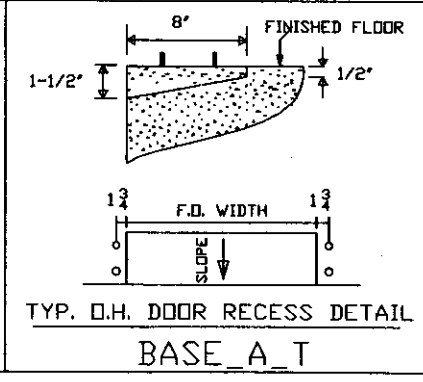
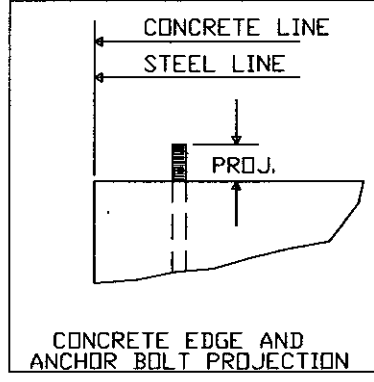
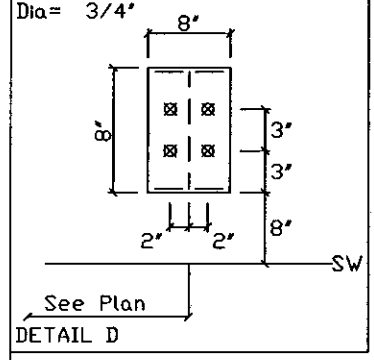
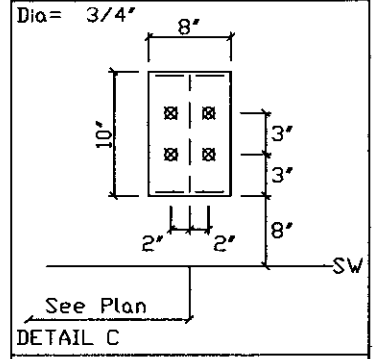
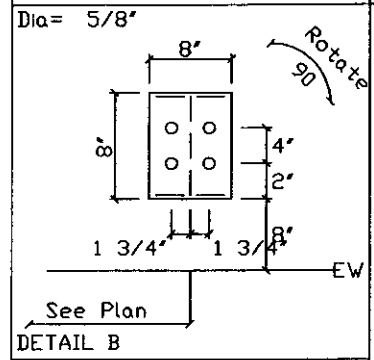
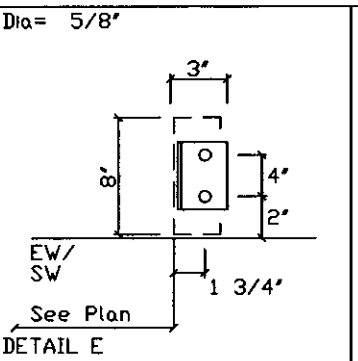
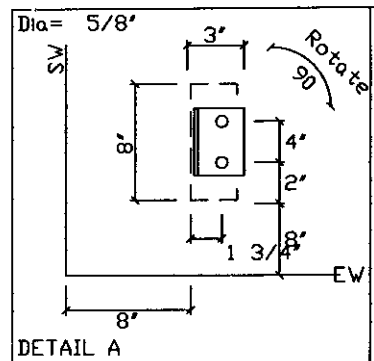
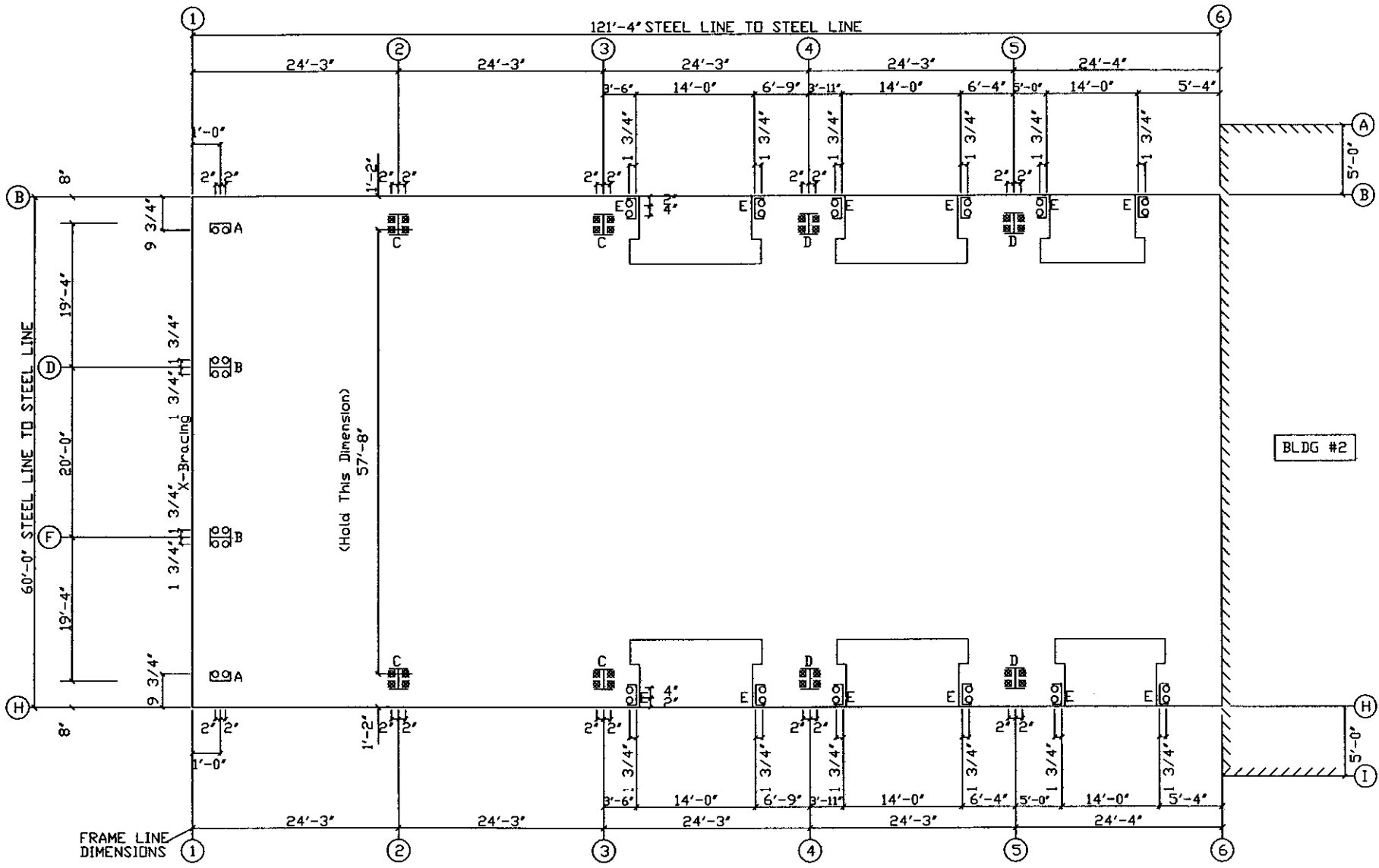


ENGINEERING CALCULATIONS			GENERAL NOTES			FLORIDA PRODUCT APPROVAL INFORMATION		
LIFETIME STEEL BLDGS		DATE10/ 7/08	1. MATERIALS			APPROVAL #		
Job Number 19066-1		DESIGNED BY:	STRUCTURAL STEEL PLATE			22 GA. 22 BATTENLOK PANEL - ROOF PANEL		
End User LIFETIME STEEL BLDGS		CHECKED BY: DB	COLD FORMED LIGHT GAGE SHAPES			FL 7353.1		
Job Location: LAKE CITY, FL 32024		DRAWN BY : ARM/MMD	BRACE RODS			26 GA. 26 PBR - WALL PANEL		
			HOT ROLLED MILL SHAPES			FL 5335.1		
			ROOF AND WALL SHEETS			9" X 10' RIDGE VENT		
			BOLTS			FL 8069.1		
			BUILT-UP MEMBERS					
DESIGN PARAMETERS		COMMENTS	2. STRUCTURAL PRIMER					
BUILDING DESCRIPTION			SHOP PRIMER PAINT IS A RUST INHIBITIVE PRIMER AND IT'S COLOR IS RED OXIDE. THIS PAINT IS NOT INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS.					
NOMINAL WIDTH: 60 feet			3. A325 BOLT TIGHTENING REQUIREMENTS					
NOMINAL LENGTH: 121.33 feet			ALL HIGH STRENGTH BOLTS ARE A325-N UNLESS SPECIFICALLY NOTED OTHERWISE.					
EAVE HEIGHT, BACK S.W: 17.25 feet			STRUCTURAL BOLTS SHALL BE TIGHTENED BY THE TURN-OF-THE-NUT METHOD IN ACCORDANCE WITH THE 13th EDITION AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS". PER SECTION 8.2, A325 BOLTS MAY BE INSTALLED WITHOUT WASHERS WHEN TIGHTENED BY THE TURN-OF-THE NUT METHOD.					
EAVE HEIGHT, FRONT S.W: 17.25 feet			ALL HIGH STRENGTH BOLTS, EXCEPT AS NOTED OTHERWISE, ARE SUBJECT TO DIRECT TENSION AND MAY REQUIRE INSPECTION AS DEFINED BY THE APPLICABLE BUILDING CODE OR STANDARD. IT IS THE RESPONSIBILITY OF THE ERECTOR TO ASSURE PROPER TIGHTNESS.					
ROOF SLOPE, LEFT: 3.0:12			4. BUILDER/CONTRACTOR RESPONSIBILITIES					
ROOF SLOPE, RIGHT: 3.0:12			THE METAL BUILDING MANUF.'S STANDARD PRODUCT SPECIFICATIONS APPLY AND UNLESS STIPULATED OTHERWISE IN THE CONTRACT DOCUMENTS, THE METAL BUILDING MANUF.'S DESIGN, FABRICATION, QUALITY CRITERIA STANDARDS AND TOLERANCES WILL GOVERN THE WORK.					
DESIGN LOADS			IN CASE OF DISCREPANCIES BETWEEN METAL BUILDINGS MANUF. STRUCTURAL PLANS AND PLANS FOR OTHER TRADES, THE METAL BUILDING MANUF.'S PLANS SHALL GOVERN.					
BUILDING CODE: FBC 06		SNOW :	IT IS THE RESPONSIBILITY OF THE BUILDER / CONTRACTOR TO OBTAIN APPROPRIATE APPROVALS AND NECESSARY PERMITS FROM CITY, COUNTY, STATE, OR FEDERAL AGENCIES, AS REQUIRED.					
FRAME SELF WEIGHT: INCLUDED		FLAT ROOF SNOW LOAD Pf : 0 psf	APPROVAL OF METAL BUILDING MANUF.'S DRAWINGS CONSTITUTES THE BUILDER / CONTRACTOR'S ACCEPTANCE OF THE METAL BUILDING MANUF.'S INTERPRETATION OF THE CONTRACT PURCHASE ORDER.					
ROOF DEAD LOAD: 2.000 psf		GROUND SNOW LOAD Pg : 0.0000 psf	ONCE THE BUILDER / CONTRACTOR OR A/E FIRM HAS SIGNED MANUF.'S APPROVAL PACKAGE, CHANGES FROM THE PURCHASE ORDER BY THE BUILDER WILL BE BILLED TO THE BUILDER / CONTRACTOR FOR MATERIAL, ENGINEERING AND HANDLING FEES. SUCH CHANGES MAY CAUSE THE PROJECT TO BE MOVED FROM THE FABRICATION AND / OR SHIPPING SCHEDULE. A PENALTY FEE MAY BE CHARGED IF THE PROJECT MUST BE MOVED FROM THE FABRICATION AND / OR SHIPPING SCHEDULE, AS LONG AS THE MANUF.'S DESIGN AND DETAILING APPROACH COMPLIES WITH THE PURCHASE ORDER.					
COLLATERAL LOAD: 5 psf		SNOW LOAD IMP. FACTOR 1.2000	THE BUILDER / CONTRACTOR OR A/E FIRM ARE RESPONSIBLE FOR THE OVERALL PROJECT CONDITION. ALL INTERFACE AND COMPATIBILITY CONCERNING ANY MATERIALS NOT FURNISHED BY THE MANUF. ARE TO BE CONSIDERED AND COORDINATED BY THE BUILDER / CONTRACTOR OR A/E FIRM. UNLESS SPECIFIC DESIGN CRITERIA CONCERNING THIS INTERFACE BETWEEN MATERIALS IS FURNISHED AS PART OF THE PURCHASE ORDER, THE METAL BUILDING MANUF.'S ASSUMPTIONS WILL GOVERN.					
LIVE LOAD: 20.00 psf		THERMAL FACTOR Ct :	THE BUILDER / CONTRACTOR IS RESPONSIBLE TO INSURE THAT ALL OTHER PROJECT PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITY. SUPPLYING SEALED ENGINEERING DESIGN DATA AND DRAWINGS BY THE BUILDING MANUF. DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT THE BUILDING MANUF. OR ITS DESIGN ENGINEER IS ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR THE CONSTRUCTION PROJECT. THESE DRAWINGS AND DESIGN DATA ARE SEALED AS TO THE STRUCTURAL SYSTEM FURNISHED BY THE METAL BUILDING MANUF. IN COMPLIANCE WITH ALL REQUIREMENTS OF THE PURCHASE ORDER.					
SNOW LOAD, ROOF: 0 psf		SNOW EXP. FACTOR Ce :	THE BUILDER / CONTRACTOR IS RESPONSIBLE FOR SETTING OF ANCHOR BOLTS AND ERECTION OF STEEL BUILDING COMPONENTS IN ACCORDANCE WITH THE METAL BUILDING MANUF.'S "FOR CONSTRUCTION" DRAWINGS. TEMPORARY SUPPORTS OR BRACING REQUIRED FOR THE BUILDING ERECTION WILL BE THE RESPONSIBILITY OF THE ERECTOR TO DETERMINE, FURNISH, AND INSTALL.					
WIND SPEED: (3 SEC GUST) 140 mph			THE METAL BUILDING MANUF. DOES NOT WARRANT STRUCTURAL INTEGRITY OF ANY COMPONENTS FIELD MODIFIED OR DESIGNED AND FABRICATED BY OTHERS. NEITHER DO WE ACCEPT DESIGN RESPONSIBILITY FOR THE EFFECTS NON STANDARD COMPONENTS DESIGNED BY OTHERS MAY HAVE ON THE SYSTEM IN GENERAL.					
WIND EXPOSURE: B								
CLOSURE "C, D, P" : Closed								
IMP. FACTOR, WIND : 1.15								
OCCUPANCY CATEGORY : IV - ESSENTIAL FACILITIES								
SEISMIC PARAMETERS:								
SEISMIC IMPORTANCE: 1.50								
SEISMIC RESPONSE COEF. : 0.057								
SEISMIC DESIGN CATEGORY: D								
DESIGNED SPECTRAL ACCELERATION PARAMETER "SDS" - (SHORT PERIODS): 0.1333								
DESIGNED SPECTRAL ACCELERATION PARAMETER "SD1" - (1 SEC PERIODS): 0.1008								
MAPPED SPEC. RESPONSE ACC. "SS" - (SHORT PERIODS): 0.1250								
MAPPED SPEC. RESPONSE ACC. "S1" - (1 SEC PERIOD): 0.0630								
RESPONSE MODIFICATION FACTOR: 3.5								
SITE CLASS (ASSUMED) : D								
BASIC SEISMIC-FORCE RESISTING SYSTEM MOMENT RESISTING FRAME								
DESIGN BASE SHEAR : V=C _s * W								
ANALYSIS PROCEDURE : EQUIVALENT LATERAL FORCE PROCEDURE								
DRAWING RELEASE HISTORY		DRAWING REVISIONS		SPECIAL NOTE: BUILDING IS NOT STRUCTURALLY SOUND UNTIL ALL WALL COVERING, ROOF SHEETS, AND PERMANENT BRACING IS INSTALLED. BUILDER / CONTRACTOR IS RESPONSIBLE FOR SUPPORTS OR TEMPORARY BRACING DURING ERECTION, HE SHALL FURNISH, AND INSTALL THESE TEMPORARY SUPPORTS WHERE NECESSARY. TEMPORARY SUPPORTS ARE NOT PROVIDED BY THE METAL BUILDING MANUFACTURER.				
DATE SENT	REASON FOR RELEASE	REVISION	DATE				BY	DESCRIPTION
A								
B								
C								
D								
E								
F								
G								
H								
I								
J								
K								
L								
M								
ENGINEER'S STAMP								
OCT 09 2008								



ANCHOR BOLT NOTES:
VALUES GIVEN FOR BENDS AND ANCHOR BOLT TOTAL LENGTHS ARE SUGGESTED LENGTHS ONLY. IT IS THE RESPONSIBILITY OF THE FOUNDATION ENGINEER TO DETERMINE THESE VALUES SINCE THEY ARE A FUNCTION OF CONCRETE STRENGTH AND OTHER FACTORS.

ANCHOR BOLT SUMMARY					
Qty	Loc	Dia (in)	Total Len (in)	Bend Len (in)	Proj (in)
24	DJ	5/8"	9.00	3.00	3.00
12	EW	5/8"	12.00	3.00	3.00
32	RF	3/4"	18.00	3.00	3.00



ANCHOR BOLT PLAN
NOTE: All Base Plates @ 100'-0" (U.N.)

REVISIONS						
REV.	DESCRIPTION	DATE	DLR	DATE	CHKR	APPR

DRAWING STATUS
[X] FOR CONSTRUCTION
[] FOR PERMIT ONLY
[] FOR APPROVAL
[] OTHER, EXPLAIN

LIFETIME STEEL BLDGS	
PROJECT	6000' x 121.3 x 17.2
ID	19066-1
PROJECT ADDRESS	LAKE CITY, FL 32024

LIFETIME STEEL BLDGS	
ANCHOR BOLT PLAN & DETAILS	
DESIGN	DRAFT: ARM/MMD CHECK: DB
DATE: 10/07/08	SHEET 1

SEAL

OCT 09 2008

NOTES FOR REACTIONS

Building reactions are based on the following building data on

Width (ft) = 60.0
Length (ft) = 121.3
Eave Height (ft) = 17.3/ 17.3
Roof Slope (rise/12) = 3.0/ 3.0
Dead Load (psf) = 2.0
Collateral Load (psf) = 5.0
Roof Live Load (psf) = 20.0
Frame Live Load (psf) = 12.0
Wind Speed (mph) = 140.0
Wind Code (3 sec gust) = FBC 06
Exposure = B
Closed/Open = C
Importance Wind = 1.15
Seismic Zone = B
Seismic Coeff (F_{as}S_s) = 0.20

1 DL+CL+LL
2 0.60DL+WLI
3 0.60DL+WLI
4 0.60DL+WLI+VS
5 0.60DL+WLI+Wind
6 0.60DL+WLI+VS

GENERAL NOTES

- (1) APPLICATION OF ENGINEER'S SEAL IS FOR METAL BUILDING ONLY AND DOES NOT REPRESENT THE PROFESSIONAL OF RECORD.
- (2) FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF THE METAL BUILDING MANUFACTURER.
- (3) ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLERANCE OF +/- 1/8" IN BOTH ELEVATION AND LOCATION.
- (4) THE BUILDING REACTION DATA REPORTS THE LOADS WHICH THIS BUILDING PLACES ON THE FOUNDATION. THE FOUNDATION IS TO BE DESIGNED BY A QUALIFIED ENGINEER TO SUPPORT THE BUILDING REACTIONS IN ADDITION TO OTHER LOADS IMPOSED BY THE BUILDING USE OR OCCUPANCY WITH RESPECT TO JOB SITE CONDITIONS.
- (5) ALL ANCHOR BOLTS TO BE ASTM SPECIFICATION A307 UNLESS OTHERWISE NOTED.
- (6) VALUES GIVEN FOR BENDS AND ANCHOR BOLT TOTAL LENGTHS ARE SUGGESTED LENGTHS ONLY. IT IS THE RESPONSIBILITY OF THE FOUNDATION ENGINEER TO DETERMINE THESE VALUES SINCE THEY ARE A FUNCTION OF CONCRETE STRENGTH AS WELL AS OTHER FACTORS.

BRACING REACTIONS, PANEL SHEAR

Panel	Shear	Reactions (k)	Seismic	Wind	Col	Loc Line	Line	Horz	Vert	Horz	Vert	Wind	Seis
1	1.10	2.75	2.04	4.12	4.68	10.18	13.18	-26.73	-21.09	-11.40	-17.77	0.80	-12.06
2	1.10	2.75	2.04	4.12	4.68	10.18	13.18	-26.73	-21.09	-11.40	-17.77	0.80	-12.06
3	1.10	2.75	2.04	4.12	4.68	10.18	13.18	-26.73	-21.09	-11.40	-17.77	0.80	-12.06
4	1.10	2.75	2.04	4.12	4.68	10.18	13.18	-26.73	-21.09	-11.40	-17.77	0.80	-12.06
5	1.10	2.75	2.04	4.12	4.68	10.18	13.18	-26.73	-21.09	-11.40	-17.77	0.80	-12.06
6	1.10	2.75	2.04	4.12	4.68	10.18	13.18	-26.73	-21.09	-11.40	-17.77	0.80	-12.06

WIND BENT REACTIONS

Reactions	Seismic	Wind	Col	Loc Line	Line	Horz	Vert	Horz	Vert	Wind	Seis
1	1.35	5.60	1.06	1.35	5.60	1.06	1.35	5.60	1.06	1.35	5.60
2	1.35	5.60	1.06	1.35	5.60	1.06	1.35	5.60	1.06	1.35	5.60
3	1.35	5.60	1.06	1.35	5.60	1.06	1.35	5.60	1.06	1.35	5.60
4	1.35	5.60	1.06	1.35	5.60	1.06	1.35	5.60	1.06	1.35	5.60
5	1.35	5.60	1.06	1.35	5.60	1.06	1.35	5.60	1.06	1.35	5.60
6	1.35	5.60	1.06	1.35	5.60	1.06	1.35	5.60	1.06	1.35	5.60

FRAME LINES: 2 3 4 5

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frame Line	Anchor Bolt	Base Plate (in)	Grout (in)
2	4	0.750	8.000
3	4	0.750	8.000
4	4	0.750	8.000
5	4	0.750	8.000

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frame Line	Anchor Bolt	Base Plate (in)	Grout (in)
2	4	0.750	8.000
3	4	0.750	8.000
4	4	0.750	8.000
5	4	0.750	8.000

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Dead	Collateral	Live	Wind	Seis
2	2.73	2.05	4.12	4.68	10.18
3	2.73	2.05	4.12	4.68	10.18
4	2.73	2.05	4.12	4.68	10.18
5	2.73	2.05	4.12	4.68	10.18

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frame Line	Dead	Collateral	Live	Wind	Seis
2	2.73	2.05	4.12	4.68	10.18
3	2.73	2.05	4.12	4.68	10.18
4	2.73	2.05	4.12	4.68	10.18
5	2.73	2.05	4.12	4.68	10.18

ENDWALL COLUMN: ANCHOR BOLTS & BASE PLATES

Frame Line	Anchor Bolt	Base Plate (in)	Grout (in)
2	4	0.750	8.000
3	4	0.750	8.000
4	4	0.750	8.000
5	4	0.750	8.000

SI: 1 inch=25.4 mm 1 foot=305 mm

Notes:

- 1.) This table is based on a maximum wind speed of 140 m.p.h. (63 m/s) and mean roof height of 45 feet (13.716 m) or less.
- 2.) Fasteners shall be installed at opposing ends of the wood structural panel.
- 3.) Where screws are attached to masonry or masonry/stucco, they shall be attached using vibration-resistant anchors having a minimum withdrawal capacity of 490 lb. (2180 kN).

FASTENER TYPE	PANEL SPAN < 2ft	2ft PANEL SPAN < 4ft	4ft PANEL SPAN < 6ft	6ft PANEL SPAN < 8ft
#8 Wood screw-based anchor with 2" embedment length.	16	16	16	16
#10 Wood screw-based anchor with 2" embedment length.	16	12	16	9
1/4" Lag screw-based anchor with 2" embedment length.	16	16	16	16

WIND-BORNE DEBRIS PROTECTION FASTENING SCHEDULE FOR WOOD STRUCTURAL PANELS (PLYWOOD)

This building is located in a wind-borne debris region. Exterior glazing is assumed to be impact resistant and meet the provisions of the missile test, or they should be protected by impact resistant covering meeting the requirements of SSTD 12, ASTM E 1886 and ASTM 1996 or Miami-Dade PA 201, 202, & 203.

Openings may also be protected by structural wood panels having a min. thickness of 7/16" and maximum panel span of 8 feet. Attachment hardware and fastening schedule shall be in accordance with the following table.

