

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

RE: 2374839 - HTH - SPEC HSE SPARR LN

MiTek USA, Inc.

6904 Parke East Blvd. Tampa, FL 33610-4115

Site Information:

Customer Info: Home Town Homes Project Name: Spec Hse Model: Custom

Lot/Block: N/A

Subdivision: N/A

Address: TBD Sparr Lane, N/A

City: Columbia Cty State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name:

License #:

Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2017/TPI2014

Design Program: MiTek 20/20 8.2

Truss Name Date

Wind Code: ASCE 7-10

Wind Speed: 130 mph

T15 TG01

Roof Load: 37.0 psf Floor Load: N/A psf

This package includes 24 individual, Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

T20450305

Seal#

No.

No. 123456789101123456789001	Seal# T20450282 T20450284 T20450285 T20450286 T20450287 T20450289 T20450290 T20450291 T20450293 T20450294 T20450295 T20450297 T20450297 T20450297 T20450297 T20450297 T20450297 T20450298 T20450297 T20450300 T20450300 T20450301	Truss Name CJ01 CJ02 CJ03 CJ04 EJ01 EJ02 HJ02 HJ02 T01 T02 T03 T04 T05 T06 T07 T08 T09 T10 T11 T12 T13	6/12/20 6/12/20
21	T20450302	T13	6/12/20
22	T20450303	T14	6/12/20



6/12/20 6/12/20

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Jacksonville.

Truss Design Engineer's Name: Velez, Joaquin

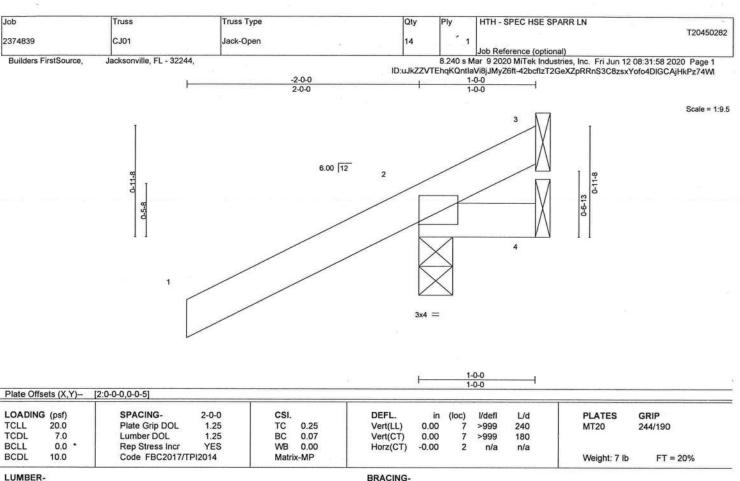
My license renewal date for the state of Florida is February 28, 2021.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

June 12,2020



TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

> (size) 3=Mechanical, 2=0-3-8, 4=Mechanical Max Horz 2=52(LC 12)

Max Uplift 3=-28(LC 1), 2=-115(LC 12), 4=-45(LC 1) Max Grav 3=17(LC 16), 2=254(LC 1), 4=31(LC 16)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

REACTIONS.

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (ft=lb)
- 6) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Structural wood sheathing directly applied or 1-0-0 oc purlins.

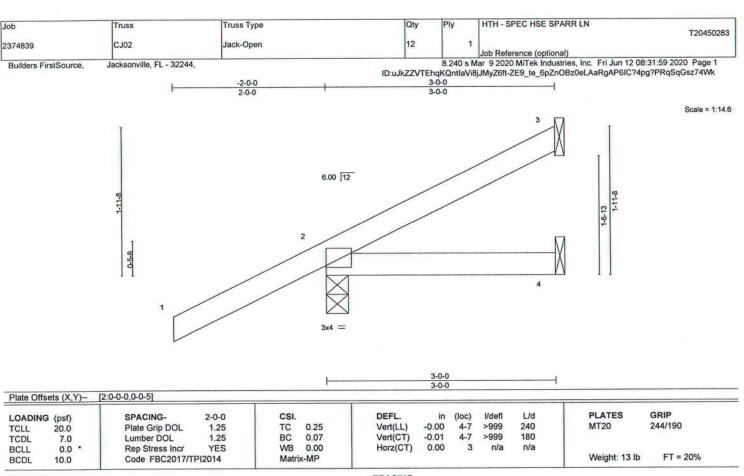
Rigid ceiling directly applied or 10-0-0 oc bracing.

Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

June 12,2020

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=90(LC 12)

Max Uplift 3=-39(LC 12), 2=-89(LC 12), 4=-17(LC 9) Max Grav 3=53(LC 1), 2=253(LC 1), 4=48(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (6)

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
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- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2, 4.
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Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

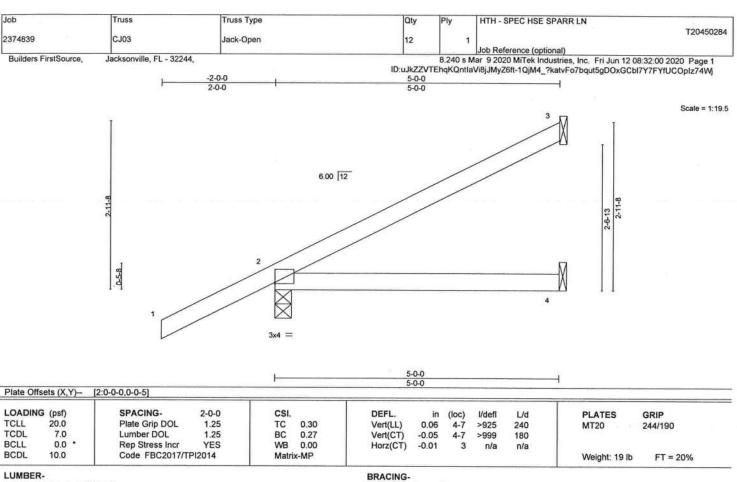
June 12,2020

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6904 Parke East Blvd. Tampa, FL 36610



TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

(size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=128(LC 12)

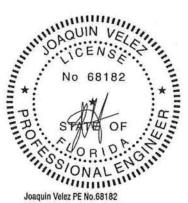
Max Uplift 3=-77(LC 12), 2=-95(LC 12), 4=-33(LC 9) Max Grav 3=109(LC 1), 2=313(LC 1), 4=87(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (6)

REACTIONS.

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
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- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2, 4.
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Structural wood sheathing directly applied or 5-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

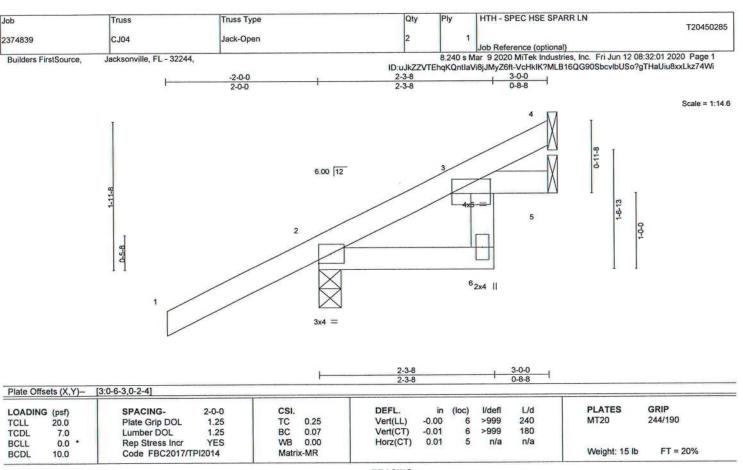
Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

June 12,2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

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LUMBER-

TOP CHORD 2x4 SP No.2

2x4 SP No.2 *Except* **BOT CHORD**

3-6: 2x4 SP No.3

BRACING-

TOP CHORD

Structural wood sheathing directly applied or 3-0-0 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing. **BOT CHORD**

REACTIONS.

(size) 4=Mechanical, 2=0-3-8, 5=Mechanical

Max Horz 2=90(LC 12)

Max Uplift 4=-23(LC 12), 2=-88(LC 12), 5=-8(LC 12) Max Grav 4=42(LC 1), 2=255(LC 1), 5=44(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2, 5.
- 6) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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June 12,2020

A WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MTI-6% connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/for chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd. Tampa, FL 36610

Job Truss HTH - SPEC HSE SPARR IN Truss Type Qty T20450286 2374839 EJ01 Jack-Partial 14 Job Reference (optional) Builders FirstSource, Jacksonville, FL - 32244. 8.240 s Mar 9 2020 MiTek Industries, Inc. Fri Jun 12 08:32:02 2020 Page 1 ID:uJkZZVTEhqKQntlaVi8jJMyZ6ft-zpr6Vg0_6U9y2QkD0l88lp1WdPuW01kr7ohUtAz74Wh 7-0-0 7-0-0 2-0-0 Scale: 1/2"=1" 6.00 12 3-6-13 0-5-8 3x4 = Plate Offsets (X,Y)-[2:0-0-0,0-0-5] LOADING (psf) SPACING-2-0-0 CSI. DEFL. I/defi PLATES GRIP (loc) L/d Plate Grip DOL TCLL 20.0 1.25 TC 0.69 Vert(LL) 0.27 4-7 >311 240 MT20 244/190 TCDL 7.0 Lumber DOL 1.25 BC 0.59 Vert(CT) 0.22 >370 180 BCLL 0.0 Rep Stress Incr YES WB 0.00 Horz(CT) -0.02 3 n/a n/a BCDL Code FBC2017/TPI2014 10.0 Matrix-MS Weight: 26 lb FT = 20%LUMBER-BRACING-

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.2 BOT CHORD

2x4 SP No.2

(size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=162(LC 12)

Max Uplift 3=-100(LC 12), 2=-108(LC 12), 4=-47(LC 9) Max Grav 3=161(LC 1), 2=380(LC 1), 4=125(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

REACTIONS.

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (jt=lb)
- 6) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

June 12,2020

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev., 10/03/2015 BEFORE USE. Design valid for use only with MTEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent occliapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/THI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



HTH - SPEC HSE SPARR LN Truss Type Qty Truss Job T20450287 1 2374839 **EJ02** Jack-Open Job Reference (optional) 8.240 s Mar 9 2020 MiTek Industries, Inc. Fri Jun 12 08:32:02 2020 Page 1 ID:uJkZZVTEhqKQntlaVi8jJMyZ6ft-zpr6Vg0_6U9y2QkD0l88lp1aWPzu01kr7ohUtAz74Wh Builders FirstSource Jacksonville, FL - 32244, Scale = 1:18.5 6.00 12 1-11-8 0-5-B 5 2x4 II [2:0-3-0,0-2-0] Plate Offsets (X,Y)-PLATES GRIP SPACING-2-0-0 CSI. DEFL. (loc) LOADING (psf) 244/190 0.06 >912 240 MT20 1.25 TC 0.44 Vert(LL) Plate Grip DOL TCLL 20.0 0.31 -0.09 5 >657 180 Lumber DOL 1.25 BC Vert(CT) TCDL 7.0 Rep Stress Incr YES WB 0.00 Horz(CT) BCLL 0.0 FT = 20%Code FBC2017/TPI2014 Matrix-MR Weight: 18 lb BCDL 10.0

LUMBER-

TOP CHORD 2x4 SP No.2

2x4 SP No.2 *Except* BOT CHORD

2-5: 2x4 SP No.3

BRACING-

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS.

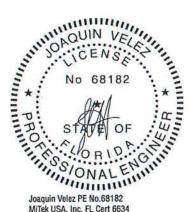
(size) 1=0-3-8, 3=Mechanical, 4=Mechanical

Max Horz 1=95(LC 12)

Max Uplift 1=-32(LC 12), 3=-67(LC 12), 4=-19(LC 12) Max Grav 1=185(LC 1), 3=108(LC 1), 4=85(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
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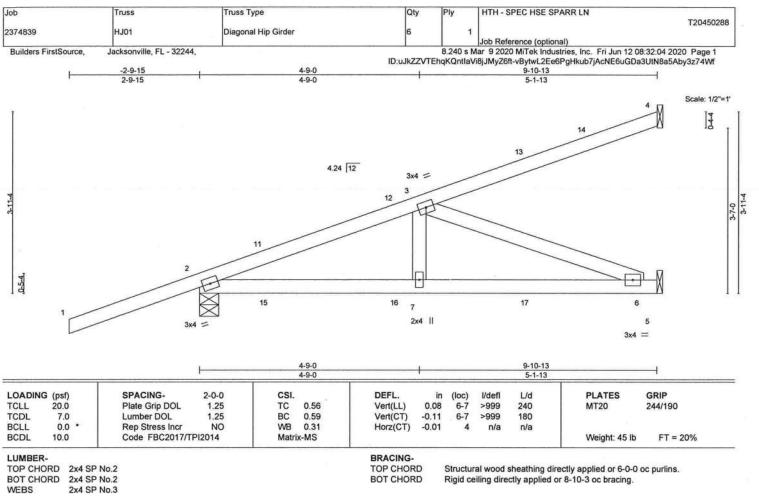


Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

June 12,2020

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REACTIONS.

