ITW Building Components Group, Inc. Periston Call

1950 Marley Drive Haines City, FL 33844

Florida Engineering Certificate of Authorization Number: 0 278

Florida Certificate of Product Approval # FL 1999

Page 1 of 1 Document ID:1THR8228Z0327090511

Page 2 of 1 Document ID:1THR8228Z0327090511

Truss Fabricator: Anderson Truss Company

Job Identification: 8-140--OWNER BUILDER Baker -- , \*\*

Truss Count: 29

Model Code: Florida Building Code 2004 and 2006 Supplement

Truss Criteria: ANSI/TPI-2002(STD)/FBC

Engineering Software: Alpine Software, Versions 7.24, 7.37.

Structural Engineer of Record: The identity of the structural EOR did not exist as of

Address: the seal date per section 61G15-31.003(5a) of the FAC

Minimum Design Loads: Roof - 32.0 PSF @ 1.25 Duration

Floor - N/A

Wind - 110 MPH ASCE 7-02 - Closed

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1

2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.

3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

Seal Date: 05/27/2008

-Truss Design Engineer-Doug Fleming Florida License Number: 66648 1950 Marley Drive Haines City, FL 33844

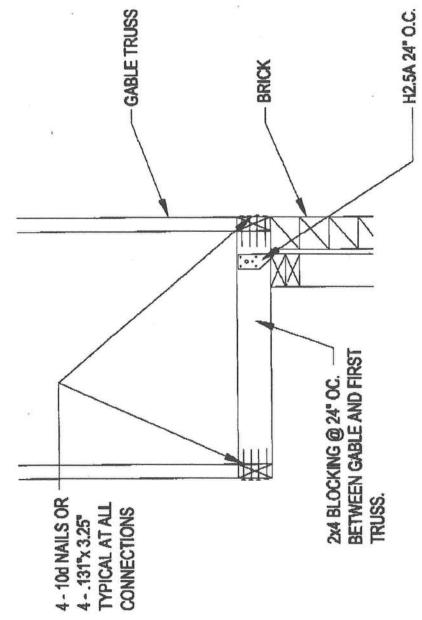
Details: BRCLBSUB-TCFILLER-BCFILLER-A13015EE-GBLLETIN-VALTRUSS-A13030EE-A11015EE-PIGBACKB-

#	Ref Description	Drawing#	Date
1	67239 A2	08148006	05/27/08
2	67240 A3	08148023	05/27/08
3	67241 A4	08148024	05/27/08
4	67242 A5	08148025	05/27/08
5	67243 A6	08148007	05/27/08
6	67244AA-GE	08148008	05/27/08
7	67245A1	08148026	05/27/08
8	67246A-GE	08148027	05/27/08
9	67247 V1	08148009	05/27/08
10	67248 V2	08148001	05/27/08
11	67249V3	08148002	05/27/08
12	67250V4	08148003	05/27/08
13	67251V5	08148004	05/27/08
14	67252V6	08148005	05/27/08
15	67253B-GE	08148010	05/27/08
16	67254B3	08148013	05/27/08
17	67255B1	08148014	05/27/08
18	67256B2	08148015	05/27/08
19	67257 C4 - GDR	08148016	05/27/08
20	67258 C1	08148017	05/27/08
21	67259C2	08148018	05/27/08
22	67260 C3	08148019	05/27/08
23	67261 C - GE	08148020	05/27/08
24	67262 CC - GE	08148028	05/27/08
25	67263 PB1	08148021	05/27/08
26	67264PB2	08148029	05/27/08
27	67265 PB3	08148030	05/27/08
28	67266PB5	08148031	05/27/08
29	67267 PB4	08148022	05/27/08



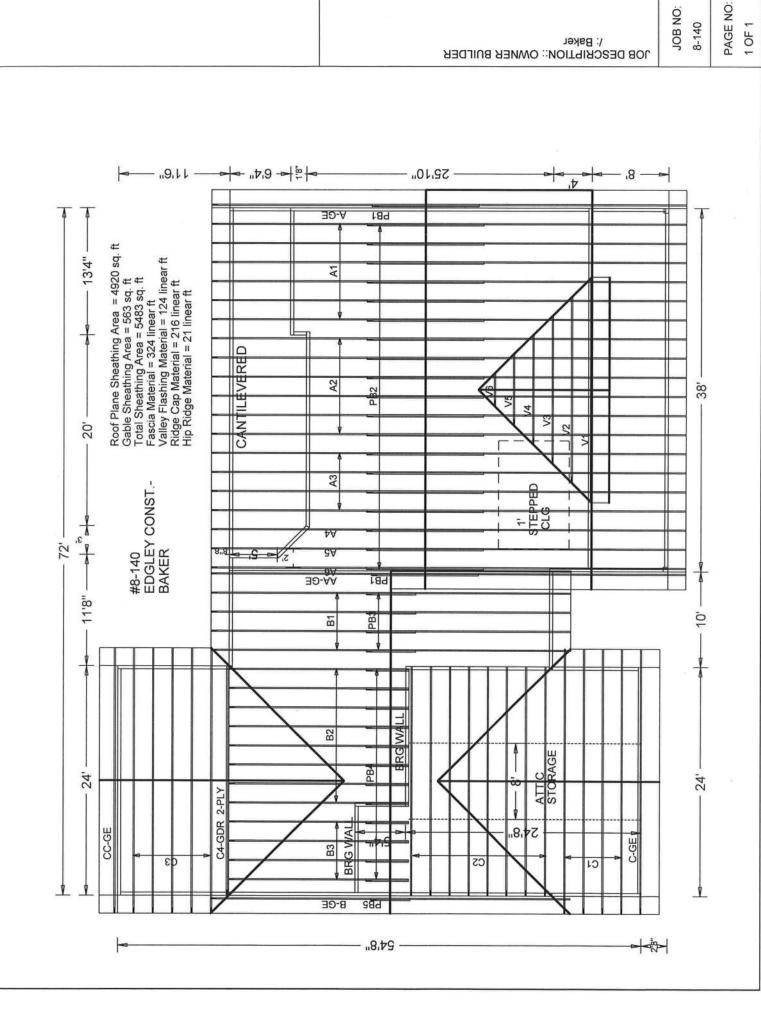
752-4904

Baker



# W40 - TYPICAL GABLE END BRICK DETAIL

SCALE: N.T.S.



Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 :Lt Slider 2x6 SP #2: BLOCK LENGTH = PLT WARNING: Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below. In lieu of structural panels use purlins to brace all flat TC @ 24"  $\,$  0C. Roof overhang supports 2.00 psf soffit load A (8-140--OWNER BUILDER Baker TW Building Components Group TYP. 0-21.5X8(E3) Continuous lateral bracing equally spaced on member Haines City, FL 33844 FL COA #0 278 ALPINE Wave 1-10-8 3X4(E3) = 3X4# -8-0-0-8 8-2-0-\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, THC. SHALL NOT BE RESPONSIBLE FOR NAY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH TPI: OF FARRICATING, INADIALING, SHEPPING, HISTALLING, A BRACHING OF TRUSSES. DESIGN FOR THE PROPLECABLE PROVISIONS OF NDS. (MATIONAL DESIGN SECE. 3. \*AFRA) AND TPI. THE GC CONNECTOR PLATES ARE HADE OF 20/18/166A (M.H/SS/M) ASTH A653 GRADE 40/60 (M. K/M.SS) GALV. STEEL APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY TREFECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A OF TPI1-2002 SEC. 3. A SEAL ON THIS DESIGN SHOWN. THE SULFABLETING RESPONSIBILITY SOLEY FOR THE FRUSS COMPONENT DESIGN SHOWN. THE SULFABLILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE \*\*WARNING\*\* IRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. RETER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS FLATE INSTITUTE, 2188 MORTH LEE STREET, SUITE 3172, ALEXANDRA, VA, 22314) AND MICA (MODED BRY TDE COUNCIL OF AMERICA, 6300 ERRIFERDES LANE, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TO FROM SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTON CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTON CHORD SHALL HAVE 3×6/ 3 X 4 Ⅲ 4X6加 R-2191 U=320 W=4" (A) **★**-6-1-13-**>★**-6-1-9-1.974' A2) Design Crit: 3 X 5 = 5 X 6 = 4X6≡ (A) -45-10-0 Over 3 1.5X4 ■ TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/0(0) 12-3-2 3X8= ¥ 6-1-9-(A)(A), 29-8-0 Supports 5 X 6 = 3 X 4 ≡ <del>-5-11-4-y</del> <del>-5-3-12-y</del> 3X4// 11-3-1 4 X 6 ≡ 3X4= 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART.\_ENC. bldg, anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC psf, Iw=1.00 GCpi(+/-)=0.55 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Calculated horizontal deflection is 0.11" due to live load and 0.17" due to dead load. Wind reactions based on MWFRS pressures. 5 X 8 / 2X4 III OSSIGEN FLEM R-1294 U-307 W-4" OSIONAL BIGINE LICENSE No. 66648 8-0-0 -8-0-0-R-558 U-126 W-4" 2.5X6(A1) =1-10-8 OTY:6 80 BC LL BC DL TC DL TC LL SPACING DUR.FAC TOT.LD. FL/-/4/-/-/R/-10.0 40.0 20.0 PSF 1.25 10.0 PSF 24.0" 0.0 PSF PSF PSF REF JREF -FROM DATE SEQN HC-ENG DRW HCUSR8228 08148006 Scale =.125"/Ft. DL=5.0 R8228-1THR8228Z03 DF / DF 05/27/08 170256 10-6-0 67239

FEET ----- 6+ -3666 --- 3-

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 Filler 2x4 SP #3 :Lt Slider 2x6 SP #2: BLOCK LENGTH =

Roof overhang supports 2.00 psf soffit load

Calculated horizontal deflection is 0.11" due to live due to dead load. load and 0.17"

Laterally brace BC at 24" OC in lieu of rigid ceiling. Laterally brace BC above filler at 24" OC.

Deflection meets L/240 live and L/180 total load. factor for dead load is 1.50. Creep increase

Laterally brace BC above filler @ 24" O.C. Including a lateral brace at chord ends.

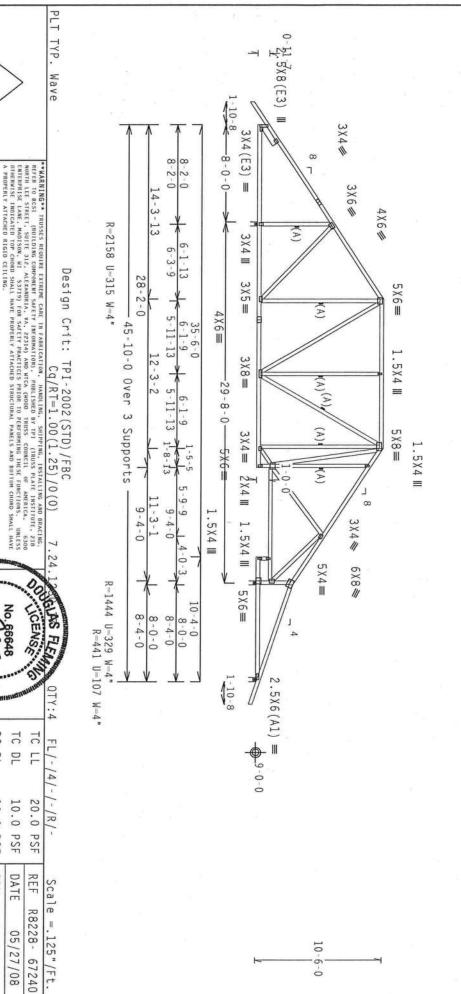
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

See DWGS TCFILLER0207 and BCFILLER0207 for filler details.

In lieu of structural panels use purlins to brace all flat  $0\ensuremath{\mathrm{C}}\xspace$  . (A) Continuous lateral bracing equally spaced on member. TC @

WARNING: Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



ITW Building Components Group

ALPINE

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN. FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH IP: OR FARBLEACHING, INSTALLING, SHALLING & BRACIENG OF TRUSSES.

DESIGN COMPORES WITH APPLICABLE PROVISIONS OF NDS (MATIONAL DESIGN SECC. N. \*AFRA) AND TPI. IT BCG CONNECTION PLATES ARE MADE OF 20/18/166A (M.H/955/M) ASIM A653 GRADE 40/60 (M. K/M.SS) GALY. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERHISE LOCATED ON THIS DESIGN, POSITION OF REDWATHES 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE FER ANNEX A3 OF TPIL-2002 SEC.3. A SEAL ON THIS DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR MAY BUILDING IS THE RESPONSIBILITY OF THE

COSTONAL ENGINEE

BC DL TC DL

10.0 PSF 0.0 PSF

DRW HCUSR8228 08148023

10.0

PSF

DATE REF

05/27/08

R8228-

67240

80

DUR.FAC.

TOT.LD.

40.0 1.25 24.0"

PSF

HC-ENG

DF / DF

170279

SPACING

JREF -FROM SEQN-

1THR8228Z03

Haines City, FL 33844 FL COA #0.278

DESIGN SHOWN. THE SUITABILITY BUILDING DESIGNER PER ANSI/TPI I

AND USE OF THIS COMPONENT SEC. 2.

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 Filler 2x4 SP #3 :Lt Slider 2x6 SP #2: BLOCK LENGTH =

Roof overhang supports 2.00 psf soffit load

Calculated horizontal deflection is 0.11" due to due to dead load. live load and 0.17"

Laterally brace BC at 24" OC in lieu of rigid ceiling. Laterally brace BC above filler at 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Laterally brace BC above filler @ 24" O.C. Including a lateral brace at chord ends.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART.\_ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

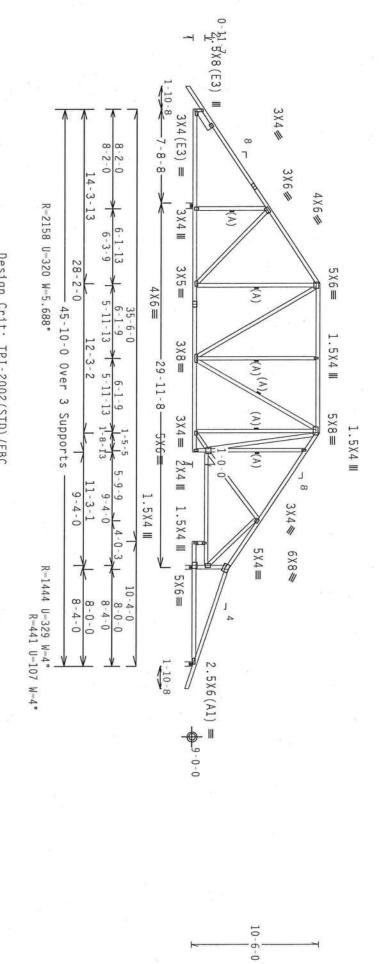
Wind reactions based on MWFRS pressures

DWGS TCFILLER0207 and BCFILLER0207 for filler details

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat OC. TC (0)

WARNING: Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



ISSSES REQUIRE EXTREME CARE IN FARRICATION. INABDLING. SHIPPING, INSTALLING AND BRACING, (BUILDING COMPONENT SAFETY HEOMAKING), DUBLISHED BY TPJ LITUSS PLATE INSTITUTE, 28 17. SUITE 312, ALEKANDRIA, YA, 25314) AND YEA, (MODD TRUSS COUNCIL OF AMERICA. UNLESS A. MADISON, AH 23719) FOR SAFETY PRACTICES PRIDM TO PERFOMPHING THESE FUNCTIONS. UNLESS MIED TOP CHOOD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHOOD SHALL HAVE Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0)

TYP.

Wave

MORTH LEE STREET, SUITE 3: ENTERPRISE LANE, MADISON, OTHERWISE INDICATED TOP CO

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL NOT BE RESPONSIBLE FOR NAY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE BRUSS IN COMPORNANCE WITH IP). OR FARBLECTHE, UNDOLLING, SHEPPIC, INSTALLING A BRAILE OF TRUSSES.

DESIGN CONFIDENCE WITH APPLICABLE PROVISIONS OF MOS (MAITONAL DESIGN SPEC, BY AFAPA) AND TPI. ITH BCG CONNECTOR PLATES ARE MADE OF 20/18/156A, (M.H/SS)K), ASTH A653 GABDE 40/60 (M.XH.SS) GALV. SITEL, APPLY DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENG OTHERWISE LOCATED ON THIS DESIGN. POSITION PER DRAWINGS 160A-Z
HALL BE PER ANNEX AS OF TPIL-ZOOZ SEC.3. A SEAL ON THIS

GOUGLAS FLET SONAL BAGALET CENS 80 BC DL TC DL DUR.FAC. TC LL TOT.LD. FL/-/4/-/-/R/-20.0

SPACING 40.0 10.0 24.0" 1.25 10.0 PSF 0.0 PSF PSF PSF PSF FROM SEQN-DATE REF JREF -HC-ENG DRW HCUSR8228 08148024 R8228-1THR8228Z03 DF / DF 05/27/08 170268 67241

Scale

=.125"/Ft.