Protection of Opening

REVISIONS

REV.	DESCRIPTION	DATE	DLR	CHKR	APPR
1					
2					
3					
4					
5					
6					
7					
8					
9					
10					

DRAWING STATUS

- [X] FOR CONSTRUCTION
[] FOR PERMIT ONLY
[] FOR APPROVAL
[] OTHER, EXPLAIN

PROJECT	ADDRESS
19066-1	LAKE CITY, FL 32024

DATE	SHEET
10/07/08	2

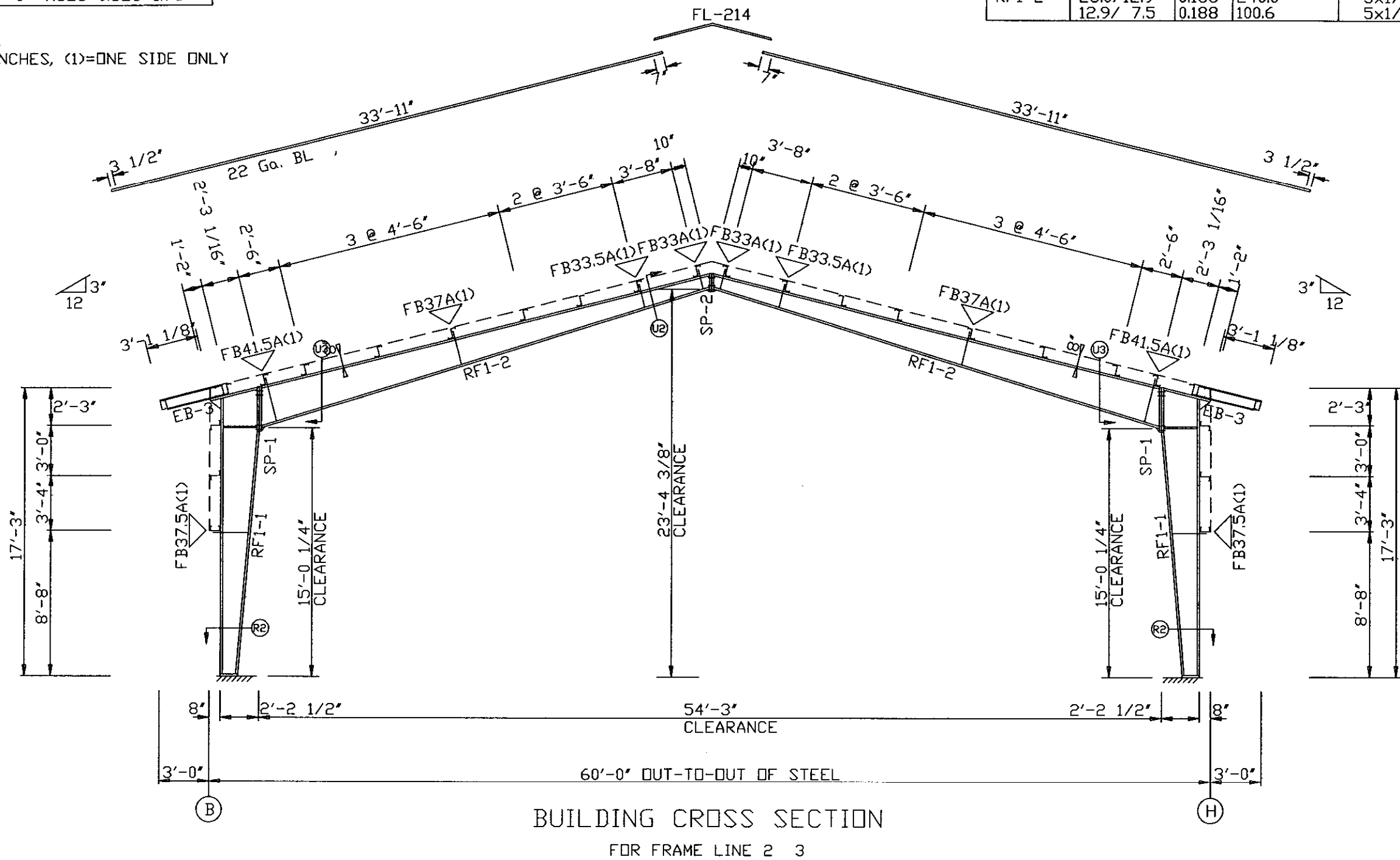
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SEAL

SPLICE BOLTS						
Splice Mark	Quan		-----Bolt-----			
	Top	Bot	Int	Type	Dia	Len
SP- 1	4	4	0	A325	0.750	2.50
SP- 2	2	4	0	A325	0.625	1.75

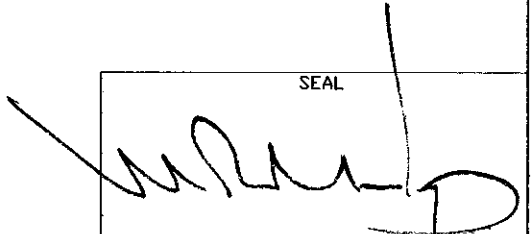
MEMBER SIZE TABLE (in)					
PIECE	WEB DEPTH		WEB PLATE		OUTSIDE FLANGE W x T x LEN
	START	END	THICK	LENGTH	
RF1-1	9.5	26.0	0.164	175.7	5x1/4" x199.9
	26.0	26.0	0.250	30.8	6x5/16" x 35.2
RF1-2	26.0	12.9	0.188	240.0	5x1/4" x240.0
	12.9	7.5	0.188	100.6	5x1/4" x 94.0

✓ FLANGE BRACES:
 FBxxxA(1): xx=INCHES, (1)=ONE SIDE ONLY
 A - L2X2X14G



GENERAL NOTES:
 * NOTICE TO ERECTOR *
 (A)It is IMPORTANT that for members exceeding 30 ft. in length that a spreader bar be used when lifting.
 (B)ALL flange braces and wind bracing must be installed prior to exterior finishes being applied.

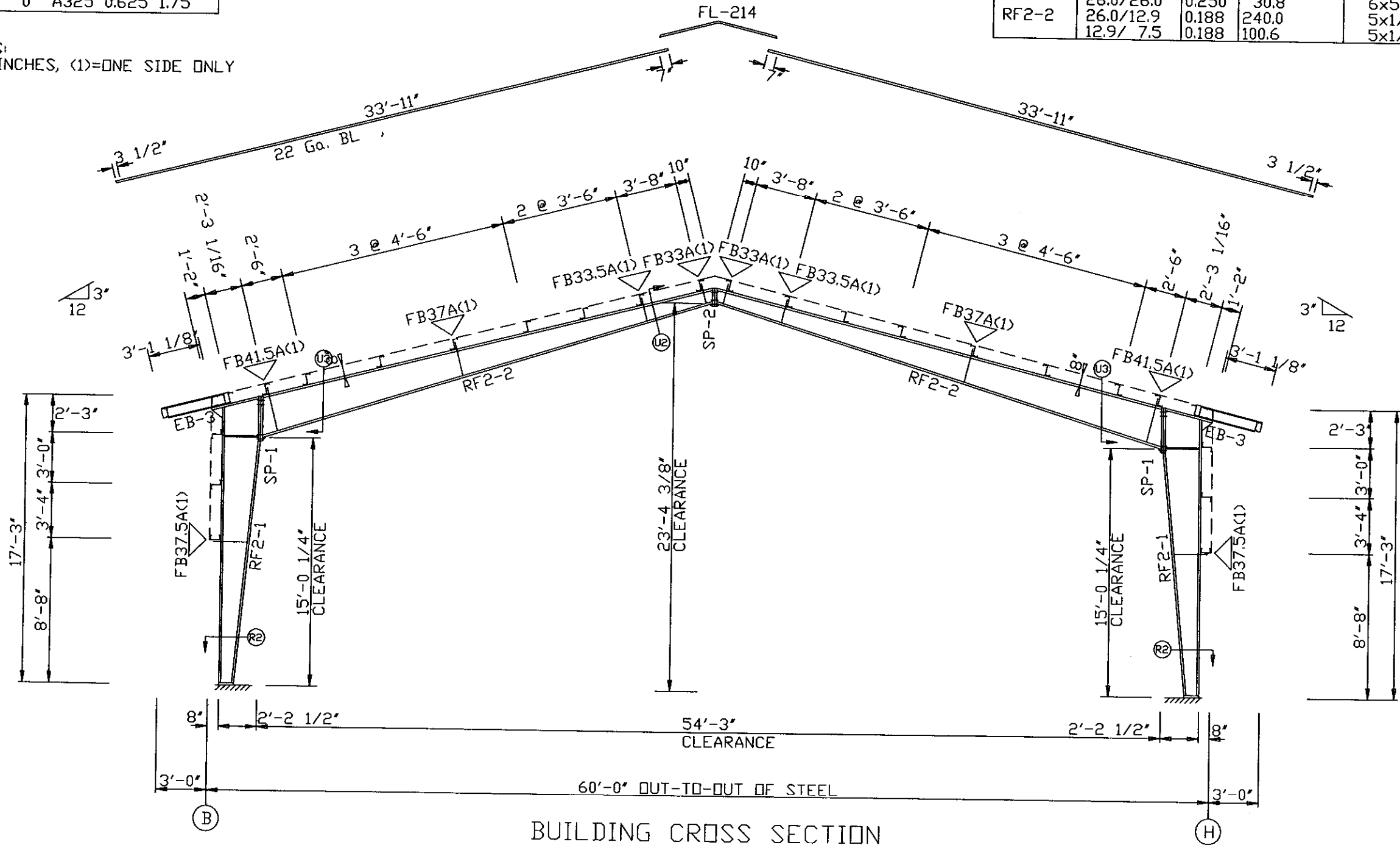
REVISIONS						DRAWING STATUS		LIFETIME STEEL BLDGS		LIFETIME STEEL BLDGS	
REV.	DESCRIPTION	DATE	DTLR	DATE	CHKR	APPD		PROJECT	6000' x 1213' x 17.2	BUILDING CROSS SECTION	
							☐ FOR CONSTRUCTION	ID	19066-1	DESIGN	DRAFT:ARM/MMD CHECK: DB
							☐ FOR PERMIT ONLY	PROJECT ADDRESS	LAKE CITY, FL 32024	DATE	10/07/08 SHEET 3
							☐ FOR APPROVAL				
							☐ OTHER, EXPLAIN				

SEAL

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SPlice BOLTS						
Splice Mark	Quan		Bolt			
	Top	Bot	Int	Type	Dia	Len
SP- 1	4	4	0	A325	0.750	2.50
SP- 2	2	4	0	A325	0.625	1.75

FLANGE BRACES:
FBxxA(1): xx=INCHES, (1)=ONE SIDE ONLY
A - L2X2X14G

MEMBER SIZE TABLE (in)					
PIECE	WEB DEPTH		WEB PLATE		OUTSIDE FLANGE W x T x LEN
	START	END	THICK	LENGTH	
RF2-1	7.5	26.0	0.164	175.7	5x1/4" x199.9
RF2-2	26.0	26.0	0.250	30.8	6x5/16" x 35.2
	26.0	12.9	0.188	240.0	5x1/4" x240.0
	12.9	7.5	0.188	100.6	5x1/4" x 94.0



BUILDING CROSS SECTION
FOR FRAME LINE 4 5

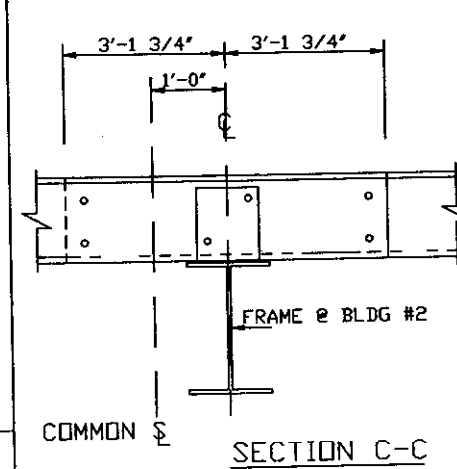
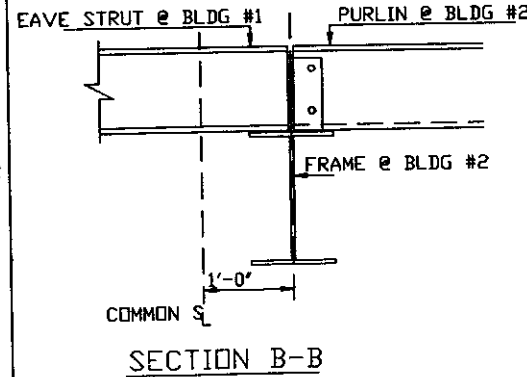
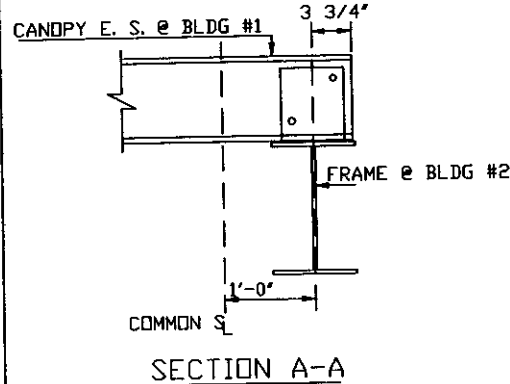
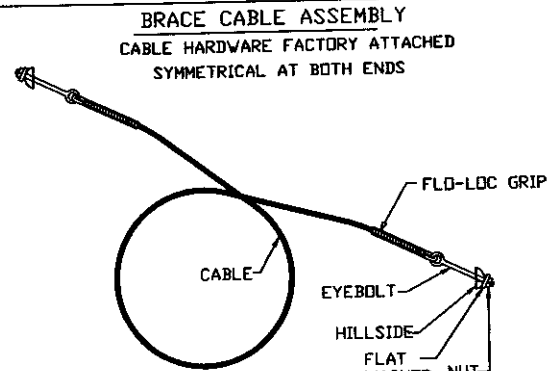
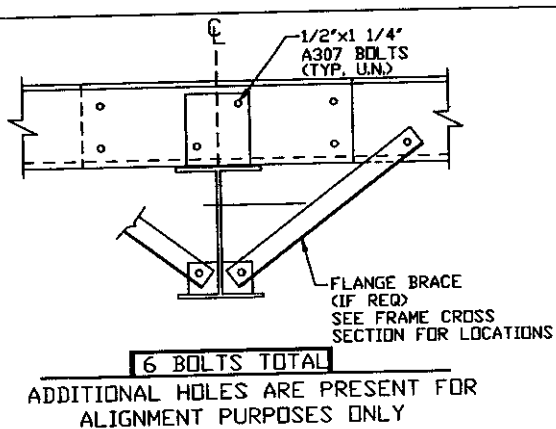
GENERAL NOTES:

- * NOTICE TO ERECTOR *
- (A)It is IMPORTANT that for members exceeding 30 ft. in length that a spreader bar be used when lifting.
- (B)ALL flange braces and wind bracing must be installed prior to exterior finishes being applied.

REVISIONS							DRAWING STATUS		LIFETIME STEEL BLDGS		LIFETIME STEEL BLDGS	
REV.	DESCRIPTION	DATE	DLR	DATE	CHKR	APPD	[] FOR CONSTRUCTION		PROJECT	60.00' x 121.3 x 17.2	BUILDING CROSS SECTION	
							[X] FOR PERMIT ONLY		ID	19066-1	DESIGN: DRAFT/ARM/MMD CHECK: DB	
							[] FOR APPROVAL		PROJECT ADDRESS	LAKE CITY, FL 32024	DATE: 10/07/08 SHEET 4	
							[] OTHER, EXPLAIN					

SEAL
[Signature]

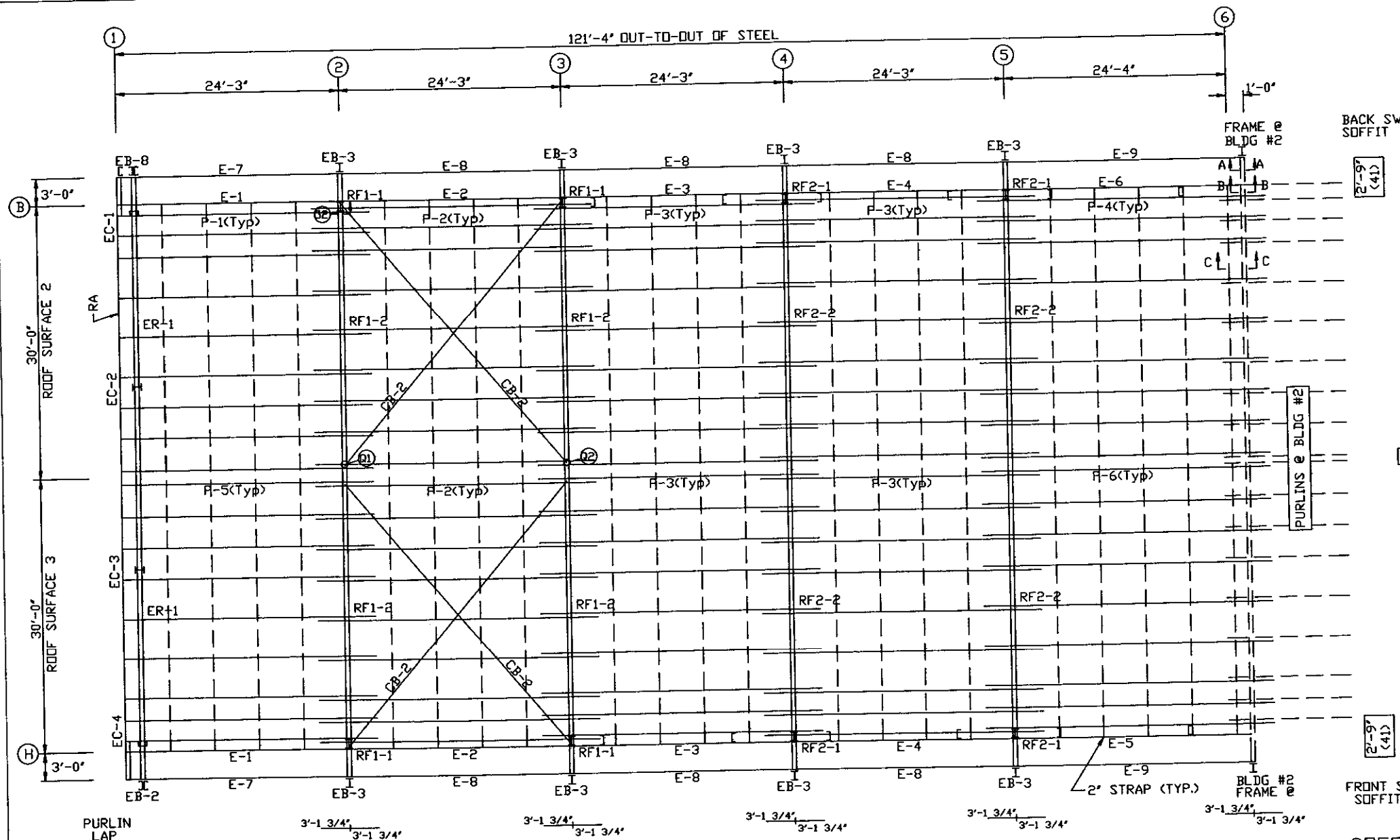
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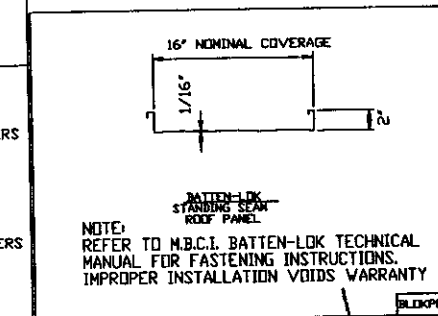
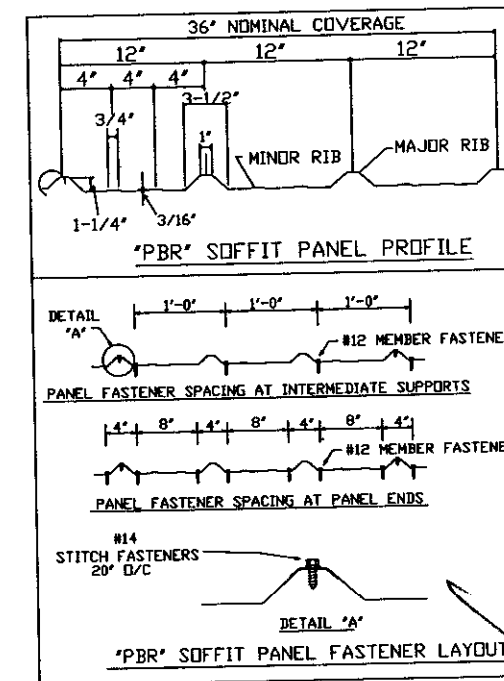
EXTENSION/CANOPY BOLTS				
ROOF PLAN				
MARK	QUAN	TYPE	DIA	LENGTH
EB-2	4	A325	5/8"	1 3/4"
EB-3	8	A325	5/8"	1 3/4"
EB-4	4	A325	5/8"	1 3/4"
EB-7	4	A325	5/8"	1 3/4"
EB-8	4	A325	5/8"	1 3/4"

TRIM TABLE		
ROOF		
MARK	LENGTH	DETAIL
1	FL-214	10'-2"

MEMBER TABLE		
ROOF PLAN		
MARK	PART	LENGTH
EB-1	8X4CH16	3'-5 5/8"
EB-2	8X50D16	3'-9 5/8"
EB-3	W8X10	3'-9 5/8"
EB-8	8X50D16	3'-9 5/8"
EB-9	8X4CH16	3'-5 5/8"
P-1	8X25Z14	27'-4 1/2"
P-2	8X25Z15	29'-6 1/2"
P-3	8X25Z15	28'-6 1/2"
P-4	8X25Z15	30'-7 1/2"
P-5	8X25Z14	27'-4 1/2"
P-6	8X25Z15	30'-7 1/2"
E-1	825E350D16	22'-7 1/2"
E-2	825E350D16	23'-7 1/2"
E-3	825E350D16	23'-7 1/2"
E-4	825E350D16	23'-7 1/2"
E-5	825E350D16	22'-8 1/2"
E-6	825E350D16	22'-8 1/2"
E-7	8X35C12	23'-11"
E-8	8X25C12	23'-7 1/2"
E-9	8X35C12	25'-0"
CB-2	0.312CBL	38'-5"



ROOF FRAMING PLAN



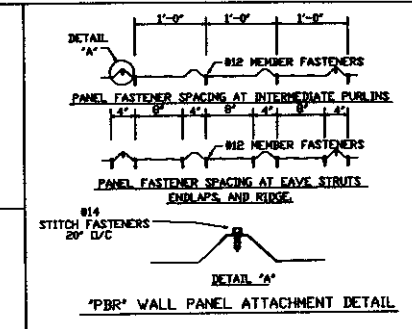
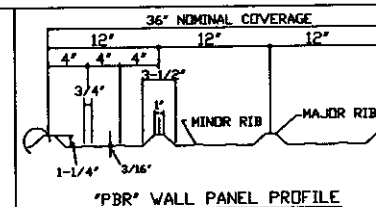
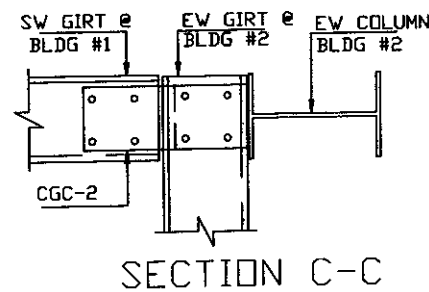
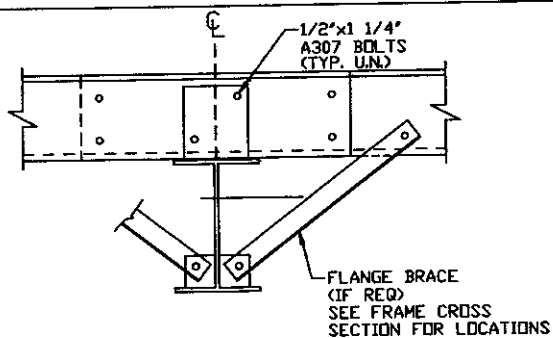
OCT 09 2008

REVISIONS					
REV.	DESCRIPTION	DATE	DTLR	DATE	CHKR

DRAWING STATUS
[] FOR CONSTRUCTION
[X] FOR PERMIT ONLY
[] FOR APPROVAL
[] OTHER, EXPLAIN

LIFETIME STEEL BLDGS	
PROJECT	60.00' x 121.3' x 17.2'
ID	19066-1
PROJECT ADDRESS	LAKE CITY, FL 32024

LIFETIME STEEL BLDGS	
ROOF FRAMING	
DESIGN	DRAFT/ARM/MMD CHECK: DB
DATE	10/07/08
SHEET	5

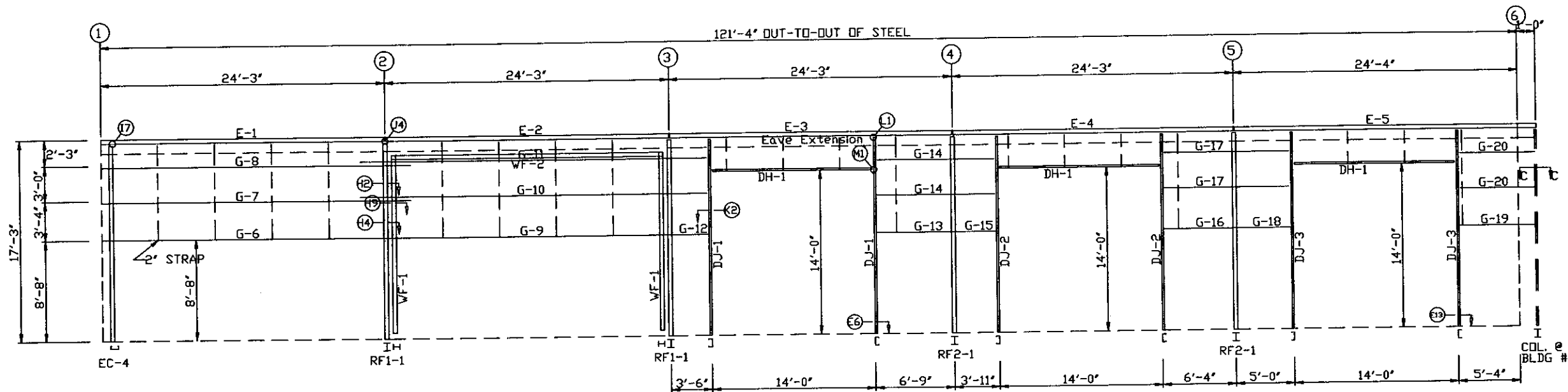


BOLT TABLE				
FRAME LINE H				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-1 - WF-2	8	A325	3/4"	2'
WF-1 - RF1-1	4	A325	5/8"	1 1/2'

TRIM TABLE				
FRAME LINE H				
QTY	MARK	LENGTH	DETAIL	
1	TBD	3'-6"	TM29 TM30	
2	TBD	11'		
3	TBD	1'-7"		
4	TBD	2'-1"		
5	CTR	8'-9"		
6	JTR	8'-9"	TM31 TM33	
7	FL-245	6'		
8	FL-248H	20'-2"		
9	JTR	5'-6"		
10	HT	14'-3"		
11	HT	20'-3"		
12	HT	1'-7"		

SPECIAL BOLTS				
QTY	ID	QUAN	TYPE	DIA
1		4	A307	1/2"
				1 1/4"
				0

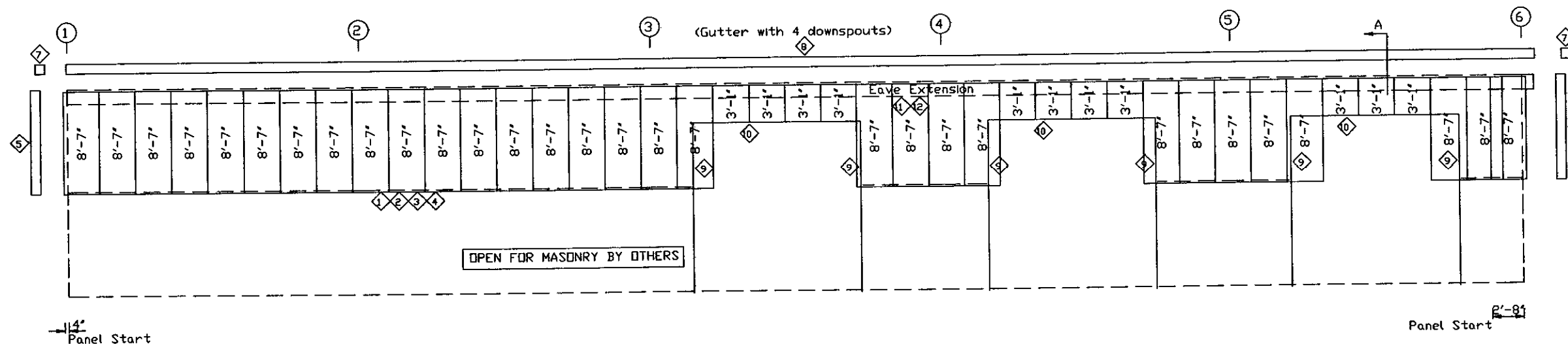
MEMBER TABLE		
FRAME LINE H		
MARK	PART	LENGTH
WF-1	W18641	15'-7"
WF-2	W16841	21'-2 1/2"
DJ-1	8x35C14	16'-6 3/4"
DJ-2	8x35C16	16'-6 3/4"
DJ-3	8x35C16	16'-6 3/4"
DH-1	8x35C16	14'-0"
E-1	825E350D16	22'-7 1/2"
E-2	825E350D16	23'-7 1/2"
E-3	825E350D16	23'-7 1/2"
E-4	825E350D16	23'-7 1/2"
E-5	825E350D16	22'-8 1/2"
E-10	825E350D16	1'-4 1/2"
G-6	8X40C14	24'-2 1/2"
G-7	8X25Z15	26'-4 1/2"
G-8	8X25Z16	26'-4 1/2"
G-9	8X40C14	24'-2 1/2"
G-10	8X25Z15	29'-7"
G-11	8X25Z16	29'-7"
G-12	8X25C16	3'-2"
G-13	8X25C16	6'-5"
G-14	8X25Z16	10'-0 1/2"
G-15	8X25C16	3'-7"
G-16	8X25C16	6'-0"
G-17	8X25Z16	10'-8 1/2"
G-18	8X25C16	4'-8"
G-19	8X25C16	5'-0"
G-20	8X25Z16	5'-0"



GIRT LAPS

2'-1 3/4" 2'-1 3/4"

SIDEWALL FRAMING: FRAME LINE H



SIDEWALL SHEETING & TRIM: FRAME LINE H

PANELS: 26 Ga. PBR - NEED KYMAR COLOR

GENERAL NOTES:

(1.) IF CABLE BRACING, WIND BENTS, WIND COLUMNS, OR WEAK AXIS DESIGN OF SIDE WALL COLUMNS WERE NOT PROVIDED IT HAS BEEN DETERMINED THAT DIAPHRAGM PANEL ACTION IS SUFFICIENT TO RESIST LONGITUDINAL FORCES. TEMPORARY BRACING SHOULD BE PROVIDED BY ERECTOR UNTIL ALL WALL AND ROOF PANELS ARE INSTALLED.

