DATE 12/18/2006

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT 000025318

APPLICANT HUGO ESCALANTE PHONE 386-288-8666

ADDRESS PO BOX 280 FORT WHITE FL 32038

OWNER HUGO ESCALANTE PHONE 386-288-8666

ADDRESS 229 SW MELBA GLEN LAKE CITY FL 32025

CONTRACTOR HUGO ESCALANTE(EWPL INC) PHONE 288-8666

LOCATION OF PROPERTY 47 S, R 242, R WISE DRIVE, L GARDNER DR, R MELBA GLEN, 5TH LOT ON LEFT

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 94400.00

HEATED FLOOR AREA 1888.00 TOTAL AREA 2716.00 HEIGHT 22.00 STORIES 1

FOUNDATION CONCRETE WALLS FRAMED ROOF PITCH 7/12 FLOOR SLAB

LAND USE & ZONING RSF-2 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 0 FLOOD ZONE APP DEVELOPMENT PERMIT NO.

PARCEL ID 24-4S-16-03113-156 SUBDIVISION WISE ESTATES

LOT 26 BLOCK C PHASE UNIT TOTAL ACRES 0.59

000001279		CRC1326976	✓ Hugo Escalante	
Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor	
CULVERT	06-01053N	BK	JH	N
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: NOC ON FILE, PLAT REQUIRES 1ST FLOOR TO BE AT 98.3 FEET, ELEVATION LETTER REQUIRED-SLAB

Check # or Cash 4836

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by

Foundation _____ date/app. by

Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by

Slab _____ date/app. by

Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by

Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by

Heat & Air Duct _____ date/app. by

Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by

C.O. Final _____ date/app. by

Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by

Pool _____ date/app. by

Reconnection _____ date/app. by

Pump pole _____ date/app. by

Utility Pole _____ date/app. by

M/H Pole _____ date/app. by

Travel Trailer _____ date/app. by

Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 475.00 CERTIFICATION FEE $ 13.58 SURCHARGE FEE $ 13.58

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ 25.00 **TOTAL FEE** 602.16

INSPECTORS OFFICE L. H CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

Revised 9-23-04

For Office Use Only Application # 061218 Date Received 12/6 By JW Permit # 1279 / 25318

Application Approved by - Zoning Official BLK Date 12.12.06 Plans Examiner OKJTH Date 12-6-06

Flood Zone Apx plat Development Permit N/A Zoning RSF-2 Land Use Plan Map Category RES. Low Density

Comments MH Notice, Plat Requires 1st Floor to be at 98.3' Elevation Letter Required

Applicants Name Hugo Escalante Phone 386-288-8666

Address P.O. Box 280, Fort White, FL 32038

Owners Name Hugo Escalante Phone 386-288-8666

911 Address 229 S.W. Melba Glen, Lake City, FL

Contractors Name Hugo Escalante, (EWPC INC) Phone 386-288-8666

Address P.O. Box 280, Fort White, FL 32038

Fee Simple Owner Name & Address None

Bonding Co. Name & Address None

Architect/Engineer Name & Address Daniel Shaheen, Lake City, FL

Mortgage Lenders Name & Address None

Circle the correct power company - FL Power & Light - (Clay Elec.) - Suwannee Valley Elec. - Progressive Energy

Property ID Number 34-4S-16-03113-156 Estimated Cost of Construction $140,000-

Subdivision Name WISE Estates Lot 26 Block C Unit ____ Phase ____

Driving Directions 47 S, TR on 242, TR on WISE DRIVE, TL on Gardner DRIVE, TR on Melba Glen, 5th lot on left.

Type of Construction SFD Number of Existing Dwellings on Property 0

Total Acreage .59 Lot Size .59 Do you need a - (Culvert Permit) or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 65' Side 25' Side 25' Rear 65'

Total Building Height 22'-0" Number of Stories 1 Heated Floor Area 1888 SF Roof Pitch 7-12

TOTAL 2716

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Hugo Escalante
Owner Builder or Agent (including Contractor)

Hugo Escalante
Contractor Signature

Contractors License Number CBC1326987

Competency Card Number ____

NOTARY STAMP/SEAL

MARSHA B. WARD
MY COMMISSION # DD 273841
EXPIRES: March 9, 2008
Bonded Thru Notary Public Underwriters

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me this 6 day of December 2006.

Personally known ____ or Produced Identification ✓ FLDL

Marsha B. Ward
Notary Signature

NOTICE OF COMMENCEMENT FORM
COLUMBIA COUNTY, FLORIDA

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

Tax Parcel ID Number 24-4S-16-03113-156

1. **Description of property: (legal description of the property and street address or 911 address)**
   Lot 26 Block "C" Wise Estates S/D WD 1030-350
   911 Address: 229 S.W. Melba Glen, Lake City, FL
2. **General description of improvement:** New Single Family Residence
3. **Owner Name & Address** Hugo Escalante, P.O. Box 280, Fort White, FL 32038
   Interest in Property 100%
4. **Name & Address of Fee Simple Owner (if other than owner):** None
5. **Contractor Name** Hugo Escalante **Phone Number** 3862888666
   Address P.O. Box 280, Fort White, FL 32038
6. **Surety Holders Name** None **Phone Number**
   Address None
   Amount of Bond None
7. **Lender Name** None
   Address None

Inst:2006028771 Date:12/06/2006 Time:11:40
DC,P.DeWitt Cason,Columbia County B:1104 P:286

8. **Persons within the State of Florida designated by the Owner upon whom notices or other documents may be served as provided by section 718.13 (1)(a) 7; Florida Statutes:**
   Name Hugo Escalante **Phone Number** 3862888066
   Address 194 S.W. Rainhouse Ct, Fort White, FL 32038
9. **In addition to himself/herself the owner designates** Marleen Escalante of Fort White, FL **to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) – (a) 7. Phone Number of the designee** 386-497-1880
10. **Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording, (Unless a different date is specified)**

NOTICE AS PER CHAPTER 713, Florida Statutes:
The owner must sign the notice of commencement and no one else may be permitted to sign in his/her stead.

Hugo Escalante
Signature of Owner

Sworn to (or affirmed) and subscribed before day of December 6, 2006

NOTARY STAMP/SEAL

MARSHA B. WARD
MY COMMISSION # DD 279841
EXPIRES: March 9, 2008
Bonded Thru Notary Public Underwriters

Marsha B Ward
Signature of Notary

Columbia County Property Appraiser

DB Last Updated: 11/20/2006

Parcel: 24-4S-16-03113-156

2007 Proposed Values

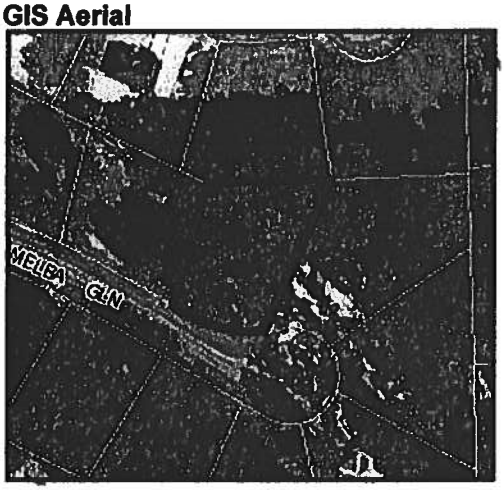
Tax Record | Property Card | Interactive GIS Map | Print

<< Prev Search Result: 6 of 10 Next >>

Owner & Property Info

Owner's Name	ESCALANTE HUGO		
Site Address	MELBA		
Mailing Address	P O BOX 280 FT WHITE, FL 32038		
Use Desc. (code)	VACANT (000000)		
Neighborhood	24416.00	**Tax District**	2
UD Codes	MKTA06	**Market Area**	06
Total Land Area	0.590 ACRES		
Description	LOT 26 BLOCK C WISE ESTATE S/D WD 1030-350.		

GIS Aerial



Property & Assessment Values

Mkt Land Value	cnt: (1)	$36,000.00
Ag Land Value	cnt: (0)	$0.00
Building Value	cnt: (0)	$0.00
XFOB Value	cnt: (0)	$0.00
Total Appraised Value		$36,000.00

Just Value	$36,000.00
Class Value	$0.00
Assessed Value	$36,000.00
Exempt Value	$0.00
Total Taxable Value	$36,000.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale Vimp	Sale Qual	Sale RCode	Sale Price
11/5/2004	1030/350	WD	V	Q		$24,900.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000000	VAC RES (MKT)	1.000 LT - (.590AC)	1.00/1.00/1.00/1.00	$36,000.00	$36,000.00

Columbia County Property Appraiser DB Last Updated: 11/20/2006

<< Prev 6 of 10 Next >>

LYNCH WELL DRILLING, INC.

173 SW Tustenuggee Ave
Lake City, FL. 32025
Phone 386-752-6677
Fax 386-752-1477

Wise Est. Lot 26

Building Permit # _____ Owner's Name Hugo Escalante

Well Depth _____ Ft. Casing Depth _____ Ft. Water Level _____ Ft.

Casing Size 4 inch Steel Pump Installation: Deep Well Submersible

Pump Make Aermotor Pump Model SQ20-106 HP 1

System Pressure (PSI) _____ On 40 Off 60 Average Pressure 50

Pumping System GPM at average pressure and pumping level 20 (GPM)

Tank Installation: (Bladder)/Galvanized Make Challenger
Model AC244 Size 81

Tank Draw-down per cycle at system pressure 25.1 gallons

I HEREBY VERTIFY THAT THIS WATER WELL SYSTEM HAS BEEN INSTALLED AS PER THE ABOVE INFORMATION.

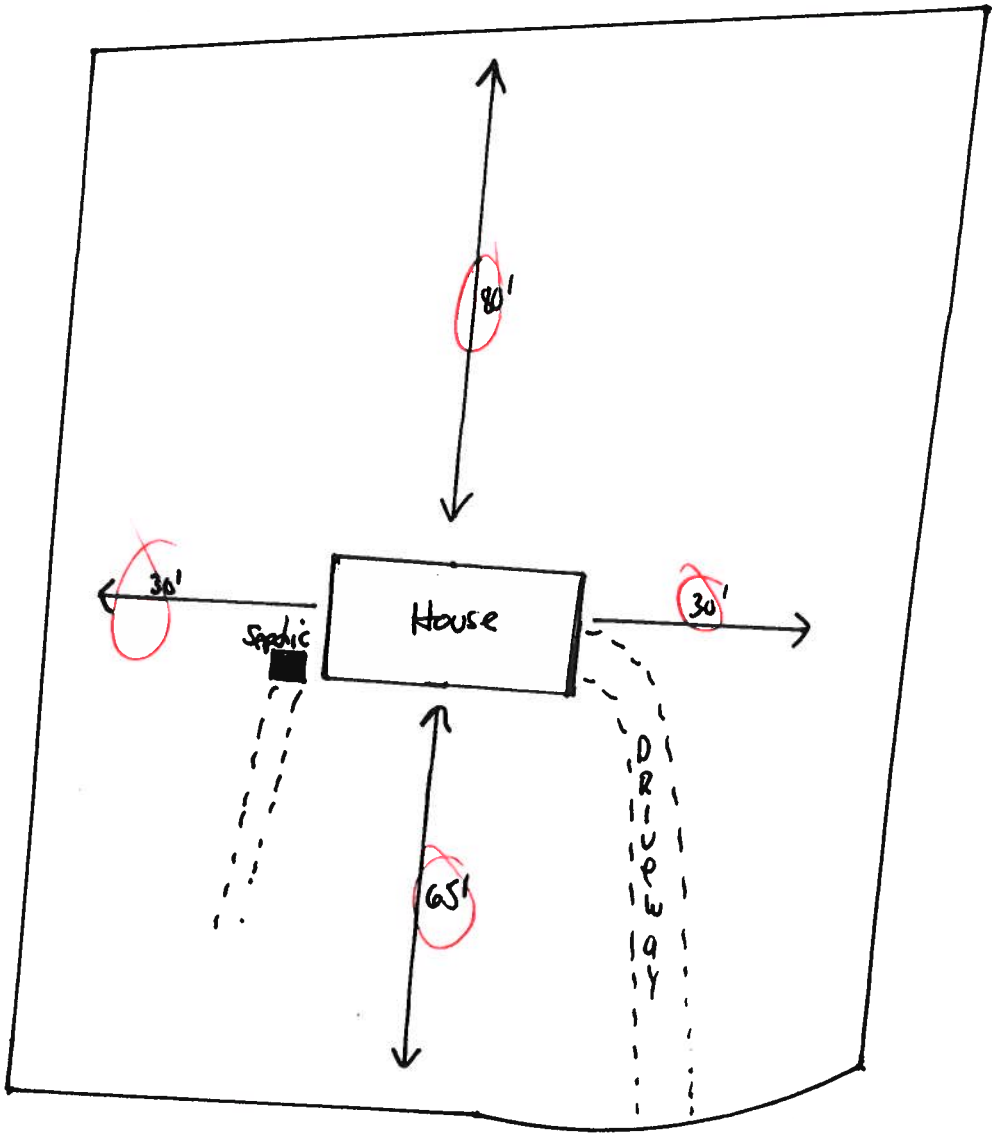
Linda Newcomb
Signature

Linda Newcomb
Print Name

2609
License Number

11-7-06
Date

Lot 96 Wise Estates
Parcel # 34-45-16-03113-156
911 Address: 229 S.W. Melba Glen



Columbia County Building Department
Culvert Permit

Culvert Permit No.
000001279

DATE 12/18/2006 PARCEL ID # 24-4S-16-03113-156

APPLICANT HUGO ESCALANTE PHONE 386-288-8666

ADDRESS PO BOX 280 FORT WHITE FL 32038

OWNER HUGO ESCALANTE PHONE 386-288-8666

ADDRESS 229 SW MELBA GLEN LAKE CITY FL 32025

CONTRACTOR HUGO ESCALANTE(EWPL INC) PHONE 288-8666

LOCATION OF PROPERTY 47 S, R 242, R WISE DRIVE, L GARDNER DR, R MELBA GLEN,

5TH LOT ON LEFT

SUBDIVISION/LOT/BLOCK/PHASE/UNIT WISE ESTATES 26 C

SIGNATURE

INSTALLATION REQUIREMENTS

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

[ ] Department of Transportation Permit installation approved standards.

[ ] Other _____

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00

FORM 600A-2004 EnergyGauge® 4.0

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**MADISON MOD - LOT 26 WISE**	Builder:	**EWPL INC**
Address:	**Lot: 26, Sub: Wise Estates, Plat:**	Permitting Office:	Columbia
City, State:	**Lake City, FL**	Permit Number:	
Owner:	**EWPL INC**	Jurisdiction Number:	22100 C
Climate Zone:	**North**		

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 36.0 kBtu/hr
3. Number of units, if multi-family	1		SEER: 13.00
4. Number of Bedrooms	3	b. N/A	
5. Is this a worst case?	Yes		
6. Conditioned floor area (ft²)	1888 ft²	c. N/A	
7. Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)			
a. U-factor: (or Single or Double DEFAULT)	Description Area 7a. (Dble Default) 225.3 ft²	13. Heating systems	
b. SHGC: (or Clear or Tint DEFAULT)	7b. (Clear) 225.3 ft²	a. Electric Heat Pump	Cap: 36.0 kBtu/hr
			HSPF: 7.20
8. Floor types		b. N/A	
a. Slab-On-Grade Edge Insulation	R=0.0, 219.0(p) ft		
b. N/A		c. N/A	
c. N/A			
9. Wall types		14. Hot water systems	
a. Frame, Wood, Adjacent	R=13.0, 198.0 ft²	a. Electric Resistance	Cap: 40.0 gallons
b. Frame, Wood, Exterior	R=13.0, 1732.0 ft²		EF: 0.92
c. N/A		b. N/A	
d. N/A			
e. N/A		c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)	
10. Ceiling types			
a. Under Attic	R=30.0, 1888.0 ft²	15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	CF,
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 160.0 ft		
b. N/A			

Glass/Floor Area: 0.14 Total as-built points: 26352 Total base points: 29288 **PASS**

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: _____

DATE: 11-14-06

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 26, Sub: Wise Estates, Plat: , Lake City, FL,** PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points
.18	1888.0	20.04	6810.4	Double, Clear	E	1.5	9.0	53.7	42.06	0.97	2189.1
				Double, Clear	E	5.0	10.0	13.3	42.06	0.73	406.9
				Double, Clear	E	5.0	4.0	9.0	42.06	0.46	175.3
				Double, Clear	E	1.5	6.0	30.0	42.06	0.91	1151.8
				Double, Clear	N	1.5	5.0	16.0	19.20	0.92	281.3
				Double, Clear	S	1.5	1.5	4.0	35.87	0.52	74.7
				Double, Clear	W	1.5	6.0	60.0	38.52	0.91	2111.2
				Double, Clear	W	8.0	10.0	24.0	38.52	0.58	539.7
				Double, Clear	N	1.5	6.0	20.0	19.20	0.94	360.4
				Double, Clear	W	1.5	7.5	42.0	38.52	0.95	1535.7
				As-Built Total:				**272.0**			**8826.0**
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points
Adjacent	198.0	0.70	138.6	Frame, Wood, Adjacent			13.0	198.0	0.60		118.8
Exterior	1732.0	1.70	2944.4	Frame, Wood, Exterior			13.0	1732.0	1.50		2598.0
Base Total:	**1930.0**		**3083.0**	**As-Built Total:**				**1930.0**			**2716.8**
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points
Adjacent	20.0	2.40	48.0	Exterior Wood				20.0	6.10		122.0
Exterior	68.0	6.10	414.8	Exterior Wood				48.0	6.10		292.8
				Adjacent Wood				20.0	2.40		48.0
Base Total:	**88.0**		**462.8**	**As-Built Total:**				**88.0**			**462.8**
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points
Under Attic	1888.0	1.73	3266.2	Under Attic			30.0	1888.0	1.73 X 1.00		3266.2
Base Total:	**1888.0**		**3266.2**	**As-Built Total:**				**1888.0**			**3266.2**
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points
Slab	219.0(p)	-37.0	-8103.0	Slab-On-Grade Edge Insulation			0.0	219.0(p	-41.20		-9022.8
Raised	0.0	0.00	0.0								
Base Total:			**-8103.0**	**As-Built Total:**				**219.0**			**-9022.8**
INFILTRATION	Area X	BSPM =	Points					Area X	SPM =		Points
	1888.0	10.21	19276.5					1888.0	10.21		19276.5

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 26, Sub: Wise Estates, Plat: , Lake City, FL,** PERMIT #:

BASE			AS-BUILT					
Summer Base Points: 24795.9			**Summer As-Built Points:**					**25525.6**
Total Summer Points	X System Multiplier	= Cooling Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points
			(sys 1: Central Unit 36000 btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0(INS)					
24795.9	**0.4266**	**10577.9**	25526 **25525.6**	1.00 **1.00**	(1.09 x 1.147 x 0.91) **1.138**	0.263 **0.263**	0.950 **0.950**	7243.1 **7243.1**

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 26, Sub: Wise Estates, Plat: , Lake City, FL,** PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points
.18	1888.0	12.74	4329.6	Double, Clear	E	1.5	9.0	53.7	18.79	1.02	1024.3
				Double, Clear	E	5.0	10.0	13.3	18.79	1.12	280.6
				Double, Clear	E	5.0	4.0	9.0	18.79	1.35	227.9
				Double, Clear	E	1.5	6.0	30.0	18.79	1.04	583.8
				Double, Clear	N	1.5	5.0	16.0	24.58	1.00	394.7
				Double, Clear	S	1.5	1.5	4.0	13.30	2.73	145.3
				Double, Clear	W	1.5	6.0	60.0	20.73	1.02	1272.9
				Double, Clear	W	8.0	10.0	24.0	20.73	1.14	568.5
				Double, Clear	N	1.5	6.0	20.0	24.58	1.00	492.7
				Double, Clear	W	1.5	7.5	42.0	20.73	1.01	882.4
				As-Built Total:				**272.0**			**5873.3**
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points
Adjacent	198.0	3.60	712.8	Frame, Wood, Adjacent			13.0	198.0	3.30		653.4
Exterior	1732.0	3.70	6408.4	Frame, Wood, Exterior			13.0	1732.0	3.40		5888.8
Base Total:	**1930.0**		**7121.2**	**As-Built Total:**				**1930.0**			**6542.2**
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =		Points
Adjacent	20.0	11.50	230.0	Exterior Wood				20.0	12.30		246.0
Exterior	68.0	12.30	836.4	Exterior Wood				48.0	12.30		590.4
				Adjacent Wood				20.0	11.50		230.0
Base Total:	**88.0**		**1066.4**	**As-Built Total:**				**88.0**			**1066.4**
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X	WCM =	Points
Under Attic	1888.0	2.05	3870.4	Under Attic			30.0	1888.0	2.05 X	1.00	3870.4
Base Total:	**1888.0**		**3870.4**	**As-Built Total:**				**1888.0**			**3870.4**
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points
Slab	219.0(p)	8.9	1949.1	Slab-On-Grade Edge Insulation			0.0	219.0(p	18.80		4117.2
Raised	0.0	0.00	0.0								
Base Total:			**1949.1**	**As-Built Total:**				**219.0**			**4117.2**
INFILTRATION	Area X	BWPM =	Points					Area X	WPM =		Points
	1888.0	-0.59	-1113.9					1888.0	-0.59		-1113.9

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 26, Sub: Wise Estates, Plat: , Lake City, FL,**	PERMIT #:

BASE			AS-BUILT					
Winter Base Points:		**17222.7**	**Winter As-Built Points:**					**20355.6**
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
			(sys 1: Electric Heat Pump 36000 btuh ,EFF(7.2) Ducts:Unc(S),Unc(R),Int(AH),R6.0					
17222.7	0.6274	10805.5	20355.6	1.00	1.162	0.474	1.000	11204.2

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 26, Sub: Wise Estates, Plat: , Lake City, FL,** PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X Multiplier	= Total		Tank Volume	EF	Number of Bedrooms	X Tank Ratio	X Multiplier	X Credit Multiplier	= Total	
3	2635.00	7905.0		40.0	0.92	3	1.00	2635.00	1.00	7905.0	
				As-Built Total:						**7905.0**	

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
10578	**10806**	**7905**	**29288**	**7243**	**11204**	**7905**	**26352**

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 26, Sub: Wise Estates, Plat: , Lake City, FL,** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 84.8

The higher the score, the more efficient the home.

EWPL INC, Lot: 26, Sub: Wise Estates, Plat: , Lake City, FL,

1.	New construction or existing	New		12.	Cooling systems	
2.	Single family or multi-family	Single family			a. Central Unit	Cap: 36.0 kBtu/hr
3.	Number of units, if multi-family	1				SEER: 13.00
4.	Number of Bedrooms	3			b. N/A	
5.	Is this a worst case?	Yes				
6.	Conditioned floor area (ft²)	1888 ft²			c. N/A	
7.	Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)					
	a. U-factor:	Description Area		13.	Heating systems	
	(or Single or Double DEFAULT)	7a. (Dble Default) 225.3 ft²			a. Electric Heat Pump	Cap: 36.0 kBtu/hr
	b. SHGC:					HSPF: 7.20
	(or Clear or Tint DEFAULT)	7b. (Clear) 225.3 ft²			b. N/A	
8.	Floor types					
	a. Slab-On-Grade Edge Insulation	R=0.0, 219.0(p) ft			c. N/A	
	b. N/A					
	c. N/A			14.	Hot water systems	
9.	Wall types				a. Electric Resistance	Cap: 40.0 gallons
	a. Frame, Wood, Adjacent	R=13.0, 198.0 ft²				EF: 0.92
	b. Frame, Wood, Exterior	R=13.0, 1732.0 ft²			b. N/A	
	c. N/A					
	d. N/A				c. Conservation credits	
	e. N/A				(HR-Heat recovery, Solar	
10.	Ceiling types				DHP-Dedicated heat pump)	
	a. Under Attic	R=30.0, 1888.0 ft²		15.	HVAC credits	CF,
	b. N/A				(CF-Ceiling fan, CV-Cross ventilation,	
	c. N/A				HF-Whole house fan,	
11.	Ducts				PT-Programmable Thermostat,	
	a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 160.0 ft			MZ-C-Multizone cooling,	
	b. N/A				MZ-H-Multizone heating)	

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

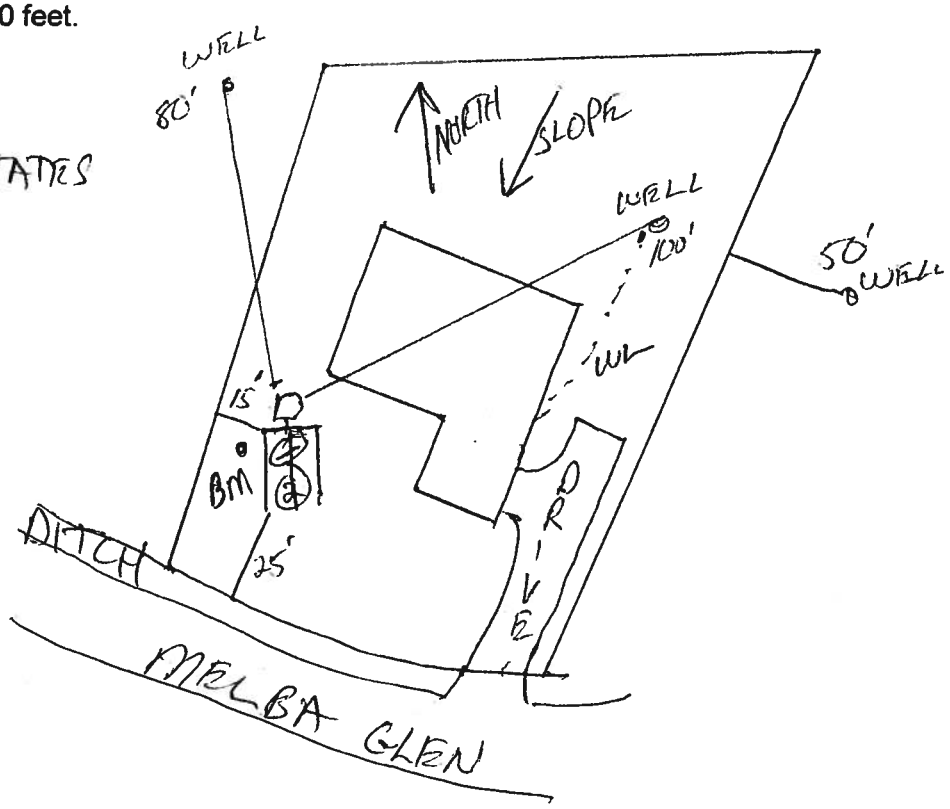
1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

STATE OF FLORIDA
DEPARTMENT OF HEALTH
APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 06-0153N

---------- PART II - SITEPLAN ----------

Scale: 1 inch = 50 feet.



Notes: _____

Site Plan submitted by: _____ MASTER CONTRACTOR

Plan Approved ✓ Not Approved _____ Date 12/5/06

By _____ Columbia County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

Notice of Treatment 12490

Applicator: Florida Pest Control & Chemical Co. (www.flapest .com)
Address: 536 SE Baya Ave
City Lake City Phone 752-1702

Site Location: Subdivision Wise Estates EWPL Const
Lot # 26 Block# _____ Permit # 25318
Address 229 SW Melba Cir

Product used	Active Ingredient	% Concentration
☐ Dursban TC	Chlorpyrifos	0.5%
☐ Termidor	Fipronil	0.06%
☒ Bora-Care	Disodium Octaborate Tetrahydrate	23.0%

Type treatment: ☐ Soil ☒ Wood

Area Treated	Square feet	Linear feet	Gallons Applied
Dwelling	2716	774	60 gals

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____.

5-17-07 Date | 8:30 Time | F Ess Print Technician's Name

Remarks: _____

Applicator - White Permit File - Canary Permit Holder - Pink
6/04 ©

Donald F. Lee & Associates, Inc.

Surveyors & Engineers

140 NW Ridgewood Avenue
Lake City, Florida 32055
(386) 755-6166
Fax (386) 755-6167
donald@dlfa.com

J5318

Thursday, January 12, 2007

FROM: Tim Delbene, P.L.S.

TO: Columbia County Building & Zoning Dept.

CC: EWPL (Hugo Escalante)

RE: Foundation Elevation Check – Lot 26, Wise Estates

We have obtained elevations on a foundation under construction on the above referenced lot (Date of Survey: 1/10/2007). The elevations are based on Local Benchmark Datum. The results are as follows:

Floor Elevation (at Stemwall): 99.52'

The record subdivision plat for Cannon Creek Place indicates a minimum floor elevation of 98.3' for the subject Lot 26.

SIGNED: _____

Timothy A. Delbene, P.L.S.
Florida Reg. Cert. No. 5594

DATE: 1/12/2007.

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 24-4S-16-03113-156 **Building permit No.** 000025318

Use Classification SFD,UTILITY **Fire:** 70.62

Permit Holder HUGO ESCALANTE(EWPL INC) **Waste:** 184.25

Owner of Building HUGO ESCALANTE **Total:** 254.87

Location: 229 SW MELBA GLEN

Date: 11/14/2007

Wayne A. Russ

Building Inspector

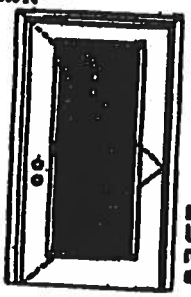
POST IN A CONSPICUOUS PLACE
(Business Places Only)

X
Glazed Inswing Unit

COP-WL-EN4141-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+50.5/-50.5
Limited water (unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistance for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the ratings required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:
1/4 GLASS:

1/2 GLASS:

*This glass lit may also be used in the following door styles: 6-panel; 8-panel with mouth; Quatrex 6-panel; Quatrex 8-panel with mouth.

Entergy
Entry Systems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

X
Glazed Inswing Unit

COP WL FL4141-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:
3/4 GLASS: **FULL GLASS:**

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9

Certifying Engineer and License Number: Barry D. Portnoy, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Door panels constructed from 25-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.032" steel. Bottom end rails constructed of 0.032" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

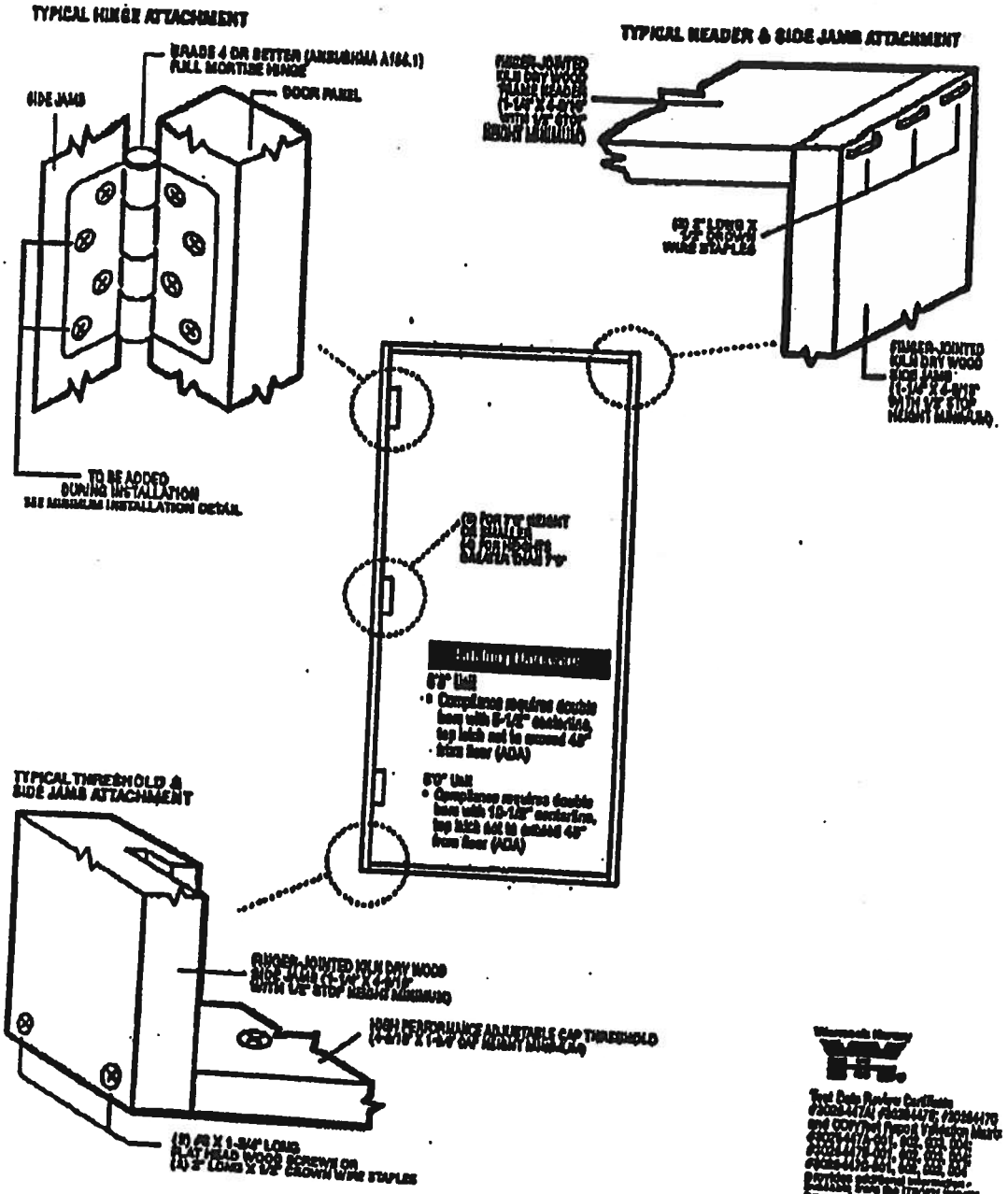
Entergy
Entry Systems
June 17, 2002

FRENDOOR
Sold solely from
Masonite®
Masonite International Corporation

X
Unit

MAD-WI-MA0001-02

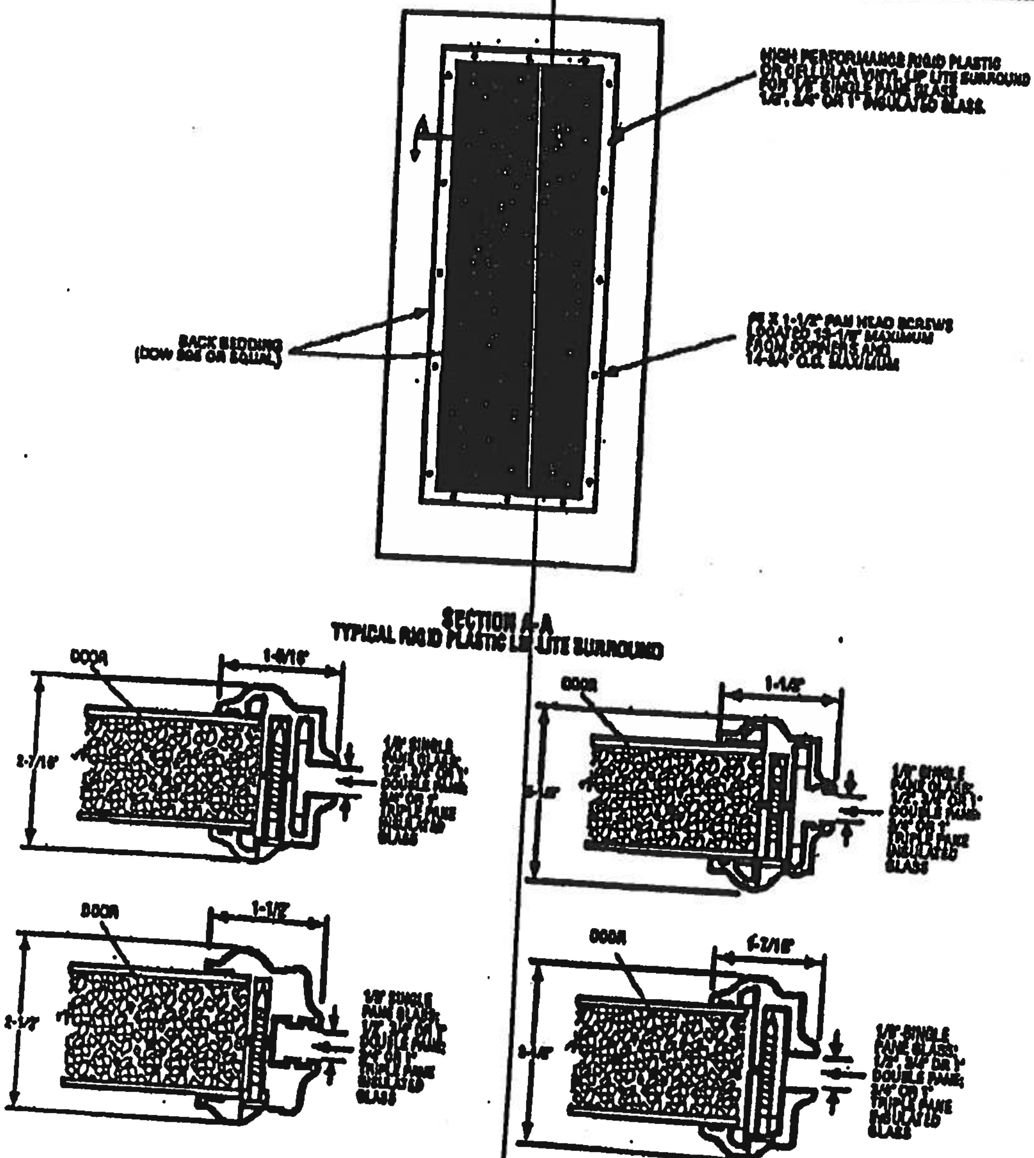
INSWING UNIT WITH SINGLE DOOR



Masonite

MAD-WL-MA0041-02

GLASS INSERT IN DOOR OR SIDELITE PANEL



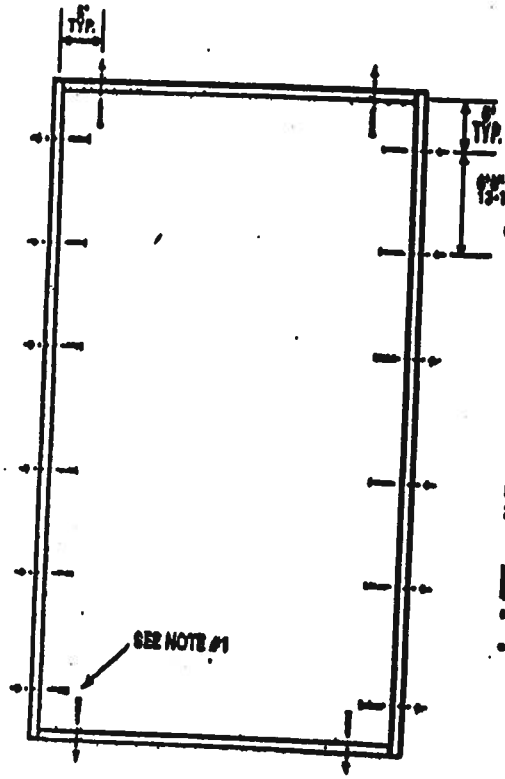
*Glass inserts to be sub-listed by Intertek Testing Services/ETL Semko or approved validation service.