TW Building Components Group Haines City, FL 33844 FL COA #0 278 ALPINE

PLT TYP. Laterally brace BC at 24" OC in lieu of rigid ceiling. Laterally brace BC above filler at 24" OC. Roof overhang supports 2.00 psf soffit load :Lt Slider 2x6 SP #2: BLOCK LENGTH = Top chord 2x4 Bot chord 2x4 Laterally brace BC above filler @ 24" O.C. Including a lateral brace at chord ends. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. See DWGS TCFILLER0207 and BCFILLER0207 for filler details (8-140--OWNER BUILDER Baker op chord 2x4 SP ot chord 2x4 SP Webs 2x4 SP Filler 2x4 SP TW Building Components Group 0-21.5x8(E3) III Haines City, FL 33844 FL COA #0.278 ALPINE Wave 1-10-8 5-8-8->  $3X4(E3) \equiv$ 3X4 ≠ 3X7 ≠ Dense 5-11-6 \*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BEG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEFIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN COMPORNANCE WITH TPI; OR FARELATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN COMPORNS WITH APPLICABLE PROVISIONS OF NOS (MATIONAL DESIGN SPEC, BY AFAPA) AND TPI.

CONNECTOR PLATES ARE MADE OF 20/18/16GA (W. M/MSS/F) ASTH AGGS GRADE 40/60 (W. K/M,SS) GAVE TO THE RESIGN FOR THE BRANDING 16GA-7. \*\*MARNING\*\* REUSSES REQUIRE EXTREME CAME IN FAMBICATION, IMMOLING, SHIPPING, INSTALLING AND BRACING. RETER TO BESI (BULLDING COMPONIENT SAFETY INFORMATION), PUBLISHED BY TPT (TRUSS PLATE INSTITUTE, 218 MORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, ZZ313) AND NTCA (MODD TRUSS COUNCIL OF AMERICA, 6300 ENTEROPISE LANE, MAJISON, NI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS OTHERWISE INFORMATION CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERWI ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE DRAWING INDICATES ACCEPTANCE 3×6/ R-2007 U-330 W-5.688" 4-3-13 3X4 III 8-6-3 A5) Design Crit: 28-2-0 3×5≡ = 9 X 9 (A) 4 X 6 ≡ 5-11-13 45-10-0 Over 3 6-0 S (MATIONAL DESIGN SPEC, BY ATER) AND TP1.

111 8G5
1) ASTH A653 GRADE 40/60 (W. K/H. SS) GALVE. STEEL, APPLY
SE LOCATED ON THIS DESIGN, POSITION PER DRAMINGS 166A-Z.
FER ARMEX A3 OF TP1-2002 SEC.3.

A SEAL ON THIS
STREET, TRUSS COPPONENT
COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE 1.5X4 III 3 X 8 ≡ TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/0(0) 12-3-2 31-11-8 3 5-11-13 Supports 5 X 8 ≡ 8 3×4≡ 1-8-13 1.5X4 III 1-5-5 5×6= (A) 700 ₹×4 III 9-9 8 3-1 9-4-0 .5X4 Ⅲ 3X4 // WARNING: Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below. 1.5X4 III In lieu of structural panels use purlins to brace all flat TC @ OC. Wind reactions based on MWFRS pressures. 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART.\_ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL-5.0 psf. Iw-1.00 GCpi(+/-)-0.55 Continuous lateral bracing equally spaced on member 5 X 4 ≡ 6X8₩ OOUGENS FLEMING R-1595 U-352 W-4" R-441 U-107 W-4" SONAL ENGINE 5 X 6 ≡ 10-4-0 8-0-0 CENS 8-4-0 8-0-0 8-4-0 1-10-8 2.5X6(A1) =80 BC LL BC DL TC DL TC LL DUR.FAC. SPACING TOT.LD. FL/-/4/-/-/R/-10.0 20.0 40.0 10.0 24.0" 1.25 0.0 PSF PSF PSF PSF PSF DATE REF JREF -FROM SEQN-HC-ENG DRW HCUSR8228 08148025 Scale =.125"/Ft. R8228-1THR8228Z03 DF / DF 05/27/08 170272 10 67242 6-0

בזיורוויז ביים בי יווחים נוו עי

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 :Lt Slider 2x6 SP #2: BLOCK LENGTH = PLT WARNING: Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below. In lieu of structural panels use purlins to brace all flat TC @ 0C. Roof overhang supports 2.00 psf soffit load (8-140--OWNER BUILDER Baker TYP. ALPINE Wave 2.5X8(E3) III 1-10-8  $3X4(E3) \equiv$ 3X4# R=1658 U=355 W=4" -6-6-0-\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BGG, INC. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN; AFF FAILURE TO BUILD THE TRUSS IN COMPORNANCE WITH FP: OR FABELFAITHG. HANDLING, SHAPPIG, INSTALLING A BRACIEG OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROPISIONS OF THOS (MATIONAL DESIGN SECE, BY AFREYA) AND TP:. ITH BGG CONNECTOR PLATES ARE MADE OF ZO/IM/166A (M.M/SS/K) ASIM A653 GRADE 40/60 (M.K/M.SS) GALY. STEEL APPLY PLATES TO EACH FACE OF TRUSS AND. UNICSS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWHENS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX AS OF TPI1-2002 SEC. 3. A SEAL ON THIS DESIGN SHOWN. THE SULTABLILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE REFERENCE INVESTS REQUIRE EXTREME CARE IN FABRICATION, IMADLING, SHIPPING, INSTALLING AND BRACKING, BROWN IN THE STREET, SUITE 312, ALEXANDRA, VA, 22314) AND UTC. 4000D TRUSS COUNCIL OF AMERICA, 6300 CHYCHROLIS LAND, MADISON, WI 15719) FOR SAFETY PRACTICES PRIOR TO PERFORM HIS LIVELY TO THE CORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS, AND BOTTON CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS, AND BOTTON CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTON CHORD SHALL HAVE 6X8#8 3 X 4 III 14-3-13 -7 - 9 - 13 -1.974 A6) Design Crit: 3 X 5 ≡ 6X6= 6-1-9-4 X 6 ≡ 45-10-0 Over 2 1.5X4 Ⅲ 3 \ 8 ≡ 12-3-2 TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0) Ŝ Supports A 5 X 6 = 3X4 =¥←5-11-4-¥←5-3-12-> R=193 PLF 1.5X4 Ⅲ 4 X 6 = 11 - 3 - 14X10 110 mph wind, 15.00 ft mean hgt, ASCE anywhere in roof, CAT II, EXP B, wind psf. Iw=1.00 GCpi(+/-)=0.55 Wind reactions based on MWFRS pressures Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. (A) Continuous lateral bracing equally spaced on member U=43 PLF 5...£X8..... 5×6 # GOUD AS FLEN USIONAL ENGINEE W=12-4-08-0-0 8-0-0-2.5X6(A1) 1-10-8 III BC DL TC DL TC TOT.LD. FL/-/4/-/-/R/-7-02, PART.\_ENC. bldg, Located TC DL-5.0 psf, wind BC DL-5.0 10.0 20.0 40.0 10.0 PSF 0.0 PSF PSF PSF PSF DATE REF SEQN-HC-ENG DRW HCUSR8228 08148007 Scale = .125"/Ft. R8228-DF / DF 47039 05/27/08 10 6-0 67243

TW Building Components Group

Haines City, FL 33844 FL COA #0.278

DESIGN SHOWN. THE BUILDING DESIGNER PER

AND USE OF THIS COMPONENT SEC. 2.

80

DUR.FAC

1.25

FROM

SPACING

24.0"

JREF -

1THR8228Z03

Top chord 2x4 SP #2 Dense Bot chord 2x6 SP #2 :B3 2x6 SP #1 Dense: Webs 2x4 SP #3 :W11 2x4 SP #2 Dense: :Stack Chord T2 2x4 SP #2 Dense: :Stack Chord T7 2x4 SP #2 Dense: :Lt Slider 2x6 SP #2: BLOCK LENGTH = 1.9

Roof overhang supports 2.00 psf soffit load

In lieu of structural panels use purlins to brace all flat TC @  $0\mathrm{C}_{\cdot}$ 

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

+ MEMBER TO BE LATERALLY BRACED FOR WIND LOADS PERPENDICULAR TO TRUSS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

GABLE END SI DESIGNED 0 SUPPORT 8" MAX RAKE OVERHANG

See DWGS A13015EE0405 & GBLLETIN0405 for more fequirements

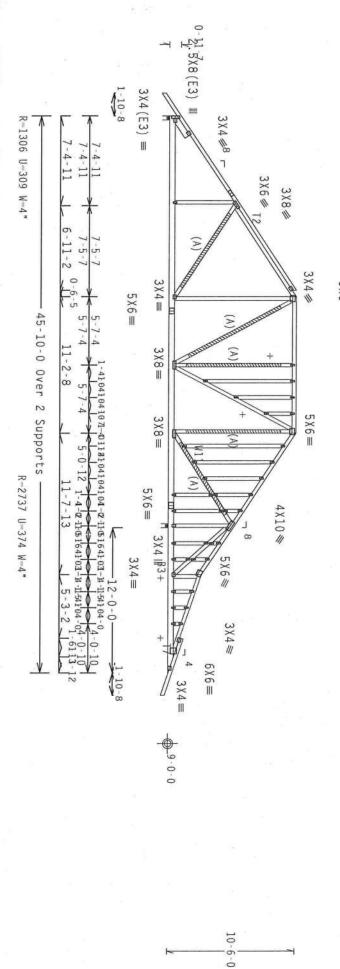
> 110 mph wind, 15.00 ft mean hgt, ASCE anywhere in roof, CAT II, EXP B, wind psf. Iw=1.00 GCpi(+/-)=0.55 7-02, PART.\_ENC. bldg, Located TC DL-5.0 psf, wind BC DL-5.0

Wind reactions based on MWFRS pressures

(A) #3 or better scab brace. Same size & 80% length of Attach with 10d Box or Gun (0.128"x3",min.)nails @ 6" ( f web member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.



Note: All Plates Are 1.5X4 Except As Shown.

TYP.

Wave

\*WARNING\*\* TRUSSES REQUIRE EXTREME Design Crit: TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/ /0(0)

A PROPERLY ATTACHED RIGID CEILING

\*\* IMPORTANT \*\* TREMENSE A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG. THE. SHALL NOT BE RESONSTABLE FOR ANY DEVIATION FROM HIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH THIS OR FARRICATING, MANDLING. SHIPPING, INSTALLING A BRACING OF TRUSSES.

DESIGN CONTROLS ARE MOSE TO FOR/BIJONA (M.H/SS/M.) ASTH AGS BRACE 40/60 (M.K.P.M.S)A AND TPI. CONTROLS ARE AGD. TO FOR THE TOP TO THE TOP PLATES TO EACH FACE OF TRUSS AND.
ANY INSPECTION OF PLATES FOLLOWED DRAWING INDICATES SEAL ON THIS

ITW Building Components Group

ALPINE

Haines City, FL 33844 FL COA #0 778

BUILDING DESIGNER PER ANSI/TPI 1

SOU ICENS CENSE No. 66648 REINEER 80 BC DL TC LL TC SPACING DUR.FAC. TOT.LD. FL/-/4/-DL 10.0 /-/R/-10.0 20.0 24.0" 1.25 40. 0.0

.

JREF -FROM SEQN-

1THR8228Z03

PSF PSF PSF

HC-ENG

DF / DF

170275

DRW HCUSR8228 08148008

PSF

R8228-

67244

Scale =.125"/Ft.

DATE REF

05/27/08



Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 :Lt Slider 2x6 SP #2: BLOCK LENGTH =

1.974

Roof overhang supports 2.00 psf soffit load

In lieu of structural panels use purlins to brace all flat TC @ 24"  $\,$  0C.

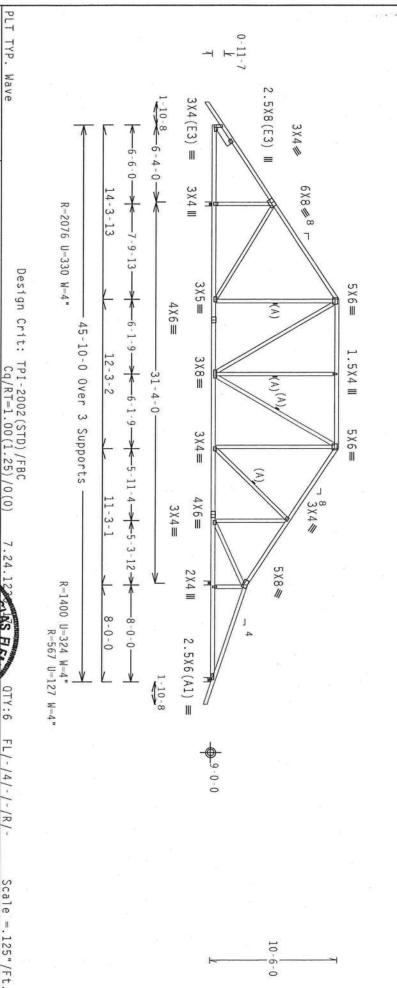
WARNING: Furnish a copy of this DWG to the installation contractor Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART.\_ENC. bldg, anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC psf. Iw=1.00 GCpi(+/-)=0.55 Located DL=5.0

Wind reactions based on MWFRS pressures

(A) Continuous lateral bracing equally spaced on member

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



A PROPERLY ATTACHED RIGID CEILING

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TIM BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BHILD THE TRUSS IN COMPORNANCE WITH IPI. OR FARRICATING, HANGLING, SHEPPIG, INSTALLING A BRACHING OF TRUSSES, DESIGN CONTROLATING, THE PROPISIONS OF HOS (MATIONAL DESIGN SPEC, BY ATREA) AND IPI. ITW BCG CONNECTION PRITES ARE HADE OF 20/18/16/CAG (ALM 1/55/X) ASTH A653 GRADE 40/60 (M. K/M.SS) GALV. STEEL APPLY PLATES TO EACH FAGE OF TRUSS AND. UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 156A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A.3 OF 1911-2002 SEC.3. A STAL ON THIS DRAWING INDICANES ACCOMPONENT FOR MAY BUILDING IS THE RESPONSIBILITY OF THE

TW Building Components Group

ALPINE

Haines City, FL 33844 FL COA #0 778

AND TOPIONAL ENGRAL SOUS AS FLEN 17 80 BC LL BC DL TC DL TC LL DUR.FAC. SPACING TOT.LD. 10.0 20.0 40.0 10.0 24.0" 1.25 0.0 PSF PSF PSF PSF PSF REF JREF -FROM SEQN-DATE HC-ENG DRW HCUSR8228 08148026

DF / DF 170250

1THR8228Z03

R8228- 67245

05/27/08

Top chord 2x4 Bot chord 2x6 Webs 2x4 SPSP #2 Dense #2 #3 :W44 2

:W44 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load

(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3",min.)nails @ 6" 0C.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MEMBER TO BE LATERALLY BRACED FOR WIND LOADS PERPENDICULAR TO TRUSS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

GABLE END IS DESIGNED TO SUPPORT 8" MAX RAKE OVERHANG

See DWGS A13015EE0405 & GBLLETIN0405 for more requirements

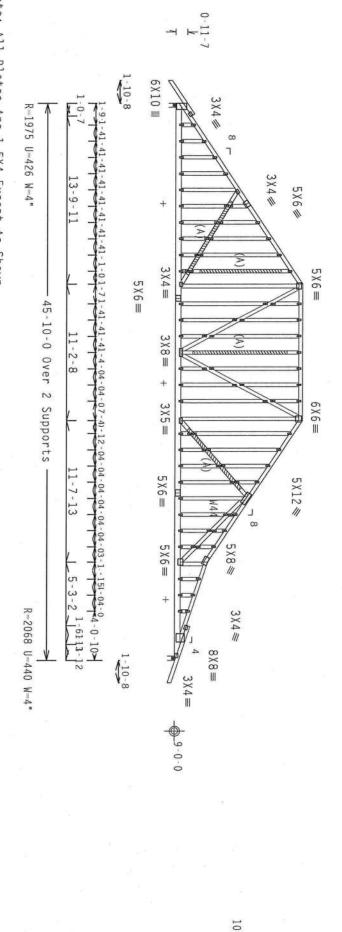
110 mph wind, 15.00 ft mean hgt, ASCE anywhere in roof, CAT II, EXP B, wind psf. Iw=1.00 GCpi(+/-)=0.55 7-02, PART.\_ENC. bldg, Located TC DL=5.0 psf, wind BC DL=5.0

Wind reactions based on MWFRS pressures

In lieu of structural panels use purlins to brace all flat TC 0C. 0

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.



6-0

Note: All Plates Are 1.5X4 Except As Shown. Design Crit:

PLT

TYP.