REVISIONS

REV.	DESCRIPTION	DATE	DTLR	DATE	CHKR	APPD

DRAWING STATUS

[] FOR CONSTRUCTION

[X] FOR PERMIT ONLY

[] FOR APPROVAL

[] OTHER, EXPLAIN

LIFETIME STEEL BLDGS

PROJECT 60.00' x 121.3' x 17.2

ID 19066-1

PROJECT ADDRESS LAKE CITY, FL 32024

LIFETIME STEEL BLDGS

SIDEWALL FRAMING

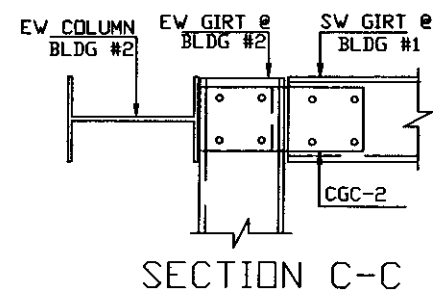
DESIGN: DRAFT: ARM/MMD CHECK: DB

DATE: 10/07/08 SHEET 6

SEAL

[Signature]

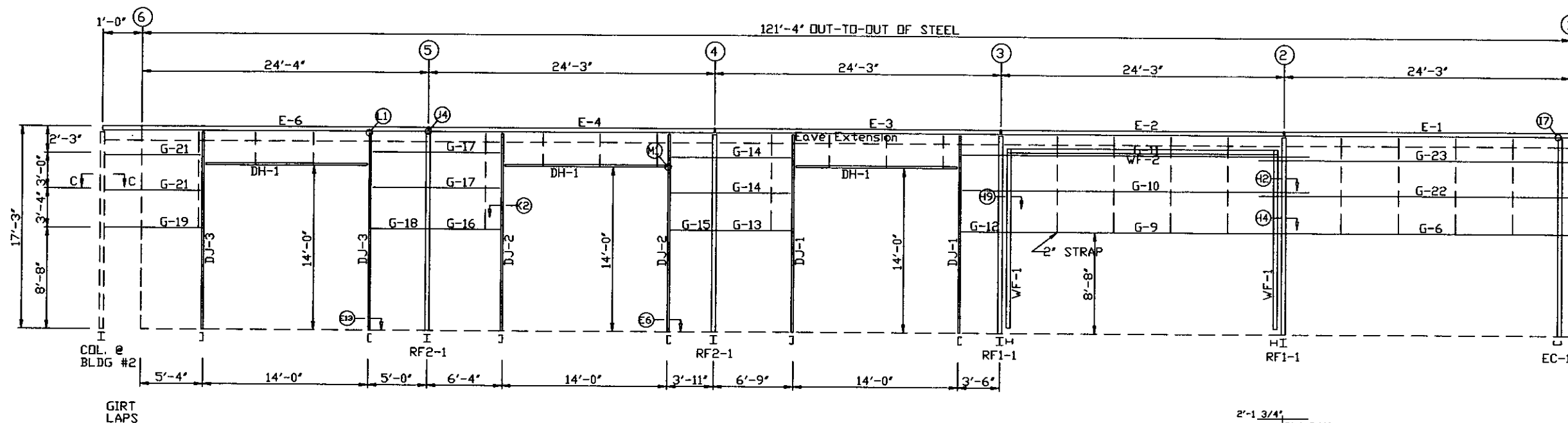
OCT 09 2008



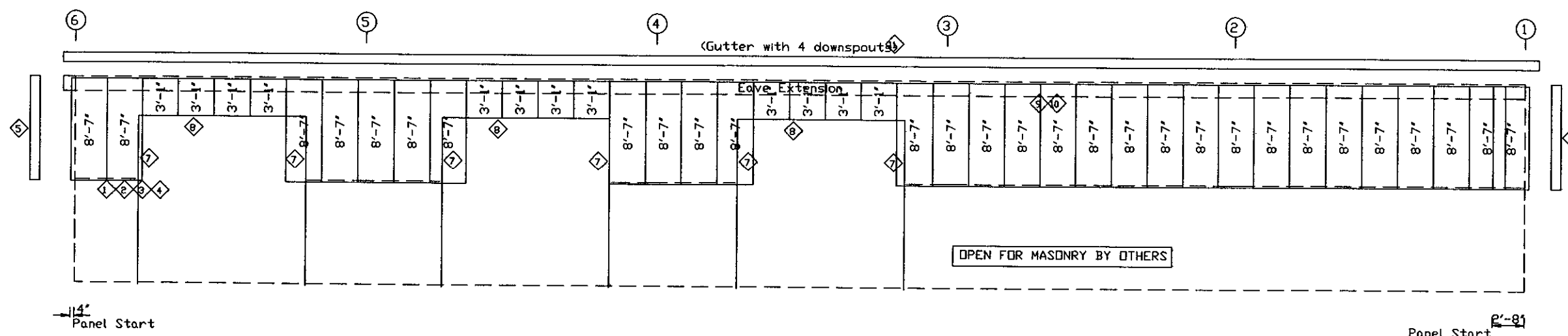
BOLT TABLE				
FRAME LINE B				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-1 - WF-2	8	A325	3/4"	2'
WF-1 - RF1-1	4	A325	5/8"	1 1/2'

TRIM TABLE			
FRAME LINE B			
OID	MARK	LENGTH	DETAIL
1	TBD	3'-6"	
2	TBD	11'	
3	TBD	1'-7"	
4	TBD	2'-1"	
5	JTR	8'-9"	TM30
6	CTR	8'-9"	TM29
7	JTR	5'-6"	TM31
8	HT	14'-3"	TM33
9	HT	20'-3"	
10	HT	1'-7"	
11	FL-248H	20'-2"	

MEMBER TABLE		
FRAME LINE B		
MARK	PART	LENGTH
WF-1	W18641	15'-7"
WF-2	W16841	21'-2 1/2"
DJ-1	8x35C14	16'-6 3/4"
DJ-2	8x35C16	16'-6 3/4"
DJ-3	C8X11.5	16'-6 3/4"
DH-1	8x35C16	14'-0"
E-1	825E350D16	22'-7 1/2"
E-2	825E350D16	23'-7 1/2"
E-3	825E350D16	23'-7 1/2"
E-4	825E350D16	23'-7 1/2"
E-6	825E350D16	22'-8 1/2"
E-10	825E350D16	1'-4 1/2"
G-6	8X40C14	24'-2 1/2"
G-9	8X40C14	24'-2 1/2"
G-10	8X25Z15	29'-7"
G-11	8X25Z16	29'-7"
G-12	8X25C16	3'-2"
G-13	8X25C16	6'-5"
G-14	8X25Z16	10'-0 1/2"
G-15	8X25C16	3'-7"
G-16	8X25C16	6'-0"
G-17	8X25Z16	10'-8 1/2"
G-18	8X25C16	4'-8"
G-19	8X25C16	5'-0"
G-21	8X25Z16	5'-0"
G-22	8X25Z15	26'-4 1/2"
G-23	8X25Z16	26'-4 1/2"



SIDEWALL FRAMING: FRAME LINE B



SIDEWALL SHEETING & TRIM: FRAME LINE B
PANELS: 26 Ga. PBR - NEED KYNAR COLOR

SEAL

OCT 09 2008

OCT 09 2008

GENERAL NOTES:

(1.) IF CABLE BRACING, WIND BENTS, WIND COLUMNS, OR WEAK AXIS DESIGN OF SIDE WALL COLUMNS WERE NOT PROVIDED IT HAS BEEN DETERMINED THAT DIAPHRAGM PANEL ACTION IS SUFFICIENT TO RESIST LONGITUDINAL FORCES. TEMPORARY BRACING SHOULD BE PROVIDED BY ERECTOR UNTIL ALL WALL AND ROOF PANELS ARE INSTALLED.

REVISIONS						
REV.	DESCRIPTION	DATE	DTLR	DATE	CHKR	APPR

DRAWING STATUS
☐ FOR CONSTRUCTION
☒ FOR PERMIT ONLY
☐ FOR APPROVAL
☐ OTHER, EXPLAIN

LIFETIME STEEL BLDGS	
PROJECT	60.00' x 121.3 x 17.2
ID	19066-1
PROJECT ADDRESS	LAKE CITY, FL 32024

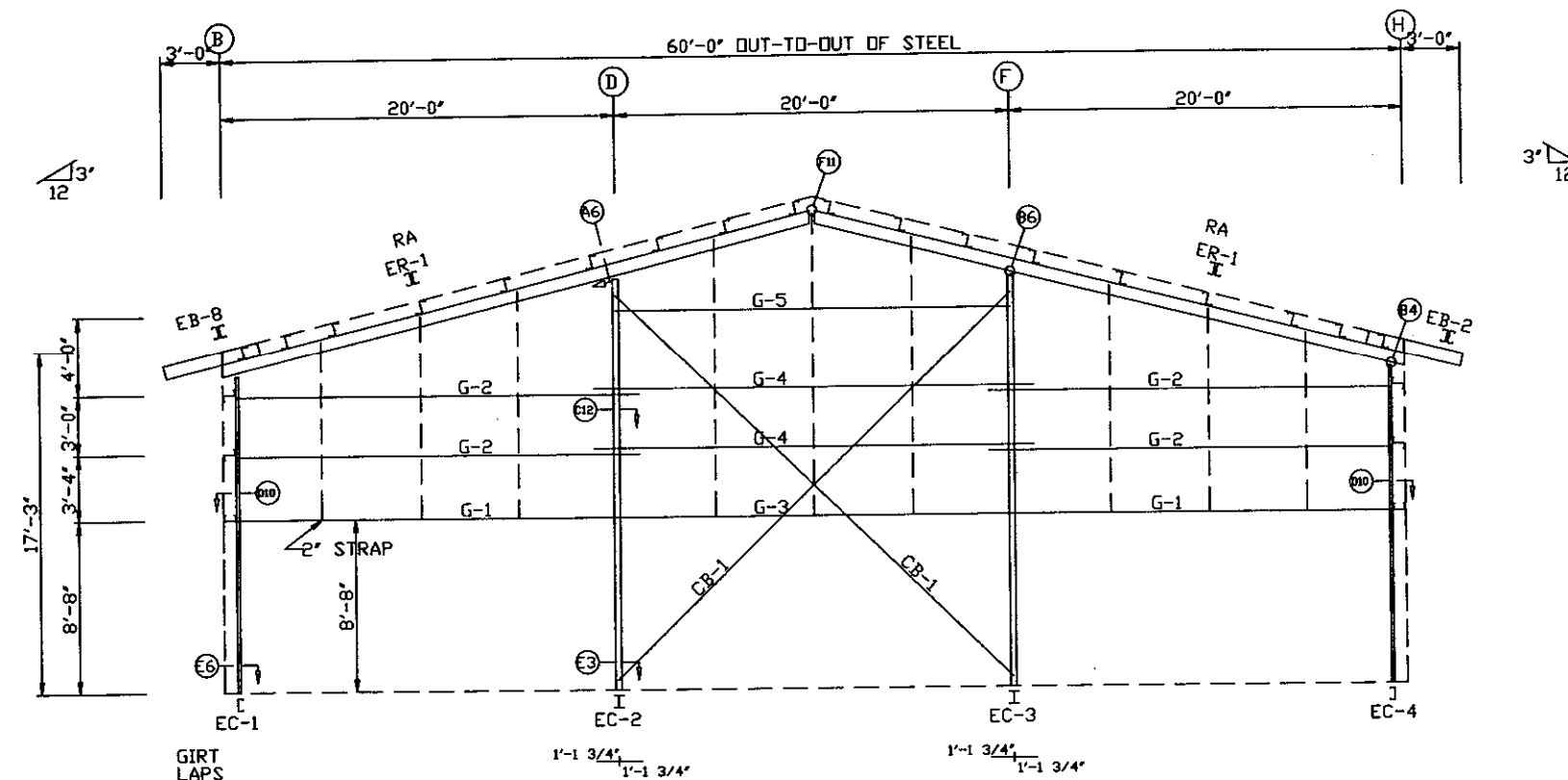
LIFETIME STEEL BLDGS	
SIDEWALL FRAMING	
DESIGN: DRAFT:ARM/MMD	CHECK: DB
DATE: 10/07/08	SHEET 7

BOLT TABLE				
FRAME LINE 1				
LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-1	8	A325	5/8"	1 3/4"
Columns	4	A325	1/2"	1 1/4"

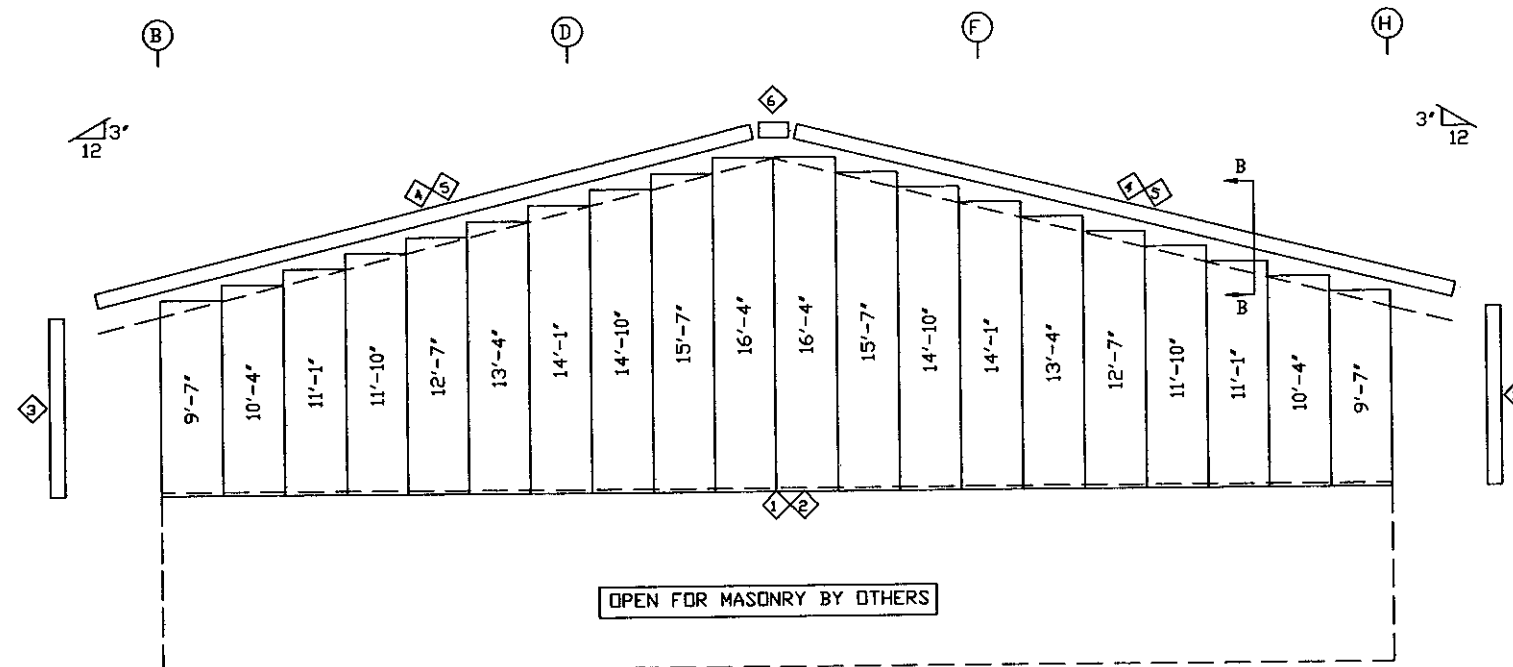
TRIM TABLE			
FRAME LINE 1			
OID	MARK	LENGTH	DETAIL
1	TBD	3'-6"	TRIM_29
2	TBD	1'-6"	
3	CTR	8'-9"	
4	FL-111	20'-3"	
5	XFL-111	14'-4"	
6	FL-125	2'-1"	

MEMBER TABLE		
FRAME LINE 1		
MARK	PART	LENGTH
EB-2	8x50D16	3'-9 5/8"
EB-8	8x50D16	3'-9 5/8"
EC-1	8x35C16	15'-9 7/16"
EC-2	W8851	20'-7 7/16"
EC-3	W8851	20'-7 7/16"
EC-4	8x35C16	15'-9 7/16"
ER-1	10x70d12	30'-11 1/16"
G-1	8x25C16	19'-3 1/2"
G-2	8x25Z16	20'-5 1/2"
G-3	8x25C16	19'-11 1/2"
G-4	8x25Z16	22'-3 1/2"
G-5	8x25Z13	20'-7 1/2"
CB-1	0.312CBL	29'-0"

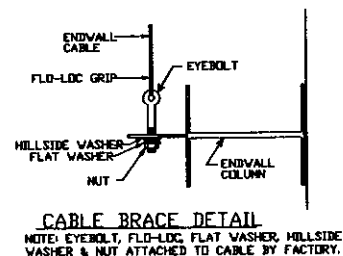
FLANGE BRACE TABLE		
FRAME LINE 1		
VID	MARK	LENGTH
1	FB34.5	2'-10 1/2"



ENDWALL FRAMING: FRAME LINE 1



ENDWALL SHEETING & TRIM: FRAME LINE 1
PANELS: 26 Ga. PBR - NEED KYNAR COLOR



GENERAL NOTES:

(1.) IF CABLE BRACING FOR END WALL IS NOT SHOWN ON ERECTION DRAWINGS IT HAS BEEN DETERMINED THAT DIAPHRAGM PANEL ACTION IS SUFFICIENT TO RESIST LONGITUDINAL FORCES. TEMPORARY BRACING SHOULD BE PROVIDED BY ERECTOR UNTIL ALL WALL AND ROOF PANELS ARE INSTALLED.

REVISIONS						
REV.	DESCRIPTION	DATE	DTLR	DATE	CHKR	APPR

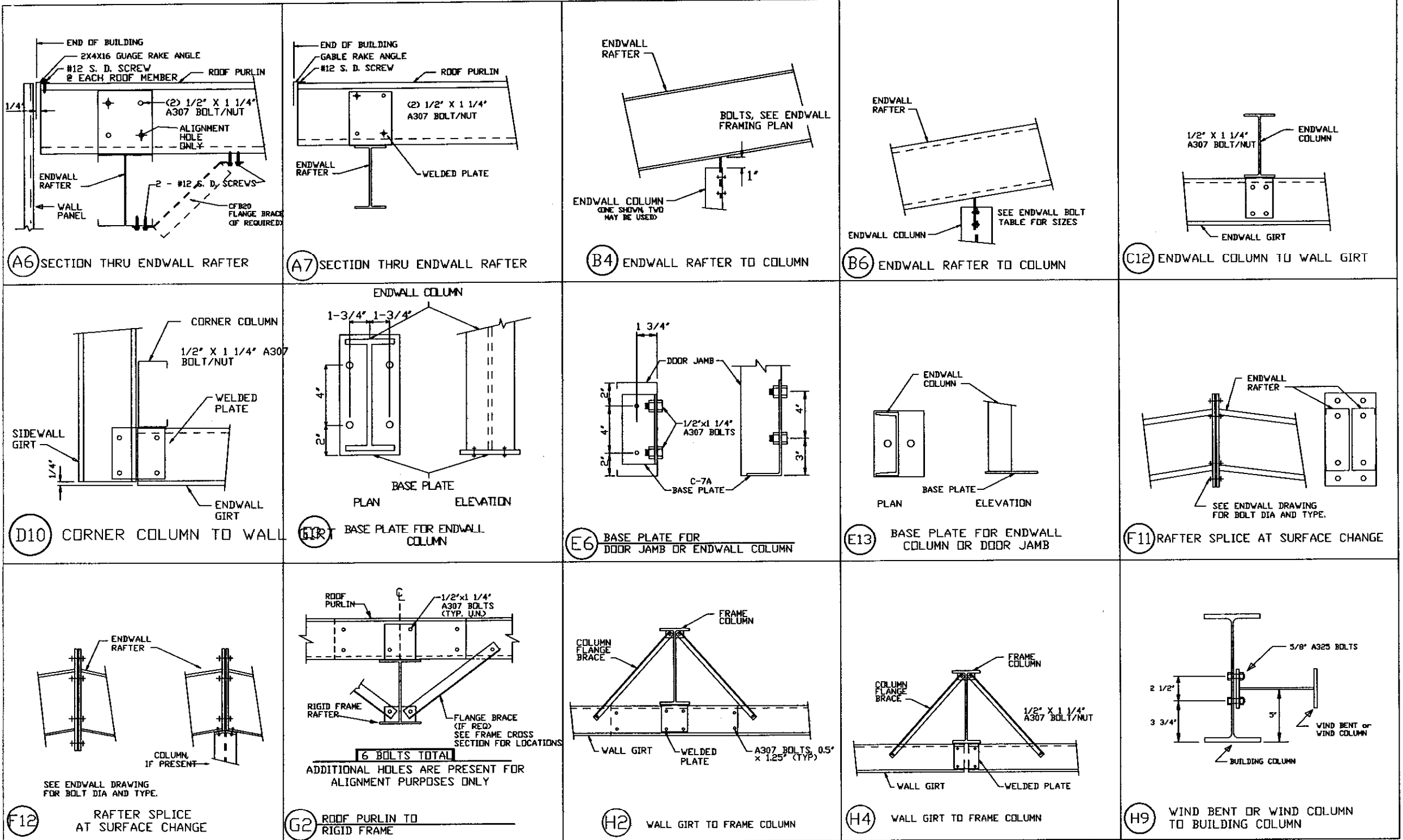
DRAWING STATUS
☐ FOR CONSTRUCTION
☐ FOR PERMIT ONLY
☐ FOR APPROVAL
☐ OTHER, EXPLAIN