Masonite International Corporation

X
Unit

MID-WL-MA0001-02

SINGLE DOOR



Minimum Fastener Count:

- 6 per vertical framing member
- 2 per horizontal framing member

Hinge and strike plates require two #-1.5" lag screws per location.

Rough Opening (RO):

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Warnock Hersey Test Data Review Certificate #3023647A, #3023647B, #3023647C and CHPET Report Validation Matrix #3023647A-001, 002, 003, 004; #3023647B-001, 002, 003, 004; #3023647C-001, 002, 003, 004 provide additional information - available from the ITW/WH website (www.warnock.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- UNITS COVERED BY COP DOCUMENT 8548*, 8256*, 8261*, 8248, 8251* or 8258
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

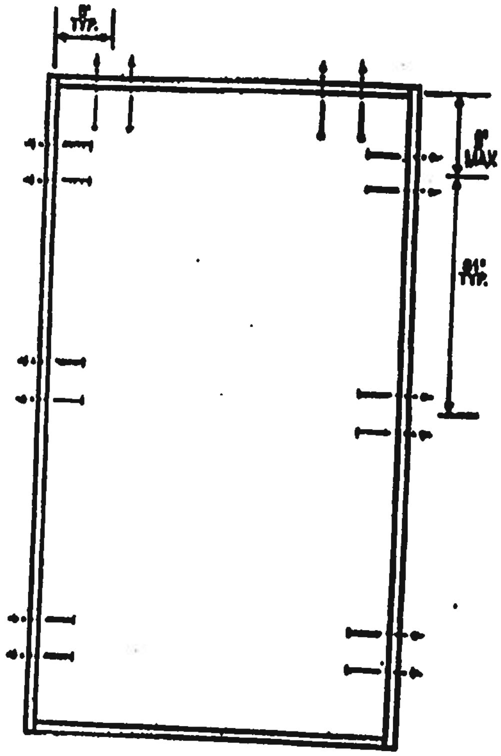
1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 400 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade County approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

Masonite

X
Unit

MID-WL-MA0001-02

SINGLE DOOR



Minimum Fastener Count

- 6 per vertical framing member for 7'0" height and smaller
- 8 per vertical framing member for heights greater than 7'0"
- 4 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Warnock Hersey [illegible]

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- UNITS COVERED BY COP DOCUMENT 3240*, 3285*, 3241*, 3245, 3251* or 3286
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include 10d common nails. Threshold fasteners analyzed for this unit include Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The common nail single shear design values come from ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment of 1-1/4".
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

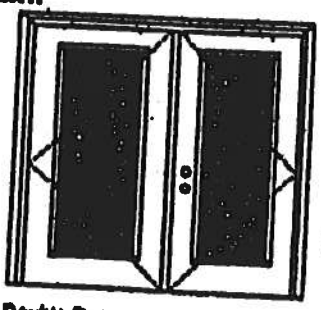
Masonite

XX
Glazed Outswing Unit

COP-WL-FN4162-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Double Door
Maximum unit size - 6'0" x 7'0"

Design Pressure
+50.5/-50.6
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressures and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed -- see MAD-WL-MA0012-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed -- see MID-WL-MA0002-02.

APPROVED DOOR STYLES:
1/4 GLASS:

1/2 GLASS:

*This glass lit may also be used in the following door styles: 6-panel 6-panel with small fanlight 6-panel fanlight 8-panel with small

Entergy
Entry Systems
June 17, 2002

Masonite International Corporation

XX
Glazed Outswing Unit

COP-WI-FN4162-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

1/4 GLASS:

FULL GLASS:

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top and rails constructed of 0.032" steel. Bottom and rails constructed of 0.032" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum bumper threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56539

Entergy
Entry Systems

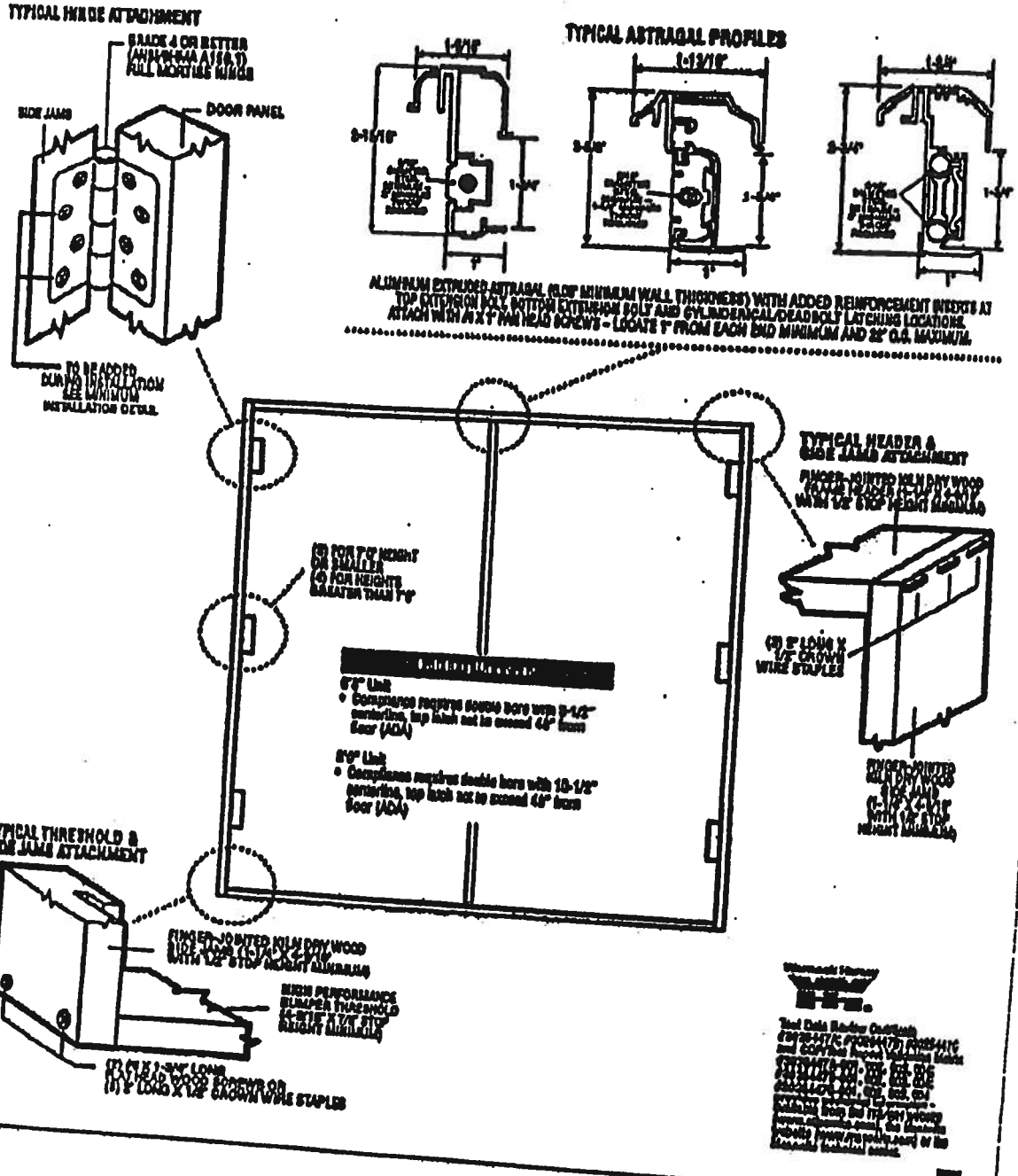
TRIMCORE Premium Quality Wood

Masonite
Masonite International Corporation

XX
Unit

MAD WL-MA0012-02

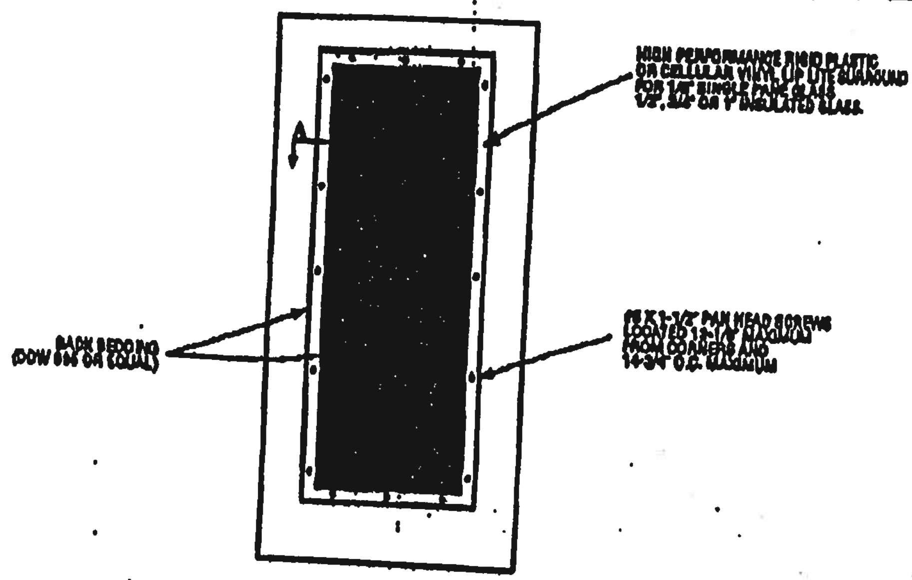
OUTSWING UNITS WITH DOUBLE DOOR



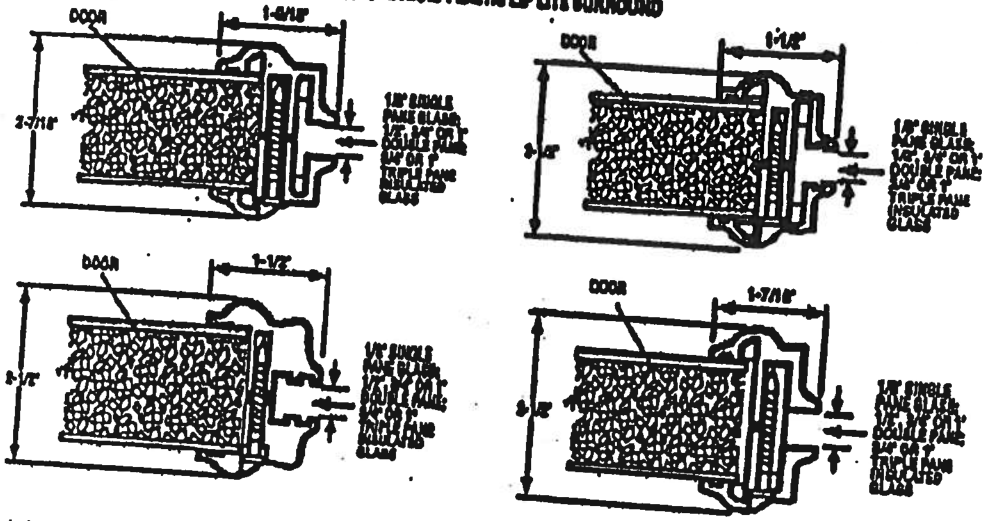
Masonite

MAD-WL-MA0041-02

GLASS INSERT IN DOOR OR SIDELITE PANEL



SECTION A-A
TYPICAL RIGID PLASTIC LIP LITE SURROUND



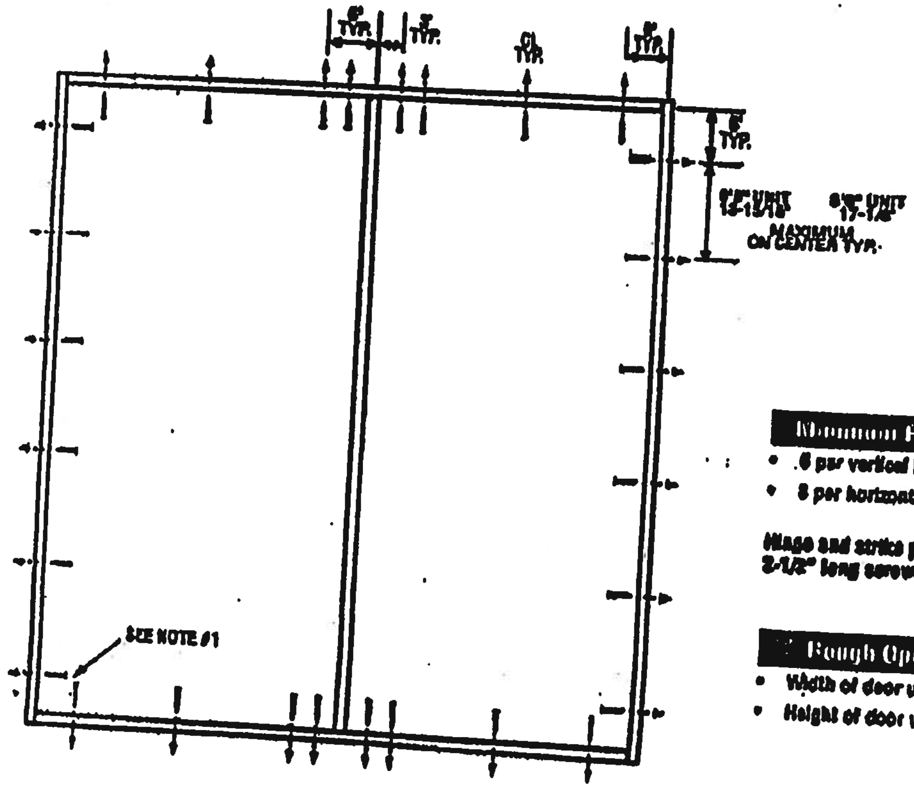
*Glass inserts to be sub-listed by Intertek Testing Services/ETL Semko or approved validation service.

Miami-Dade [illegible]

XX
Unit

MID-WL-MA0002-02

DOUBLE DOOR



Minimum Fastener Count

- 6 per vertical framing member
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4".

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- UNITS COVERED BY COP DOCUMENT 8247*, 8287*, 8242*, 8247, 8232* or 8287 Compliance requires that 6" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

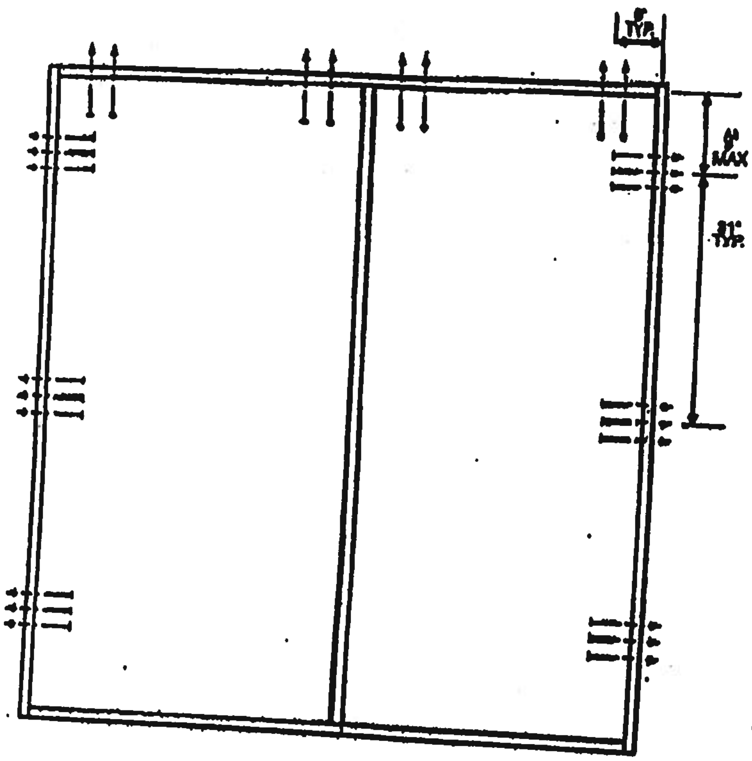
1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade County approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

Masonite

XX
Unit

MID-WL-MA0002-02

DOUBLE DOOR



Minimum Fastener Count

- 6 per vertical framing member for 7'0" heights and smaller
- 8 per vertical framing member for heights greater than 7'0"
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

This Data Review Certificate #30204473; #30204478; #30204470 and COP/TAS Based Validation Matrix #30204474-001, 002, 003, 004; #30204478-001, 002, 003, 004 #30204470-001, 002, 003, 004 provides additional information - available from the ITS/WH website [illegible] (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 2 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- UNITS COVERED BY COP DOCUMENT 0247*, 0257*, 0242*, 0247, 0252* or 0257 Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 wood screws and 10d common nails. Threshold fasteners analyzed for this unit include Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw and common nail single shear design values come from ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment of 1-1/4".
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

MI HOME PRODUCTS

MI Home Products, Inc.
650 West Market St.
P.O. Box 370
Gratz, PA 17030-0370

(717) 365-3300
(717) 362-7025 Fax

740/744 Single Hung (Fin & Flange)
165 Single Hung (Fin & Flange)
BB165/740/744 Fixed (Fin & Flange)

- Test Reports
  - 165 Single Hung
    - #CTLA-787W (Fin)
    - #CTLA-787W-1 (Flange)
  - 740/744 Single Hung
    - #01-40351.03 (Fin)
    - #01-40351.04 (Flange)
  - 165/740/744 Fixed
    - #NCTL-310-0005-2.1 (Fin)
    - # NCTL-310-0005-5.1 (Flange)
    - #01-40486.03 (2-Panel Fixed)
- Installation Instructions
- Sample 110/120/140 MPH Labels

AAMA/NWWDA 101/I.S.2-97
TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 740/744
TYPE: Aluminum Single Hung Window with Nail Fin

Title of Test	Results
Rating	H R45 52 x 72
Overall Design Pressure	45 psf
Operating Force	24 lb max.
Air Infiltration	0.10 cfm/ft^2
Water Resistance	6.75 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-40351.03 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess

Mark A. Hess, Technician

MAH:baw

Allen N. Reeves
15 FEBRUARY 2002

ALLEN N. REEVES
CERTIFICATE
NO. 19357
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

THIS FENESTRATION PRODUCT COMPLIES* WITH THE

NEW FLORIDA BUILDING CODE

FOR RESIDENTIAL BUILDINGS WITH A MEAN ROOF HEIGHT OF 30 FT. OR LESS, *EXPOSURE "B"* (WHICH IS INLAND OF A LINE THAT IS 1500 FT. FROM THE COAST), AND *WALL ZONE "5"* (INSTALLED NEAR THE CORNER OF THE BUILDING).

PER *ASTM E1300*, THE CORRECT GLASS THICKNESS, BASED ON THE *NEGATIVE* DESIGN PRESSURE (DP) LISTED BELOW, HAS BEEN INSTALLED IN THIS UNIT. THE GLASS THICKNESS IS BASED ON ITS' WIDTH, HEIGHT, AND ASPECT RATIO.

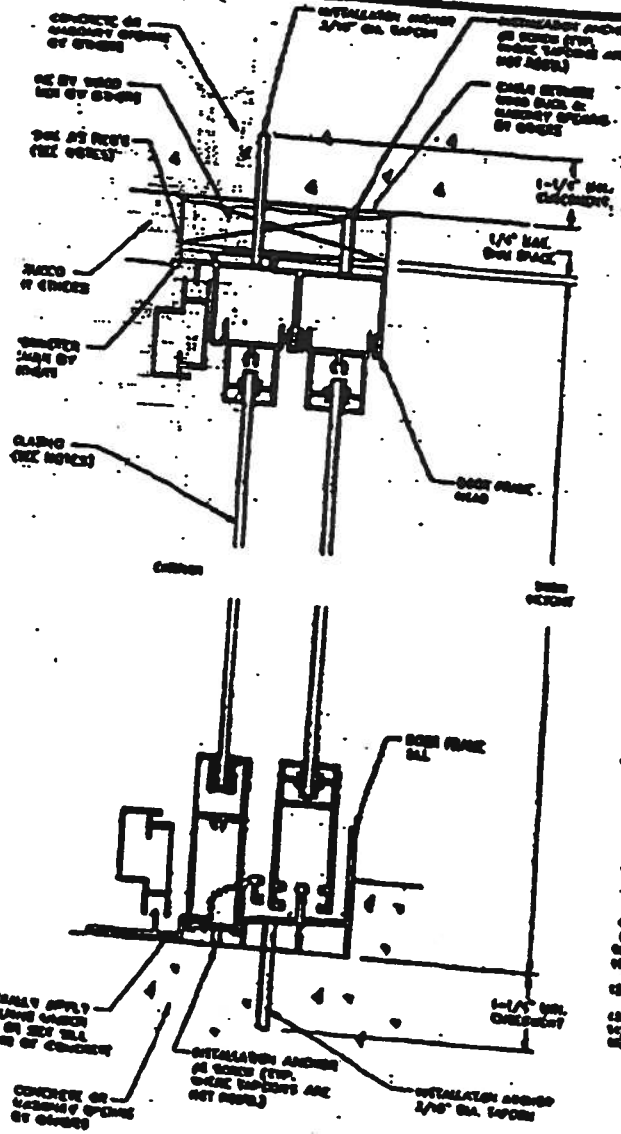
Series 470HP SLIDING GLASS DOOR – all 6'- 8" High Panels

- 2'- 6" WIDE DP +40.0 / -55.4
- 3'- 0" WIDE DP +40.0 / -48.5
- 4'- 0" WIDE DP +40.0 / -40.3

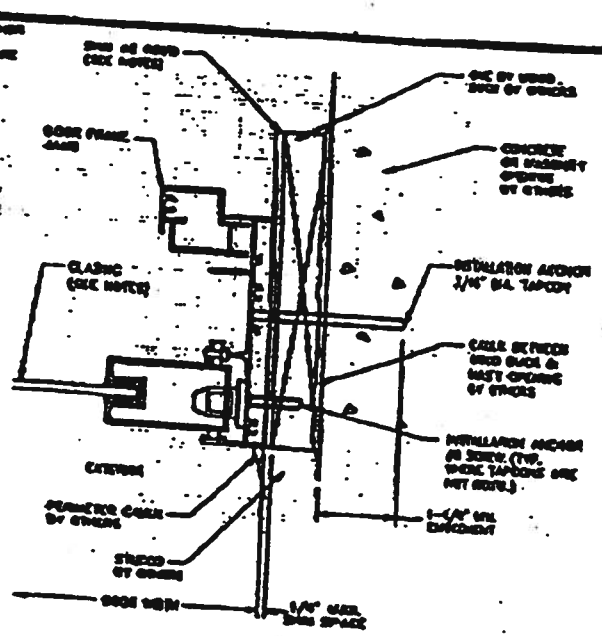
THIS PRODUCT MEETS THE REQUIREMENTS FOR STRUCTURAL LOADS, WATER AND AIR INFILTRATION PER ATTACHED *AAMA* PERFORMANCE LABEL. BE ADVISED THAT IF LOADS ARE PLACED UP TO OR EXCEEDING THE TESTED LEVELS, THIS PRODUCT MAY BE ALTERED IN SUCH A WAY THAT FUTURE PERFORMANCE WILL BE REDUCED.

* COMPLIANCE MUST INCLUDE INSTALLATION ACCORDING TO MANUFACTURER'S INSTRUCTIONS AND FLORIDA CODE REQUIREMENTS.

MIP-686

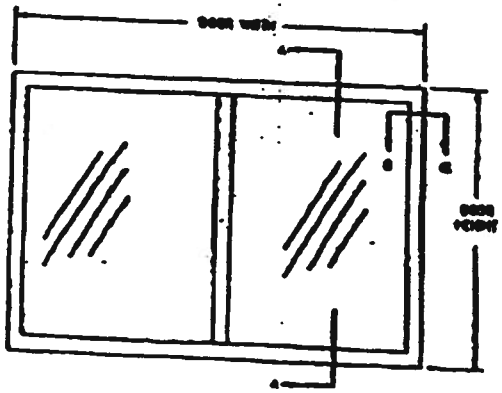


SECTION A–A



SECTION B–B

TAPCON INSTALLATION CHART					
CALL SIZE	DOOR SIZE				



EXTERIOR ELEVATION

MI HOME PRODUCTS
GRATZ, PA.

SERIES 470 SLIDING GLASS DOOR
INSTALLATION WITH TAPCONS

MANUEL MARTINEZ	98	1/23/02
CIVIL	N.T.S.	
47182		1 OF 1

TOTAL P.04

DOCUMENT CONTROL ADDENDUM #01-40351.00

Current Issue Date: 02/15/02

Report No.: 01-40351.01

Requested by: William Emley, MI Home Products, Inc.
Purpose: AAMA/NWWDA 101/I.S.2-97 testing of Series/Model 744 aluminum single hung window with flange.
Issued Date: 12/28/01
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories, Inc.

Report No.: 01-40351.02

Requested by: William Emley, MI Home Products, Inc.
Purpose: Change of glass type.
Issued Date: 12/28/01
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories.

Report No.: 01-40351.03

Requested by: William Emley, MI Home Products, Inc.
Purpose: AAMA/NWWDA 101/I.S.2-97 testing of Series/Model 740/744 aluminum single hung window with nail fin.
Issued Date: 02/15/02
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories, Inc.

Allen N. Reeves
15 FEBRUARY 2002

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.1.8	Forced Entry Resistance per ASTM F 588-97		
	Type: A Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 thru A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.4.1	Uniform Load Deflection per ASTM E 330 (Measurements reported were taken on the meting rail) (Loads were held for 52 seconds)		
	@ 45.0 psf (positive)	0.91"*	0.29" max.
	@ 45.0 psf (negative)	0.97"*	0.29" max.
4.4.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads held for 10 seconds)		
	@ 67.5 psf (positive)	0.14"	0.20" max.
	@ 67.5 psf (negative)	0.19"	0.20" max.
4.4.2	@ 70.8 psf (negative)	0.20"	0.20" max.

** Exceeds L/175 for deflection, but meets all other test requirements.*

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

MAH:baw
01-40351.03

Allen N. Reeves, P.E.
Director - Engineering Services
15 FEBRUARY 2002

ALLEN N. REEVES
CERTIFICATE
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Drainage: Sloped sill.

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into the #2 2 x 8 Spruce-Pine-Fir wood buck with 1" galvanized roofing nails through the nail fin every 8" on center. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	24 lbs	30 lbs max.
2.1.2	Air Infiltration (ASTM E 283) @ 1.57 psf (25 mph)	0.10 cfm/ft^2	0.30 cfm/ft^2 max.

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

2.1.3	Water Resistance (ASTM E 547-96) (with and without screen) WTP = 6.75 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads were held for 52 seconds)		
	@ 15.0 psf (positive)	0.86"*	0.29" max.
	@ 15.0 psf (negative)	0.81"*	0.29" max.
2.1.4.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 22.5 psf (positive)	0.01"	0.20" max.
	@ 22.5 psf (negative)	<0.01"	0.20" max.
2.2.1.6.2	Deglazing Test per ASTM E 987		
	In operating direction at 70 lbs		
	Top rail	0.06"/12%	0.50"/100%
	Bottom rail	0.06"/12%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.03"/6%	0.50"/100%
	Right stile	0.03"/6%	0.50"/100%

*Note: * Exceeds L/175 for deflection, but meets all other test requirements.*

Allen N. Reeves
15 FEBRUARY 2002

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.330" high by 0.187" backed polypile with center fin	1 Row	Fixed meeting rail interlock
0.170" high by 0.187" backed polypile with center fin	1 Row	Fixed lite, stiles and top rail
3/8" diameter hollow bulb gasket	1 Row	Bottom rail
0.310" high by 0.187" backed polypile with center fin	1 Row	Active sash stiles
0.150" high by 0.187" wide polypile	1 Row	Active sash stiles

Frame Construction: All frame members were constructed of extruded aluminum with coped, butted and sealed corners fastened with two screws each. Fixed meeting rail was secured utilizing one screw in each end directly through exterior face into jamb. Silicone was utilized around exterior meeting rail/jamb joinery.

Sash Construction: All sash members were constructed of extruded aluminum with coped and butted corners fastened with one screw each.

Screen Construction: The screen frame was constructed from roll-formed aluminum members with plastic keyed corners. The screening consisted of a fiberglass mesh and was secured with a flexible vinyl spline.

Hardware:

Description	Quantity	Location
Plastic tilt latch	2	One each end of the interior Meeting rail
Metal sweep lock	2	13" from meeting rail ends
Balance assembly	2	One per jamb
Screen tension spring	2	One per end of screen stile
Tilt pin	2	One each end of bottom rail

Allen N. Reeves
15 FEBRUARY 2002

ALLEN N. REEVES
CERTIFICATE NO. 19336
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-40351.03
Test Dates: 10/22/01
And: 10/23/01
Report Date: 02/15/02
Expiration Date: 10/23/05

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to witness performance testing on a Series/Model 740/744, aluminum single hung window at MI Home Products, Inc.'s test facility in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-R45 52 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 740/744

Type: Aluminum Single Hung Window With Nail Fin

Overall Size: 4' 4-1/8" wide by 5' 11-5/8" high

Active Sash Size: 4' 2-3/4" wide by 2' 11-5/8" high

Fixed Daylight Opening Size: 4' 1-1/8" wide by 2' 9" high

Screen Size: 4' 1-7/8" wide by 2' 11-5/16" high

Finish: All aluminum was polished.

Glazing Details: The active sash and fixed lite were glazed with one sheet of 1/8" thick clear tempered glass. Each sash was channel glazed using a flexible vinyl gasket.

ALLEN M. REEVES ★ CERTIFICATE NO. 19355 ★ STATE OF FLORIDA ★ PROFESSIONAL ENGINEER ★

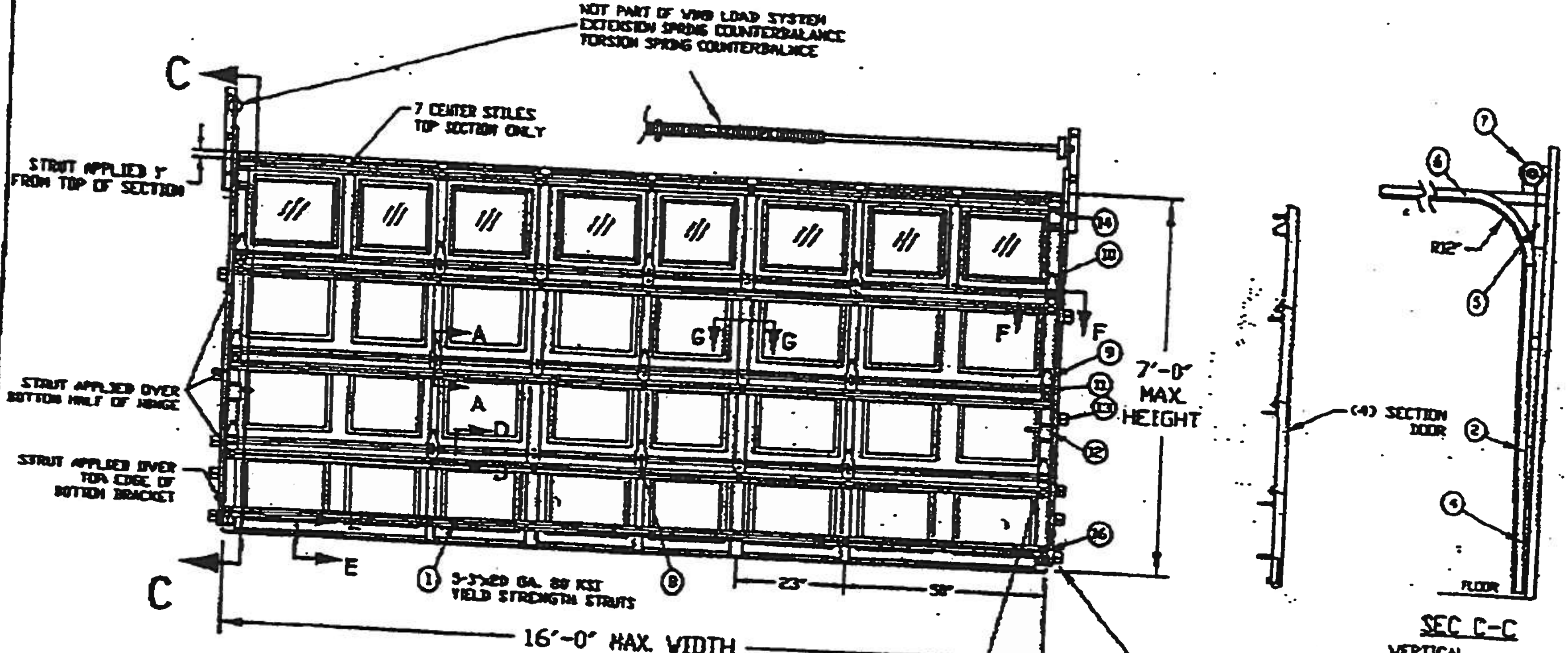
Allen M. Reeves

130 Derry Court
York, PA 17402-9405
Phone: 717.764.7700
Fax: 717.764.4129
www.testati.com

REV.	DATE	BY	DESCRIPTION
A–A	01-10-00	DW	SEC C.C.A. 631

NOTES

1. TESTED TO POSITIVE AND NEGATIVE 20 PSF DESIGN AND POSITIVE AND NEGATIVE 30 PSF TEST PRESSURES PER ASTM E-330
2. MAXIMUM SECTION HEIGHT= 21"
3. SECTION HEIGHTS OF 21.00" AND 19.50" ARE AVAILABLE AND MAY BE USED IN ANY COMBINATION TO ACHIEVE VARIOUS DOOR HEIGHTS.
4. WINDOWS MAY BE INSTALLED IN THE TOP SECTION (AS TESTED WITH 1/8" SSB GLASS OR EQUIVALENT) OR IN THE SECTION IMMEDIATELY BELOW THE TOP SECTION.
5. MINIMUM LENGTH OF ROLLER STEM IS 5¼" (7" AS TESTED)
6. THE STRUT PLACEMENT ON DOOR MUST BE CONSISTENT WITH THE DOOR SHOWN.
7. STRUTS SECURED AT ALL LOCATIONS WITH TEK SCREWS.
8. QUANTITY OF SIDE LOCKS CAN BE 0,1, OR 2 AS TESTED
9. △ DROP IN TYPE OF INSULATION IS OPTIONAL.