Wave

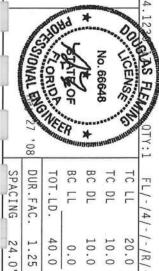
NORTH LEE STREET, SUITE 3 ENTERPRISE LANE, MADISON, OTHERWISE INDICATED TOP C A PROPERLY ATTACHED RIGID CEILING \*WARNING\*\* TRUS INSSES REQUISE EXTREME CARE IN FARRICATION. HANDLING. SHIPPING, INSTALLING AND BACHG.
(BUILDING COPPORTENT SAFETY INFORMATION, PUBLICISED BY TEP (TRUES PLATE INSTALLING BACHG.)
(SUITE 312, ALEXANDRIA, YA, 22314) AND WICA, (MOOD TRUES, COUNCIL OF AMERICA. (6300)
(MODISON, MI S2719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE DUNCTIONS. UNLESS
ANDER TO CHORD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/ /0(0)

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FALLURE TO BUILD THE TRUSS IN COMPORNANCE WITH TPI; OR FARELACHING, HANDLING, SHEPPING, INSTALLING A BRACHING OF TRUSSES, DESIGN CONTROLS WITH APPLICABLE PROVISIONS OF THIS CONTROLS SPEC, BY AFRA, AND TPI. THE BCG CONNECTOR PLATES ARE MADE OF 20/18/166A (M.H/SS/N) ASTM A653 GRADE 40/06 (M. K/M/SS) GALV. STEEL, APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERHISE LOCATED ON THIS DESIGN, POSITION PER DRAWNINGS 166A-Z, DAY IN THE STEEL APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERHISE LOCATED ON THIS DESIGN, POSITION PER DRAWNINGS 166A-Z, ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX AS OF THIS DESIGN, POSITION PER DRAWNINGS 166A-Z, ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX AS OF THIS DESIGN. ANY INSPECTION OF PLATES FOLLOHED BY (1) SHALL BE DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL FUR. OZ SEC.3. A SEAL ON THIS SOLELY FOR THE TRUSS COMPONENT NG IS THE RESPONSIBILITY OF THE

TW Building Components Group

ALPINE

Haines City, FL 33844 FL CO's #0 278



PSF PSF

DRW HCUSR8228 08148027

DATE REF

05/27/08

PSF

R8228-

67246

Scale = .125"

/Ft.

PSF

HC-ENG

DF / DF

170182

PSF

FROM SEQN-

JREF -

1THR8228Z03

TC DL=5.0

located psf.

110 mph wind, 15.94 ft mean hgt, ASCE 7-02, CLOSED within 4.50 ft from roof edge, CAT II, EXP B, wind wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures. See DWG VALTRUSS0207 for valley details.

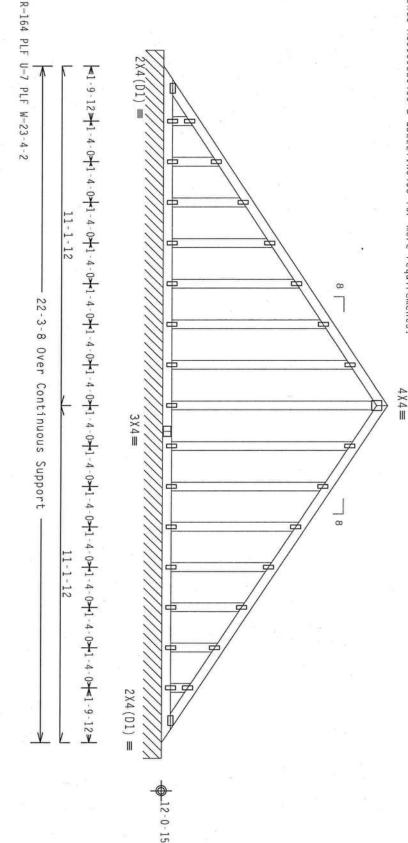
Top chord 2x4 SP Bot chord 2x4 SP Webs 2x4 SP #2 Dense #2 Dense #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.

In lieu of structural panels use purlins to brace all flat TC @ 0C.

See DWGS A13030EE0405 & GBLLETIN0405 for more requirements 4 X 4 ==



Note: All Plates Are 1.5X4 Except As Shown.

TYP.

Wave

Design Crit: TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/0(0)

FL/-/4/-

/-/R/-

R8228- 67247

05/27/08

A PROPERLY ATTACHED RIGID CEILING

\*\*IMPORTANT\*\*TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN FAR FAILURE TO BUILD THE TRUSS IN COMFORMAGE WITH THIS DESIGN CONFORMAGE WITH ADDITION, SHIPPING, INSTALLING A BRACING OF TRUSSES.

DESIGN COMFORTS WITH ADPLICANT PROVISIONS OF NOS (MATIONAL DESIGN SPEC, BY AFRA) AND THIS CONNECTOR PLATES ARE MADE OF 20/18/166A (M.H/SS/K) ASTN A653 GRADE 40/60 (M.K/M.SS) GAVE. STEEL APPLY CONNECTOR PLATES ARE MADE OF 20/18/166A (M.H/SS/K) ASTN A653 GRADE 40/60 (M.K/M.SS) GAVE. STEEL APPLY DRAWING INDICATES ACCEPTANCE PLATES OF PLATES FOLLOWED SIGN SPEC, BY AFRPA) AND TPI. ITH BCG
KADE 40/60 (W. K/H.SS) GALV. STEEL. APPLY
THIS DESIGN, POSITION PER DRAWINGS 160A.Z. SEAL ON THIS

TW Building Components Group

ALPINE

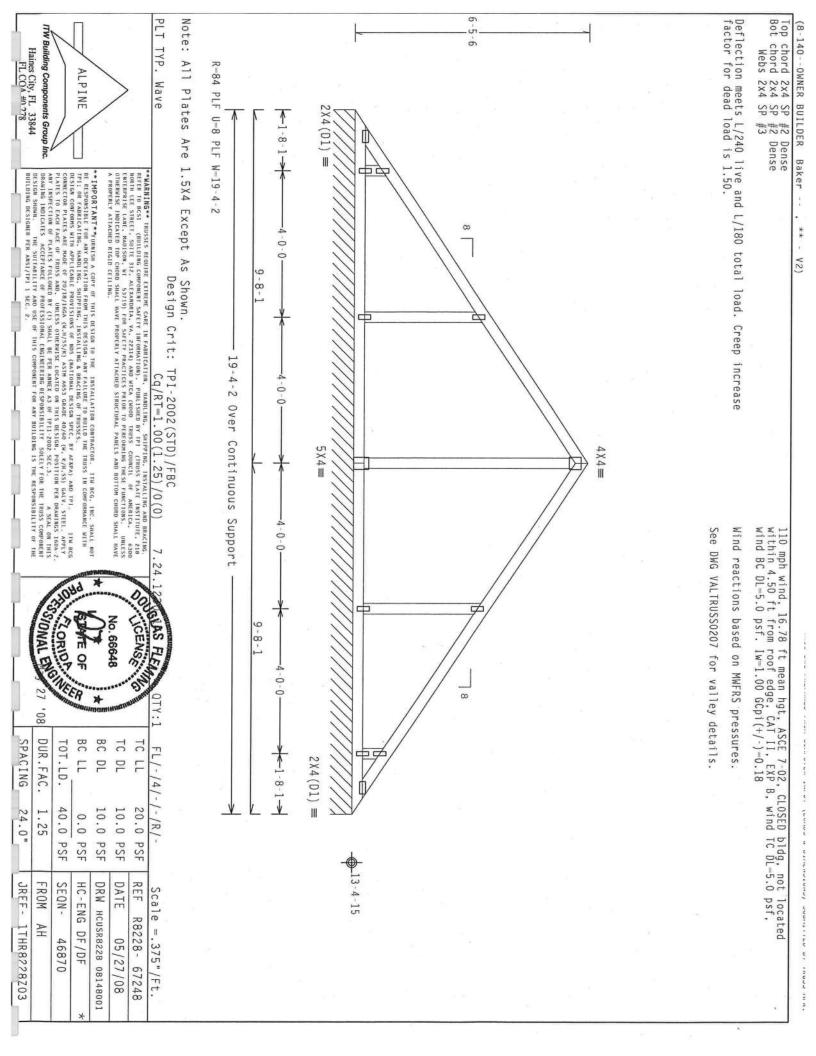
FL CO<sup>A</sup> #0 278

SOLELY FOR THE TRUSS COMPONENT
NG IS THE RESPONSIBILITY OF THE SON ICENS STONAL CENSE No. 66648 CORIOR ELINER MER 80 BC DL TC DL TC LL DUR.FAC. SPACING TOT.LD. SEE 40.0 10.0 1.25 10.0 20.0 0.0 ABOVE PSF PSF PSF PSF PSF JREF -FROM SEQN-DATE REF HC-ENG DRW HCUSR8228 08148009 Scale =.3125"/Ft.

DF / DF

46877

1THR8228Z03



Top chord 2x4 SP Bot chord 2x4 SP Webs 2x4 SP PLT Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. -9-6 (8-140--OWNER BUILDER Baker TW Building Components Group TYP. Wave Haines City, FL 33844 FL CO<sup>A</sup> #0.278 ALPINE #2 Dense #2 Dense #3 R-84 PLF U-9 PLF W-11-4-2 \*\* IMPORTANT\*\* "SUBMISH A COPY OF THIS DESIGN TO THE HISTALLATINE CONTRACTOR. IT WEGG, INC. SHALL N
BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH
THIS OR FARBICATING, NAMBLING, SHIPPING, HISTALLING & BRACING OF TRUSSES.
DESIGN COMPORES WITH APPLICABLE PROVISIONS OF NOS (MATIDNAL DESIGN SPEC, BY AFFA) AND TPI,
CONNECTOR PLATES ARE MODE OF 20/181/160A, (4/1/1528) ASTH ASS GRADE 40/40 (4/ K/N/152) GALV, STEEL APPL DESIGN SHOWN. THE BUILDING DESIGNER PER PLATES TO EACH FACE OF TRUSS AND.
ANY INSPECTION OF PLATES FOLLOWED
DRAWING INDICATES ACCEPTANCE OF P GUIRE EXTREME CARE IN FARRICATION, INABLING, SHIPPING, INSTALLING AND BRACING, NG COMPONENT SAFETY INFORMATION). PUBLISHED BY POT (TRUSS PLANE INSTITUTE, 218 112, ALEXANDRIA, WA. 22314) AND NYCA, (MODO TRUSS COUNCIL OF AMERICA, 6300 MW. NI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS. CHOSED SHALL HAVE ST. Design Crit: 中文 1.5X4 III 5-8-1 NDS (NATIONAL DESIGN SPEC, BY AFRPA) AND TPI. ITW BCG 'SS/K) ASTM A653 GRADE 40/60 (W. K/H.SS) GALV. STEEL, APPLY TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/ 4-0-0-11-4-2 Over Continuous Support DZ SEC.3. A SEAL ON THIS SOLELY FOR THE TRUSS COMPONENT NG IS THE RESPONSIBILITY OF THE 4×4= .5X4 中 (0)0/ 110 mph wind, 18.11 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18 Wind reactions based on MWFRS pressures. See DWG VALTRUSS0207 for valley details. 4-0-0-8 OSIONAL ENGINEE -8-1 CENS No. 66648 1.5X4 III 中 5 2X4(D1) = 80 BC DL BC LL TC DL TC LL DUR.FAC. SPACING TOT.LD. FL/-/4/-/-/R/-10.0 40.0 10.0 20.0 PSF 24.0" 1.25 0.0 \_16-0-15 PSF PSF PSF PSF JREF -FROM SEQN-DATE REF HC-ENG DRW HCUSR8228 08148003 Scale = .5"/Ft. R8228- 67250 1THR8228Z03 DF / DF 05/27/08 46860

mois com usen antus (comos a usrenssuna) subrisseu de inuss rich.

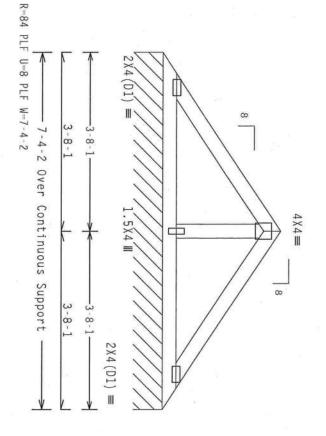
Top chord 2x4 SP #
Bot chord 2x4 SP #
Webs 2x4 SP # #2 Dense #2 Dense #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 18.78 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

See DWG VALTRUSS0207 for valley details.



Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0)

FL/-/4/-/-/R/-

Scale = .5"/Ft.

R8228- 67251

PSF

DATE REF

05/27/08

PLT

TYP.

Wave

A PROPERLY ATTACHED RIGID CEILING

\*\*IMPORTANT\*\*\*URNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL HOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN COMPORNANCE WITH TP: OR FARBICATION, INSTALLING, INSTALLING & BRACTING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF HOS (MATIONAL DESIGN SPEC, BY AFRA) AND TPI. THE BCG CONNECTION PLATES ARE MODE OF ZO/BAJIONA (M. MISSEY) ASTH MCS GRADE 40/50 (M. M. JESSEY) ASTH MCS GRADE 40/50 (M. JESSEY) AND TPI. PLATES 10 EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 150A-2 PLATES 10 EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 150A-2 PLATES 10 EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 150A-2 ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEED DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMP. BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2. IN DG (WATIONAL DESIGN SPEC, BY AFREN) AND TPI.

SE LOCATED ON THIS DESIGN, FOSTION PER ORANINGS 16GA-Z.

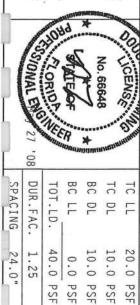
FOR ANIEX AS OF TPI1-2002 SEC.3.

SPER ANIEX AS OF

TW Building Components Group Inc.

ALPINE

Haines City, FL 33844 FL CC \* #0 778



40.0 1.25 24.0" 10.0 PSF 0.0 PSF PSF JREF -FROM SEQN-HC-ENG DRW HCUSR8228 08148004 1THR8228Z03 DF / DF 46856

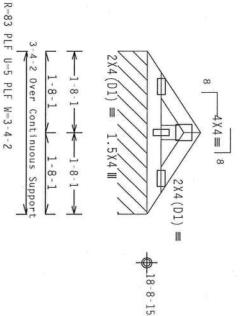
Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 19.45 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures

See DWG VALTRUSS0207 for valley details



Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0)

FL/-/4/-/-/R/-

Scale =.5"/Ft.

R8228- 67252

PLT TYP.

Wave

\*\*MARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FARRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING, REFER TO BOSI (BUILDING COMPONIENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, Z2314) AND NICA (MODO TRUSS COUNCIL OF AMERICA, 6300 ENTERGRISE LAKE, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS OTHERWISE INDICATED TOP COMOS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH TP: OR FARBICATING, MANUFULOR, SHIPPING, HESTALLING & BRACTING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF RDS (MATIONAL DESIGN SPEC, BY ATAPA) AND TP: DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF RDS (MATIONAL DESIGN SPEC, BY ATAPA) AND TP: APPLY APP

DESIGN SHOWN. THE SUITABILITY AND USE OF BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNE DRAWLING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING

TW Building Components Group Inc. Haines City, FL 33844 FL COA #0 278

ALPINE

COURT FLEA SONAL BUGINE CENSE No. 66648 80 BC DL TC DL DUR.FAC. BC LL TC LL SPACING TOT.LD. 20.0 40.0 10.0 1.25 24.0" 10.0 PSF 0.0

PSF PSF

HC-ENG

DF / DF 46852

DRW HCUSR8228 08148005

JREF -FROM SEQN-

1THR8228Z03

PSF PSF

DATE REF

05/27/08

(MATIONAL DESIGN SPEC, BY MYEAD, AND TPL.

THA UGG.) ASTM A653 GRADE 60/60 (M. K/H.SS) GALY. STEEL. APPLY
SE LOCATED ON THIS DESIGN, POSITION FRO DRAKINGS 160A-Z.
PER AMRIX A3 OF TPL-2002 SEC.3. A SEAL ON THIS
SHEEKING RESPONSIBILITY SOLELY FOR THE TRUSS COPPONENT
COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

Top chord 2x4 SP + Bot chord 2x4 SP + Webs 2x4 SP + #2 Dense #2 Dense #3

Roof overhang supports 2.00 psf soffit load

(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3",min.)nails @ 6" 0C.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MEMBER TO BE LATERALLY BRACED FOR WIND LOADS PERPENDICULAR TO TRUSS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

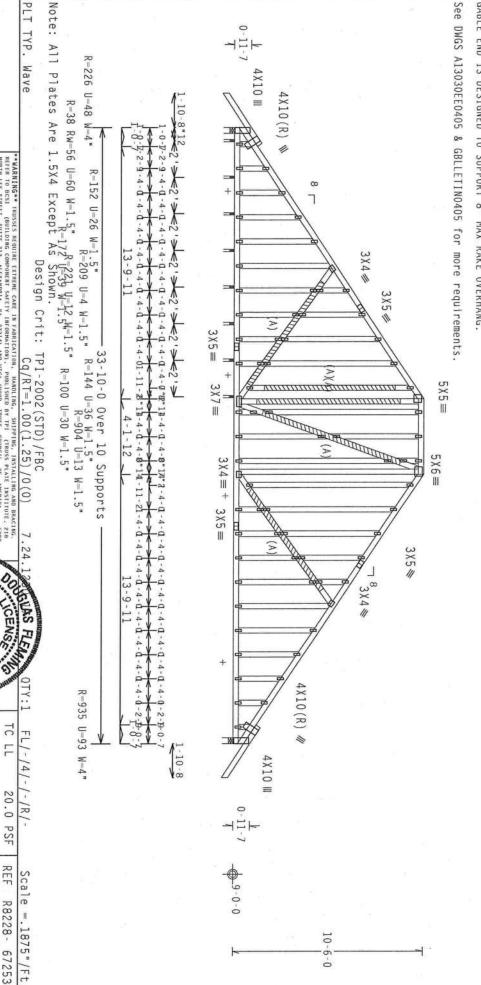
GABLE END SI DESIGNED TO SUPPORT 8" MAX RAKE OVERHANG

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat  $0\text{C}_{\scriptscriptstyle{\bullet}}$ TC @

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WAL AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER. SHEAR WALLS.



