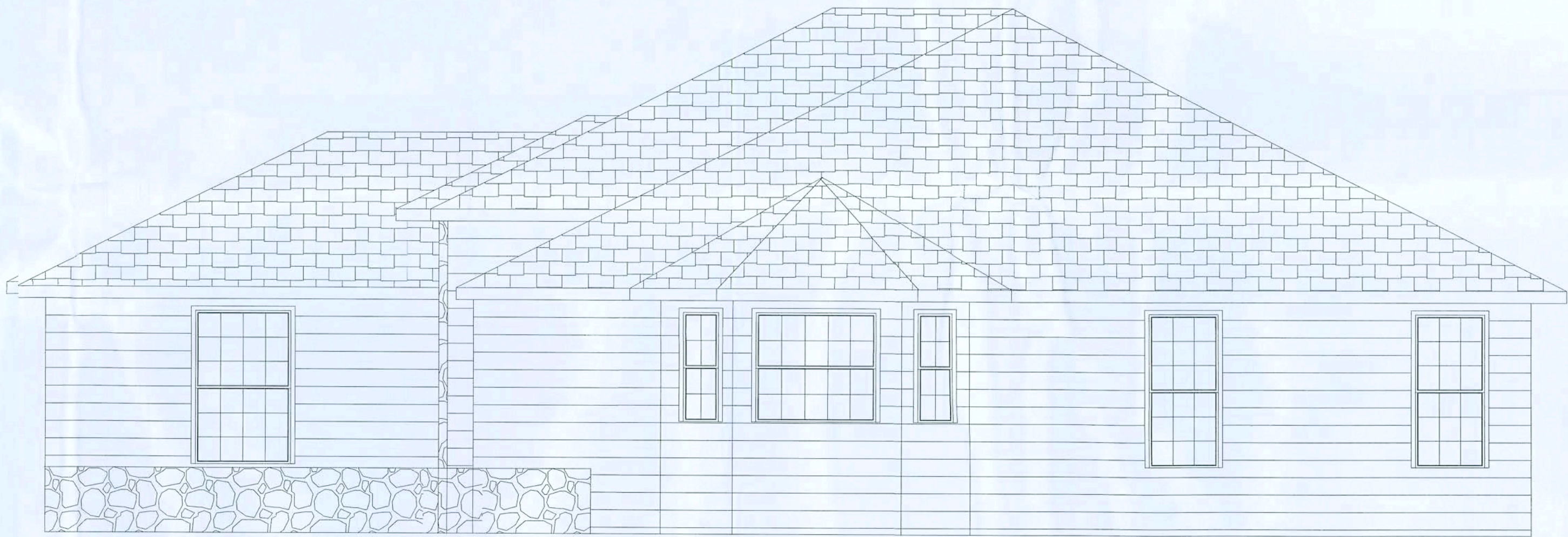




**FRONT ELEVATION**  
 SCALE: 3/8" = 1'-0"



**LEFT ELEVATION**  
 SCALE: 1/8" = 1'-0"



**RIGHT ELEVATION**  
 SCALE: 1/8" = 1'-0"



**REAR ELEVATION**  
 SCALE: 1/8" = 1'-0"

**AREA SUMMARY**

LIVING AREA -	2312 SF
GARAGE -	580 SF
PORCHES -	562 SF
TOTAL AREA -	3454 SF

New Custom Home For:  
**Michael Streicher**

**Brian S. Crawford**  
 Architectural Design  