LIFETIME STEEL BLDGS	
PROJECT	6000' x 1213 x 172
ID	19066-1
PROJECT ADDRESS	LAKE CITY, FL 32024

LIFETIME STEEL BLDGS	
ENDWALL FRAMING	DESIGN: DRAFT/ARM/MMD CHECK: DB
DATE: 10/07/08	SHEET 8

SEAL

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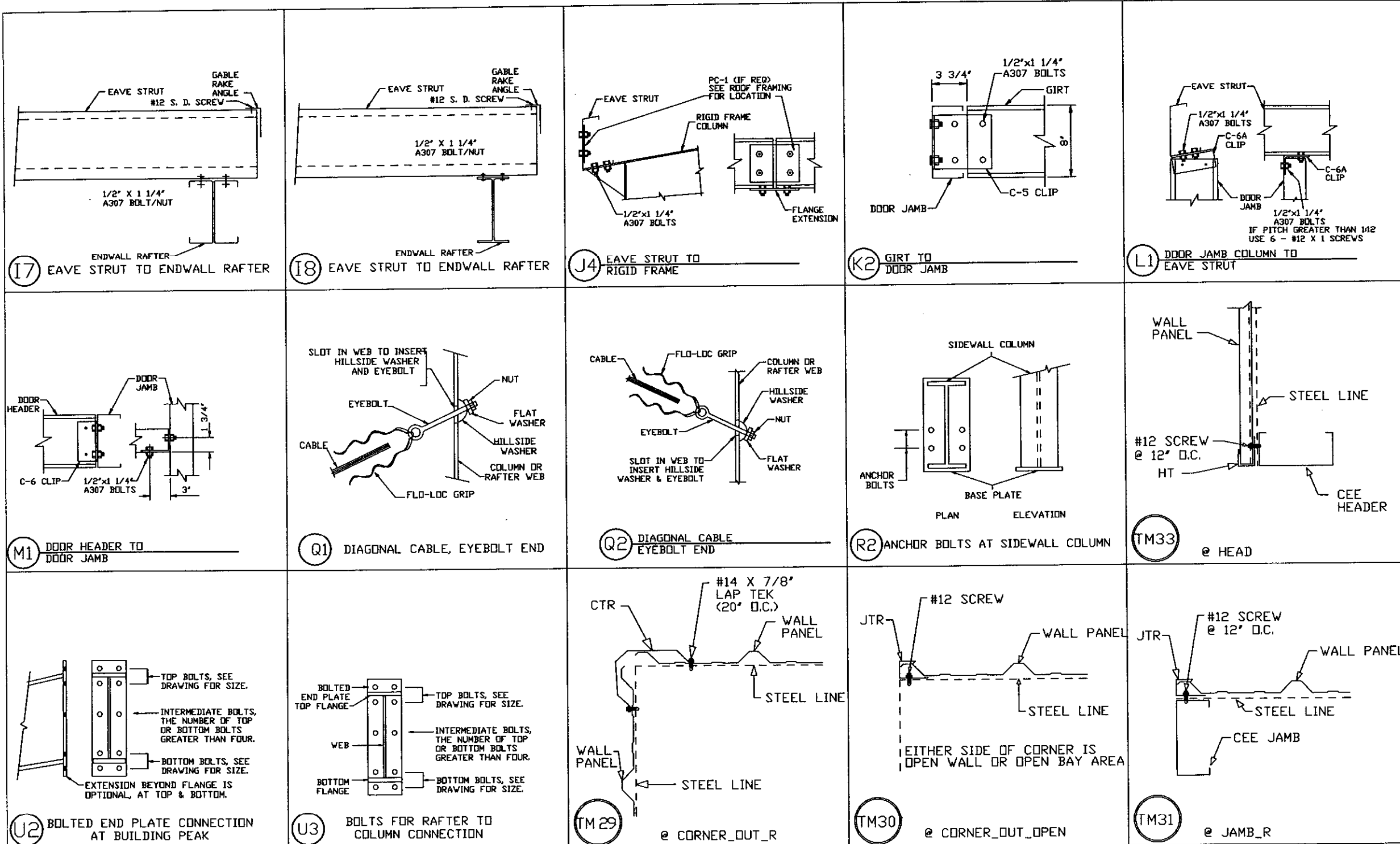


SEAL

[Signature]

OCT 09 2008

REVISIONS						DRAWING STATUS		LIFETIME STEEL BLDGS		LIFETIME STEEL BLDGS	
REV.	DESCRIPTION	DATE	DTLR	DATE	CHKR	APPD		PROJECT	60.00' x 121.3' x 17.2'	Detail Page	
							[] FOR CONSTRUCTION	ID	19066-1	DESIGN	DRAFT:ARM/MMD CHECK: DB
							[] FOR PERMIT ONLY	PROJECT ADDRESS	LAKE CITY, FL 32024	DATE: 10/07/08	SHEET 9
							[] FOR APPROVAL				
							[] OTHER, EXPLAIN				



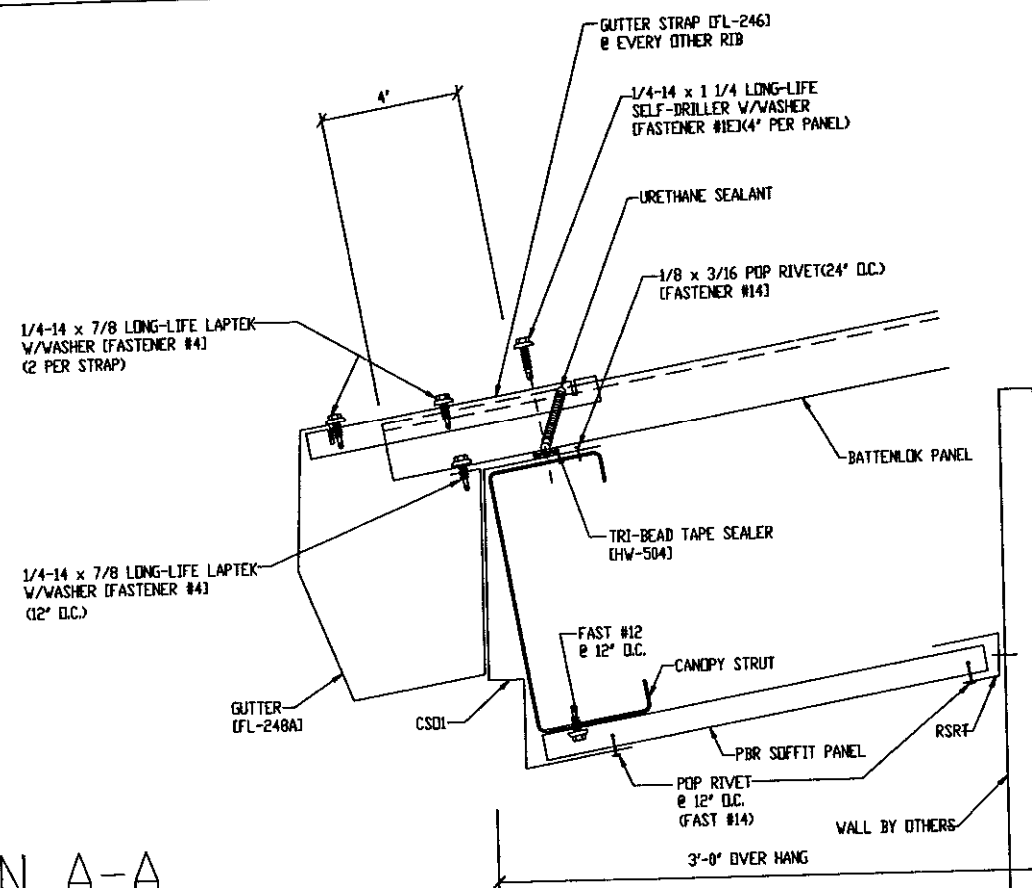
SEAL

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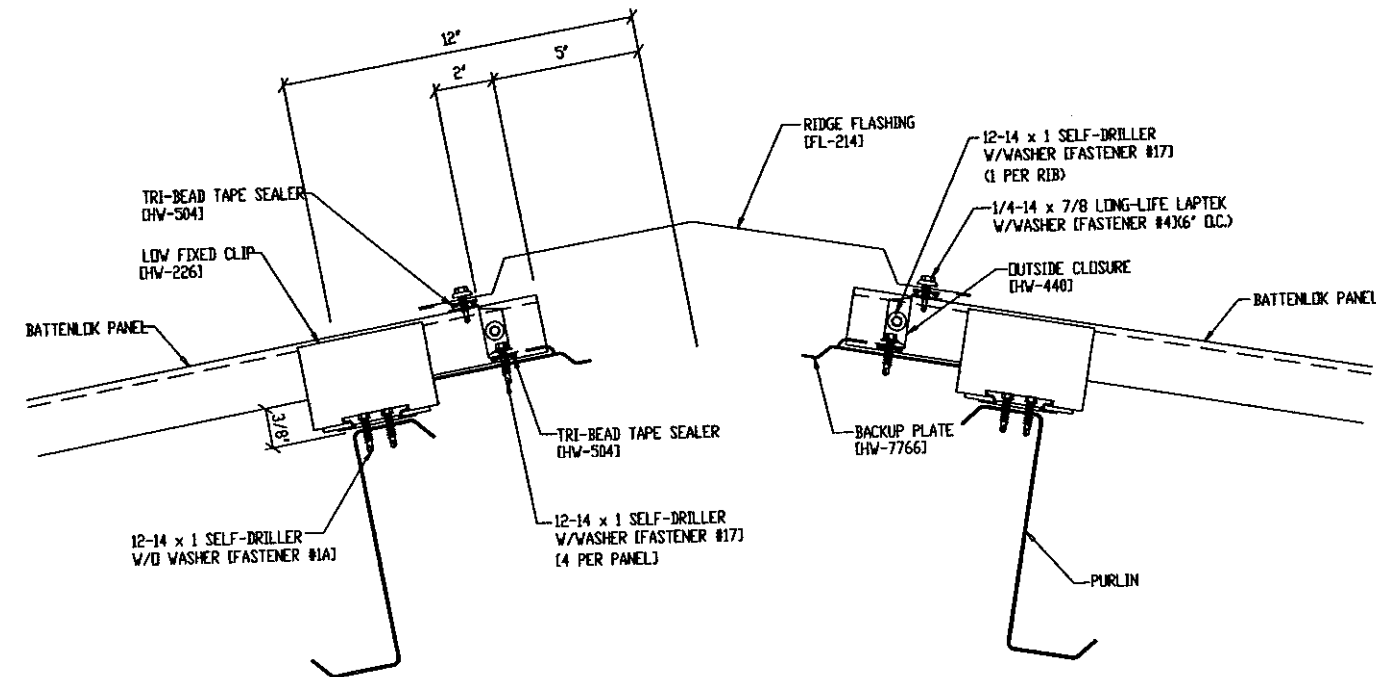
OCT 09 2008

REVISIONS							DRAWING STATUS [] FOR CONSTRUCTION [X] FOR PERMIT ONLY [] FOR APPROVAL [] OTHER, EXPLAIN -----	LIFETIME STEEL BLDGS		LIFETIME STEEL BLDGS		
REV.	DESCRIPTION	DATE	DTLR	DATE	CHKR	APPD		PROJECT	6000' x 1213 x 172	Detail Page		
								ID	19066-1	DESIGN	DRAFT/ARM/MMD	CHECK/DB
								PROJECT ADDRESS	LAKE CITY, FL 32024	DATE	10/07/08	SHEET 10

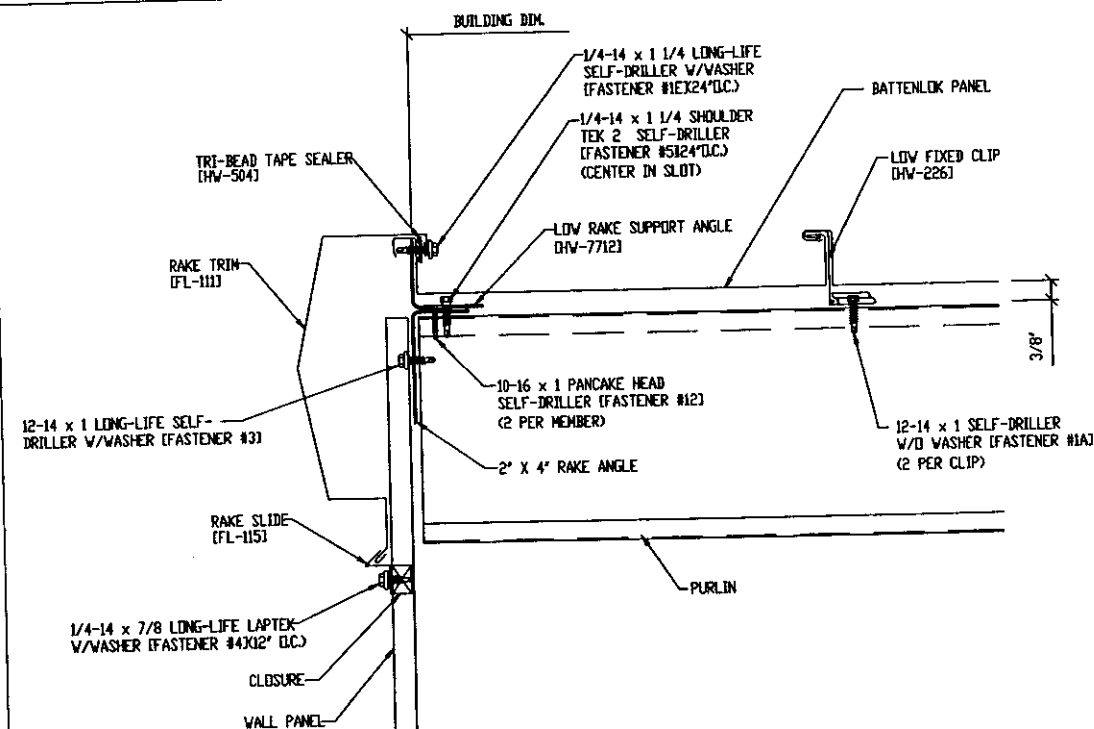
SECTION A-A



SECTION C-C



SECTION B-B



REVISIONS

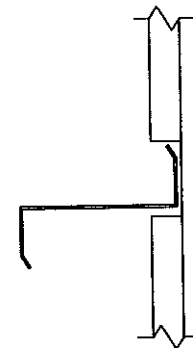
REV.	DESCRIPTION	DATE	DLR	DATE	CHKR	APPD

DRAWING STATUS
☐ FOR CONSTRUCTION
☒ FOR PERMIT ONLY
☐ FOR APPROVAL
☐ OTHER, EXPLAIN

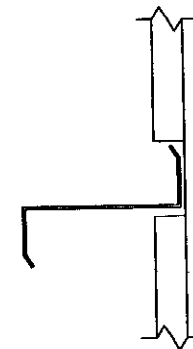
LIFETIME STEEL BLDGS
 PROJECT 60.00' x 121.3 x 17.2
 ID 19066-1
 PROJECT ADDRESS LAKE CITY, FL 32024

LIFETIME STEEL BLDGS
 Detail Page
 DESIGN: DRAFT:ARM/MMD CHECK: DB
 DATE: 10/07/08 SHEET 11

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6-D-3



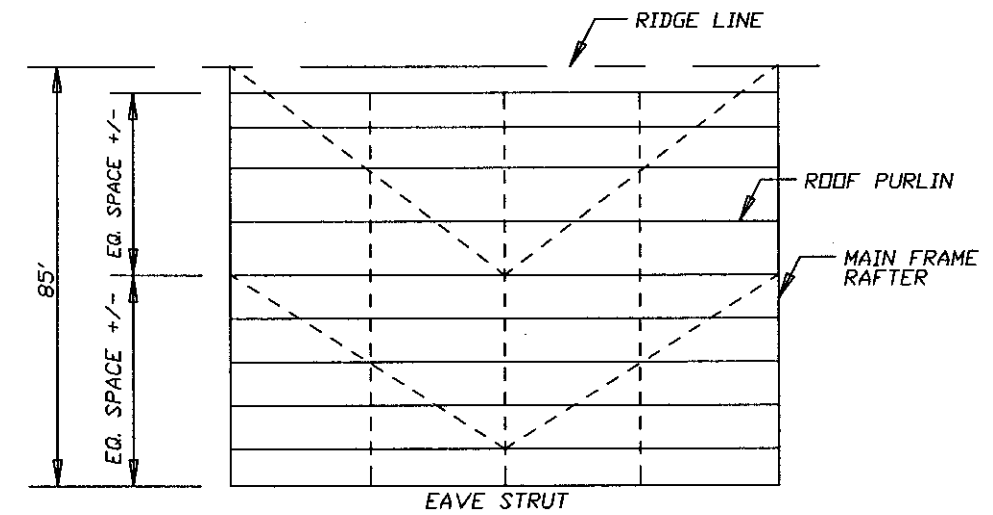
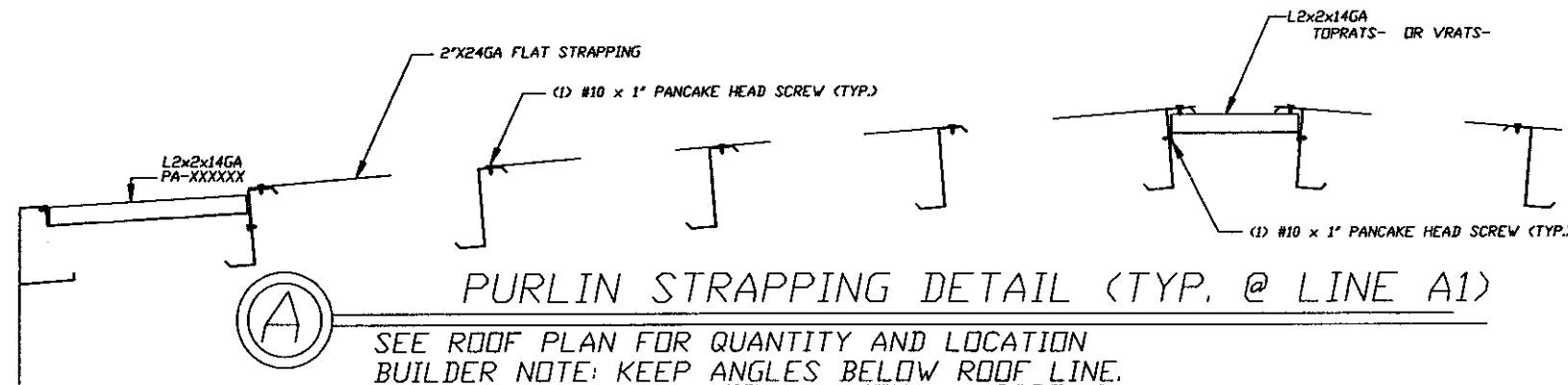
6-D-6

REVISIONS							DRAWING STATUS		LIFETIME STEEL BLDGS		LIFETIME STEEL BLDGS	
REV.	DESCRIPTION	DATE	DTLR	DATE	CHKR	APPD	[] FOR CONSTRUCTION	PROJECT	60.00' x 121.3 x 17.2	Detail Page		
							[X] FOR PERMIT ONLY	ID	19066-1	DESIGN:	DRAFT/ARM/MMD	CHECK: DB
							[] FOR APPROVAL	PROJECT	LAKE CITY, FL 32024	DATE: 10/07/08	SHEET	12
							[] OTHER, EXPLAIN	ADDRESS				

STANDING SEAM ROOFS (TOP FLANGE ONLY)

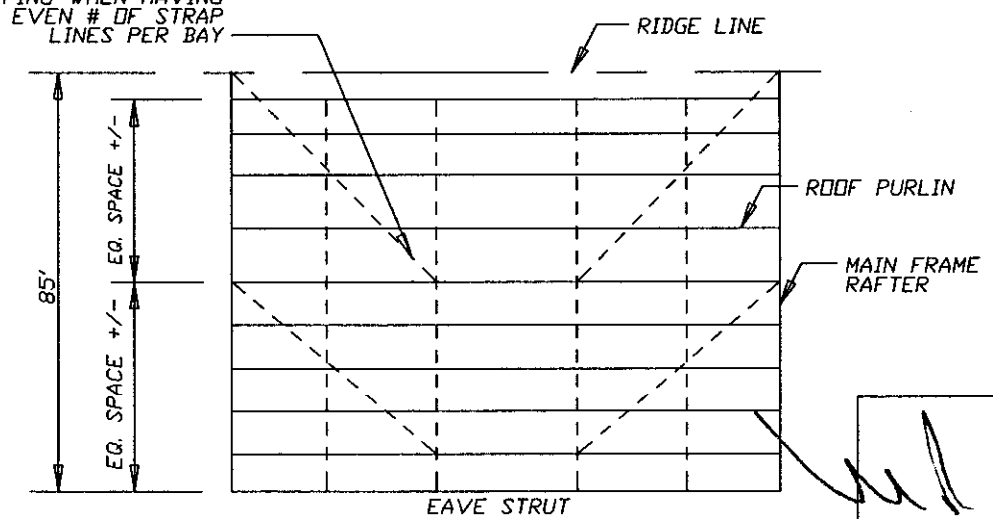
> 2 1/2" 12" SLOPE

USE FLAT STRAP WITH ANGLE AT 1st EAVE SPACE AND RIDGE ONLY
DESCENDING DIAGONALS AT PEAK (45° MAX. FROM PLANE OF FRAME)
DESCENDING DIAGONALS NOT TO EXCEED 75' INCREMENTS (NOMINAL EQUAL SPACES)
MAX 5'-0" O.C. WITH EQUAL SPACES OR SPACES PER DESIGN

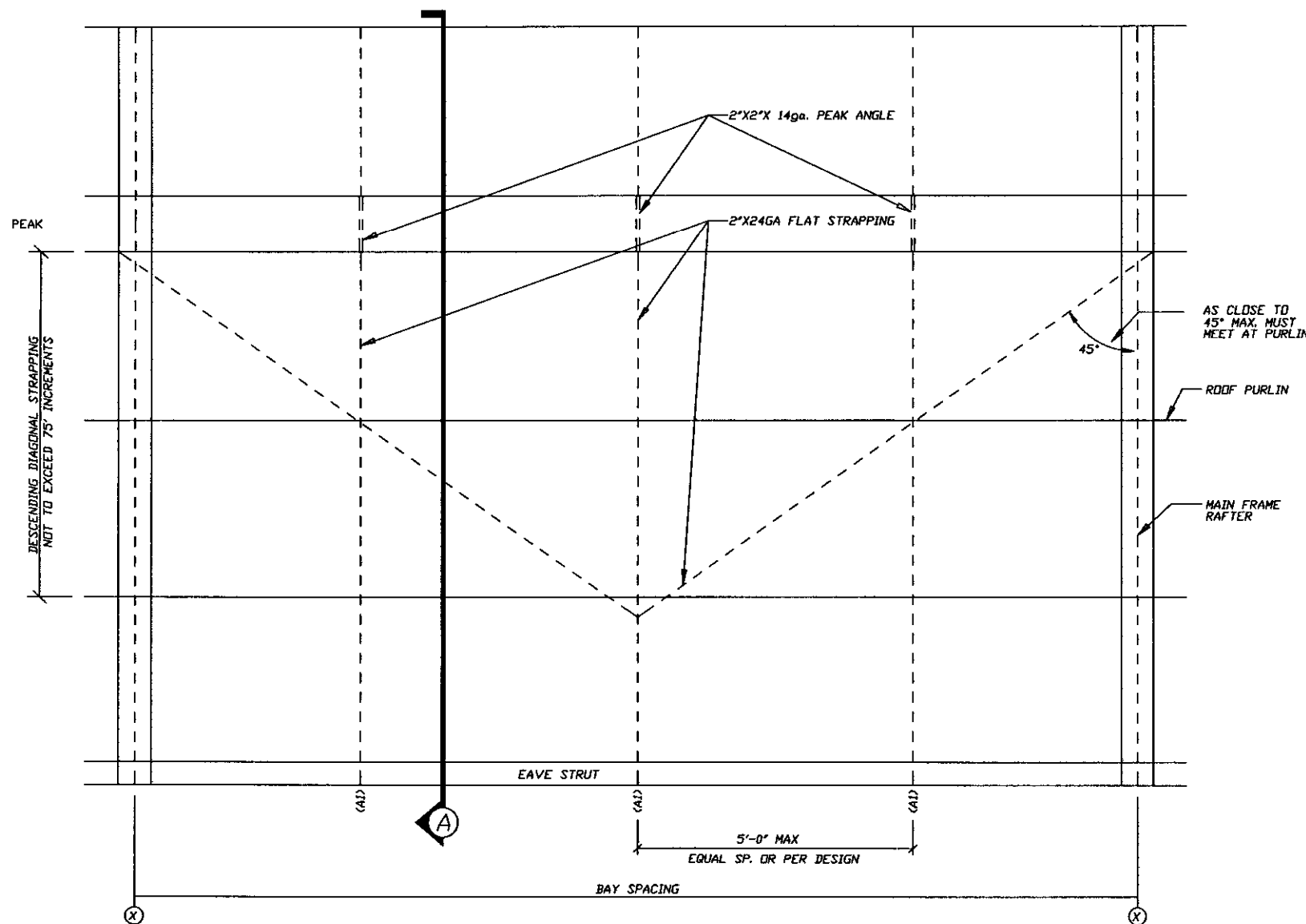


EXAMPLE #1: ODD # OF STRAPPING PER BAY
EQUAL SPACES +/-
NOT TO EXCEED 75'-0"

STRAPPING WHEN HAVING
AN EVEN # OF STRAP
LINES PER BAY



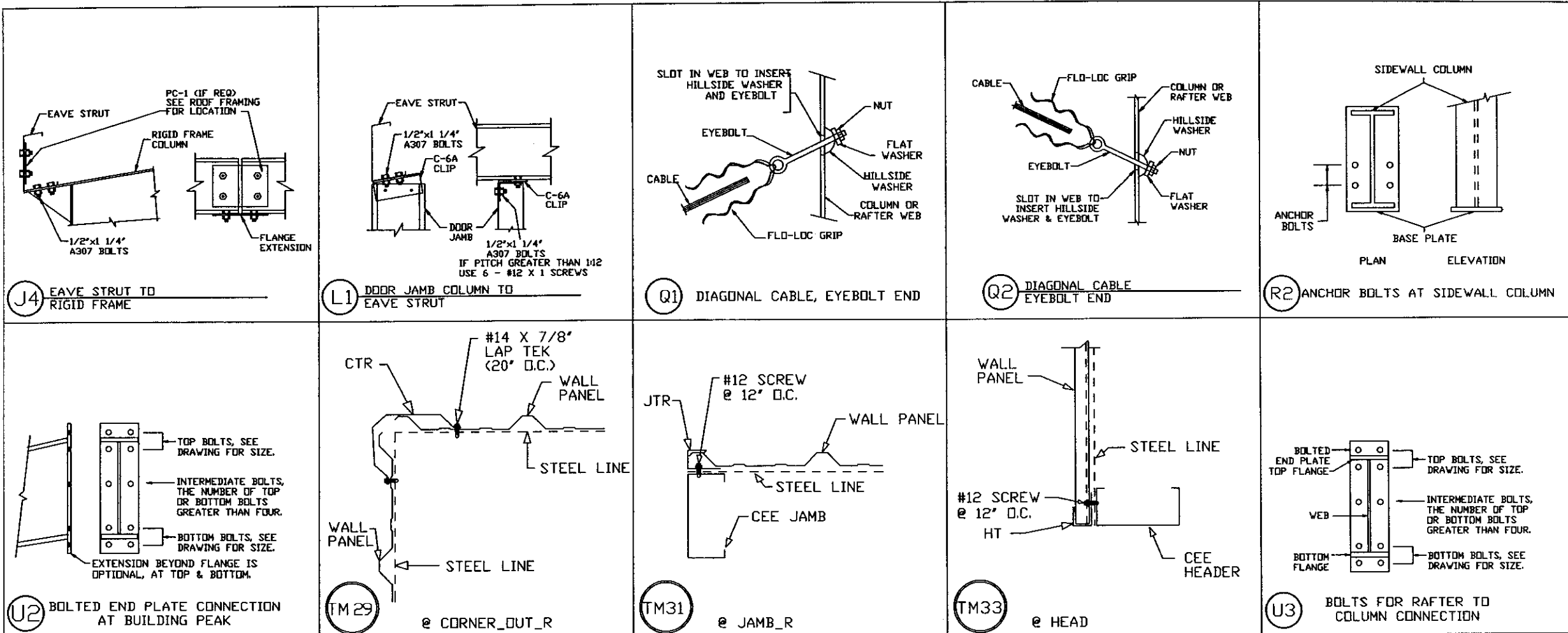
EXAMPLE #2: EVEN # OF STRAPPING PER BAY
EQUAL SPACES +/-
NOT TO EXCEED 75'-0"