INSIDE ELEVATION

ALL ROLLER CARRIERS AND HINGES ARE 14 GA.

12 GA. JAMB BRACKETS, MAXIMUM SPACING = 19-1/2" WITH LOWEST BRACKET APPROX. 3" FROM FLOOR, 2ND BRACKET NEAR THE HORIZONTAL ℄ OF THE BOTTOM SECTION, AND 3RD BRACKET NEAR THE TOP OF THE BOTTOM SECTION

The seal on this drawing only certifies that the product(s) illustrated and described herein represent the configuration(s) of dimensions and installation(s) of the door as tested.

NORTH CAROLINA PROFESSIONAL ENGINEER SEAL PE No. 024280 ... R. ... YVAN

LISTED SBCCI PST & ESI REPORT No. 2202

TEST REPORTS ON FILE VIDEO 10/19/00 400293

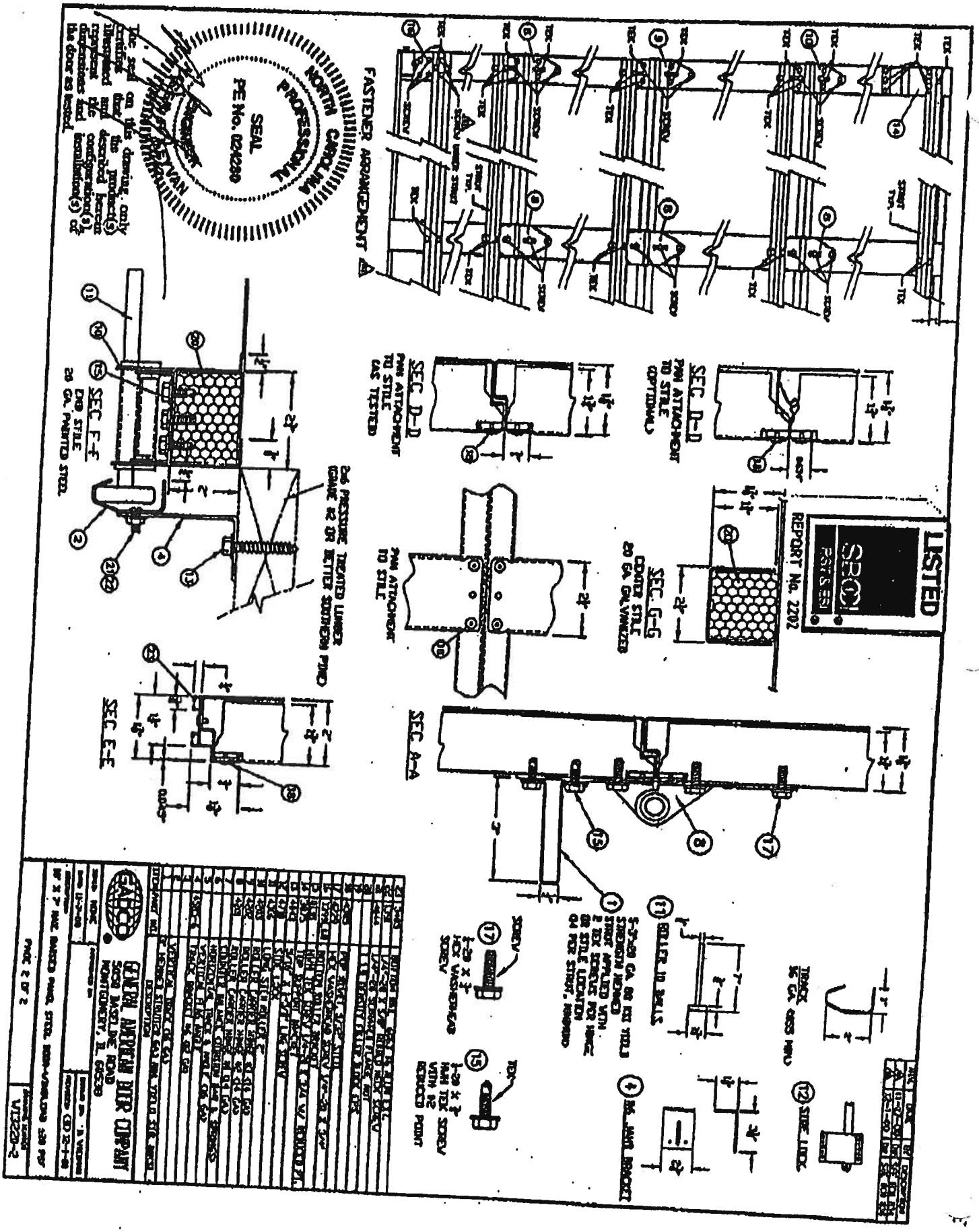
DESIGN LOAD +20.0 PSF & -20.0 PSF
TEST LOAD +30.0 PSF & -30.0 PSF

GADCO DOORS
SERIES 7400, EXTERIOR STEEL =.017 MIN (AS TESTED)
SERIES 7825, EXTERIOR STEEL =.019" MIN △
SERIES 7324, EXTERIOR STEEL =.024" MIN △
(TESTED WITH WINDOWS

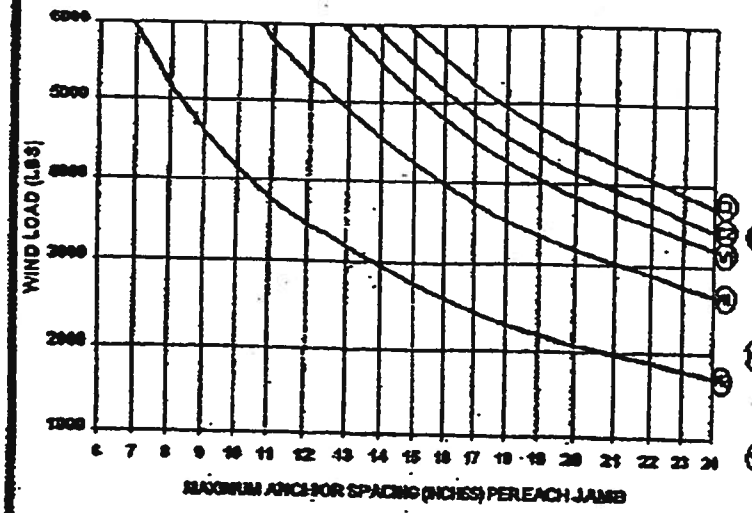
MAXIMUM DOOR WIDTH	MAXIMUM DOOR HEIGHT	TYPICAL CTR. STILE SPACING	STRUTS 80 KSI SIZE	STRUTS 80 KSI QTY.	VERTICAL TRACK
16'	7'	23"	3"	5	2 IN.

GADCO GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538

SCALE: NONE | APPROVED BY: | DRAWN BY: ... | DATE: 10-20-00 | REVISED: (A) 11-10-00
DESCRIPTION: 16' x 7' MAX RAISED PANEL STEEL DOOR -W/DLOAD ±20 PSF
PART NUMBER: | PAGE 1 OF 2 | DRAWING NUMBER: V13220-1


FASTENER ARRANGEMENT A
SEC F-F
SEC D-D
SEC G-G
SEC A-A
SEC E-E
LISTED
REPORT No. 2202
SEAL
PE No. 024289
NORTH CAROLINA
PROFESSIONAL
CENTRAL AMERICAN DOOR COMPANY

WIND LOAD vs ANCHOR SPACING



(1) CONCRETE BACKUP
HILTI KWIK BOLT II EXPANSION ANCHOR
3/8" DIA.
1-5/8" EMBEDMENT

(2) CONCRETE BACKUP
RAWL LDK/BOLT SLEEVE ANCHOR
3/8" DIA.
1-3/8" EMBEDMENT

(3) MASONRY/WOOD BACKUP
RAWL LDK/BOLT SLEEVE ANCHOR
3/8" DIA.
1-3/8" EMBEDMENT

(4) MASONRY/WOOD BACKUP
ITW BUILDEX TAPCON MASONRY ANCHOR
1/4" DIA.
1-3/4" EMBEDMENT

(5) WOOD STUD BACKUP
LAG SCREWS
5/16" DIA.
1-1/2" EMBEDMENT

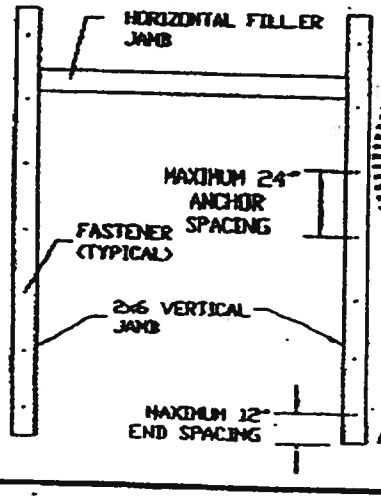
DESIGN LOAD (LBS)/FT² X GARAGE DOOR AREA(WIDTH-FT X HEIGHT-FT) = WIND LOAD(LBS)

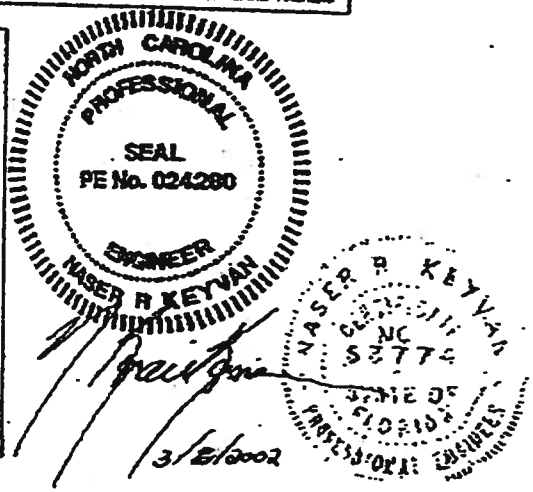
EXAMPLE

30 LBS/FT² X 16 FT WIDE X 8 FT HIGH = 3840 LBS

(1) USE 22" SPACING
(2) USE 21" SPACING
(3) USE 19" SPACING
(4) USE 16" SPACING
(5) USE 10" SPACING

SEE NOTE 13 FOR ADDITIONAL REQUIRED 2X6 WOOD JAMB ANCHORS





2x6 JAMB TO SUPPORTING STRUCTURE ATTACHMENT

2x6 PRESSURE TREATED (GRADE #2 OR BETTER SOUTHERN PINE) WOOD JAMB SHALL BE ANCHORED TO BUILDING WOOD FRAME, GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS.

NOTES:

1) ALL DOOR OPENING SURROUNDING STRUCTURE TO BE DESIGNED BY REGISTERED ENGINEER OR ARCHITECT WITH DUE CONSIDERATION GIVEN TO INSTALLATIONS USING CENTER "HURRICANE" POSTS.

2) ALL DOOR OPENING STRUCTURE AND FASTENERS TO COMPLY WITH ALL APPLICABLE CODES INCLUDING SBCCI "STANDARD FOR HURRICANE RESISTANT RESIDENTIAL CONSTRUCTION" SSTD 10," CURRENT EDITION.

3) ALL FASTENERS TO BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS, INSTRUCTIONS AND RECOMMENDATIONS.

4) WOOD FRAME BUILDINGS: STUDS AT EACH SIDE OF DOOR OPENING SHALL BE PROPERLY DESIGNED, CONNECTED, ANCHORED AND SHALL CONSIST OF A MINIMUM OF THREE (3) LAMINATIONS OF 2X6 PRESSURE TREATED SOUTHERN PINE #2 GRADE OR BETTER) WALL STUDS CONTINUOUS FROM FOOTING TO DOUBLE TOP PLATE.

5) REINFORCED CMU OR CONCRETE: 2X6 WOOD JAMB SHALL BE ANCHORED TO SOLIDLY GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS. ANCHOR SPACING AND EMBEDMENT IS BASED ON CONCRETE MASONRY UNITS COMPLYING WITH ASTM C90 WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 2150 PSI, GROUT WITH A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI, & REINFORCED CONCRETE COLUMNS WITH A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI.

6) EMBEDMENTS LISTED ARE THE MINIMUM ALLOWABLE EMBEDMENTS.

7) ANCHORS FOR CONCRETE AND CONCRETE MASONRY UNITS (CMU) SHALL HAVE A MINIMUM 3" EDGE DISTANCE FROM ALL EDGES OF CONCRETE OR CONCRETE MASONRY UNITS. ANCHORS FOR CONCRETE AND CMU SHALL HAVE A MINIMUM SPACING OF 3-3/4".

8) LAG SCREWS SHALL BE CENTERED IN ONE OF THE 1-1/2" DIMENSION FACES OF THE TRIPLE 2X6 WALL STUDS.

9) WASHERS ARE REQUIRED ON ALL FASTENERS.

10) THE WIND LOAD VS. ANCHOR SPACING CHART IS FOR A MAXIMUM DOOR SIZE OF 18' X 8' AT A MAXIMUM 42 PSF DESIGN WIND LOAD.

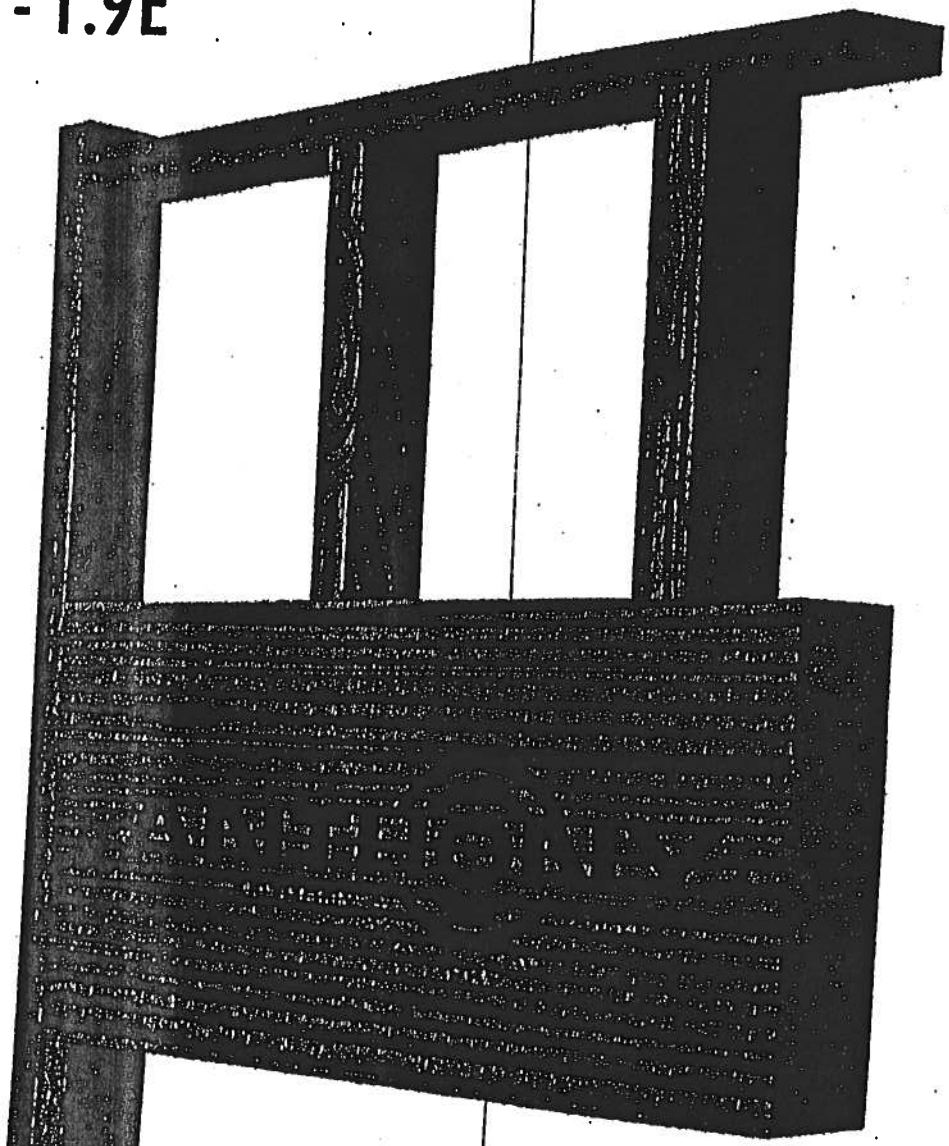
11) FOR THE UPPER THREE INDIVIDUAL STEEL JAMB BRACKETS, BRACKETS SHALL BE CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS. IF THE STEEL JAMB BRACKET IS NOT CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS, ADD AN ADDITIONAL 2X6 WOOD JAMB ANCHOR NEAR THAT STEEL BRACKET TO INSURE THAT THE LOAD FROM THE STEEL BRACKET IS EQUALLY TRANSFERRED TO TWO WOOD JAMB ANCHORS.

GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538

SCALE: NONE	APPROVED BY:	ISSUED BY: BV
DATE: 8-30-99		REVISED:
DESCRIPTION: JAMB TO STRUCTURE ATTACHMENT FOR WIND LOADED GARAGE DOORS		
	DRAWING NUMBER: A10560	DRAWING NUMBER:

Anthony POWER HEADER®

2600F_b - 1.9E



Anthony POWER HEADER® Advantages

- Less Expensive than LVL or PSL
- Lighter than Steel, LVL or PSL
- Pre-Cut Lengths
- Renewable Resource
- Cambered or Non-cambered
- 3-1/2" Width to Match Framing
- One Piece - No Nail Laminating
- Lifetime Warranty

Garage Header Sizing Tables

ANTHONY®
ANTHONY FOREST PRODUCTS CO.

Anthony POWER HEADER®

26F_b - 1.9E

3-1/2" WIDTH GARAGE HEADER APPLICATION - SINGLE STORY

HEADER SUPPORTING: 1/2 ROOF SPAN

9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"
8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	14	15-3/8	8-3/8	14	16-3/4
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8-3/8	14	16-3/4	8-3/8	15-3/8		9-3/4	15-3/8		9-3/4			9-3/4			11-1/4		

9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"
8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14
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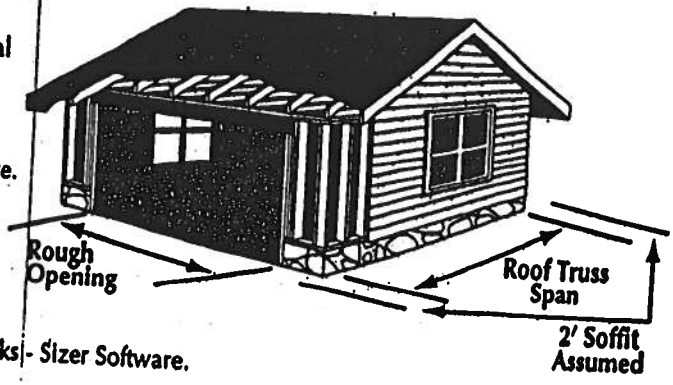
8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14
8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8
8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	14	15-3/8
8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8	8-3/8	14	15-3/8
8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8	8-3/8	14	15-3/8	8-3/8	14	
8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8	

NOTES:

1. Table assumes a simple span header supporting a uniform load transferred from 1/2 the roof span plus a 2' soffit.
2. Roof live and dead loads shown are applied vertically to the horizontal projection. No reductions in roof live loads or snow loads were considered. The header weight is accounted for in the table.
3. Deflection is limited to L/240 for live load and L/180 for total load.
4. Headers are assumed to have continuous lateral support along top edge.
5. Bearing length based on full width bearing is indicated as follows:
   Non-shaded sizes require two trimmers (3" bearing).
   Shaded sizes require three trimmers (4.5" bearing).
   Shaded & outlined sizes require four trimmers (6" bearing).
6. ** Applications where load carrying capacity of 16-3/4" depth has been exceeded. See AFP 30F_b POWER BEAM® literature or AFP's WoodWorks - Sizer Software.



Anthony POWER HEADER®

26F_b - 1.9E

3-1/2" WIDTH GARAGE HEADER PLF CAPACITY

844	896		1216		1573						
161	207	254	330	390	510	552	669	752	824		
114	145	180	231	277	359	391	510	534	653	707	789

844	975		1322								
161	207	254	330	390	510	552	724	752	897		
114	145	180	231	277	359	391	510	534	699	693	

562	778	888	1056	1363	1367		1582				
107	153	169	245	260	380	368	540	501	715	664	864	840
76	107	120	171	185	267	261	380	356	521	471	684	609	813

NOTES:

1. Values shown are the maximum uniform loads in pounds per lineal foot (PLF) that can be applied to the header. Header weight has been subtracted from the allowable total load.
2. Tables are based on simple span uniform load conditions using a design span equal to the center-to-center of bearing. Non-shaded areas are based on 3" of bearing at each support, shaded areas on 4.5" of bearing, and shaded & outlined areas on 6" of bearing at supports.
3. Headers are assumed to be loaded on the top edge with continuous lateral support along compression edge.
4. When no live load is listed, total load controls.
5. Deflection limits are listed within the PLF table heading.

GARAGE HEADER SIZING USING PLF TABLES:

To size a garage header supporting roof only, determine the total load & live load in pounds per lineal foot (PLF). Check the appropriate PLF table for a header supporting roof loads only (125% Non-Snow vs. 115% Snow) and select a member with a total load and live load capacity which meets or exceeds the design load for the rough opening size. For a garage header supporting roof, wall, and floor framing, determine the total load and live load in pounds per lineal foot (PLF). Select a header size from the roof, wall, and floor table (100% load duration) which has a total load and live load capacity equal to or greater than the design load for the appropriate rough opening.

Anthony Power Header®

$26F_b$ - 1.9E

ENGINEERED WOOD SECTION PROPERTIES AND LOAD CAPACITIES

ALLOWABLE DESIGN STRESSES (PSI):

FLEXURAL STRESS (F_b) =	2600
COMPRESSION PERP. TO GRAIN ($F_{c\perp}$) =	740
HORIZONTAL SHEAR (F_v) =	225
MODULUS OF ELASTICITY (MOE) =	1.9×10^6

	7.7	9.0	10.4	11.7	12.9	14.2	15.5
	326	514	789	1115	1521	2014	2604
	8865	12015	15996	20145	24772	29877	35460
	3908	4550	5250	5892	6533	7175	7817

NOTES:

1. Beam weights are based on 38 pcf.
2. Moment capacities are based on a span of 21 feet and must be modified for other spans.
3. Flexural Stress, F_b, shall be modified by the Volume Factor, C_v, as outlined in AITC 117 - Design 1993 and the NDS for Wood Construction 1997.
4. Allowable design properties and load capacities are based on a load duration of 100 percent and dry use conditions.
5. The AITC NER 466 was used in calculating the above allowable design stresses for Power Header®.

GARAGE HEADER COMPARISONS

810 / 540	3-1/2" x 8-3/8"	3-1/2" x 9-5/8"	3-1/2" x 9"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"**
990 / 720	3-1/2" x 9-3/4"	3-1/2" x 9-5/8"	3-1/2" x 10-1/2"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"**
640 / 400	3-1/2" x 12-5/8"	3-1/2" x 13-3/4"	3-1/2" x 13-1/2"	3-1/2" x 14"	3-1/2" x 14"*
765 / 510	3-1/2" x 14"	3-1/2" x 15-1/8"	3-1/2" x 15"	3-1/2" x 14"	3-1/2" x 16"*
750 / 480	3-1/2" x 15-3/8"	3-1/2" x 16-1/2"	3-1/2" x 16-1/2"	3-1/2" x 16"	3-1/2" x 18"*
900 / 600	3-1/2" x 16-3/4"	3-1/2" x 17-7/8"	3-1/2" x 18"	3-1/2" x 16"	-----

For more information on Power Header®, or other laminated structural products from Anthony Forest Products Company please call 1-800-221-2326 or FAX at 870-862-6502.

Distributed by:

Anthony Forest Products Company
Post Office Box 1877 • El Dorado, Arkansas 71731
Internet address: http:// www.anthonyforest.com
e-mail: info@anthonyforest.com

ELK

PRESTIQUE®
HIGH DEFINITION®

RAISED PROFILE™

Prestique Plus *High Definition* and Prestique Gallery Collection™

Product size . . 13¼"x 39¾"
Exposure . . . . . 5⅝"
Pieces/Bundle 16
Bundles/Square . . . 4/98.5 sq.ft.
Squares/Pallet 11

50-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

Prestique I *High Definition*

Product size . . . . . 13¼"x 39¾"
Exposure . . . . . 5⅝"
Pieces/Bundle 16
Bundles/Square . . . 4/98.5 sq.ft.
Squares/Pallet 14

40-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

Prestique *High Definition*

Product size . . . . . 13¼"x 38¾"
Exposure . . . . . 5⅝"
Pieces/Bundle 22
Bundles/Square . . . 3/100 sq.ft.
Squares/Pallet 16

30-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

Raised Profile

Product size . . 13¼"x 38¾"
Exposure . . . . . 5⅝"
Pieces/Bundle 22
Bundles/Square . . . 3/100 sq.ft.
Squares/Pallet 16

30-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

HIP AND RIDGE SHINGLES

Seal-A-Ridge® w/*FLX*™

Size: 12"x 12"
Exposure: 6⅛"
Pieces/Bundle: 45
Coverage: 4 Bundles = 100 linear feet

Elk Starter Strip

52 Bundles/Pallet
18 Pallets/Truck
936 Bundles/Truck
19 Pieces/Bundle
1 Bundle = 120.33 linear feet

Available Colors: Antique Slate, Weatheredwood, Shakewood, Sablewood, Hickory, Barkwood**, Forest Green, Wedgewood**, Birchwood**, Sandalwood. Gallery Collection: Balsam Forest™, Weathered Sage™, Sienna Sunset™.

All Prestique, Raised Profile and Seal-A-Ridge roofing products contain Elk WindGuard® sealant. WindGuard activates with the sun's heat, bonding shingles into a wind and weather resistant cover that resists blow-offs and leaks.

Check for availability with built-in StainGuard® treatment to inhibit the discoloration of roofing granules caused by the growth of certain types of algae. Not available in Sablewood.

All Prestique and Raised Profile shingles meet UL® Wind Resistant (UL 997) and Class "A" Fire Ratings (UL 790); and ASTM Specifications D 3018, Type-I; D 3161, Type-I; E 108 and the requirements of ASTM D 3462.

All Prestique and Raised Profile shingles meet the latest Metro Dade building code requirements.

*See actual limited warranty for conditions and limitations.
**Check for product availability.

SPECIFICATIONS

SCOPE: Work includes furnishing all labor, materials and equipment necessary to complete installation of (name) shingles specified herein. Color shall be (name of color).

MATERIALS: Underlayment for standard roof slopes, 4" per foot (101.6/304.8mm) or greater: apply non-perforated No. 15 or 30 asphalt-saturated felt underlayment. Eaves...

warranties are contingent upon the correct installation as shown on the instructions. These instructions are the

Residential System Sizing Calculation

Summary

EWPL INC

Lake City, FL

Project Title:
MADISON MOD - LOT 26 WISE

Code Only
Professional Version
Climate: North

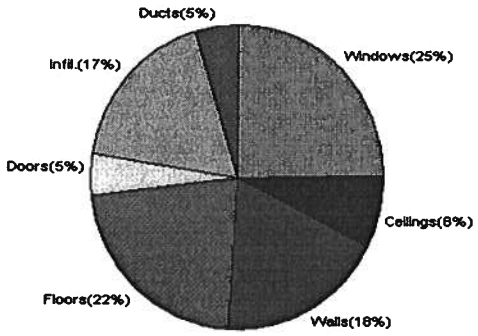
11/14/2006

Location for weather data: Gainesville - Defaults: Latitude(29) Temp Range(M)					
Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(51gr.)					
Winter design temperature	31	F	Summer design temperature	93	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	39	F	Summer temperature difference	18	F
Total heating load calculation	**31051**	**Btuh**	**Total cooling load calculation**	**30270**	**Btuh**
Submitted heating capacity	36000	Btuh	Submitted cooling capacity	36000	Btuh
Submitted as % of calculated	115.9	%	Submitted as % of calculated	118.9	%

WINTER CALCULATIONS

Winter Heating Load (for 1888 sqft)

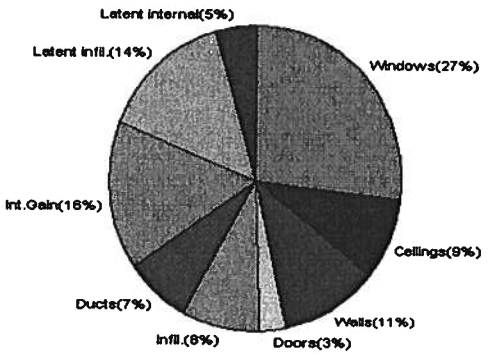
Load component			Load	
Window total	272	sqft	7698	Btuh
Wall total	1930	sqft	5686	Btuh
Door total	88	sqft	1404	Btuh
Ceiling total	1888	sqft	2454	Btuh
Floor total	219	ft	6920	Btuh
Infiltration	126	cfm	5410	Btuh
Subtotal			**29573**	**Btuh**
Duct loss			1479	Btuh
TOTAL HEAT LOSS			**31051**	**Btuh**



SUMMER CALCULATIONS

Summer Cooling Load (for 1888 sqft)

Load component			Load	
Window total	272	sqft	8212	Btuh
Wall total	1930	sqft	3220	Btuh
Door total	88	sqft	878	Btuh
Ceiling total	1888	sqft	2681	Btuh
Floor total			0	Btuh
Infiltration	126	cfm	2497	Btuh
Internal gain			4800	Btuh
Subtotal(sensible)			**22287**	**Btuh**
Duct gain			2229	Btuh
Total sensible gain			**24516**	**Btuh**
Latent gain(infiltration)			4374	Btuh
Latent gain(internal)			1380	Btuh
Total latent gain			**5754**	**Btuh**
TOTAL HEAT GAIN			**30270**	**Btuh**



EnergyGauge® System Sizing based on ACCA Manual J.
PREPARED BY:
DATE: 11-14-06

System Sizing Calculations - Winter

Residential Load - Component Details

EWPL INC

Project Title:
MADISON MOD - LOT 26 WISE

Code Only
Professional Version
Climate: North

Lake City, FL

Reference City: Gainesville (Defaults) Winter Temperature Difference: 39.0 F

11/14/2006

Window	Panes/SHGC/Frame/U	Orientation	Area X	HTM=	Load
1	2, Clear, Metal, DEF	N	53.7	28.3	1519 Btuh
2	2, Clear, Metal, DEF	N	13.3	28.3	377 Btuh
3	2, Clear, Metal, DEF	N	9.0	28.3	255 Btuh
4	2, Clear, Metal, DEF	N	30.0	28.3	849 Btuh
5	2, Clear, Metal, DEF	W	16.0	28.3	453 Btuh
6	2, Clear, Metal, DEF	E	4.0	28.3	113 Btuh
7	2, Clear, Metal, DEF	S	60.0	28.3	1698 Btuh
8	2, Clear, Metal, DEF	S	24.0	28.3	679 Btuh
9	2, Clear, Metal, DEF	W	20.0	28.3	566 Btuh
10	2, Clear, Metal, DEF	S	42.0	28.3	1189 Btuh
	Window Total		272		7698 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Adjacent	13.0	198	1.6	317 Btuh
2	Frame - Exterior	13.0	1732	3.1	5369 Btuh
	Wall Total		1930		5686 Btuh
Doors	Type		Area X	HTM=	Load
1	Wood - Exter		20	17.9	359 Btuh
2	Wood - Exter		48	17.9	861 Btuh
3	Wood - Adjac		20	9.2	184 Btuh
	Door Total		88		1404Btuh
Ceilings	Type	R-Value	Area X	HTM=	Load
1	Under Attic	30.0	1888	1.3	2454 Btuh
	Ceiling Total		1888		2454Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab-On-Grade Edge Insul	0	219.0 ft(p)	31.6	6920 Btuh
	Floor Total		219		6920 Btuh
Infiltration	Type	ACH X	Building Volume	CFM=	Load
	Natural	0.40	18880(sqft)	126	5410 Btuh
	Mechanical			0	0 Btuh
	Infiltration Total			126	5410 Btuh

Totals for Heating	**Subtotal**	**29573 Btuh**
	Duct Loss(using duct multiplier of 0.05)	**1479 Btuh**
	Total Btuh Loss	**31051 Btuh**

Manual J Winter Calculations

Residential Load - Component Details (continued)

EWPL INC

Lake City, FL

Project Title:
MADISON MOD - LOT 26 WISE

Code Only
Professional Version
Climate: North

11/14/2006

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

System Sizing Calculations - Summer

Residential Load - Component Details

EWPL INC
Lake City, FL

Project Title:
MADISON MOD - LOT 26 WISE

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 18.0 F 11/14/2006

Window	Type		Overhang Len	Overhang Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	HTM Unshaded	Load
1	2, Clear, DEF, N, N	N	1.5	9	53.7	0.0	53.7	22	22	1181 Btuh
2	2, Clear, DEF, N, N	N	5	10	13.3	0.0	13.3	22	22	293 Btuh
3	2, Clear, DEF, N, N	N	5	4	9.0	0.0	9.0	22	22	198 Btuh
4	2, Clear, DEF, N, N	N	1.5	6	30.0	0.0	30.0	22	22	660 Btuh
5	2, Clear, DEF, N, N	W	1.5	5	16.0	1.0	15.0	22	72	1103 Btuh
6	2, Clear, DEF, N, N	E	1.5	1.5	4.0	3.0	1.0	22	72	139 Btuh
7	2, Clear, DEF, N, N	S	1.5	6	60.0	30.0	30.0	22	37	1770 Btuh
8	2, Clear, DEF, N, N	S	8	10	24.0	24.0	0.0	22	37	528 Btuh
9	2, Clear, DEF, N, N	W	1.5	6	20.0	0.5	19.5	22	72	1416 Btuh
10	2, Clear, DEF, N, N	S	1.5	7.5	42.0	42.0	0.0	22	37	924 Btuh
	Window Total				272					8212 Btuh

Walls	Type	R-Value	Area	HTM	Load
1	Frame - Adjacent	13.0	198.0	1.0	206 Btuh
2	Frame - Exterior	13.0	1732.0	1.7	3014 Btuh
	Wall Total		1930.0		3220 Btuh

Doors	Type	Area	HTM	Load
1	Wood - Exter	20.0	10.0	200 Btuh
2	Wood - Exter	48.0	10.0	479 Btuh
3	Wood - Adjac	20.0	10.0	200 Btuh
	Door Total	88.0		878 Btuh

Ceilings	Type/Color	R-Value	Area	HTM	Load
1	Under Attic/Dark	30.0	1888.0	1.4	2681 Btuh
	Ceiling Total		1888.0		2681 Btuh

Floors	Type	R-Value	Size	HTM	Load
1	Slab-On-Grade Edge Insulation	0.0	219.0 ft(p)	0.0	0 Btuh
	Floor Total		219.0		0 Btuh

Infiltration	Type	ACH	Volume	CFM=	Load
	Natural	0.40	18880	126.1	2497 Btuh
	Mechanical			0	0 Btuh
	Infiltration Total			126	2497 Btuh

Internal gain		Occupants	Btuh/occupant			Appliance	Load
		6	X	300	+	3000	4800 Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