(size) 4=Mechanical, 2=0-4-15, 5=Mechanical Max Horz 2=180(LC 4)

Max Uplift 4=-89(LC 4), 2=-328(LC 4), 5=-184(LC 5) Max Grav 4=145(LC 1), 2=464(LC 1), 5=270(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-593/367

BOT CHORD 2-7=-410/534, 6-7=-410/534

WEBS 3-6=-570/438

NOTES- (8)

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
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- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=328, 5=184.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 63 lb down and 104 lb up at 1-5-12, 63 lb down and 104 lb up at 1-5-12, 22 lb down and 40 lb up at 4-3-11, 22 lb down and 40 lb up at 4-3-11, and 44 lb down and 87 lb up at 7-1-10, and 44 lb down and 87 lb up at 7-1-10 on top chord, and 49 lb down and 73 lb up at 1-5-12, 49 lb down and 73 lb up at 1-5-12, 45 lb down and 24 lb up at 4-3-11, 45 lb down and 24 lb up at 4-3-11, and 39 lb down and 47 lb up at 7-1-10, and 39 lb down and 47 lb up at 7-1-10 on bottom chord. The design/selection of such connection device(s) is the responsibility of others
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard

Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)

Vert: 1-4=-54, 5-8=-20

Concentrated Loads (lb)
Vert: 11=51(F=26, B=26) 13=-66(F=-33, B=-33) 15=68(F=34, B=34) 16=7(F=3, B=3) 17=-46(F=-23, B=-23)

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Joaquin Velez PE No.68182

Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

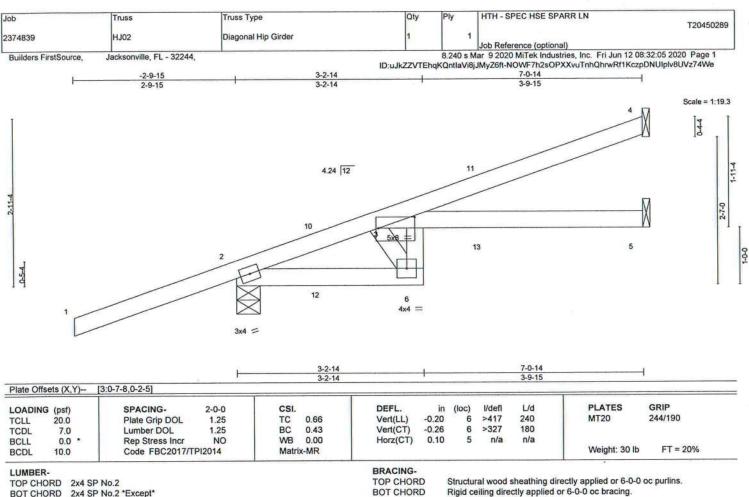
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6904 Parke East Blvd



2x4 SP No.2 *Except* **BOT CHORD**

3-6: 2x4 SP No.3

WEBS 2x4 SP No.3

REACTIONS.

(size) 4=Mechanical, 2=0-4-15, 5=Mechanical

Max Horz 2=147(LC 22)

Max Uplift 4=-79(LC 8), 2=-201(LC 4), 5=-21(LC 8) Max Grav 4=133(LC 1), 2=359(LC 1), 5=108(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 5 except (jt=lb) 2=201.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 63 lb down and 104 lb up at 1-5-12, 63 lb down and 104 lb up at 1-5-12, and 71 lb down and 23 lb up at 4-3-11, and 71 lb down and 23 lb up at 4-3-11 on top chord, and 23 lb down and 73 lb up at 1-5-12, 23 lb down and 73 lb up at 1-5-12, and 25 lb down and 16 lb up at 4-3-11, and 25 lb down and 16 lb up at 4-3-11 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-3=-54, 3-4=-54, 6-7=-20, 3-5=-20

Concentrated Loads (lb)

Vert: 10=51(F=26, B=26) 12=68(F=34, B=34) 13=-18(F=-9, B=-9)



Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

June 12,2020

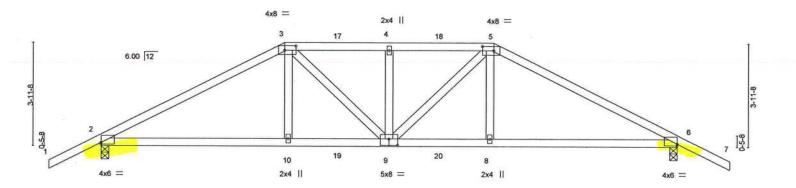
A WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer. Bracing individual temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/T91 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd. Tampa, FL 36610

Job Truss Truss Type HTH - SPEC HSE SPARR LN Qty T20450290 2374839 T01 Hip Girder 1 Job Reference (optional) **Builders FirstSource** Jacksonville, FL - 32244, 8.240 s Mar 9 2020 MiTek Industries, Inc. Fri Jun 12 08:32:06 2020 Page 1 ID:uJkZZVTEhqKQntlaVi8jJMyZ6ft-ra4dL13V9jfOX22_F8C4SfBAp0BMynMR2Pfi0yz74Wd 11-0-0 7-0-0 15-0-0 24-0-0 2-0-0

Scale = 1:42.4



		7-0-0		1	11-0-0	, 15	-0-0			2	2-0-0	- 6
		7-0-0			4-0-0	4	-0-0		6		r-0-0	
Plate Offse	ets (X,Y)-	[2:0-0-0,0-0-13], [3:0-5-4,	0-2-0], [5:0-5-4	,0-2-0], [6:0-	0-0,0-0-13],	[9:0-4-0,0-3-0]						
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.81	Vert(LL)	0.18	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.92	Vert(CT)	-0.22	8-9	>999	180	331.50-80	
BCLL	0.0	Rep Stress Incr	NO	WB	0.22	Horz(CT)	0.08	6	n/a	n/a		
BCDL	10.0	Code FBC2017/TI	PI2014	Matri	x-MS						Weight: 105 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied or 5-0-7 oc bracing.

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS.

(size) 2=0-3-8, 6=0-3-8

Max Horz 2=-75(LC 9)

Max Uplift 2=-842(LC 8), 6=-842(LC 9) Max Grav 2=1638(LC 1), 6=1638(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-2807/1529, 3-4=-2801/1605, 4-5=-2801/1605, 5-6=-2807/1530 2-10=-1318/2425, 9-10=-1326/2441, 8-9=-1275/2441, 6-8=-1266/2425 WEBS 3-10=-223/557, 3-9=-355/568, 4-9=-483/305, 5-9=-355/568, 5-8=-223/557

NOTES- (9

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=842, 6=842.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 223 lb down and 196 lb up at 7-0-0, 107 lb down and 103 lb up at 9-0-12, 107 lb down and 95 lb up at 11-0-0, and 107 lb down and 103 lb up at 12-11-4, and 223 lb down and 196 lb up at 15-0-0 on top chord, and 299 lb down and 285 lb up at 7-0-0, 85 lb down and 67 lb up at 9-0-12, 85 lb down and 67 lb up at 11-0-0, and 85 lb down and 67 lb up at 12-11-4, and 299 lb down and 285 lb up at 14-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-3=-54, 3-5=-54, 5-7=-54, 11-14=-20

Concentrated Loads (lb)

Vert: 3=-176(B) 5=-176(B) 10=-289(B) 9=-60(B) 4=-107(B) 8=-289(B) 17=-107(B) 18=-107(B) 19=-60(B) 20=-60(B)



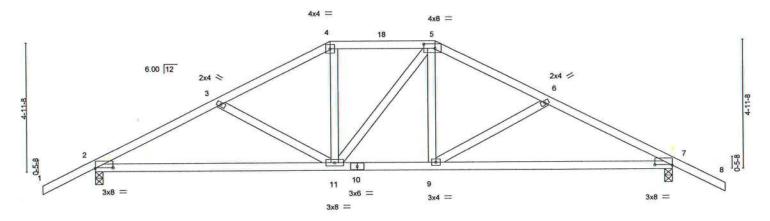
Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

June 12,2020



HTH - SPEC HSE SPARR LN Truss Type Qty Job Truss T20450291 1 T02 2374839 Hip Job Reference (optional) 8.240 s Mar 9 2020 MiTek Industries, Inc. Fri Jun 12 08:32:07 2020 Page 1 Builders FirstSource, Jacksonville, FL - 32244, ID:uJkZZVTEhgKQntlaVi8jJMyZ6ft-Kme?YN47w1nF8BdAprjJ?skTDQaEhFqaG3OFZOz74Wc 22-0-0 24-0-0 4-10-2 9-0-0 4-0-0 4-1-14 4-10-2 2-0-0

Scale = 1:42.4



	9-0-0					13-0-0 4-0-0	1			22-0-0 9-0-0		-1
Plate Offse	ets (X,Y)-			0-0-3]								
LOADING TCLL TCDL	(psf) 20.0 7.0	SPACING- Plate Grip DOL Lumber DOL	2-0-0 1.25 1.25	CSI. TC BC	0.32 0.69	DEFL. Vert(LL) Vert(CT)	in -0.15 -0.31		l/defl >999 >853	L/d 240 180	PLATES MT20	GRIP 244/190
BCLL	0.0 • 10.0	Rep Stress Incr Code FBC2017/TF	YES PI2014	WB Matrix	0.14 c-MS	Horz(CT)	0.04	7.	n/a	n/a	Weight: 111 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 2x4 SP No.2 **BOT CHORD**

2x4 SP No.3 WEBS

> (size) 2=0-3-8, 7=0-3-8 Max Horz 2=92(LC 16)

Max Uplift 2=-264(LC 12), 7=-264(LC 13) Max Grav 2=922(LC 1), 7=922(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD

2-3=-1348/525, 3-4=-1092/427, 4-5=-933/420, 5-6=-1091/427, 6-7=-1348/524

2-11=-346/1159, 9-11=-190/932, 7-9=-366/1159 **BOT CHORD**

3-11=-273/203, 4-11=-57/309, 5-9=-63/309, 6-9=-273/203 WEBS

NOTES-

Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=264, 7=264.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Structural wood sheathing directly applied or 4-11-10 oc purlins.

Rigid ceiling directly applied or 9-7-10 oc bracing.

Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

June 12,2020

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property amage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANS/ITP1 Quality Criteria, DSB-89 and BCSI Building Compo Safety Information

available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job HTH - SPEC HSE SPARR LN Truss Truss Type Qty Ply T20450292 T03 2374839 Common Job Reference (optional) 8.240 s Mar 9 2020 MiTek Industries, Inc. Fri Jun 12 08:32:08 2020 Page 1 Jacksonville, FL - 32244, Builders FirstSource ID:uJkZZVTEhqKQntlaVi8jJMyZ6ft-ozCNmj5lhKv6mLCMMZEYY4HbEqsqQgukVj8p5qz74Wb 2-0-0 16-5-10 5-5-10 22-0-0 2-0-0 Scale = 1:43.0 4x4 || 6.00 12 2x4 \\ 2x4 // 10 8 3x6 = 3x4 = 3x6 = 3x6 = 3x4 = 14-8-7 Plate Offsets (X,Y)- [2:0-0-0,0-0-5], [6:0-0-0,0-0-5] LOADING (psf) SPACING-2-0-0 CSI. DEFL (loc) L/d **PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.25 TC 0.49 Vert(LL) -0.21 8-10 240 >999 MT20 244/190 TCDL 7.0 Lumber DOL 1.25 BC 0.92 Vert(CT) -0.40 8-10 >655 180 BCLL 00 Rep Stress Incr NO WB 0.28 Horz(CT) 0.04 6 n/a n/a BCDL 10.0 Code FBC2017/TPI2014 Matrix-MS Weight: 105 lb FT = 20% LUMBER-BRACING-TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 3-10-12 oc purlins. 2x4 SP No.2 *Except* **BOT CHORD BOT CHORD** Rigid ceiling directly applied or 8-0-6 oc bracing. 6-9: 2x4 SP M 31 WEBS 2x4 SP No.3 REACTIONS. (size) 2=0-3-8, 6=0-3-8 Max Horz 2=109(LC 12) Max Uplift 2=-333(LC 12), 6=-333(LC 13) Max Grav 2=1144(LC 1), 6=1144(LC 1) (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-1866/716, 3-4=-1716/714, 4-5=-1728/717, 5-6=-1878/720 TOP CHORD **BOT CHORD** 2-10=-514/1606, 8-10=-266/1093, 6-8=-534/1616

WEBS

FORCES.

4-8=-277/730, 5-8=-255/214, 4-10=-271/711, 3-10=-254/214

NOTES-(7)

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads,
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb)
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-7=-54, 10-11=-20, 8-10=-80(F=-60), 8-14=-20

No 68182

No 68182

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Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

June 12,2020

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd. Tampa, FL 36610

HTH - SPEC HSE SPARR IN Qty Truss Truss Type Job T20450293 2374839 T04 Common 1 Job Reference (optional) 8 240 s Mar 9 2020 MiTek Industries, Inc. Fri Jun 12 08:32:09 2020 Page 1 ID:uJkZZVTEhqKQntlaVi8jJMyZ6ft-G9mmz36NSe1zOVnZwGmn4HppyEl595LtkNtMdGz74Wa Jacksonville, FL - 32244, Builders FirstSource. 14-10-8 11-0-0 3-10-8 Scale = 1:37.5 4x4 = 2x4 || 5 6.00 12 2x4 \ 0-5-8 7 11 12 6 3x4 = 3x6 = 14-10-8 PLATES GRIP l/defi CSI DEFL LOADING (psf) SPACING-2-0-0 in (loc) >999 240 MT20 244/190 TC 0.30 Vert(LL) -0.10 6-7 TCLL 20.0 Plate Grip DOL 1.25 0.53 Vert(CT) -0.17 >999 180 1.25 BC TCDL 7.0 Lumber DOL YES WB 0.39 Horz(CT) 0.01 n/a n/a BCIL 0.0 Rep Stress Incr Weight: 80 lb FT = 20%Code FBC2017/TPI2014 Matrix-MS BCDL 10.0 BRACING-LUMBER-TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, TOP CHORD 2x4 SP No.2 except end verticals. 2x4 SP No.2 **BOT CHORD** BOT CHORD Rigid ceiling directly applied or 9-10-1 oc bracing. 2x4 SP No.3 WEBS

REACTIONS.