SEAL

OCT 09 2008

REVISIONS							DRAWING STATUS		LIFETIME STEEL BLDGS		LIFETIME STEEL BLDGS	
REV.	DESCRIPTION	DATE	DTLR	DATE	CHKR	APPD	[] FOR CONSTRUCTION		PROJECT	60.00' x 121.3' x 17.2	Detail Page	
							[] FOR PERMIT ONLY		ID	19066-1	DESIGN: DRAFT/ARM/MMD CHECK: DB	
							[] FOR APPROVAL		PROJECT ADDRESS	LAKE CITY, FL 32024	DATE: 10/07/08 SHEET 13	
							[] OTHER, EXPLAIN					

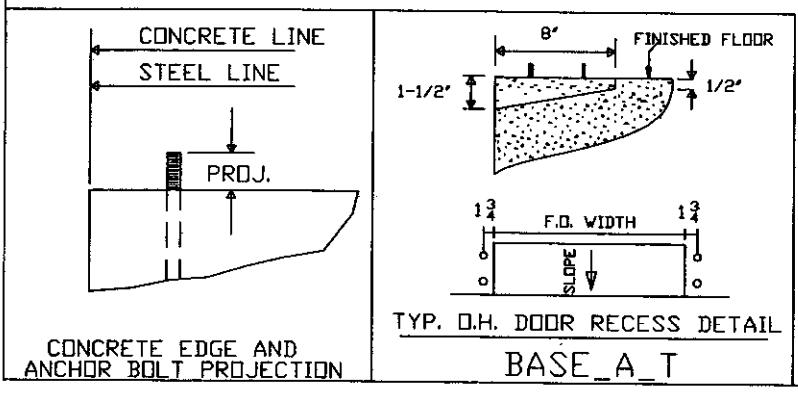
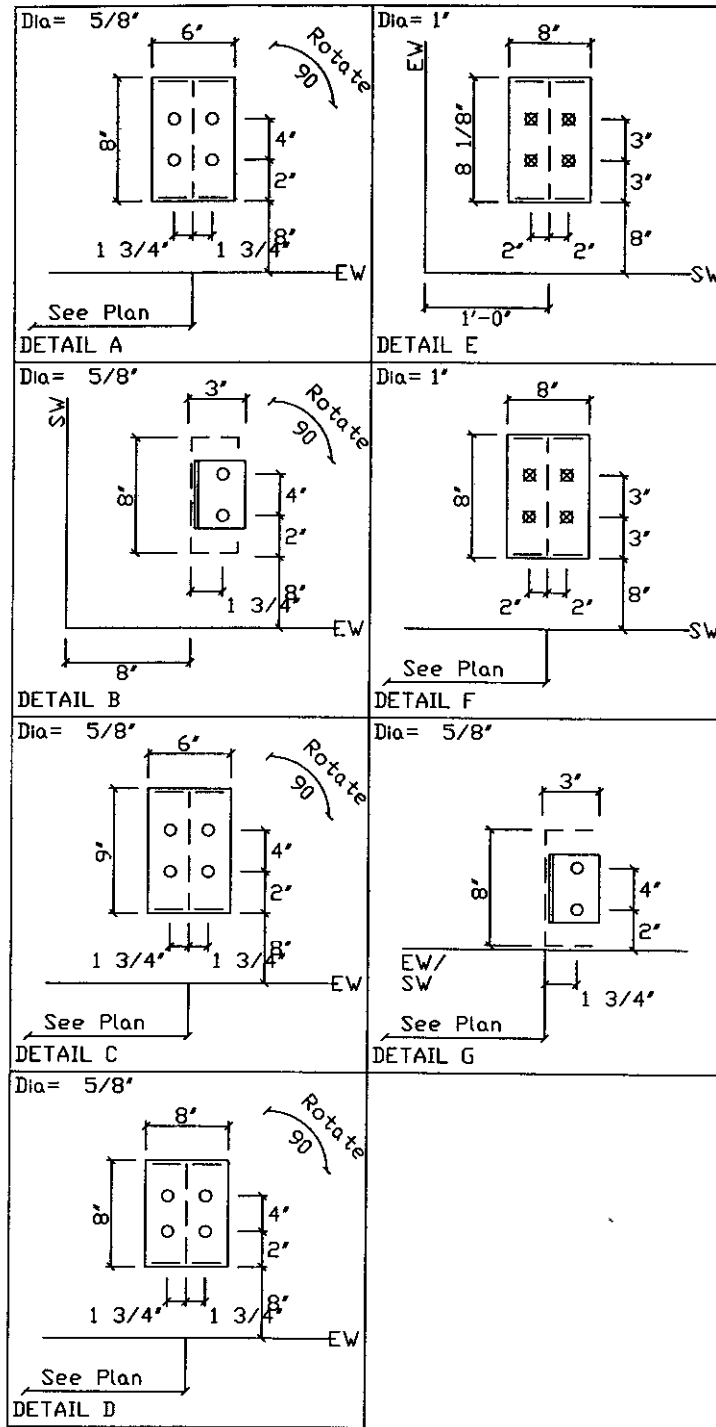


SEAL

[Signature]

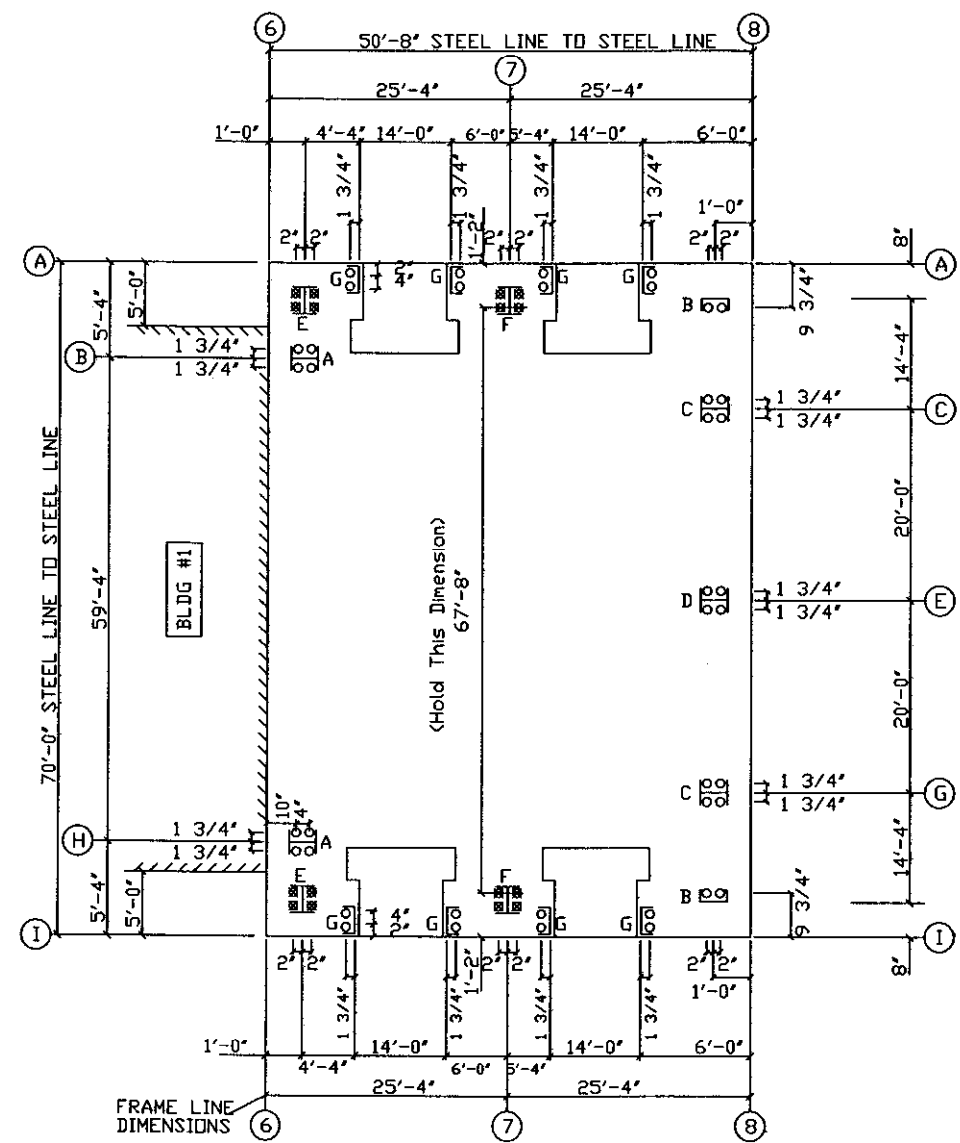
OCT 09 2008

REVISIONS							DRAWING STATUS	LIFETIME STEEL BLDGS		LIFETIME STEEL BLDGS		
REV.	DESCRIPTION:	DATE	DTLR	DATE	CHKR	APPD	☐ FOR CONSTRUCTION ☒ FOR PERMIT ONLY ☐ FOR APPROVAL ☐ OTHER, EXPLAIN -----	PROJECT	7000' x 50.6 x 16.0	Detail Page		
								ID	19066-2	DESIGN:	DRAFT:ARM/MMD	CHECK: DB
								PROJECT	LAKE CITY, FL 32024	DATE: 9/29/08	SHEET	11
								ADDRESS				



ANCHOR BOLT NOTES:
 VALUES GIVEN FOR BENDS AND ANCHOR BOLT TOTAL LENGTHS ARE SUGGESTED LENGTHS ONLY. IT IS THE RESPONSIBILITY OF THE FOUNDATION ENGINEER TO DETERMINE THESE VALUES SINCE THEY ARE A FUNCTION OF CONCRETE STRENGTH AND OTHER FACTORS.

ANCHOR BOLT SUMMARY					
Qty	Loc	Dia (in)	Total Len (in)	Bend Len (in)	Proj (in)
16	DJ	5/8"	9.00	3.00	3.00
24	EW	5/8"	12.00	3.00	3.00
16	RF	1"	22.00	4.00	3.00



ANCHOR BOLT PLAN
 NOTE: All Base Plates @ 100'-0" (U.N.)

SEAL

OCT 09 2008

REVISIONS						DRAWING STATUS		LIFETIME STEEL BLDGS		LIFETIME STEEL BLDGS	
REV.	DESCRIPTION	DATE	DTLR	DATE	CHKR	APPD		PROJECT	7000' x 50.6 x 16.0	ANCHOR BOLT PLAN & DETAILS	
								ID	19066-2	DESIGN	DRAFT:ARM/MMD CHECK: DB
								PROJECT ADDRESS	LAKE CITY, FL 32024	DATE: 9/29/08	SHEET 1

NOTES FOR REACTIONS

Building reactions are based on the following building data:
Width (ft) = 70.0
Length (ft) = 50.7
Eave Height (ft) = 16.0/ 16.0
Roof Slope (rise/12) = 3.0/ 3.0
Dead Load (psf) = 2.0
Collateral Load (psf) = 1.0
Roof Live Load (psf) = 20.0
Frame Live Load (psf) = 12.0
Wind Speed (mph) = 140.0
Wind Code (3 sec gust) = FBC 06
Exposure = B
Closed/Open = C
Importance Wind = 1.15
Importance Seismic = 1.50
Seismic Zone = C
Seismic Coeff (Fa/Sa) = 0.20

Id Description

- 1 DL+CL+LL
2 0.60DL+WL1
3 0.60DL+WR1
4 1.02DL+1.02CL-0.70SeisL
5 0.60DL+WR1+VS
6 0.60DL+WP+LnWnd1
7 0.60DL+WL1+VS

GENERAL NOTES

- (1.) APPLICATION OF ENGINEERS SEAL IS FOR METAL BUILDING ONLY AND DOES NOT REPRESENT THE PROFESSIONAL OF RECORD.
(2.) FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF THE METAL BUILDING MANUFACTURER.
(3.) ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLERANCE OF +/- 1/8" IN BOTH ELEVATION AND LOCATION.
(4.) THE BUILDING REACTION DATA REPORTS THE LOADS WHICH THIS BUILDING PLACES ON THE FOUNDATION. THE FOUNDATION IS TO BE DESIGNED BY A QUALIFIED ENGINEER TO SUPPORT THE BUILDING REACTIONS IN ADDITION TO OTHER LOADS IMPOSED BY THE BUILDING USE OR OCCUPANCY WITH RESPECT TO JOB SITE CONDITIONS.
(5.) ALL ANCHOR BOLTS TO BE ASTM SPECIFICATION A307 UNLESS OTHERWISE NOTED.
(6.) VALUES GIVEN FOR BENDS AND ANCHOR BOLT TOTAL LENGTHS ARE SUGGESTED LENGTHS ONLY. IT IS THE RESPONSIBILITY OF THE FOUNDATION ENGINEER TO DETERMINE THESE VALUES SINCE THEY ARE A FUNCTION OF CONCRETE STRENGTH AS WELL AS OTHER FACTORS.

BRACING REACTIONS, PANEL SHEAR

---Wall---		Col		* Reactions (k)		Panel Shear (lb/ft)
Loc	Line	Line	Line	Horz	Vert	
L_EW	6	Rigid Frame At Endwall				1000
F_SW	I	Bracing In Roof To Rigid Frame				1000
R_EW	8					
B_SW	A					

Protection of Opening

This building is located in a wind-borne debris region. Exterior glazing is assumed to be impact resistant and meet the provisions of the missile test, or they should be protected by impact resistant covering meeting the requirements of SST12, ASTM E 1886 and ASTM E 1996 or Miami-Dade PA 201, 202, & 203.
Openings may also be protected by structural wood panels having a min. thickness of 7/16" and maximum panel span of 8 feet. Attachment hardware and fastening schedule shall be in accordance with the following table.

WIND-BORNE DEBRIS PROTECTION FASTENING SCHEDULE FOR WOOD STRUCTURAL PANELS (PLYWOOD)

FASTENER TYPE	FASTENER SPACING (in.) ^{1,2}			
	PANEL SPAN ≤ 2ft	2ft< PANEL SPAN <4 ft.	4ft< PANEL SPAN <6 ft.	6ft< PANEL SPAN <8 ft.
#8 Wood screw-based anchor with 2" embedment length.	16	16	10	8
#10 Wood screw-based anchor with 2" embedment length.	16	16	12	9
1/4" Lag screw-based anchor with 2" embedment length.	16	16	16	16

SI: 1 inch=25.4 mm 1 foot=305 mm

Notes:

- 1.) This table is based on a maximum wind speed of 140 m.p.h. (63 m/s) and mean roof height of 45 feet (13.716 m) or less.
2.) Fasteners shall be installed at opposing ends of the wood structural panel.
3.) Where screws are attached to masonry or masonry/stucco, they shall be attached using vibration-resistant anchors having a minimum withdrawal capacity of 490 lb. (2180 kN).

REVISIONS

REV.	DESCRIPTION	DATE	DTLR	DATE	CHKR	APPD

DRAWING STATUS
[X] FOR CONSTRUCTION
[] FOR PERMIT ONLY
[] FOR APPROVAL
[] OTHER, EXPLAIN

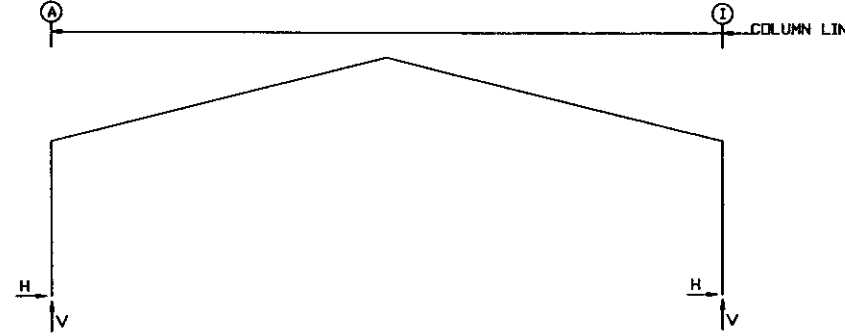
LIFETIME STEEL BLDGS

PROJECT	70.00' x 50.6 x 16.0
ID	19066-2
PROJECT ADDRESS	LAKE CITY, FL 32024

LIFETIME STEEL BLDGS

ANCHOR BOLT REACTIONS	
DESIGN:	DRAFT/ARM/MMD
DATE: 9/29/08	CHECK: DB
SHEET 2	

FRAME LINES: 6 7



RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frn Line	Col Line	Anc. No	Bolt D(in)	Base Wid	Plate Len	Grout (in)
6	A	4	1.000	8.000	8.130	0.500
6	I	4	1.000	8.000	8.130	0.500

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frn Line	Col Line	Anc. No	Bolt D(in)	Base Wid	Plate Len	Grout (in)
7	A	4	1.000	8.000	8.004	0.625
7	I	4	1.000	8.000	8.004	0.625

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	---Dead---		---Collateral---		---Live---		---Wind_L1---		---Wind_R1---		---Wind_L2---	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
6	A	2.09	4.01	0.75	1.21	8.63	14.87	-21.25	-37.71	-9.10	-30.83	-16.25	-24.35
6	I	-2.09	4.00	-0.75	1.21	-8.63	14.83	9.10	-30.83	21.25	-37.71	4.09	-17.43
7	A	2.11	3.91	0.77	1.20	8.85	14.58	-22.85	-37.03	-8.77	-29.38	-17.74	-23.63
7	I	-2.11	3.90	-0.77	1.20	-8.85	14.54	8.77	-29.38	22.85	-37.03	3.66	-15.92

Frame Line	Column Line	---Wind_R2---		---Wind_Ln1---		---Wind_Ln2---		---Seismic_L---		---Seismic_R---	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
6	A	-4.09	-17.43	-9.12	-28.66	-12.10	-24.13	-0.59	-0.25	0.59	0.25
6	I	16.25	-24.35	12.10	-24.13	9.12	-28.66	-0.59	0.25	0.59	-0.25
7	A	-3.66	-15.92	-9.35	-28.76	-12.33	-24.22	-0.72	-0.30	0.72	0.30
7	I	17.74	-23.63	12.33	-24.22	9.35	-28.76	-0.72	0.30	0.72	-0.30

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frn Line	Col Line	Dead	Collat	Live	Rafter Wind_L	Rafter Wind_R	Brace Wind_L	Brace Wind_R	Wind_P	Wind_S	Wind_Ln	Seis_L	Seis_R
6	B	0.18	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.68	0.00	0.00	0.00
6	H	0.18	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.68	0.00	0.00	0.00
8	I	0.59	0.08	2.38	-5.72	-5.67	-5.72	-5.67	0.00	0.00	-3.00	0.01	0.04
8	G	0.97	0.26	4.82	-9.68	-5.31	-9.68	-5.31	-5.09	5.58	-10.10	-0.04	-0.04
8	E	1.52	0.28	5.57	-8.44	-8.44	-8.44	-8.44	-7.56	8.28	-8.95	0.03	0.03
8	C	0.97	0.26	4.82	-5.31	-9.68	-5.31	-9.68	-5.09	5.58	-10.10	-0.04	-0.04
8	A	0.59	0.08	2.38	-5.67	-5.72	-5.67	-5.72	0.00	0.00	-3.00	0.04	0.01

ENDWALL COLUMN: ANCHOR BOLTS & BASE PLATES

Frn Line	Col Line	Anc. No	Bolt D(in)	Base Wid	Plate Len	Grout (in)
6	B	4	0.625	6.000	8.000	0.500
6	H	4	0.625	6.000	8.000	0.500
8	I	2	0.625	3.000	5.750	0.134
8	G	4	0.625	6.000	9.000	0.500
8	E	4	0.625	8.000	8.000	0.500
8	C	4	0.625	6.000	9.000	0.500
8	A	2	0.625	3.000	5.750	0.134

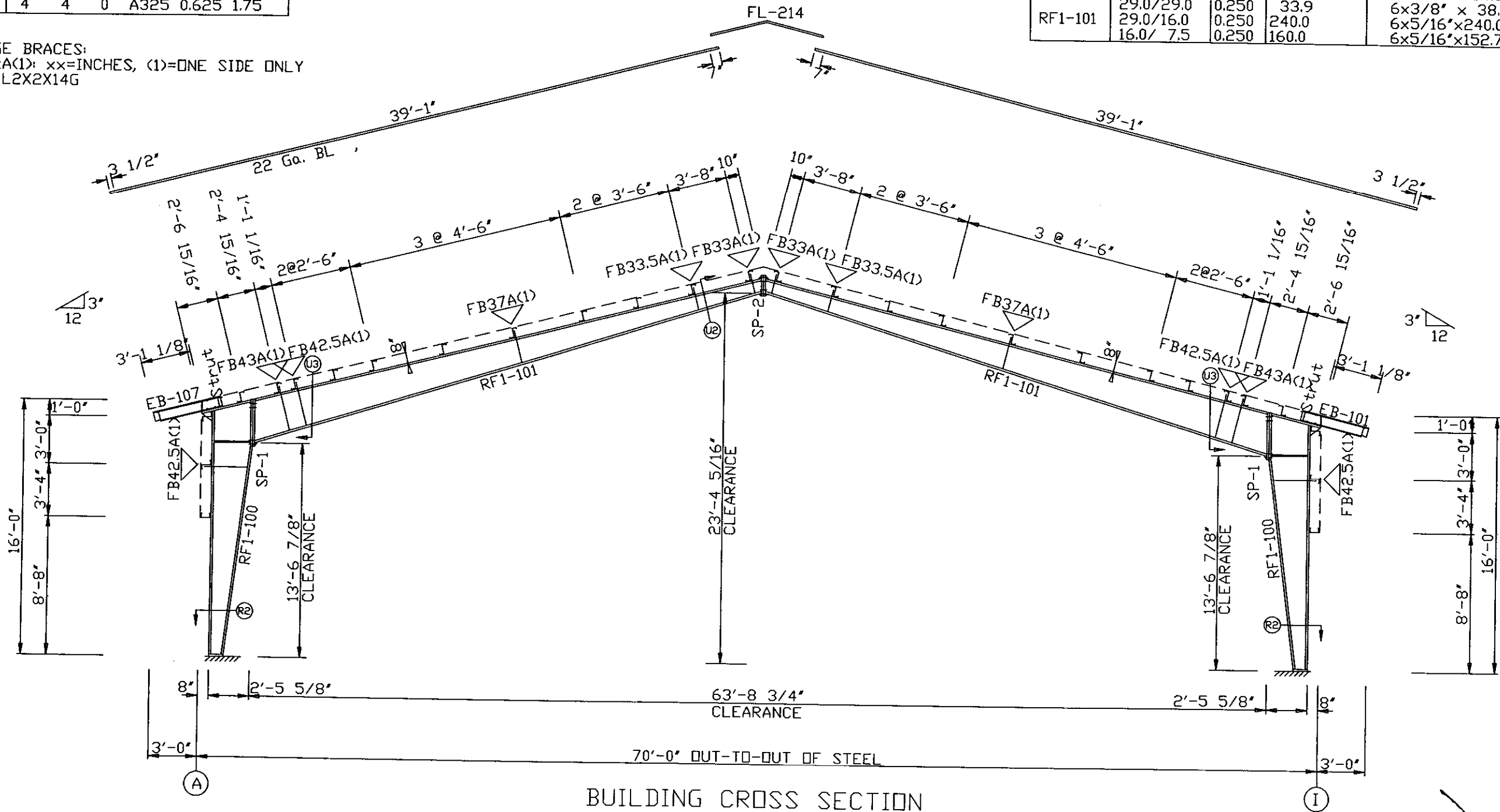
SEAL

OCT 09 2008

SPlice BOLTS						
Splice Mark	Quan		Bolt			
	Top	Bot	Int	Type	Dia	Len
SP- 1	4	4	0	A325	0.875	2.50
SP- 2	4	4	0	A325	0.625	1.75

MEMBER SIZE TABLE (in)					
PIECE	WEB DEPTH		WEB PLATE		OUTSIDE FLANGE W x T x LEN
	START	END	THICK	LENGTH	
RF1-100	7.5	29.0	0.188	158.3	6x5/16"x184.9
RF1-101	29.0	29.0	0.250	33.9	6x3/8" x 38.4
	29.0	16.0	0.250	240.0	6x5/16"x240.0
	16.0	7.5	0.250	160.0	6x5/16"x152.7
					6x1/4" x240.0
					6x1/4" x158.7

FLANGE BRACES:
FBxxA(1): xx=INCHES, (1)=ONE SIDE ONLY
A - L2X2X14G



GENERAL NOTES:

* NOTICE TO ERECTOR *

(A)It is IMPORTANT that for members exceeding 30 ft. in length that a spreader bar be used when lifting.

(B)ALL flange braces and wind bracing must be installed prior to exterior finishes being applied.