EWPL INC

Lake City, FL

Project Title:
MADISON MOD - LOT 26 WISE

Code Only
Professional Version
Climate: North

11/14/2006

Totals for Cooling	**Subtotal**	**22287 Btuh**
	Duct gain(using duct multiplier of 0.10)	**2229 Btuh**
	Total sensible gain	**24516 Btuh**
	Latent infiltration gain (for 51 gr. humidity difference)	**4374 Btuh**
	Latent occupant gain (6 people @ 230 Btuh per person)	**1380 Btuh**
	Latent other gain	**0 Btuh**
	TOTAL GAIN	**30270 Btuh**

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds/Daperies(B) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(Ornt - compass orientation)

Project Information for:	L217897		
Builder:	HUGO ESCALANTE	Date:	11/16/2006
Lot:	LOT 15 FORT WHITE STATION	Start Number:	**1003**
Subdivision:	N/A	SEI Ref:	L217897
County or City:	COLUMBIA COUNTY		
Truss Page Count:	41		

Truss Design Load Information (UNO) Design Program: MiTek

Gravity		Wind		Building Code:	FBC2004
Roof (psf):	42	Wind Standard:	ASCE 7-02		
Floor (psf):	55	Wind Speed (mph):	120		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

ESCALANTE, HUGO CRC 1326967
Address: P.O. BOX 280
FORT WHITE, FL. 32038 Designer: 41

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549 Phone: 813-849-5769

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.
5. Where hangers are shown, Carried Member hanger capacity per Simpson C-2006 (SYP/Full Nailing Value) as an individual component. Building Designer shall verify the suitablity and use of Carrying Member hanger capacity.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	1116061003	11/16/2006	41	T32	1116061043	11/16/2006
2	CJ3	1116061004	11/16/2006				
3	CJ5	1116061005	11/16/2006				
4	EJ7	1116061006	11/16/2006				
5	EJ7A	1116061007	11/16/2006				
6	EJ7B	1116061008	11/16/2006				
7	EJ7C	1116061009	11/16/2006				
8	EJ7T	1116061010	11/16/2006				
9	HJ9	1116061011	11/16/2006				
10	T01	1116061012	11/16/2006				
11	T02	1116061013	11/16/2006				
12	T03	1116061014	11/16/2006				
13	T04	1116061015	11/16/2006				
14	T05	1116061016	11/16/2006				
15	T06	1116061017	11/16/2006				
16	T07	1116061018	11/16/2006				
17	T08	1116061019	11/16/2006				
18	T09	1116061020	11/16/2006				
19	T10	1116061021	11/16/2006				
20	T11	1116061022	11/16/2006				
21	T12	1116061023	11/16/2006				
22	T13	1116061024	11/16/2006				
23	T14	1116061025	11/16/2006				
24	T15	1116061026	11/16/2006				
25	T16	1116061027	11/16/2006				
26	T17	1116061028	11/16/2006				
27	T18	1116061029	11/16/2006				
28	T19	1116061030	11/16/2006				
29	T20	1116061031	11/16/2006				
30	T21	1116061032	11/16/2006				
31	T22	1116061033	11/16/2006				
32	T23	1116061034	11/16/2006				
33	T24	1116061035	11/16/2006				
34	T25	1116061036	11/16/2006				
35	T26	1116061037	11/16/2006				
36	T27	1116061038	11/16/2006				
37	T28	1116061039	11/16/2006				
38	T29	1116061040	11/16/2006				
39	T30	1116061041	11/16/2006				
40	T31	1116061042	11/16/2006				

NOV 16 2006

Dwg.#1116061002

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Licensee Details

Licensee Information

Name:	**ESCALANTE, HUGO (Primary Name)**
	EWPL INC (DBA Name)
Main Address:	**P.O. BOX 280** **FORT WHITE, Florida 32038**

License Information

License Type:	**Certified Residential Contractor**
Rank:	**Cert Residental**
License Number:	**CRC1326967**
Status:	**Current, Active**
Licensure Date:	**11/24/2003**
Expires:	**08/31/2006**

Special Qualifications	Effective Date
Qualified Business License Required	11/24/2003

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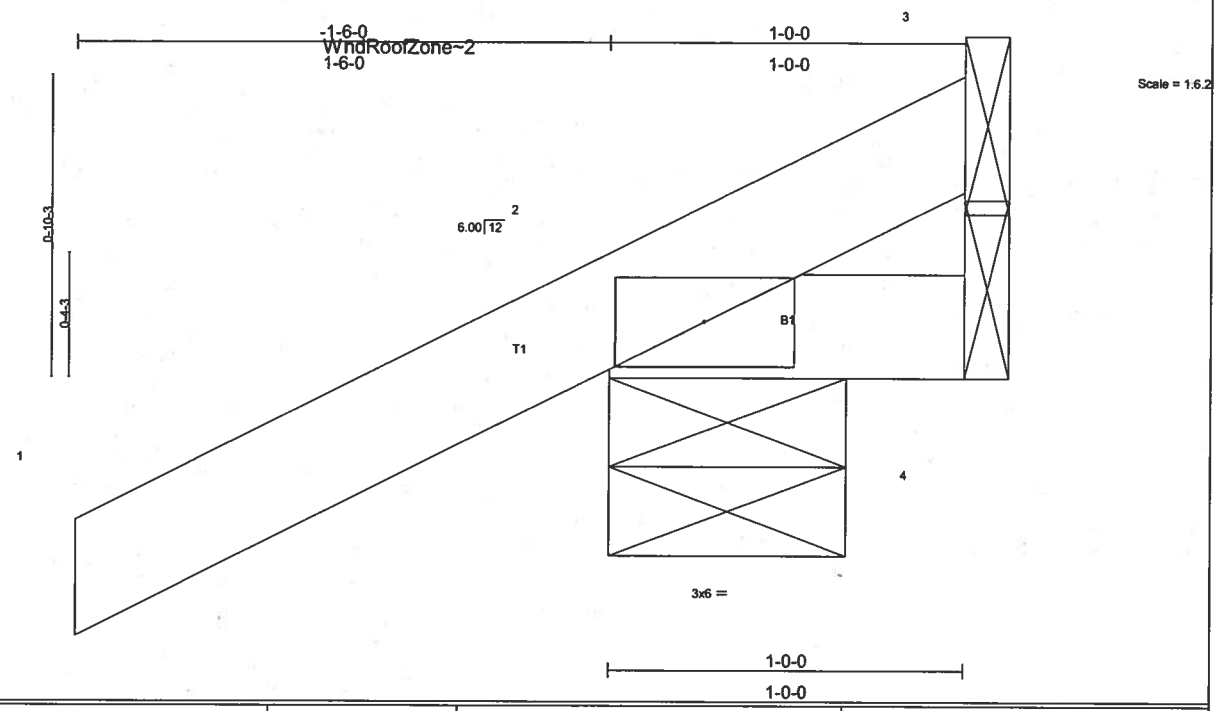
NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1116061003

Job	Truss	Truss Type	Qty	Ply	LOT 15--FORT WHITE STATION
L217897	CJ1	JACK	10	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Nov 15 15:25:25 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.19	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 6 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=195/0-8-0, 4=14/Mechanical, 3=-47/Mechanical
Max Horz 2=84(load case 5)
Max Uplift2=-230(load case 5), 3=-47(load case 1)
Max Grav 2=195(load case 1), 4=14(load case 1), 3=82(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-49/48
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.12

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 230 lb uplift at joint 2 and 47 lb uplift at joint 3.

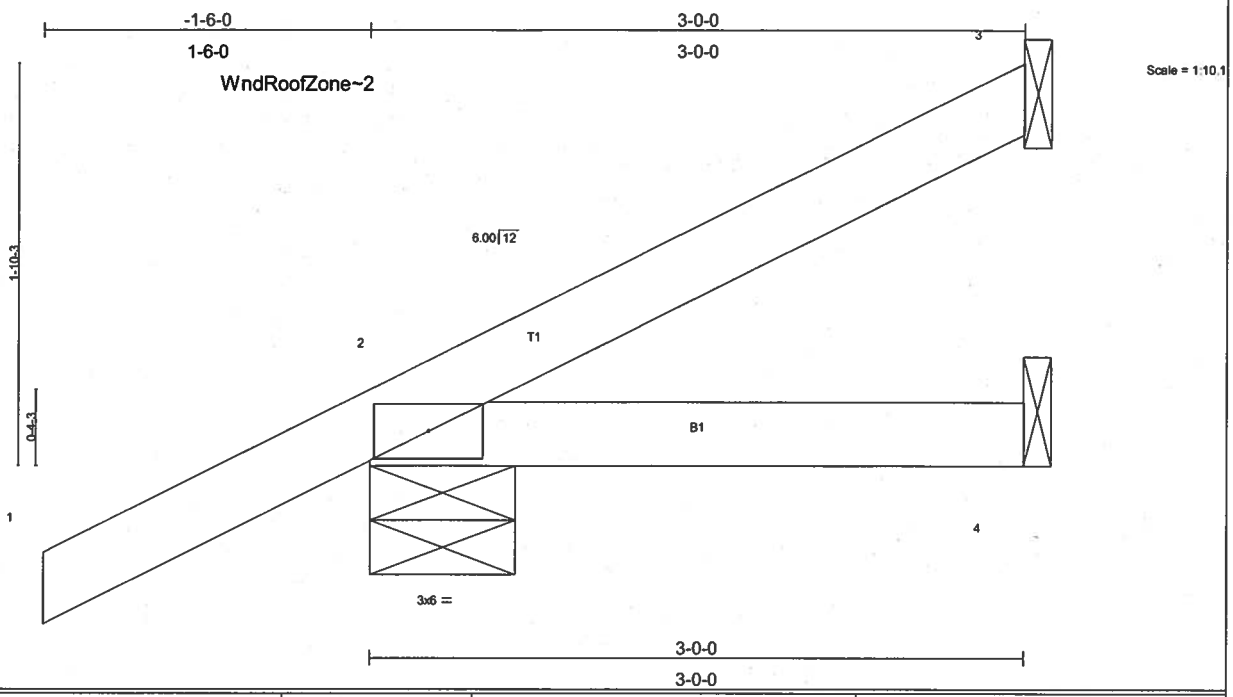
LOAD CASE(S) Standard

NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1116061004

Job	Truss	Truss Type	Qty	Ply	LOT 15--FORT WHITE STATION
L217897	CJ3	JACK	10	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Nov 15 15:25:37 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.25	Vert(LL)	0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=35/Mechanical, 2=243/0-8-0, 4=39/Mechanical
Max Horz 2=137(load case 5)
Max Uplift3=-38(load case 6), 2=-245(load case 5), 4=-31(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/36, 2-3=-53/11
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.13

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 3, 245 lb uplift at joint 2 and 31 lb uplift at joint 4.

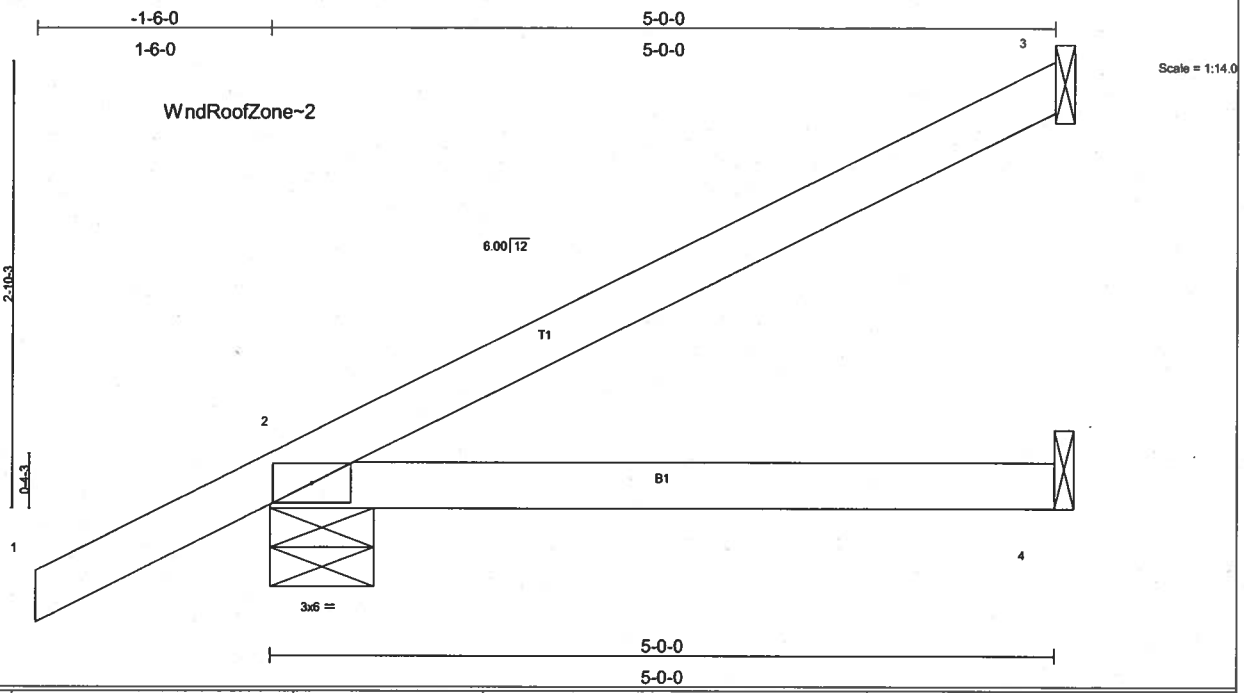
LOAD CASE(S) Standard

NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1116061005

Job	Truss	Truss Type	Qty	Ply	LOT 15--FORT WHITE STATION
L217897	CJ5	JACK	10	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Nov 15 15:25:51 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.26	Vert(LL)	0.09	2-4	>628	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.27	Vert(TL)	0.08	2-4	>714	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 18 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=105/Mechanical, 2=312/0-8-0, 4=69/Mechanical
Max Horz 2=192(load case 5)
Max Uplift3=-115(load case 5), 2=-279(load case 5), 4=-55(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/36, 2-3=-109/38
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.16

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 115 lb uplift at joint 3, 279 lb uplift at joint 2 and 55 lb uplift at joint 4.

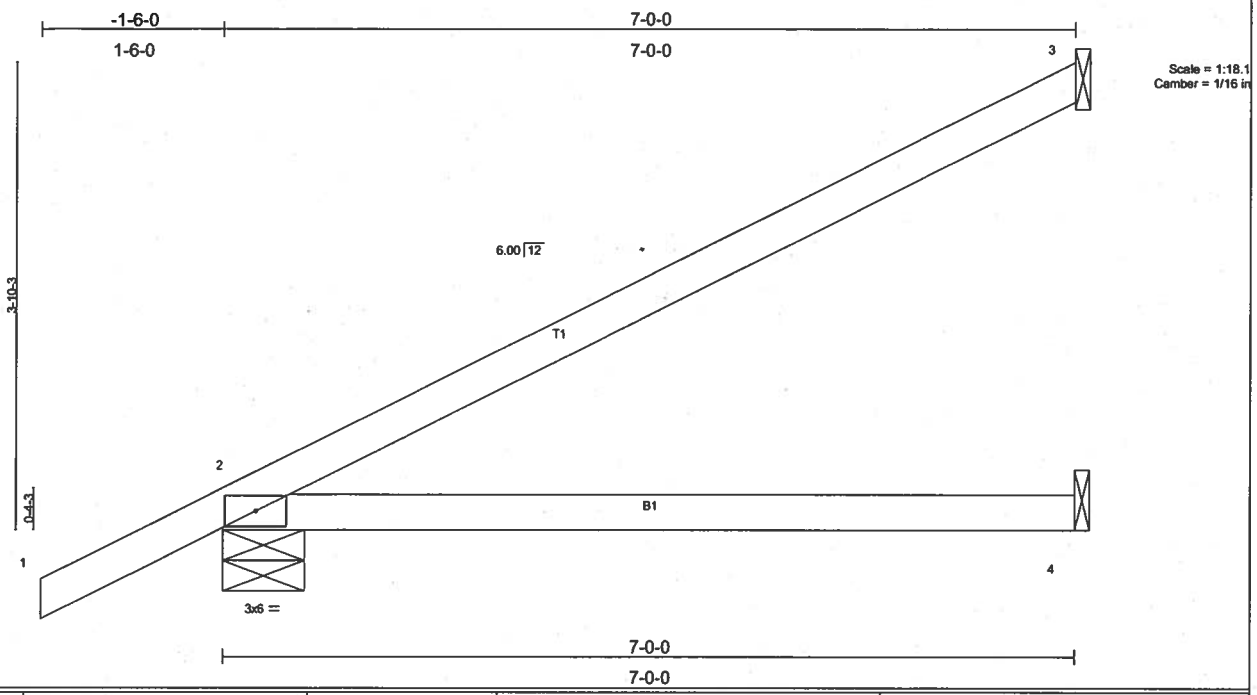
LOAD CASE(S) Standard

NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1116061006

Job	Truss	Truss Type	Qty	Ply	LOT 15--FORT WHITE STATION
L217897	EJ7	MONO TRUSS	22	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Nov 15 15:26:03 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	0.28	2-4	>284	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.36	Vert(TL)	0.24	2-4	>335	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 25 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=158/Mechanical, 2=390/0-8-0, 4=105/Mechanical
Max Horz 2=247(load case 5)
Max Uplift3=-176(load case 5), 2=-327(load case 5), 4=-87(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/36, 2-3=-108/57
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.61

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 176 lb uplift at joint 3, 327 lb uplift at joint 2 and 87 lb uplift at joint 4.

LOAD CASE(S) Standard

NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1116061007

Job	Truss	Truss Type	Qty	Ply	LOT 15--FORT WHITE STATION
L217897	EJ7A	MONO TRUSS	11	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Nov 15 15:27:23 2006 Page 1

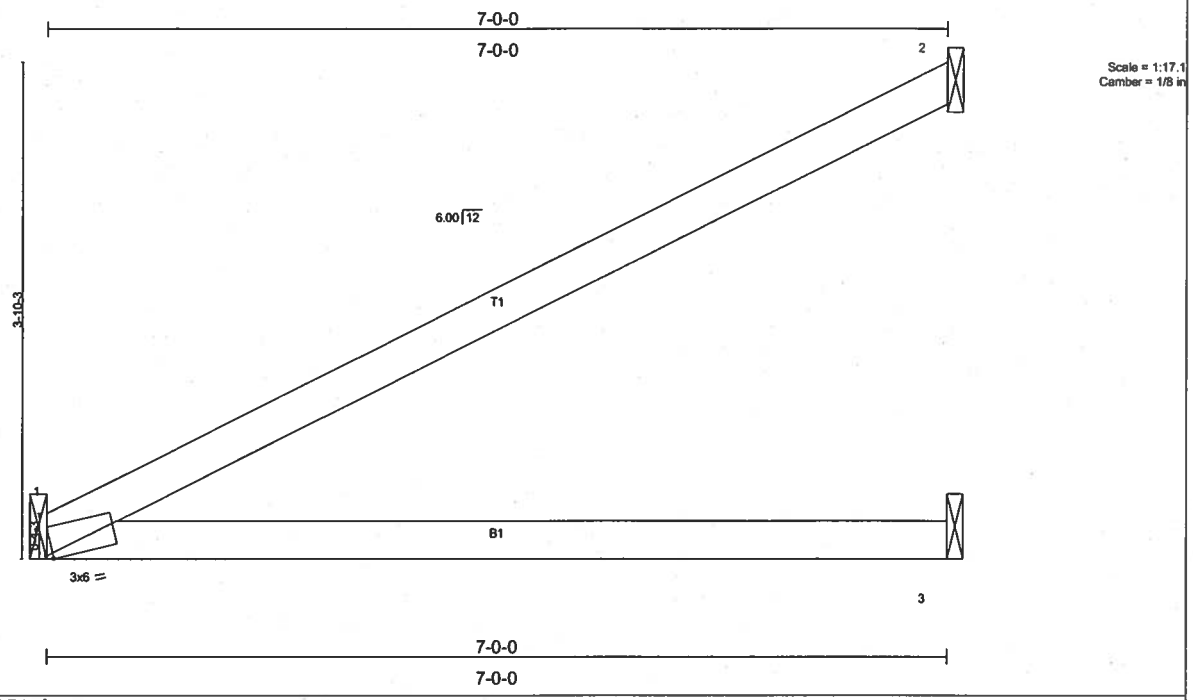


Plate Offsets (X,Y): [1:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	0.32	1-3	>255	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.36	Vert(TL)	0.27	1-3	>309	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 22 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=289/Mechanical, 2=171/Mechanical, 3=118/Mechanical
Max Horz 1=162(load case 5)
Max Uplift1=-154(load case 5), 2=-154(load case 5), 3=-82(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-96/61
BOT CHORD 1-3=0/0

JOINT STRESS INDEX
1 = 0.87

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 154 lb uplift at joint 1, 154 lb uplift at joint 2 and 82 lb uplift at joint 3.

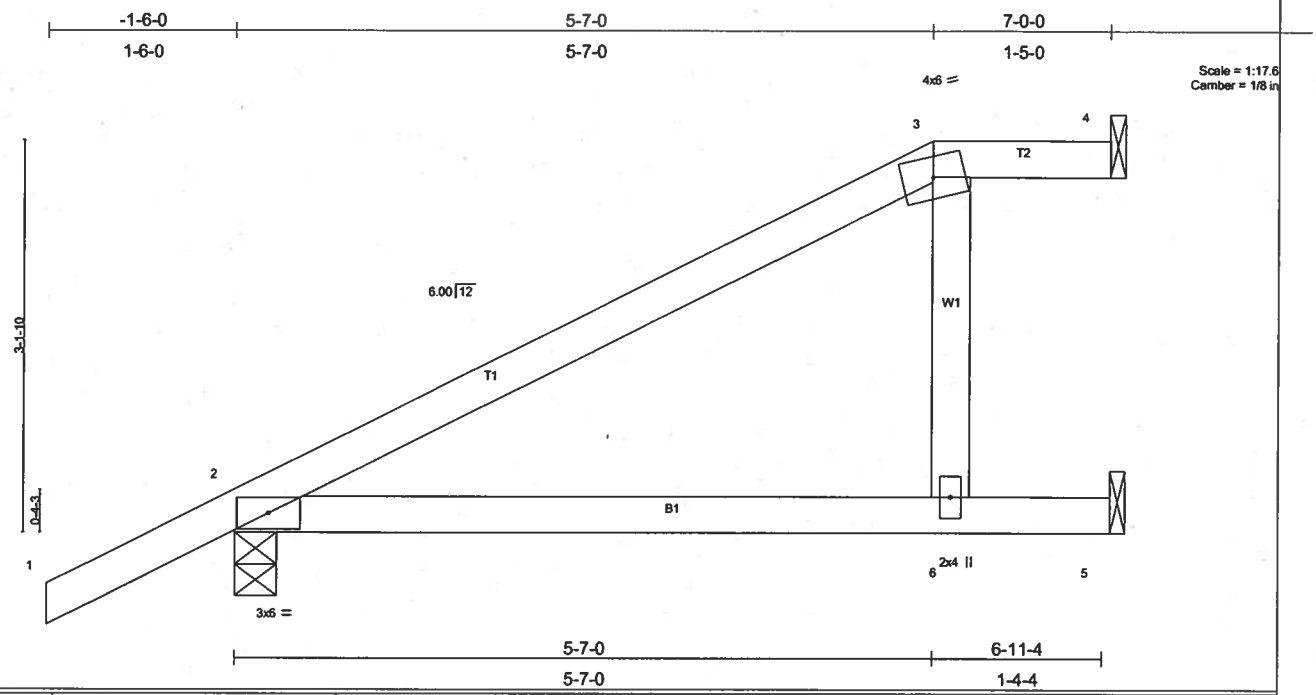
LOAD CASE(S) Standard

NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1116061008

Job	Truss	Truss Type	Qty	Ply	LOT 15--FORT WHITE STATION
L217897	EJ7B	MONO HIP	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Nov 15 15:27:55 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.15	2-6	>528	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.25	2-6	>325	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.03	Horz(TL)	0.07	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 28 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 4=101/Mechanical, 2=385/0-4-0, 5=173/Mechanical
Max Horz 2=177(load case 5)
Max Uplift4=-23(load case 4), 2=-185(load case 5), 5=-96(load case 5)
Max Grav 4=101(load case 10), 2=385(load case 1), 5=173(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-86/36, 3-4=-0/1
BOT CHORD 2-6=-7/5, 5-6=0/0
WEBS 3-6=-89/137

JOINT STRESS INDEX
2 = 0.27, 3 = 0.53 and 6 = 0.08

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 23 lb uplift at joint 4, 185 lb uplift at joint 2 and 96 lb uplift at joint 5.

LOAD CASE(S) Standard

NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1116061009

Job	Truss	Truss Type	Qty	Ply	LOT 15--FORT WHITE STATION
L217897	EJ7C	MONO HIP	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Nov 15 15:28:54 2006 Page 1

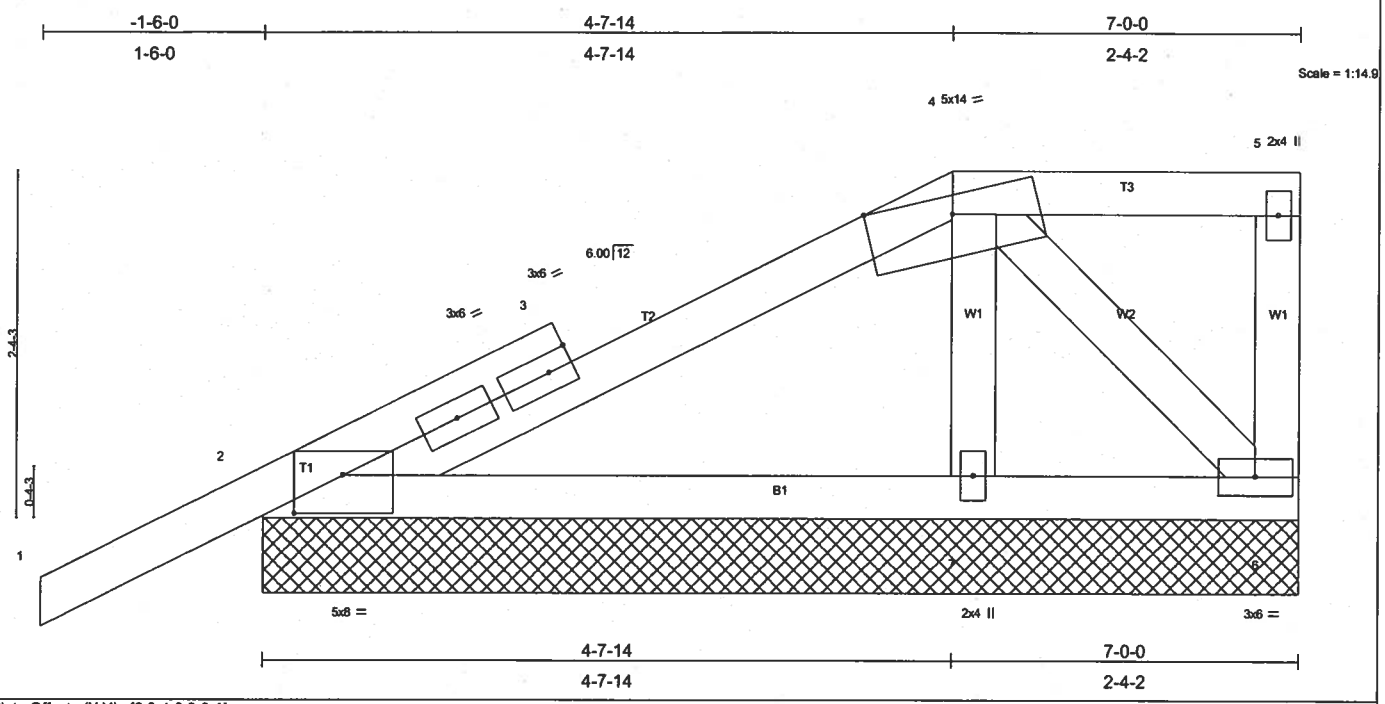


Plate Offsets (X,Y): [2:0-4-0,0-3-1]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.16	Vert(LL)	0.01	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	0.01	1	n/r	90		
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 36 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 7-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=244/Mechanical, 6=-35/Mechanical, 7=448/Mechanical
Max Horz 2=141(load case 5)
Max Uplift2=-140(load case 5), 6=-53(load case 3), 7=-152(load case 5)
Max Grav 2=244(load case 1), 6=21(load case 5), 7=448(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-72/43, 3-4=-61/105, 4-5=-5/10, 5-6=-17/42
BOT CHORD 2-7=-40/9, 6-7=-62/21
WEBS 4-7=-304/169, 4-6=-22/71

JOINT STRESS INDEX
2 = 0.59, 3 = 0.00, 3 = 0.18, 3 = 0.18, 4 = 0.42, 5 = 0.10, 6 = 0.06 and 7 = 0.11

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Gable requires continuous bottom chord bearing.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 140 lb uplift at joint 2, 53 lb uplift at joint 6 and 152 lb uplift at joint 7.

LOAD CASE(S) Standard

NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1116061010

Job	Truss	Truss Type	Qty	Ply	LOT 15 FORT WHITE STATION
L217897	EJ7T	SPECIAL	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 16 07:41:44 2006 Page 1

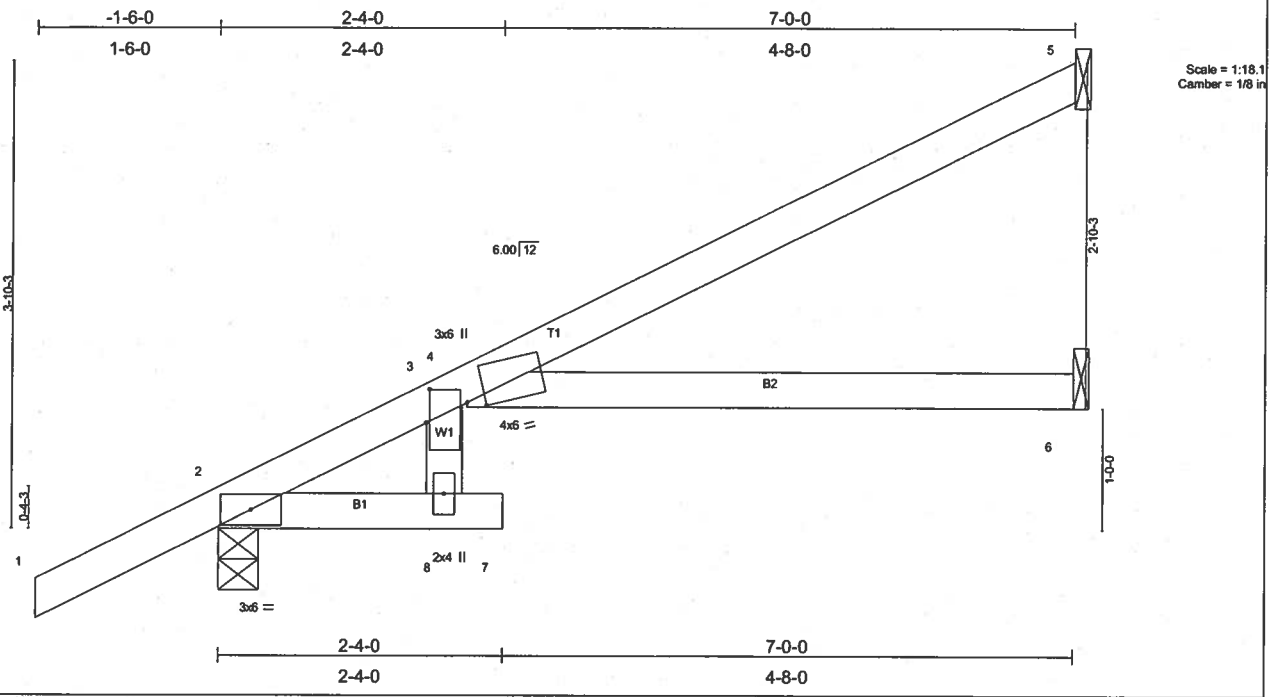


Plate Offsets (X,Y): [3:0-3-4,0-0-5], [4:0-1-12,0-0-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.63	Vert(LL)	-0.17	4-6	>473	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	-0.28	4-6	>292	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.02	Horz(TL)	0.13	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=156/Mechanical, 2=392/0-4-0, 6=119/Mechanical
Max Horz 2=208(load case 5)
Max Uplift5=-117(load case 5), 2=-172(load case 5), 6=-18(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-153/0, 3-4=-110/0, 4-5=-70/59
BOT CHORD 2-8=0/0, 7-8=0/0, 4-6=0/0
WEBS 3-8=-2/72

JOINT STRESS INDEX
2 = 0.46, 3 = 0.05, 4 = 0.93 and 8 = 0.05

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 117 lb uplift at joint 5, 172 lb uplift at joint 2 and 18 lb uplift at joint 6.

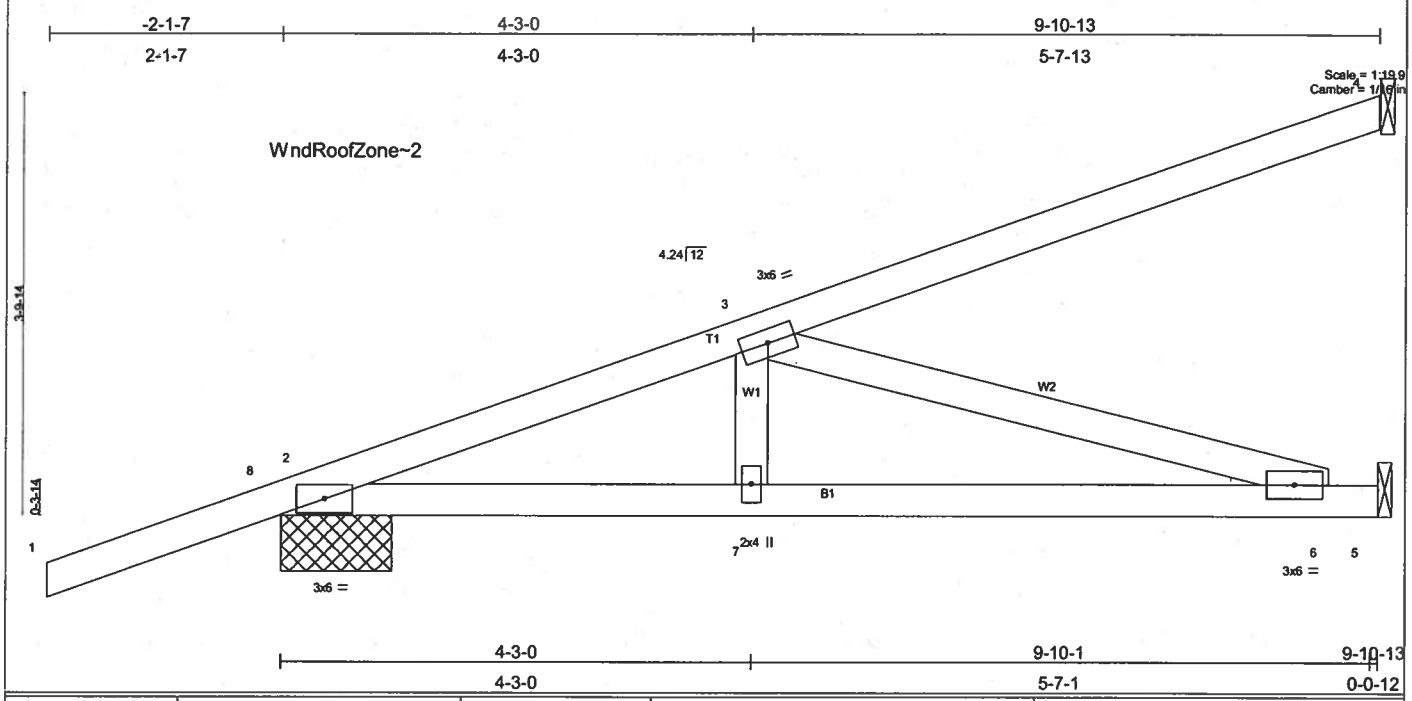
LOAD CASE(S) Standard

NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1116061011

Job	Truss	Truss Type	Qty	Ply	LOT 15--FORT WHITE STATION
L217897	HJ9	MONO TRUSS	5	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Nov 15 15:26:35 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	0.12	6-7	>974	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.18	6-7	>622	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.49	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 43 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-11-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-0-3 oc bracing.