(size) 2=0-3-8, 6=Mechanical

Max Horz 2=205(LC 12)

Max Uplift 2=-195(LC 12), 6=-149(LC 12) Max Grav 2=660(LC 1), 6=538(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

2-3=-813/296, 3-4=-658/291 2-7=-358/676, 6-7=-121/263

BOT CHORD WEBS

3-7=-288/247, 4-7=-188/480, 4-6=-447/215

NOTES- (7)

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=195.6=149.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

June 12,2020

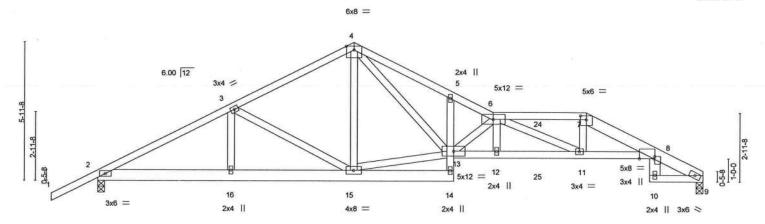
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual bullding component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and pertyl damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qty HTH - SPEC HSE SPARR LN Ply T20450294 2374839 T05 Roof Special Girder Job Reference (optional) Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Mar 9 2020 MiTek Industries, Inc. Fri Jun 12 08:32:11 2020 Page 1 ID:uJkZZVTEhqKQntlaVi8jJMyZ6ft-CYtWOl7d_FHhdpwx2hoF9iv6m1yXdx6ABhMTi9z74WY 15-3-8 11-0-0 17-0-0 23-8-8 21-0-0 26-0-0 4-3-8 4-0-0

Scale: 1/4"=1"



	4.	5-8-11		11-0-0	T .	15-3-8	17-0-0	- 1	21-0-0	23-8-8	26-0-0
		5-8-11	1	5-3-5	,	4-3-8	1-8-8		4-0-0	2-8-8	2-3-8
Plate Offs	ets (X,Y)-	[7:0-3-0,0-2-0], [8:0-7-8,0	0-0-0], [8:0-1-3,	0-0-8]							
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.47	Vert(LL)	-0.21 12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.66	Vert(CT)	-0.38 12-13	>820	180		
BCLL	0.0	Rep Stress Incr	NO	WB	0.70	Horz(CT)	0.19 9	n/a	n/a		
BCDL	10.0	Code FBC2017/T	PI2014	Matri	x-MS	2 8				Weight: 313 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 *Except*

7-9: 2x6 SP M 26

BOT CHORD 2x6 SP No.2 *Except*

5-14: 2x4 SP No.3, 8-13: 2x4 SP M 31, 9-10: 2x4 SP No.2

WERS 2v4 SP No 3

REACTIONS.

(size) 9=0-3-8, 2=0-3-8

Max Horz 2=125(LC 31)

Max Uplift 9=-562(LC 9), 2=-442(LC 8) Max Grav 9=1630(LC 1), 2=1417(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2443/720, 3-4=-2079/665, 4-5=-4758/1610, 5-6=-4819/1559, 6-7=-4464/1575,

7-8=-4639/1605, 8-19=-906/332 BOT CHORD

2-16=-668/2132, 15-16=-668/2132, 14-15=-118/406, 13-14=-180/618, 12-13=-1949/6146,

11-12=-1949/6153, 8-11=-1447/4356, 8-10=-100/316

WEBS 3-15=-399/231, 13-15=-388/1425, 4-13=-1291/3651, 6-13=-2451/889, 6-11=-1840/530,

7-11=-351/1247

NOTES-(10)

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc.

Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc, 2x4 - 1 row at 0-9-0 oc.

Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60

5) Provide adequate drainage to prevent water ponding.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 9=562, 2=442.

9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 94 lb down and 71 lb up at 17-0-0, and 94 lb down and 71 lb up at 18-11-4, and 207 lb down and 153 lb up at 21-0-0 on top chord, and 576 lb down and 203 lb up at 15-1-12, 56 lb down and 36 lb up at 16-11-4, and 56 lb down and 36 lb up at 18-11-4, and 123 lb down and 71 lb up at 20-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

10) This manufactured product is designed as an individual building component. The suitability and use of this component for any Continuation of the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Structural wood sheathing directly applied or 5-7-1 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

June 12,2020

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property departed great guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	HTH - SPEC HSE SPARR LN	T20450294
2374839	то5	Roof Special Girder	1	2	Job Reference (optional)	

Builders FirstSource,

Jacksonville, FL - 32244,

8.240 s Mar 9 2020 MiTek Industries, Inc. Fri Jun 12 08:32:11 2020 Page 2 ID:uJkZZVTEhqKQntlaVi8jJMyZ6ff-CYtWOl7d_FHhdpwx2hoF9iv6m1yXdx6ABhMTi9z74WY

LOAD CASE(S) Standard

Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)

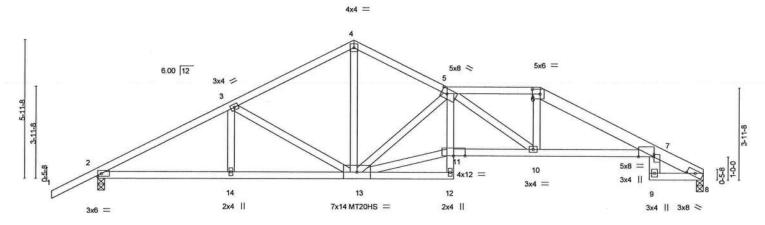
Vert: 1-4=-54, 4-6=-54, 6-7=-54, 7-8=-54, 8-19=-54, 14-21=-20, 8-13=-20, 9-10=-20

Concentrated Loads (lb) Vert: 6=-54(B) 7=-111(B) 14=-576(B) 12=-56(B) 11=-112(B) 24=-54(B) 25=-56(B)

MiTek'

Job	Truss	Truss Type		Qty	Ply	HTH - SPEC	HSE SPARR LN	-2000	
2374839	Т06	Roof Special		1	15000	1	NAT THE STATE OF T	T20	0450295
						Job Reference	e (optional)		
Builders FirstSource,	Jacksonville, FL - 32244,				8.240 s	s Mar 9 2020 MiT	ek Industries, Inc. Fri Jun	12 08:32:12 2020 Pa	age 1
			ID:uJkZZ				8FIZPYFzV8bOJUiwRGZ		
-2-0-0	5-8-11	11-0-0	15-0-0	15 _T	3 ₁ 8	19-0-0	23-8-8	26-0-0	
2-0-0	5-8-11	5-3-5	4-0-0	0-3	3-8	3-8-8	4-8-8	2-3-8	

Scale: 1/4"=1"



	1	5-8-11	9	11-0-0	7	15-3-8	1		19-0-0	- 1	23-8-8	26-0-0
	1	5-8-11	1	5-3-5	1	4-3-8	- 1		3-8-8		4-8-8	2-3-8
Plate Offse	ets (X,Y)-	[5:0-3-0,0-2-8], [6:0-4-0,0	-2-8], [7:0-1-3,	0-0-4], [7:0-7	-12,0-0-0], [8	8:0-4-2,0-1-8]						
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L∕d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.47	Vert(LL)	-0.21	7-10	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.56	Vert(CT)	-0.40	7-10	>777	180	MT20HS	187/143
BCLL	0.0	Rep Stress Incr	YES	WB	0.86	Horz(CT)	0.24	8	n/a	n/a		252NA33
BCDL	10.0	Code FBC2017/T	PI2014	Matri	x-MS						Weight: 142 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 *Except*

6-8: 2x6 SP M 26

BOT CHORD 2x4 SP No.2 *Except*

5-12: 2x4 SP No.3, 7-11: 2x4 SP M 31, 7-9: 2x6 SP No.2

WEBS 2x4 SP No.3

REACTIONS.

(size) 8=0-3-8, 2=0-3-8

Max Horz 2=125(LC 16)

Max Uplift 8=-261(LC 13), 2=-284(LC 12) Max Grav 8=959(LC 1), 2=1069(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1671/624, 3-4=-1261/529, 4-5=-1270/551, 5-6=-1919/752, 6-7=-2080/747,

7-17=-525/214

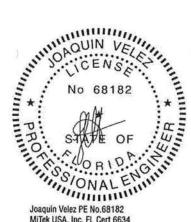
BOT CHORD 2-14=-484/1433, 13-14=-484/1433, 5-11=-139/542, 10-11=-728/2240, 7-10=-595/1893
WEBS 3-13=-439/232, 4-13=-318/829, 5-13=-1498/584, 11-13=-672/2053, 5-10=-390/154

3-13=-439/232, 4-13=-318/829, 5-13=-1498/584, 11-13=-672/2053, 5-10=-390/154, 6-10=-115/464

NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are MT20 plates unless otherwise indicated.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=261, 2=284.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Structural wood sheathing directly applied or 4-1-0 oc purlins.

Rigid ceiling directly applied or 8-5-15 oc bracing.

Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

June 12,2020

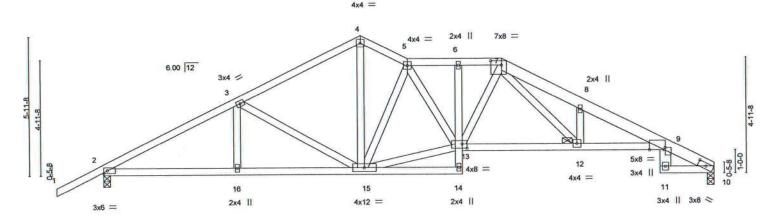
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTER REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type		Qty	Ply	HTH - SPEC HSE SPA	ARR LN	T204502
2374839	Т07	Roof Special		1	1	Job Reference (options	al)	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,
Builders FirstSource,	Jacksonville, FL - 32244,		10	:uJkZZVTI	8.240 s M	ar 9 2020 MiTek Indust	ries, Inc. Fri Jun 12	08:32:13 2020 Page 1 rgY5t6Tf?ram2z74WW
-2-0-0	5-8-11	11-0-0	13-0-0	15-3-8	17-0-0	20-4-4	23-8-8	26-0-0
2-0-0	5-8-11	5-3-5	2-0-0	2-3-8	1-8-8	3-4-4	3-4-4	2-3-8

Scale = 1:47.3



	29	5-8-11	300	11-0-0		15-3-8	- 1		20-4-	4	23-8-8	26-0-0
	1	5-8-11	1000	5-3-5		4-3-8	- 1		5-0-1	2	3-4-4	2-3-8
Plate Offse	ets (X,Y)-	[7:0-5-8,0-2-4], [9:0-1-3,0	-0-8], [9:0-7-8,0	0-0-0], [10:0-4	4-2,0-1-8], [1	3:0-2-8,0-2-4]						
LOADING TCLL TCDL BCLL	(psf) 20.0 7.0 0.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.25 1.25 YES	CSI. TC BC WB	0.47 0.56 0.54	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.17 -0.31 0.20	(loc) 9-12 9-12 10	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 244/190
BCDL	10.0	Code FBC2017/TI	PI2014	Matrix	(-MS						Weight: 156 lb	FT = 20%

BRACING-

JOINTS

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 *Except*

7-10: 2x6 SP M 26

BOT CHORD 2x4 SP No.2 *Except*

6-14: 2x4 SP No.3, 9-13: 2x4 SP M 31, 9-11: 2x6 SP No.2

WEBS 2x4 SP No.3

REACTIONS.

(size) 10=0-3-8, 2=0-3-8

Max Horz 2=125(LC 16)

Max Uplift 10=-261(LC 13), 2=-284(LC 12) Max Grav 10=959(LC 1), 2=1069(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

2-3=-1671/622, 3-4=-1262/527, 4-5=-1234/558, 5-6=-1611/665, 6-7=-1625/668, TOP CHORD

7-8=-2711/1075, 8-9=-2490/901, 9-19=-525/214

2-16=-483/1433, 15-16=-483/1433, 12-13=-442/1505, 9-12=-777/2335 **BOT CHORD** WEBS

3-15=-436/233, 4-15=-340/852, 5-15=-958/385, 13-15=-410/1407, 5-13=-90/358, 7-13=-72/267, 7-12=-497/1230, 8-12=-687/354

NOTES-

Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=261, 2=284.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Structural wood sheathing directly applied or 4-4-1 oc purlins.

Rigid ceiling directly applied or 8-6-2 oc bracing.