REVISIONS							DRAWING STATUS		LIFETIME STEEL BLDGS		LIFETIME STEEL BLDGS	
REV.	DESCRIPTION	DATE	DLR	DATE	CHKR	APPD	[] FOR CONSTRUCTION		PROJECT	70.00' x 50.6 x 16.0	BUILDING CROSS SECTION	
							[X] FOR PERMIT ONLY		ID	19066-2	DESIGN: DRAFT/ARM/MMD CHECK: DB	
							[] FOR APPROVAL		PROJECT ADDRESS	LAKE CITY, FL 32024	DATE: 9/29/08 SHEET 3	
							[] OTHER, EXPLAIN					

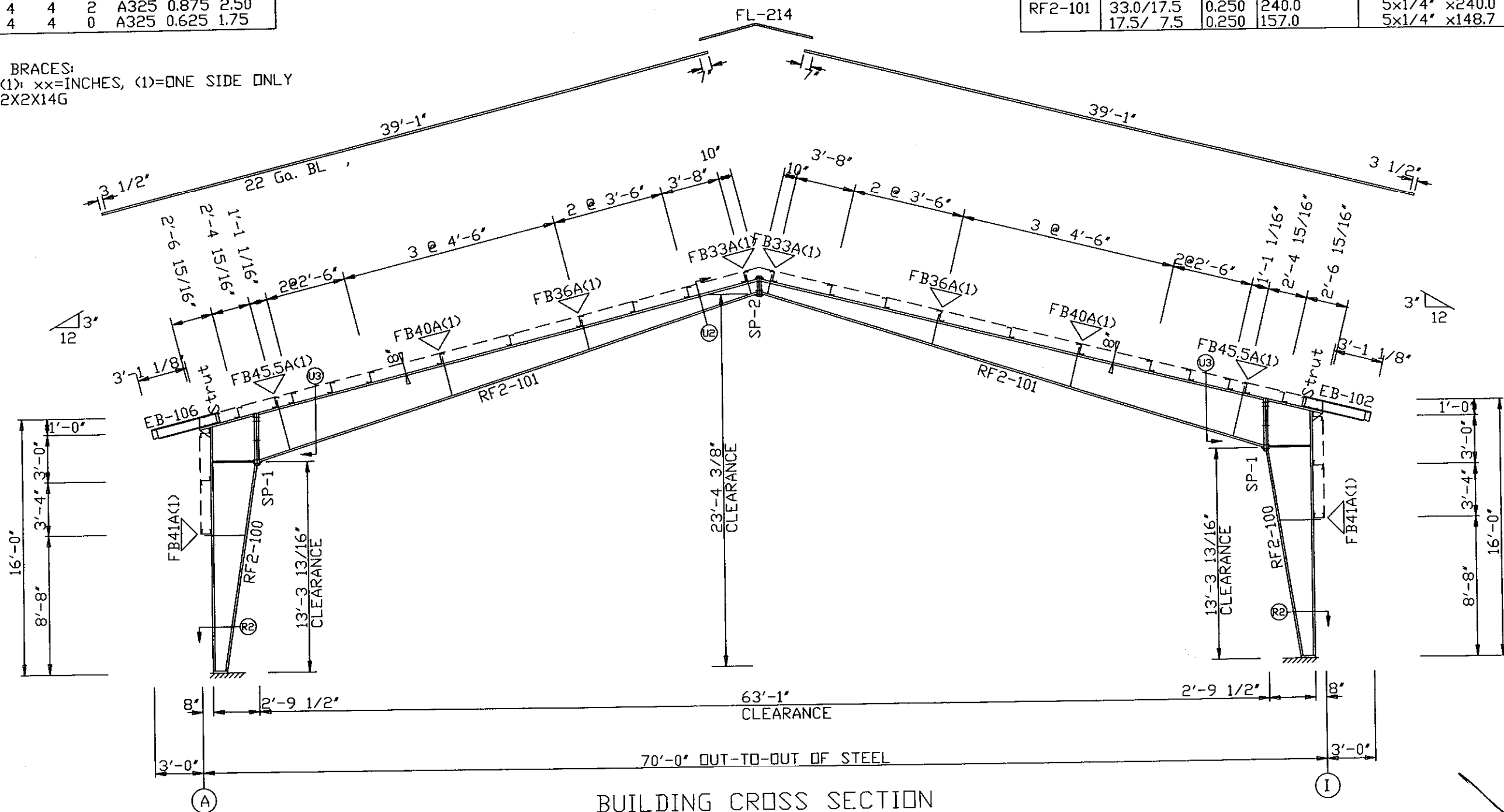
SEAL

OCT 09 2008

SPlice BOLTS						
Splice Mark	Quan		Bolt		Type	
	Top	Bot	Int	Type	Dia	Len
SP-1	4	4	2	A325	0.875	2.50
SP-2	4	4	0	A325	0.625	1.75

MEMBER SIZE TABLE (in)					
PIECE	WEB DEPTH		WEB PLATE THICK	OUTSIDE FLANGE	
	START/END	LENGTH		W x T x LEN	INSIDE FLANGE W x T x LEN
RF2-100	7.5/33.0	0.250	193.1	5x1/4" x184.8	5x1/4" x157.1
RF2-101	33.0/17.5	0.250	240.0	6x5/16" x 42.4	5x1/4" x240.0
	17.5/ 7.5	0.250	157.0	5x1/4" x148.7	5x1/4" x155.8

FLANGE BRACES:
 FBxxA(1): xx=INCHES, (1)=ONE SIDE ONLY
 A - L2X2X14G



BUILDING CROSS SECTION
 FOR FRAME LINE 7

GENERAL NOTES:

- * NOTICE TO ERECTOR *
- (A) It is IMPORTANT that for members exceeding 30 ft. in length that a spreader bar be used when lifting.
- (B) ALL flange braces and wind bracing must be installed prior to exterior finishes being applied.

REVISIONS						
REV.	DESCRIPTION	DATE	DLR	DATE	CHKR	APPD

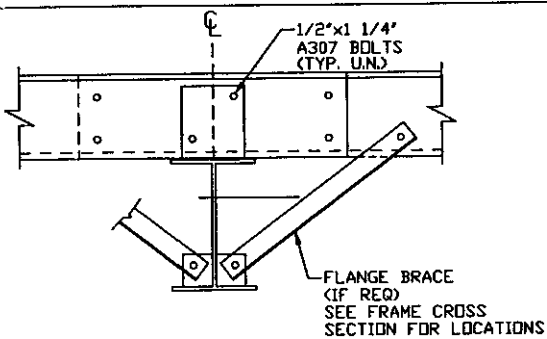
DRAWING STATUS	
☐	FOR CONSTRUCTION
☐	FOR PERMIT ONLY
☐	FOR APPROVAL
☐	OTHER, EXPLAIN

LIFETIME STEEL BLDGS	
PROJECT	7000' x 50.6 x 16.0
ID	19066-2
PROJECT ADDRESS	LAKE CITY, FL 32024

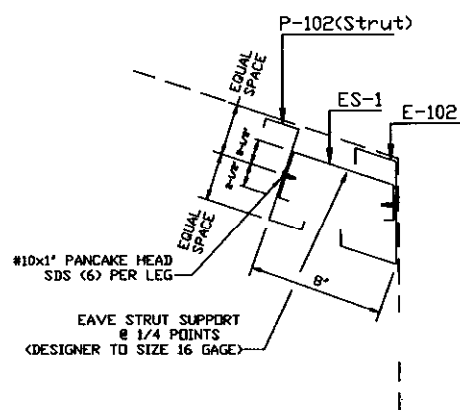
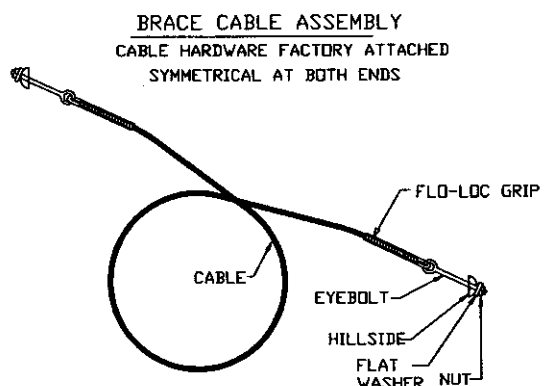
LIFETIME STEEL BLDGS	
BUILDING CROSS SECTION	DESIGN: DRAFT:ARM/MMD CHECK: DB
DATE: 9/29/08	SHEET 4

SEAL

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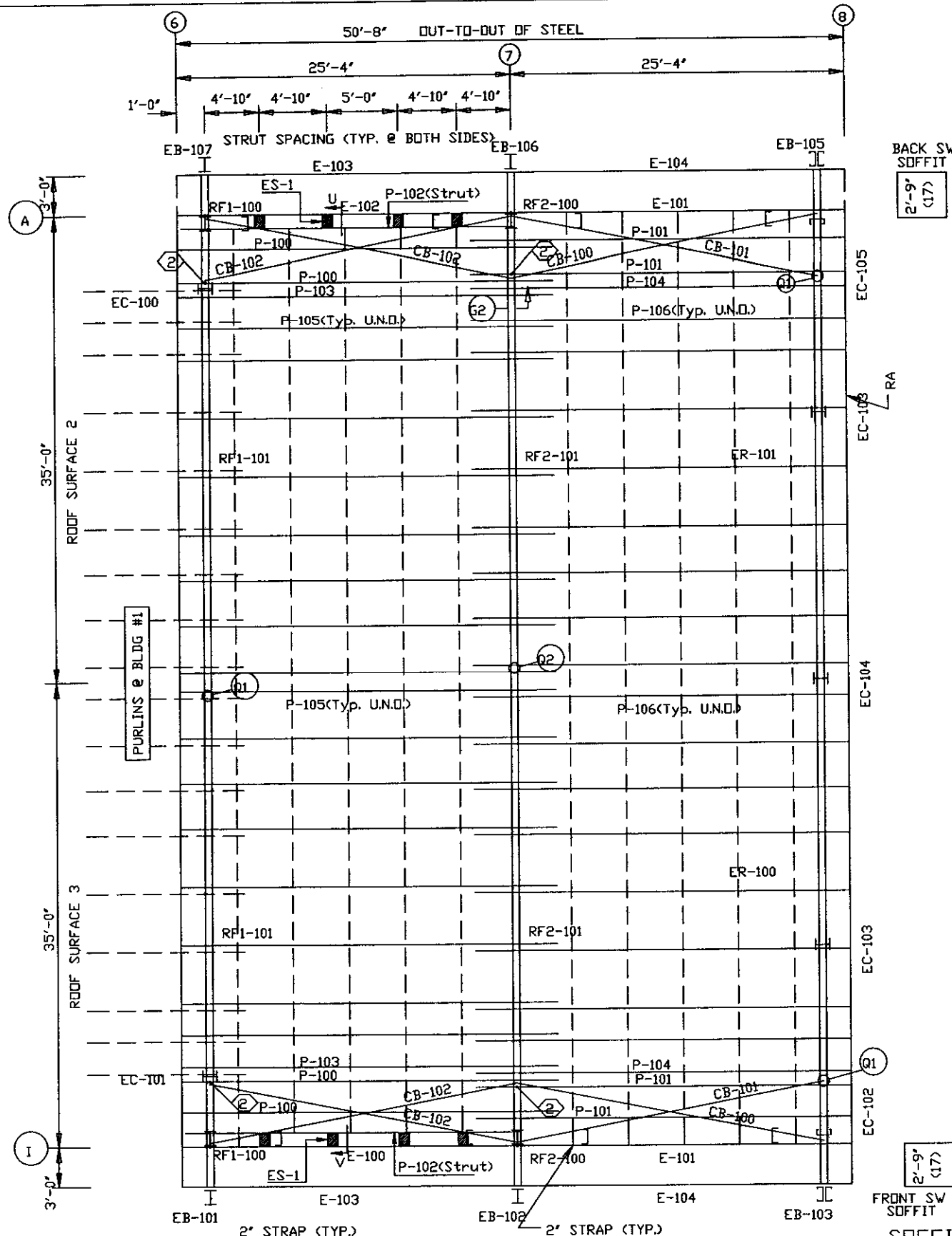


ADDITIONAL HOLES ARE PRESENT FOR ALIGNMENT PURPOSES ONLY



SECTION U-U (AS SHOWN)

SECTION V-V (OPPOSITE)



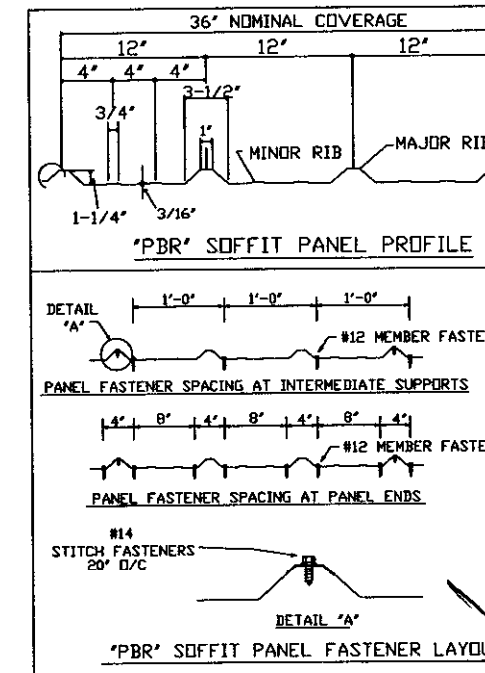
ROOF FRAMING PLAN

TRIM TABLE		
MARK	LENGTH	DETAIL
1	FL-214	10'-2"

EXTENSION/CANOPY BOLTS				
MARK	QUAN	TYPE	DIA	LENGTH
EB-101	4	A325	5/8"	1 3/4"
EB-102	4	A325	5/8"	1 3/4"
EB-103	4	A325	5/8"	1 3/4"
EB-105	4	A325	5/8"	1 3/4"
EB-106	4	A325	5/8"	1 3/4"
EB-107	4	A325	5/8"	1 3/4"

SPECIAL BOLTS				
MARK	QUAN	TYPE	DIA	LENGTH
1	2	A307	1/2"	1 1/4"
2	2	A325	1/2"	1 1/4"

MEMBER TABLE		
MARK	PART	LENGTH
EB-100	8X4CH16	4'-10 9/16"
EB-101	W8X10	4'-3 1/16"
EB-102	W8X10	4'-3 1/8"
EB-103	8X50D16	4'-3 1/8"
EB-104	8X4CH16	4'-10 9/16"
EB-105	8X50D16	4'-3 1/8"
EB-106	W8X10	4'-3 1/8"
EB-107	W8X10	4'-3 1/16"
P-100	8X35Z14	28'-5 1/2"
P-101	8X35Z14	28'-5 1/2"
P-102	8X25Z16	24'-11 1/2"
P-103	8X25Z15	28'-5 1/2"
P-104	8X25Z13	28'-5 1/2"
P-105	8X25Z16	28'-5 1/2"
P-106	8X25Z16	28'-5 1/2"
E-100	825E350D16	23'-8 1/2"
E-101	825E350D16	23'-8 1/2"
E-102	825E350D16	23'-8 1/2"
ES-1	825E350D16	1'-0"
E-103	8X25C12	25'-0"
E-104	8X35C12	25'-0"
CB-100	0.250CBL	25'-2"
CB-101	0.250CBL	25'-3"
CB-102	0.500CBL	25'-3"



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REVISIONS						
REV.	DESCRIPTION	DATE	DLR	DATE	CHKR	APPR

DRAWING STATUS

[] FOR CONSTRUCTION

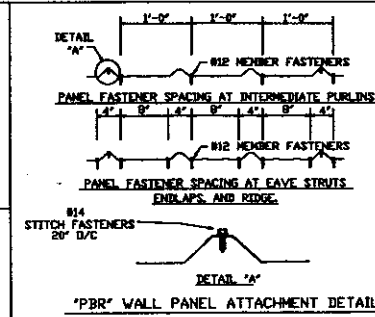
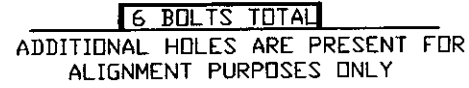
[X] FOR PERMIT ONLY

[] FOR APPROVAL

[] OTHER, EXPLAIN

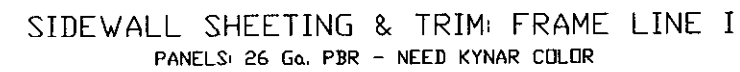
LIFETIME STEEL BLDGS	
PROJECT	70.00' x 50.6' x 16.0'
ID	19066-2
PROJECT ADDRESS	LAKE CITY, FL 32024

LIFETIME STEEL BLDGS	
ROOF FRAMING	
DESIGN	DRAFT/ARM/MMI
DATE: 9/29/08	SHEET 5



TRIM TABLE			
FRAME LINE I			
OID	MARK	LENGTH	DETAIL
1	TBD	3'-6"	
2	TBD	2'-1"	
3	TBD	1'-7"	
4	TBD	2'-9"	
5	CTR	7'-6"	TM29
6	JTR	5'-6"	TM31
7	HT	14'-3"	TM33
8	HT	20'-3"	
9	HT	10'-11"	
10	FL-248H	20'-2"	

MEMBER TABLE		
FRAME LINE 1		
MARK	PART	LENGTH
DJ-100	C8X11.5	15'-3 3/4"
DH-100	8x35C16	14'-0"
E-100	825E350D16	23'-8 1/2"
E-101	825E350D16	23'-8 1/2"
G-109	8X25C16	5'-0"
G-110	8X25Z16	5'-0"
G-111	8X25Z16	5'-0"
G-112	8X25C16	5'-8"
G-113	8X25Z16	7'-10"
G-114	8X25C16	5'-0"
G-115	8X25Z16	7'-2"
G-116	8X25C16	5'-8"
G-117	8X25Z16	5'-8"



(1.) IF CABLE BRACING, WIND BENTS, WIND COLUMNS, OR WEAK AXIS DESIGN OF SIDE WALL COLUMNS WERE NOT PROVIDED IT HAS BEEN DETERMINED THAT DIAPHRAGM PANEL ACTION IS SUFFICIENT TO RESIST LONGITUDINAL FORCES. TEMPORARY BRACING SHOULD BE PROVIDED BY ERECTOR UNTIL ALL WALL AND ROOF PANELS ARE INSTALLED.

REVISIONS						
REV.	DESCRIPTION	DATE	DTLR	DATE	CHKR	APPR

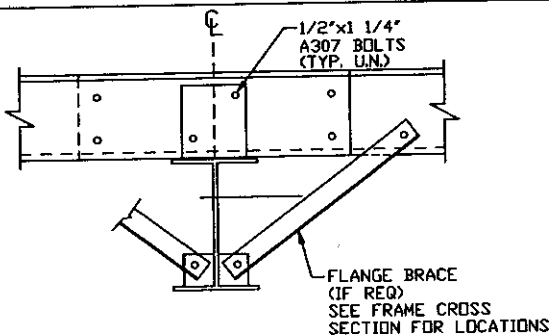
DRAWING STATUS
☐ FOR CONSTRUCTION
☒ FOR PERMIT ONLY
☐ FOR APPROVAL
☐ OTHER, EXPLAIN

LIFETIME STEEL BLDGS	
PROJECT	70.00' x 50.6 x 16.0
ID	19066-2
PROJECT ADDRESS	LAKE CITY, FL 32024

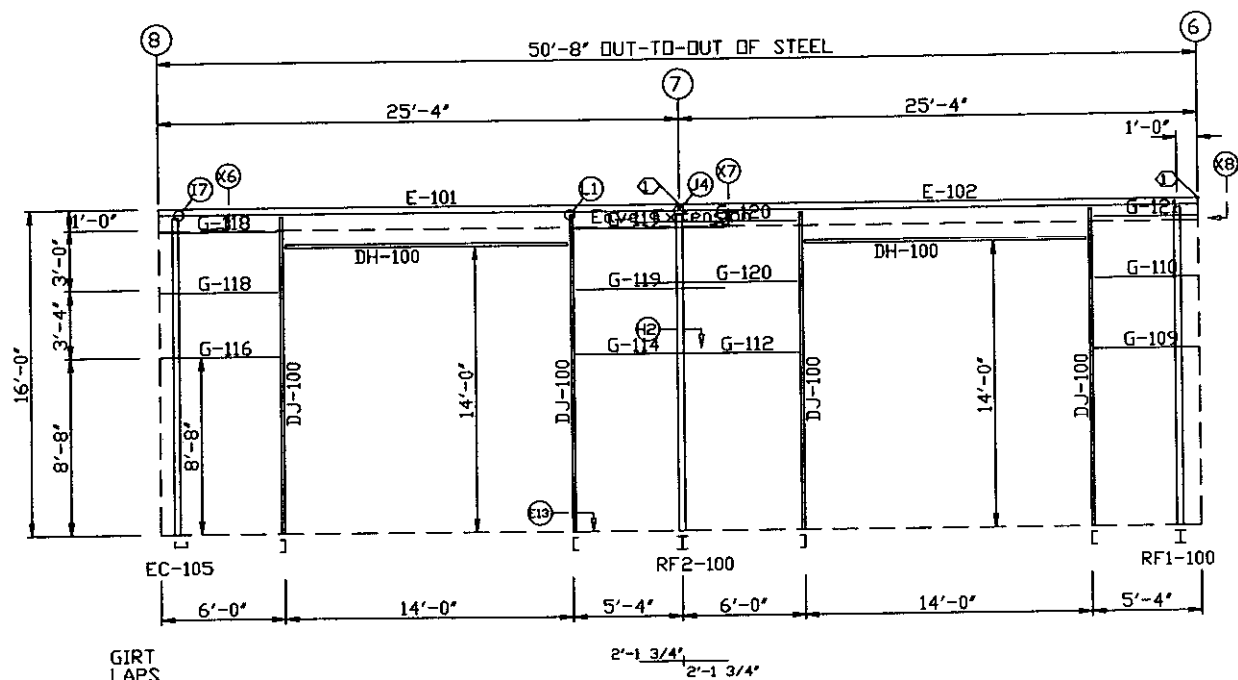
LIFETIME STEEL BLDGS		
SIDEWALL FRAMING		
DESIGN:	DRAFT: ARM/MMD	CHECK: DB
DATE: 9/29/08	SHEET	6

SEAL

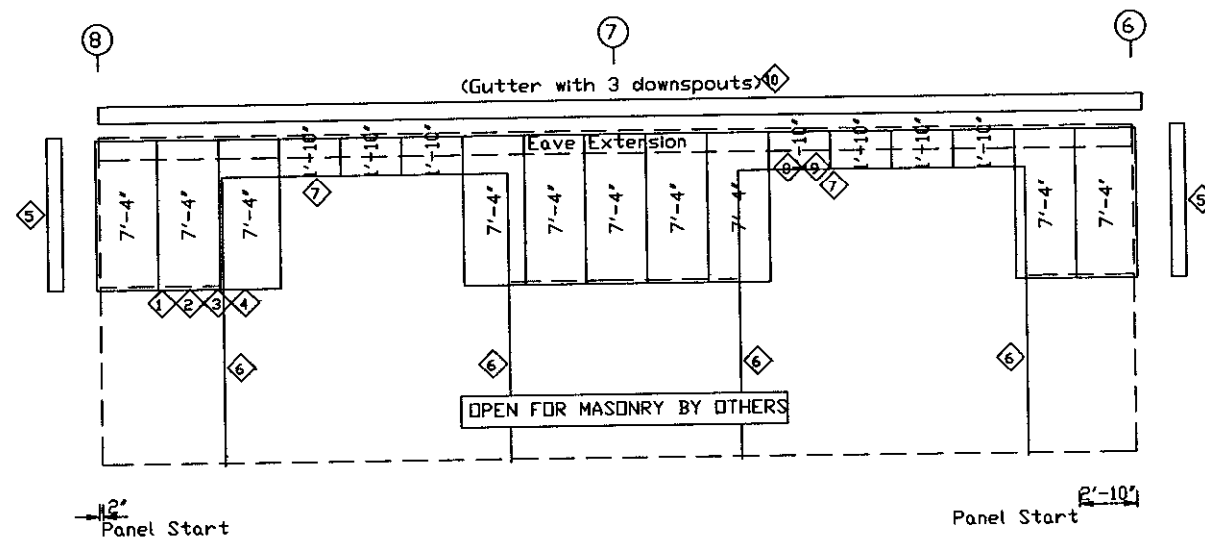
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ADDITIONAL HOLES ARE PRESENT FOR ALIGNMENT PURPOSES ONLY



SIDEWALL FRAMING: FRAME LINE A



SIDEWALL SHEETING & TRIM: FRAME LINE A
PANELS: 26 Ga. PBR - NEED KYNAR COLOR

TRIM TABLE			
FRAME LINE A			
ID	MARK	LENGTH	DETAIL
1	TBD	3'-6"	
2	TBD	2'-1"	
3	TBD	1'-7"	
4	TBD	2'-9"	
5	CTR	7'-6"	TM29
6	JTR	5'-6"	TM31
7	HT	14'-3"	TM33
8	HT	20'-3"	
9	HT	10'-11"	

SPECIAL BOLTS				
ID	QUAN	TYPE	DIA	LENGTH WASH
1	4	A307	1/2"	1 1/4" 0

MEMBER TABLE		
FRAME LINE A		
MARK	PART	LENGTH
DJ-100	C8X11.5	15'-3 3/4"
DH-100	8X35C16	14'-0"
E-101	825E350D16	23'-8 1/2"
E-102	825E350D16	23'-8 1/2"
G-109	8X25C16	5'-0"
G-110	8X25Z16	5'-0"
G-112	8X25C16	5'-8"
G-114	8X25C16	5'-0"
G-116	8X25C16	5'-8"
G-118	8X25Z16	5'-8"
G-119	8X25Z16	7'-2"
G-120	8X25Z16	7'-10"
G-121	8X25Z16	5'-0"

GENERAL NOTES:
(1.) IF CABLE BRACING, WIND BENTS, WIND COLUMNS, OR WEAK AXIS DESIGN OF SIDE WALL COLUMNS WERE NOT PROVIDED IT HAS BEEN DETERMINED THAT DIAPHRAGM PANEL ACTION IS SUFFICIENT TO RESIST LONGITUDINAL FORCES. TEMPORARY BRACING SHOULD BE PROVIDED BY ERECTOR UNTIL ALL WALL AND ROOF PANELS ARE INSTALLED.