REACTIONS (lb/size) 4=268/Mechanical, 2=484/1-0-1, 5=385/Mechanical
Max Horz 2=296(load case 2)
Max Uplift4=-283(load case 2), 2=-420(load case 2), 5=-242(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-8=0/33, 2-8=0/33, 2-3=-929/516, 3-4=-122/65
BOT CHORD 2-7=-722/862, 6-7=-722/862, 5-6=0/0
WEBS 3-7=-121/211, 3-6=-898/752

JOINT STRESS INDEX
2 = 0.77, 3 = 0.31, 6 = 0.25 and 7 = 0.15

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 283 lb uplift at joint 4, 420 lb uplift at joint 2 and 242 lb uplift at joint 5.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-8=-54
Trapezoidal Loads (plf)
Vert: 8=0(F=27, B=27)-to-4=-134(F=-40, B=-40), 2=-3(F=14, B=14)-to-5=-74(F=-22, B=-22)

NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1116061012

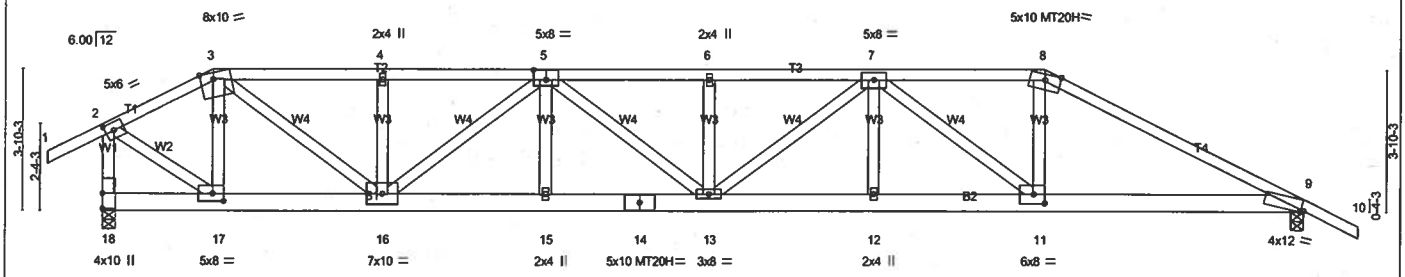
Job	Truss	Truss Type	Qty	Ply	LOT 15 FORT WHITE STATION
L217897	T01	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 16 07:12:23 2006 Page 1

-1-6-0 | 2-11-15 | 7-7-7 | 12-1-2 | 16-6-13 | 21-0-9 | 25-8-0 | 32-8-0 | 34-2-0

1-6-0 | 2-11-15 | 4-7-7 | 4-5-11 | 4-5-11 | 4-5-11 | 4-7-7 | 7-0-0 | 1-6-0

Scale = 1:59.7
Camber = 5/16 in



2-11-15 | 7-7-7 | 12-1-2 | 16-6-13 | 21-0-9 | 25-8-0 | 32-8-0

2-11-15 | 4-7-7 | 4-5-11 | 4-5-11 | 4-5-11 | 4-7-7 | 7-0-0

Plate Offsets (X,Y): [2:0-2-11,0-2-8], [3:0-4-3,Edge], [5:0-4-0,0-3-4], [9:0-0-13,Edge], [11:0-3-8,0-3-0], [17:0-3-8,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.87	Vert(LL)	-0.47	13	>828	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.80	Vert(TL)	-0.75	13	>518	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.97	Horz(TL)	0.15	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 206 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W4 2 X 4 SYP No.2, W4 2 X 4 SYP No.2, W4 2 X 4 SYP No.2, W4 2 X 4 SYP No.2
W4 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-10-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-10-15 oc bracing.

REACTIONS (lb/size) 18=3078/0-4-0, 9=2854/0-4-0
Max Horz 18=137(load case 6)
Max Uplift18=-1290(load case 3), 9=-1200(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-2711/1221, 3-4=-4984/2299, 4-5=-4984/2299, 5-6=-7069/3198, 6-7=-7069/3198, 7-8=-5008/2225, 8-9=-5598/2411, 9-10=0/39, 2-18=-2989/1299
BOT CHORD 17-18=-133/58, 16-17=-1059/2309, 15-16=-2918/6591, 14-15=-2918/6591, 13-14=-2918/6591, 12-13=-2863/6586, 11-12=-2863/6586, 9-11=-2076/4922
WEBS 3-17=-1279/701, 3-16=-1494/3351, 4-16=-529/436, 5-16=-2039/908, 5-15=0/344, 5-13=-249/611, 6-13=-509/414, 7-13=-303/643, 7-12=0/331, 7-11=-2097/1018, 8-11=-814/2055, 2-17=-1178/2791

JOINT STRESS INDEX
2 = 0.89, 3 = 0.81, 4 = 0.34, 5 = 1.00, 6 = 0.34, 7 = 0.49, 8 = 0.93, 9 = 0.83, 11 = 0.57, 12 = 0.34, 13 = 0.62, 14 = 0.91, 15 = 0.34, 16 = 0.80, 17 = 0.66 and 18 = 0.37

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1290 lb uplift at joint 18 and 1200 lb uplift at joint 9.
7) Girder carries hip end with 7-0-0 right side setback, 0-0-0 left side setback, and 7-0-0 end setback.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 25-8-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-3=-117(F=-63), 3-8=-117(F=-63), 8-10=-54, 11-18=-65(F=-35), 9-11=-30
Concentrated Loads (lb)
Vert: 11=-539(F)

NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

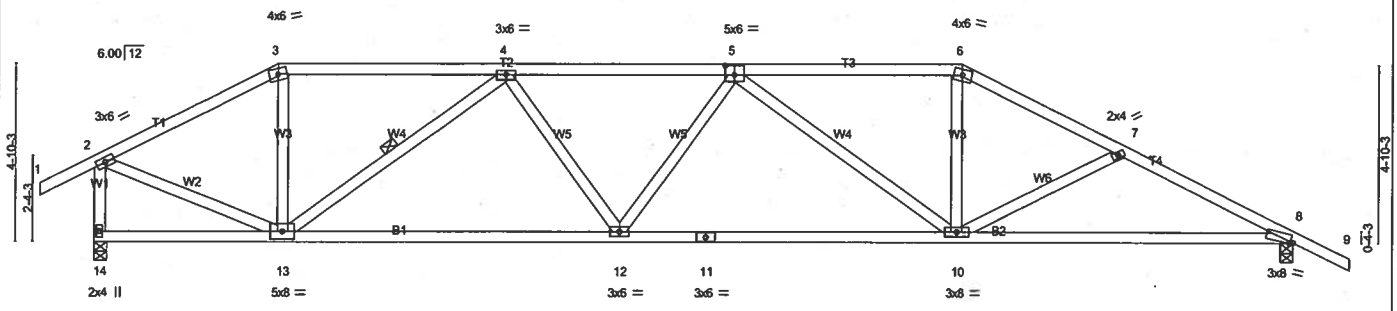
Dwg.#1116061013

Job	Truss	Truss Type	Qty	Ply	LOT 15 FORT WHITE STATION
L217897	T02	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 16 07:13:01 2006 Page 1

-1-6-0	4-11-15	11-2-10	17-5-5	23-8-0	27-10-12	32-8-0	34-2-0
1-6-0	4-11-15	6-2-11	6-2-11	6-2-11	4-2-12	4-9-4	1-6-0

Scale = 1:59.7
Camber = 3/16 in



4-11-15	14-4-0	23-8-0	32-8-0
4-11-15	9-4-0	9-4-0	9-0-0

Plate Offsets (X,Y): [5:0-3-0,0-3-0], [8:0-0-13,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.24	12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.69	Vert(TL)	-0.40	12-13	>964	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.62	Horz(TL)	0.10	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 173 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-8-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-5-4 oc bracing.
WEBS 1 Row at midpt 4-13

REACTIONS (lb/size) 14=1448/0-4-0, 8=1449/0-4-0
Max Horz 14=-124(load case 3)
Max Uplift14=-437(load case 5), 8=-485(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1504/481, 3-4=-1307/462, 4-5=-2386/796, 5-6=-1995/652, 6-7=-2253/691, 7-8=-2478/724, 8-9=0/35, 2-14=-1398/516
BOT CHORD 13-14=-33/120, 12-13=-690/2174, 11-12=-723/2406, 10-11=-723/2406, 8-10=-580/2162
WEBS 3-13=-62/395, 4-13=-1117/456, 4-12=-35/390, 5-12=-50/104, 5-10=-603/316, 6-10=-136/692, 7-10=-214/206, 2-13=-419/1380

JOINT STRESS INDEX
2 = 0.79, 3 = 0.63, 4 = 0.41, 5 = 0.47, 6 = 0.69, 7 = 0.34, 8 = 0.81, 10 = 0.57, 11 = 0.85, 12 = 0.41, 13 = 0.63 and 14 = 0.51

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 437 lb uplift at joint 14 and 485 lb uplift at joint 8.

LOAD CASE(S) Standard

NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1116061014

Job	Truss	Truss Type	Qty	Ply	LOT 15 FORT WHITE STATION
L217897	T03	HIP	1	1	Job Reference (optional)

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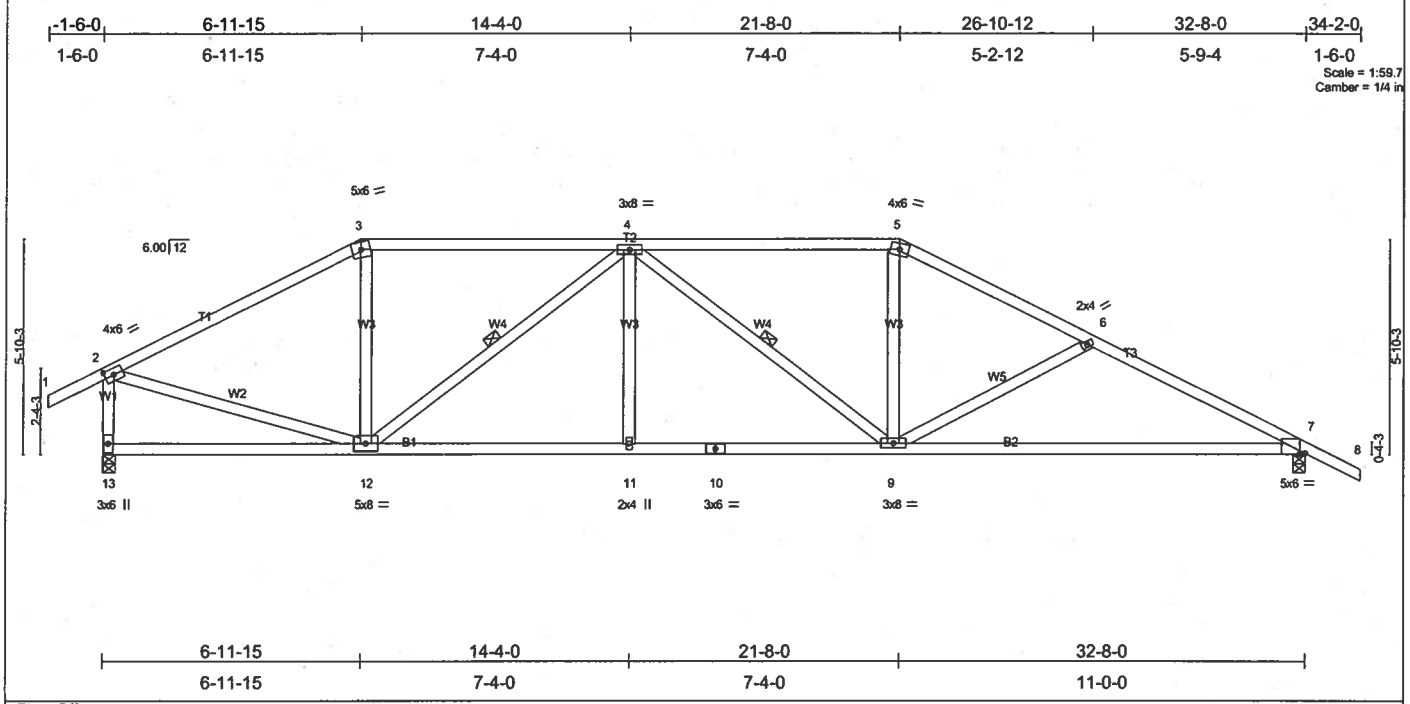


Plate Offsets (X,Y): [2:0-2-15,0-2-0], [7:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.39	7-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.84	Vert(TL)	-0.66	7-9	>589	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.42	Horz(TL)	0.09	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 177 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-7-5 oc bracing.
WEBS 1 Row at midpt 4-12, 4-9

REACTIONS (lb/size) 13=1448/0-4-0, 7=1449/0-4-0
Max Horz 13=-171(load case 6)
Max Uplift13=-460(load case 5), 7=-501(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1626/490, 3-4=-1389/489, 4-5=-1839/597, 5-6=-2102/617, 6-7=-2407/722, 7-8=0/35, 2-13=-1345/518
BOT CHORD 12-13=-56/136, 11-12=-527/1995, 10-11=-527/1995, 9-10=-527/1995, 7-9=-514/2103
WEBS 3-12=-38/377, 4-12=-833/314, 4-11=0/184, 4-9=-326/200, 5-9=-78/579, 6-9=-322/270, 2-12=-373/1325

JOINT STRESS INDEX
2 = 0.75, 3 = 0.67, 4 = 0.57, 5 = 0.81, 6 = 0.34, 7 = 0.71, 9 = 0.57, 10 = 0.71, 11 = 0.34, 12 = 0.62 and 13 = 0.40

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 460 lb uplift at joint 13 and 501 lb uplift at joint 7.

LOAD CASE(S) Standard

NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

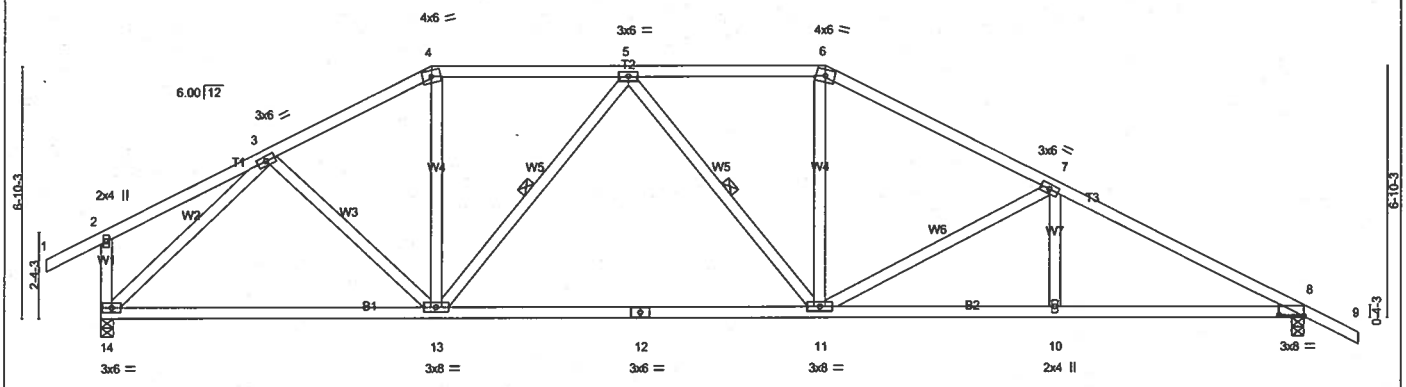
Dwg.#1116061015

Job	Truss	Truss Type	Qty	Ply	LOT 15 FORT WHITE STATION
L217897	T04	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 16 07:13:49 2006 Page 1

-1-6-0	4-7-12	8-11-15	14-4-0	19-8-0	25-10-12	32-8-0	34-2-0
1-6-0	4-7-12	4-4-4	5-4-0	5-4-0	6-2-12	6-9-4	1-6-0

Scale = 1:59.7
Camber = 3/16 in



8-11-15	19-8-0	25-10-12	32-8-0
8-11-15	10-8-1	6-2-12	6-9-4

Plate Offsets (X,Y): [8:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.56	Vert(LL)	-0.30	11-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.75	Vert(TL)	-0.50	11-13	>776	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.99	Horz(TL)	0.09	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 184 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-8-7 oc bracing.
WEBS 1 Row at midpt 5-13, 5-11

REACTIONS (lb/size) 14=1448/0-4-0, 8=1449/0-4-0
Max Horz 14=-153(load case 3)
Max Uplift14=-477(load case 5), 8=-518(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-174/139, 3-4=-1594/513, 4-5=-1392/496, 5-6=-1653/578, 6-7=-1915/583, 7-8=-2492/729, 8-9=0/35, 2-14=-276/247
BOT CHORD 13-14=-309/1169, 12-13=-362/1632, 11-12=-362/1632, 10-11=-513/2147, 8-10=-513/2147
WEBS 3-13=-91/370, 4-13=-81/414, 5-13=-456/225, 5-11=-115/148, 6-11=-84/510, 7-11=-577/323, 7-10=0/189, 3-14=-1497/382

JOINT STRESS INDEX
2 = 0.67, 3 = 0.44, 4 = 0.56, 5 = 0.40, 6 = 0.69, 7 = 0.41, 8 = 0.78, 10 = 0.34, 11 = 0.57, 12 = 0.91, 13 = 0.57 and 14 = 0.75

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 477 lb uplift at joint 14 and 518 lb uplift at joint 8.

LOAD CASE(S) Standard

NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1116061016

Job	Truss	Truss Type	Qty	Ply	LOT 15 FORT WHITE STATION
L217897	T05	HIP	1	1	Job Reference (optional)

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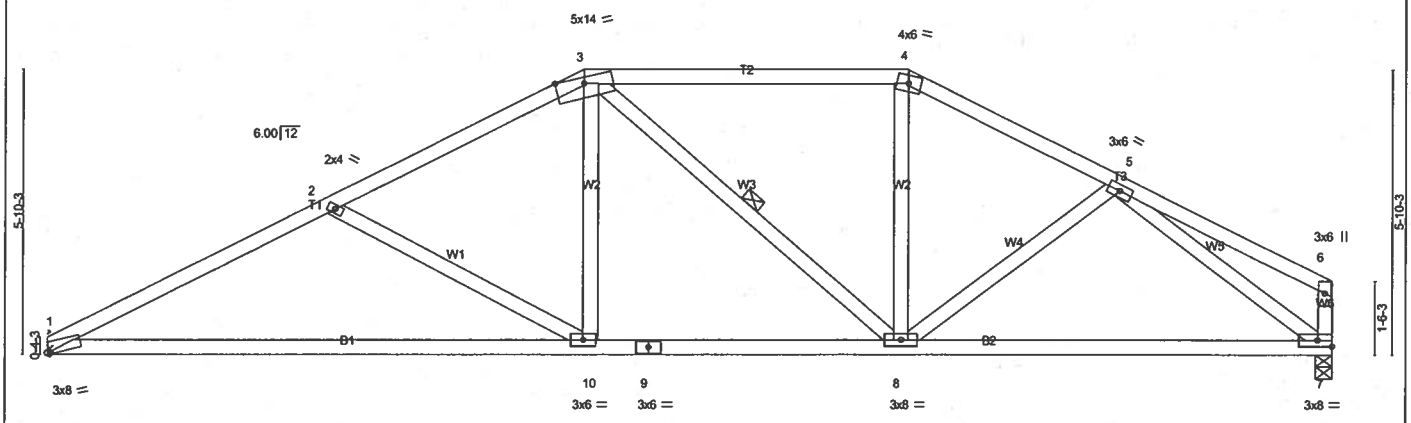
5-9-3	11-0-0	17-8-0	21-10-4	26-4-0
5-9-3	5-2-13	6-8-0	4-2-4	4-5-12

Scale = 1:45.2
Camber = 1/4 in



5-9-3	11-0-0	17-8-0	26-4-0
5-9-3	5-2-13	6-8-0	8-8-0

Plate Offsets (X,Y): [1:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.71	Vert(LL)	-0.39	1-10	>802	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.72	Vert(TL)	-0.66	1-10	>471	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.57	Horz(TL)	0.06	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 136 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-0-6 oc bracing.
WEBS 1 Row at midpt 3-8

REACTIONS (lb/size) 1=1094/Mechanical, 7=1094/0-4-0
Max Horz 1=119(load case 5)
Max Uplift1=-342(load case 5), 7=-323(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1864/614, 2-3=-1536/468, 3-4=-1185/422, 4-5=-1357/431, 5-6=-349/87, 6-7=-263/112
BOT CHORD 1-10=-589/1629, 9-10=-333/1326, 8-9=-333/1326, 7-8=-318/1115
WEBS 2-10=-358/293, 3-10=-92/466, 3-8=-278/101, 4-8=-47/261, 5-8=-54/195, 5-7=-1098/378

JOINT STRESS INDEX
1 = 0.89, 2 = 0.34, 3 = 0.78, 4 = 0.73, 5 = 0.41, 6 = 0.40, 7 = 0.71, 8 = 0.57, 9 = 0.74 and 10 = 0.35

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 342 lb uplift at joint 1 and 323 lb uplift at joint 7.

LOAD CASE(S) Standard

NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1116061017

Job	Truss	Truss Type	Qty	Ply	LOT 15 FORT WHITE STATION
L217897	T06	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 16 07:17:02 2006 Page 1

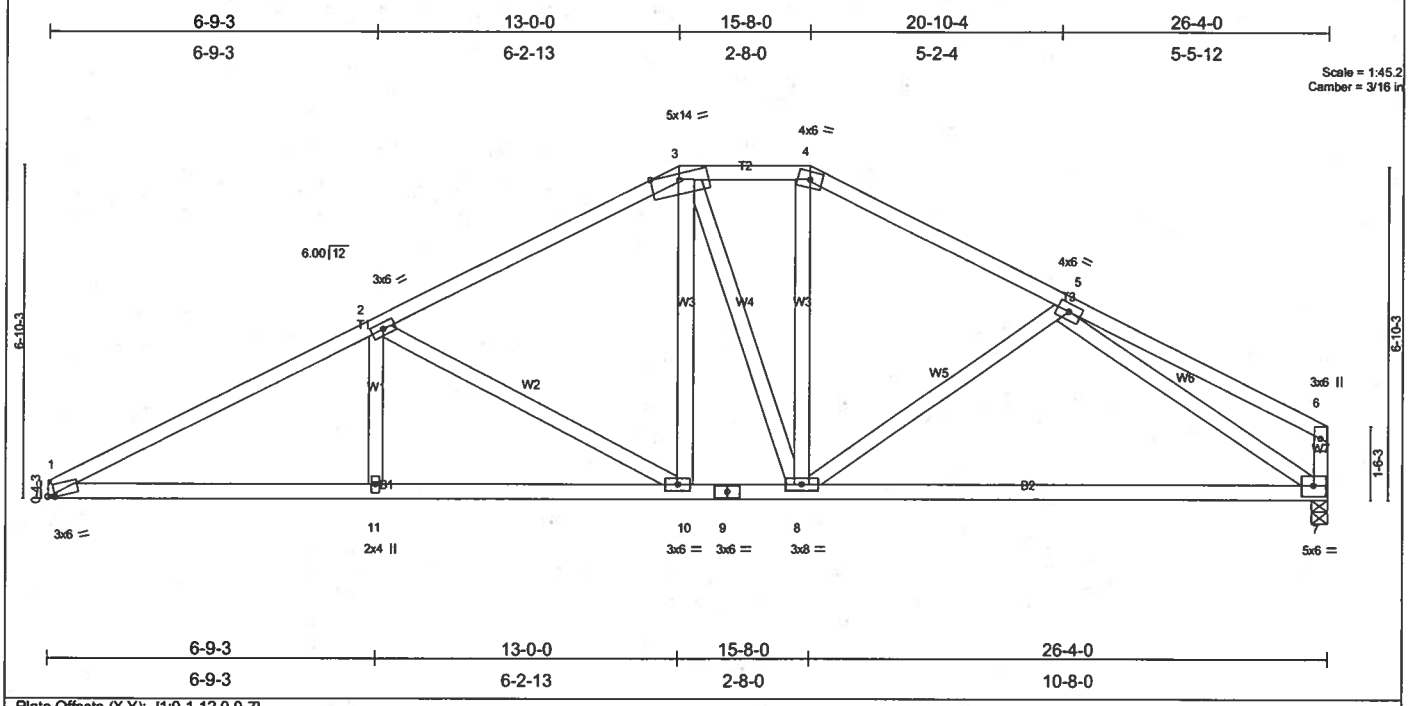


Plate Offsets (X,Y): [1:0-1-12,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.97	Vert(LL)	-0.22	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.39	7-8	>806	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.78	Horz(TL)	0.06	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 147 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-1-11 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-11-5 oc bracing.

REACTIONS (lb/size) 1=1093/Mechanical, 7=1093/0-4-0
Max Horz 1=133(load case 5)
Max Uplift1=-353(load case 5), 7=-338(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1961/609, 2-3=-1311/446, 3-4=-1098/422, 4-5=-1286/425, 5-6=-442/108, 6-7=-314/135
BOT CHORD 1-11=-589/1679, 10-11=-589/1679, 9-10=-288/1105, 8-9=-288/1105, 7-8=-344/1159
WEBS 2-11=0/243, 2-10=-662/345, 3-10=-150/342, 3-8=-138/148, 4-8=-119/365, 5-8=-125/203, 5-7=-1025/397

JOINT STRESS INDEX
1 = 0.80, 2 = 0.41, 3 = 0.56, 4 = 0.43, 5 = 0.30, 6 = 0.48, 7 = 0.66, 8 = 0.66, 9 = 0.54, 10 = 0.35 and 11 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 353 lb uplift at joint 1 and 338 lb uplift at joint 7.

LOAD CASE(S) Standard

NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1116061018

Job	Truss	Truss Type	Qty	Ply	LOT 15 FORT WHITE STATION
L217897	T07	COMMON	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 16 07:20:42 2006 Page 1

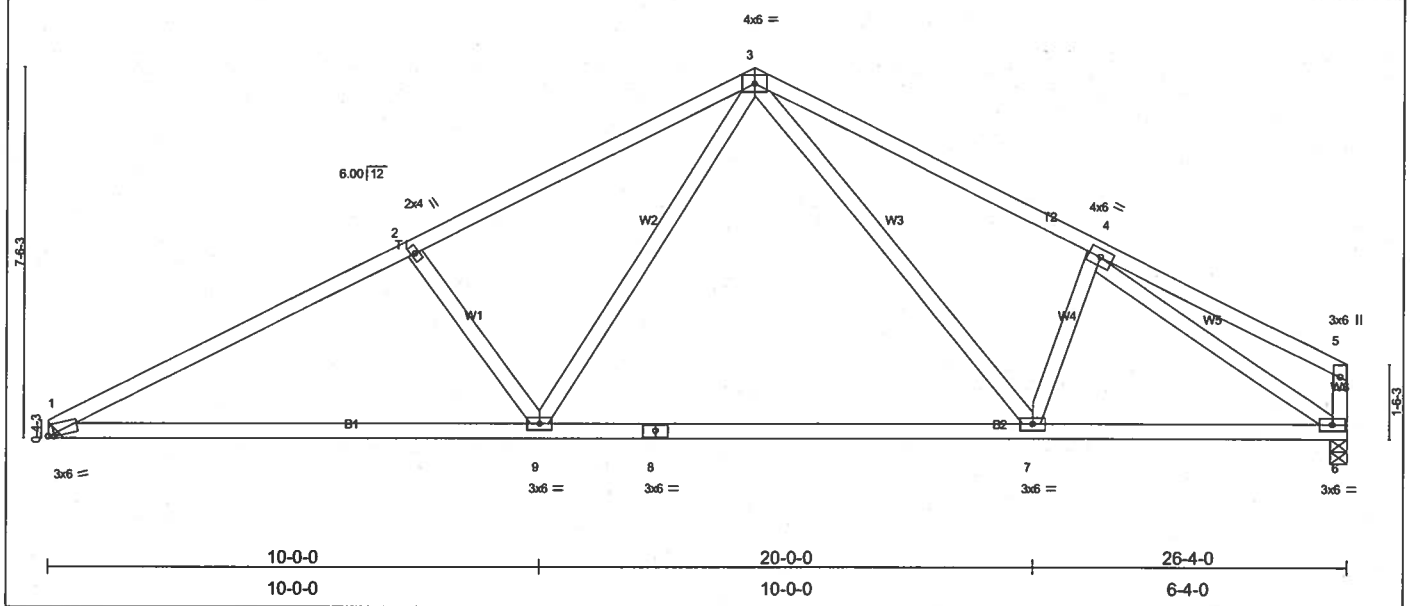


Plate Offsets (X,Y): [1:0-1-8,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	-0.28	1-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.69	Vert(TL)	-0.46	1-9	>678	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.88	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 131 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-10-5 oc bracing.

REACTIONS (lb/size) 1=1093/Mechanical, 6=1093/0-4-0
Max Horz 1=142(load case 5)
Max Uplift1=-360(load case 5), 6=-346(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1842/612, 2-3=-1627/589, 3-4=-1434/550, 4-5=-212/119, 5-6=-191/124
BOT CHORD 1-9=-594/1589, 8-9=-269/993, 7-8=-269/993, 6-7=-355/1223
WEBS 2-9=-387/331, 3-9=-258/764, 3-7=-181/435, 4-7=-100/212, 4-6=-1359/383

JOINT STRESS INDEX
1 = 0.78, 2 = 0.34, 3 = 0.69, 4 = 0.39, 5 = 0.32, 6 = 0.49, 7 = 0.50, 8 = 0.34 and 9 = 0.60

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 360 lb uplift at joint 1 and 346 lb uplift at joint 6.

LOAD CASE(S) Standard

NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

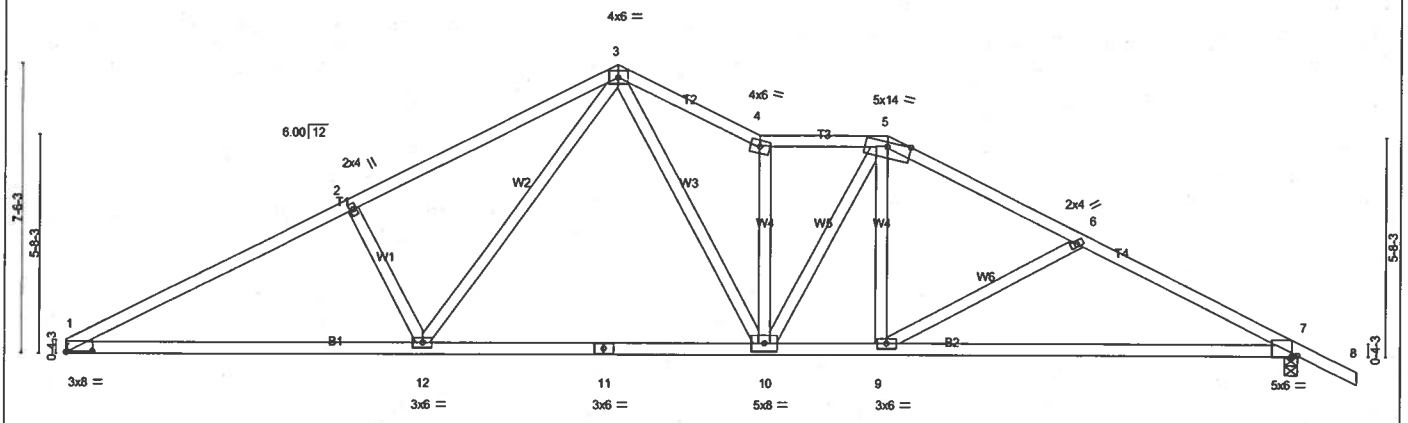
Dwg.#1116061019

Job	Truss	Truss Type	Qty	Ply	LOT 15 FORT WHITE STATION
L217897	T08	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 16 07:25:14 2006 Page 1

7-5-3	14-4-0	18-0-0	21-4-0	26-4-12	32-0-0	33-6-0
7-5-3	6-10-12	3-8-0	3-4-0	5-0-12	5-7-4	1-6-0

Scale = 1:57.0
Camber = 1/4 in



9-3-4	18-0-0	21-4-0	32-0-0
9-3-4	8-8-12	3-4-0	10-8-0

Plate Offsets (X,Y): [1:0-8-4,0-0-6], [7:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	-0.36	7-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.85	Vert(TL)	-0.61	7-9	>619	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.56	Horz(TL)	0.10	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 166 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-5-15 oc bracing.

REACTIONS (lb/size) 1=1328/Mechanical, 7=1422/0-4-0
Max Horz 1=-147(load case 6)
Max Uplift1=-422(load case 5), 7=-543(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2370/747, 2-3=-2209/772, 3-4=-2194/827, 4-5=-1924/691, 5-6=-2059/687, 6-7=-2367/816, 7-8=0/35
BOT CHORD 1-12=-654/2057, 11-12=-328/1447, 10-11=-328/1447, 9-10=-371/1791, 7-9=-599/2069
WEBS 2-12=-369/330, 3-12=-307/792, 3-10=-425/1091, 4-10=-1075/449, 5-10=-117/260, 5-9=-79/424, 6-9=-338/262

JOINT STRESS INDEX
1 = 0.78, 2 = 0.34, 3 = 0.90, 4 = 0.65, 5 = 0.45, 6 = 0.34, 7 = 0.70, 9 = 0.35, 10 = 0.56, 11 = 0.74 and 12 = 0.58

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 422 lb uplift at joint 1 and 543 lb uplift at joint 7.

LOAD CASE(S) Standard

NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

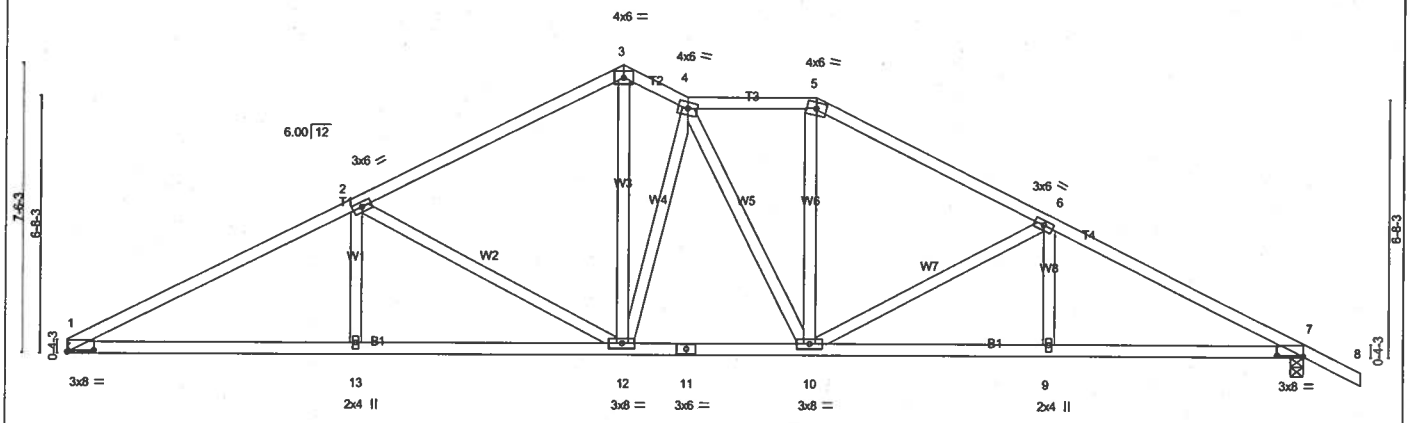
Dwg.#1116061020

Job	Truss	Truss Type	Qty	Ply	LOT 15 FORT WHITE STATION
L217897	T09	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 16 07:25:56 2006 Page 1

7-5-3	14-4-0	16-0-0	19-4-0	25-4-12	32-0-0	33-6-0
7-5-3	6-10-12	1-8-0	3-4-0	6-0-12	6-7-4	1-6-0

Scale = 1:57.0
Camber = 1/8 in



7-5-3	14-4-0	19-4-0	25-4-12	32-0-0
7-5-3	6-10-12	5-0-0	6-0-12	6-7-4

Plate Offsets (X,Y): [1:0-8-4,0-0-10], [7:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.18	1-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.66	Vert(TL)	-0.29	1-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.76	Horz(TL)	0.10	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 173 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-5-13 oc bracing.