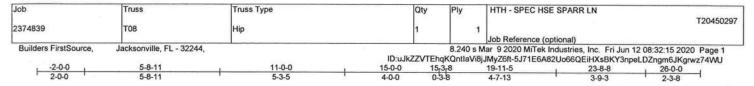
1 Brace at Jt(s): 12

Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

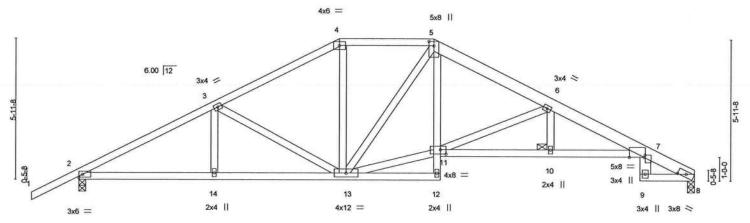
June 12,2020

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property language. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.









		5-8-11		11-0-0		15-3-8	6		19-11-5	- 7	23-8-8	26-0-0
		5-8-11		5-3-5		4-3-8	1		4-7-13		3-9-3	2-3-8
Plate Offs	ets (X,Y)-	[5:0-2-4,0-2-8], [7:0-1-3,0	-0-8], [7:0-7-8,	0-0-0], [8:0-4	-2,0-1-8], [1	1:0-2-12,0-2-4]						
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.47	Vert(LL)	-0.17	7-10	>999	240	MT20	244/190
CDL	7.0	Lumber DOL	1.25	BC	0.55	Vert(CT)	-0.32	7-10	>979	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.53	Horz(CT)	0.20	8	n/a	n/a		
BCDL	10.0	Code FBC2017/T	PI2014	Matri	x-MS	1					Weight: 151 lb	FT = 20%

BRACING-

JOINTS

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 *Except*

5-8: 2x6 SP M 26

BOT CHORD 2x4 SP No.2 *Except*

5-12: 2x4 SP No.3, 7-11: 2x4 SP M 31, 7-9: 2x6 SP No.2

WEBS 2x4 SP No.3

REACTIONS. (size)

(size) 8=0-3-8, 2=0-3-8

Max Horz 2=125(LC 16)

Max Uplift 8=-228(LC 13), 2=-283(LC 12) Max Grav 8=959(LC 1), 2=1069(LC 1)

FORCES. (ib) - Max. Comp./Max. Ten. - All forces 250 (ib) or less except when shown.

TOP CHORD 2-3=-1672/620, 3-4=-1254/521, 4-5=-1082/514, 5-6=-1559/612, 6-7=-2406/886,

7-17=-525/215 BOT CHORD 2-14=-481/143-

2-14=-481/1434, 13-14=-481/1434, 5-11=-236/785, 10-11=-759/2249, 7-10=-759/2249

3-13=-433/237, 4-13=-89/334, 5-13=-427/108, 11-13=-322/1257, 6-11=-1017/450,

6-10=-11/281

NOTES- (7)

WEBS

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ff; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=228, 2=283.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Structural wood sheathing directly applied or 4-4-1 oc purlins.

Rigid ceiling directly applied or 8-6-4 oc bracing.

1 Brace at Jt(s): 10

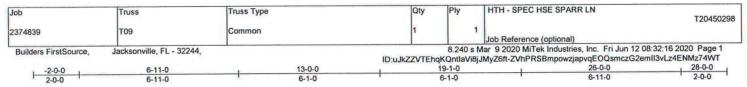
Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

June 12,2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





Scale: 1/4"=1"

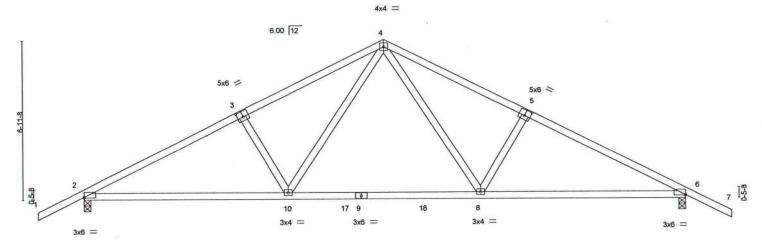


Plate Offsets (X,Y) [8-10-3 2:0-0-0,0-0-5], [3:0-3-0,0-3-	-01 [5:0-3-0 0-3-0] [6:0-0-	8-3-9		-		8-10-3	
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	CORRECTED STATE OF THE STATE OF	2-0-0 CSI. 1.25 TC 1.25 BC YES WB	DEFL. 0.43 Vert(LL) 0.72 Vert(CT) 0.27 Horz(CT)	in (k -0.21 8- -0.31 8- 0.05	100000000000000000000000000000000000000	L/d 240 180 n/a	PLATES MT20 Weight: 123 lb	GRIP 244/190 FT = 20%

17-1-13

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 4-2-14 oc purlins. Rigid ceiling directly applied or 8-11-5 oc bracing.

26-0-0

REACTIONS.

(size) 2=0-3-8, 6=0-3-8 Max Horz 2=125(LC 12)

Max Uplift 2=-298(LC 12), 6=-298(LC 13) Max Grav 2=1070(LC 1), 6=1070(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

8 10 3

TOP CHORD 2-3=-1614/621, 3-4=-1443/634, 4-5=-1443/634, 5-6=-1614/621

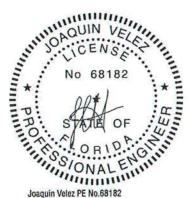
BOT CHORD 2-10=-419/1376, 8-10=-184/929, 6-8=-432/1376

WEBS 4-8=-223/560, 5-8=-343/271, 4-10=-223/560, 3-10=-343/271

NOTES- (6)

Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=298, 6=298.
- 6) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



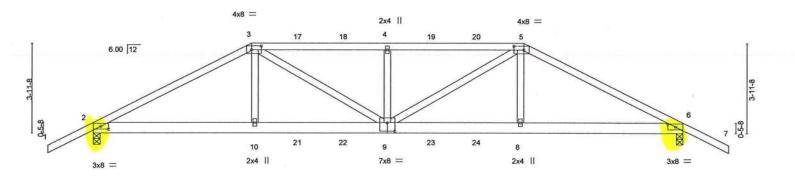
Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

June 12,2020



Job Truss Truss Type Qty HTH - SPEC HSE SPARR IN T20450299 2374839 T10 Hip Girder Job Reference (optional) Builders FirstSource. Jacksonville, FL - 32244, 8.240 s Mar 9 2020 MiTek Industries, Inc. Fri Jun 12 08:32:17 2020 Page 1 ID:uJkZZVTEhqKQntlaVi8jJMyZ6ft-1hFnfoCOa52qLkO5OyvfPz8?FS?z1hz3Zdpnvpz74WS -2-0-0 2-0-0 13-0-0 19-0-0 26-0-0 28-0-0 6-0-0 7-0-0

Scale = 1:48.9



	1	7-0-0			-0-0		19-0-0				26-0-0	
		7-0-0		AND DESCRIPTION OF THE PARTY OF	0-0		6-0-0				7-0-0	
Plate Off	sets (X,Y)	[2:0-4-1,0-1-8], [3:0-5-4,0	-2-0], [5:0-5-4,	0-2-0], [6:0-4	-1,0-1-8], [9:	0-4-0,0-4-8]						
LOADIN	G (psf)	SPACING-	2-0-0	CSI.)	DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.99	Vert(LL)	0.22	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.66	Vert(CT)	-0.30	8-9	>999	180	2000,000	
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.54	Horz(CT)	0.08	6	n/a	n/a		
BCDL	10.0	Code FBC2017/T	PI2014	Matri	x-MS						Weight: 142 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied or 5-7-10 oc bracing.

LUMBER-

TOP CHORD 2x4 SP No.2 *Except*

3-5: 2x4 SP M 31

BOT CHORD 2x6 SP No.2 2x4 SP No.3 WEBS

REACTIONS.

(size) 2=0-3-8, 6=0-3-8

Max Horz 2=75(LC 8)

Max Uplift 2=-1009(LC 8), 6=-1009(LC 9) Max Grav 2=1953(LC 1), 6=1953(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-3568/1938, 3-4=-3998/2224, 4-5=-3998/2224, 5-6=-3568/1939 TOP CHORD **BOT CHORD** 2-10=-1684/3115, 9-10=-1695/3135, 8-9=-1625/3135, 6-8=-1615/3115 3-10=-265/643, 3-9=-628/1084, 4-9=-746/457, 5-9=-628/1084, 5-8=-265/643

NOTES-

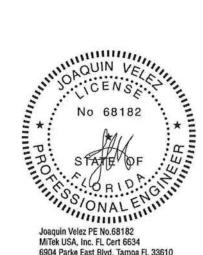
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=1009, 6=1009,
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 223 lb down and 196 lb up at 7-0-0, 107 lb down and 103 lb up at 9-0-12, 107 lb down and 103 lb up at 11-0-12, 107 lb down and 95 lb up at 13-0-0, 107 lb down and 103 lb up at 14-11-4, and 107 lb down and 103 lb up at 16-11-4, and 223 lb down and 196 lb up at 19-0-0 on top chord, and 299 lb down and 285 lb up at 7-0-0, 85 lb down and 67 lb up at 9-0-12, 85 lb down and 67 lb up at 11-0-12, 85 lb down and 67 Ib up at 13-0-0, 85 lb down and 67 lb up at 14-11-4, and 85 lb down and 67 lb up at 16-11-4, and 299 lb down and 285 lb up at 18-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-3=-54, 3-5=-54, 5-7=-54, 11-14=-20

Continued on page 2



MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

June 12,2020

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE, Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/for chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

**ANSI/THI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd. Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	HTH - SPEC HSE SPARR LN	T20450299
2374839	T10	Hip Girder	1	1	er -0000000 07 00 00-00	1944,134,44
	20.000				Job Reference (optional)	

Builders FirstSource,

Jacksonville, FL - 32244,

8.240 s Mar 9 2020 MiTek Industries, Inc. Fri Jun 12 08:32:17 2020 Page 2 ID:uJkZZVTEhqKQntlaVi8jJMyZ6ft-1hFnfoCOa52qLkO5OyvfPz8?FS?z1hz3Zdpnvpz74WS

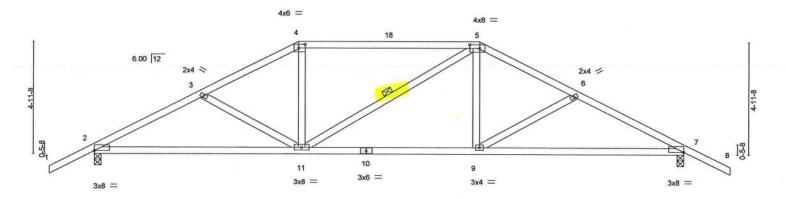
LOAD CASE(S) Standard Concentrated Loads (lb)

Vert: 3=-176(F) 5=-176(F) 10=-289(F) 9=-60(F) 4=-107(F) 8=-289(F) 17=-107(F) 18=-107(F) 19=-107(F) 20=-107(F) 21=-60(F) 22=-60(F) 23=-60(F) 24=-60(F)



Job HTH - SPEC HSE SPARR LN Truss Truss Type Qty T20450300 2374839 T11 Hip | Job Reference (optional) 8.240 s Mar 9 2020 MiTek Industries, Inc. Fri Jun 12 08:32:18 2020 Page 1 Jacksonville, FL - 32244, Builders FirstSource, ID:uJkZZVTEhqKQntlaVi8jJMyZ6ft-Vuo9s8C0KPAhzuzHyfQuxBhKZsKKmEQCoHZKRFz74WR -2-0-0 2-0-0 4-10-2 9-0-0 4-1-14 17-0-0 21-1-14 26-0-0 2-0-0

Scale = 1:48.9



9-0-0				17-0-0 8-0-0			26-0-0			Tr.		
9-0-0							10	1				
Plate Offse	ets (X,Y)-	[2:0-0-0,0-0-1], [4:0-3-8,0	-2-0], [5:0-5-4,	0-2-0], [7:0-0	-0,0-0-1]							
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defi	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.37	Vert(LL)	-0.14	9-17	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.72	Vert(CT)	-0.28	9-17	>999	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.15	Horz(CT)	0.05	7	n/a	n/a		
BCDL	10.0	Code FBC2017/T	PI2014	Matri	x-MS						Weight: 128 lb	FT = 20%

BRACING-

WEBS

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 *Except*

4-5: 2x4 SP M 31

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS.

(size) 2=0-3-8, 7=0-3-8

Max Horz 2=92(LC 12)

Max Uplift 2=-305(LC 12), 7=-305(LC 13) Max Grav 2=1070(LC 1), 7=1070(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1655/626, 3-4=-1427/541, 4-5=-1244/527, 5-6=-1426/541, 6-7=-1655/626

BOT CHORD 2-11=-433/1427, 9-11=-298/1244, 7-9=-454/1427

WEBS 4-11=-34/383, 5-9=-37/383

NOTES- (7)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=305, 7=305.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Structural wood sheathing directly applied or 4-5-9 oc purlins.

Rigid ceiling directly applied or 8-8-2 oc bracing.