REVISIONS						
REV.	DESCRIPTION	DATE	DTLR	DATE	CHKR	APPD

DRAWING STATUS
☐ FOR CONSTRUCTION
☒ FOR PERMIT ONLY
☐ FOR APPROVAL
☐ OTHER, EXPLAIN

LIFETIME STEEL BLDGS	
PROJECT	7000' x 50.6 x 16.0
ID	19066-2
PROJECT ADDRESS	LAKE CITY, FL 32024

LIFETIME STEEL BLDGS	
SIDEWALL FRAMING	
DESIGN	DRAFT:ARM/MMD CHECK: DB
DATE: 9/29/08	SHEET 7

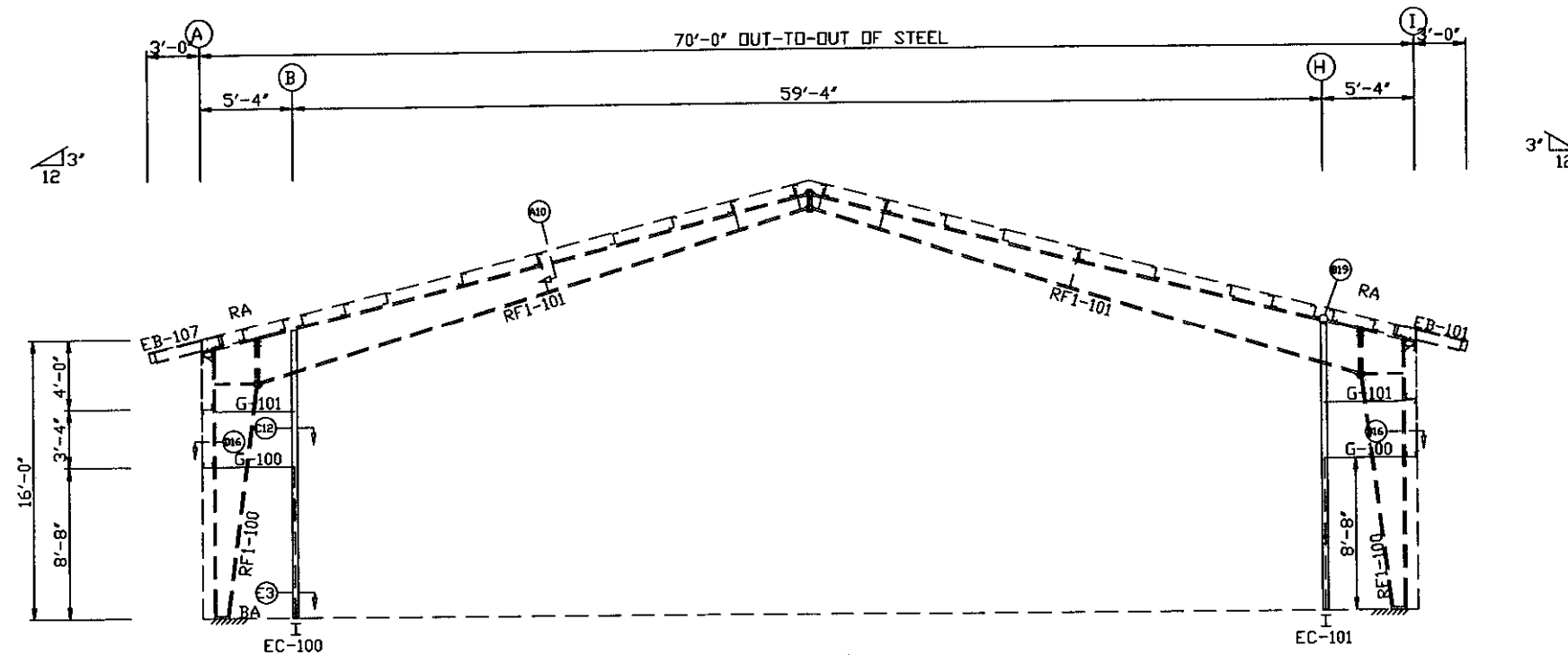
SEAL

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BOLT TABLE				
FRAME LINE 6				
LOCATION	QUAN	TYPE	DIA	LENGTH
Columns	4	A325	1/2"	1 1/4"

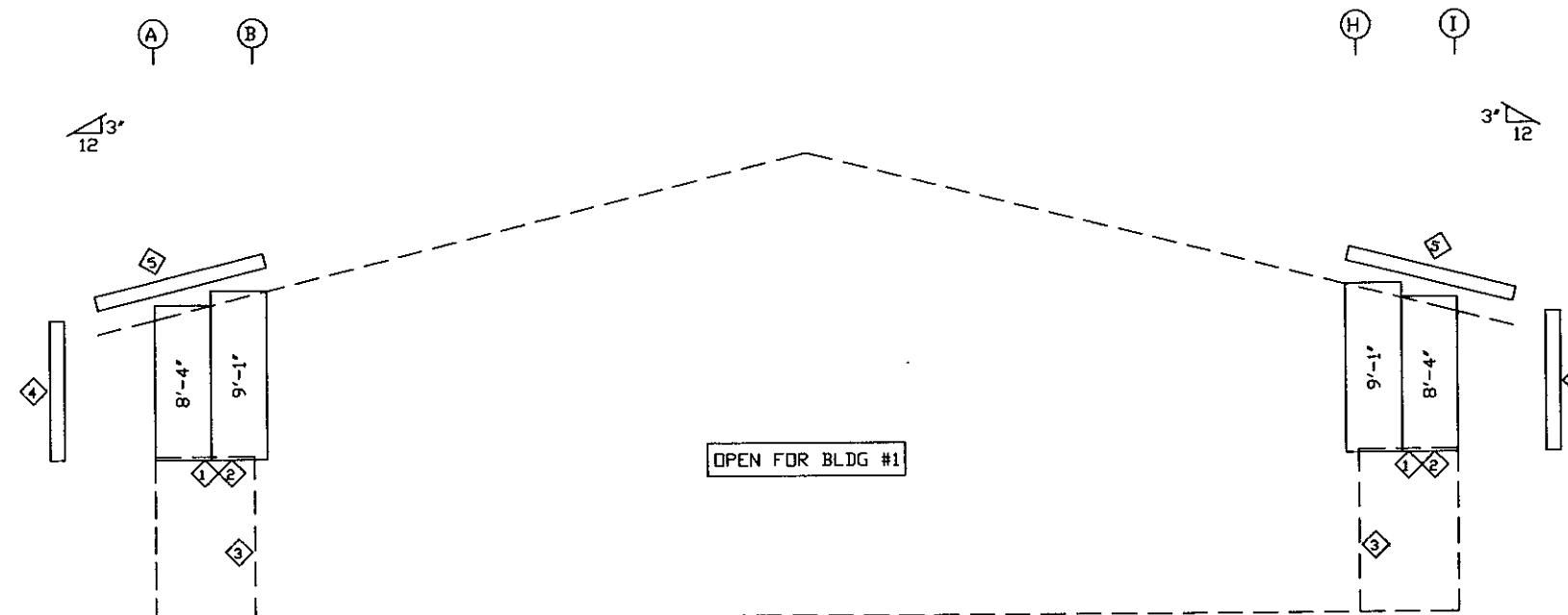
TRIM TABLE			
FRAME LINE 6			
ID	MARK	LENGTH	DETAIL
1	TBD	3'-6"	
2	TBD	2'-1"	
3	JTR	8'-10"	TRIM_31
4	CTR	7'-6"	TRIM_29
5	XFL-110	5'-3"	TRIM_11

MEMBER TABLE			
FRAME LINE 6			
MARK	PART	LENGTH	
EC-100	W8541	14'-2"	7/16"
EC-101	W8541	14'-2"	7/16"
G-100	8X25C16	4'-7"	1/2"
G-101	8X25Z16	4'-11"	1/2"



GIRT
LAPS

ENDWALL FRAMING: FRAME LINE 6



ENDWALL SHEETING & TRIM: FRAME LINE 6
PANELS: 26 Ga. PBR - NEED KYNAR COLOR

GENERAL NOTES:

(1) IF CABLE BRACING FOR END WALL IS NOT SHOWN ON ERECTION DRAWINGS IT HAS BEEN DETERMINED THAT DIAPHRAGM PANEL ACTION IS SUFFICIENT TO RESIST LONGITUDINAL FORCES. TEMPORARY BRACING SHOULD BE PROVIDED BY ERECTOR UNTIL ALL WALL AND ROOF PANELS ARE INSTALLED.

REVISIONS						
REV.	DESCRIPTION	DATE	DTLR	DATE	CHKR	APPR

DRAWING STATUS
☐ FOR CONSTRUCTION
☒ FOR PERMIT ONLY
☐ FOR APPROVAL
☐ OTHER, EXPLAIN

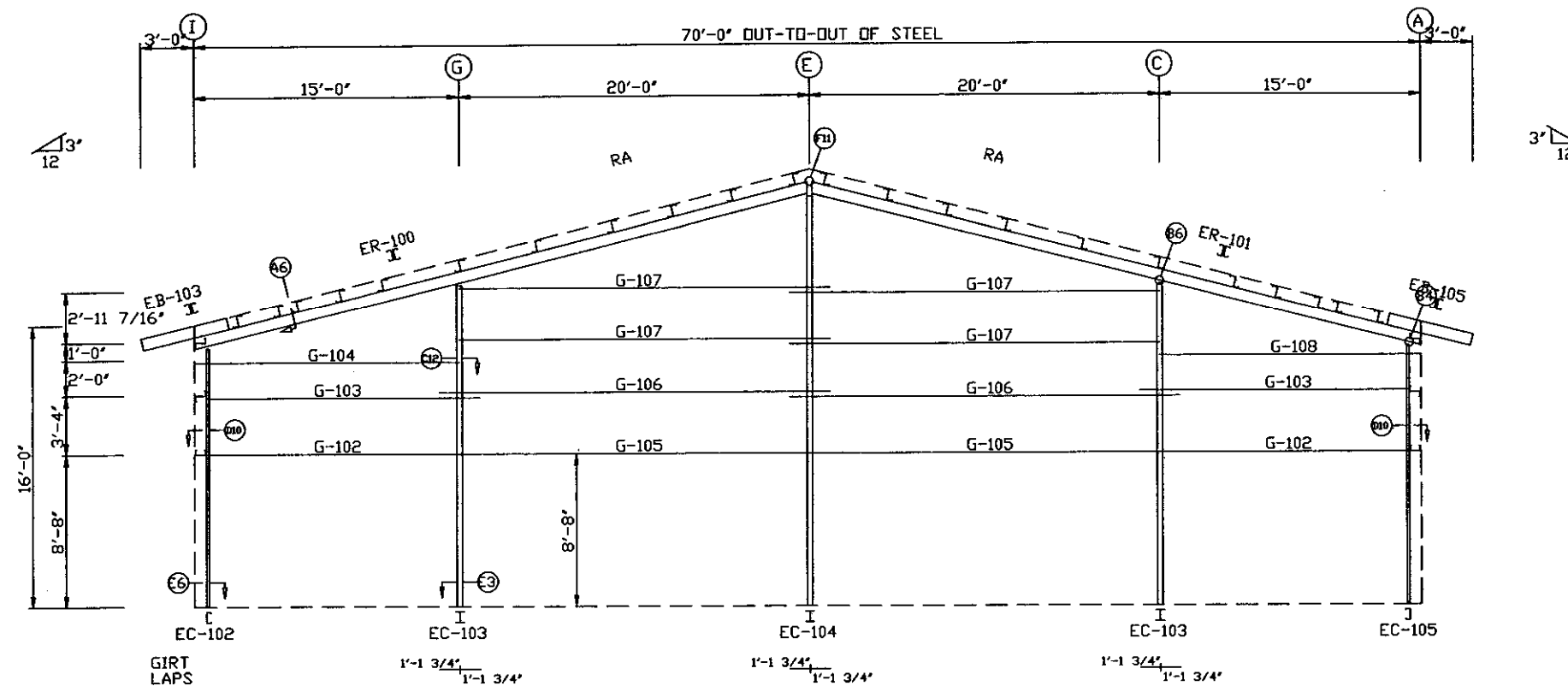
LIFETIME STEEL BLDGS	
PROJECT	70.00' x 50.6 x 16.0
ID	19066-2
PROJECT ADDRESS	LAKE CITY, FL 32024

LIFETIME STEEL BLDGS	
ENDWALL FRAMING	
DESIGN	DRAFT/ARM/MMD CHECK/DB
DATE: 9/29/08	SHEET 8

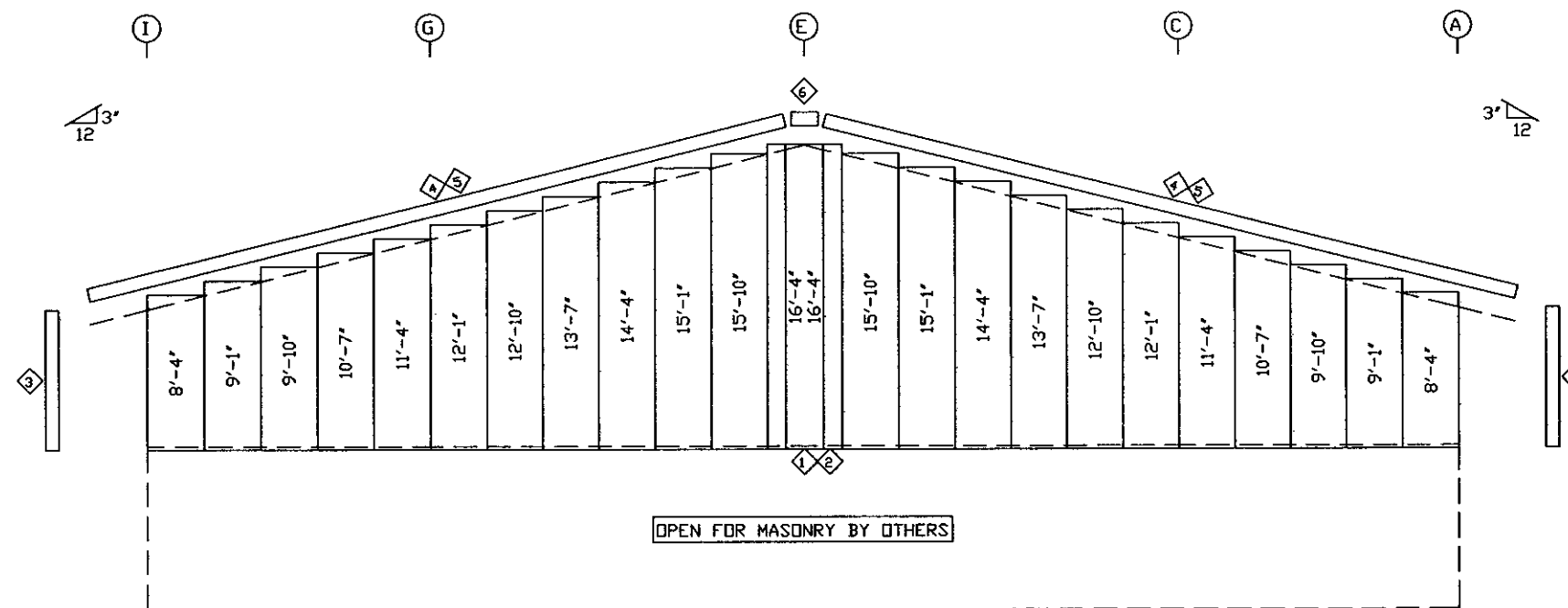
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ENDWALL FRAMING: FRAME LINE 8



ENDWALL SHEETING & TRIM: FRAME LINE 8
PANELS: 26 Ga. PBR - NEED KYNAR COLOR

BOLT TABLE				
FRAME LINE 8				
LOCATION	QUAN	TYPE	DIA	LENGTH
ER-100/ER-101	8	A325	5/8"	1 3/4"
Cor. Column/Raf	4	A325	1/2"	1 1/4"
Int. Column/Raf	4	A325	1/2"	1 1/4"
Int. Column/Raf	4	A325	5/8"	1 1/4"

TRIM TABLE		
FRAME LINE 8		
ITEM	MARK	LENGTH
1	TBD	3'-6"
2	TBD	1'-9"
3	CTR	7'-6"
4	VRT	20'-3"
5	VRT	19'-6"
6	VPB	1'-4"
		DETAIL
		TRIM_29
		TRIM_11
		TRIM_11
		TRIM_39

MEMBER TABLE		
FRAME LINE 8		
MARK	PART	LENGTH
EB-103	8x50D16	4'-3 1/8"
EB-105	8x50D16	4'-3 1/8"
EC-102	8x35C16	14'-6 7/16"
EC-103	W8X18	18'-1 7/16"
EC-104	W8X31	22'-10 3/16"
EC-105	8x35C16	14'-6 7/16"
ER-100	10x50d13	36'-0 15/16"
ER-101	10x50d13	36'-0 15/16"
G-102	8X25C16	14'-3 1/2"
G-103	8X25Z16	15'-5 1/2"
G-104	8X25Z16	15'-3 1/2"
G-105	8X25C16	19'-11 1/2"
G-106	8X25Z16	22'-3 1/2"
G-107	8X25Z16	21'-5 1/2"
G-108	8X25Z16	15'-3 1/2"

GENERAL NOTES:

(1.) IF CABLE BRACING FOR END WALL IS NOT SHOWN ON ERECTION DRAWINGS IT HAS BEEN DETERMINED THAT DIAPHRAGM PANEL ACTION IS SUFFICIENT TO RESIST LONGITUDINAL FORCES. TEMPORARY BRACING SHOULD BE PROVIDED BY ERECTOR UNTIL ALL WALL AND ROOF PANELS ARE INSTALLED.

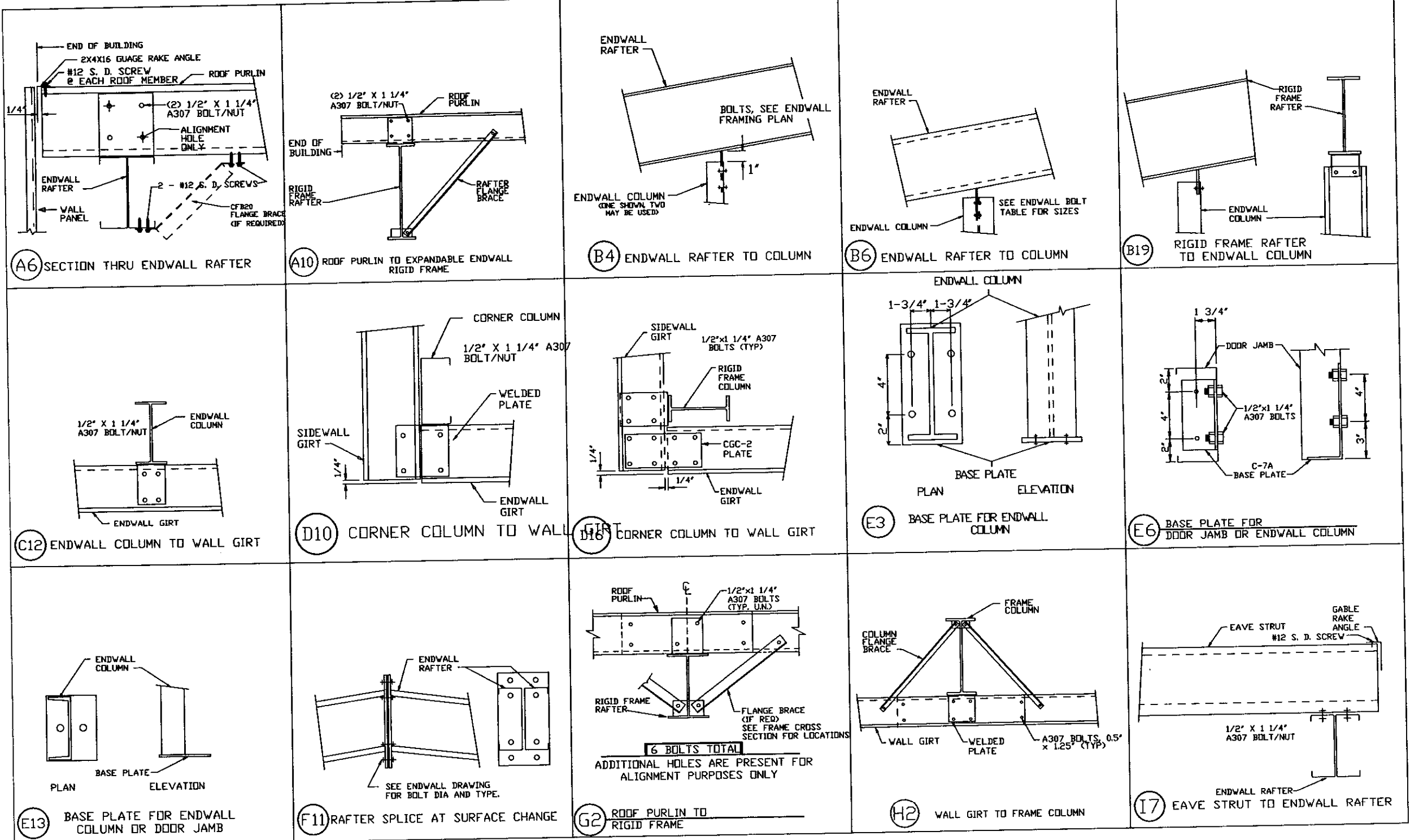
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REV.	DESCRIPTION	DATE	DTLR	DATE	CHKR	APPR

DRAWING STATUS	
[]	FOR CONSTRUCTION
[X]	FOR PERMIT ONLY
[]	FOR APPROVAL
[]	OTHER, EXPLAIN

LIFETIME STEEL BLDGS	
PROJECT	70.00' x 50.6' x 16.0'
ID	19066-2
PROJECT ADDRESS	LAKE CITY, FL 32024

LIFETIME STEEL BLDGS	
ENDWALL FRAMING	
DESIGN: DRAFT/ARM/MMD	CHECK: DB
DATE: 9/29/08	SHEET 9

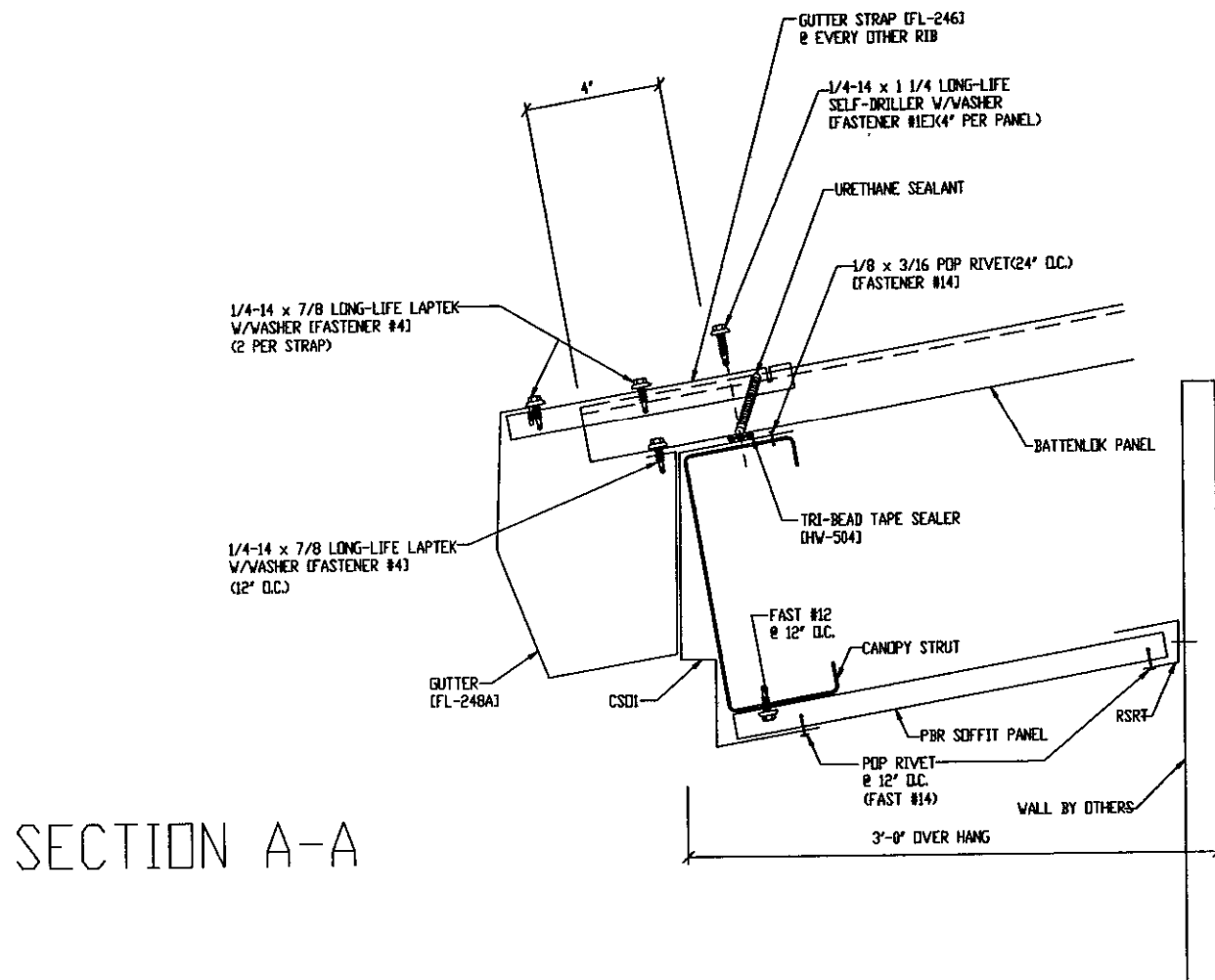
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OCT 09 2008