REACTIONS (lb/size) 1=1328/Mechanical, 7=1422/0-4-0
Max Horz 1=-147(load case 6)
Max Uplift1=-422(load case 5), 7=-543(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2430/744, 2-3=-1738/604, 3-4=-1599/594, 4-5=-1605/639, 5-6=-1860/650, 6-7=-2453/783, 7-8=0/35
BOT CHORD 1-13=-650/2089, 12-13=-650/2089, 11-12=-357/1632, 10-11=-357/1632, 9-10=-563/2113, 7-9=-563/2113
WEBS 2-13=0/258, 2-12=-710/373, 3-12=-352/1095, 4-12=-671/296, 4-10=-158/68, 5-10=-110/489, 6-10=-597/302, 6-9=0/217

JOINT STRESS INDEX
1 = 0.78, 2 = 0.41, 3 = 0.62, 4 = 0.44, 5 = 0.66, 6 = 0.41, 7 = 0.77, 9 = 0.34, 10 = 0.62, 11 = 0.55, 12 = 0.68 and 13 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 422 lb uplift at joint 1 and 543 lb uplift at joint 7.

LOAD CASE(S) Standard

NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

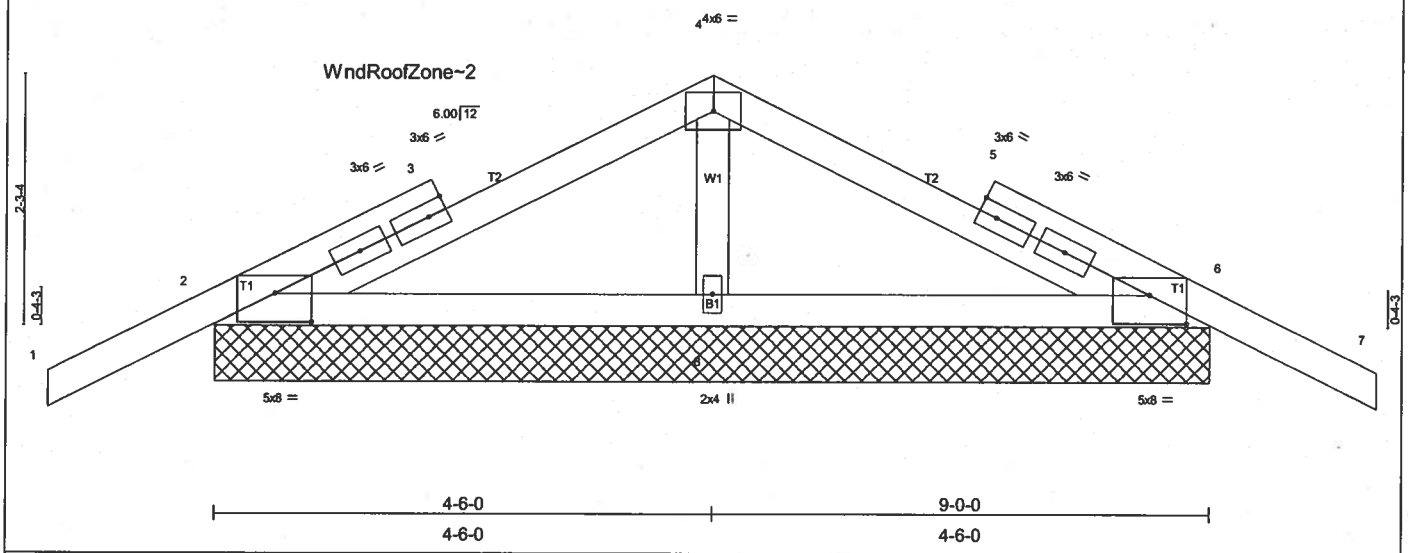
Dwg.#1116061021

Job	Truss	Truss Type	Qty	Ply	LOT 15 FORT WHITE STATION
L217897	T10	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 16 07:28:07 2006 Page 1

-1-10-0 | 4-6-0 | 9-0-0 | 10-6-0
1-10-0 | 4-6-0 | 4-6-0 | 1-6-0

Scale = 1:19.8



4-6-0 | 9-0-0
4-6-0 | 4-6-0

Plate Offsets (X,Y): [2:0-4-0,0-3-1], [6:0-4-0,0-3-1]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.20	Vert(LL)	0.01	7	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	0.01	7	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.07	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 41 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 9-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=243/9-0-0, 6=243/9-0-0, 8=552/9-0-0
Max Horz 2=-55(load case 6)
Max Uplift2=-160(load case 5), 6=-169(load case 6), 8=-153(load case 5)
Max Grav 2=261(load case 9), 6=261(load case 10), 8=552(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/41, 2-3=-46/101, 3-4=-40/173, 4-5=-31/173, 5-6=-37/101, 6-7=0/41
BOT CHORD 2-8=-95/142, 6-8=-95/142
WEBS 4-8=-395/259

JOINT STRESS INDEX
2 = 0.67, 3 = 0.00, 3 = 0.24, 3 = 0.24, 4 = 0.55, 5 = 0.00, 5 = 0.24, 5 = 0.24, 6 = 0.67 and 8 = 0.15

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Gable requires continuous bottom chord bearing.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 160 lb uplift at joint 2, 169 lb uplift at joint 6 and 153 lb uplift at joint 8.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-64(F=-10), 4-7=-64(F=-10), 2-6=-30

NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1116061022

Job	Truss	Truss Type	Qty	Ply	LOT 15 FORT WHITE STATION
L217897	T11	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 16 07:29:56 2006 Page 1

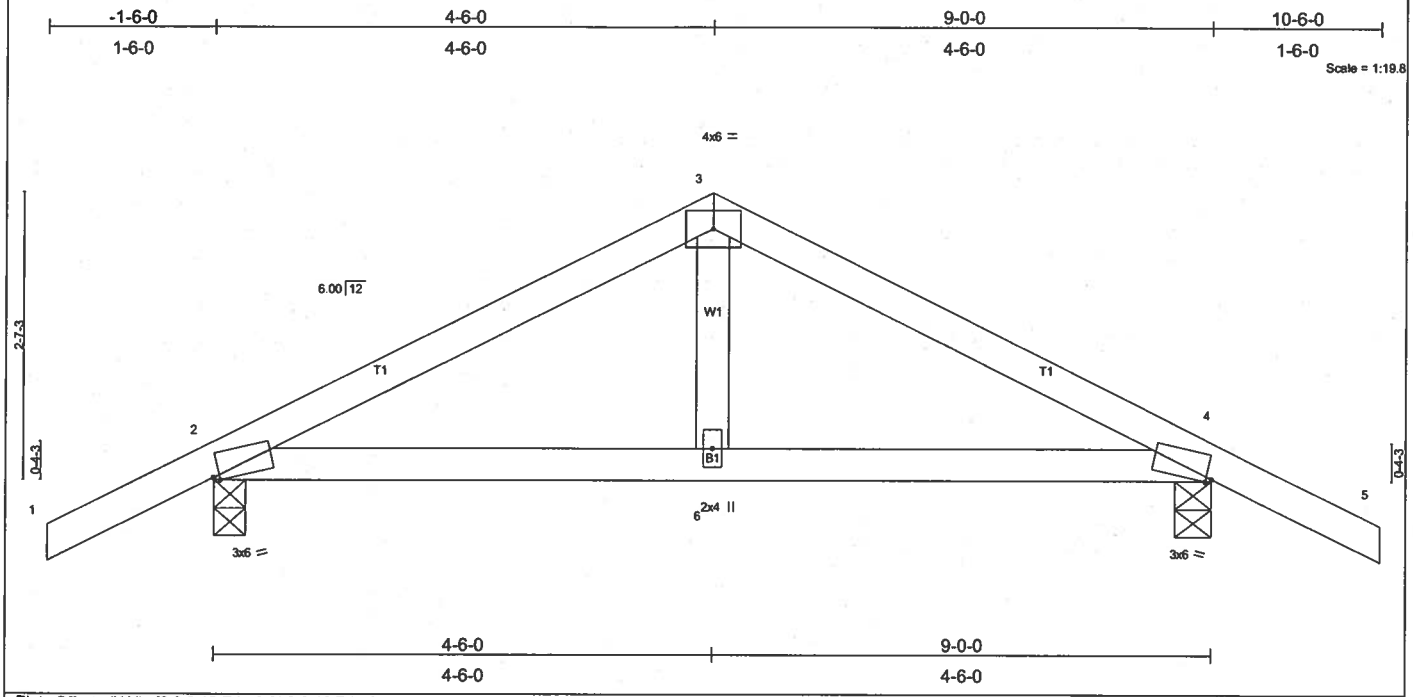

Plate Offsets (X,Y): [2:0-0-10,Edge], [4:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.26	Vert(LL)	-0.04	2-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.06	2-6	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.20	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 37 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-11-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=883/0-3-8, 4=885/0-4-0
Max Horz 2=-60(load case 5)
Max Uplift2=-380(load case 4), 4=-381(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1020/334, 3-4=-1021/334, 4-5=0/35
BOT CHORD 2-6=-239/877, 4-6=-239/877
WEBS 3-6=-153/629

JOINT STRESS INDEX
2 = 0.76, 3 = 0.33, 4 = 0.76 and 6 = 0.46

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 380 lb uplift at joint 2 and 381 lb uplift at joint 4.
5) Girder carries tie-in span(s): 7-0-0 from 0-0-0 to 9-0-0
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-54, 2-4=-129(F=-99)

NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1116061023

Job	Truss	Truss Type	Qty	Ply	LOT 15 FORT WHITE STATION
L217897	T12	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 16 07:31:01 2006 Page 1

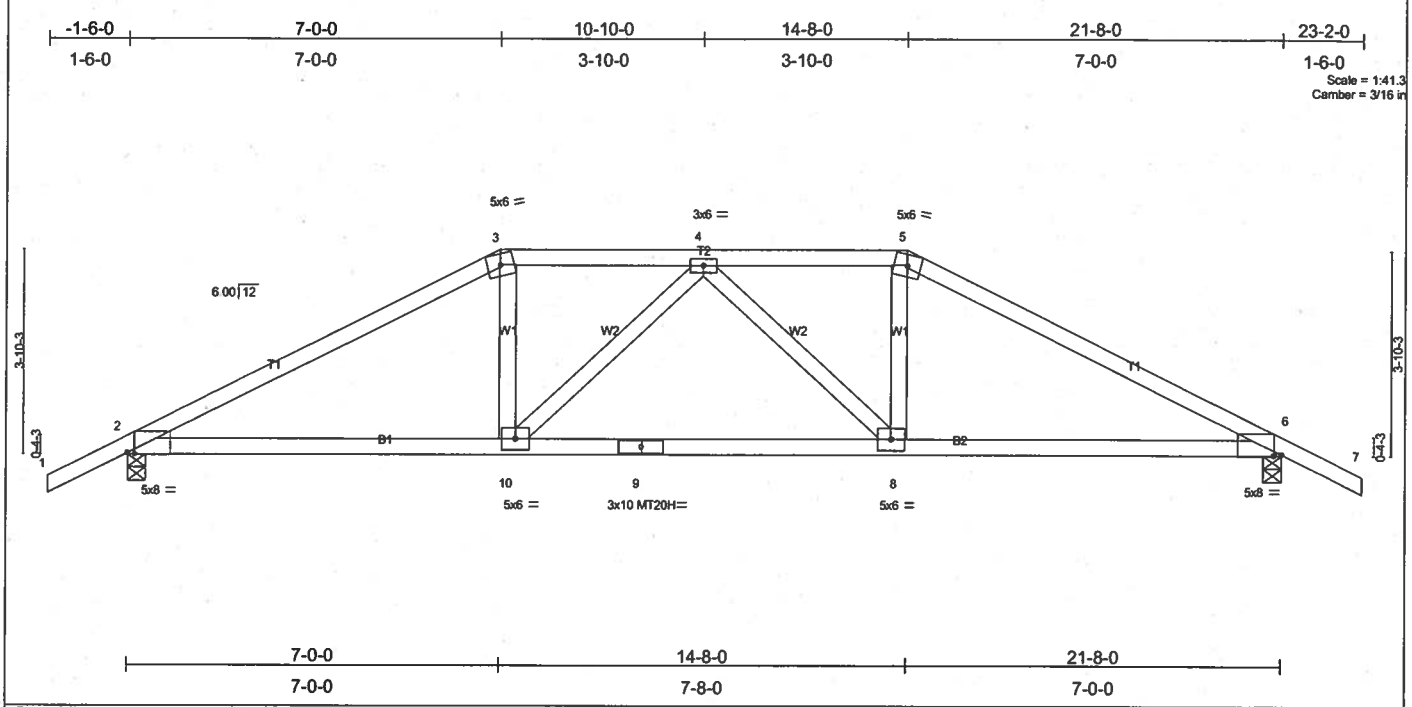

Plate Offsets (X,Y): [2:0-1-11,Edge], [6:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.25	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.78	Vert(TL)	-0.42	8-10	>605	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.38	Horz(TL)	0.10	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 96 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-9-3 oc bracing.

REACTIONS (lb/size) 2=1896/0-4-0, 6=1896/0-4-0
Max Horz 2=78(load case 4)
Max Uplift2=-834(load case 4), 6=-834(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-3484/1394, 3-4=-3068/1321, 4-5=-3068/1321, 5-6=-3484/1394, 6-7=0/35
BOT CHORD 2-10=-1185/3022, 9-10=-1364/3261, 8-9=-1364/3261, 6-8=-1141/3022
WEBS 3-10=-417/1179, 4-10=-389/318, 4-8=-389/318, 5-8=-417/1179

JOINT STRESS INDEX
2 = 0.79, 3 = 0.77, 4 = 0.36, 5 = 0.77, 6 = 0.79, 8 = 0.41, 9 = 0.92 and 10 = 0.41

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 834 lb uplift at joint 2 and 834 lb uplift at joint 6.
7) Girder carries hip end with 7-0-0 end setback.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 14-8-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-117(F=-63), 5-7=-54, 2-10=-30, 8-10=-65(F=-35), 6-8=-30
Concentrated Loads (lb)
Vert: 10=-539(F) 8=-539(F)

NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1116061024

Job	Truss	Truss Type	Qty	Ply	LOT 15 FORT WHITE STATION
L217897	T13	HIP	1	1	Job Reference (optional)

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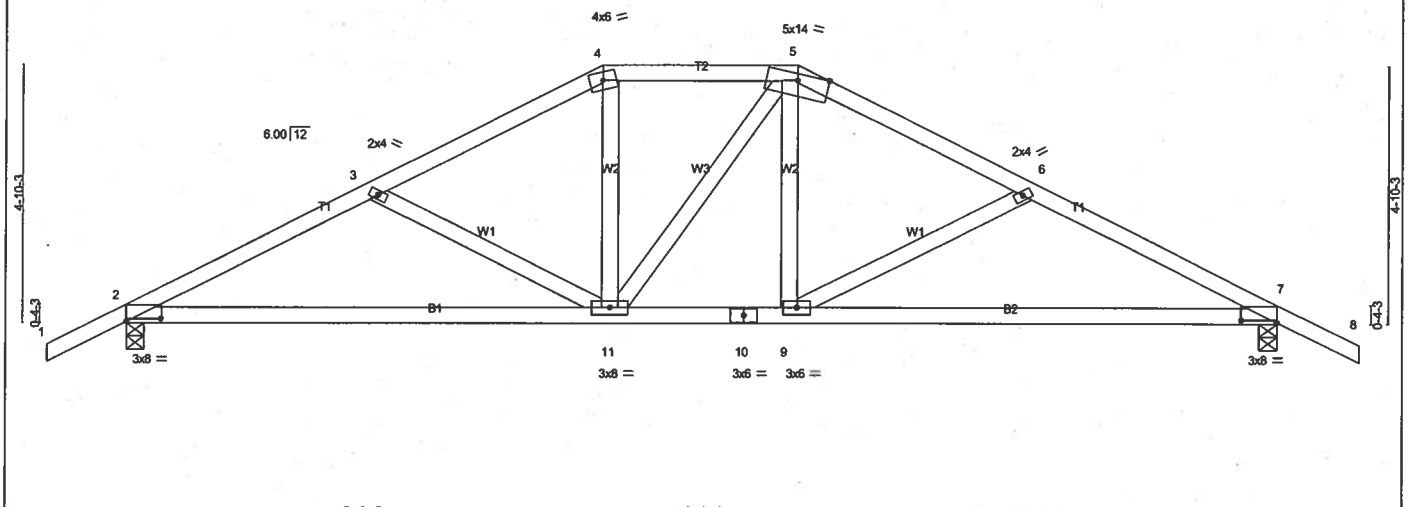
-1-6-0	4-9-4	9-0-0	12-8-0	16-10-12	21-8-0	23-2-0
1-6-0	4-9-4	4-2-12	3-8-0	4-2-12	4-9-4	1-6-0

Scale = 1:41.3
Camber = 1/8 in



9-0-0	12-8-0	21-8-0
9-0-0	3-8-0	9-0-0

Plate Offsets (X,Y): [2:0-8-0,0-0-10], [7:0-8-0,0-0-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.26	Vert(LL)	-0.18	7-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	-0.31	7-9	>838	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.13	Horz(TL)	0.04	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 108 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-11-12 oc bracing.

REACTIONS (lb/size) 2=986/0-4-0, 7=986/0-4-0
Max Horz 2=-92(load case 6)
Max Uplift2=-377(load case 5), 7=-377(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1489/472, 3-4=-1224/360, 4-5=-1053/365, 5-6=-1224/361, 6-7=-1489/473, 7-8=0/35
BOT CHORD 2-11=-397/1294, 10-11=-175/1052, 9-10=-175/1052, 7-9=-306/1294
WEBS 3-11=-284/221, 4-11=-65/315, 5-11=-102/106, 5-9=-62/316, 6-9=-286/222

JOINT STRESS INDEX
2 = 0.74, 3 = 0.34, 4 = 0.34, 5 = 0.34, 6 = 0.34, 7 = 0.74, 9 = 0.35, 10 = 0.60 and 11 = 0.58

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 377 lb uplift at joint 2 and 377 lb uplift at joint 7.

LOAD CASE(S) Standard

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Dwg.#1116061025

Job	Truss	Truss Type	Qty	Ply	LOT 15 FORT WHITE STATION
L217897	T14	COMMON	6	1	Job Reference (optional)

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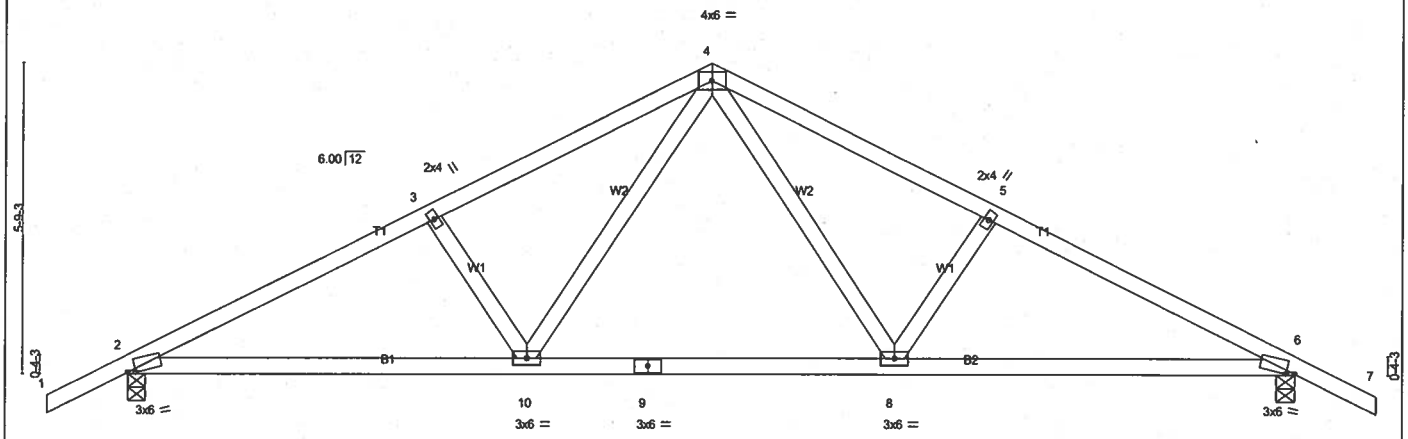
-1-6-0 | 5-8-4 | 10-10-0 | 15-11-12 | 21-8-0 | 23-2-0

1-6-0 | 5-8-4 | 5-1-12 | 5-1-12 | 5-8-4 | 1-6-0

Scale = 1:40.8
Camber = 1/8 in



7-4-13 | 14-3-3 | 21-8-0

7-4-13 | 6-10-5 | 7-4-13

Plate Offsets (X,Y): [2:0-1-13,0-0-7], [6:0-1-13,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	-0.20	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.83	Vert(TL)	-0.32	8-10	>793	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.24	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 101 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-7-3 oc bracing.

REACTIONS (lb/size) 2=1158/0-4-0, 6=1158/0-4-0
Max Horz 2=104(load case 5)
Max Uplift2=-453(load case 5), 6=-453(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1912/621, 3-4=-1755/620, 4-5=-1755/620, 5-6=-1912/621, 6-7=0/35
BOT CHORD 2-10=-532/1638, 9-10=-273/1126, 8-9=-273/1126, 6-8=-437/1638
WEBS 3-10=-252/225, 4-10=-260/743, 4-8=-260/743, 5-8=-252/225

JOINT STRESS INDEX
2 = 0.80, 3 = 0.34, 4 = 0.60, 5 = 0.34, 6 = 0.80, 8 = 0.57, 9 = 0.73 and 10 = 0.57

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 453 lb uplift at joint 2 and 453 lb uplift at joint 6.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-10=-30, 8-10=-80(F=-50), 6-8=-30

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Dwg.#1116061026

Job	Truss	Truss Type	Qty	Ply	LOT 15 FORT WHITE STATION
L217897	T15	GABLE	1	1	Job Reference (optional)

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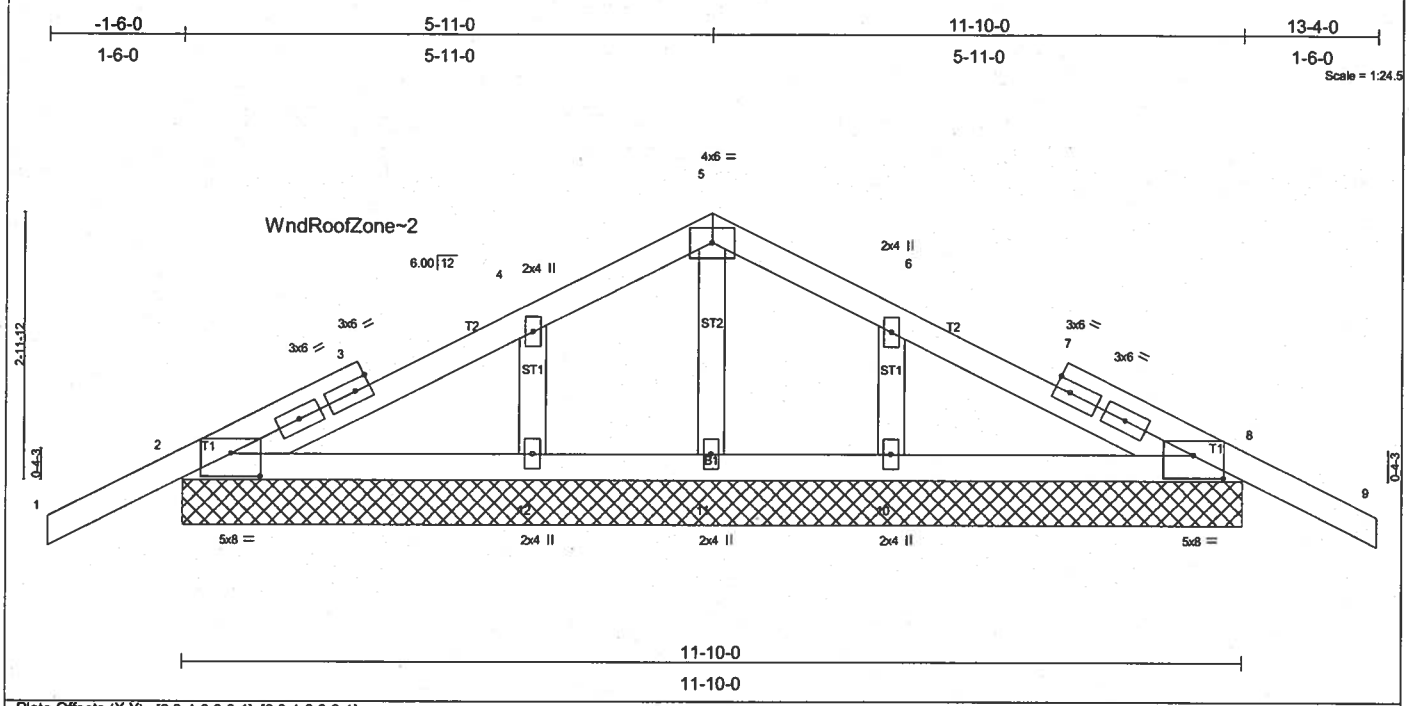

Plate Offsets (X,Y): [2:0-4-0,0-3-1], [8:0-4-0,0-3-1]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	-0.00	9	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.00	9	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.05	Horz(TL)	0.00	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 55 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=250/11-10-0, 8=250/11-10-0, 11=145/11-10-0, 12=329/11-10-0, 10=329/11-10-0
Max Horz 2=-65(load case 6)
Max Uplift2=-156(load case 5), 8=-166(load case 6), 11=-12(load case 5), 12=-131(load case 5), 10=-134(load case 6)
Max Grav 2=256(load case 9), 8=256(load case 10), 11=145(load case 1), 12=331(load case 9), 10=331(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/41, 2-3=-47/59, 3-4=-42/117, 4-5=0/80, 5-6=0/80, 6-7=-18/117, 7-8=-21/59, 8-9=0/41
BOT CHORD 2-12=-55/119, 11-12=-55/119, 10-11=-55/119, 8-10=-55/119
WEBS 5-11=-126/20, 4-12=-218/187, 6-10=-218/187

JOINT STRESS INDEX
2 = 0.61, 3 = 0.00, 3 = 0.21, 3 = 0.21, 4 = 0.10, 5 = 0.06, 6 = 0.10, 7 = 0.00, 7 = 0.21, 7 = 0.21, 8 = 0.61, 10 = 0.11, 11 = 0.05 and 12 = 0.11

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 156 lb uplift at joint 2, 166 lb uplift at joint 8, 12 lb uplift at joint 11, 131 lb uplift at joint 12 and 134 lb uplift at joint 10.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-64(F=-10), 5-9=-64(F=-10), 2-8=-30

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Dwg.#1116061027

Job	Truss	Truss Type	Qty	Ply	LOT 15 FORT WHITE STATION
L217897	T16	COMMON	1	1	Job Reference (optional)

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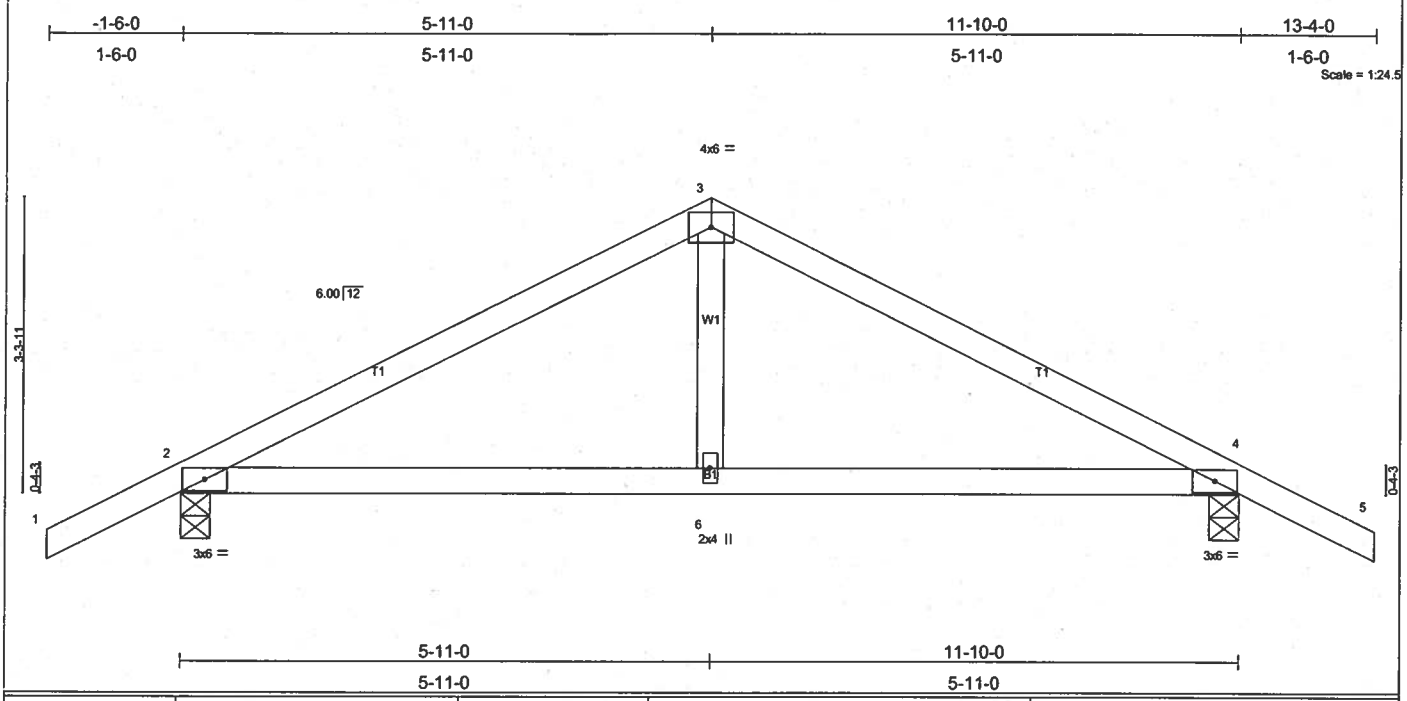

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	-0.04	2-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.28	Vert(TL)	-0.06	2-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.07	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 47 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=573/0-4-0, 4=573/0-4-0
Max Horz 2=-70(load case 6)
Max Uplift2=-256(load case 5), 4=-256(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-668/191, 3-4=-668/190, 4-5=0/35
BOT CHORD 2-6=-97/536, 4-6=-97/536
WEBS 3-6=0/212

JOINT STRESS INDEX
2 = 0.37, 3 = 0.50, 4 = 0.37 and 6 = 0.15

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 256 lb uplift at joint 2 and 256 lb uplift at joint 4.

LOAD CASE(S) Standard

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Dwg.#1116061028

Job	Truss	Truss Type	Qty	Ply	LOT 15 FORT WHITE STATION
L217897	T17	COMMON	1	1	Job Reference (optional)

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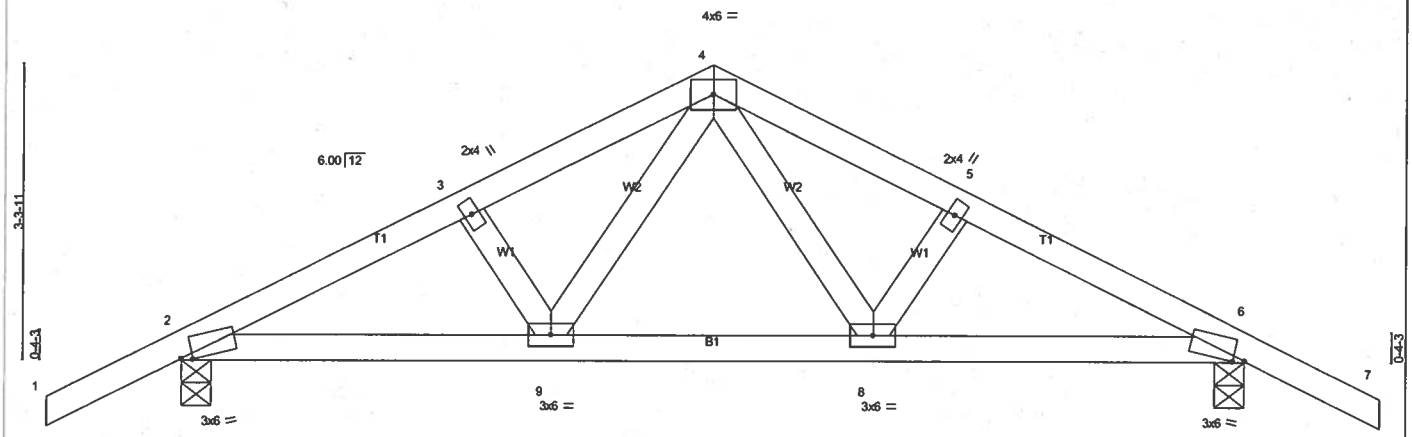
-1-6-0 | 3-2-12 | 5-11-0 | 8-7-4 | 11-10-0 | 13-4-0
1-6-0 | 3-2-12 | 2-8-4 | 2-8-4 | 3-2-12 | 1-6-0

Scale = 1:24.5



4-1-8 | 7-8-8 | 11-10-0
4-1-8 | 3-7-0 | 4-1-8

Plate Offsets (X,Y): [2:0-1-9,0-0-7], [6:0-1-9,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	-0.04	2-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	-0.07	2-9	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.22	Horz(TL)	0.02	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 57 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-10-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-6-9 oc bracing.