1 Row at midpt

Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

June 12,2020

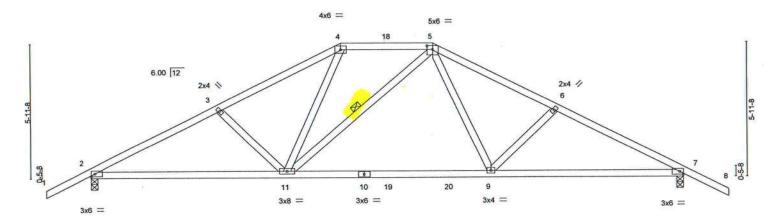
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-1473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, crection and bracing of trusses and truss systems, see ANSI/P11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	C	Qty	Ply	HTH - SPEC HSE SPARR	LN	T2045030
2374839	T12	Hip	1	1	1	Job Reference (optional)		1201000
Builders FirstSource,	Jacksonville, FL - 32244,		ID:u.lk77\	VTEho	8.240 s M	lar 9 2020 MiTek Industries, MyZ6ft-z4MY4UDe5jlYa1YU		
2-0-0	5-8-0	11-0-0	15-0-0		20-4		26-0-0	28-0-0
200	590	5-4-0	4-0-0		5-4	-0	5-8-0	2-0-0

Scale = 1:48.9



8-7-6 8-7-6			T	17-6-9 8-11-2			26-0-0			_		
			1				!	8-5-7				
Plate Offse	ets (X,Y)	[5:0-3-0,0-2-0]										
LOADING TCLL TCDL	20.0 7.0	SPACING- Plate Grip DOL Lumber DOL	2-0-0 1.25 1.25	CSI. TC BC	0.34 0.74	DEFL. Vert(LL) Vert(CT)	-0.20 -0.35	9-11	l/defl >999 >893	L/d 240 180	PLATES MT20	GRIP 244/190
BCLL BCDL	10.0	Rep Stress Incr Code FBC2017/T	YES PI2014	WB Matrix	0.18 c-MS	Horz(CT)	0.05	i	n/a	n/a	Weight: 130 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 2x4 SP No.2 **BOT CHORD** WEBS

2x4 SP No.3

(size) 2=0-3-8, 7=0-3-8 Max Horz 2=109(LC 16)

Max Uplift 2=-302(LC 12), 7=-302(LC 13) Max Grav 2=1070(LC 1), 7=1070(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1660/633, 3-4=-1441/576, 4-5=-1063/512, 5-6=-1447/580, 6-7=-1662/633

2-11=-440/1428, 9-11=-239/1061, 7-9=-457/1429 **BOT CHORD**

3-11=-290/238, 4-11=-118/458, 5-9=-125/461, 6-9=-288/237 WEBS

NOTES-(7)

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=302, 7=302.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Structural wood sheathing directly applied or 4-4-15 oc purlins.

5-11

Rigid ceiling directly applied or 8-8-7 oc bracing.

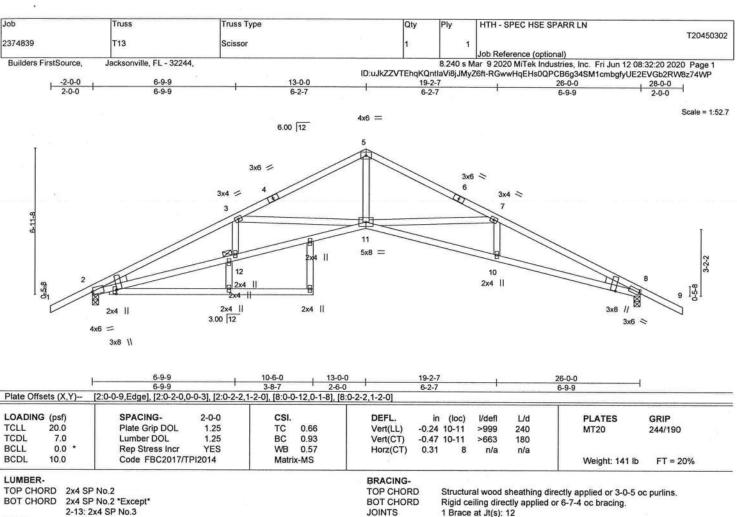
1 Row at midpt

Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

June 12,2020

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property amage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





TOP CHORD

2-13: 2x4 SP No.3

WEBS 2x4 SP No.3

WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS.

(size) 2=0-3-8, 8=0-3-8

Max Horz 2=125(LC 12)

Max Uplift 2=-297(LC 12), 8=-297(LC 13) Max Grav 2=1070(LC 1), 8=1070(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-2880/981, 3-5=-2108/683, 5-7=-2108/684, 7-8=-2880/1007

BOT CHORD

2-12=-768/2602, 11-12=-774/2613, 10-11=-800/2613, 8-10=-797/2602

5-11=-400/1483, 7-11=-764/421, 3-11=-764/425 WEBS

NOTES-(7)

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb)
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

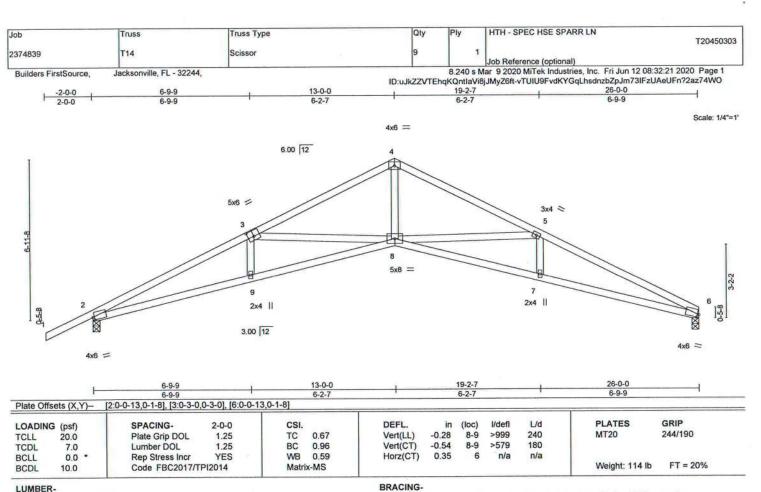


Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

June 12,2020

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 2x4 SP No.2 **BOT CHORD**

WEBS 2x4 SP No.3 (size) 6=0-3-8, 2=0-3-8

Max Horz 2=142(LC 16) Max Uplift 6=-246(LC 13), 2=-298(LC 12)

Max Grav 6=958(LC 1), 2=1074(LC 1)

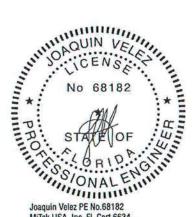
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-2900/1048, 3-4=-2125/746, 4-5=-2127/745, 5-6=-2947/1079 TOP CHORD 2-9=-873/2591, 8-9=-881/2603, 7-8=-911/2654, 6-7=-908/2641 **BOT CHORD** 4-8=-462/1502, 5-8=-790/438, 3-8=-740/409

WEBS

NOTES-(7)

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Bearing at joint(s) 6, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=246, 2=298.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Structural wood sheathing directly applied or 2-7-2 oc purlins.

Rigid ceiling directly applied or 2-2-0 oc bracing.

Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

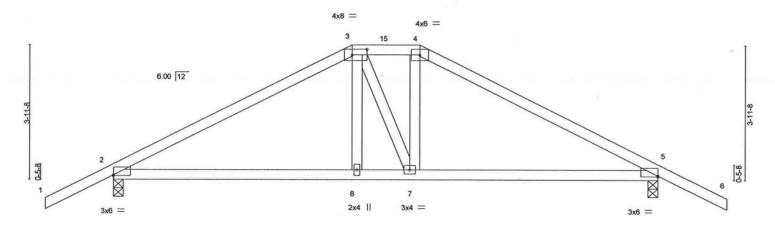
June 12,2020

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ASI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Qty Job Truss Truss Type HTH - SPEC HSE SPARR LN T20450304 2374839 T15 Hip Girder Job Reference (optional) Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Mar- 9 2020 MiTek Industries, Inc. Fri Jun 12 08:32:23 2020 Page 1 ID:uJkZZVTEhqKQntlaVi8jJMyZ6ft-src2vrG99xo_3frFlC03eEO7Mt3MRU9xyZG57Sz74WM 2-0-0

Scale = 1:32.6



			7-0-0 7-0-0			9-0-0				7-0-0		1
Plate Offs	sets (X,Y)-	[2:0-0-0,0-0-5], [3:0-5-4,0	-2-0], [5:0-0-0,	0-0-5]								
LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.63	Vert(LL)	0.10	8-11	>999	240	MT20	244/190
CDL	7.0	Lumber DOL	1.25	BC	0.60	Vert(CT)	-0.14	8-11	>999	180		
BCLL	0.0	Rep Stress Incr	NO	WB	0.18	Horz(CT)	0.04	5	n/a	n/a		
BCDL	10.0	Code FBC2017/T	PI2014	Matri	x-MS						Weight: 73 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS.

(size) 2=0-3-8, 5=0-3-8

Max Horz 2=-75(LC 13)

Max Uplift 2=-590(LC 8), 5=-590(LC 9) Max Grav 2=1165(LC 1), 5=1165(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1800/1015, 3-4=-1545/944, 4-5=-1802/1016

BOT CHORD 2-8=-868/1528, 7-8=-879/1543, 5-7=-843/1530

WEBS

3-8=-274/464, 4-7=-284/468

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60

Provide adequate drainage to prevent water ponding.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=590, 5=590.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 223 lb down and 192 lb up at 7-0-0, and 223 lb down and 192 lb up at 9-0-0 on top chord, and 299 lb down and 285 lb up at 7-0-0, and 299 lb down and 285 lb up at 8-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-4=-54, 4-6=-54, 9-12=-20

Concentrated Loads (lb)

Vert: 3=-176(F) 4=-176(F) 8=-289(F) 7=-289(F)



Structural wood sheathing directly applied or 3-6-10 oc purlins.

Rigid ceiling directly applied or 6-2-15 oc bracing.

Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

June 12,2020

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property and property design. Bracing individual truss with possible personal injury and property and property design. Bracing individual truss with possible personal injury and property and property design. Bracing in the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



HTH - SPEC HSE SPARR LN Truss Type Qty Job Truss T20450305 Flat Girder 1 TG01 2374839 Job Reference (optional) B.240 s Mar 9 2020 MiTek Industries, Inc. Fri Jun 12 08:32:23 2020 Page 1 ID:uJkZZVTEhgKQntlaVi8jJMyZ6ft-src2vrG99xo_3frFlC03eEOApt_ZRWxxyZG57Sz74WM Jacksonville, FL - 32244, Builders FirstSource, Scale = 1:25.4 2x4 || 3x4 = 2 6 4x4 2x4 II PLATES GRIP LOADING (psf) (loc) SPACING-2-0-0 CSI. 244/190 Vert(LL) -0.06 3-4 >999 240 MT20 20.0 Plate Grip DOL 1.25 TC 0.41 TCLL 0.91 Vert(CT) -0.11 >532 180 1.25 BC TCDL 7.0 Lumber DOL 0.00 -0.00 n/a BCLL 0.0 Rep Stress Incr NO WB Horz(CT) 3 n/a FT = 20% Code FBC2017/TPI2014 Weight: 37 lb 10.0 Matrix-MP BCDL

> BRACING-TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 2x6 SP No.2 BOT CHORD

2x4 SP No.3 WEBS

REACTIONS. (size) 4=0-3-8, 3=Mechanical Max Uplift 4=-240(LC 4), 3=-183(LC 4) Max Grav 4=789(LC 1), 3=596(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Provide adequate drainage to prevent water ponding.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=240, 3=183,
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 518 lb down and 168 lb up at 1-0-12, and 518 lb down and 169 lb up at 3-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-2=-54, 3-4=-20 Concentrated Loads (lb)

Vert: 5=-518(F) 6=-518(F)

No 68182

No 68182

No 68182

No 68182

No 68182

Structural wood sheathing directly applied or 5-0-0 oc purlins,

Rigid ceiling directly applied or 6-10-8 oc bracing

except end verticals.

Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

June 12,2020

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Pracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/THI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

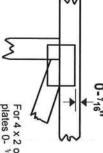


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y Apply plates to both sides of truss and fully embed teeth. offsets are indicated. Dimensions are in ft-in-sixteenths.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.

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required direction of slots in connector plates. This symbol indicates the

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE



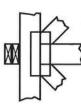
width measured perpendicular to slots. Second dimension is The first dimension is the plate the length parallel to slots.

LATERAL BRACING LOCATION



output. Use T or I bracing by text in the bracing section of the Indicated by symbol shown and/or

BEARING



number where bearings occur.

Min size shown is for crushing only. reaction section indicates joint Indicates location where bearings (supports) occur. Icons vary but

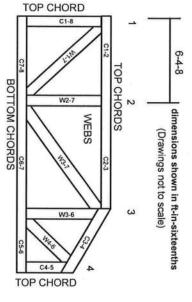
Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction. Design Standard for Bracing.

DSB-89:

Installing & Bracing of Metal Plate Connected Wood Trusses. Guide to Good Practice for Handling, Building Component Safety Information,

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

truss unless otherwise shown. Trusses are designed for wind loads in the plane of the

section 6.3 These truss designs rely on lumber values Lumber design values are in accordance with ANSI/TPI 1 established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer, For wide truss spacing, individual lateral braces themselves bracing should be considered. may require bracing, or alternative Tor I
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.