A PROPERLY ATTACHED RIGID CEILING

\*\*IMPORTANT\*\*GURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FALLURE TO BUILD THE TRUSS IN COMPORNANCE MITH IP: OR FARELGATHG, ANNOLING, SHIPPING, INSTALLING A BRACING OF TRUSSES.

DESIGN CONFORTS WITH APPLICABLE PROVISIONS OF BDS (MATIONAL DESIGN SPEC, BY AEAPA) AND TPI. ITH BCG CONFECTOR PLATES ARE MADE OF 20/18/106A (M. M. MSSM.) ASTH AGS GRADE 40/50 (M. K.M. SSS, SCALLY STEEL, APPLY PLATES TO EACH FACE OF TRUSS AND, UNITES OTHERWISE LOCATED DN THIS DESIGN POSITION PER DRAMING SHOOT STEEL APPLY DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLICLY FOR THE TRUSS COMPONERY

SONAL ENGINE CENSE lo. 66648 JE OF 80 BC DL TC DL TC LL DUR.FAC SPACING TOT.LD. 10.0 20.0 40.0 10.0 PSF 0.0

PSF

DATE REF

05/27/08

PSF

R8228-

67253

24.0" 1.25 FROM JREF -1THR8228Z03

PSF PSF

SEQN-

HC-ENG

DF / DF 46919

DRW HCUSR8228 08148010

DESIGN SHOWN. THE BUILDING DESIGNER PER AND USE OF THIS COMPONENT SEC. 2. SOLELY FOR THE TRUSS COMPONENT NG IS THE RESPONSIBILITY OF THE

TW Building Components Group

ALPINE

Haines City, FL 33844 FL CO<sup>A</sup> #0 778

Top chord 2x4 SP #
Bot chord 2x4 SP #
Webs 2x4 SP # PLT TYP. In lieu of structural panels use purlins to brace all flat TC @ 24"  $0\text{C}_{\cdot}$ . :Lt Slider 2x6 SP"#2: BLOCK LENGTH = (A) (8-140--OWNER BUILDER Baker TW Building Components Group Inc. Continuous lateral bracing equally spaced on member. Haines City, FL 33844 FL COA #0.278 ALPINE Wave #2 Dense #2 Dense #3 \*\*WARNING\*\* TRUSSES REQUIRE EXTREME CA REFER TO BEST (MULLDING COMPONENT SAF WORTH LEE STREET, SUITE JIZ, ALEXANDER ENTERPRISE LANE, MADISON, HI \$3719) F OTHERMISE INDECATED TOP CHORD SHALL HAV A PROPERLY ATTACHED RIGHD CELLING. 2.5X8(E3) III \*\* IMPORTANT\*\* "BURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW EGG. HE. SHALL HOT BE RESONSTHEE FOR ANY DEVIATION FORM HIS DESIGN. ANY FALLER TO BUILD THE RUSS IN COPPORMANCE WITH FIRST ANY FALLER TO BUILD THE RUSS IN COPPORMANCE WITH PROPERTY OF THE RUSS IN COPPORMANCE WITH A REPORT OF THE RUSS IN COPPORMANCE WITH A REPORT OF THE RUSS AND THE RUSS IN COPPORMANCE WITH A REPORT OF THE RUSS AND THE RUSS IN COPPORMANCE WITH A REPORT OF THE RUSS IN COPPORMANCE WITH A REPORT OF THE RUSS AND THE RUSS IN COPPORMANCE WITH A REPORT OF THE RUSS AND THE RUSS IN COPPORMANCE WITH A REPORT OF THE RUSS AND THE RUSS IN COPPORMANCE WITH A REPORT OF THE RUSS AND THE RUSS IN COPPORMANCE WITH A REPORT OF THE RUSS AND THE RUSS IN COPPORMANCE WITH A REPORT OF THE RUSS AND THE RUSS IN COPPORMANCE WITH A REPORT OF THE RUSS AND THE RUSS IN COPPORMANCE WITH A REPORT OF THE RUSS AND THE RUSS IN COPPORMANCE WITH A REPORT OF THE RUSS AND THE RUSS IN COPPORMANCE WITH A REPORT OF THE RUSS AND THE RUSS IN COPPORMANCE WITH A REPORT OF THE RUSS AND THE RUSS IN COPPORMANCE WITH A REPORT OF THE RUSS AND THE RUSS IN COPPORMANCE WITH A REPORT OF THE RUSS AND THE RUSS IN COPPORMANCE WITH A REPORT OF THE RUSS AND THE RUSS IN COPPORMANCE WITH A REPORT OF THE RUSS IN COPPORMANCE WITH A RUSS IN COPPORMANCE WITH A REPORT OF THE RUSS IN COPPORMA BUILDING DESIGNER PER ANSI 3X4(E3) = INDICATES R=465 U=0 3X4 / UNIONES EXTREME CAME IN FARRICATION, HANDLING, SHIPPING, HEXTALLING AND BRACING.
UNIONEG COMPORATE SAETY HOMORMAIGH), PUBLISHED BY TH! (HRUSS PLAIE HEXITHIE, 218
SHITE 312, ALEXANDRIA, YA, 22314) AND WICA (MOOD TRUSS COUNCIL OF MAGRICA, 6300
ADJSON, WI S3719) FOR SAFIY PRACTICES PRIOR TO PEFORHRIGH HEXE THROTHORS. UNLESS
TO DE CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PAREES AND BOTTOM CHORD SHALL HAVE
TO DE CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PAREES AND BOTTOM CHORD SHALL HAVE 1.974' B3) Design Crit: 8 18-10-0 Over 1.5X4 Ⅲ 14-3-13 TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0) 3X4# 3X4# RUSS COUNCIL OF AMERICA, 6300 PERFORMING THESE FUNCTIONS. UNLESS PANELS AND BOTTOM CHORD SHALL HAVE 2 6-2-2 Supports A R=1118 U=195 W=4" 3 X 4 ≡ 3 3×4≡ 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi(+/-)=0.18 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Right end vertical not exposed to wind pressure Wind reactions based on MWFRS pressures 5 X 4 ≡ 5-6-0 5-4-0-4-6-3 4-6-3 CENS 8 3X4(R) Ⅲ 2X4 III QTY:4 80 BC LL BC DL TC DL DUR.FAC. TC LL SPACING TOT.LD. FL/-/4/-/-/R/oren turor (como a partenatora) applitito di indaa men. 10 -6-0 40.0 10.0 20.0 24.0" 10.0 PSF 1.25 0.0 PSF PSF PSF PSF FROM DATE REF JREF -SEQN-HC-ENG DRW HCUSR8228 08148013 Scale = .25"/Ft. R8228-1THR8228Z03 DF / DF 170290 05/27/08 67254

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 :Lt Slider 2x6 SP #2: BLOCK LENGTH = :Rt Slider 2x6 SP #2: BLOCK LENGTH = 1.974'

Roof overhang supports 2.00 psf soffit load

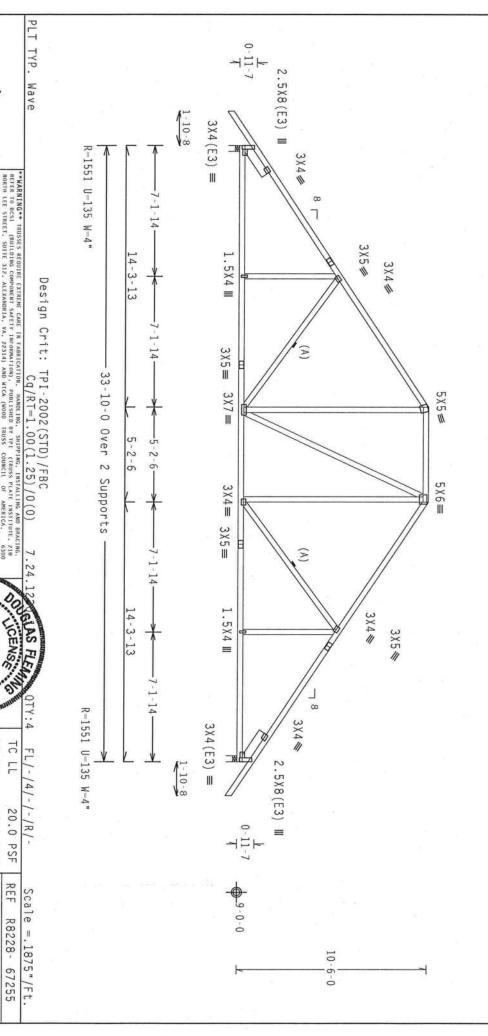
In lieu of structural panels use purlins to brace all flat TC @ 24"  $\,$  0C.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



TW Building Components Group Haines City, FL 33844 FL CO<sup>4</sup> #0 278

ANY IMSPECTION OF PLATES FOLLOWED BY (1) SHAD DRAWING INDICATES ACCEPTANCE OF PROFESSIONA DESIGN SHOWN. THE SUITABILITY AND USE OF BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

THIS COMP

ALPINE

\*\*IMPORTANT\*\*\*USHISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH THE IDEA FARRICATION, IMPAULIO, SHIPPING, INSTALLING A BRACING OF TRUSSES.

DESIGN COMPORES WITH APPLICABLE PROVISIONS OF NDS (MATIONAL DESIGN SPEC, BY ARRAY) AND IPI. APPLY PLATES TO EACH FACE OF TRUSS AND. UNITES DUBRANCE GRADE GARDE AD/SO (M. K./H.SS) GALVE. STELL APPLY PLATES TO EACH FACE OF TRUSS AND. UNITES DUBRANCE LOCATED ON THIS DESIGN, POSITION PER BRANINGS 160A-Z.

OF TRUSSES.

SIGN SPEC. 11M BCG
SIGN SPEC. 20
RADE 40/50 (W. K/H.55) GALV. STELL APPLY
THIS DESIGN, POSITION PER DRAWINGS 160A-Z.
A SEAL ON THIS
OF IP11-2002 SEC. 3.
A SEAL ON THIS

STONAL BIGINE

BC LL BC DL TC DL

0.0 PSF PSF

HC-ENG DF/DF

DRW HCUSR8228 08148014

10.0 PSF 10.0 PSF

> DATE REF

05/27/08

80

DUR.FAC.

TOT.LD.

40.0

SEQN-

170283

SPACING

24.0" 1.25

JREF-FROM

1THR8228Z03

No. 66648

TC LL

20.0 PSF

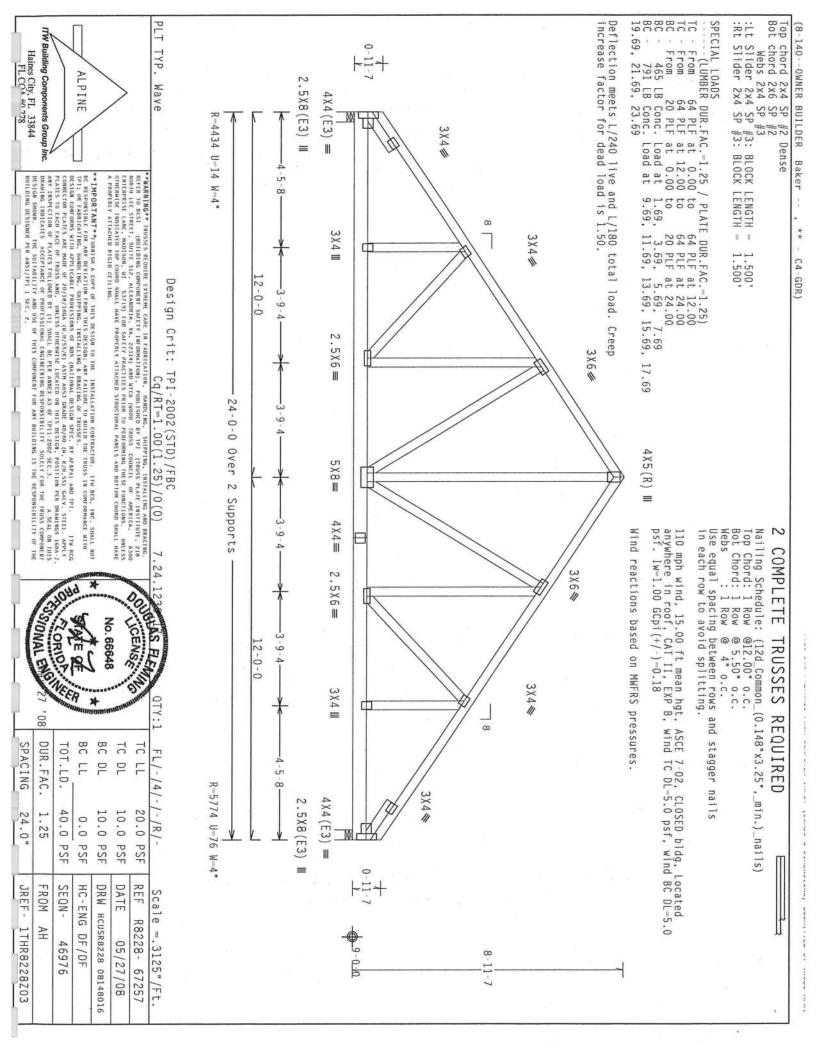
R8228- 67255

DZ SEC.3. A SEAL ON THIS SOLELY FOR THE TRUSS COMPONENT NG IS THE RESPONSIBILITY OF THE

A PROPERLY ATTACHED RIGID CEILING

PLT In lieu of structural panels use purlins to brace all flat TC @ 24"  $\,$  0C. Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 :Lt Slider 2x6 SP #2: BLOCK LENGTH = (A) Continuous lateral bracing equally spaced on member. (8-140--OWNER BUILDER Baker TW Building Components Group TYP. Haines City, FL 33844 FL CC 4 40 778 ALPINE Wave 2.5X8(E3) III \*\*IMPORTANT\*\*\*URNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT UR RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY TALLINE TO BUILD THE TRUSS IN COMPONHANCE WITH IP: OR FAMILICATION, AND INCL. INSTALLING A BRACING OF TRUSSES.

DESIGN CONFERRY WITH APPLICABLE PROVISIONS OF NOS (MATIONAL DESIGN SPEC, BY ARANA) AND IPI. THE RGG CONNECTION FLATES, ARE MADE OF ZO/187/68A (4.14/58/), ASTH AGS GRADE 40/50 (4.14/58/), ASTH AGS GRADE 40/50 (4.14/58/), ASTH AGS GRADE 50/60/68A (4.14/58/), ASTH AGS GRADE 40/50 (4.14/58/), ASTH AGS GRADE 50/60/68A (4.14/58/), ASTH AGS GRADE 40/50 (4.14/58/), ASTH AGS GRADE 50/60/68A (4.14/58/), ASTH AGS GRADE \*\*MARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FARRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING.
METER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THI (TRUSS PLATE INSTITUTE, 218
MORTH LEE STREET, SUITE 312, ALEXANDRIA, VA. 22314) AND WICA (1000 TRUSS COUNCIL OF AMERICA, 6300
ERITERRISE LANE, MAISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS
OTHERWISE INDICATED TOP COMED SMALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED RIGID CEILING. BUILDING DESIGNER PER 3X4(E3) = R=791 U=0 3X4# \* 1.974 B2) Design Crit: -1-14 00 18-10-0 Over 1.5X4 III 14-3-13 TPI-2002 (STD) /FBC Cq/RT=1.00(1.25) /0(0) 3X4# 3X4# 2 SOLELY FOR THE TRUSS COMPONENT Supports -1-14 B 3 X 4 ≡ 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18 Right end vertical not exposed to wind pressure Wind reactions based on MWFRS pressures. (A) Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. 3 X 7 = 4X4= GOUBLAS FLA 4-6-3 4-6-3 4-6-3 R-791 U-131 W-4" ONAL ENGLISE CENS No. 66648 8 3 X 4 == 2X4 III 0TY:8 80 BC LL BC DL TC DL TC LL DUR.FAC. SPACING TOT.LD. FL/-/4/-/-/R/-10 -6-0 40.0 10.0 PSF 20.0 PSF 24.0" 1.25 10.0 PSF 0.0 PSF PSF JREF -REF FROM DATE SEQN-HC-ENG DRW HCUSR8228 08148015 Scale =.25"/Ft. R8228-1THR8228Z03 DF / DF 170287 05/27/08 67256



Top chord 2x4 SP #2 Dense :T2, T3 2x4
Bot chord 2x6 SP #2 :B2 2x6 SP #1 Den
:B3 2x4 SP #2 Dense:
 Webs 2x4 SP #3
:Lt Slider 2x4 SP #3: BLOCK LENGTH =
:Rt Slider 2x4 SP #3: BLOCK LENGTH = T3 2x6 SP #1 Dense: #1 Dense: 11 11 1.500

Calculated horizontal deflection is 0.08" due to live load and 0.17" due to dead load.

BC attic room floor loading: LL = 40.00 psf; DL = 8-0-0 to 16-0-0 .