REACTIONS (lb/size) 2=1136/0-4-0, 6=1136/0-4-0
Max Horz 2=-70(load case 5)
Max Uplift2=-469(load case 4), 6=-469(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1576/522, 3-4=-1501/526, 4-5=-1501/526, 5-6=-1576/522, 6-7=0/35
BOT CHORD 2-9=-437/1367, 8-9=-268/972, 6-8=-405/1367
WEBS 3-9=-110/100, 4-9=-227/679, 4-8=-228/679, 5-8=-110/101

JOINT STRESS INDEX
2 = 0.83, 3 = 0.07, 4 = 0.55, 5 = 0.07, 6 = 0.83, 8 = 0.52 and 9 = 0.52

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 469 lb uplift at joint 2 and 469 lb uplift at joint 6.
5) Girder carries tie-in span(s): 7-0-0 from 0-0-0 to 11-10-0
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-6=-128(F=-98)

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Dwg.#1116061029

Job	Truss	Truss Type	Qty	Ply	LOT 15 FORT WHITE STATION
L217897	T18	SPECIAL	1	2	Job Reference (optional)

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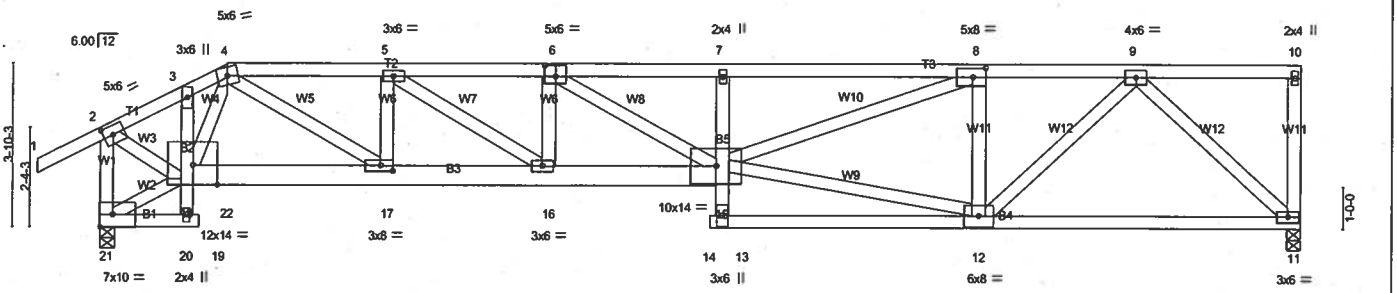
-1-6-0 | 2-11-15 | 6-9-5 | 10-6-10 | 20-7-13 | 24-5-2 | 28-2-8

1-6-0 | 2-11-15 | 3-9-6 | 3-9-6 | 10-1-2 | 3-9-6 | 3-9-6

Scale = 1:51.7
Camber = 1/4 in



3-0-0

2-2-4 | 2-4-0 | 6-9-5 | 10-6-10 | 14-4-0 | 14-5-12 | 22-10-7 | 28-2-8

2-2-4 | 0-1-12 | 3-9-5 | 3-9-6 | 3-9-6 | 0-1-12 | 8-4-11 | 5-4-1

0-8-0

Plate Offsets (X,Y): [2:0-2-11,0-2-8], [6:0-3-0,0-3-0], [8:0-3-8,0-2-8], [17:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.37	14	>894	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.99	Vert(TL)	-0.60	14	>559	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.80	Horz(TL)	0.20	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 371 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except* T3 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.2 *Except* B3 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except* W10 2 X 4 SYP No.2, W9 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 11=3311/0-4-0, 21=4905/0-4-0
Max Horz 21=265(load case 4)
Max Uplift11=-1503(load case 3), 21=-2151(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-5126/2435, 3-4=-5310/2596, 4-5=-8391/3844, 5-6=-10293/4648, 6-7=-11096/5062, 7-8=-11033/5076, 8-9=-5644/2540, 9-10=-120/15, 10-11=-245/172, 2-21=-4996/2339
BOT CHORD 20-21=-325/155, 19-20=0/0, 18-20=0/51, 3-18=-11/84, 18-22=-2380/4911, 17-22=-2380/4911, 16-17=-3844/8390, 15-16=-4674/10344, 13-15=0/238, 7-15=-550/409, 13-14=0/0, 12-13=-431/1120, 11-12=-1441/3059
WEBS 4-18=-344/66, 4-17=-1760/4154, 5-17=-1599/735, 5-16=-975/2274, 6-16=-868/454, 6-15=-473/888, 8-15=-2741/5826, 12-15=-2143/4597, 8-12=-2872/1556, 9-12=-1524/3582, 9-11=-4074/1976, 2-18=-2301/5014, 18-21=-447/452

JOINT STRESS INDEX
2 = 0.80, 3 = 0.31, 4 = 0.73, 5 = 0.66, 6 = 0.67, 7 = 0.43, 8 = 0.65, 9 = 0.80, 10 = 0.85, 11 = 0.74, 12 = 0.83, 13 = 0.61, 15 = 0.70, 16 = 0.66, 17 = 0.91, 18 = 0.57, 20 = 0.37 and 21 = 0.31

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc, 2 X 6 - 2 rows at 0-6-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
4) Provide adequate drainage to prevent water ponding.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1503 lb uplift at joint 11 and 2151 lb uplift at joint 21.
7) Girder carries tie-in span(s): 4-8-10 from 3-0-0 to 14-4-0; 4-4-15 from 3-0-0 to 14-4-0
8) Girder carries hip end with 25-2-8 right side setback, 0-0-0 left side setback, and 7-0-0 end setback.
9) Girder carries hip end with 0-0-0 right side setback, 14-4-0 left side setback, and 7-0-0 end setback.
10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1941 lb down and 999 lb up at 3-0-0, and 1104 lb down and 568 lb up at 14-4-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-4=-117(F=-63), 4-7=-104(F=-50), 7-10=-117(F=-63), 20-21=-65(F=-35), 19-20=-65(F=-35), 18-22=-65(F=-35), 15-22=-74(F=-44), 13-14=-109(F=-79), 11-13=-65(F=-35)
Concentrated Loads (lb)
Vert: 15=-1104(F) 22=-1941(F)

NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1116061030

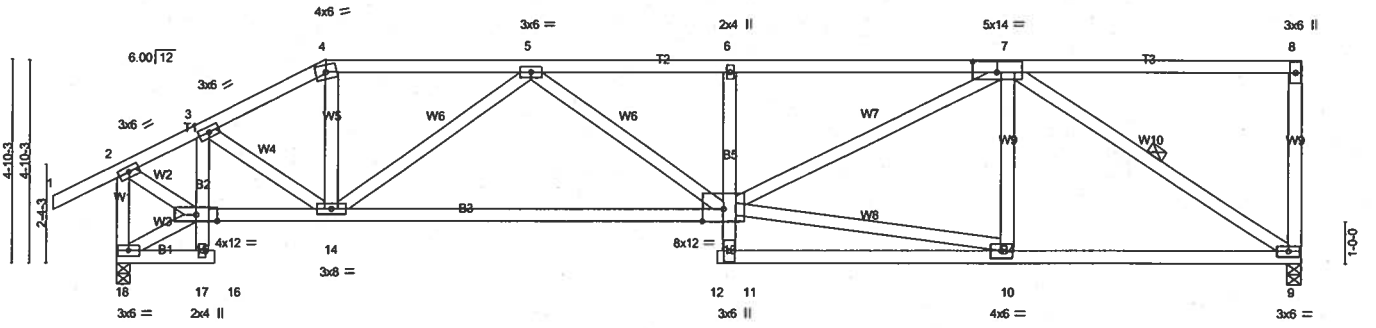
Job	Truss	Truss Type	Qty	Ply	LOT 15 FORT WHITE STATION
L217897	T19	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 16 07:53:43 2006 Page 1

-1-6-0 | 2-4-0 | 5-0-0 | 9-10-10 | 14-4-0 | 21-3-4 | 28-2-8

1-6-0 | 2-4-0 | 2-8-0 | 4-10-10 | 4-5-6 | 6-11-4 | 6-11-4

Scale = 1:52.4
Camber = 3/16 in



2-2-4

1-3-12 | 2-4-0 | 4-11-15 | 5-0-0 | 14-4-0 | 14-5-12 | 21-3-4 | 28-2-8

1-3-12 | 0-1-12 | 2-7-15 | 0-0-1 | 9-4-0 | 0-1-12 | 6-9-8 | 6-11-4

0-10-8

Plate Offsets (X,Y): [7:0-7-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.26	13-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.44	13-14	>766	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.54	Horz(TL)	0.11	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 178 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3, B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-11-3 oc bracing.
WEBS 1 Row at midpt 7-9

REACTIONS (lb/size) 9=1175/0-4-0, 18=1276/0-4-0
Max Horz 18=215(load case 5)
Max Uplift9=-445(load case 4), 18=-383(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1094/428, 3-4=-1588/565, 4-5=-1426/534, 5-6=-2395/881, 6-7=-2357/886, 7-8=-54/21, 8-9=-171/116, 2-18=-1206/535
BOT CHORD 17-18=-34/84, 16-17=0/0, 15-17=-12/39, 3-15=-592/144, 14-15=-448/953, 13-14=-827/2074, 11-13=0/111, 6-13=-303/209, 11-12=0/0, 10-11=-69/74, 9-10=-529/1399
WEBS 3-14=-171/545, 4-14=-113/459, 5-14=-810/392, 5-13=-90/401, 10-13=-465/1341, 7-13=-405/1086, 7-10=-1/124, 7-9=-1620/613, 15-18=-154/0, 2-15=-366/1089

JOINT STRESS INDEX
2 = 0.72, 3 = 0.41, 4 = 0.48, 5 = 0.35, 6 = 0.34, 7 = 0.56, 8 = 0.41, 9 = 0.58, 10 = 0.58, 11 = 0.45, 13 = 0.41, 14 = 0.57, 15 = 0.44, 17 = 0.34 and 18 = 0.39

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 445 lb uplift at joint 9 and 383 lb uplift at joint 18.

LOAD CASE(S) Standard

NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

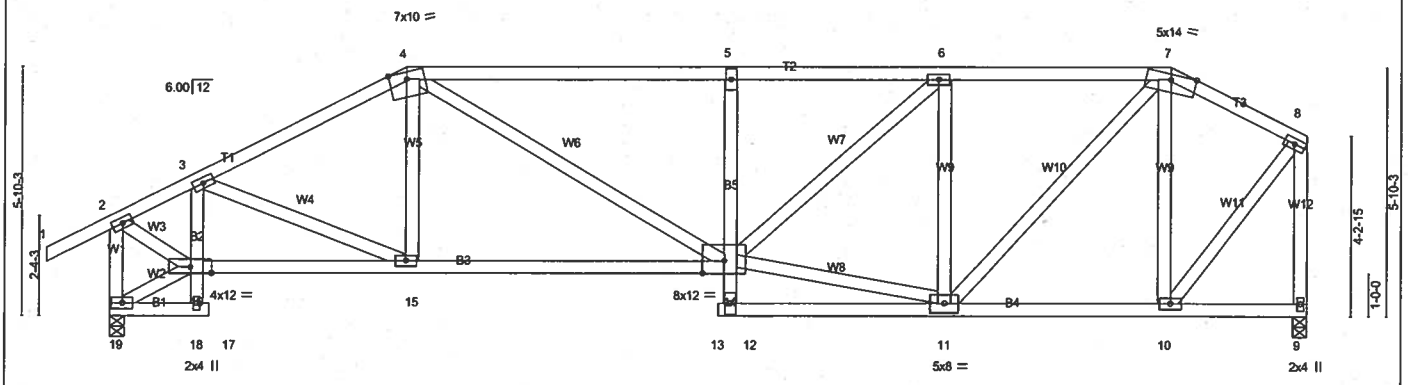
Dwg.#1116061031

Job	Truss	Truss Type	Qty	Ply	LOT 15 FORT WHITE STATION
L217897	T20	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 16 07:53:31 2006 Page 1

-1-6-0	2-2-4	7-0-0	14-4-0	19-8-0	25-0-0	28-2-8
1-6-0	2-2-4	4-9-12	7-4-0	5-4-0	5-4-0	3-2-8

Scale = 1:51.7
Camber = 1/8 in



2-2-4	2-4-0	7-0-0	14-4-0	14-5-12	19-8-0	25-0-0	28-2-8
2-2-4	0-1-12	4-8-0	7-4-0	0-1-12	5-2-4	5-4-0	3-2-8

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	-0.15	14-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.41	Vert(TL)	-0.24	14-15	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.44	Horz(TL)	0.07	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 191 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3, B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-11 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
9-2-9 oc bracing: 15-16
8-11-2 oc bracing: 14-15.

REACTIONS (lb/size) 9=1175/0-4-0, 19=1276/0-4-0
Max Horz 19=211(load case 5)
Max Uplift9=-361(load case 3), 19=-406(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1121/411, 3-4=-1618/545, 4-5=-1875/695, 5-6=-1831/676, 6-7=-1273/471, 7-8=-724/243, 2-19=-1227/514, 8-9=-1136/366
BOT CHORD 18-19=-51/35, 17-18=0/0, 16-18=-14/35, 3-16=-496/139, 15-16=-472/1014, 14-15=-502/1409, 12-14=0/88, 5-14=-359/253, 12-13=0/0, 11-12=-73/83, 10-11=-197/608, 9-10=-6/10
WEBS 3-15=-201/476, 4-15=0/113, 4-14=-263/618, 11-14=-385/1214, 6-14=-276/754, 6-11=-784/376, 7-11=-365/974, 7-10=-633/283, 2-16=-334/1124, 8-10=-329/982, 16-19=-122/5

JOINT STRESS INDEX
2 = 0.74, 3 = 0.50, 4 = 0.67, 5 = 0.33, 6 = 0.46, 7 = 0.49, 8 = 0.80, 9 = 0.42, 10 = 0.70, 11 = 0.59, 12 = 0.30, 14 = 0.29, 15 = 0.35, 16 = 0.31, 18 = 0.34 and 19 = 0.40

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are 3x6 MT20 unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 361 lb uplift at joint 9 and 406 lb uplift at joint 19.

LOAD CASE(S) Standard

NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1116061032

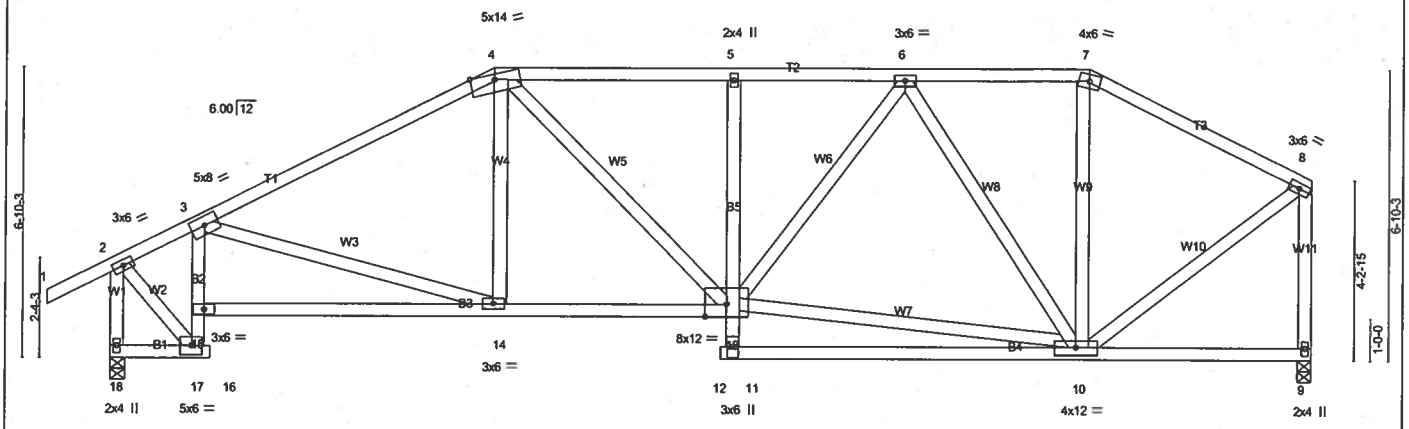
Job	Truss	Truss Type	Qty	Ply	LOT 15 FORT WHITE STATION
L217897	T21	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 16 07:53:20 2006 Page 1

-1-6-0 | 2-2-4 | 9-0-0 | 14-5-12 | 18-8-0 | 23-0-0 | 28-2-8

1-6-0 | 2-2-4 | 6-9-12 | 5-5-12 | 4-2-4 | 4-4-0 | 5-2-8

Scale = 1:51.7
Camber = 1/16 in



2-2-4 | 2-4-0 | 9-0-0 | 14-4-0 | 14-5-12 | 23-0-0 | 28-2-8

2-2-4 | 0-1-12 | 6-8-0 | 5-4-0 | 0-1-12 | 8-6-4 | 5-2-8

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.13	14-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.97	Vert(TL)	-0.21	14-15	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.81	Horz(TL)	0.13	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 195 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 18=1276/0-4-0, 9=1175/0-4-0
Max Horz 18=225(load case 5)
Max Uplift18=-423(load case 5), 9=-313(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-871/315, 3-4=-1581/497, 4-5=-1493/528, 5-6=-1477/525, 6-7=-801/308, 7-8=-949/301, 2-18=-1414/518, 8-9=-1110/350
BOT CHORD 17-18=-162/15, 16-17=0/0, 15-17=-611/151, 3-15=-522/180, 14-15=-542/1090, 13-14=-433/1342, 11-13=0/139, 5-13=-273/193, 11-12=0/0, 10-11=-25/252, 9-10=-21/32
WEBS 3-14=-156/345, 4-14=-13/167, 4-13=-168/325, 10-13=-375/941, 6-13=-149/504, 6-10=-758/331, 7-10=-20/207, 2-17=-312/1094, 8-10=-274/961

JOINT STRESS INDEX
2 = 0.87, 3 = 0.72, 4 = 0.95, 5 = 0.47, 6 = 0.43, 7 = 0.47, 8 = 0.77, 9 = 0.60, 10 = 0.71, 11 = 0.72, 13 = 0.24, 14 = 0.35, 15 = 0.40, 17 = 0.70 and 18 = 0.58

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 423 lb uplift at joint 18 and 313 lb uplift at joint 9.

LOAD CASE(S) Standard

NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1116061033

Job	Truss	Truss Type	Qty	Ply	LOT 15 FORT WHITE STATION
L217897	T22	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 16 07:53:02 2006 Page 1

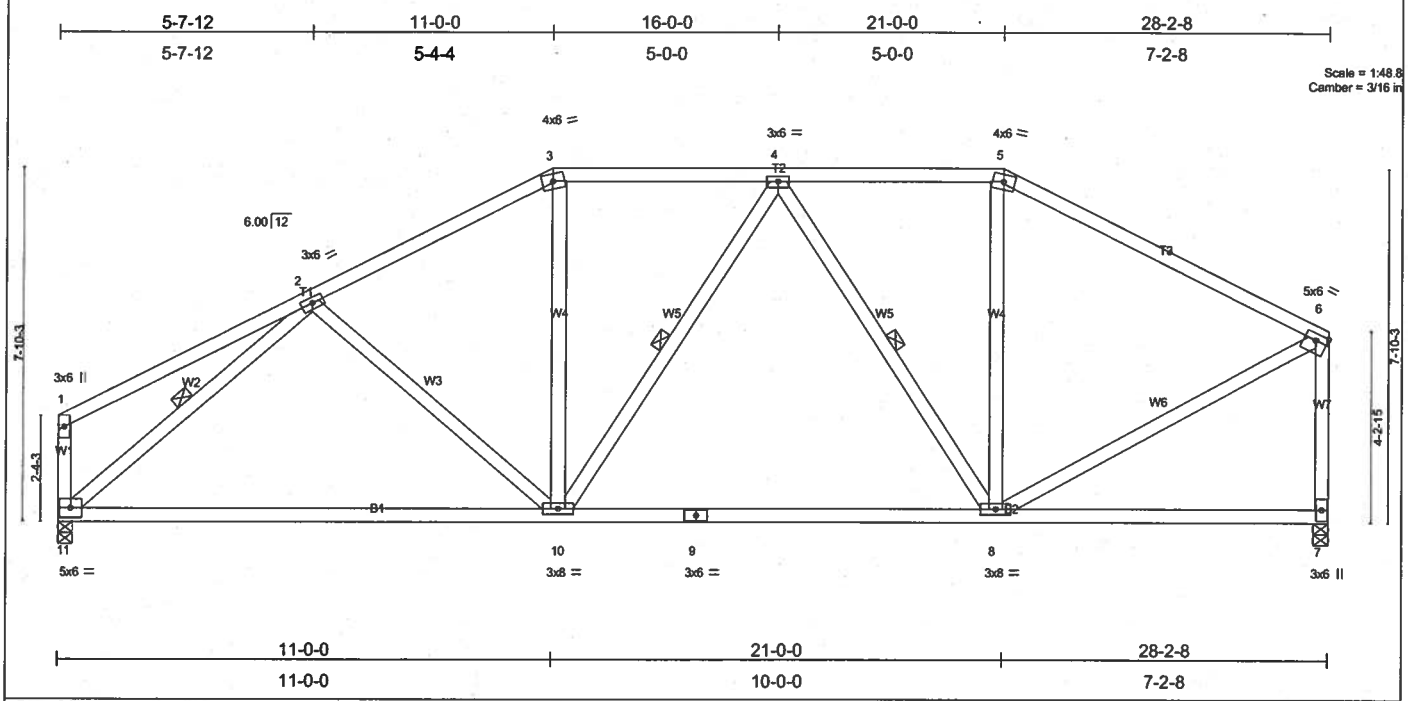


Plate Offsets (X,Y): [6:Edge,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.86	Vert(LL)	-0.22	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.57	Vert(TL)	-0.38	10-11	>873	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.32	Horz(TL)	0.04	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 176 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W1 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-5-11 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-5-3 oc bracing.
WEBS 1 Row at midpt 4-10, 4-8, 2-11

REACTIONS (lb/size) 7=1172/0-4-0, 11=1172/0-4-0
Max Horz 11=186(load case 5)
Max Uplift7=-332(load case 6), 11=-348(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-299/129, 2-3=-1275/420, 3-4=-1088/422, 4-5=-878/354, 5-6=-1064/337, 1-11=-258/150, 6-7=-1074/349
BOT CHORD 10-11=-442/1053, 9-10=-312/1057, 8-9=-312/1057, 7-8=-41/69
WEBS 2-10=-35/178, 3-10=-25/260, 4-10=-59/122, 4-8=-410/198, 5-8=-1/197, 6-8=-232/924, 2-11=-1132/346

JOINT STRESS INDEX
1 = 0.49, 2 = 0.41, 3 = 0.56, 4 = 0.43, 5 = 0.82, 6 = 0.65, 7 = 0.35, 8 = 0.87, 9 = 0.46, 10 = 0.59 and 11 = 0.64

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 332 lb uplift at joint 7 and 348 lb uplift at joint 11.

LOAD CASE(S) Standard

NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1116061034

Job	Truss	Truss Type	Qty	Ply	LOT 15 FORT WHITE STATION
L217897	T23	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 16 07:58:51 2006 Page 1

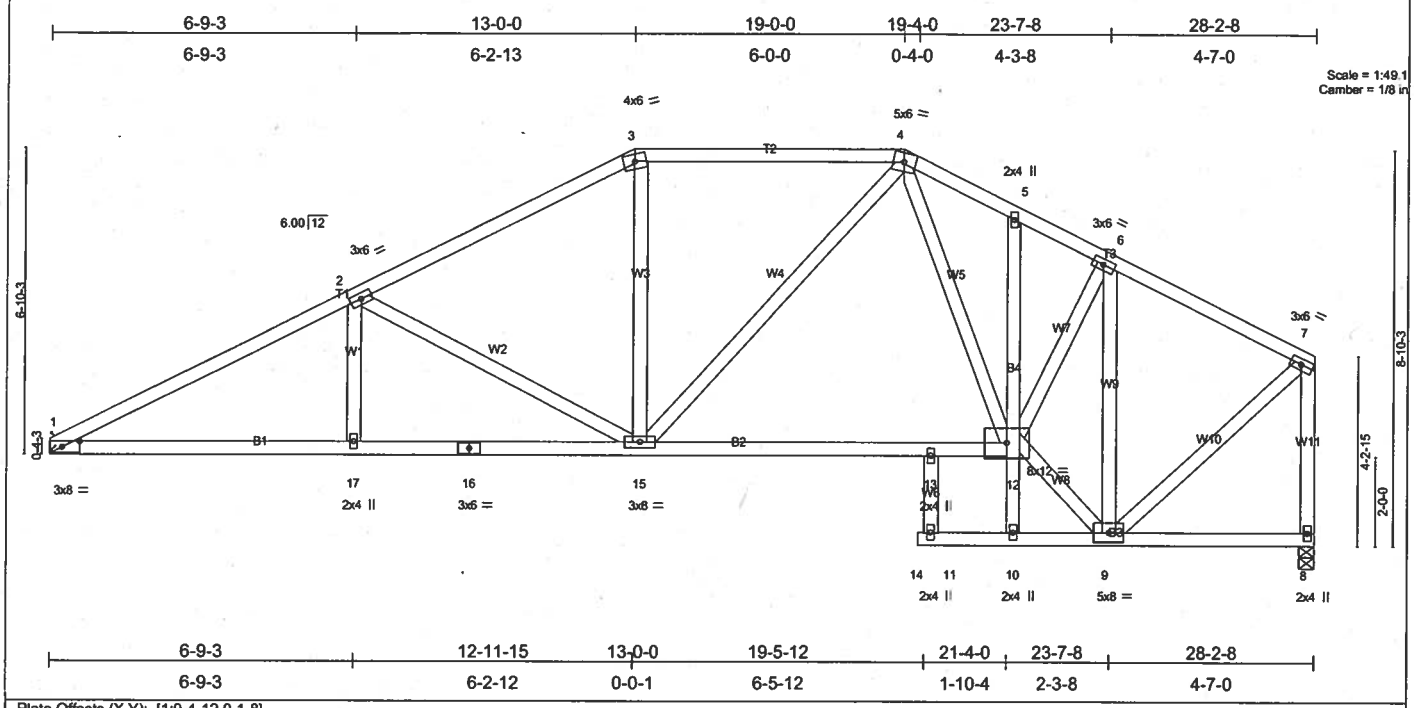


Plate Offsets (X,Y): [1:0-4-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	-0.16	13-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.58	Vert(TL)	-0.26	13-15	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.87	Horz(TL)	0.10	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 184 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
JOINTS 1 Brace at Jt(s): 12

REACTIONS (lb/size) 1=1175/Mechanical, 8=1179/0-4-0
Max Horz 1=156(load case 5)
Max Uplift1=-372(load case 5), 8=-345(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2130/651, 2-3=-1515/486, 3-4=-1291/492, 4-5=-1292/466, 5-6=-1324/430, 6-7=-863/272, 7-8=-1107/353
BOT CHORD 1-17=-649/1829, 16-17=-649/1829, 15-16=-649/1829, 13-15=-261/1115, 12-13=-261/1115, 10-12=0/121, 5-12=-37/56, 11-14=0/0, 10-11=0/0, 9-10=-57/0, 8-9=-15/31
WEBS 2-17=0/218, 2-15=-621/345, 3-15=-45/313, 4-15=-127/345, 9-12=-220/1055, 6-12=-172/921, 6-9=-1258/335, 7-9=-234/933, 4-12=-75/144, 11-13=-20/0

JOINT STRESS INDEX
1 = 0.76, 2 = 0.41, 3 = 0.71, 4 = 0.36, 5 = 0.34, 6 = 0.78, 7 = 0.70, 8 = 0.64, 9 = 0.51, 10 = 0.73, 11 = 0.34, 12 = 0.83, 13 = 0.34, 15 = 0.57, 16 = 0.65 and 17 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 372 lb uplift at joint 1 and 345 lb uplift at joint 8.

LOAD CASE(S) Standard

NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1116061035

Job	Truss	Truss Type	Qty	Ply	LOT 15 FORT WHITE STATION
L217897	T24	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 16 08:00:44 2006 Page 1

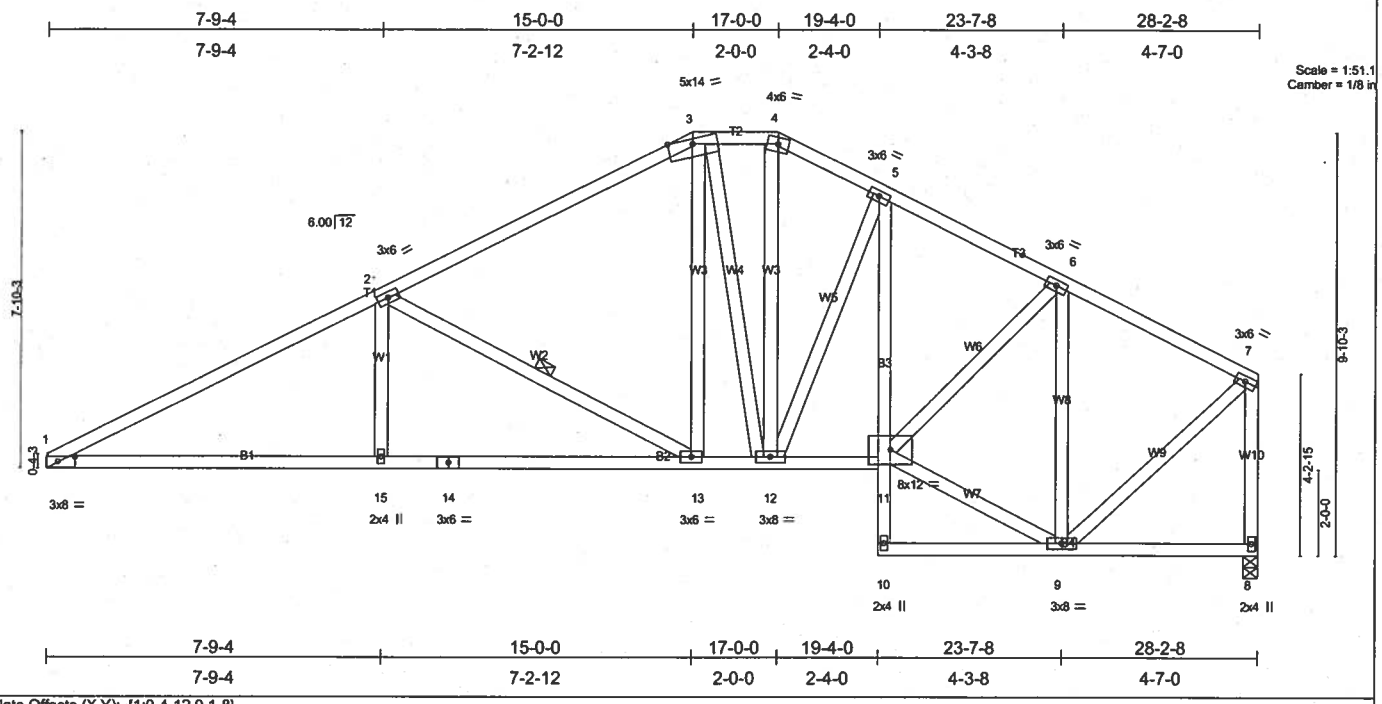


Plate Offsets (X,Y): [1:0-4-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.18	1-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.67	Vert(TL)	-0.30	1-15	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.60	Horz(TL)	0.09	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 198 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
7-4-5 oc bracing: 1-15
7-9-1 oc bracing: 13-15.
WEBS 1 Row at midpt 2-13

REACTIONS (lb/size) 1=1176/Mechanical, 8=1176/0-4-0
Max Horz 1=170(load case 5)
Max Uplift1=-385(load case 5), 8=-361(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2082/661, 2-3=-1335/462, 3-4=-1054/443, 4-5=-1204/478, 5-6=-1344/464, 6-7=-863/285, 7-8=-1108/368
BOT CHORD 1-15=-662/1800, 14-15=-662/1800, 13-14=-662/1800, 12-13=-303/1116, 11-12=-299/1157, 10-11=0/62, 5-11=-46/166, 9-10=-7/36, 8-9=-14/29
WEBS 2-15=0/277, 2-13=-788/410, 3-13=-130/514, 3-12=-358/159, 4-12=-250/482, 5-12=-305/194, 9-11=-218/783, 6-11=-147/593, 6-9=-872/309, 7-9=-251/940

JOINT STRESS INDEX
1 = 0.75, 2 = 0.41, 3 = 0.77, 4 = 0.28, 5 = 0.46, 6 = 0.45, 7 = 0.69, 8 = 0.61, 9 = 0.92, 10 = 0.51, 11 = 0.77, 12 = 0.65, 13 = 0.35, 14 = 0.66 and 15 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 385 lb uplift at joint 1 and 361 lb uplift at joint 8.

LOAD CASE(S) Standard

NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1116061036

Job	Truss	Truss Type	Qty	Ply	LOT 15 FORT WHITE STATION
L217897	T25	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 16 08:01:25 2006 Page 1

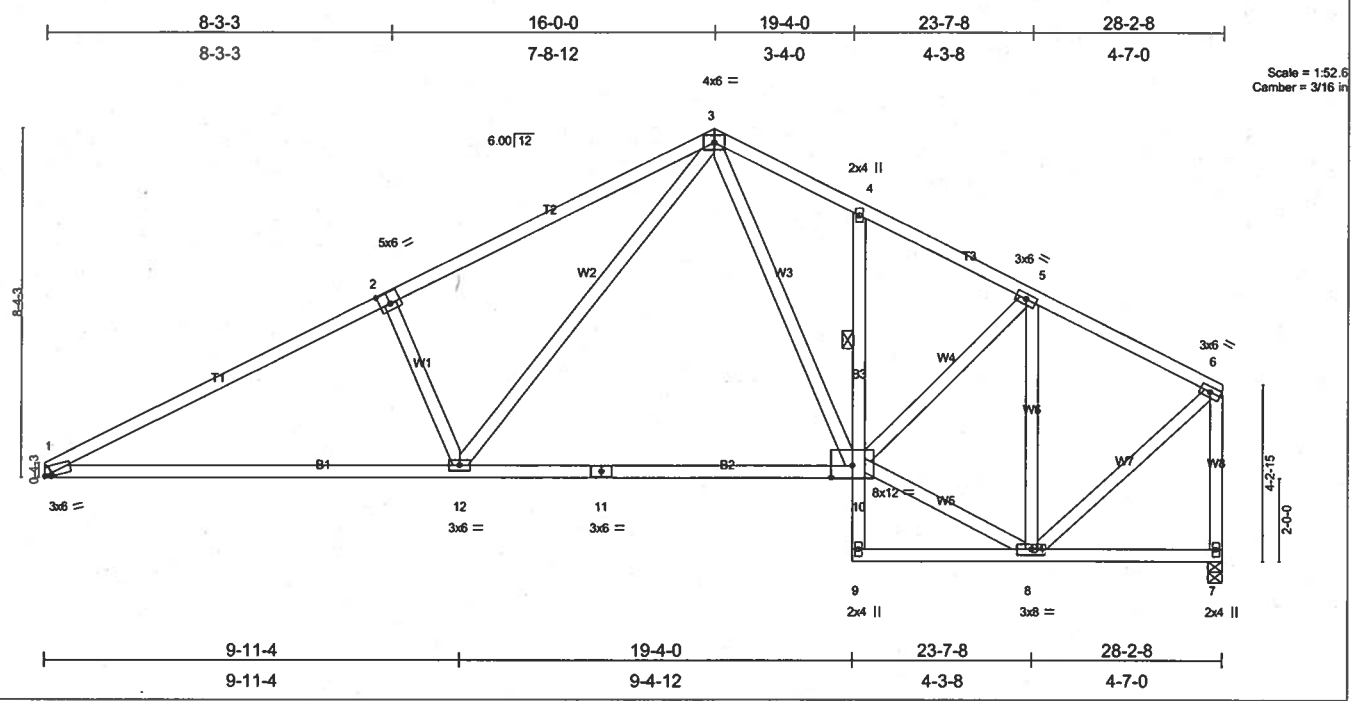


Plate Offsets (X,Y): [1:0-1-12,0-0-7], [2:0-3-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	-0.31	1-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.87	Vert(TL)	-0.52	1-12	>644	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.69	Horz(TL)	0.08	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 172 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
1 Row at midpt 4-10

REACTIONS (lb/size) 1=1172/Mechanical, 7=1172/0-4-0
Max Horz 1=177(load case 5)
Max Uplift1=-388(load case 5), 7=-367(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1992/656, 2-3=-1844/713, 3-4=-1288/532, 4-5=-1316/458, 5-6=-862/289, 6-7=-1106/373
BOT CHORD 1-12=-659/1718, 11-12=-285/1009, 10-11=-285/1009, 9-10=0/54, 4-10=-201/145, 8-9=-47/0, 7-8=-14/28
WEBS 2-12=-425/375, 3-12=-381/915, 3-10=-167/438, 8-10=-205/851, 5-10=-133/570, 5-8=-864/300, 6-8=-258/941

JOINT STRESS INDEX
1 = 0.82, 2 = 0.79, 3 = 0.73, 4 = 0.34, 5 = 0.43, 6 = 0.70, 7 = 0.59, 8 = 0.93, 9 = 0.34, 10 = 0.35, 11 = 0.47 and 12 = 0.65

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 388 lb uplift at joint 1 and 367 lb uplift at joint 7.