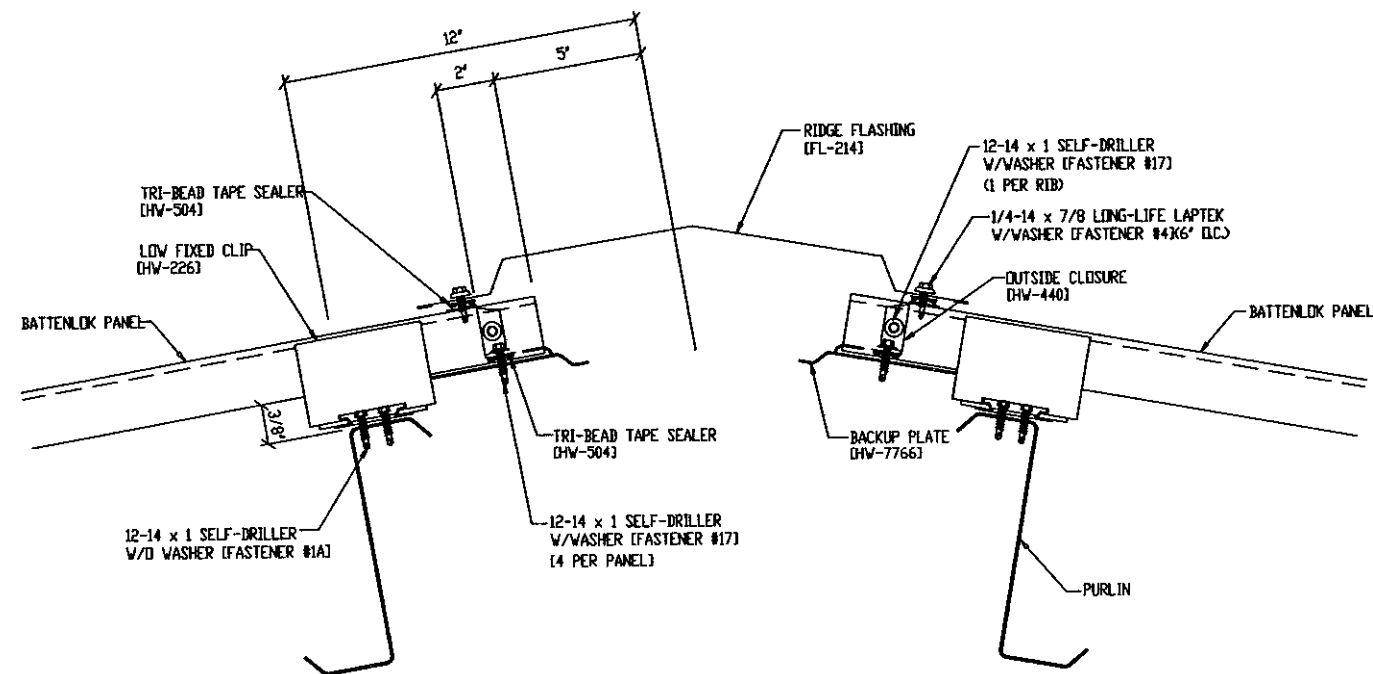
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OCT 09 2008

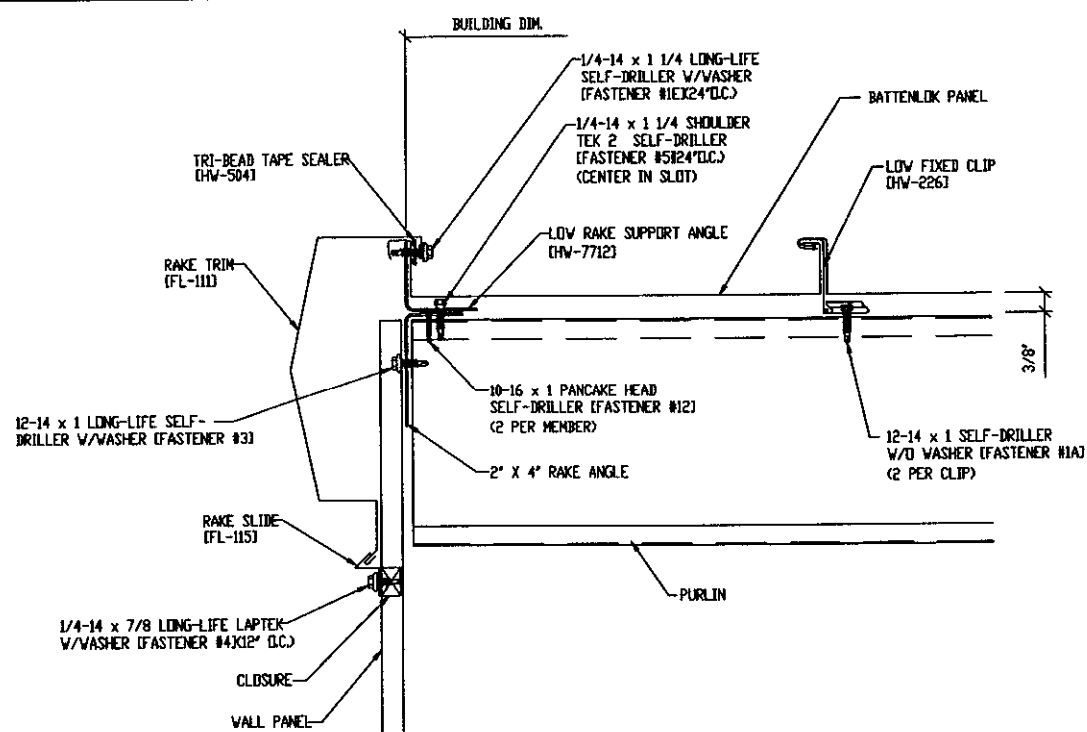
REVISIONS						DRAWING STATUS		LIFETIME STEEL BLDGS		LIFETIME STEEL BLDGS	
REV.	DESCRIPTION	DATE	DLR	DATE	CHKR	APPD		PROJECT	70.00' x 50.6' x 16.0	Detail Page	
							[] FOR CONSTRUCTION	ID	19066-2	DESIGN	DRAFT/ARM/MMD CHECK/DB
							[] FOR PERMIT ONLY	PROJECT	LAKE CITY, FL 32024	DATE: 9/29/08	SHEET 10
							[] FOR APPROVAL	ADDRESS			
							[] OTHER, EXPLAIN				



SECTION A-A



SECTION C-C



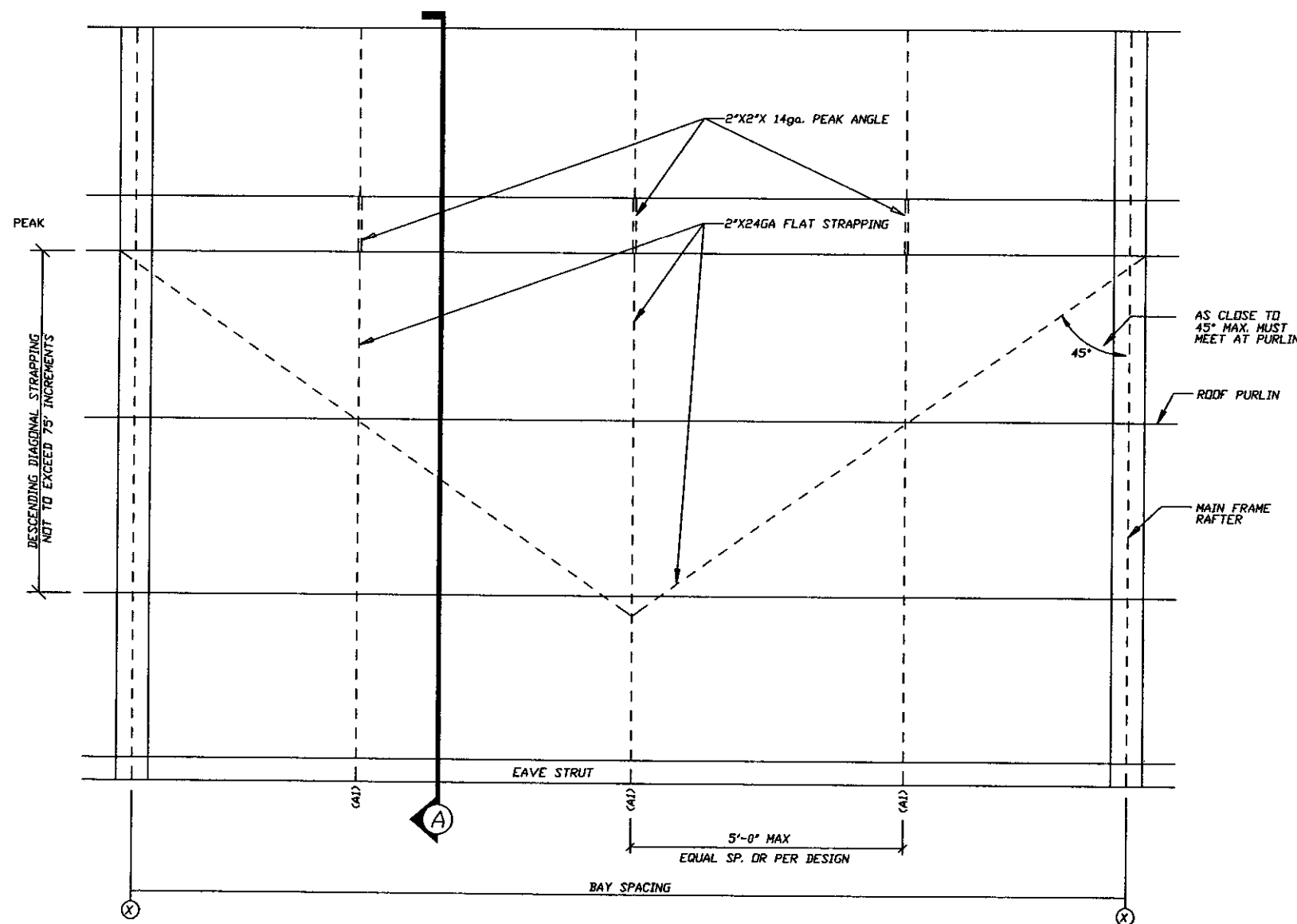
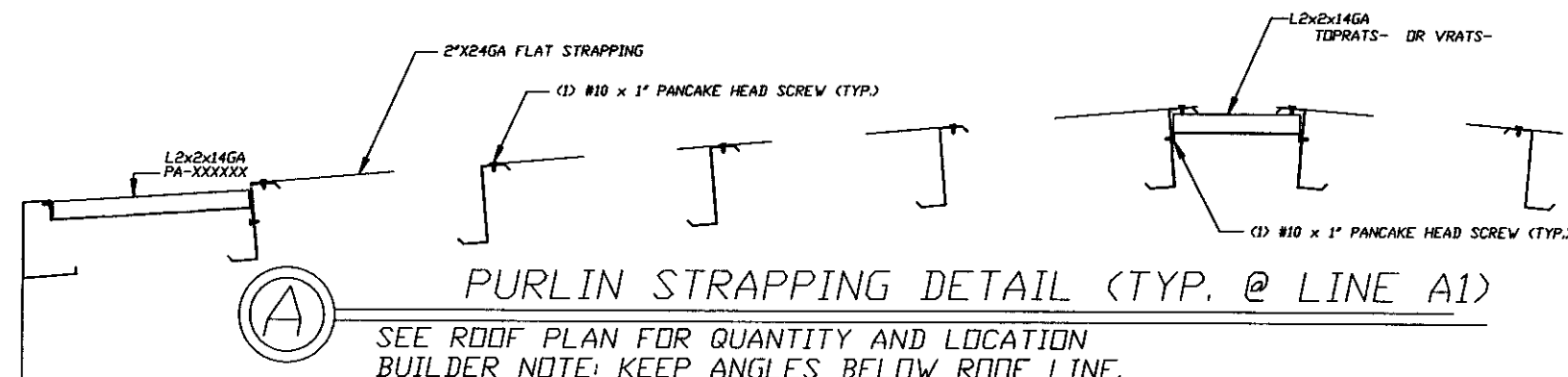
SECTION B-B

SEAL

[Signature]

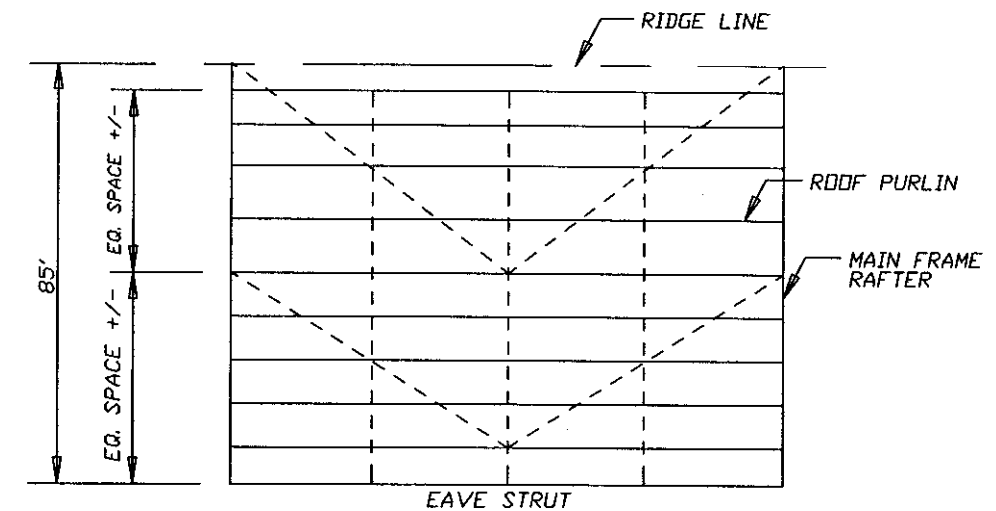
OCT 09 2008

REVISIONS						DRAWING STATUS		LIFETIME STEEL BLDGS		LIFETIME STEEL BLDGS	
REV.	DESCRIPTION	DATE	DLR	DATE	CHKR	APPD		PROJECT	70.00' x 50.6 x 16.0	Detail Page	
							☐ FOR CONSTRUCTION	ID	19066-2	DESIGN: DRAFT:ARM/MMD	CHECK: DB
							☐ FOR PERMIT ONLY	PROJECT	LAKE CITY, FL 32024	DATE: 9/29/08	SHEET 12
							☐ FOR APPROVAL	ADDRESS			
							☐ OTHER, EXPLAIN				

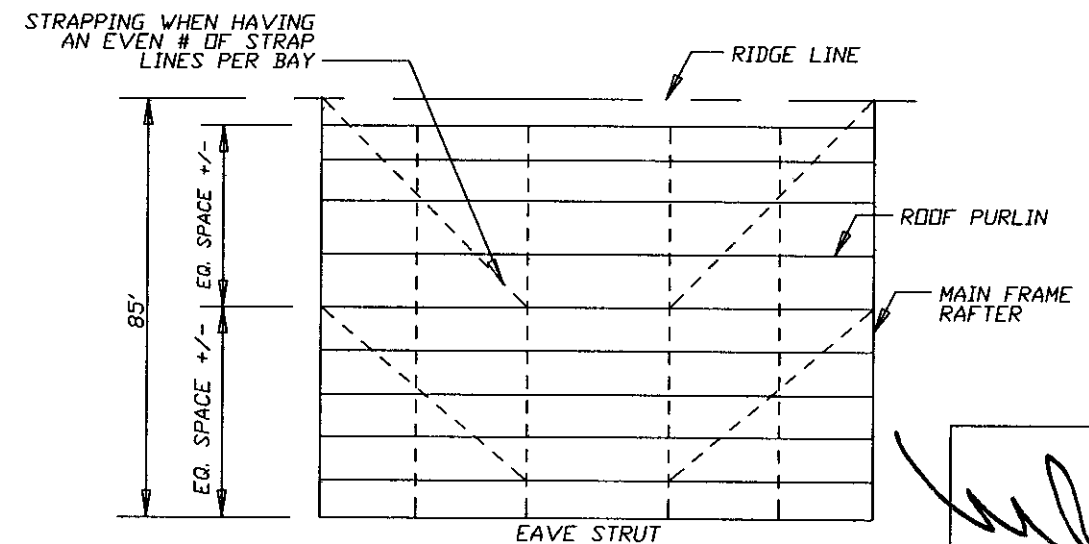


STANDING SEAM ROOFS (TOP FLANGE ONLY)
 $> 2 \frac{1}{2} : 12$ SLOPE

USE FLAT STRAP WITH ANGLE AT 1st EAVE SPACE AND RIDGE ONLY
 DESCENDING DIAGONALS AT PEAK (45° MAX. FROM PLANE OF FRAME)
 DESCENDING DIAGONALS NOT TO EXCEED 75' INCREMENTS (NOMINAL EQUAL SPACES)
 MAX 5'-0" O.C. WITH EQUAL SPACES OR SPACES PER DESIGN



EXAMPLE #1: ODD # OF STRAPPING PER BAY
 EQUAL SPACES +/-
 NOT TO EXCEED 75'-0"



EXAMPLE #2: EVEN # OF STRAPPING PER BAY
 EQUAL SPACES +/-
 NOT TO EXCEED 75'-0"

SEAL

OCT 09 2008

REVISIONS							DRAWING STATUS		LIFETIME STEEL BLDGS		LIFETIME STEEL BLDGS	
REV.	DESCRIPTION	DATE	DLR	DATE	CHKR	APPR	[] FOR CONSTRUCTION		PROJECT	70.00' x 50.6' x 16.0	Detail Page	
							[X] FOR PERMIT ONLY		ID	19066-2	DESIGN: DRAFT/ARM/MMD CHECK: DB	
							[] FOR APPROVAL		PROJECT	LAKE CITY, FL 32024	DATE: 9/29/08 SHEET 13	
							[] OTHER, EXPLAIN		ADDRESS			

ENGINEERING CALCULATIONS

LIFETIME STEEL BLDGS

Job Number 19066-2

End User LIFETIME STEEL BLDGS

Job Location LAKE CITY, FL 32024

DATE: 9/29/08

DESIGNED BY: BLS

CHECKED BY: DB

DRAWN BY: ARM/MMD

DESIGN PARAMETERS

COMMENTS

BUILDING DESCRIPTION

NOMINAL WIDTH: 70 feet

NOMINAL LENGTH: 50.67 feet

EAVE HEIGHT, BACK S.W: 16 feet

EAVE HEIGHT, FRONT S.W: 16 feet

ROOF SLOPE, LEFT: 3.0:12

ROOF SLOPE, RIGHT: 3.0:12

DESIGN LOADS

BUILDING CODE: FBC 06

FRAME SELF WEIGHT: INCLUDED

ROOF DEAD LOAD: 2.000 psf

COLLATERAL LOAD: 1 psf

LIVE LOAD: 20.00 psf

SNOW LOAD, ROOF: NR

WIND SPEED: (3 SEC GUST) 140 mph

WIND EXPOSURE: B

CLOSURE "C, D, P": Closed

IMP. FACTOR, WIND: 1.15

OCCUPANCY CATEGORY: IV - ESSENTIAL FACILITIES

SEISMIC PARAMETERS:

SEISMIC IMPORTANCE: 1.50

SEISMIC RESPONSE COEF.: 0.057

SEISMIC DESIGN CATEGORY: D

DESIGNED SPECTRAL ACCELERATION PARAMETER "SDS" - (SHORT PERIODS): 0.1333

DESIGNED SPECTRAL ACCELERATION PARAMETER "SD1" - (1 SEC PERIODS): 0.1008

MAPPED SPEC. RESPONSE ACC. "SS" - (SHORT PERIODS): 0.1250

MAPPED SPEC. RESPONSE ACC. "S1" - (1 SEC PERIOD): 0.0630

RESPONSE MODIFICATION FACTOR: 3.5

SITE CLASS (ASSUMED): D

BASIC SEISMIC-FORCE RESISTING SYSTEM MOMENT RESISTING FRAME

DESIGN BASE SHEAR: $V=C_s \times W$

ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE

DRAWING RELEASE HISTORY

DRAWING REVISIONS

DATE SENT

REASON FOR RELEASE

REVISION

DATE

BY

DESCRIPTION

A

B

C

D

E

F

G

H

I

J

K

L

M

GENERAL NOTES

1. MATERIALS

STRUCTURAL STEEL PLATE
COLD FORMED LIGHT GAGE SHAPES
BRACE RODS
HOT ROLLED MILL SHAPES
ROOF AND WALL SHEETS
BOLTS
BUILT-UP MEMBERS

ASTM DESIGNATION
A529 OR A572 OR A570 OR A607
A570 OR A607
A572
A36 OR A572
A653 OR A792
A307, A325, AND A490
A570

GRADE 50
GRADE 55
GRADE 65, UNLESS NOTED
FY= 36 KSI OR GRADE 50
GRADE 50
A307 UNLESS NOTED
GRADE 50 OR 55

2. STRUCTURAL PRIMER

SHOP PRIMER PAINT IS A RUST INHIBITIVE PRIMER AND IT'S COLOR IS RED OXIDE.
THIS PAINT IS NOT INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS.

3. A325 BOLT TIGHTENING REQUIREMENTS

ALL HIGH STRENGTH BOLTS ARE A325-N UNLESS SPECIFICALLY NOTED OTHERWISE.

STRUCTURAL BOLTS SHALL BE TIGHTENED BY THE TURN-OF-THE-NUT METHOD IN ACCORDANCE WITH THE 13th EDITION AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS". PER SECTION 8.2, A325 BOLTS MAY BE INSTALLED WITHOUT WASHERS WHEN TIGHTENED BY THE TURN-OF-THE NUT METHOD.

ALL HIGH STRENGTH BOLTS, EXCEPT AS NOTED OTHERWISE, ARE SUBJECT TO DIRECT TENSION AND MAY REQUIRE INSPECTION AS DEFINED BY THE APPLICABLE BUILDING CODE OR STANDARD. IT IS THE RESPONSIBILITY OF THE ERECTOR TO ASSURE PROPER TIGHTNESS.

4. BUILDER/CONTRACTOR RESPONSIBILITIES

THE METAL BUILDING MANUF.'S STANDARD PRODUCT SPECIFICATIONS APPLY AND UNLESS STIPULATED OTHERWISE IN THE CONTRACT DOCUMENTS, THE METAL BUILDING MANUF.'S DESIGN, FABRICATION, QUALITY CRITERIA STANDARDS AND TOLERANCES WILL GOVERN THE WORK.

IN CASE OF DISCREPANCIES BETWEEN METAL BUILDINGS MANUF. STRUCTURAL PLANS AND PLANS FOR OTHER TRADES, THE METAL BUILDING MANUF.'S PLANS SHALL GOVERN.

IT IS THE RESPONSIBILITY OF THE BUILDER / CONTRACTOR TO OBTAIN APPROPRIATE APPROVALS AND NECESSARY PERMITS FROM CITY, COUNTY, STATE, OR FEDERAL AGENCIES, AS REQUIRED.

APPROVAL OF METAL BUILDING MANUF.'S DRAWINGS CONSTITUTES THE BUILDER / CONTRACTOR'S ACCEPTANCE OF THE METAL BUILDING MANUF.'S INTERPRETATION OF THE CONTRACT PURCHASE ORDER.

ONCE THE BUILDER / CONTRACTOR OR A/E FIRM HAS SIGNED MANUF.'S APPROVAL PACKAGE, CHANGES FROM THE PURCHASE ORDER BY THE BUILDER WILL BE BILLED TO THE BUILDER / CONTRACTOR FOR MATERIAL, ENGINEERING AND HANDLING FEES. SUCH CHANGES MAY CAUSE THE PROJECT TO BE MOVED FROM THE FABRICATION AND / OR SHIPPING SCHEDULE. A PENALTY FEE MAY BE CHARGED IF THE PROJECT MUST BE MOVED FROM THE FABRICATION AND / OR SHIPPING SCHEDULE, AS LONG AS THE MANUF.'S DESIGN AND DETAILING APPROACH COMPLIES WITH THE PURCHASE ORDER.

THE BUILDER / CONTRACTOR OR A/E FIRM ARE RESPONSIBLE FOR THE OVERALL PROJECT CONDITION. ALL INTERFACE AND COMPATIBILITY CONCERNING ANY MATERIALS NOT FURNISHED BY THE MANUF. ARE TO BE CONSIDERED AND COORDINATED BY THE BUILDER / CONTRACTOR OR A/E FIRM, UNLESS SPECIFIC DESIGN CRITERIA CONCERNING THIS INTERFACE BETWEEN MATERIALS IS FURNISHED AS PART OF THE PURCHASE ORDER. THE METAL BUILDING MANUF.'S ASSUMPTIONS WILL GOVERN.

THE BUILDER / CONTRACTOR IS RESPONSIBLE TO INSURE THAT ALL OTHER PROJECT PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITY. SUPPLYING SEALED ENGINEERING DESIGN DATA AND DRAWINGS BY THE BUILDING MANUF. DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT THE BUILDING MANUF. OR ITS DESIGN ENGINEER IS ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR THE CONSTRUCTION PROJECT. THESE DRAWINGS AND DESIGN DATA ARE SEALED AS TO THE STRUCTURAL SYSTEM FURNISHED BY THE METAL BUILDING MANUF. IN COMPLIANCE WITH ALL REQUIREMENTS OF THE PURCHASE ORDER.

THE BUILDER / CONTRACTOR IS RESPONSIBLE FOR SETTING OF ANCHOR BOLTS AND ERECTION OF STEEL BUILDING COMPONENTS IN ACCORDANCE WITH THE METAL BUILDING MANUF.'S "FOR CONSTRUCTION" DRAWINGS. TEMPORARY SUPPORTS OR BRACING REQUIRED FOR THE BUILDING ERECTION WILL BE THE RESPONSIBILITY OF THE ERECTOR TO DETERMINE, FURNISH, AND INSTALL.

THE METAL BUILDING MANUF. DOES NOT WARRANT STRUCTURAL INTEGRITY OF ANY COMPONENTS FIELD MODIFIED OR DESIGNED AND FABRICATED BY OTHERS. NEITHER DO WE ACCEPT DESIGN RESPONSIBILITY FOR THE EFFECTS NON STANDARD COMPONENTS DESIGNED BY OTHERS MAY HAVE ON THE SYSTEM IN GENERAL.

SPECIAL NOTE

BUILDING IS NOT STRUCTURALLY SOUND UNTIL ALL WALL COVERING, ROOF SHEETS, AND PERMANENT BRACING IS INSTALLED. BUILDER / CONTRACTOR IS RESPONSIBLE FOR SUPPORTS OR TEMPORARY BRACING DURING ERECTION. HE SHALL FURNISH, AND INSTALL THESE TEMPORARY SUPPORTS WHERE NECESSARY. TEMPORARY SUPPORTS ARE NOT PROVIDED BY THE METAL BUILDING MANUFACTURER.

FLORIDA PRODUCT APPROVAL INFORMATION

APPROVAL #

22 GA. 22 BATTENLOK PANEL - ROOF PANEL FL 7353.1

26 GA. 26 PBR - WALL PANEL FL 5335.1

9' X 10' RIDGE VENT FL 8069.1

BLDG #1

BLDG #2

SHEETING

ROOF PANEL:

GAUGE

PANEL TYPE

COLOR

22 GA. 22 BL-HS NEED KYNAR COLOR

WALL PANEL:

GAUGE

PANEL TYPE

COLOR

26 GA. 26 PBR NEED KYNAR COLOR

SOFFIT PANEL:

GAUGE

PANEL TYPE

COLOR

26 GA. 26 PBR NEED KYNAR COLOR

WALL TRIM COLOR:

NEED KYNAR COLOR

GABLE TRIM COLOR:

NEED KYNAR COLOR

ENGINEER'S STAMP

OCT 09 2008

COLUMBIA COUNTY BUILDING DEPARTMENT

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