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 Lake City, FL 32025  
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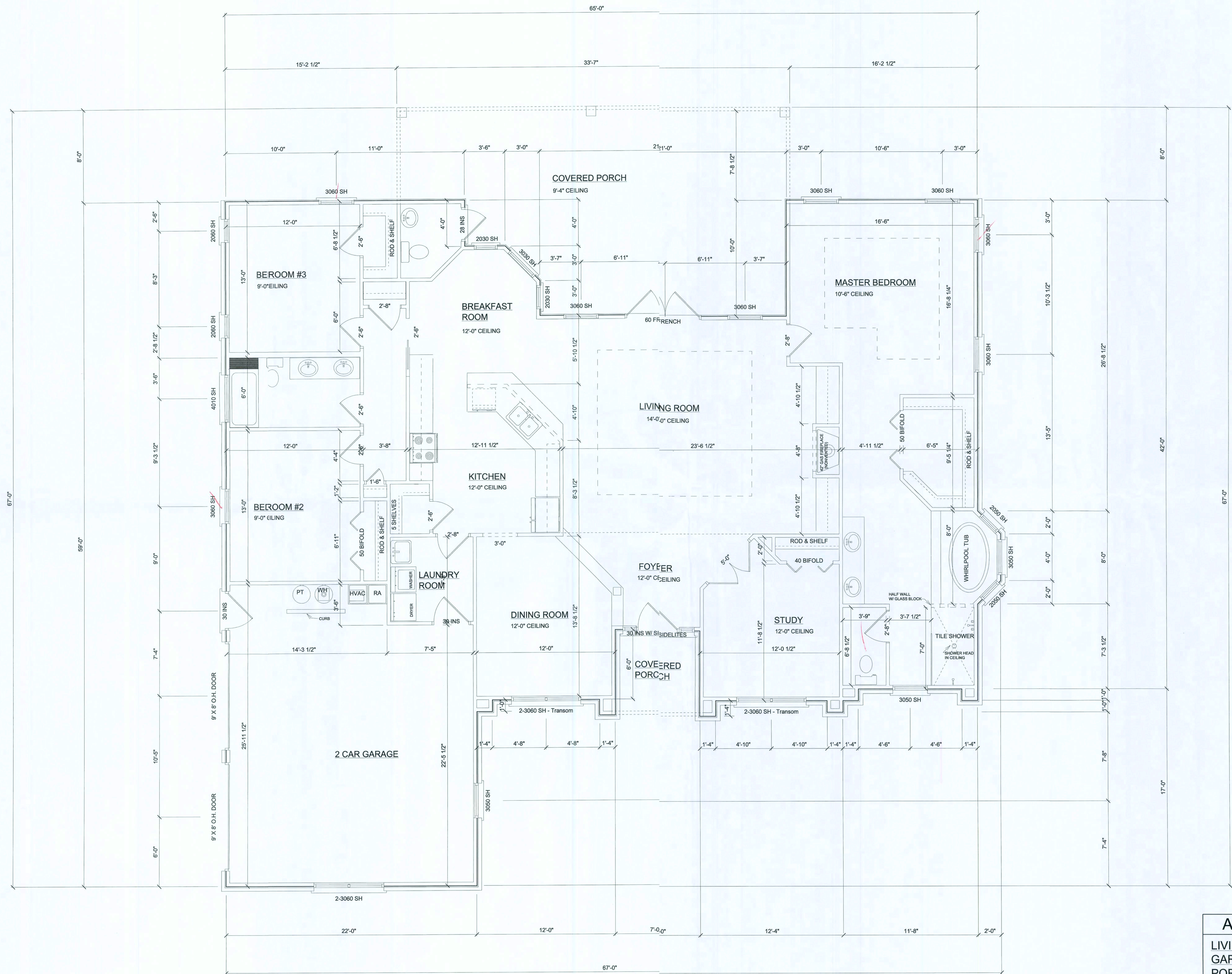
ELEVATIONS

SHEET NUMBER

**A-1**

OF 4 SHEETS





AREA SUMMARY	
LIVING AREA -	2312 SF
GARAGE -	580 SF
PORCHES -	562 SF
TOTAL AREA -	3454 SF

# MAIN FLOOR PLAN

SCALE: 1/4"=1'-0"