10.00 psf; from

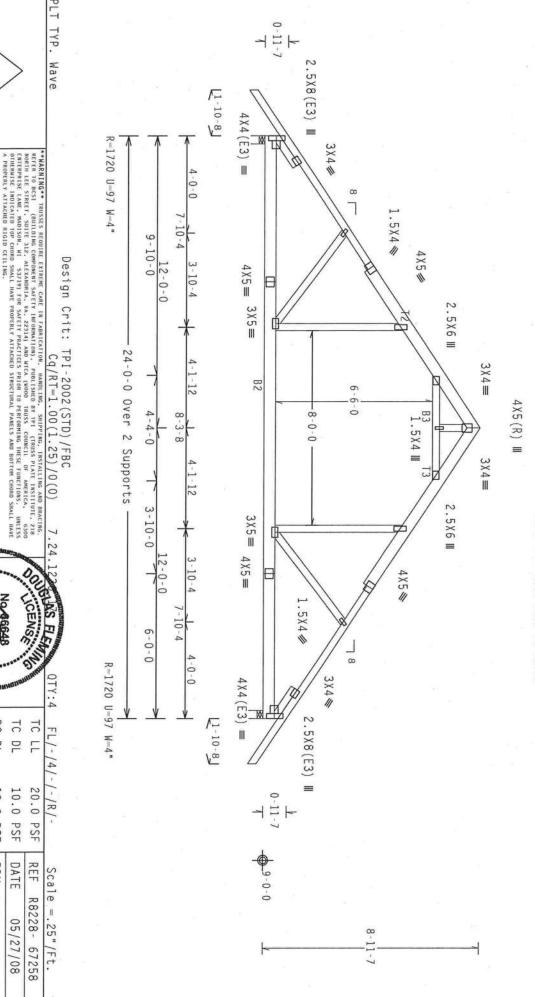
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures

Roof overhang supports 2.00 psf soffit load

Collar-tie braced with continuous lateral bracing at 24" rigid ceiling. OC. 00

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



TW Building Components Group

ALPINE

IGN SPEC. BY AFKPA) AND TPI. ITW BCG
ADE 40/50 (W. K/M.SS) GALV. STEEL. APPLY
THIS DESIGN, POSITION PER DRAWINGS 160A Z

D2 SEC.3. A SEAL ON THIS SOLELY FOR THE TRUSS COMPONENT NG IS THE RESPONSIBILITY OF THE

UNION ING

80

DUR.FAC. SPACING

24.0" 1.25

JREF -FROM

1THR8228Z03

TOT.LD.

40.0

SEQN-HC-ENG

0.0

PSF PSF

BC DL TC DL

10.0 PSF

DRW HCUSR8228 08148017

DF / DF 170239

10.0 20.0 PSF

PSF

DATE REF

05/27/08

R8228- 67258

TC LL

Haines City, FL 33844 FL CC 4 40 778

BUILDING DESIGNER PER

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING

INTO CHARLET FROM COMPUTER THEOL (LUADS & DIMENSIONS) SUBMITTED BY TRUSS MER.

Top chord 2x4 SP + Bot chord 2x4 SP + Webs 2x4 SP + #2 Dense #2 Dense #3

Roof overhang supports 2.00 psf soffit load

Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

(A) 1x4 with 8d #3 or better "L" brace. Box or Gun (0.113"x2.5", 80% length of web member. Attach ,min.)nails @ 6" OC.

See DWGS A13015EE0405 & GBLLETIN0405 for more requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

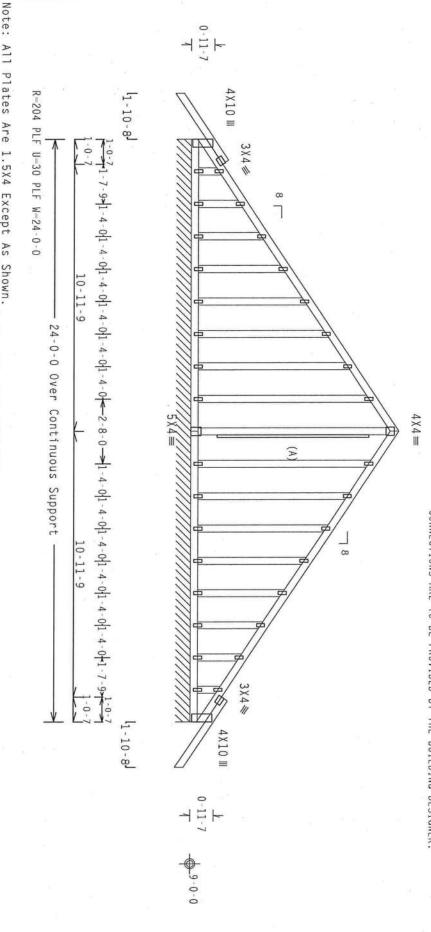
מ הדיירשיזרמין יהחמודיורה חו יצחים נוו עי

Wind reactions based on MWFRS pressures.

See DWGS A11015EE0207 & GBLLETIN0207 for more requirements

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALAND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER. SHEAR WALLS



PLT TYP.

Wave

TW Building Components Group Haines City, FL 33844 FL CO<sup>A</sup> #0.278

DESIGN SHOWN. THE BUILDING DESIGNER PER

R ANSI/TPI 1 SEC. 2.

ALPINE

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL NOT BE RESPONSIBLE FOR NAY DEVIATION FROM THIS DESIGN, ANY FALLURE TO BUILD THE TRUSS IN COMPORMANCE WITH IPI OR FARRICATING, HANDLING, SHEPPING, INSTALLING A BRACKING OF TRUSSES, DESIGN COMPORES WITH APPLICABLE PROVISIONS OF TRUS (MATIDAL DESIGN SEC. B. WIREA) AND TPI. BCG COMMERCION PLATES ARE HADE OF 20/18/166A (N.H/SS/N). ASYN A653 GRADE 40/60 (M. K/M.SS) GALV. SIELE APPLY PAATES TO EACH FACE OF TRUSS AND. UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWHMES 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER AHREX AS OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL REGIONIES IN RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABLITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

SSONAL BIGHT

80

DUR.FAC.

SPACING

SEE

ABOVE 25

JREF -

1THR8228Z03

FROM SEQN-HC-ENG

TOT.LD.

40.0

\*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, MANDLING, SHIPPING, INSTALLING AND BRACING.
RETER TO BCSI. (BUILDING COMPORENT SAFETY INFORMATION), PUBLISHED BY FPI (TRUSS SLATE HESTITUTE, 218
NORTH LEE SIREET, SUITE 312, ALEXANDRIN, VA, 22314) AND WICA (MODD TRUSS COUNCIL OF AMERICA, 6300
ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNILESS
OTHERWISE HOLICATED FOR COMOD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED RIGID CEILING.

Design Crit:

TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0)

GOUGLAS FLA

CENS No. 66648

TC LL

20.0 PSF

10.0

PSF

DATE REF

05/27/08 67261 FL/-/4/-/-/R/-

Scale =.25"/Ft. R8228-

BC DL TC DL

10.0 PSF 0.0

DRW HCUSR8228 08148020

DF / DF 47047

BC LL

PSF PSF

Roof overhang supports 2.00 psf soffit load

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,.$ 

+ MEMBER TO BE LATERALLY BRACED FOR WIND LOADS PERPENDICULAR TO TRUSS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

GABLE END IS DESIGNED TO SUPPORT 8" MAX RAKE OVERHANG

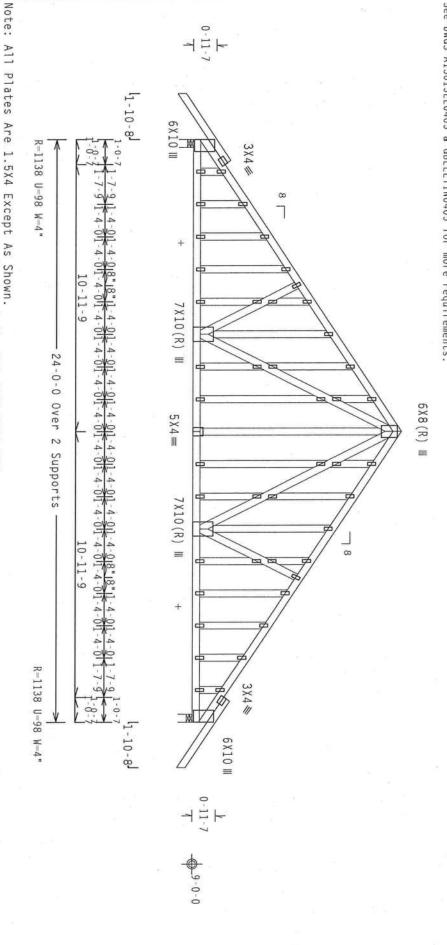
See DWGS A13015EE0405 & GBLLETIN0405 for more requirements

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi(+/-)=0.18

\*\*\*\*\*\*\*\*

Wind reactions based on MWFRS pressures

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.



TW Building Components Group

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG. INC. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN OF FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH PI: OR FAREIGATHO, HANDLIGS, SHEPPING, INSTALLING & BRACING OF TRUSSES, BY AFAFA, AND TPI. DESIGN COMPORES WITH APPLICABLE PROVISIONS OF HIS (MATIONAL DESIGN SPEC, BY AFAFA, AND TPI. ITH BCG CONNECTOR PLATES ARE MADE OF 20/18/18/CA (W.H.SYS) ASTH ASSA GRADE 40/60 (M. K.W.). SS) GALV. STEEL APPLY PLATES TO EACH FACE OF TRUSS AND. INMESS OTHERHISE LOCATED ON THIS DESIGN, POSITION PER BRAHINGS 150A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMENA AS OF TPI1-2002 SCC. 3. AS SLA, ON THIS DESIGN SHOWN. THE SUITABLITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AMSI/PPI 1 SCC. 2.

SOUNAL ENGINE

BC LL BC DL

0.0

HC-ENG DF/DF

80

DUR.FAC.

TOT.LD.

40.0

PSF PSF

46837

SPACING

24.0" 1.25

JREF -FROM SEQN-

1THR8228Z03

Design Crit:

TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/

(0) (0)

200 PLES

CENSE

lo. 66648

TC DL

10.0 20.0

PSF PSF

DATE

05/27/08

REF

Scale = .25"/Ft. R8228- 67262

10.0 PSF

DRW HCUSR8228 08148028

TC LL

FL/-/4/-/-/R/-

A PROPERLY ATTACHED RIGID CEILING

ALPINE

Haines City, FL 33844 FL CO's #0 278

TYP.

Wave

Top chord 2x4 SP Bot chord 2x4 SP Webs 2x4 SP THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER. Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED. Note: All Plates Are 1.5X4 Except As Shown. See DWGS Al3030EE0405 & GBLLETIN0405 for more requirements Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,.$ (8-140--OWNER BUILDER Baker TW Building Components Group BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF Haines City, FL 33844 FL CC 4 40 278 ALPINE Wave #2 Dense #2 Dense #3 1.5X4(\*\*)\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN COMPORNANCE WITH TPI: OR FARREACTHING, HANDLING, SHEPPING, HISTALLING A BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF ROS. (MATIONAL DESIGN SPEC, BY ATAPA) AND TPI. THE BCG CONNECTOR PLATES ARE MADE OF 20/18/166A (M.H/SS/K) ANTHA A653 GRADE 40/60 (M.K/M.SS) GALV. STEEL APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERMISE LOCATED ON THIS DESIGN, POSITION PER DRAWNINGS 160A.2. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A.3 OF TPII-2002 SEC.3. A SEAL ON THIS NORTH LES STREET, SUITE 112, ALEXANDRIA, VA. 22314) AND HTCA (MODO TRUSS COMECTL OF AMERICA. 6:300 CHIERRETISE, LAME, MADISON, H. 53719) FOR SAFETY PARCITICES RETURE TO PERFORMING THESE ENGLIGIOUS. UNLISS OTHERWISE, INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE DESIGN SHOWN. THE SUITABILITY AND USE OF BUILDING DESIGNER PER ANSI/TP1 1 SEC. 2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNE DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING A PROPERLY ATTACHED RIGID CEILING = R-21 Rw-49 R-71 2X4(A1) =<9"15</p>
1-4-0-x
1-4-0-x
1-4-0-x
1-4-0-x
1-5 PB1) U=45 W=6.31" PLF U=23 PLF Design Crit: 5X4(\*+4-yll-15 4-9-15 卣 TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0) 11-2-8  $\Rightarrow$ SOLELY FOR THE TRUSS COMPONENT NG IS THE RESPONSIBILITY OF THE Over 4 X 4 == 中 w Supports plot 110 mph wind, 21.37 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf.  $Iw=1.00\ GCpi(+/-)=0.18$ Wind reactions based on MWFRS pressures. 4 plate(s) require special positioning. Refer details for special positioning requirements. 4-9-15 SOLUCENSE CORNOT IE 由 No. 66648 1.5X4(\*\*) Ⅲ R=21 .5X4(\*\*) III  $2X4(A1) \equiv$ U=8 W=6.309 80 BC DL TC DL TC LL DUR.FAC. TOT.LD. FL/-/4/-32.0 10.0 20.0 /-/R/-24.0" 1.25 0.0 2.0 PSF PSF 01 PSF PSF scaled plate FROM DATE REF SEON-HC-ENG DRW HCUSR8228 08148021 Scale = .5"/Ft R8228-DF / DF 05/27/08 46848 67263

SPACING

JREF -

1THR8228Z03

Bot PLT Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24° OC, UNLESS OTHERWISE SPECIFIED. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. (8-140--OWNER BUILDER Baker TW Building Components Group chord 2x4 SP #2 Dense chord 2x4 SP #2 Dense Webs 2x4 SP #3 TYP. Haines City, FL 33844 FL CO<sup>A</sup> #0.278 ALPINE Wave 2X4(A1) =R=-126 Rw=106 U=126 W=6.31" R=98 PLF U=37 PLF W=10-8-8 \*\*IMPORTANIT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH FPI; OR FARREACHING, HANDLING, SHEPPING, INSTALLING A BRACHING OF TRUSSES, DESIGN CONFIDENCE, THE ADDITIONS OF THE PILOT OF THE DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2. A PROPERLY ATTACHED RIGID CEILING. 8 P82) 5-4-4 5-4-4 5-4-4 Design Crit: 12-3-2 TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/ 0ver 4X4= ω Supports /0(0) 5-4-4 5-4-4 110 mph wind, 21.54 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf. Iw=1.00 GCpi(+/-)=0.18 Wind reactions based on MWFRS pressures GOUGLAS FLEN STONAL BUSINES 2X4(A1) R=-126 Rw=71 U=74 W=6.309" QTY:19 80 BC DL DUR.FAC. BC LL TC LL SPACING 2 TOT.LD. FL/-/4/-/-/R/-DL 1.25 32.0 20.0 24.0" 10.0 PSF 0.0 2.0 PSF PSF PSF PSF DATE JREF-FROM SEQN-REF HC-ENG DRW HCUSR8228 08148029 Scale =.5"/Ft. R8228- 67264 1THR8228Z03 DF / DF 05/27/08 170260

(8-140--OWNER BUILDER Baker

משוורווים מחוודווורם מז ונומים נוועי

110 mph wind, 20.36 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf. Iw=1.00 GCpi(+/-)=0.18

In lieu of rigid ceiling use purlins to brace

BC

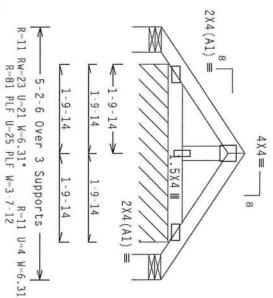
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Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3

Wind reactions based on MWFRS pressures

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.



Rw=23 U=21 W=6.31" R=11 U=4 W=6.31" R=81 PLF U=25 PLF W=3-7-12

Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0)

TYP.

Wave

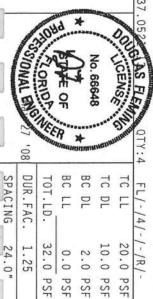
\*\*\*\*\*MARNING\*\*\* RUSSES REQUIRE EXTREME CARE IN FARRICATION, INNOVINE, SHIPPING, INSTALLING AND REACHE.
RETER TO REST QUALIDING COMPONENT SACREY INFORMATION), PRUNCISHED BY THE (TRUST OF AMERICA, 03300
MORTH LEE SIREET, SHITE 312, ALEXANDRIA, MA. 22314) AND WICA (4000) TRUSS COUNCIL OF AMERICA, 03300
HIERRISS LAME, MADISON, HI SAJI9) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE UNCTIONS. UNLESS
DIRECTURAL PROJECTION OF CHORD SHALL HAVE PROPERTY ATTACHED FRACTURAL PARTY AND DOTTON CHORD SHALL HAVE
OFFICENTISE HOLD, CALLED TO CHORD SHALL HAVE PROPERTY ATTACHED FRACTURAL PARTY AND DOTTON CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH IP: OR FARRICATING, HANGLING, SHIPPIG, HISTALLING & BRACHING OF TRUSSES, DESIGN CONTROLATION, AND LIGHT PROPUTSIONS OF THOS (MAIDONAL DESIGN FOR A MEAN AND TRI. ITH BCG, CONNECTION PRICE ARE HADE OF 20/18/166A (M.H/SS/M) ASKIN A653 GRADE 40/60 (M. K/H.SS) AGALY. STELL APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERSISE LOCATED ON THIS DESIGN, POSITION PER DRAWHING 150A-Z, ANY INSPECTION OF FLATES FOLLOWED BY (I) SHALL BE FER ARREX A.O OF TRIT 2002 SEC. 3. A SEAL ON THIS DRAWHING INDICATES ACCEPTANCE OF PROFESSIONAL REGIONERAL RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT BUILDING DESIGNER PER ANSI/TPI I SEC. 2.