LOAD CASE(S) Standard

NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1116061037

Job	Truss	Truss Type	Qty	Ply	LOT 15 FORT WHITE STATION
L217897	T26	COMMON	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 16 08:03:04 2006 Page 1

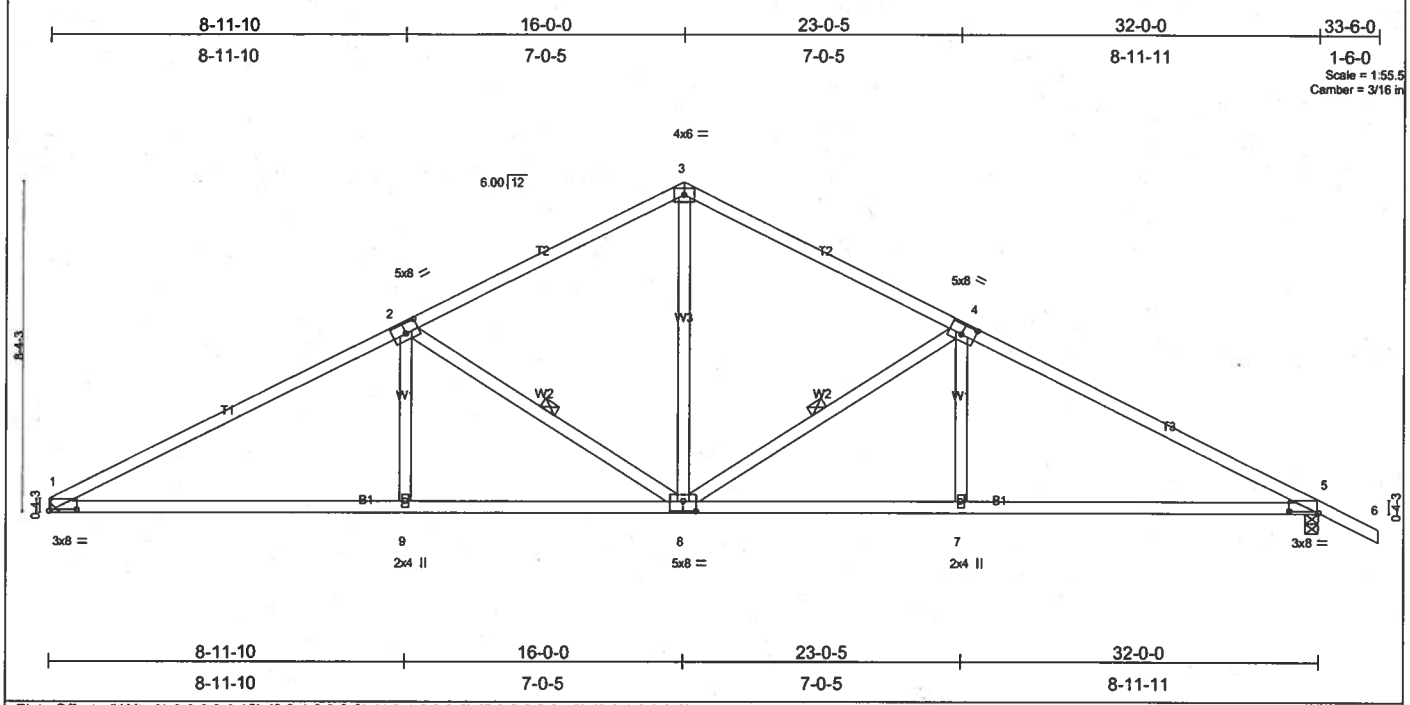


Plate Offsets (X,Y): [1:0-8-8,0-0-10], [2:0-4-0,0-3-0], [4:0-4-0,0-3-0], [5:0-8-8,0-0-10], [8:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.74	Vert(LL)	-0.31	1-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.80	Vert(TL)	-0.50	1-9	>760	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.34	Horz(TL)	0.10	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 152 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-9-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-3-10 oc bracing.
WEBS 1 Row at midpt 2-8, 4-8

REACTIONS (lb/size) 1=1328/Mechanical, 5=1422/0-4-0
Max Horz 1=-159(load case 6)
Max Uplift1=-431(load case 5), 5=-529(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2340/731, 2-3=-1589/567, 3-4=-1589/570, 4-5=-2329/713, 5-6=0/35
BOT CHORD 1-9=-632/1997, 8-9=-631/2001, 7-8=-473/1990, 5-7=-474/1986
WEBS 2-9=0/299, 2-8=-808/420, 3-8=-298/1003, 4-8=-793/402, 4-7=0/292

JOINT STRESS INDEX
1 = 0.76, 2 = 0.64, 3 = 0.55, 4 = 0.64, 5 = 0.77, 7 = 0.34, 8 = 0.44 and 9 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 431 lb uplift at joint 1 and 529 lb uplift at joint 5.

LOAD CASE(S) Standard

NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

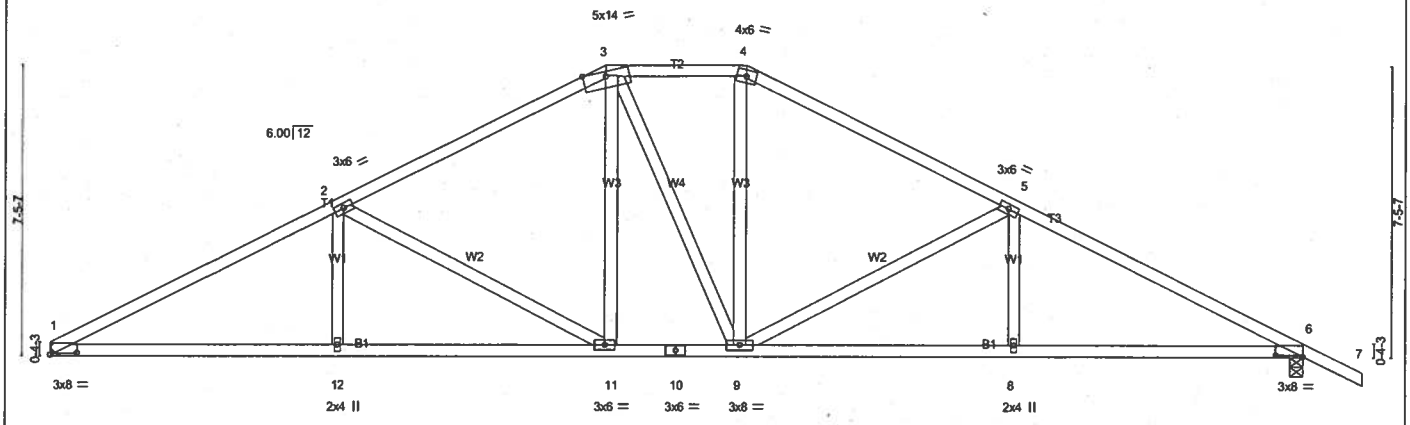
Dwg.#1116061038

Job	Truss	Truss Type	Qty	Ply	LOT 15 FORT WHITE STATION
L217897	T27	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 16 08:03:14 2006 Page 1

7-4-7	14-2-7	17-9-8	24-7-8	32-0-0	33-6-0
7-4-7	6-10-0	3-7-1	6-10-0	7-4-8	1-6-0

Scale = 1:56.3
Camber = 1/8 in



7-4-7	14-2-7	17-9-8	24-7-8	32-0-0
7-4-7	6-10-0	3-7-1	6-10-0	7-4-8

Plate Offsets (X,Y): [1:0-8-4,0-0-10], [6:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	-0.17	11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.66	Vert(TL)	-0.28	11-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.78	Horz(TL)	0.10	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 167 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-5-15 oc bracing.

REACTIONS (lb/size) 1=1328/Mechanical, 6=1422/0-4-0
Max Horz 1=-146(load case 6)
Max Uplift1=-421(load case 5), 6=-519(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2438/745, 2-3=-1733/564, 3-4=-1478/562, 4-5=-1733/558, 5-6=-2422/721, 6-7=0/35
BOT CHORD 1-12=-652/2098, 11-12=-652/2098, 10-11=-319/1476, 9-10=-319/1476, 8-9=-500/2081, 6-8=-500/2081
WEBS 2-12=0/260, 2-11=-717/380, 3-11=-144/473, 3-9=-165/172, 4-9=-140/470, 5-9=-697/357, 5-8=0/250

JOINT STRESS INDEX
1 = 0.79, 2 = 0.41, 3 = 0.69, 4 = 0.64, 5 = 0.41, 6 = 0.76, 8 = 0.34, 9 = 0.64, 10 = 0.54, 11 = 0.35 and 12 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 421 lb uplift at joint 1 and 519 lb uplift at joint 6.

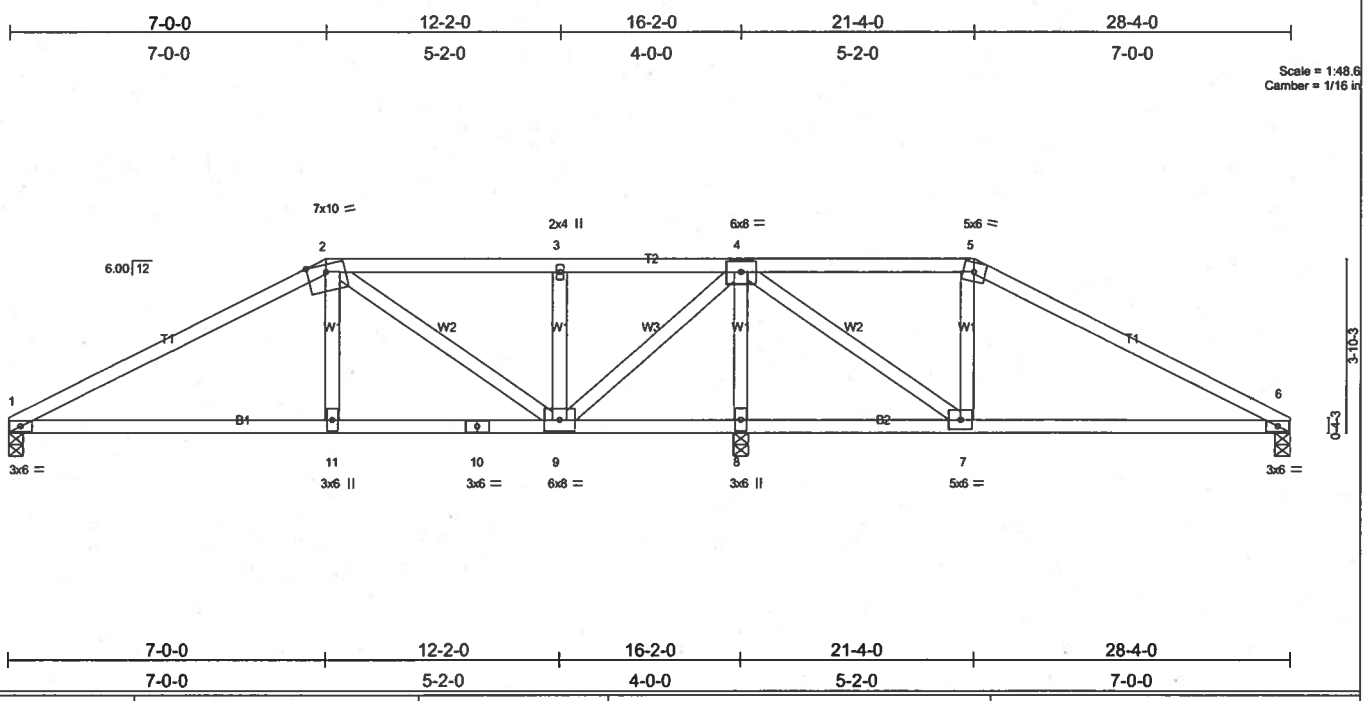
LOAD CASE(S) Standard

NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1116061039

Job	Truss	Truss Type	Qty	Ply	LOT 15 FORT WHITE STATION
L217897	T28	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 16 08:04:46 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.56	Vert(LL)	0.15	1-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.63	Vert(TL)	-0.18	1-11	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.76	Horz(TL)	0.03	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 131 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=1025/0-4-0, 6=563/0-4-0, 8=3237/0-4-0
Max Horz 1=52(load case 3)
Max Uplift1=-605(load case 4), 6=-224(load case 5), 8=-1609(load case 3)
Max Grav 1=1031(load case 8), 6=577(load case 9), 8=3237(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1802/992, 2-3=-728/498, 3-4=-727/499, 4-5=-667/352, 5-6=-812/320
BOT CHORD 1-11=-861/1553, 10-11=-877/1588, 9-10=-877/1588, 8-9=-817/455, 7-8=-817/455, 6-7=-197/674
WEBS 2-11=-414/855, 2-9=-1051/610, 3-9=-543/440, 4-9=-1130/2042, 4-8=-2927/1599, 4-7=-787/1782, 5-7=-194/223

JOINT STRESS INDEX
1 = 0.76, 2 = 0.88, 3 = 0.34, 4 = 0.75, 5 = 0.73, 6 = 0.63, 7 = 0.57, 8 = 0.52, 9 = 0.75, 10 = 0.70 and 11 = 0.28

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 605 lb uplift at joint 1, 224 lb uplift at joint 6 and 1609 lb uplift at joint 8.
6) Girder carries hip end with 7-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 21-4-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-5=-117(F=-63), 5-6=-54, 1-11=-30, 7-11=-65(F=-35), 6-7=-30
Concentrated Loads (lb)
Vert: 11=-539(F) 7=-539(F)

NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1116061040

Job	Truss	Truss Type	Qty	Ply	LOT 15 FORT WHITE STATION
L217897	T29	MONO HIP	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 16 08:10:45 2006 Page 1

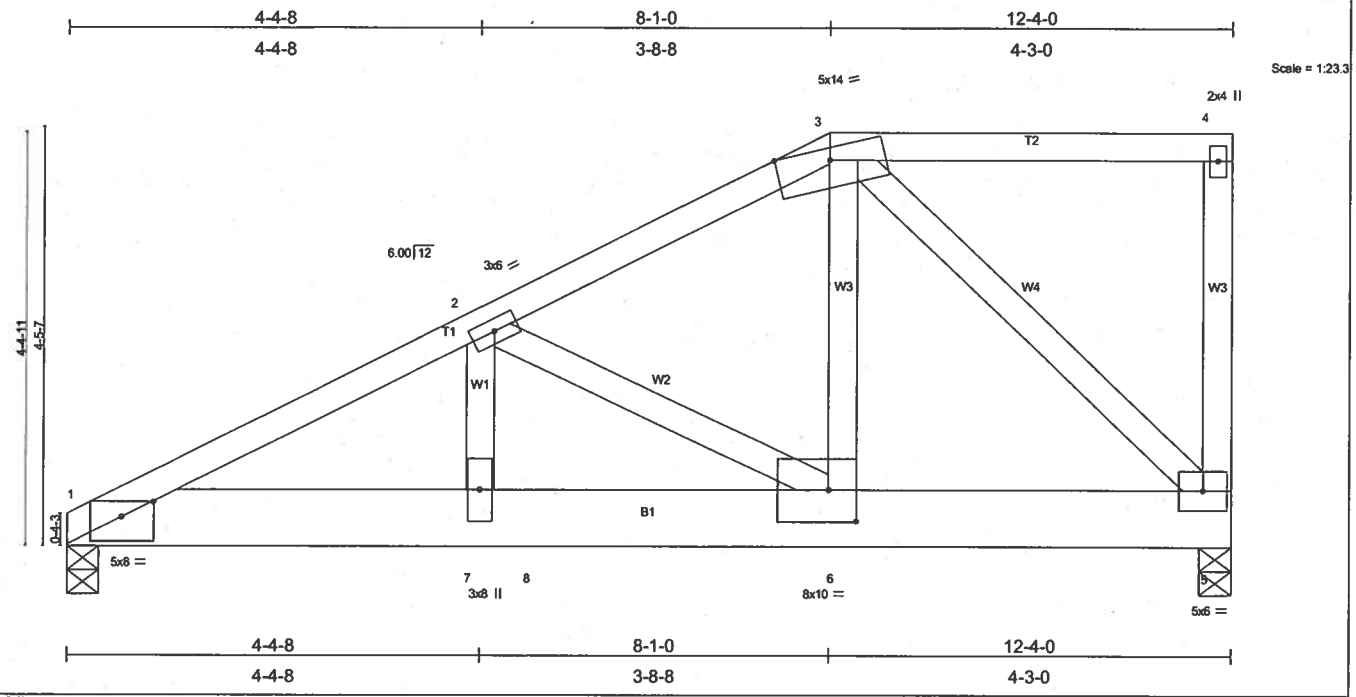


Plate Offsets (X,Y): [1:0-4-0,0-1-15], [6:0-3-8,0-4-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	-0.06	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.31	Vert(TL)	-0.10	6-7	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.62	Horz(TL)	0.02	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 168 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-4-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=4084/0-4-0, 5=3861/0-4-0
Max Horz 1=182(load case 4)
Max Uplift1=-1502(load case 4), 5=-1440(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-5995/2179, 2-3=-3112/1131, 4-5=-122/83, 3-4=-70/31
BOT CHORD 1-7=-2083/5346, 7-8=-2083/5346, 6-8=-2083/5346, 5-6=-1099/2889
WEBS 2-7=-859/2440, 2-6=-2952/1176, 3-6=-1417/3872, 3-5=-3905/1488

JOINT STRESS INDEX
1 = 0.67, 2 = 0.90, 3 = 0.64, 4 = 0.53, 5 = 0.42, 6 = 0.40 and 7 = 0.40

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
4) Provide adequate drainage to prevent water ponding.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1502 lb uplift at joint 1 and 1440 lb uplift at joint 5.
7) Girder carries tie-in span(s): 32-0-0 from 0-0-0 to 5-0-0; 28-4-0 from 5-0-0 to 12-4-0

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 1-8=-653(F=-623), 5-8=-576(F=-546), 3-4=-54

NOVEMBER16, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1116061041

Job	Truss	Truss Type	Qty	Ply	LOT 15 FORT WHITE STATION
L217897	T30	GABLE	1	1	Job Reference (optional)

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-1-6-0 | 4-10-0 | 9-8-0 | 11-2-0
1-6-0 | 4-10-0 | 4-10-0 | 1-6-0

Scale = 1:20.9

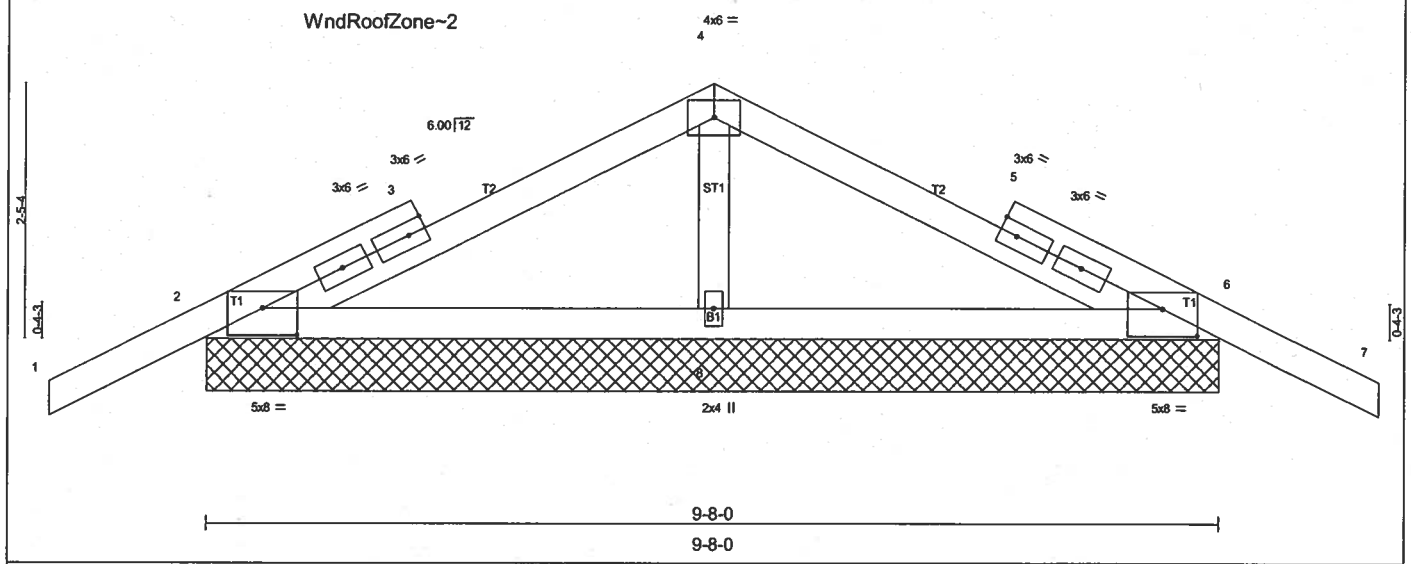

Plate Offsets (X,Y): [2:0-4-0,0-3-1], [6:0-4-0,0-3-1]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	0.01	7	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.12	Vert(TL)	0.01	7	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.08	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 43 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 9-8-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=238/9-8-0, 6=238/9-8-0, 8=626/9-8-0
Max Horz 2=58(load case 5)
Max Uplift2=-155(load case 5), 6=-165(load case 6), 8=-183(load case 5)
Max Grav 2=260(load case 9), 6=260(load case 10), 8=626(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/41, 2-3=-67/138, 3-4=-60/218, 4-5=-58/218, 5-6=-66/138, 6-7=0/41
BOT CHORD 2-8=-130/170, 6-8=-130/170
WEBS 4-8=-455/298

JOINT STRESS INDEX
2 = 0.70, 3 = 0.00, 3 = 0.24, 3 = 0.25, 4 = 0.64, 5 = 0.00, 5 = 0.25, 5 = 0.24, 6 = 0.70 and 8 = 0.17

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 155 lb uplift at joint 2, 165 lb uplift at joint 6 and 183 lb uplift at joint 8.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-64(F=-10), 4-7=-64(F=-10), 2-6=-30

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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1116061042

Job	Truss	Truss Type	Qty	Ply	LOT 15 FORT WHITE STATION
L217897	T31	COMMON	1	1	Job Reference (optional)

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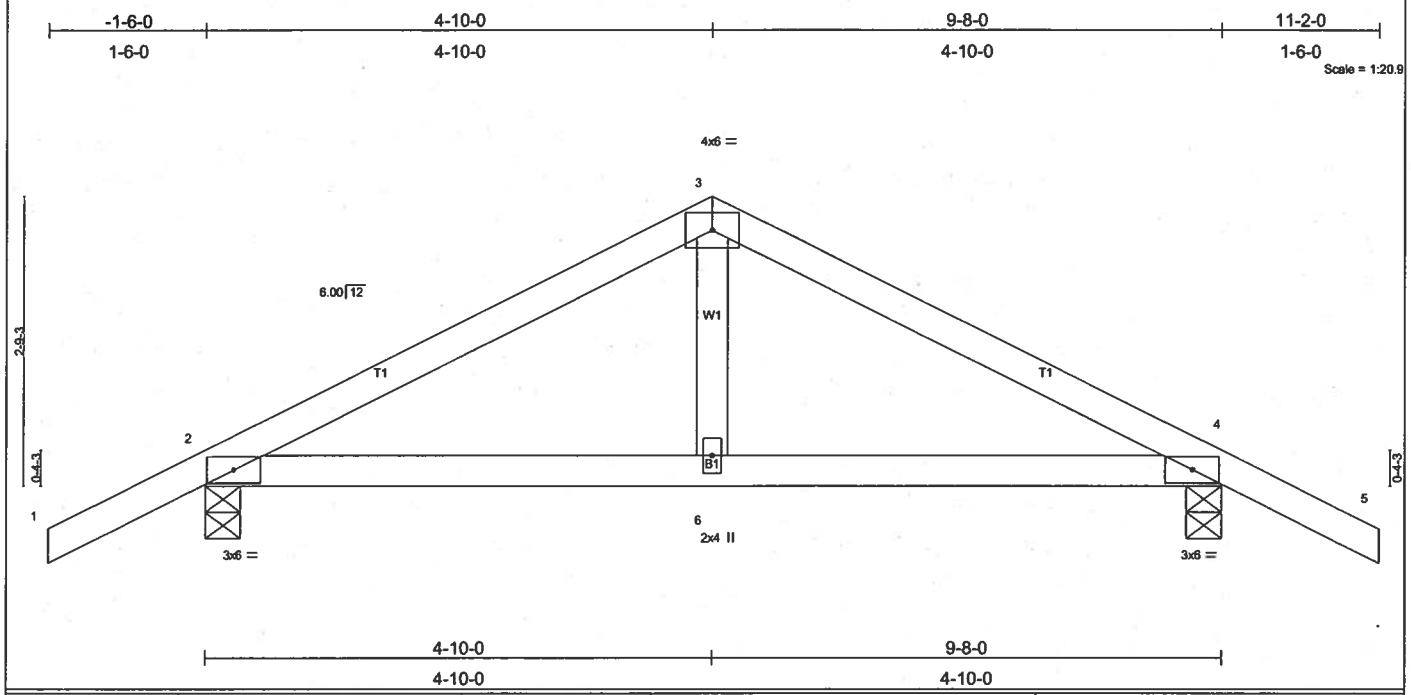

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	0.03	4-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.03	4-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 39 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=482/0-4-0, 4=482/0-4-0
Max Horz 2=62(load case 5)
Max Uplift2=-345(load case 5), 4=-345(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-521/458, 3-4=-521/458, 4-5=0/35
BOT CHORD 2-6=-296/413, 4-6=-296/413
WEBS 3-6=-269/162

JOINT STRESS INDEX
2 = 0.47, 3 = 0.34, 4 = 0.47 and 6 = 0.12

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 345 lb uplift at joint 2 and 345 lb uplift at joint 4.

LOAD CASE(S) Standard

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1116061043

Job	Truss	Truss Type	Qty	Ply	LOT 15 FORT WHITE STATION
L217897	T32	COMMON	4	1	Job Reference (optional)

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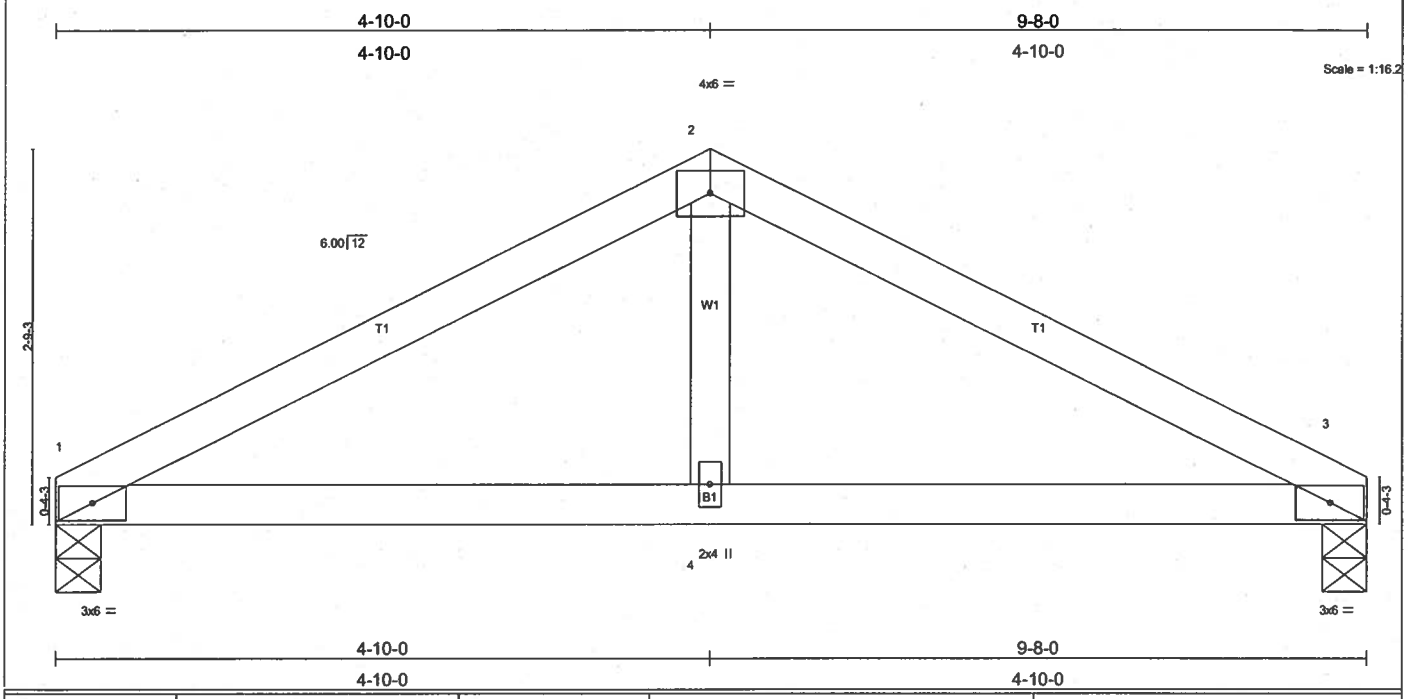

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	0.04	3-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	-0.04	3-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.01	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 34 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-5-3 oc bracing.

REACTIONS (lb/size) 1=392/0-4-0, 3=392/0-4-0
Max Horz 1=-36(load case 3)
Max Uplift1=-244(load case 5), 3=-244(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-567/532, 2-3=-567/532
BOT CHORD 1-4=-426/459, 3-4=-426/459
WEBS 2-4=-309/187

JOINT STRESS INDEX
1 = 0.33, 2 = 0.45, 3 = 0.33 and 4 = 0.14

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 244 lb uplift at joint 1 and 244 lb uplift at joint 3.

LOAD CASE(S) Standard

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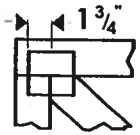
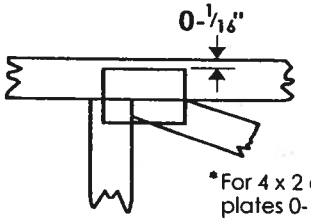
Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

*This symbol indicates the required direction of slots in connector plates.

***Plate location details available in MiTek 20/20 software or upon request.**

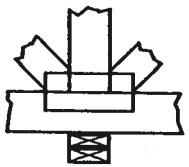
PLATE SIZE

4 X 4 — The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

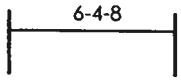


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

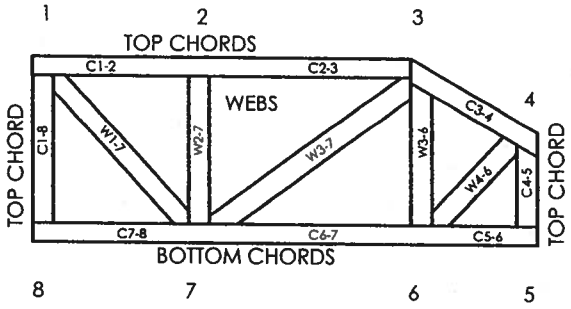
Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI1: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 95-43, 96-20-1, 96-67, 84-32
ICBO	4922, 5243, 5363, 3907
SBCCI	9667, 9730, 9604B, 9511, 9432A

MiTek®

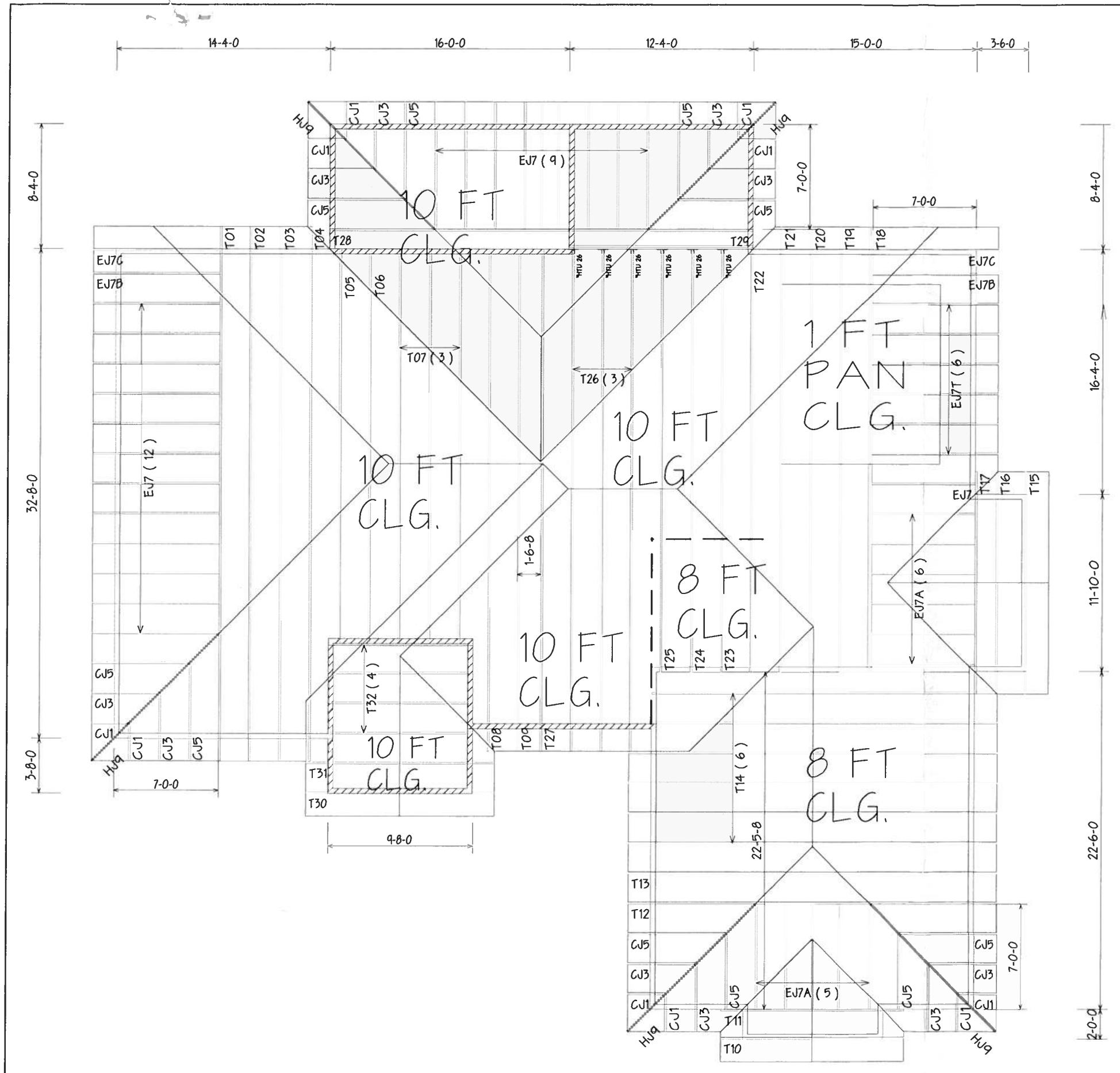
TEE-LOK®

MiTek Engineering Reference Sheet: MII-7473

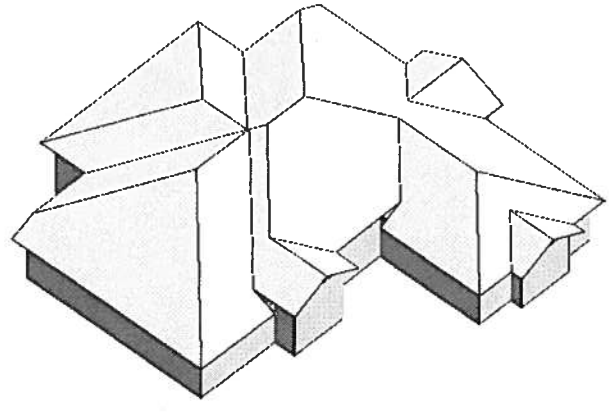
General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI1.
2. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
3. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
4. Cut members to bear tightly against each other.
5. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI1.
6. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI1.
7. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
8. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
9. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
10. Plate type, size, orientation and location dimensions shown indicate minimum plating requirements.
11. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
12. Top chords must be sheathed or purlins provided at spacing shown on design.
13. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
14. Connections not shown are the responsibility of others.
15. Do not cut or alter truss member or plate without prior approval of a professional engineer.
16. Install and load vertically unless indicated otherwise.



ROOF/FLOOR TRUSS HANGER LIST			
Truss			
Location	Manuf.	Product	Member
Girder : T29			
2-0-12	User defined	*HTU 26	T23
4-0-12	User defined	*HTU 26	T24
6-0-12	User defined	*HTU 26	T25
8-0-12	User defined	*HTU 26	T26
10-0-12	User defined	*HTU 26	T26
12-0-12	User defined	*HTU 26	T26



BEARING HEIGHT SCHEDULE

8'0"

10'0"

6/12 PITCH

18" O.H.

NOTES:

1.) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

2.) ALL TRUSSES (INSLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V105 FOR ALTERNATE BRACING REQUIREMENTS.

3.) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4.) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.

5.) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6.) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7.) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8.) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date : _____

Approved By _____ Date _____

Builders FirstSource

Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994

Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973

Lake City
PHONE: 386-755-6894 FAX: 386-755-7973

Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

BUILDER: EWPL		
LEGAL ADDRESS: LOT 15 FORT WHITE STATION		
MODEL: CUSTOM		REVISION:
		SCALE: NTS
DATE: 11/16/06	DRAWN BY: JOE	JOB #: L217897