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designer, erection supervisor, property owner and all other interested parties. Provide copies of this truss design to the building

4.

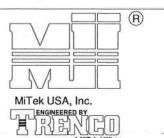
- Cut members to bear tightly against each other
- Place plates on each face of truss at each locations are regulated by ANSI/TPI 1. oint and embed fully. Knots and wane at joint
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the camber for dead load deflection responsibility of truss fabricator. General practice is to
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- 12. Lumber used shall be of the species and size, and specified in all respects, equal to or better than that
- Top chords must be sheathed or purlins provided at spacing indicated on design
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted
- Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable project engineer before use. environmental, health or performance risks. Consult with
- Review all portions of this design (front, back, words is not sufficient and pictures) before use. Reviewing pictures alone
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

			*	

T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

MII-T-BRACE 2

MiTek USA, Inc. Page 1 of 1



Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

Nailing Pattern						
T-Brace size	Nail Size	Nail Spacing				
2x4 or 2x6 or 2x8	10d (0.131" X 3")	6" o.c.				

Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)

Nails	
SPACING	
WEB WEB	
T-BR.	ACE
Nails Section Detail	
T-Brace Web	
155.555	

Nails	
Web	I-Brace
Nails	

	Brace Size for One-Ply Truss Specified Continuous Rows of Lateral Bracing					
Web Size	1	2				
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace				
2x6	2x6 T-Brace	2x6 I-Brace				
2x8	2x8 T-Brace	2x8 I-Brace				

		e Size -Ply Truss				
	Specified Continuous Rows of Lateral Bracing					
Web Size	1	2				
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace				
2x6	2x6 T-Brace	2x6 I-Brace				
2x8	2x8 T-Brace	2x8 I-Brace				

T-Brace / I-Brace must be same species and grade (or better) as web member.



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February 12, 2018

SCAB-BRACE DETAIL

MII-SCAB-BRACE

MiTek USA, Inc.

Page 1 of 1

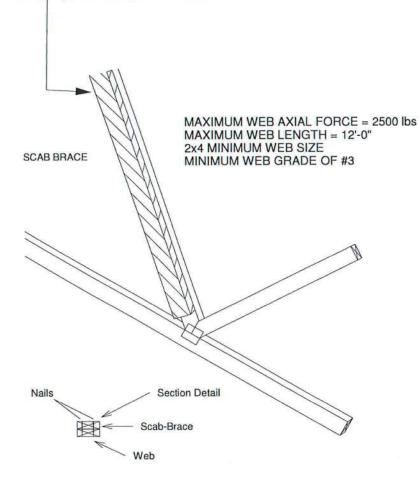


Note: Scab-Bracing to be used when continuous lateral bracing at midpoint (or T-Brace) is impractical.

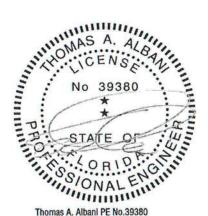
Scab must cover full length of web +/- 6".

*** THIS DETAIL IS NOT APLICABLE WHEN BRACING IS *** REQUIRED AT 1/3 POINTS OR I-BRACE IS SPECIFIED.

APPLY 2x___ SCAB TO ONE FACE OF WEB WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. SCAB MUST BE THE SAME GRADE, SIZE AND SPECIES (OR BETTER) AS THE WEB.



Scab-Brace must be same species grade (or better) as web member.



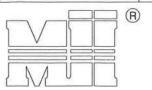
Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

February 12, 2018

STANDARD REPAIR TO REMOVE END VERTICAL (RIBBON NOTCH VERTICAL)

MII-REP05

MiTek USA, Inc. Page 1 of 1



MiTek USA, Inc. ENGINEERED BY VICTO 1. THIS IS A SPECIFIC REPAIR DETAIL TO BE USED ONLY FOR ITS ORIGINAL INTENTION, THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING

THE LOADS INDICATED.

2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLYING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.

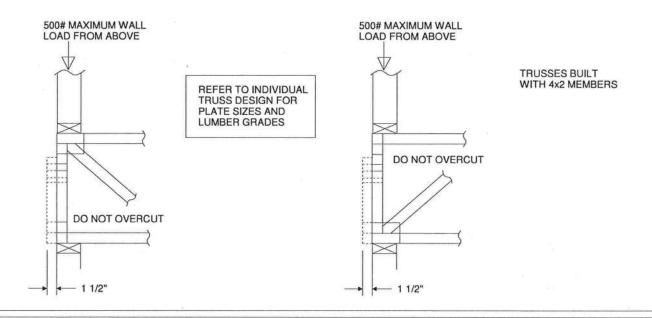
3. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE

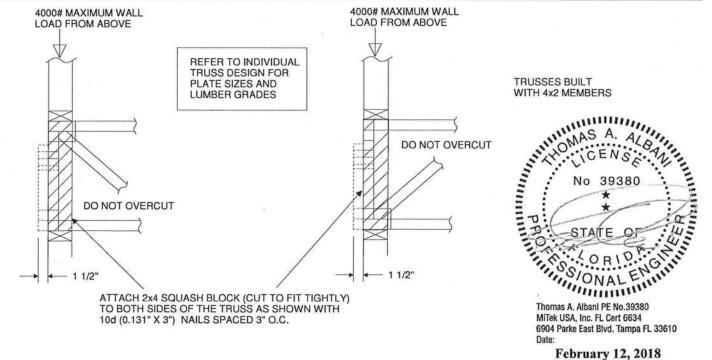
SUCH AS TO AVOID SPLITTING OF THE WOOD.

4. LUMBER MUST BE CUT CLEANLY AND ACCURATELY AND THE REMAINING WOOD MUST BE UNDAMAGED.

5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 4X_ORIENTATION ONLY.

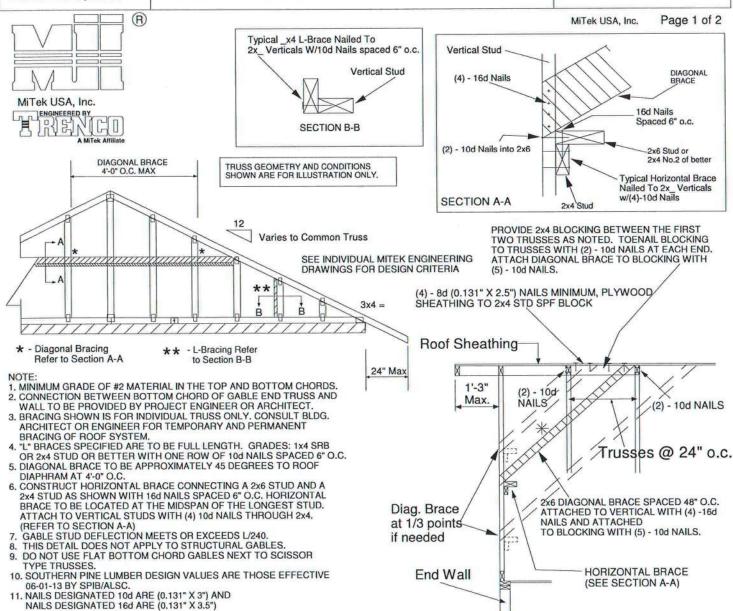
6. CONNECTOR PLATES MUST BE FULLY IMBEDDED AND UNDISTURBED.





Standard Gable End Detail

MII-GE130-D-SP



Minimum Stud Size	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS		
Species and Grade		Maximum Stud Length						
2x4 SP No. 3 / Stud	12" O.C.	3-9-13	4-1-1	5-9-6	7-1-3	11-5-7		
2x4 SP No. 3 / Stud	16" O.C.	3-5-4	3-6-8	5-0-2	6-10-8	10-3-13		
2x4 SP No. 3 / Stud	24" O.C.	2-9-11	2-10-11	4-1-1	5-7-6	8-5-1		

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE D ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH ASCE 7-10 160 MPH DURATION OF LOAD INCREASE: 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.

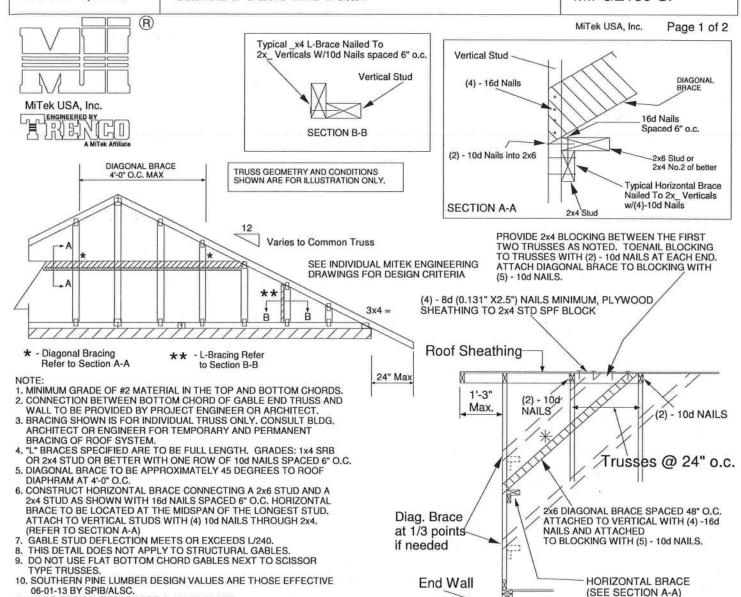


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February 12, 2018

Standard Gable End Detail

MII-GE130-SP



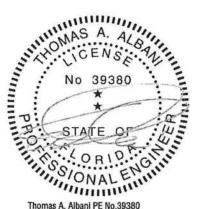
Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
		Maximum Stud Length				
2x4 SP No. 3 / Stud	12" O.C.	4-0-7	4-5-6	6-3-8	8-0-15	12-1-6
2x4 SP No. 3 / Stud	16" O.C.	3-8-0	3-10-4	5-5-6	7-4-1	11-0-1
2x4 SP No. 3 / Stud	24" O.C.	3-0-10	3-1-12	4-5-6	6-1-5	9-1-15

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

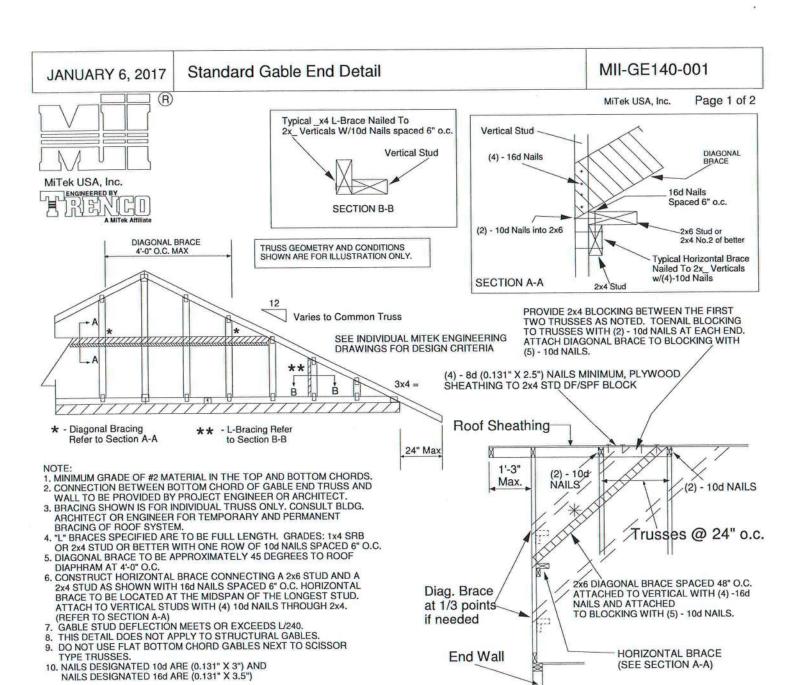
MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH ASCE 7-10 160 MPH **DURATION OF LOAD INCREASE: 1.60**

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date

February 12, 2018

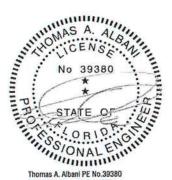


Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
		Maximum Stud Length				
2x4 DF/SPF Std/Stud	12" O.C.	3-10-1	3-11-7	5-7-2	7-8-2	11-6-4
2x4 DF/SPF Std/Stud	16" O.C.	3-3-14	3-5-1	4-10-2	6-7-13	9-11-11
2x4 DF/SPF Std/Stud	24" O.C.	2-8-9	2-9-8	3-11-7	5-5-2	8-1-12

Diagonal braces over 6-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAXIMUM WIND SPEED = 140 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.

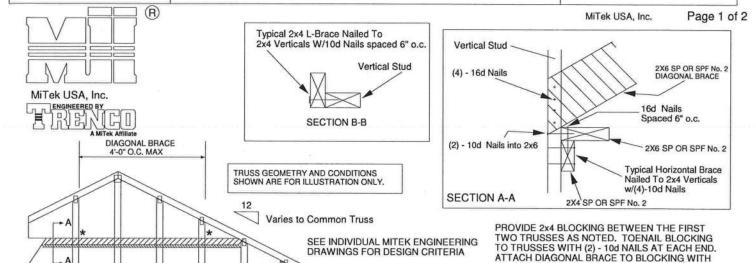


Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

January 19, 2018

Standard Gable End Detail

MII-GE170-D-SP



DRAWINGS FOR DESIGN CRITERIA

24" Max

Diag. Brace

at 1/3 points

End Wall

if needed

3x4 =

(5) - 10d NAILS

SHEATHING TO 2x4 STD SPF BLOCK

Roof Sheathing

1'-0"

Max.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD,

- 10d

NAILS

- Diagonal Bracing Refer to Section A-A

** - L-Bracing Refer to Section B-B

**

NOTE:

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS. 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3 OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.