SHEET NUMBER

A-2

OF 4 SHEETS

Brian S. Crawford

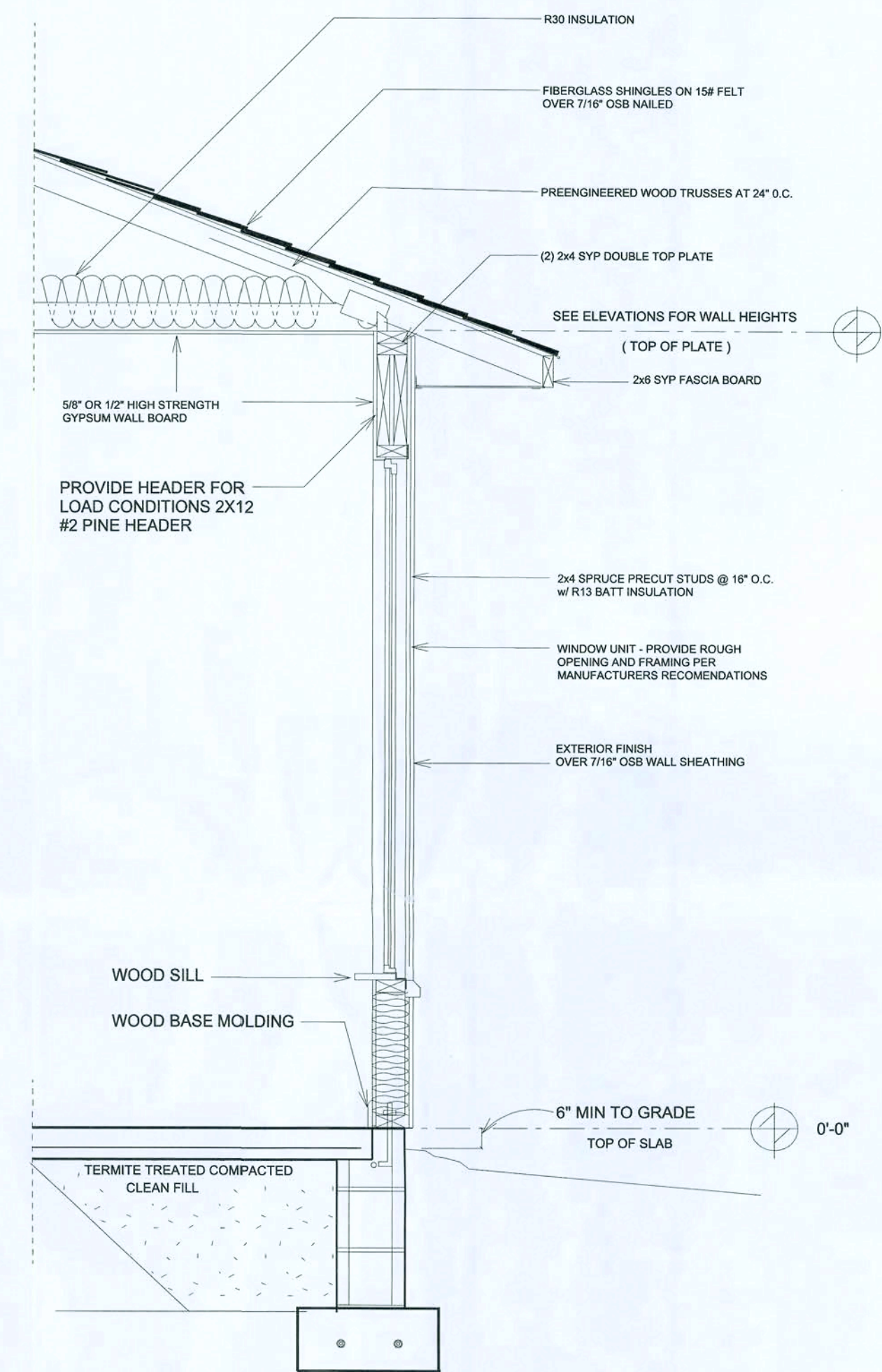
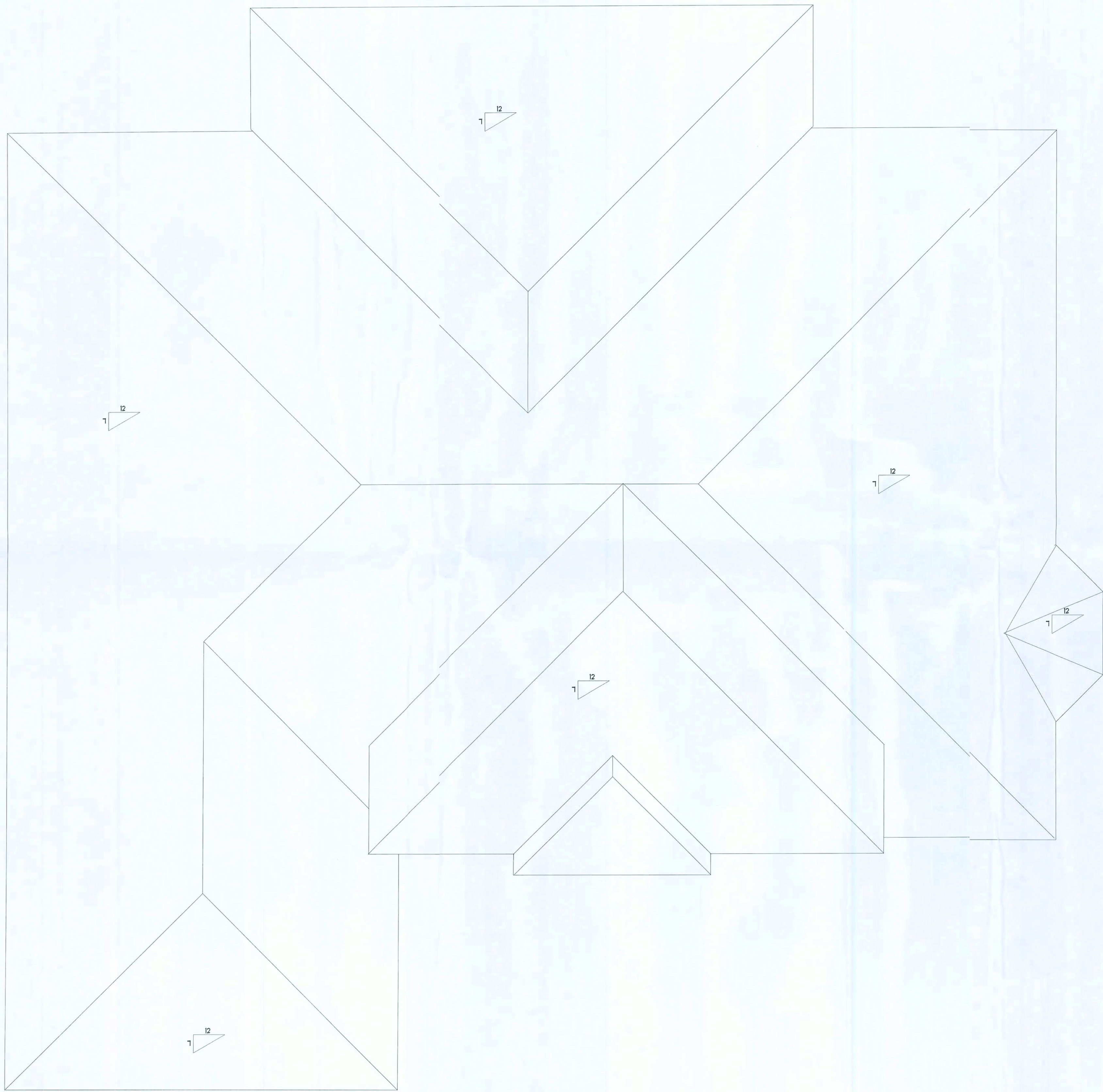
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TYPICAL WALL SECTION  
2 X 4 STUD WALL