ITW Building Components Group

ALPINE

Haines City, FL 33844 FL COA #0 778



PSF

PSF

JREF -FROM SEQN-HC-ENG

1THR8228Z03

PSF

Scale =.5"/Ft.

R8228- 67265

DATE REF

05/27/08

DRW HCUSR8228 08148030

DF / DF

30186

REV

(8-140--OWNER BUILDER Baker PB5)

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3

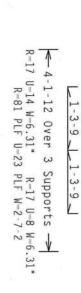
Wind reactions based on MWFRS pressures

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER. THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL

2X4(A1) =4 X 4 == 5×4 2X4(A1) = 8



**←**1-3-9**>** 

1-3-9

1-3-9

REFER TO BOSI (BUILDING COMPONENT SAFE MODELLEE STREET, SUITE 312, ALEXANDRIA, ERFERPRISE LAME, MADISON, HI 53719) FOR OTHERMISE INDICACED TOP CHORD SHALL HAVE A PROPERTY ATTACHED RIGHD CILLING. \*WARNING\*\* TRUSSES RE IRE EXTREME CARE IN FARRICATION, INAMOLING, SHIPPING, HISTALLING AND BRACING COMPOULNI SAFETY IMFORMATION), PUBLISHED NY TOT (TRUSS FLATE HISTITUEE, 218 12, ALEXANDRIA, VA, 22314) AND NTCA (MOOD TRUSS COUNCIL OF AMERICA, 630 Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0)

CENS

TC LL

0.66648

TYP.

Wave

\*\*IMPORTANT\*\*FIRMING A CORY OF THIS DESIGN TO THE INSTALLING CONTRACTOR. THE REG., THE, SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGNE, ANY FALLINE TO BULLOT BE RIMES IN COMPORMANCE WITH THE TOP THIS PROPERTY. THE PROPERTY OF THE PROPERTY O BUILDING DESIGNER PER ANSI/TPI I SEC DRAWING INDICATES SOLELY FOR THE TRUSS COMPONEN

ORIOT IS

08

DUR. FAC. SPACING

24.0" 1.25

JREF -

1THR8228Z03

REV

TOT.LD.

TAJE OF

BC LL BC DL TC DL

TW Building Components Group Inc.

ALPINE

Haines City, FL 33844 FL CO\* #0 778

110 mph wind, 20.19 ft mean hgt, anywhere in roof, CAT II, EXP B, psf. Iw=1.00 GCpi(+/-)=0.18 ASCE 7-02, CLOSED bldg, Located TC DL=5.0 psf, wind BC DL=1.2

In lieu of rigid ceiling use purlins to brace BC @ 24"

FL/-/4/-/-/R/-32.0 PSF 10.0 PSF 20.0 PSF 0.0 PSF 2.0 PSF FROM SEQN-DATE REF HC-ENG DF/DF DRW HCUSR8228 08148031 Scale =.5"/Ft. R8228- 67266 30189 05/27/08

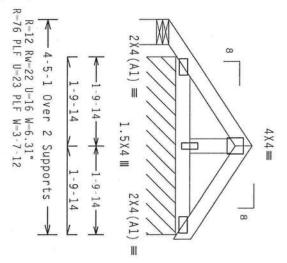
Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Wind reactions based on MWFRS pressures

110 mph wind, 20.36 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf. lw=1.00 GCpi(+/-)=0.18

Refer to DNG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.





Design Crit: TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/0(0)

Scale =.5"/Ft.

R8228- 67267

05/27/08

PLT TYP.

Wave

\*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST. (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TH (TRUSS PLATE INSTITUTE, 2738 HORFILLE STREET, SHITE 317, ALEXANDRIA, VA, 22314) AND NICA (MOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, NI 55719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTON CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTON CHORD SHALL HAVE

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN COMPORNANCE WITH PIT. OR FARRICATING, HANDLING, SHEPPIG, HISTALLING & BRACHING OF TRUSSES, BESIGN COMPOREATION PIT. THE BCG CONNECTOR PLATES ARE HADE OF 20/18/16/36 (M.H/SS/R) ASTA A653 GRADE 40/50 (M. K/M.SS) GALV STEEL APPLY PLATES TO EACH FACE OF TRUSS AND, BULES OTHERSISE LOCATED ON THIS DESIGN, POSITION PER DRAHING 150A-Z. ANY INSPECTION OF PLATES FOLOMED BY (I) SHALL BE PER ANNEX A.O FPIT: 2002 SEC.3. A SLAL ON THIS DESIGN SHOWN. THE SULTABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGN SHOWN.

ITW Building Components Group Haines City, FL 33844 FL COA #0 778

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNE DRAING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING DESIGN SHOWN. THE SUITABLITY AND USE OF THIS COMPONENT BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ALPINE

SONAL ENGLISH OUGENOUS PLENS CENSE ATE OF 0.66648 QTY:12 80 BC LL BC DL SPACING DUR.FAC. TC DL TC LL TOT.LD. FL/-/4/-/-/R/-32.0 20.0 24.0" 1.25 10.0 PSF 0.0 2.0 PSF PSF PSF PSF REF JREF -FROM DATE SEQN-HC-ENG DRW HCUSR8228 08148022

DF / DF 46941

1THR8228Z03

## CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

#### NOTES:

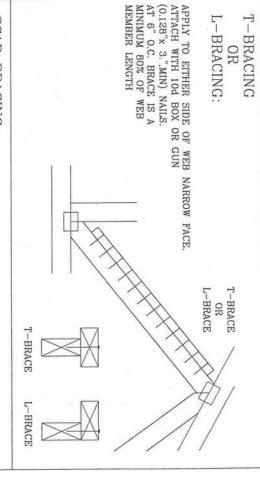
THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

2X4 2X6 2X6	1 ROW 2 ROWS 1 ROW	2X6 2X6 2X8
2X6	2 ROWS	
2X4	1 ROW	OR 2X4
T OR L-BRACE	BRACING T	SIZE
ALTERNATIVE BRACING	SPECIFIED CLB	WEB MEMBER

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(\*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.



#### SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB.

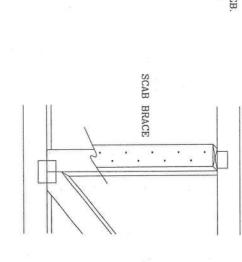
NO MORE THAN (1) SCAB PER FACE.

ATTACH WITH 10d BOX OR GUN

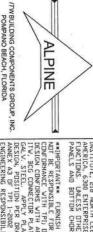
(0.128"x 3.",MIN) NAILS.

AT 6" O.C. BRACE IS A MINIMUM

80% OF WEB MEMBER LENGTH



THIS DRAWING REPLACES DRAWING 579,640



\*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATING, MUDPING, INSTALLING AND BRACING. REFER TO BESS (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NURTH LEE STR., SUITE 312, ALEXANDRIA, VA. 22314) AND YICA (VUDDI TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, VI 33719) FOR SAFETY PACCITICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERVISE (INCIATED, TOP GUDDE) SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL

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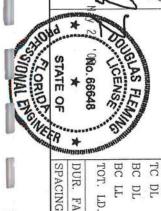
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		D. PSF -ENG	DL PSF DRWG LL PSF —ENG LD. PSF	DL PSF DATE DL PSF DRWG LL PSF -ENG LD. PSF

## TOP CHORD FILLER DETAIL

+ 2X4 CONTINUOUS LATERAL BRACING AT 24" O.C.

MAXIMUM SPACING. ATTACH TO EACH TOP CHORD WITH

(2) 16d COMMON (0.162"X 3.5", MIN) NAILS.

BRACING MATERIAL TO BE SUBBLIED AND ATTACHED

(2) 16d COMMON (0.162"X 3.5",MIN) NAILS. BRACING MATERIAL TO BE SUPPLIED AND ATTACHED AT BOTH ENDS TO A SUITABLE SUPPORT BY ERECTION CONTRACTOR.

++ 2X4 SO. PINE #2 N OR SPF #1/#2 FILLER TOP CHORD +++ 2X4 SO. PINE #3 OR SPF #1/#2 VERTICAL WEBS SPA

+++ 2X4 SO. PINE #3 OR SPF #1/#2 VERTICAL WEBS SPACED
48" OC MAXIMUM.

8/12 MAXIMUM PITCH.

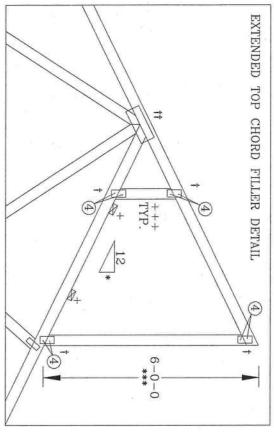
\*\* 2X8.25 PIGGYBACK SPECIAL PLATE. SEE DRAWING PIGBACKB0699 FOR PIGGYBACK SPECIAL PLATE INFORMATION.

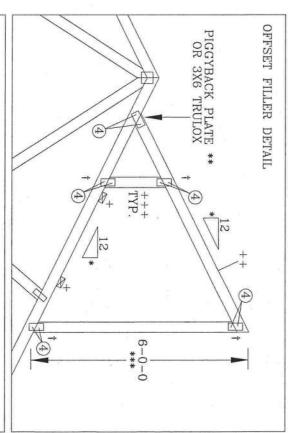
\*\*\* 6'0" MAXIMUM HEIGHT.

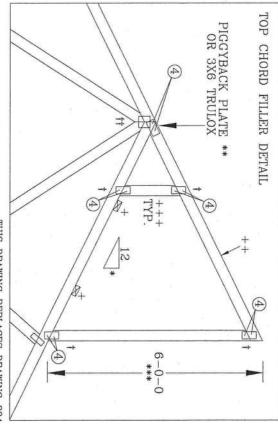
† W2X4 OR 3X6 TRULOX.

th refer to engineer's sealed design referencing this detail for lumber, plates, and other information not shown.

0.120"X 1.375" NAILS REQUIRED
FOR TRULOX PLATE ATTACHMENT. NAILS SPECIFIED
IN CIRCLES MUST BE APPLIED TO EACH FACE OF EACH TRUSS PLY.
SEE DWG. 160TL FOR NAILING AND TRULOX PLATE REQUIREMENTS





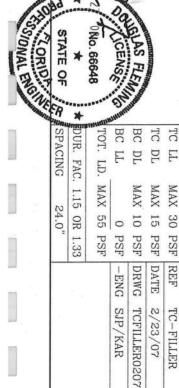


THIS DRAWING REPLACES DRAWING 884,080



\*\*\*WARHIND\*\* TRUSSES REDUIRE EXTREME CARE IN FABRICATING, THANDLING, SHEPING, INSTALLING AND BRACING. REFER TO BOSS IGBULDING GIDEPIDENT SAFETY IN FEBRATION» PUBLISHED BY THE CROSS PLATE INSTITUTE, 218 NURTH LEE STR., SUITE 312, ALEXANDRIA, VA. 223(4) AND WICA CYDDD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LIN, MAISSIN, WI 53799 THE SAFETY FRACTICES PRIDE TO FEBROHNIG THESE FUNCTIONS. UNLESS CHIERPAISE INDICATED, THE OLHED SHALL HAVE PRIDERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PRIDERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROBERLY ATTACHED RIGID CEILING.

\*\*\*IMPORTANT\*\* FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITV BCG, INC., SHALL NOT BE RESPONSIBLE FOR NAW EVANTION FROM THIS DESIGN ANY FAILURE TO BUILD THE TRUSSS IN CONFIDENCE WITH FIJE OF FARRICATING, HANDLING, SHEPFUN, INSTALLING & BRACING OF TRUSSES. DESIGN CONFIDENT SHITH FIJE OF FARRICATING, HANDLING, SHEPFUN, INSTALLING & BRACING OF TRUSSES. THE BUILD FOR THE SHEPPUN FAILES ARE HADE OF PROFISED OF THIS CONTINUAL DESIGN SHEPTUN FOR THE SHEPPUN FAILES ARE HADE OF PROFISED OF THIS STANDING SHEPTUN FOR THE SHEPTUN FOR T



## BOTTOM CHORD FILLER DETAIL

SIZES (1X3 WAVE) MAY BE USED IF BEARING IS OMITTED. WEDGE OPTIONAL INTERIOR OR CANTILEVER BEARING. MINIMUM PLATE OR VERTICAL MEMBER MUST COINCIDE WITH BEARING LOCATION.

TO EACH FACE OF THE TRUSS. SEE DWG. 160TL FOR NAILING AND TRULOX PLATE REQUIREMENTS FOR TRULOX PLATE ATTACHMENT.

NAILS SPECIFIED IN CIRCLES MUST BE APPLIED 0.120" X 1.375", NAILS, REQUIRED

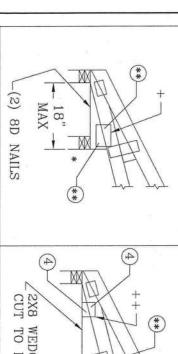
3X4 WAVE OR 4X8 TRULOX

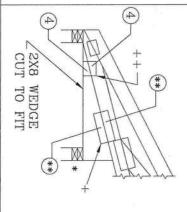
++ 2X4 WAVE OR 3X6 TRULOX

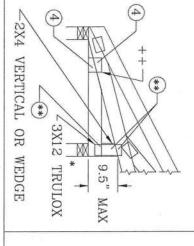
SHOWN DETAIL REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS FOR LUMBER, PLATES, AND OTHER INFORMATION NOT

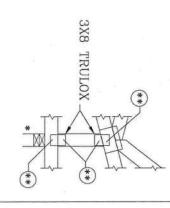
MAY BE REQUIRED TO ACCOMODATE REQUIRED NAILS TRULOX PLATES SHOWN ARE MINIMUMS. LARGER PLATES (\*\*)

FILLER BOTTOM CHORD	MAXIMUM REACTION	EACTION	MINIMIM	** REQUIRED	NAILS	PER FACE WITH	I TRULOX F	LATES
OR WEDGE SPECIES	DOWNWARD	TALLAU	BEARING AREA 1.00 D.O.L. 1.15 D	1.00 D.O.L.	1.15 D.O.L.	1.25 D.O.L.	D.O.L. 1.33 D.O.L. 1.60 D.O.L.	1.60 D.O.I
DOUGLAS FIR-LARCH	3281#	1656#	1.5" X 3.5"	12	11	10	9	8
HEM-FIR	2126#	1095#	1.5" X 3.5"	9	8	7	7	6
SPRUCE-PINE-FIR	2231#	1192#	1.5" X 3.5"	10	9	8	8	6
SOUTHERN PINE DENSE	3465#	1791#	1.5" X 3.5"	12	11	10	9	8
SOUTHERN PINE	2966#	1492#	1.5" X 3.5"	10	9	8	8	7
SOUTHERN PINE NON-DENSE		1343#	1.5" X 3.5"	9	8	7	7	6









THIS DRAWING REPLACES DRAWINGS A115 A115/R & 884,132

ITW BUILDING COMPONENTS GROUP, INC. POMPANO BEACH, FLORIDA	1	ALPINE	\ /	>
ANNEX A3 OF TPI 1-2 ENGINEERING RESPONS	ANE	NOT BE RESPONSIBLE CONFORMANCE WITH TE	PANELS AND BOTTOM	INSTITUTE, 218 NORTH AMERICA, 6300 ENTERF

\*\*\*VARNING\*\*\* TRUSSES REQUIRE EXTREME CARE IN FARRICATING, HANGLIANG, SHIPPING, INSTALLING AND BRACING. REFER TO BESS (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TRI CIRUSS PLATE INSTITUTE, ZIB NORTH LEE STR., SUITE 312, ALEXANDRIA, VA. 22314) AND WTCA CYUDD TRUSS COUNCIL, OF AMERICA, 6300 ENTERPRESE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIDE TO PERFORMING THESE FUNCTIONS. UNELSS DIFFERVISE NODICATED, TOP CARD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERTY ATTACHED STRUCTURAL

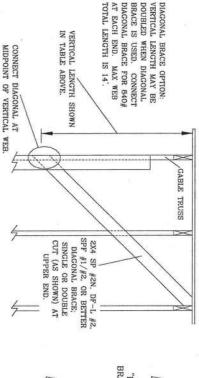
ANSI/TPI 1 SEC. 2. NIISH COPY OF THIS BESIGN TO INSTALLATION CONTRACTOR. ITV BCG, NC., S
FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS.
FIFT OF FARRICATING, HANDLING, SHIPPING, INSTALLING S. BRACING OF TRUSSES.
FIFT IN FAPLICABLE PROVISIONS OF NIIS (NATIONAL DESIGN SPEC, BY AFRA) AND PLATES ARE HADE OF ZOULDING OF NIIS (NATIONAL DESIGN SPEC, BY AFRA) AND PLATES ARE HADE OF ZOULDING ON THIS PLATES ARE HADE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS PLATES ARE HADE OF TRUSS AND PLATES ARE HADE OF TRUSS COMPONENT DESIGN SHOWN THE SUITABILITY SCILLTY FOR THE SUITABILITY SCILLTY FOR THE SUITABILITY OF THE BUILDING DESIGNAED. D ON THIS SHALL BE PER PROFESSIONAL UITABILITY AND