5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

DIAPHRAM AT 4'-0" O.C.

6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)

7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.

THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.

11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS			
Species and Grade		Maximum Stud Length						
2x4 SP No. 3 / Stud	12" O.C.	3-9-7	5-8-8	6-11-1	11-4-4			
2x4 SP No. 3 / Stud	16" O.C.	3-4-12	4-11-15	6-9-8	10-2-3			
2x4 SP No. 3 / Stud	24" O.C.	2-9-4	4-0-7	5-6-8	8-3-13			
2x4 SP No. 2	12" O.C.	3-11-13	5-8-8	6-11-1	11-11-7			
2x4 SP No. 2	16" O.C.	3-7-7	4-11-5	6-11-1	10-10-5			
2x4 SP No. 2	24" O.C.	3-1-15	4-0-7	6-3-14	9-5-14			

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

MAX MEAN ROOF HEIGHT = 30 FEET EXPOSURE D ASCE 7-10 170 MPH DURATION OF LOAD INCREASE: 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



(2) - 10d NAILS

Trusses @ 24" o.c.

2x6 DIAGONAL BRACE SPACED

48" O.C. ATTACHED TO VERTICAL WITH

HORIZONTAL BRACE (SEE SECTION A-A)

(4) -16d NAILS, AND ATTACHED TO BLOCKING WITH (5) -10d NAILS.

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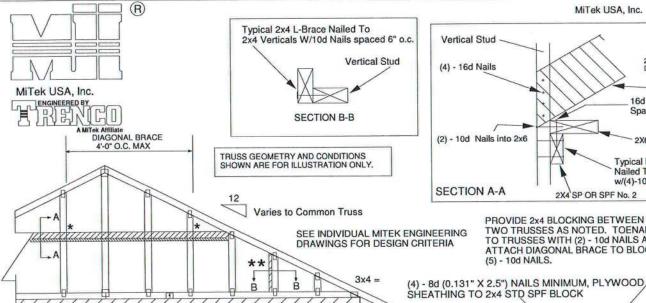
Standard Gable End Detail

MII-GE180-D-SP

Page 1 of 2

2X6 SP OR SPF No. 2 DIAGONAL BRACE

16d Nails



24" Max

NOTE:

- Diagonal Bracing

Refer to Section A-A

I. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.

2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

- L-Bracing Refer

to Section B-B

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3 OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C. 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

DIAPHRAM AT 4'-0" O.C.

6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)

GABLE STUD DEFLECTION MEETS OR EXCEEDS L'240.

THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES

DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.

11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

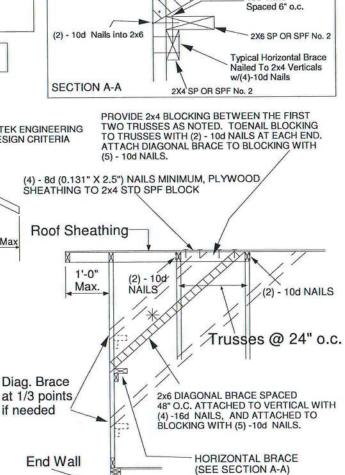
Minimum Stud Size	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS		
Species and Grade	180 80	Maximum Stud Length					
2x4 SP No. 3 / Stud	12" O.C.	3-7-12	5-4-11	6-2-1	10-11-3		
2x4 SP No. 3 / Stud	16" O.C.	3-2-8	4-8-1	6-2-1	9-7-7		
2x4 SP No. 3 / Stud	24" O.C.	2-7-7	3-9-12	5-2-13	7-10-4		
2x4 SP No. 2	12" O.C.	3-10-0	5-4-11	6-2-1	11-6-1		
2x4 SP No. 2	16" O.C.	3-5-13	4-8-1	6-2-1	10-5-7		
2x4 SP No. 2	24" O.C.	3-0-8	3-9-12	6-1-1	9-1-9		

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6in o.c., with 3in minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

MAX MEAN ROOF HEIGHT = 30 FEET EXPOSURE D ASCE 7-10 180 MPH

DURATION OF LOAD INCREASE: 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.

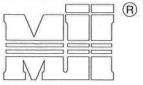




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STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

MiTek USA, Inc. Page 1 of 1



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ENGINEERED BY

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING EXPOSURE B or C **ASCE 7-10 DURATION OF LOAD INCREASE: 1.60**

DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.

A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.

A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING. SHALL BE CONNECTED TO EACH PURLIN WITH (2) (0.131" X 3.57 TOE-NAILED.

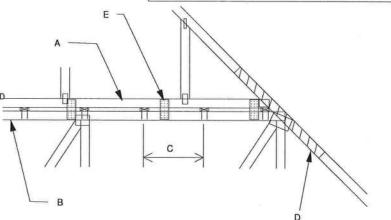
B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.

C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH,

D - 2 X _ X 4"-0" SCAB, SIZE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED. ON INTERSECTION, WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:

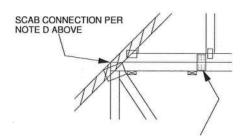
IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT, IN BOTH DIRECTIONS AND: 1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR 2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.

E - FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C. W/ (4) (0.131" X 1.5") NAILS PER MEMBER. STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5" EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)

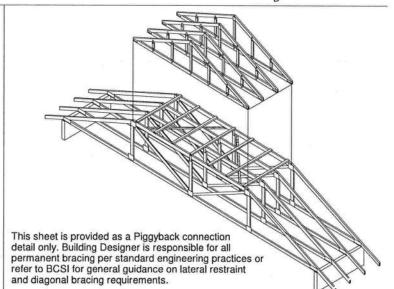


WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

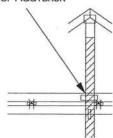
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



FOR ALL WIND SPEEDS, ATTACH MITEK 3X6 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) (0.131" X 1.5") PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE.



VERTICAL WEB TO EXTEND THROUGH **BOTTOM CHORD** OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL

AS SHOWN IN DE I AIL.

2) ATTACH 2 × ___ × 4'-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.)

(MINIMUM 2X4) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15), REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.

FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS. NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
CONCENTRATED LOAD MUST BE APPLIED TO BOTH
THE PIGGYBACK AND THE BASE TRUSS DESIGN.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

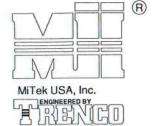
MII-PIGGY-ALT 7-10

MiTek USA, Inc. Page 1 of 1

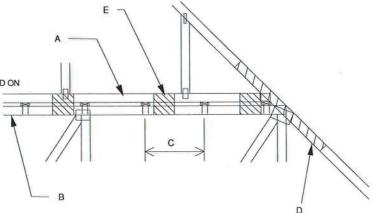
MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING **EXPOSURE B or C ASCE 7-10 DURATION OF LOAD INCREASE: 1.60**

DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES).

ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.

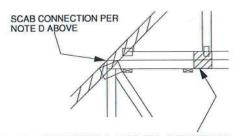


A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.

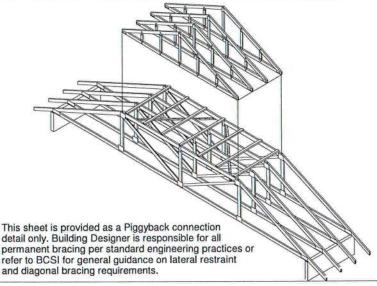


WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

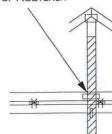
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH PLYWOOD GUSSETS AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



7" x 7" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 24" O.C. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)



VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.

ATTACH 2 X ___ x 4'-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.)

(MINIMUM 2X4) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS

FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS,

NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS. CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN.



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STANDARD REPAIR DETAIL FOR BROKEN CHORDS, WEBS AND DAMAGED OR MISSING CHORD SPLICE PLATES

MII-REP01A1

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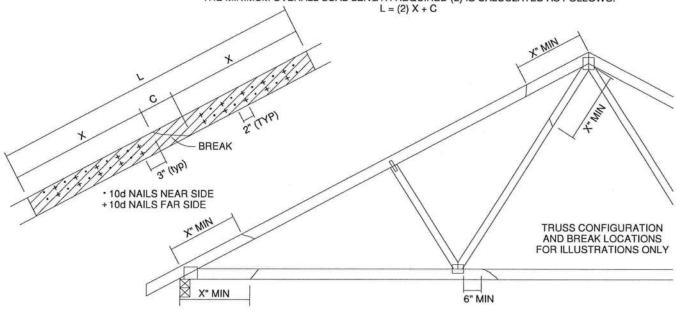


TOTAL NUMBER OF NAILS EACH SIDE OF BREAK *		MAXIMUM FORCE (lbs) 15% LOAD DURATION								
			SP		DF		SPF		HF	
2x4	2x6		2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6
20	30	24"	1706	2559	1561	2342	1320	1980	1352	2028
26	39	30"	2194	3291	2007	3011	1697	2546	1738	2608
32	48	36"	2681	4022	2454	3681	2074	3111	2125	3187
38	57	42"	3169	4754	2900	4350	2451	3677	2511	3767
44	66	48"	3657	5485	3346	5019	2829	4243	2898	4347

* DIVIDE EQUALLY FRONT AND BACK

ATTACH 2x SCAB OF THE SAME SIZE AND GRADE AS THE BROKEN MEMBER TO EACH FACE OF THE TRUSS (CENTER ON BREAK OR SPLICE) WITH 10d (0.131" X 3") NAILS (TWO ROWS FOR 2x4, THREE ROWS FOR 2x6) SPACED 4" O.C. AS SHOWN. STAGGER NAIL SPACING FROM FRONT FACE AND BACK FACE FOR A NET 0-2-0 O.C. SPACING IN THE MAIN MEMBER. USE A MIN. 0-3-0 MEMBER END DISTANCE.

THE LENGTH OF THE BREAK (C) SHALL NOT EXCEED 12". (C=PLATE LENGTH FOR SPLICE REPAIRS) THE MINIMUM OVERALL SCAB LENGTH REQUIRED (L) IS CALCULATED AS FOLLOWS:



THE LOCATION OF THE BREAK MUST BE GREATER THAN OR EQUAL TO THE REQUIRED X DIMENSION FROM ANY PERIMETER BREAK OR HEEL JOINT AND A MINIMUM OF 6" FROM ANY INTERIOR JOINT (SEE SKETCH ABOVE)

DO NOT USE REPAIR FOR JOINT SPLICES

- NOTES:

 1. THIS REPAIR DETAIL IS TO BE USED ONLY FOR THE APPLICATION SHOWN. THIS REPAIR DOES

 NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS

 SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED

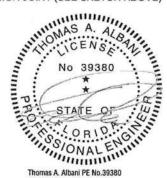
 REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.

 2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLING REPAIR

 AND HELD IN PLACE DURING APPLICATION OF REPAIR.
- THE END DISTANCE, EDGE DISTANCE AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
 WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID
- LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.

 THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 2x_ ORIENTATION ONLY.

 THIS REPAIR IS LIMITED TO TRUSSES WITH NO MORE THAN THREE BROKEN MEMBERS.



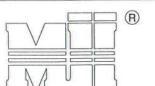
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LATERAL TOE-NAIL DETAIL

MII-TOENAIL SP

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NOTES:

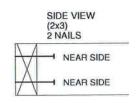
1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.

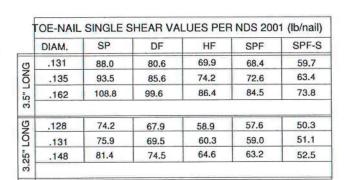
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.

3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

TUBER FUR DETAIL O CUIONNI DEL OL	
THREE END DETAILS SHOWN BELOV	٨

VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY



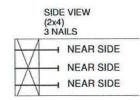


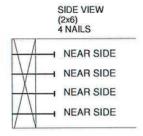
VALUES SHOWN ARE CAPACITY PER TOE-NAIL APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

(3) - 16d (0.162" X 3.5") NAILS WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1.15:

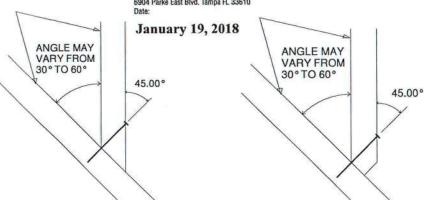
3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity

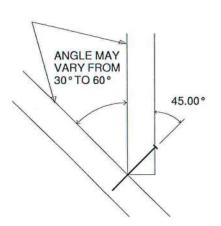






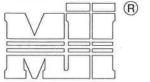
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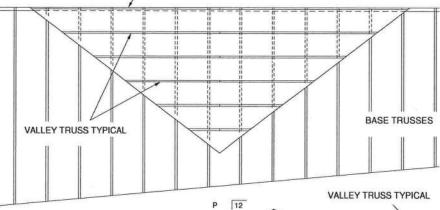
MiTek USA, Inc.