AREA SUMMARY	
LIVING AREA -	2312 SF
GARAGE -	580 SF
PORCHES -	562 SF
TOTAL AREA -	3454 SF

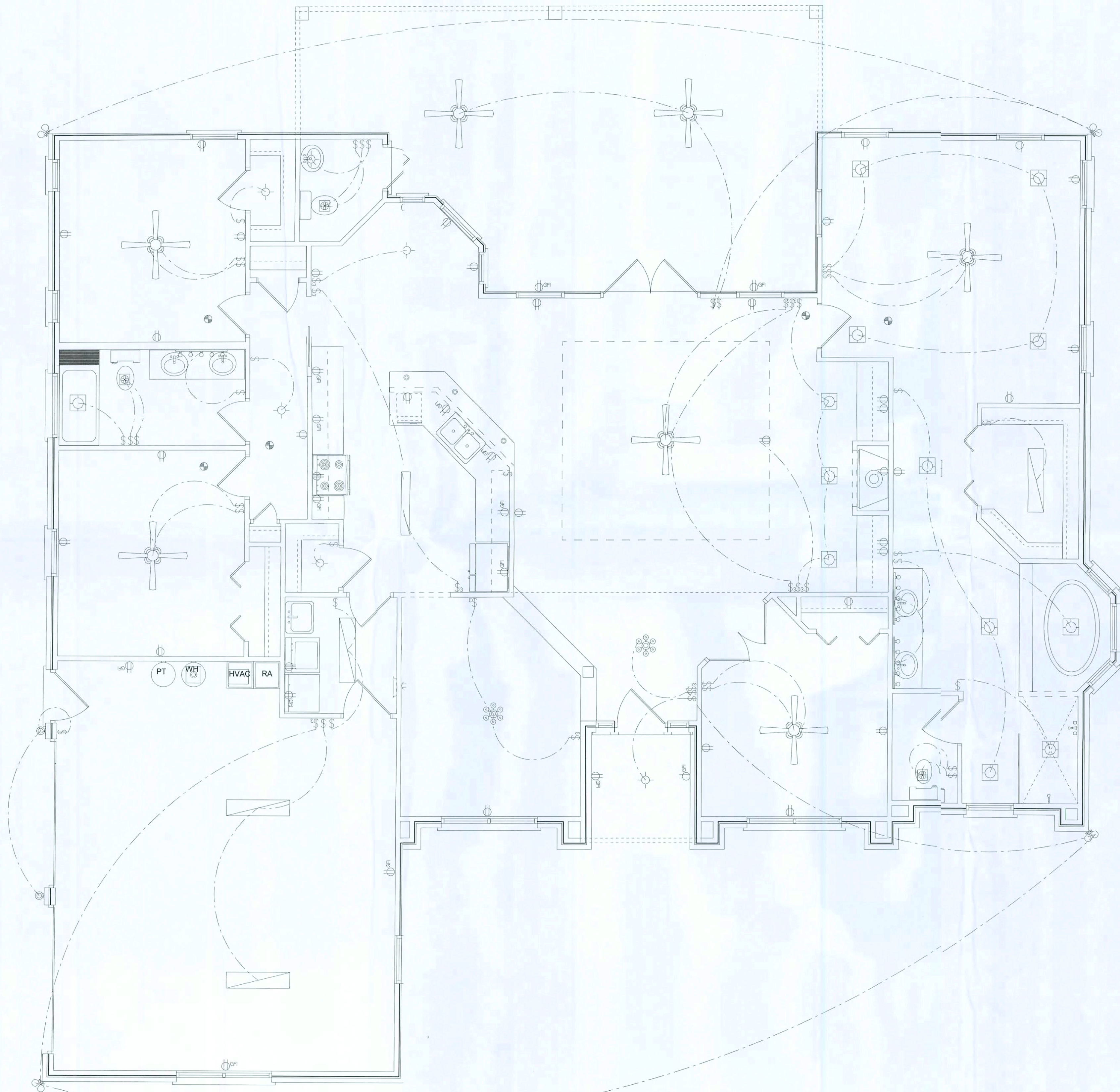
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ROOF PLAN  
SCALE: 1/4"=1'-0"

SHEET NUMBER  
**A-3**  
OF 4 SHEETS

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**ELECTRICAL PLAN NOTES**

ALL RECEPTICALS IN ALL BEDROOMS SHALL BE AFCI CIRCUITS

WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.

CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.

INSTALLATION SHALL BE PER NAT'L. ELECTRIC CODE.

ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.

TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.

ELECTRICAL CONTR' SHALL PREPARE "AS-BUILT" SHOP DWGS INDICATING ALL ELECTRICAL WORK, INCLUDING ANY CHANGES TO THE ELEC. PLAN, ADD'NS TO THE ELEC. PLAN, RISER DIAGRAM, AS-BUILT PANEL SCHEDULE W/ ALL CKTS IDENTIFIED W/ CKT N<sup>o</sup>, DESCRIPTION & BRKR, SERVICE ENT. & ALL UNDERGROUND WIRE LOCATIONS/ROUTING/DEPTH, RISER DIA. SHALL INCLUDE WIRE SIZES/TYPE & EQUIPMENT TYPE W/ RATINGS & LOADS.

CONTRACTOR SHALL PROVIDE 1 COPY OF AS-BUILT DWGS TO OWNER & 1 COPY TO THE PERMIT ISSUING AUTHORITY.

ELECTRICAL	COUNT	SYMBOL
ceiling fan spotlights 2	7	
Can Light	13	
chandelier	2	
double spotlight	4	
fluorescent fixture	5	
pendant globe	3	
vanity bar light	3	
wall mount 1	2	
electrical panel	1	
Fan - Light Combo	3	
light	6	
outlet	31	
outlet 220v	2	
outlet gfi	16	
smoke detector	5	
switch	35	
switch 3 way	12	

AREA SUMMARY
LIVING AREA - 2312 SF
GARAGE - 580 SF
PORCHES - 562 SF
TOTAL AREA - 3454 SF

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2109 W US 90 STE 170-144  
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**ELECTRICAL PLAN**  
SCALE: 1/4"=1'-0"

SHEET NUMBER

**A-4**  
OF 4 SHEETS

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