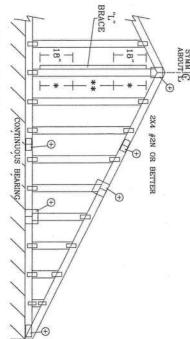


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SPACING 24.0"	DUR. FAC. 1.0/1.15/1.25/1.33	TOT. LD.	T	DL	DL	T
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#### 7-02: 130 MPH WIND SPEED, 15 MEAN HEIGHT, ENCLOSED, 11 1.00, EXPOSURE 0

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4' 3"	4' 4"	4' 4"		4' 8"	4. 2"	4. 2."	10.0	4' 3"	3' 10"		4' 0"	4, 2,		3' 9"			3' 10"	3' 4"	100	3' 6"	3' 7"	3' 8"	3' 3"	3' 3"	100	3' 4"	BRACES	NO
6' 1"	7' 1"		1	7' 4"	1.3	6' 11"	6' 11"		5, 3,	1	1	6, 8,		1		6′ 0″		1		5' 0"	5' 10"	5' 10"		4' 11"	4' 11"	5' 10"	GROUP A	(1) 1X4 L
6' 1"	- 1	7' 2"	300		5' 11"	- 27	6' 11"		5' 3"			7. 2"		5, 5,		6' 0"	6' 10"		V	5' 0"	6' 3"	-	4' 2"	4' 11"	4' 11"	6' 0"	GROUP B	BRACE .
		8' 9"			7' 10"		8' 9"	-	6' 11"	200		7' 11"		6' 10"		7' 11"		5' 8"		6' 8"	6' 11"	6' 11"	5' 6"	6' 5"	6' 6"	6' 11"	GROUP A	(1) 2X4
-	9' 2"	9' 2"	. 9	9' 5"	7' 10"	8' 9"	8' 9"	0.5	6' 11"	-		8' 6"		6' 10"		7' 11"	8' 1"	5' 8"			7' 5"	7' 5"		6' 5"	6' 6"	7' 1"	GROUP B	L BRACE *
- 17		10' 5"	10' 5"	10' 5"	10' 5"		10' 5"	10' 5"	9' 4"		7.00					9' 5"				8, 3,		8' 3"		8' 3"	100	8' 3"	GROUP A	(2) 2X4 L
10' 8"		10' 11"	11' 2"	1111	10' 5"	11.00	10′ 5″	10' 8"	9' 4"	9' 11"	9' 11"	10' 2"				9' 5"	1.00	0.384	1.00			8' 11"		8, 3,,	- 3	8' 6"	GROUP B	BRACE **
12' 6"	-	13' 8"	3	13' 8"		13' 8"			10' 10"		130	12' 5"		10' 7"			12' 5"		10' 3"			10' 10"	8' 8"	10' 0"	10' 1"	10' 10"	GROUP A	(1) 2X6 "L"
-1		14' 0"	14' 0"	14' 0"	12' 3"	13' 8"	13' 8"	14' 0"	10' 10"	12' 6"		13' 5"	13' 5"	10' 7"	12' 4"	12' 4"	12' 9"	8' 10"	10' 3"	10' 4"	11' 8"	11' 8"	8' 8"	10, 0,	10' 1"	11' 2"	GROUP B	" BRACE *
14' 0"		14' 0"	14' 0"	14' 0"		14' 0"	14' 0"	14. 0.	14' 0"	14' 0"		14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	12' 0"	12' 11"	12' 11"	- 1	12' 11"	11' 8"	12' 11"	12' 11"		GROUP A	(2) 2X6 "L"
- 1	14' 0"	14' 0"	14' 0"	14' 0"		14' 0"		14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	12' 0"	13' 7"			13' 11"		12' 11"		13' 3"	GROUP B	BRACE **





LIVE LOAD DEFLECTION CRITERIA IS L/240.

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.

\* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C.

\* FOR (2) "L" BRACE: SPACE NAILS AT 3" O.C.

\*\* FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C.

IN 18" END ZONES AND 6" O.C. BETWEEN ZONES

DESIGN FOR	REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.
2.5X4	GREATER THAN 11' 6"
2X4	GREATER THAN 4' O", BUT LESS THAN 11' 6"
1X4 OR 2X3	LESS THAN 4' 0"
NO SPLICE	VERTICAL LENGTH
SIZES	GABLE VERTICAL PLATE SIZES

\*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BESS GOULDING COMPINENT SAFETY INFORMATION, PUBLISHED BY TELCTRUSS PLATE INSTITUTE, 218 NIDRIH LEE SER, SUITE 312, ALEXANDRIA, VA. 22314) AND WICK AVOIDD INISS COUNCIL MARRICA, 6300 ENTERPRISE LN, MADISON, VI 53719) FOR SAFETY PRACTICES PRIDE TO PERFORMING THESE FUNCTIONS. UNESESS OTHERVISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL

REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

WHORDERANIES FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR TITY BGG, INC., SMAN

OUNTBRANCE VITH, PPI, DR KARREATING HANDLING, SHEPPING, INTALLINE I BRILD HE PROSS. IN

BESIGN CONTRRANCE VITH, APPLICABLE PROVISIONS OF MIS WAND MALESION SERVING OF TRINSSES.

BESIGN CONNECTION PLATES OF MADE DE BUYSIONS OF MIS WAND MALESION SERVING MORNE WORLD. AND TRI
ILSUS COUNCITION PLATES OF MADE DE BUYSIONS OF MIS WAND MALESION SERVING WORLD. AND TRI
BESIGN, POSITION PER DRAVINGS MORNE WAY INSECTION OF PLATES FILLDWED BY (I) SMALL BE FER

MANEX AS OF THILL-PROSE SEC. 3. A SEAL ON THIS DRAVING INDICARS ACCEPTANCE OF PROFESSIONAL

LONGINGERING RESPONSIBILITY SULLY FOR THE TRUSS COMPONENT DESIGN SUMMY. THE SULTABILITY AND USE OF THIS CONFIDENT FOR ANY BUILDING DESIGNER, PER

MESS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER

ITW BUILDING COMPONENTS GROUP, INC. POMPANO BEACH, FLORIDA

ALPINE

SOUBLAS FLEN OSIONAL ENGINEE No. 66648 CENS \* MAX. MAX. TOT. SPACING LD. 60

PSF DRWG DATE REF A13015EE0207 2/23/07 ASCE7-02-GAB13015

24.0"

#1 / #2 STANDARD

BRACING GROUP SPECIES

AND GRADES:

GROUP

A:

#3

HEM-FIR
STUD
STANDARD

DOUGLAS FIR-LARCH SOUTHERN PINE #3 STUD STANDARD

STANDARD

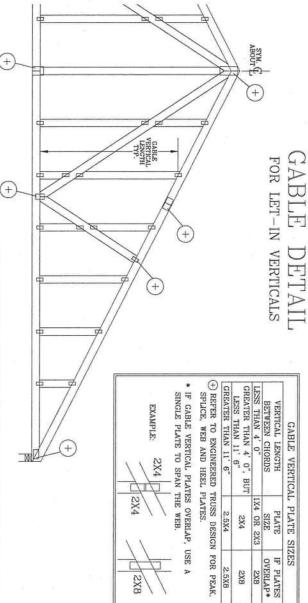
GROUP HEM-FIR ₽:

SOUTHERN PINE #2 DOUGLAS FIR-LARCH #2

GABLE TRUSS DETAIL NOTES

PROVIDE UPLIFT CONNECTIONS FOR 135 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

MEMBER LENGTH. "L" BRACING MUST BE A MINIMUM OF 80% OF WEB



ATTACH EACH "T" REINFORCING MEMBER WITH PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN

HAND DRIVEN NAILS:

GUN DRIVEN NAILS: 10d COMMON (0.148"X 3.",MIN) TOENAILS AT 4" O.C. PLUS (4) 16d COMMON (0.162" X 3.5",MIN) TOENAILS IN TOP AND BOTTOM CHORD.

8d COMMON (0.131"X 2.5", MIN) TOENAILS AT 4" O.C. PLUS (4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

"T"
REINFORCINGMEMBER

4 TOENAILS

RIGID SHEATHING

GABLE

TOENAILS SPACED AT 4" O.C.

ASCE 7-93 GABLE DETAIL DRAWINGS

ASCE 7-98 GABLE DETAIL DRAWINGS A11015EN0207, A10015EN0207, A09015EN0207, A08015EN0207, A07015EN0207, A11030EN0207, A10030EN0207, A09030EN0207, A08030EN0207, A07030EN0207

ASCE 7-02 GABLE DETAIL DRAWINGS A13030EC0207, A12030EC0207, A11030EC0207, A10030EC0207, A13015EC0207, A12015EC0207, A11015EC0207, A10015EC0207, A08515EC0207 A08530EC0207

ASCE 7-05 GABLE DETAIL DRAWINGS A13030EE0207, A12030EE0207, A11030EE0207, A10030EE0207, A08530EE0207 A13015EE0207, A12015EE0207, A11015EE0207, A10015EE0207, A08515EE0207

A13030E50207, A12030E50207, A11030E50207, A10030E50207, A08530E50207 A13015E50207, A12015E50207, A11015E50207, A10015E50207, A08515E50207

SEE APPROPRIATE ALPINE GABLE DETAIL (ASCE OR SECCI WIND LOAD) FOR MAXIMUM UNREINFORCED GABLE VERTICAL LENGTH.

4 TOENAILS

CEILING

TOENAIL 2X4 "T" REINFORCING MEMBER 2X6 "T" REINFORCING MEMBER TOENAIL

SBCCI WIND LOAD. APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR VERTICAL SPECIES, GRADE AND SPACING) FOR (1) TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" FACTOR BY LENGTH (BASED ON GABLE 2X4 "L" BRACE, GROUP A, OBTAINED FROM THE

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WEB LENGTH INCREASE W/ BRACE

30 FT	70 MPH	15 FT	70 MPH	30 FT	80 MPH	15 FT	80 MPH	30 FT	90 MPH	15 FT	90 MPH	30 FT	100 MPH	15 FT	100 MPH	30 FT	110 MPH	15 FT	110 MPH	WIND SPEED AND MRH
2x6	2x4	2x6	2x4	2x6	2x4	2x6	2x4	"T" REINF. MBR. SIZE												
2 01	10 %	0 %	0 %	20 %	20 %	10 %	2 01	30 %	10 %	20 %	20 %	40 %	10 %	30 %	10 %	50 %	2 01	40 %	10 %	SBCCI
30 %	20 %	20 %	20 %	40 %	2 01	30 %	20 %	50 %	2 01	40 %	2 01	40 %	2 01	50 %	2 01	50 %	10 %	50 %	% 01	ASCE

THIS DRAWING REPLACES DRAWINGS GAB98117 876,719 & HC26294035

"T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10 (1) 2X4 "L" BRACE LENGTH = 6' 7"

MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH  $1.10 \times 6' 7'' = 7' 3''$ 

GABLE VERTICAL = 24" O.C. SP #3

"T" REINFORCING MEMBER SIZE = 2X4

MEAN ROOF HEIGHT = 30 FT ASCE WIND SPEED = 100 MPH

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24.0"		60 PSF				
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\*\*WARNING\*\* TRUSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BESI (BUILIDING COPPOIENT SAFETY INFORMATIDN), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STR., SUITE 312, ALEXANDRÍA, VA. 22314) AND WTLA (WIDD TRUSS COUNCIL (MERICA, 6300 ENTERRISE IN, MADÍSIN, VI 53719) FID SAFETY PRACTICES PRIDR TO PERFORMING THESI FUNCTIONS. UNLESS OTHERWISE INDICATED, TIPC CHORD SHALL HAVE PRIPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

WHORDER/WITH FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITY BCG, INC., SMALL NOT BE RESPONSIBLE FOR AWE DEVAILUN FROM THIS DESIGN, ANY FAILURE OF BRUID HE FRUSS IN CONTRACTOR. ITY BCG AWE FACTOR TO PROPERTY OF THE STATE OF THE

#### VALLEYTRUSS DETAIL

TOP CHORD 2X4 SP #3 OR 2X4 SP #2 OR SPF #1/#2 OR BETTER. 2X3(\*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER BETTER.

- 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).
- ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: SBC 110 MPH, ASCE 7-93 110 MPH OR ASCE 7-98, HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL WIND TC DL=5 PSF ASCE 7-02 OR ASCE 7-05 130 MPH. (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9". OR CONTINUOUS LATERAL BRACING

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0"

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY INSTALLATION TRUSS

PURLINS AT 24" OC OR AS BY VALLEY TRUSSES ENGINEERS' SEALED DESIGN. USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN

\*\* NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

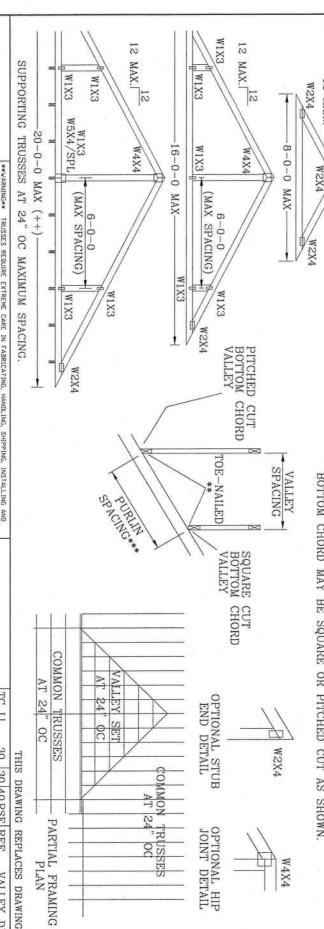
CUT FROM 2X6 OR LARGER AS REQ'D

4-0-0 MAX

12 MAX.

++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".





THIS DRAWING REPLACES DRAWING A105

ITWBUILDING COMPONENTS GROUP, INC. POMPANO BEACH, FLORIDA ALPINE

\*\*VARNING\*\* TRUSSES REDUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BOSI (BUILDING COMPINENT SAFETY INCREMATION, PUBLISHED BY THE (TRUSS PLATE INSTITUTE, 218 MIRTH LEE STER, SUITE 312, ALEXANDRINA, VA. 25314) AND VTCA (VOIDD TRUSS COUNCIL DIAMERICA, 6300 ENTERRISE LN, MADISON, VI 53719) FOR SAFETY PRACTICES PRICE TO PERCORNING THESE TUNCTIONS. UNLESS STHERWISE INDICATED, TOP ORDER SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL

WHERDERMINE FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITY BCG, INC., SHALL

MOT BE RESPONSIBLE FOR ANY DEVIATION FOR THIS DESIGN, ANY FAILURE OF BUILD HE FRUSS. IN

COMPONEME WITH JPH, DR FARRICATING, HANDLING, SHPPPING, INSTALLING & BRACING OF TRUSSES.

BESIGN CONVERSE VITH JPH DR FARRICATING, HANDLING, SHPPPING, INSTALLING & BRACING 40/60 CW.M.A.S.S.

BESIGN CONVERSE VITH JPH DRAFES ARE HADE OF BUTSHING OF MISS SHALLING, SPEC. BY AFREN AND TPH

INSTALLING CONNECTER PLATES ARE HADE OF BUTSHINGS AND, UNLESS OTHERWISE LICATION IN HIS

BESIGN, POSSTURU PER DRAVINGS 160A-C. ANY INSECTION OF PLATES FOLLINGED BY OT SHALL BE PER

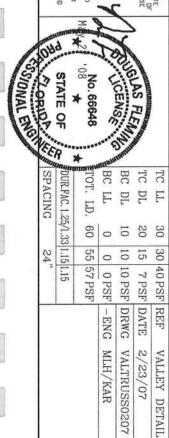
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#### ASCE 7-02: 130 MPH WIND SPEED, 30, MEAN HEIGHT, ENCLOSED, I 11 1.00, EXPOSURE

RACING GROUP SPECIES

AND GRADES:

GROUP

A.