ENGINEERED BY

GABLE END, COMMON TRUSS OR GIRDER TRUSS

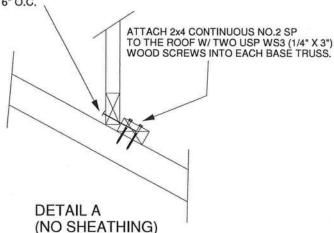
GENERAL SPECIFICATIONS

- NAIL SIZE 10d (0.131" X 3")
 WOOD SCREW = 3" WS3 USP OR EQUIVALENT DO NOT USE DRYWALL OR DECKING TYPE SCREW
 INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND
- SECURE PER DETAIL A
- 4. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
- 5. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING
- EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
- 6. NAILING DONE PER NDS 01
- 7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.



GABLE END, COMMON TRUSS OR GIRDER TRUSS SEE DETAIL A BELOW (TYP.)

SECURE VALLEY TRUSS W/ ONE ROW OF 10d NAILS 6" O.C.



N.T.S.

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12 CATEGORY II BUILDING **EXPOSURE C** WIND DURATION OF LOAD INCREASE: 1.60 MAX TOP CHORD TOTAL LOAD = 50 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 6 PSF

ON THE TRUSSES

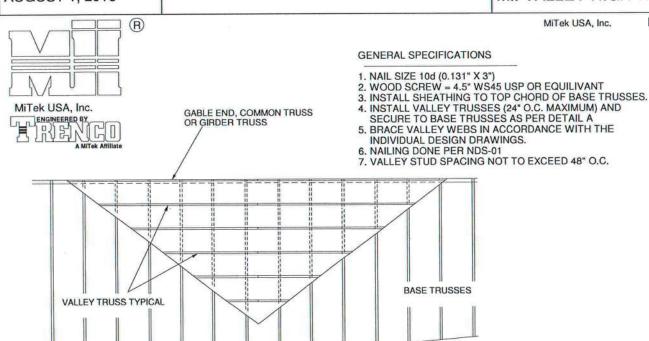


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TRUSSED VALLEY SET DETAIL

MII-VALLEY HIGH WIND2

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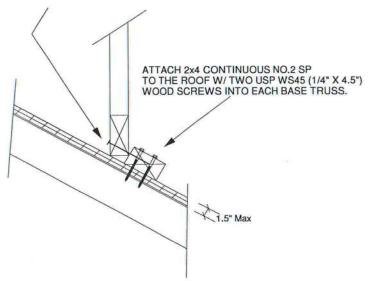


VALLEY TRUSS TYPICAL

GABLE END, COMMON TRUSS OR GIRDER TRUSS

SEE DETAIL
A BELOW (TYP.)

SECURE VALLEY TRUSS W/ ONE ROW OF 10d NAILS 6" O.C.



WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH
WIND DESIGN PER ASCE 7-10 160 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12
CATEGORY II BUILDING
EXPOSURE C
WIND DURATION OF LOAD INCREASE: 1.60
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 6 PSF
ON THE TRUSSES



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

MiTek USA, Inc.

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MiTek USA, Inc.

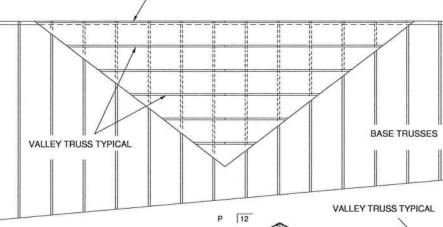
Page 1 of 1



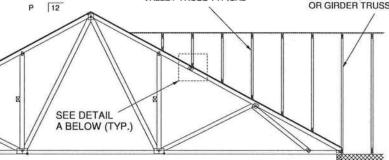
GABLE END, COMMON TRUSS

OR GIRDER TRUSS

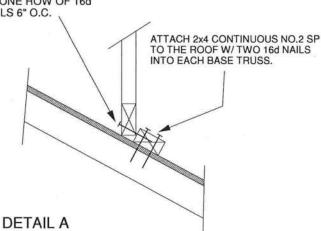
- 1. NAIL SIZE 16d (0.131" X 3.5")
- INSTALL VALLÈY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
- BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
- BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
- 5. NAILING DONE PER NDS 01
- 6. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.
- 7. ALL LUMBER SPECIES TO BE SP.



GABLE END, COMMON TRUSS OR GIRDER TRUSS



SECURE VALLEY TRUSS W/ ONE ROW OF 16d NAILS 6" O.C.



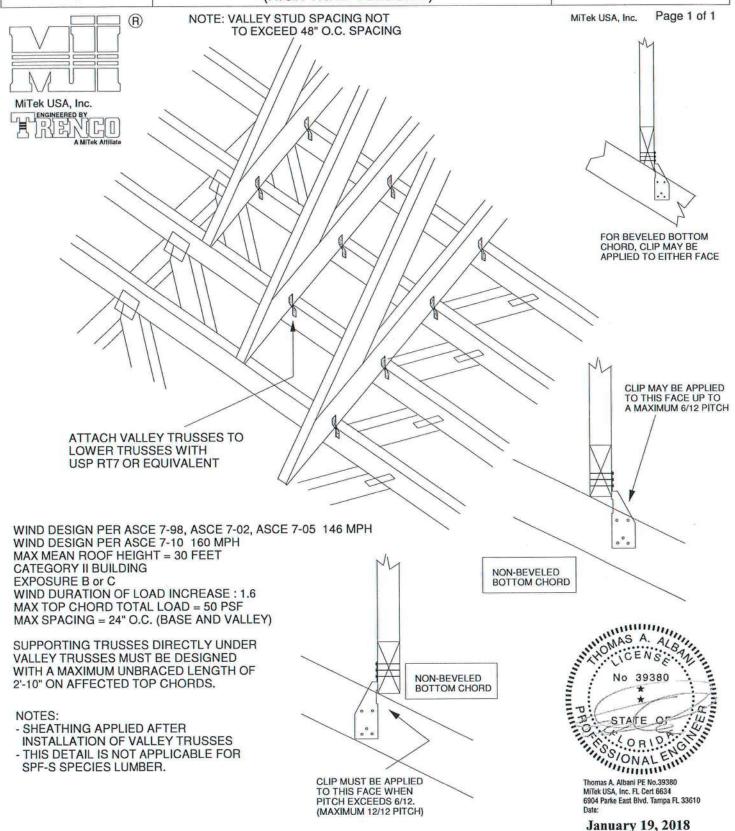
(MAXIMUM 1" SHEATHING) N.T.S. WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 120 MPH WIND DESIGN PER ASCE 7-10 150 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 10/12 CATEGORY II BUILDING EXPOSURE C OR B WIND DURATION OF LOAD INCREASE: 1.60 MAX TOP CHORD TOTAL LOAD = 60 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 4.2 PSF ON THE TRUSSES



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

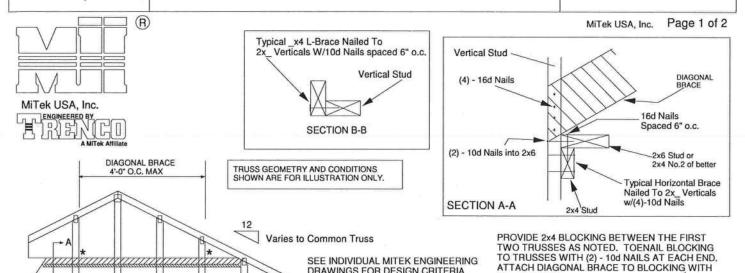
TRUSSED VALLEY SET DETAIL (HIGH WIND VELOCITY)

MII-VALLEY



Standard Gable End Detail

MII-GE146-001



DRAWINGS FOR DESIGN CRITERIA

24" Max

3x4 =

- Diagonal Bracing Refer to Section A-A

- L-Bracing Refer to Section B-B

NOTE:

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.

2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND

WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG.
ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 2x4 No 3/STUD SP OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.

5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

DIAPHRAM AT 4-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)

7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR

TYPE TRUSSES.

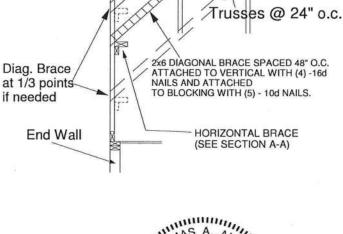
10. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS			
Species and Grade		Maximum Stud Length						
2x4 SP No 3/Stud	12" O.C.	3-11-3	6-8-0	7-2-14	11-9-10			
2x4 SP No 3/Stud	16" O.C.	3-6-14	5-9-5	7-1-13	10-8-11			
2x4 SP No 3/Stud	24" O.C.	3-1-8	4-8-9	6-2-15	9-4-7			

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAXIMUM WIND SPEED = 146 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



(5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SP BLOCK

- 10d

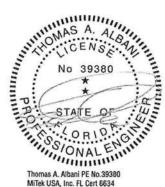
(2) - 10d NAILS

NAILS

Roof Sheathing

1'-3"

Max.



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OCTOBER 5, 2016

REPLACE BROKEN OVERHANG

MII-REP13B

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(R)

MiTek USA, Inc.

ENGINEERED BY A MiTek Affilia TRUSS CRITERIA:

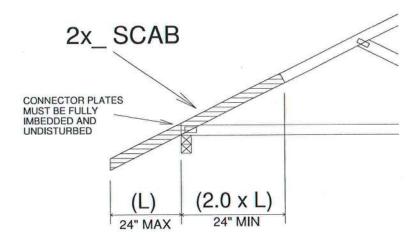
LOADING: 40-10-0-10 **DURATION FACTOR: 1.15** SPACING: 24" O.C. TOP CHORD: 2x4 OR 2x6 PITCH: 4/12 - 12/12

HEEL HEIGHT: STANDARD HEEL UP TO 12" ENERGY HEEL

END BEARING CONDITION

NOTES:

1. ATTACH 2x_ SCAB (MINIMUM NO.2 GRADE SPF, HF, SP, DF) TO ONE FACE OF TRUSS WITH TWO ROWS OF 10d (0.131" X 3") SPACED 6" O.C.
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.



This detail to be used only with trusses (spans less than 40') spaced 24" o.c. maximum and having pitches between 4/12 and 12/12 and total top chord loads not exceeding 50 psf. Trusses not fitting these criteria should be examined individually.

REFER TO INDIVIDUAL TRUSS DESIGN FOR PLATE SIZES AND LUMBER GRADES



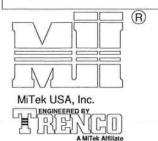
Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

LATERAL BRACING RECOMMENDATIONS

MII-STRGBCK

MiTek USA, Inc.

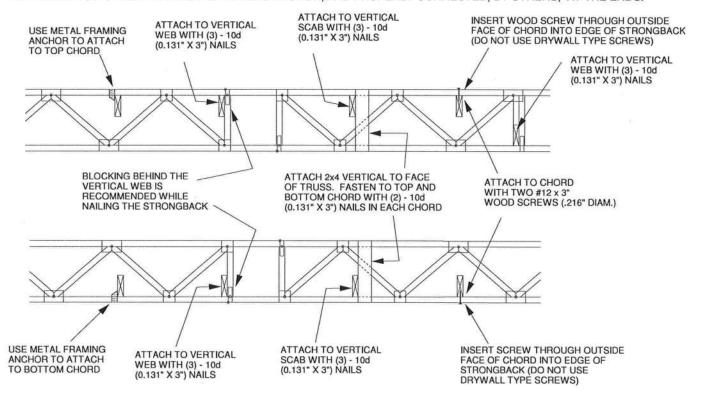
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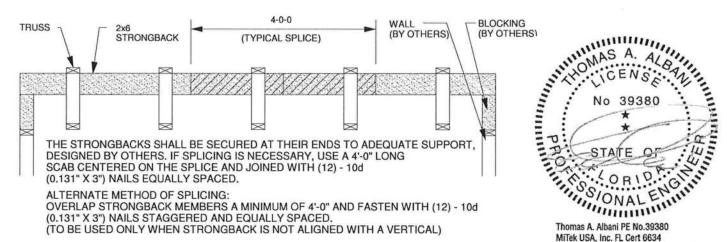


TO MINIMIZE VIBRATION COMMON TO ALL SHALLOW FRAMING SYSTEMS, 2x6 "STRONGBACK" IS RECOMMENDED, LOCATED EVERY 8 TO 10 FEET ALONG A FLOOR TRUSS.

NOTE 1: 2X6 STRONGBACK ORIENTED VERTICALLY MAY BE POSITIONED DIRECTLY UNDER THE TOP CHORD OR DIRECTLY ABOVE THE BOTTOM CHORD. SECURELY FASTENED TO THE TRUSS USING ANY OF THE METHODS ILLUSTRATED BELOW.

NOTE 2: STRONGBACK BRACING ALSO SATISFIES THE LATERAL BRACING REQUIREMENTS FOR THE BOTTOM CHORD OF THE TRUSS WHEN IT IS PLACED ON TOP OF THE BOTTOM CHORD, IS CONTINUOUS FROM END TO END, CONNECTED WITH A METHOD OTHER THAN METAL FRAMING ANCHOR, AND PROPERLY CONNECTED, BY OTHERS, AT THE ENDS.





6904 Parke East Blvd. Tampa FL 33610 Date:

and grant Williams Control