HEM-FIR

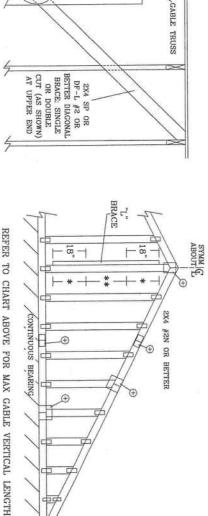
#3

STANDARD STUD

SOUTHERN PINE

STANDARD

	1	N.	,,	-	_	G . C				1 (		,,	(	)	. (	۲,		,	2	4	,,	(	0	.(	Э.		SPACING SPECIES	CADIE
		(	S		II.	111	ひて	) †	ָּדְּ	J F I	(	2		ПГ	1111	ひてって		1	DFI.	,	D		DF T		ひて	2	SPECIES	CVBIE ABBLICVI
STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	GRADE	BRACE
4' 0"	4' 2"	4. 2.	4. 4.	4, 5,"	3' 11"		- 1	4. 0.	3 8	3' 9"	3, 9,	3′ 11″	4, 0,	3' 7"	3' 7"	3' 7"	3, 8,	3' 0"	3, 3,		3' 5"	-	2' 11"	3' 1"	3' 1"	ය හ	BRACES	S
5' 6"	6' 4"	6' 5"	6' 11"	6' 11"	5.4"	-	6, 3,	6' 11"	4' 9"	5, 6,	5, 7,,	6' 4"		4' 8"	5, 5,		6' 4"	3' 10"	4, 6,	4' 6"	ō, 6,		3' 9"	4' 5"	4' 5"	5' 6"	GROUP A	/1/ 1/4 h
5, 6,	6, 4,"	6' 5"	7' 6"	7' 6"	5,4	6' 3"	6' 3"	7' 2"	4' 9"	5' 6"	5' 7"	6' 10"	6' 10"	4' 8"	5, 2,	5, 5,	6' 6"	3' 10"	4' 6"	4. 6.	5' 11"	5' 11"	3' 9"	4' 5"	4' 5"	5' 8"	GROUP B	DIMINU
7' 3"	8, 3,	8' 3"	8' 3"	8' 3"	7' 1"	8' 3"	8' 3"	8' 3"	6' 3"	7' 3"	7' 4"	7' 6"	7' 6"	6' 2"	7' 2"	7' 2"	7' 6"	5' 1"	5' 11"	6' 0"	6' 6"	6' 6"	5, 0,	5' 10"	5' 10"	6' 6"	GROUP A	(1) NOT 1
1	8, 5,"	8' 6"	8' 11"	8' 11"	7' 1"	8' 3"		8' 6"	6' 3"	7' 3"	7' 4"	8' 1"	8' 1"	6' 2"	7' 2"	7' 2"	7' 8"	5' 1"	5' 11"	6' 0"	7' 0"	7' 0"	5' 0"	5′ 10″	5' 10"	6' 9"	GROUP B	- Divisor
9' 9"	100			9' 10"	9' 6"	9' 10"	9' 10"		8, 2,	8' 11"	8' 11"	8' 11"	8' 11"	8' 3"	8' 11"	8' 11"	8' 11"	6' 11"	7' 10"	7' 10"	7' 10"	7' 10"	6' 9"	7' 10"	7' 10"	7' 10"	GROUP A	(4)
9' 9"			10' 7"	10' 7"	9' 6"	9' 10"	9' 10"	10' 1"	8' 5"	9' 5"	9' 5"	9' 7"		1 3	8' 11"	8' 11"	9' 2"	6' 11"	8' 0"	8' 1"	8' 5"	8' 5"	6' 9"	7' 10"	7' 10"	8, 0,	GROUP B	Civion
11' 4"	12, 11,		18	12' 11"	11' 1"	12' 10"	12' 11"	12' 11"	9' 9"	11' 4"	11' 5"	11. 9.	11' 9"	9' 7"	11' 1"	11' 2"	11' 9"	8' 0"	9' 3"	1.5	10' 3"	10' 3"	7' 10"		9' 1"	10' 3"	GROUP A	(a) man a
11' 4"	13' 1"	13' 3"	13' 11"	13' 11"	11' 1"	12' 10"	12' 11"	13' 4"	9' 9"	11' 4"	11' 5"	12' 8"	12' 8"	9' 7"	11' 1"	11' 2"	12' 1"	8, 0,		9' 4"	11, 1,	111' 1"	7' 10"		1		GROUP B	
14' 0"			14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	12' 11"	14' 0"	14' 0"	14' 0"	10' 10"	12' 3"	12' 3"	12' 3"	12' 3"			12 3"	12' 3"	GROUP B GROUP A GROUP	Tay annua to
14. 0	100	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	12' 11"	14' 0"	14' 0"	14' 0"	10' 10"	12' 6"	12' 8"	13' 2"	13, 2,	10' 7"				GROUP B	
O GABLE END SUPPORTS LOA	L	O" PROVIDE UPLIFT CONNECTION	O" LIVE LOAD DEFLECTION CRI		O" GABLE TRUSS	0,	0,	0"	I	SOUTH		0,1	0,			J.		ST	STUD STUD	DOUGLAS	_			SPRUCE-PINE-FIR		BRACING GROUP SPE	B	



DIAGONAL BRACE OPTION:
VERTICAL LENGTH MAY BE
DOUBLED WHEN DIAGONAL
BRACE IS USED. CONNECT
DIAGONAL BRACE FOR 980#
AT EACH END. MAX WEB
TOTAL LENGTH IS 14.

VERTICAL LENGTH SHOWN IN TABLE ABOVE.

CONNECT DIAGONAL AT MIDPOINT OF VERTICAL WEB.

DOUGLAS FIR-LARCH

GROUP B: HEM-FIR #1 & BTR #1

OUTLOOKERS WITH 2' O" OVERHANG, OR 12" COVIDE UPLIFT CONNECTIONS FOR 160 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD). PLYWOOD OVERHANG. BLE END SUPPORTS LOAD FROM 4' 0"

\* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C.
IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
\*\* FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C.
IN 18" END ZONES AND 6" O.C. BETWEEN ZONES. MEMBER LENGTH. ATTACH EACH "L" BRACE WITH 10d NAILS. 'L" BRACING MUST BE A MINIMUM OF 80% OF WEB

+	-	1.000	1		
REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.	GREATER THAN 11' 6"	GREATER THAN 4' 0". BUT LESS THAN 11' 6"	LESS THAN 4' 0"	VERTICAL LENGTH	GABLE VERTICAL PLATE SIZES
PLATES.	2.5X4	2X4	1X4 OR 2X3	NO SPLICE	TE SIZES

WEMPERTANTAM FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITY BCG, INC., SHALL

OUT SE RESPONSIBLE FIR ANY DEVIATION FROM THIS DESIGN ANY FAILURE OF BUILD THE TRUSS IN

CONTRAMACE THITH THE DE FARRICATION.

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ITW BUILDING COMPONENTS GROUP, INC. POMPANO BEACH, FLORIDA

ALPINE

SUGUAS FLEMIN 27 No. 66648 CENS \* MAX. MAX. TOT. SPACING

LD. 60 PSF

24.0" DRWG A13030EE0207

DATE REF

2/23/07 ASCE7-02-GAB13030

#### ASCE 7-02: 110 MPH WIND SPEED, 15 MEAN HEIGHT, ENCLOSED, 11 1.00, EXPOSURE 0

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STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD		#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	GRADE	DIVACE
4' 11"	5' 0"	5' 0"		5' 4"	4' 9"	4' 9"	4' 9"	4' 11"		4' 6"	4' 6"	4' 9"		4' 4"	4' 4"	4' 4"	4. 5.		4' 0"	4' 0"	4' 2"	4' 3"	3' 9"	3' 9"	3 9"	3' 10"	BRACES	S
7' 5"	8' 5"	8' 5"	8' 5"	8, 5,	- 1	8' 5"	8' 5"	0.00	6' 5"	7' 6"	. 5		7' 8"		7' 4"	7' 4"	11 67	-	6' 1"	5	6' 8"	- 2.5	2,	6' 0"	6' 0"	6' 8"	GROUP A	EVT (1)
7' 5"	8' 7"	8' 5"	9' 1"	9' 1"	7' 3"	8' 5"	8. 5.	8' 8"	6, 5,	7' 6"	7' 7"	8' 3"	8' 3"	6' 4"	7' 4"	7' 4"	7' 10"		6' 1"	6' 2"	7' 2"	7' 2"	5, 5,	6' 0"	6' 0"	6' 10"	GROUP B	DIMOD W
9' 10"		10' 0"		10' 0"		10' 0"	10' 0"	10' 0"	8' 6"	9, 1,,	9' 1"	9' 1"		8' 4"	9' 1"	9' 1"	9' 1"	6' 11"	7' 11"	7' 11"	7' 11"	7' 11"	6' 9"	7' 11"	7' 11"	7' 11"	GROUP A	(1)
9' 10"	10' 6"	10' 6"	10' 9"	10' 9"	9' 7"	10' 0"	10' 0"	10' 3"		9' 6"		9' 9"		8' 4"	9' 1"	9' 1"	9' 4"	6' 11"	8' 0"		8' 6"	8' 6"	6' 9"	7' 11"	7' 11"	8' 1"	GROUP B	DIMES OF
11, 11,	11' 11"	11, 11,	11. 11.	11' 11"		11' 11"	11' 11"	11' 11"		10' 10"		10' 10"	10' 10"	10' 10"	10' 10"		10' 10"	9' 4"	9' 5"	9' 5"	1.5	9' 5"	9' 1"	9' 5"	9' 5"	9' 5"	GROUP A	100 (0)
12' 3"	12, 6,	12' 6"	E 33	12' 10"	1	111' 11"	11' 11"	12' 3"	11' 1"	11' 4"	11' 4"	11' 8"	11' 8"	10' 10"	10' 10"	10' 10"	11' 1"	9' 4"		9' 11"		10' 2"	9' 1"	9' 5"		9' 8"	GROUP B	DIMINO
14' 0"	14' 0"	14' 0"	14. 0.	1	1	14' 0"	14' 0"	14' 0"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	12' 11"	14' 0"	14' 0"	14' 0"	10' 10"			12' 5"		10' 7"	12' 3"		12' 5"	GROUP A	(1)
14' 0"		14' 0"	14 0"	14' 0"		14' 0"		14' 0"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	12' 11"		14' 0"	14' 0"		12' 6"	12' 8"		13' 5"		12' 3"	12' 4"	12' 9"	GROUP B	DIMIGH
14' 0"	1	1	14' 0"	14' 0"		14' 0"		14' 0"	14' 0"	14' 0"	14' 0"		14' 0"		14' 0"	14' 0"	14' 0"	100	14' 0"	14' 0"		14' 0"	14' 0"	14' 0"		14' 0"	GROUP A	(m)
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DOUGLAS FIR-LARCH
#3
STUD
STANDARD

SOUTHERN PINE #3

STUD

GROUP

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#1 & BTR #1

#1 / #2 STANDARD

#3

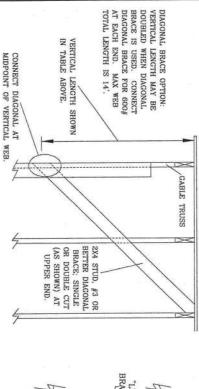
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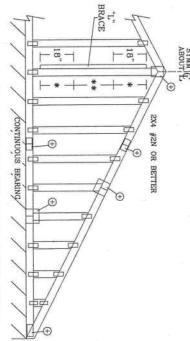
#3

STANDARD STUD BRACING GROUP SPECIES AND GRADES:

GROUP

A:





GABLE TRUSS DETAIL NOTES

SOUTHERN PINE

DOUGLAS FIR-LARCH

#2

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD). LIVE LOAD DEFLECTION CRITERIA IS L/240.

PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.

\*\* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C.

\*\* FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C.

IN 18" END ZONES AND 6" O.C. BETWEEN ZONES. "L" BRACING MUST BE A MINIMUM OF 80% OF WEB

MEMBER LENGTH.

	+ REFER TO COMMON TRUSS DESIGN FOR	VERTICAL LENGTH NO SE LESS THAN 4' 0" 1740 GREATER THAN 4' 0", BUT LESS THAN 11' 6" LESS THAN 11' 6" GREATER THAN 11' 6" GREATER THAN 11' 6" 2.53 PEAK, SPLICE, AND HEEL PLATESS.
	BUT	
	BUT	
TH		BUT

\*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SMPPING, INSTALLING AND BRACING. REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE CIRCUSS PLATE INSTITUTE, 218 MORTH LEE STER, SUITE 312, ALEXANDRIA, VA. 25214) AND VTCA VOUDD TRUSS COUNCIL BY AMERICA, 6300 ENTERRISE LN, HADISON, WI 53719) FOR SAFETY PRACTICES PRIDA TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED. TOP CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERTY ATTACHED RIGID CEILING.

REFER

TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH

WHORKMANEW FURNISH COPY OF THIS DESIGN TO INSTALLATION COMPRECTOR. ITY BCG, INC., SHALL

NOT BE RESONNESS, CTOR MAY DEVIATION FOR THIS DESIGN, ANY FAILURE TO BUILD THE RUSS. IN

CONTORNANCE THIS DAY BY EXCHANGING SHIPPING, METALLING & BRACING OF TRINSES.

ESTEN CONTORNATOR THIS DE FARRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRINSES.

FLY, BCG CONNESS TO BE HATE SEE HANDLING SHIPPING, INSTALLING SHIPPING, AND THE STALLING SHIPPING SHIPPING, METALLING SHIPPING SHIP

ITWBUILDING COMPONENTS GROUP, INC POMPANO BEACH, FLORIDA

ALPINE

SSIONAL ENGINEER No. 66648 \* MAX. MAX.

TOT. LD. 60 PSF DATE DRWG A11015EE0207 2/23/07

REF

ASCE7-02-GAB11015

SPACING 24.0"

TOP CHORD 2X4 #2 OR BETTER BOT CHORD 2X4 #2 OR BETTER WEBS 2X4 #3 OR BETTER

## PIGGYBACK DETAII

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.
THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

130 MPH WIND, 30 MEAN HGT, ASCE 7-98, ASCE 7-02 OR
ASCE 7-05, CLOSED BLGD, LOCATED ANYWHERE IN ROOF, CAT II,
EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

110 MPH WIND, 30' MEAN HGT, SBC
ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF
WIND TC DL=5 PSF, WIND BC DL=5 PSF
FRONT FACE (E,\*) PLATES MAY BE OFFSET FROM BACK FACE
PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.

E

MAX SIZE OF 2X12 #2 OR BETTER

20' FLAT TOP CHORD MAX SPAN

EITHER PLATE LOCATION IS ACCEPTABLE

OPTIONAL

TO FACE) MAY BE I ATTACH WITH (6) PER GUSSET.

(4) 6d BOX (0.099"X 2.",MIN) NAILS.

FACE) MAY BE USED SHEATHING GUSSETS (EACH FACE) MAY BE USED IN LIEU OF TRULOX PLATES. ATTACH WITH (6) 6d BOX (0.099"X 2.",MIN) NAILS PER GUSSET.

(4) IN CAP BC AND (4) IN BASE TRUSS FLAT TC.

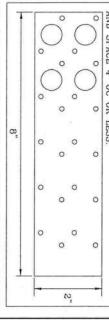
JOINT H D C B A 4X6 1.5X3 5X4 2X4 4X6 30 OR 3X6 TRULOX AT 4'
ROTATED VERTICALLY 2.5X4 SPANS 1.5X4 5X5 5X6 34 UP 2.5X4 1.5X4 5X6 38 TO 1.5X4 5X6 3X5 52 00,

ATTACH TRULOX PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

10' TO 14'	7'9" TO 10'	0' TO 7'9"	WEB LENGTH	
2x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d BOX (0.135"X 3.5",MIN) NAILS AT 4" OC	13x4 "I" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d BOX (0.113"X 2.5",MIN) NAILS AT 4" OC.	O' TO 7'9"   NO BRACING	REQUIRED BRACING	WEB BRACING CHART

### \* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4" OC OR LESS.



THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045

ALPINE  TWBUILDING COMPONENTS GROUP, INC.	>
**XINDERTANT** FURNI NOT BE RESPONSIBLE FO CONFORMANCE WITH TPI DESIGN CONFORMS WITH ITW. BCG CONNECTOR GALV STEEL APPLY F GALV STEEL APPLY F ANNEX A3 DF TPI 1-201	BRACING. REFER TO BO INSTITUTE, 218 NORTH A AMERICA, 6300 ENTERPR FUNCTIONS. UNLESS OF PANELS AND BOTTOM CH

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\*\*VARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BOSS (BUILDING COMPONENT SAFETY INSTRUCTIONS, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 260 NURTH LEE STR., SUITE 312, ALEXANDRA, VA. 223149 AND VICA VOUDD TRUSS COUNCIL OF AMERICA. 6300 ENTERPRISE IN, MADISON, VI 537199, FOR SAFETY PRACTICES PRIOR TO PERCORNING THESE FUNCTIONS. UNLESS OFFERVISE IN, MADISON, VI 537199, FOR SAFETY PRACTICES PRIOR TO PERCORNING THESE FUNCTIONS. UNLESS OFFERVISE IN, MADISON, VI 537199, FOR SAFETY PRACTICES PRIOR TO PERCORNING THESE FUNCTIONS.

PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.

NATI BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN TO INSTALLATION CONTRACTOR. IT'N BCG, INC., SHALL CONTRACTOR. IT'N BCG, INC., STATE, ARE HADE OF 20/18/16/16 NDS (NATIONAL DESIGN SEC), BY AFRAY AND IT'N BCG, GENWECTOR PLATES ARE HADE OF 20/18/16/16 (NATIONAL DESIGN SEC), BY AFRAY AND IT'N BCG, GENWECTOR PLATES ARE HADE OF 20/18/16/16 (NATIONAL DESIGN SEC), BY GO CONNECTOR PLATES TO EACH FACE OF 1905 AND UNLESS OTHERWISE LOCATED BY HIS SHALL BE FER SHALL SHALL BE FER SHALL SHALL BE FER SHALL B

Mary.	I Serverens CALLE	dim			
CDACINIC	47 PSF AT 1.15 DUR. FAC	50 PSF AT 1.25 DUR. FAC	1.33	55 PSF AT	MAX
	PSF DUR.	PSF DUR.	DUR.	PSF	MAX LOADING
0 1 0 "	AT FAC.	FAC.	FAC.	AT	DING
		-ENG	DRWG	DATE	REF
		-ENG DLJ/KAR	DRWG PIGBACKB0207	2/23/07	PIGGYBACK